



Drainage Report City/County File No. PPR2413 Dutch Bros Coffee (CO0907)

PREPARED BY

Barghausen Consulting Engineers, Inc.

PREPARED FOR

Dutch Bros Coffee

CLIENT ADDRESS

110 S.W. 4th Street, Grants Pass, OR 97526

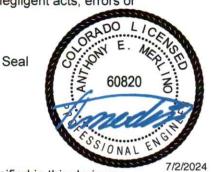
SITE ADDRESS	PROJECT NO.	DATE	JURISDICTION	
5810 Omaha Boulevard, Colorado Springs, Colorado 80915	23098	July 2, 2024	County of El Paso	

DESIGN ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Anthony E. Merlino, PE#60820

July 2, 2024



OWNER/DEVELOPER'S STATEMENT:

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

Tom Souza Eastbound & Down, LLC c/o Sansome Pacific 221 Pine Street, 4th Floor San Francisco, CA 94104

24

EI PASO COUNTY:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volume 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

Joshua Palmer, PE County Engineer/ECM Administrator

Conditions:

9/17/2024

Date

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INTRODUCTION

PURPOSE OF STUDY

This document is the drainage report for 5810 Omaha Blvd, Colorado Springs, CO 80915. The proposed development includes a building footprint of 950 square feet and a 272 square-foot trash enclosure. The planned site improvements include approximately paved asphalt driving area, reinforced concrete driving area, on-site sidewalk area, and landscaping. This report is to summarize the drainage improvements for the Dutch Bros Coffee and is intended to demonstrate that the drainage improvements for the proposed project is in conformance with El Paso County and will not negatively impact downstream drainage facilities, surrounding development, and receiving water course.

LOCATION

This project is located on the northwest corner of Powers Blvd and Omaha Blvd, Colorado Springs, Colorado and is currently an existing gas station. The parcel is bounded by Powers Blvd to the west, existing commercial development on the east and north, and Omaha Blvd to the south. The site is within the El Paso County jurisdiction. Refer to Appendix A for a vicinity map.

DESCRIPTION OF SITE

The property is zoned as Commercial Regional. The parcel is approximately 0.62 ac. The disturbed area consists of approximately 0.62. The existing site is currently an existing gas station. The site has existing drainage system with existing drainage inlets near the south side of the site. There is an existing storm drain system located near the south end of the site. Overall, the site slopes from the northeast to the southwest.

SOILS

Per the Natural Resources Conservation Service web soils survey, soils for this project, delineated on the soils map within Appendix B of this report, are classified as Blendon Sandy Loam. Blendon Sandy Loam has been classified as Hydrologic Soil Type "B". The study area consists of undeveloped land with sparse, grassy vegetation.

FLOODPLAIN STATEMENT

Subject property is located in Zone "X" (Area determined to be outside the 0.2% annual chance floodplain) per flood insurance rate map for County of El Paso, Colorado map number 08041C0751G, revised December 7, 2018.

PROJECT BACKGROUND

EXISTING DRAINAGE

In existing conditions, Basin A-1 is approximately 0.55 acres and approximately 89% impervious. The site generally sheet flows from the northeast to the southwest. The runoff currently sheet flows towards curb and gutter along the south side and is conveyed to and collected by dual inlets located near the parking area. The runoff is then conveyed via an existing 18-inch storm drain.

The composite runoff coefficient for the 10-year and 100-year is 0.83 and 0.88, respectively for Hydrologlic Soil Type B per Table 5-1 of the El Paso County Drainage Criteria Manual (EPCDM). A minimum time of concentration of 5 minutes was utilized to calculate the flow rate. The rainfall intensity is approximately 6 in/hr for the 10-year storm event and 9 in/hr for the 100-year storm event per Figure 5-1 of the EPCDM. The runoff is approximately 2.75 cfs for 10-year and 4.40 cfs for 100-year storm event. Refer to Appendix D for Table 5-1 and Figure 5-1.

Refer to Appendix B for Existing Conditions Drainage Map and Appendix C for Hydrology Calculations.

PROPOSED DRAINAGE

In proposed conditions, the Dutch Bros site proposes more landscaping than existing conditions, which reduces the amount of runoff than existing conditions. The site is approximately 70% impervious, which is less than existing conditions at 89% impervious. Therefore, it is anticipated that the discharge and impact to the existing storm infrastructure will be negligible and even less than the existing condition.

HYDROLOGIC AND HYDRAULIC DESIGN

BASINS

Basin A-1

Basin A-1 is approximately 0.47 acres and approximately 74% impervious. The runoff maintains similar drainage pattern as existing conditions and sheet flows northeast to the southwest. Runoff sheet flows towards curb and gutter and is conveyed towards curb cuts along the south curb and gutter. Runoff will then flow through the curb cuts and along graded swale and collected by a proposed drainage inlet that will convey the runoff via storm drain and connect to an existing 30-inch storm drain.

The composite runoff coefficient for the 10-year and 100-year is 0.73 and 0.79, respectively for Hydrologic Soil Type B per Table 5-1 of the EPCDM. A minimum time of concentration of 5 minutes was utilized to calculate the flow rate. The rain fall intensity is 6 in/hr for the 10-year storm event and 9 in/hr for the 100-yr storm event per Figure 5-1 of the EPCDM. The run off is approximately 2.06 cfs for the 10-yr and 3.36 cfs for the 100-yr event.

Basin A-2

Basin A-2 is approximately 0.10 acres and approximately 53% impervious. The runoff maintains similar drainage pattern as existing conditions and sheet flows towards the south. Runoff sheet flows towards curb and gutter and is conveyed towards a curb cut along near the east property line. Runoff will then flow through the curb cut and is conveyed along graded swale and collected by the existing dual drainage inlets. The runoff is then conveyed via an existing 18-inch storm drain.

The composite runoff coefficient for the 10-year and 100-year is 0.59 and 0.67, respectively for Hydrologic Soil Type B per Table 5-1 of the EPCDM. A minimum time of concentration of 5 minutes was utilized to calculate the flow rate. The rain fall intensity is 6 in/hr for the 10-year storm event and 9 in/hr for the 100-yr storm event per Figure 5-1 of the EPCDM. The runoff is approximately 0.34 cfs for the 10-yr and 0.57 cfs for the 100-yr event.

The total area between Basin A-1 and A-2 is 0.57 acres. The total flow between Basin A-1 and A-2 is about 2.4 cfs for the 10-year and 3.93 cfs for the 100-year storm events. Refer to Appendix B for Proposed Conditions Drainage Map and Appendix C for Hydrology Calculations.

WATER QUALITY EXEMPTION

The proposed site is disturbing approximately 0.62 acres. Storm water quality is not required for any project disturbing activity less than 1 acre per the El Paso County Drainage Criteria Manual.

HYDRAULIC DESIGN

The proposed drainage inlet and storm drain were sized based on the 100-yr design storm using the Hydraflow Express. A Manning's n of 0.011 was used for the PVC storm drain and a 0.035 for a graded swale. Refer to Appendix C for Hydraulic Calculations.

MAINTENANCE

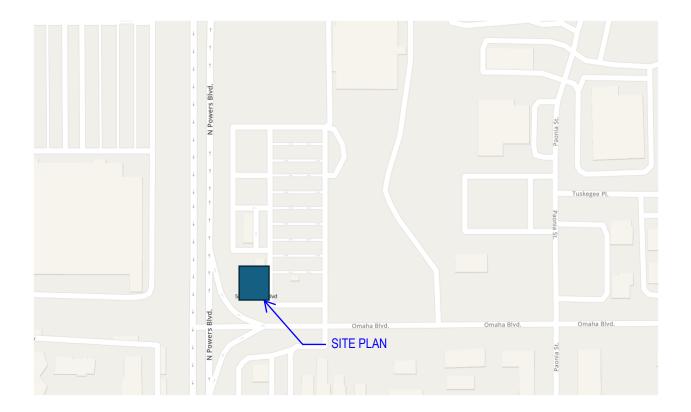
All drainage facilities within the public right-of-way shall be the responsibility of El Paso County.

SUMMARY

It has been concluded that the proposed project and the constructed improvements will maintain thresholds of existing conditions. The proposed project is less than an acre, therefore the site is exempt from water quality requirements. The site maintains existing drainage patterns to the maximum extent possible. The site also reduces the amount of existing impervious area from 89% impervious in existing conditions to 70% impervious in proposed conditions. The total runoff of 2.40 cfs for 10-year and 3.93 cfs for the 100-year storm event is less than existing flow rates of 2.75 cfs for 10-year and 4.40 cfs for 100-year storm event. Therefore, the proposed site is in conformance with El Paso County standards and requirements and does not negatively impact downstream facilities, surrounding areas, and receiving water courses.

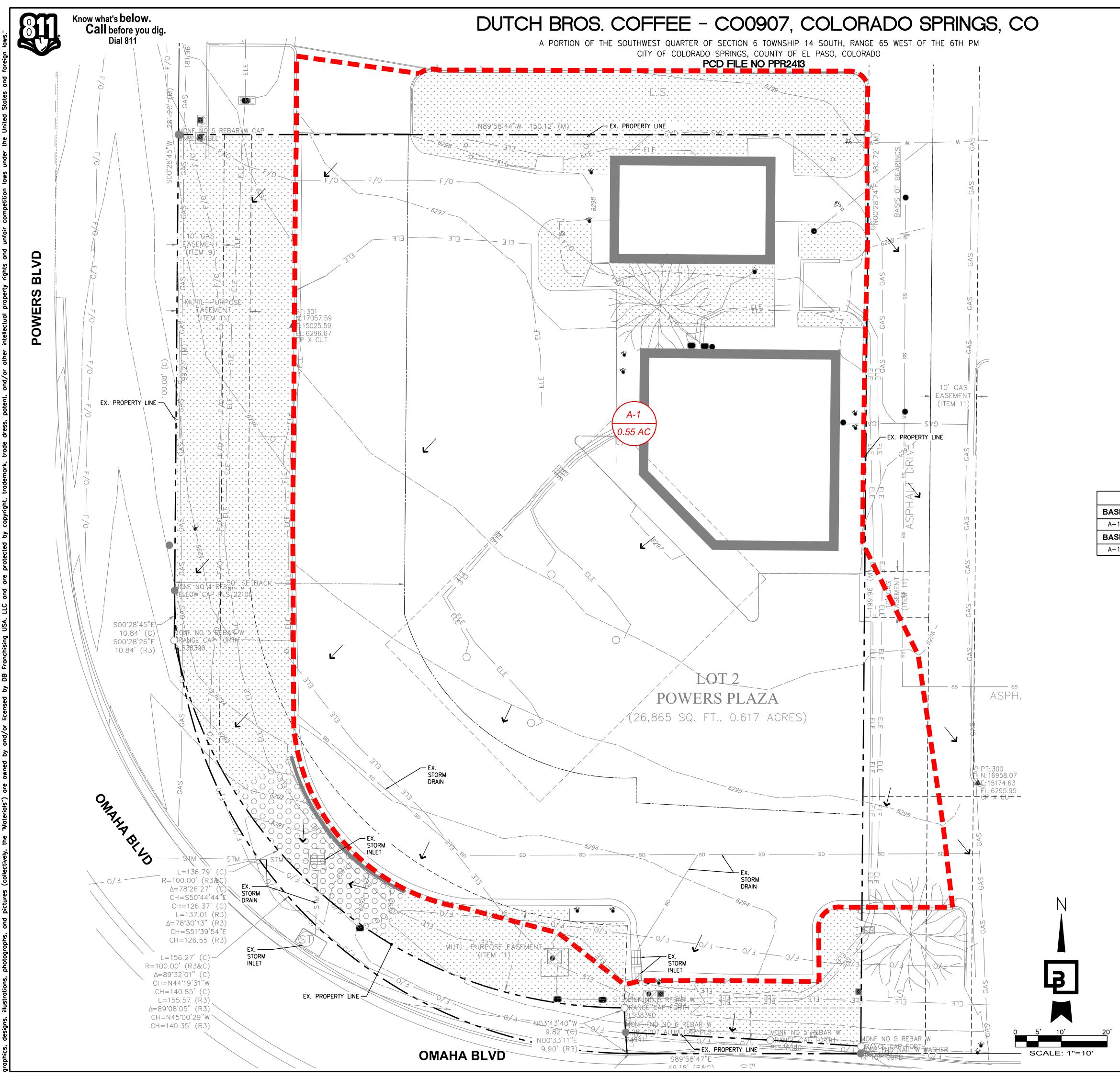
APPENDIX A

VICINITY MAP



APPENDIX B

EXISTING CONDITIONS DRAINAGE MAP
 PROPOSED CONDITIONS DRAINAGE MAP

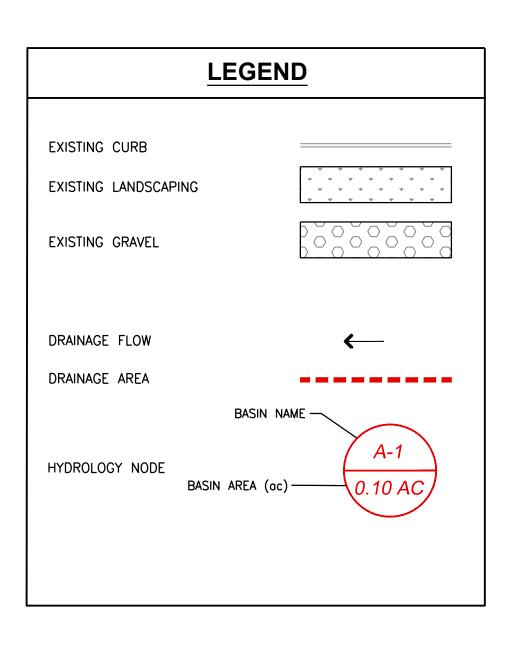


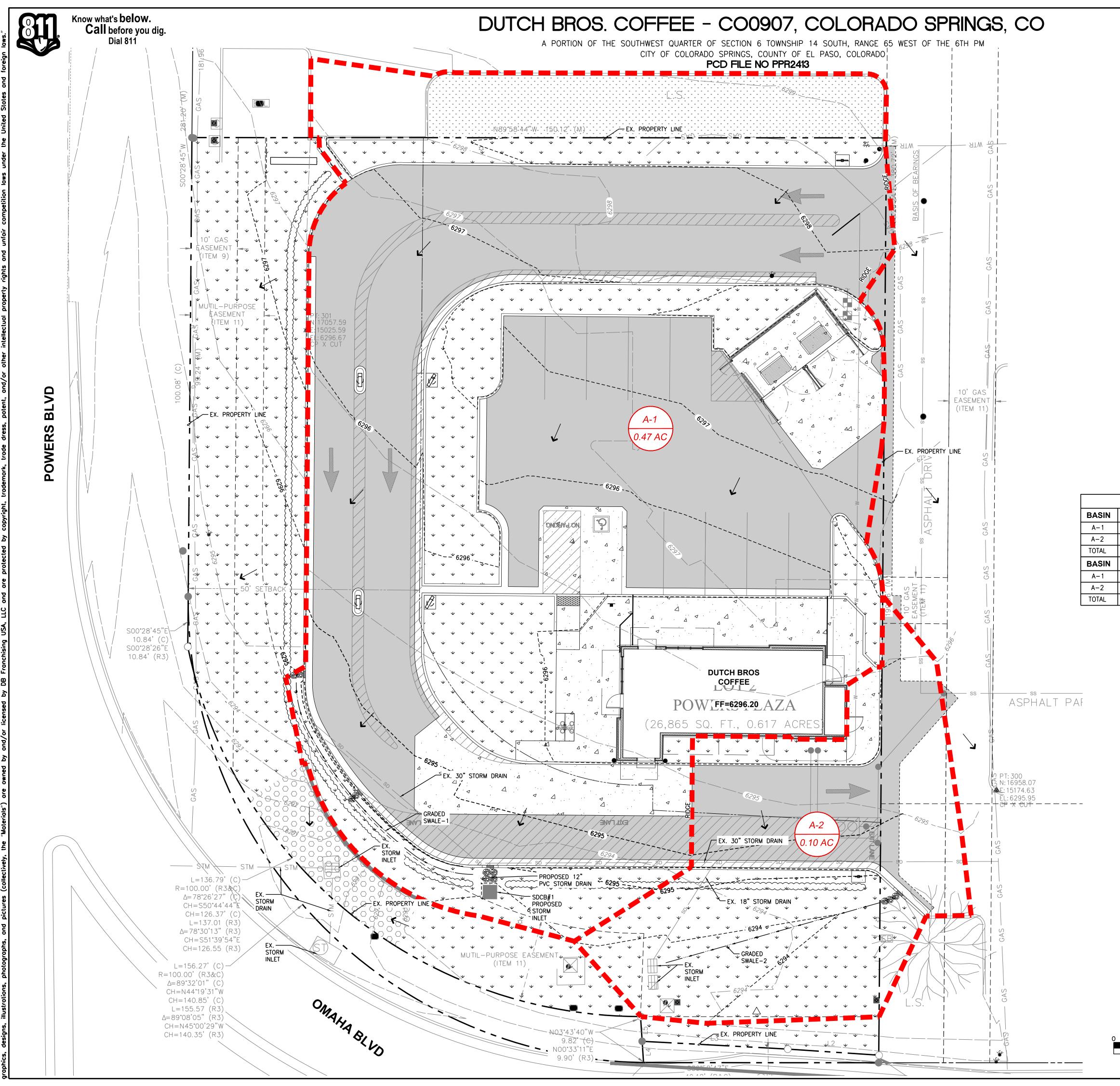
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Revision	PROPOSED DRAINAGE PLAN 5810 OMAHA BLVD COLORADO SPRINGS, CO 80915
Ckd. Appr.	PROPO 58 Colora
Date	itie
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	TO NAL ENGLAS
	N Scale: N/A N/A
	Designed <u>EMM</u> Drawn <u>EMM</u> Checked <u>AEM</u> Approved <u>AEM</u> Date <u>05/24/24</u>
	BarghausenBarghausenBarghausenBarghausen.comBarghausen.com
PPR2413	Job Number 23098 Sheet 1 OF 1 Franchising USA, LLC

DRAINAGE SUMMARY TABLE

EXISTING CONDITIONS					
SIN	AREA	RUNOFF (c)	TC (min)	i (10-YR) (in/hr)	PEAK Q ₁₀ (cfs)
- 1	24,160 SF (0.55 AC)	0.83	5	6.00	2.75
SIN	AREA	RUNOFF (c)	TC (min)	i (100-YR) (in/hr)	PEAK Q ₁₀₀ (cfs)
- 1	24,160 SF (0.55 AC)	0.88	5	9.00	4.40





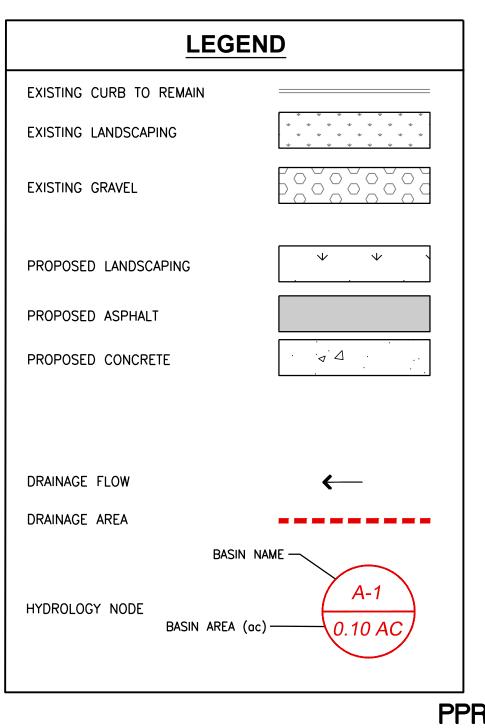
No. Dote By Ckd. Appr. Revision	Tile: PROPOSED DRAINAGE PLAN 5810 OMAHA BLVD COLORADO SPRINGS, CO 80915
	e lo,
	Designed EMMScale:DrawnEMMHorizontalDrawnEMMHorizontalCheckedAEM1" = 10'ApprovedAEMVerticalDate05/29/24N/A
	Barghausen Consulting Engineers, Inc. 18215 72nd Avenue South Kent, WA 98032 425.251.6222 barghausen.com
R2413	Job Number 23098 Sheet 1 OF 1 ^{2019 DB}

DRAINAGE SUMMARY TABLE

B

SCALE: 1"=10'

	PROPOSE	D CONDITIO	NS	
AREA	RUNOFF (c)	TC (min)	i (10-YR) (in/hr)	PEAK Q ₁₀ (cfs)
20,555 SF (0.47 AC)	0.73	5	6.00	2.06
4,170 SF (0.10 AC)	0.59	5	6.00	0.34
24,725 SF (0.57 AC)				2.40
AREA	RUNOFF (c)	TC (min)	i (100-YR) (in/hr)	PEAK Q ₁₀₀ (cfs)
20,555 SF (0.47 AC)	0.79	5	9.00	3.36
4,170 SF (0.10 AC)	0.67	5	9.00	0.57
24,725 SF (0.57 AC)				3.93



APPENDIX C

- HYDROLOGY CALCULATIONS
- HYDRAULIC CALCULATIONS

Project:Dutch Bros - CO0907Location:5810 Omaha Blvd, Colorado Springs, COBCE#23098

Rational Method

EXISTING			
Basin:	A-1		
Total Area (sf)	24,160		
Total Area (ac)	0.55		
Roof (sf)	2,605		
Impervious Area (sf)	18,780		
Pervious Area (sf)	2,775		
Total Area (sf)	24,160		
%Impervious (i)	0.89		
Runoff Coefficient, c			(per Table 5-1 Rund
Soil Type	В		(per NRCS Web Soil
	10 YR	100 YR	
Roof	0.9	0.95	
Paved, Drive & Walk	0.9	0.95	
Lawns	0.25	0.35	
	A-1		
C10 =	0.83		
C100 =	0.88		

per Table 5-1 Runoff Coefficient - Commercial) per NRCS Web Soil Survey)

(per Figure 5-1 - Colorado Springs Rainfall Intensity Duration Frequency)

Rainfall	Intensity,	i ((in/	hr))
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	5 (min)
i10 (in/hr) =	6.00
i100 (in/hr) =	9.00

Runoff, Q (cfs), assume min Tc = 5 min

	A-1
Q10 (cfs) =	2.75
Q100 (cfs) =	4.40

Q = C * i * A

Project:Dutch Bros - CO0907Location:5810 Omaha Blvd, Colorado Springs, COBCE#23098

Rational Method

PROPOSE	D		
Basin:	A-1	A-2	SITE
Total Area (sf)	20,555	4,170	24,725
Total Area (ac)	0.47	0.10	0.57
Roof (sf)	980	0	980
Impervious Area (sf)	14,150	2,200	16350
Pervious Area (sf)	5,425	1,970	7395
Total Area (sf)	20,555	4,170	24,725
%Impervious (i)	0.74	0.53	0.70

Runoff Coefficient, c		
Soil Type	В	
	10 YR	100 YR
Roof	0.9	0.95
Paved, Drive & Walk	0.9	0.95
Lawns	0.25	0.35
	A-1	A-2
C10 =	0.73	0.59
C100 =	0.79	0.67

(per Table 5-1 Runoff Coefficient - Commercial) (per NRCS Web Soil Survey)

(per Figure 5-1 - Colorado Springs Rainfall Intensity Duration Frequency)

i10 (in/hr) =
i100 (in/hr) =

Rainfall Intensity, i (in/hr)

Runoff, Q (cfs), assume min Tc = 5 min			Q = C * i * A
	A-1	A-2	
Q10 (cfs) =	2.06	0.34	
Q100 (cfs) =	3.36	0.57	

5 (min)

6.00 9.00

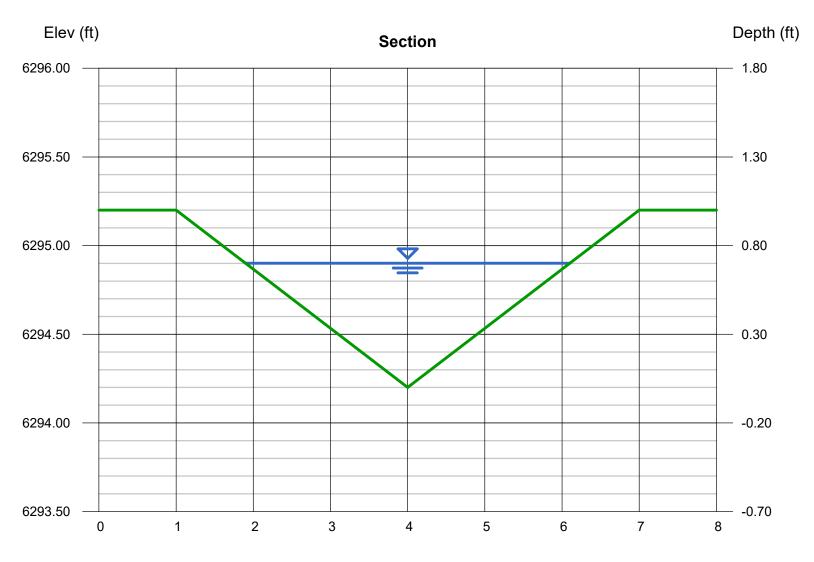
Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Thursday, May 23 2024

Graded Swale - 1

Triangular		Highlighted	
Side Slopes (z:1)	= 3.00, 3.00	Depth (ft)	= 0.70
Total Depth (ft)	= 1.00	Q (cfs)	= 3.360
		Area (sqft)	= 1.47
Invert Elev (ft)	= 6294.20	Velocity (ft/s)	= 2.29
Slope (%)	= 1.00	Wetted Perim (ft)	= 4.43
N-Value	= 0.030	Crit Depth, Yc (ft)	= 0.61
		Top Width (ft)	= 4.20
Calculations		EGL (ft)	= 0.78
Compute by:	Known Q		
Known Q (cfs)	= 3.36		



Reach (ft)

Channel Report

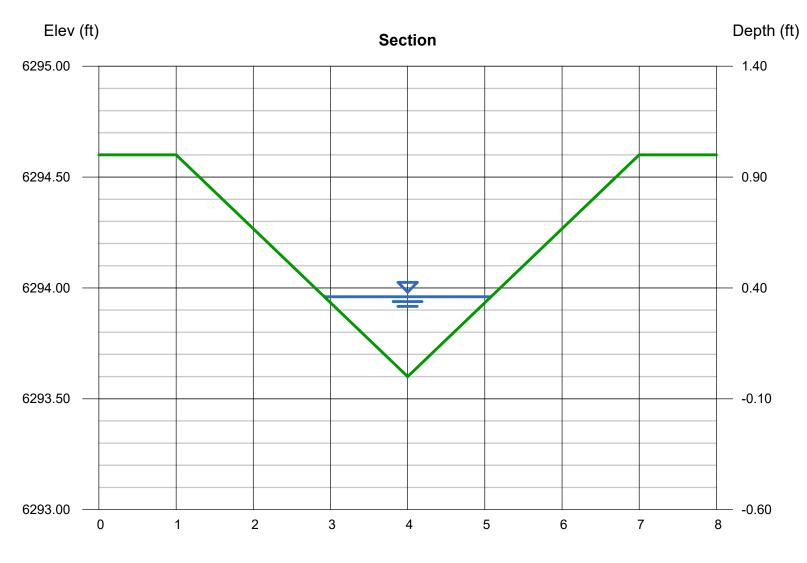
Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Thursday, May 23 2024

Graded Swale - 2

Tri	an	gu	lar
		J	

Triangular		Highlighted	
Side Slopes (z:1)	= 3.00, 3.00	Depth (ft)	= 0.36
Total Depth (ft)	= 1.00	Q (cfs)	= 0.570
		Area (sqft)	= 0.39
Invert Elev (ft)	= 6293.60	Velocity (ft/s)	= 1.47
Slope (%)	= 1.00	Wetted Perim (ft)	= 2.28
N-Value	= 0.030	Crit Depth, Yc (ft)	= 0.30
		Top Width (ft)	= 2.16
Calculations		EGL (ft)	= 0.39
Compute by:	Known Q		
Known Q (cfs)	= 0.57		



Reach (ft)

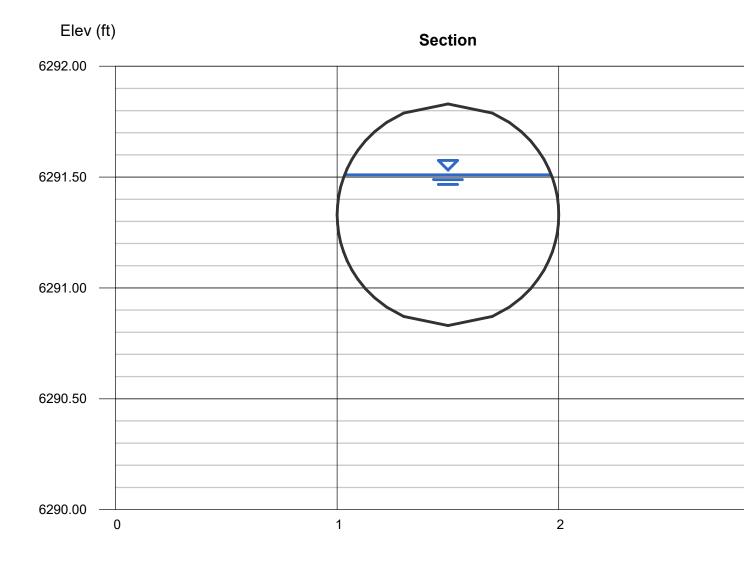
Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Thursday, May 23 2024

12-in PVC SD

Circular		Highlighted	
Diameter (ft)	= 1.00	Depth (ft)	= 0.68
		Q (cfs)	= 3.360
		Area (sqft)	= 0.57
Invert Elev (ft)	= 6290.83	Velocity (ft/s)	= 5.90
Slope (%)	= 1.00	Wetted Perim (ft)	= 1.94
N-Value	= 0.011	Crit Depth, Yc (ft)	= 0.79
		Top Width (ft)	= 0.93
Calculations		EGL (ft)	= 1.22
Compute by:	Known Q		
Known Q (cfs)	= 3.36		



Reach (ft)

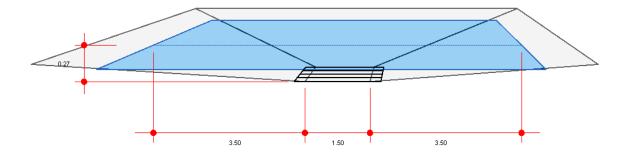
Inlet Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

SDCB#1

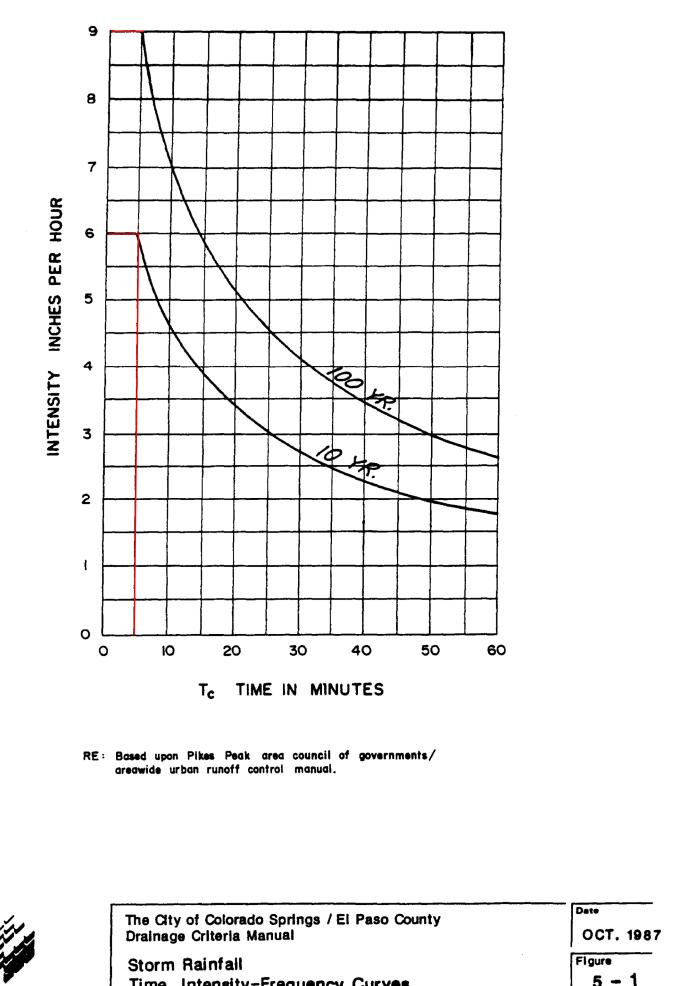
Drop Grate Inlet		Calculations	
Location	= Sag	Compute by:	Known Q
Curb Length (ft)	= -0-	Q (cfs)	= 3.36
Throat Height (in)	= -0-		
Grate Area (sqft)	= 4.00	Highlighted	
Grate Width (ft)	= 2.00	Q Total (cfs)	= 3.36
Grate Length (ft)	= 2.00	Q Capt (cfs)	= 3.36
		Q Bypass (cfs)	= -0-
Gutter		Depth at Inlet (in)	= 3.23
Slope, Sw (ft/ft)	= 0.083	Efficiency (%)	= 100
Slope, Sx (ft/ft)	= 0.083	Gutter Spread (ft)	= 8.49
Local Depr (in)	= -0-	Gutter Vel (ft/s)	= -0-
Gutter Width (ft)	= 1.50	Bypass Spread (ft)	= -0-
Gutter Slope (%)	= -0-	Bypass Depth (in)	= -0-
Gutter n-value	= -0-		

All dimensions in feet



APPENDIX D

REFERENCES



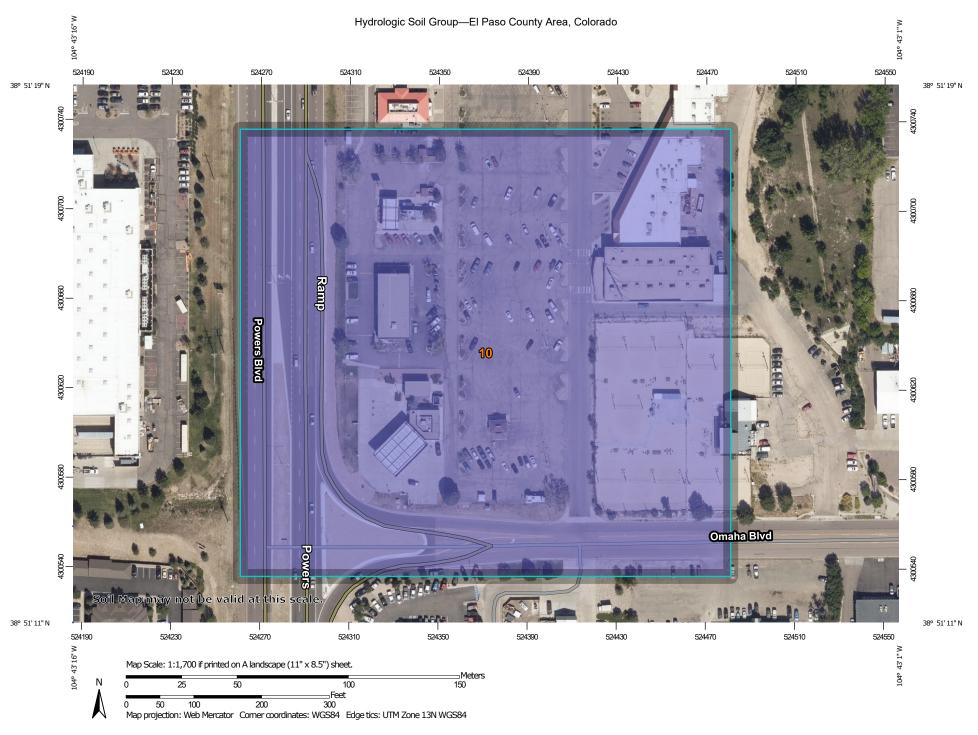
HDR Infrastructure, Inc A Centerra Company

Time Intensity-Frequency Curves

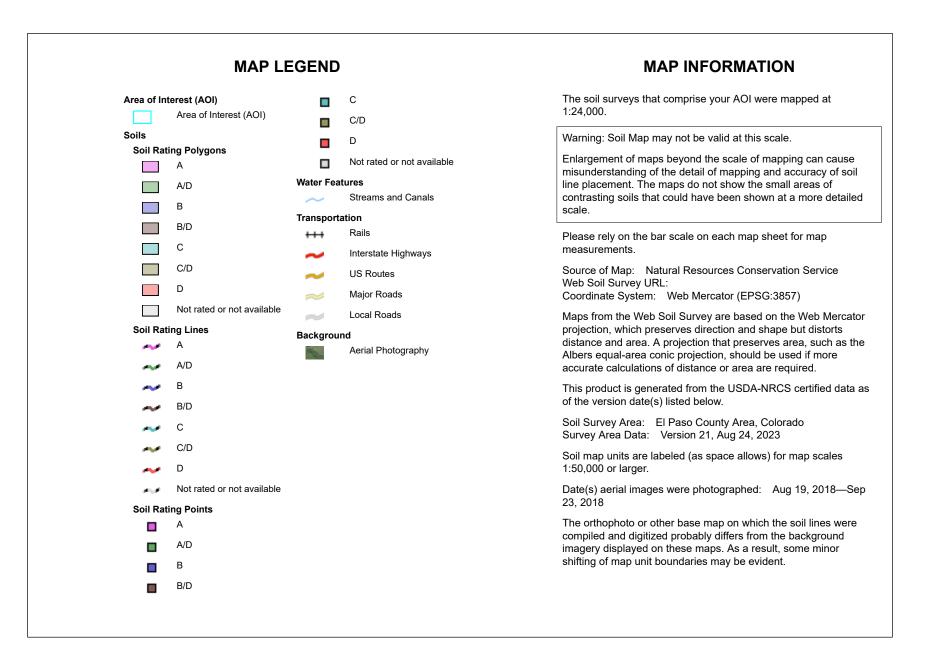
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					EXPAND	
LAND USE OR SURFACE CHARACTERISTICS	PERCENT	"C" FREQUENCY				
	IMPERVIOUS	10		100		
		A&B*	C&D*	A&B*	C&D*	
Business	Business					
Commercial Areas	95	0.90	0.90	0.90	0.90	
Neighborhood Areas	70	0.75	0.75	0.80	0.80	
Residential	·	·				
⅓ Acre or less	65	0.60	0.70	0.70	0.80	
1⁄4 Acre	40	0.50	0.60	0.60	0.70	
⅓ Acre	30	0.40	0.50	0.55	0.60	
1/2 Acre	25	0.35	0.45	0.45	0.55	
1 Acre	20	0.30	0.40	0.40	0.50	
Industrial						
Light Areas	80	0.70	0.70	0.80	0.80	
Heavy Areas	90	0.80	0.80	0.90	0.90	
Parks and Cemeteries	7	0.30	0.35	0.55	0.60	
Playgrounds	13	0.30	0.35	0.60	0.65	
Railroad Yard Areas	40	0.50	0.55	0.60	0.65	
Undeveloped Areas						
Historic Flow Analysis-Greenbelts, Agricultural	2	0.15	0.25	0.20	0.30	
Pasture/Meadow	0	0.25	0.30	0.35	0.45	
Forest	0	0.10	0.15	0.15	0.20	
Exposed Rock	100	0.90	0.90	0.95	0.95	
Offsite Flow Analysis (when land use not defined)	45	0.55	0.60	0.65	0.70	
Streets			·	·		
Paved	100	0.90	0.90	0.95	0.95	
Gravel	80	0.80	0.80	0.85	0.85	
Drive and Walks	100	0.90	0.90	0.95	0.95	
Roofs	90	0.90	0.90	0.95	0.95	
Lawns	0	0.25	0.30	0.35	0.45	
*Hydrologic Soil Group		·				

EXPAND



USDA Natural Resources Conservation Service



Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
10	Blendon sandy loam, 0 to 3 percent slopes	В	10.9	100.0%
Totals for Area of Intere	st		10.9	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

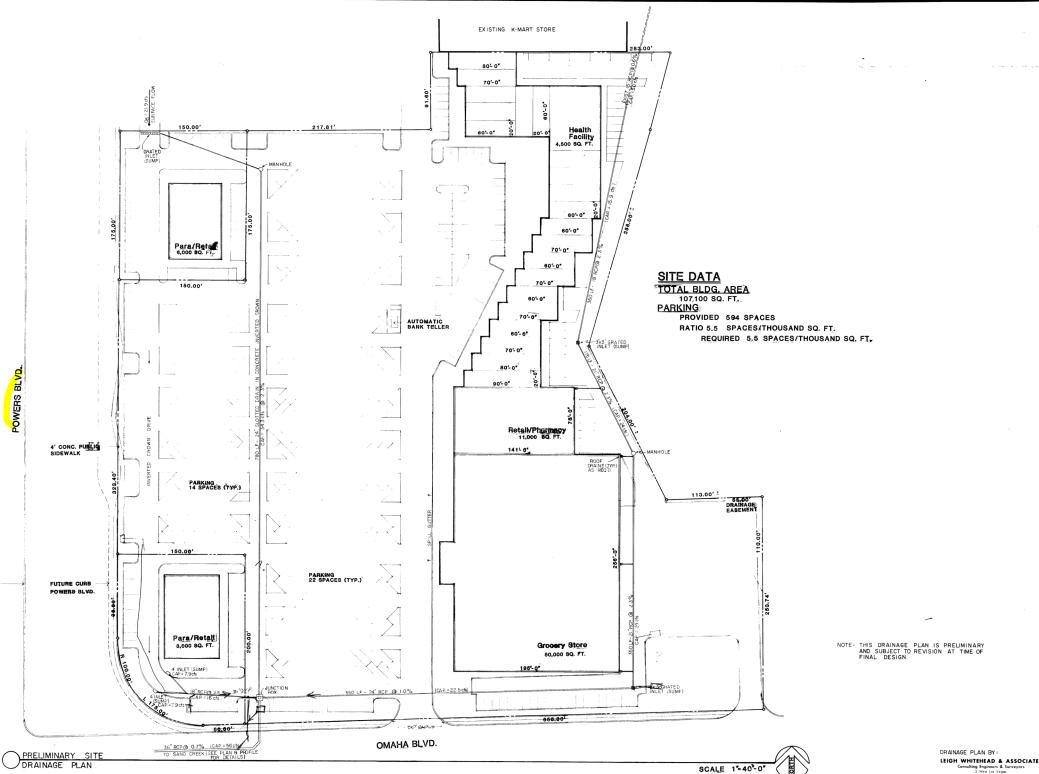
Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified

USDA



Consulting Engineers & Surveyors S. West Los Vegos COLORADO SPRINGS, COLORADO 80903 Phone (303) 636-5179

URB

