

***WASTEWATER DISPOSAL REPORT***

***for***

***EAGLEVIEW SUBDIVISION***

***EPC Parcel #: 522600001 AND 5226000002***

**December 2021**

**Prepared By:**



EAGLEVIEW SUBDIVISION  
EPC Parcel #: 522600001 AND 5226000002

WASTEWATER DISPOSAL REPORT

December 2021

Prepared for:

ProTerra Properties  
1864 Woodmoor Drive, Unit 100  
Monument, CO 80132

Prepared by:

JDS-Hydro Consultants, Inc.  
5540 Tech Center Drive, Suite 100  
Colorado Springs, CO 80919

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## 1.0 INTRODUCTION AND EXECUTIVE SUMMARY

The purpose of this report is to address the specific wastewater loads for the proposed Eagleview Subdivision in Colorado Springs, CO.

**EXECUTIVE SUMMARY:** The water rights and replacement plan in place for the existing parcel are adequate to meet the needs of thirty-eight (38) lots proposed for the subdivision on a 300-year basis.

## 2.0 PROJECTED LAND USES

### 2.1 Projected Land Uses

Lands within the subject area have been planned as a residential development. This report pertains to the existing 121.21-acre parcel that is proposed to be divided into thirty-eight (38) lots. Please refer to the *Land Use Exhibit* in **Appendix A**.

## 3.0 WASTEWATER REPORT

### 3.1 Wastewater Loads

There are thirty-eight (38) residential units proposed in the subdivision, which will all have on-site septic systems. A breakdown of projected wastewater loads is summarized in Table 3-1. Average daily wastewater loads are expected to be 90% of average daily indoor use.

**Table 3-1: Summary of Expected Water Demands & Wastewater Loads**

Water						Wastewater
# of SFE's	Annual Indoor Use 0.26 (AF/YR/SFE)	Average Daily Indoor Use (GPD)	Irrigation 0.0566 (AF/1,000 SF)	Domestic Watering 0.011 (AF/Horse/Year)	Total Indoor, Watering, & Irrigation (AF)	ADF (@ 90% Indoor Use) (GPD)
38	Note 1 9.50	8481	Note 2 5.94	Note 3 2.42	17.86	7633

Note 1: Per replacement plan No. 746-BD

Note 2: Per replacement plan No. 746-BD at 2,000 ft<sup>2</sup> of irrigatable land

Note 3: Per replacement plan No. 746-BD at 4 horses per lot



### 3.2 *On-Site Wastewater Treatment Systems (OWTS)*

The proposed single-family homes will be served by individual on-site wastewater treatment systems. The site was evaluated for *on-site wastewater treatment systems* (OWTS) by Entech Engineering, Inc. and summarized in their Wastewater Study for the Eagleview Subdivision released on November 19, 2021. Overall, six (6) previously drilled test borings, and eight (8) percolation test pits were previously performed on the site in July 2003 to determine general suitability for the use of OWTS. Since that time another eight (8) tactile test pits were excavated and evaluated by Entech Engineering to gauge site suitability against more recent on-site wastewater regulations.

Laboratory testing was also performed to classify and determine the soils engineering characteristics. Soils in the area are described as having moderate to rapid permeabilities, which would be considered suitable for conventional OWTS facilities. Because bedrock was encountered in the test pits at depths ranging from 1.5' to 8', and shallow bedrock (less than 5') was encountered in two test pits, designed OWTS facilities are recommended where there is shallow bedrock.

The Natural Resource Conservation Service (NRCS) has mapped two (2) soil types on the site, consisting of Columbine gravelly sandy loam and Pring coarse sandy loam. The limiting layers encountered in the test pits were found to be sandy loam (Soil Type 2 and 2A), sandy clay loam (Soil Type 3 and 3A), sandy clay (Soil Type 4 and 4A) and silty to clayey sandstone (Soil Types 3A and 4A) which corresponds to LTAR values ranging from 0.50 and 0.15 gallons per day per square foot.

There was no groundwater encountered in any of the percolation test pits, which were drilled to 10' to 15'. However, in some of the test pits there was evidence of seasonally shallow groundwater at depths between 3' to 5' below ground surface. In addition, the site is not mapped to lie within a floodplain, though there were areas of stagnant surface water and seasonally shallow groundwater. Designed systems are required where groundwater is encountered at 4', and then shallow leach fields perched at 4' are required where groundwater is encountered at 6'. Also, septic fields should not be located within 25' of drainage areas due to the potential of periodic high groundwater.

According to Entech's report, the site is suitable for OWTS, and contamination of surface and subsurface water resources should not occur if the OWTS sites are evaluated, installed, and maintained according to El Paso County and State guidelines. However, "designed systems" will be required for the majority of the proposed lots. Overall, the existing geologic engineering and geologic conditions will impose some minor constraints on the development and construction site.

The previously referenced *Wastewater Study for the Eagleview Subdivision* released on November 19, 2021 by Entech Engineering, Inc., is included in **Appendix B**. The *Soil, Geology, and Geologic Hazard Study* released on November 19, 2021 by Entech Engineering, Inc., is included in **Appendix C**.

# *Appendix A*

DEVELOPMENT NOTES:

1. WATER TO BE PROVIDED VIA INDIVIDUAL ON SITE WELLS OPERATED THROUGH A STATE APPROVED WATER REPLACEMENT PLAN
2. WASTE WATER TO BE INDIVIDUAL ON SITE SEPTIC SYSTEMS.
3. THIS PROPERTY IS LOCATED WITHIN AND SERVICED BY MOUNTAIN VIEW ELECTRIC ASSOCIATION SERVICE DISTRICT, AQUILA NATURAL GAS SERVICE DISTRICT, FALCON FIRE PROTECTION DISTRICT, AND THE FALCON SCHOOL DISTRICT.
4. EXISTING ZONING IS RR-3. PROPOSED ZONING IS RR-2
5. EXISTING ZONING IS RR-3. PROPOSED ZONING IS RR-2
6. BUILDING SETBACKS:
  - FRONT YARD - 15'
  - REAR YARD - 25'
  - SIDE YARD - 15'
7. DRAINAGE AND UTILITY EASEMENTS:
  - FRONT - 15'
  - REAR - 10'
  - PERMETER - 20'
8. ALL ROADWAYS AND DRAINAGE FACILITIES WITHIN DEDICATED RIGHTS OF WAY AND SPECIFIED EASEMENTS TO BE OWNED AND MAINTAINED BY EL PASO COUNTY
9. PROPOSED METHOD OF GUARANTEEING FUNDS WILL BE ONE OF ANY APPROVED ACCEPTABLE TO THE EL PASO COUNTY ATTORNEY'S OFFICE.
10. LOTS 31 & 32 SHALL HAVE A SHARED DRIVEWAY TO BE CONSTRUCTED BY THE DEVELOPER OF GRADING AND DRAINAGE EASEMENTS.
11. MOUNTAIN VIEW ELECTRIC ASSOC. REQUIRES THAT THEIR EASEMENTS BE THE DEVELOPER OF GRADING AND DRAINAGE EASEMENTS.
12. A 16' WIDE GRAVEL DRIVEWAY AND CULVERT SHALL BE CONSTRUCTED OVER THE DRAINAGE EASEMENT LOCATED ON LOT NO. 5. SAID DRIVEWAY TO BE CONSTRUCTED BY THE DEVELOPER AND INCLUDED WITHIN THE SUBDIVISION'S ESTIMATE OF GUARANTEED FUNDS.
13. DRIVEWAY ACCESS TO LOT NO. 5 SHALL BE LIMITED TO FLAMING SUN DRIVE.
14. THE DEVELOPER SHALL BE RESPONSIBLE FOR CONSTRUCTING THE DRIVEWAY AND BE INCLUDED WITHIN THE ESTIMATE OF GUARANTEED FUNDS.

DEVELOPMENT DATA

SINGLE FAMILY RESIDENTIAL	112.27 AC	93 %
OPEN SPACE TRACTS	1.79 AC	1.5%
ROAD ROW	71.5 AC	5.5%
TOTAL	121.21 AC	100%
TOTAL NUMBER OF LOTS = 38		
MINIMUM LOT SIZE = 2.50 AC		
AVERAGE LOT SIZE = 2.85 AC		
GROSS DENSITY = 1 LOT PER 3.19 AC		
TOTAL LF OF ROW = 5192 LF		

INDICATES WETLANDS

INDICATES MINIMUM FRONT YARD SETBACK AT MINIMUM 200' LOT WIDTH

PROPERTY OWNER:

JERRY L LANDRESS  
12585 BRIDLE BIT DRIVE  
COLORADO SPRINGS, CO 80908  
719.495.2145

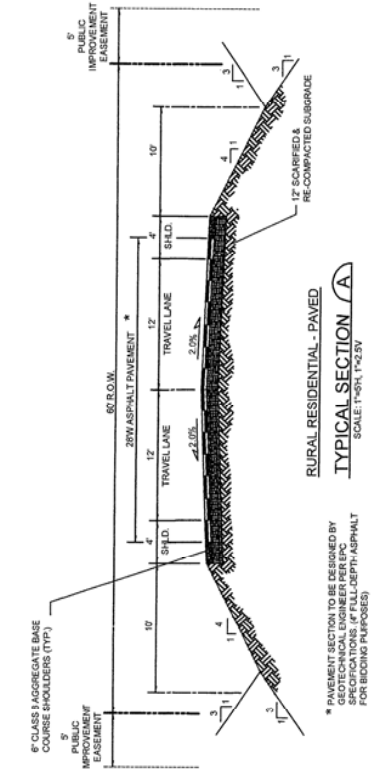
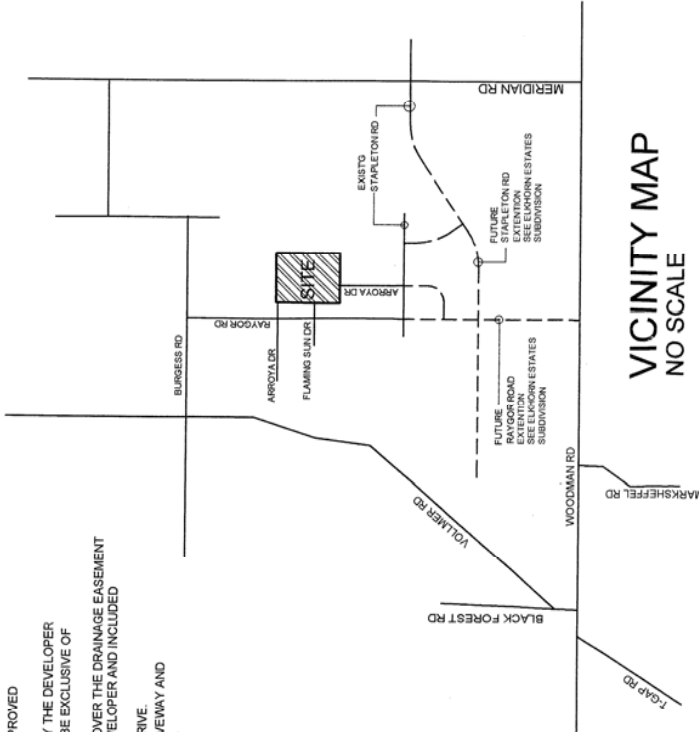
MINERAL RIGHTS OWNER:

MOUNTAIN VIEW PROPERTIES OF PEYTON, LLP (50%)  
12585 BRIDLE BIT DRIVE  
P.O. BOX 124, PEYTON, COLORADO 80801

JERRY L LANDRESS (50%)  
12585 BRIDLE BIT DRIVE  
COLORADO SPRINGS, CO 80908  
719.495.2145

ADJOINING PROPERTY OWNERS

1. KRISTI S MARQUESS 10475 RAYGOR RD COLO SPGS CO 80908-4803
2. JAMES SPRINGS TRUST 10535 RAYGOR RD COLO SPGS CO 80908-4421
3. LAUREL M MARQUESS 10535 RAYGOR RD COLO SPGS CO 80908-4421
4. DONALD M BLASUS 10535 ARROYA LN COLO SPGS CO 80903-2653
5. SIX NINETY NINE LA LLC 545 E PIKES PEAK AVE COLO SPGS CO 80903-2653
6. SANDRA M & GREGORY SMITH 9725 ARROYA LN COLORADO SPRINGS, CO 80908-4441
7. DAVID L & KAREN B WOLFE 9825 ARROYA LN COLO SPGS CO 80909-4427
8. JAMES D & APRIL D BORST 9850 ARROYA LN COLO SPGS CO 80909-4805
9. CARL W SPRAY 9915 RAYGOR RD COLO SPGS CO 80909-4805
10. CARL W SPRAY 9915 RAYGOR RD COLO SPGS CO 80909-4805
11. CARL W SPRAY 9915 RAYGOR RD COLO SPGS CO 80909-4805
12. LARRY D & BONNIE L ROBERTS 10155 RAYGOR RD COLO SPGS CO 80908-4815
13. JASON & CHRISTINA MATTHEWS 9750 FLAMING SUN DR 80908-4815
14. RICHARD J & DOROTHY L FAVINGER 10295 RAYGOR RD COLO SPGS CO 80908-4805
15. SUSAN G. WITTER, 10385 RAYGOR RD, COLO SPGS, CO 80908-4803



LAND DESCRIPTION

A portion of the Northwest One-Quarter (N.W.1/4) of Section 26, Township 12 South, Range 65 West of the 6th P.M., County of El Paso, State of Colorado, more particularly described as follows:

Beginning at the Center One-Quarter (1/4) Section corner of said Section 26; Thence N89°31'29\"/>

Said tract contains 5,279,822.173 Square feet (121.208 Acres) more or less.

RECEIVED

DEC 19 2007

ELPC DEVELOPMENT SERVICES

EAGLEVIEW SUBDIVISION

PRELIMINARY PLAN



PARCEL NO. S 52260 00 001 & 52260 00 002  
SCALE 1\"/>

CONT. INT. = 2'

25' 100'

0' 50'

1000'

EL PASO COUNTY, COLORADO

DEVELOPED BY: THE PRO-LAND GROUP LLC.

CO 80908

LAND RESOURCE ASSOCIATES

4455 FOUNTAIN AVE.  
CASCADE, CO 80809  
719-684-2298

## *Appendix B*



**ENTECH**  
ENGINEERING, INC.

505 ELKTON DRIVE  
COLORADO SPRINGS, CO 80907  
PHONE (719) 531-5599  
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**WASTEWATER STUDY  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYOR ROAD  
PARCEL NOS. 52260-00-001 & 52260-00-002  
EL PASO COUNTY, COLORADO**

Prepared for

**PT Eagleview, LLC**  
1865 Woodmoor Drive, Suite 100  
Monument, Colorado 80132

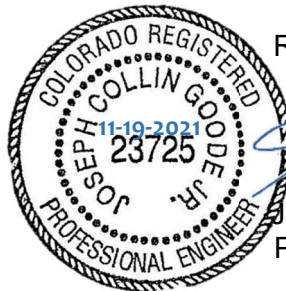
Attn: Joseph DesJardin

November 19, 2021

Respectfully Submitted,

ENTECH ENGINEERING, INC.

Logan L. Langford, P.G.  
Geologist



Reviewed by:

Joseph C. Goode, Jr., P.E.  
President

LLL/jhr

Encl.

Entech Job No. 212684  
AAprojects/2021/212684 county ww

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APPENDIX B: Test Pit Logs

APPENDIX C: Laboratory Test Results

APPENDIX D: Soil Survey Descriptions

## 1.0 SUMMARY

### ***Project Location***

The project site lies in portions of the NW $\frac{1}{4}$  and SW $\frac{1}{4}$  of Section 26, Township 12 South, Range 65 West of the 6<sup>th</sup> Principal Meridian in the northern portion of El Paso County, Colorado. The site is located approximately 14 miles southeast of Monument, Colorado, southwest of Burgess Road and Goodson Road.

### ***Project Description***

Total acreage involved in the subdivision is approximately 124 acres. The proposed development consists of thirty-eight rural residential lots. The development will utilize individual wells and on-site wastewater treatment systems.

### ***Scope of Report***

This report presents the results of our site evaluation to determine suitability for on-site wastewater treatment systems.

### ***Land Use and Engineering Geology***

This site was found to be suitable for the proposed development. Areas were encountered where the geologic conditions will impose some constraints on development and land use. These include areas of seasonally shallow groundwater, potential seasonally shallow groundwater, drainage areas, artificial fill, expansive soils, and potentially unstable slopes along the main drainages on site. Based on the proposed development plan, it appears that these areas will have some impact on the development. These conditions will be discussed in greater detail in the report.

In general, it is our opinion that the development can be achieved if the observed geologic conditions on site are either avoided or properly mitigated. All recommendations are subject to the limitations discussed in the report.

## **2.0 GENERAL SITE CONDITIONS AND PROJECT DESCRIPTION**

The site is located in portions of the NW¼ and SW¼ of Section 26, Township 12 South, Range 65 West of the 6th Principal Meridian in the northern portion of El Paso County, Colorado. The site is located approximately 14 miles southeast of Monument, Colorado, southwest of Goodson Road and Burgess Road. The location of the site is as shown on the Vicinity Map, Figure 1.

The topography of the site varies from gently to moderately sloping generally to the south and southeast with some steeper slopes along the drainages on-site. The drainages on site flow in southeasterly and southerly directions through the property. No water was observed flowing in any of the drainages at the time of this investigation. The site boundaries are indicated on the USGS Map, Figure 2. Previous land uses have included grazing and pasture land. The site contains primarily field grasses, and weeds with areas of scattered pine trees and willow along the drainages. Site photographs are included in Appendix D. The approximate locations and directions of the photographs are indicated on Figure 3.

Total acreage involved in the proposed development is approximately 124 acres with thirty-eight rural residential lots. The proposed residential lots are approximately 2.5 to 4.2 acres each. The area will be serviced by individual wells and on-site wastewater treatment systems. The proposed Site Plan/Testing Location Map is presented in Figure 3.

The site was previously investigated as part of a Soil, Geology, and Wastewater Study, performed by Entech Job No. 80503 (Reference 1). Six (6) previously drilled test borings, and eight (8) percolation test were performed on the site to determine general suitability of the site for the use of on-site wastewater treatment systems. The previous report/investigation was utilized to evaluate the site as part of this investigation.



### **3.0 SCOPE OF THE REPORT**

The scope of this report includes:

- A general geologic analysis utilizing published geologic data. Detailed site-specific mapping will be conducted to obtain general information in respect to major geographic and geologic features, geologic descriptions and their effects on the development of the property.

### **4.0 FIELD INVESTIGATION**

Our field investigation consisted of the preparation of a geologic map of any bedrock features and significant surficial deposits. The Natural Resource Conservation Service (NRCS), previously the Soil Conservation Service (SCS) survey was also reviewed to evaluate the site. The position of mappable units within the subject property are shown on the Geologic Map. Our mapping procedures involved both field reconnaissance and measurements and air photo reconnaissance and interpretation. The same mapping procedures have also been utilized to produce the Geology/Engineering Geology Map which identified pertinent geologic conditions affecting development. The field mapping was performed by personnel of Entech Engineering, Inc. for the previous report referenced above (Reference, 1) and verified on October 13, 2021.

Test borings from the Soil, Geology, and Wastewater Study, performed by Entech Job No. 80503, were used as part of the investigation. Six (6) test borings, and eight (8) percolation test were previously performed on the site to determine general suitability of the site for residential construction and the use of on-site wastewater treatment systems respectively. Eight (8) tactile test pits were recently excavated and evaluated by Entech Engineering, Inc. personnel due to the current on-site wastewater regulations. The locations of the test borings, and test pits are indicated on the Site Plan/Testing Location Map, Figure 3. The Test Pit and Test Boring Logs are presented in Appendix B. Results of this testing will be discussed later in this report.

Laboratory testing was also performed on some of the soils to classify and determine the soils engineering characteristics. Laboratory tests included grain-size analysis, ASTM D-422, and Atterberg Limits, ASTM D-4318. Results of the laboratory testing are included in Appendix C. A Summary of Laboratory Test Results is presented in Table 1. A Summary of Laboratory Test Results, Test Boring Logs from the previous investigation are included in Appendix D.

## 5.0 SOIL, GEOLOGY AND ENGINEERING GEOLOGY

### 5.1 General Geology

Physiographically, the site lies in the western portion of the Great Plains Physiographic Province. Approximately 13 miles to the west is a major structural feature known as the Rampart Range Fault. This fault marks the boundary between the Great Plains Physiographic Province and the Southern Rocky Mountain Province. The site exists within the southeastern edge of a large structural feature known as the Denver Basin. Bedrock in the area tends to be very gently dipping in an easterly direction. The rocks in the area of the site are sedimentary in nature, and typically Tertiary to Upper Cretaceous in age. The bedrock underlying the site consists of the Dawson Arkose Formation. Overlying this formation are unconsolidated deposits of residual, colluvial, man-made, and alluvial soils of the Quaternary Age. The residual soils are produced by the in-situ action of weathering of the bedrock on site. Some colluvial soils exist which are deposited by gravity and sheetwash. The alluvial soils were deposited by water in the major drainages on site. Man-made soils exist as fill berms and earthen dams. The site's stratigraphy will be discussed in more detail in Section 5.3.

### 5.2 Soil Conservation Survey

The Natural Resource Conservation Service (Reference 2), previously the Soil Conservation Service (Reference 3) has mapped two soil types on the site (Figure 4). In general, they vary from coarse sandy loam to gravelly sandy loam. The soils are described as follows:

<u>Type</u>	<u>Description</u>
19	Columbine Gravelly sandy loam, 0 to 3% slopes
71	Pring Coarse Sandy Loam, 3-8% slopes

Complete descriptions of each soil type are presented in Appendix D. The soils have generally been described to typically have slow to rapid permeabilities. The majority of the soils have moderate to rapid permeabilities. Limitations described for local soils include shrink-swell potential. Roads may need to be designed to minimize frost-heave potential. Possible hazards with soil erosion are present on the site. The erosion potential can be controlled with vegetation,

erosion control blankets, and waddles. The majority of the soils have been described to have moderate erosion hazards.

### **5.3 Site Stratigraphy**

The Falcon NW Quadrangle Geology Map is presented in Figure 5 (Reference 4). These maps in conjunction with site specific mapping were used to prepare the site Geology Map. The Geology Map prepared for the site is presented in Figure 6. Four mappable units were identified on this site which are identified as follows:

**Qaf Artificial Fill of Quaternary Age:** These are man-made fill deposits associated with berms and small earth dam embankments on-site.

**Qal Recent Alluvium of Quaternary Age:** These are recent stream deposits in the channels of the main drainages on site.

**Qao<sub>1</sub> Old Alluvium one of late middle - Pleistocene Age:** These deposits are light brown silty sands which contain an abundance of gravels. They commonly occur as stream terrace deposits above the valley floors. Old alluvium one may correlate with the Louviers Alluvium in the Denver area.

**Tkd Dawson Arkose of Tertiary to Cretaceous Age:** The Dawson formation consists of arkosic sandstone with interbedded lenses of fine-grained sandstone, claystone or siltstone. Typically, it is buff to light brown and light gray in color. Overlying the Dawson is a variable layer of colluvial and residual soil. These materials consist of silty to clayey sands and gravels deposited by the action of sheetwash and gravity. Some residual soils derived from the in-situ weathering of the bedrock on site exist in this mapping. These soils are overlying the Dawson Formation in many places on this site.

The soils listed above were mapped from site-specific mapping, the *Geologic Map of the NW Falcon Quadrangle* distributed by the Colorado Geological Survey in 2012 (Reference 4), and the *Geologic Map of the Denver 1° x 2° Quadrangle*, distributed by the US Geological Survey in 1981 (Reference 5). The Test Pits and Test Borings were also used in evaluating the site and are

included in Appendices B and D. The Geology Map prepared for the site is presented in Figure 6.

#### **5.4 Soil Conditions**

The soils encountered in the test pits, test borings and the profile holes of the percolation tests can be grouped into three general soil types. The soils were classified using the Unified Soil Classification System (USCS). Several soil descriptors are results of the previously studied Soils, Geology, and Waste Water Study, performed by Entech Engineering, Inc. (Reference 1). Descriptions of the soil types are discussed as follows:

Soil Type 1: consists of slightly silty to silty and clayey sand (SW-SM, SC). The sands were encountered in the upper soil profile of nearly all of the profile holes and test borings. These soils were encountered at loose to dense states and moist conditions. The clayey sands are slightly to highly expansive. Swell pressures of 694 psf and 2233 psf were measured on the clayey sands. The slightly silty and silty sands are generally considered non-expansive.

Soil Type 2: is a sandy to silty clay soil (CL), encountered in the upper soil profile in Test Boring No. 2 and Profile Hole Nos. 2 and 4. The clays were encountered at soft to stiff consistencies and moist conditions. A swell pressure of 2060 psf was measured in the FHA Swell Test. This swell is in the high expansion range.

Soil Type 3: consists of silty to clayey sandstone bedrock (SC, SM-SC). The sandstone was encountered in all of the test borings and in Profile Hole Nos. 2, 3, 7 and 8 at depths ranging from the surface to 9 feet below the surface. The sandstone was encountered at very dense states and moist conditions. The clayey sandstone is slightly expansive. FHA Swell pressures of 425 psf and 447 psf were measured on the sandstone.

The Test Pit Logs are presented in Appendix B. Laboratory Test Results are presented in Appendix C. Test Pit Boring Logs and Laboratory Test Results from the previous investigation are presented in Appendix D (Reference, 1).

## **5.5 Groundwater**

Groundwater was not encountered in the test borings which were drilled to 10 to 15 feet. However, seasonally shallow groundwater evidence was encountered in Test Pit Nos. 3 and 6 at depths of 3 to 5 feet below ground surface. The remaining Test Pits which were excavated to 5 to 8 feet did not encounter water or signs of seasonally occurring groundwater. Areas of seasonal and potentially seasonal shallow groundwater have been mapped in low-lying areas and in the drainages on-site. These areas are discussed in the following section. Fluctuation in groundwater conditions may occur due to variations in rainfall and other factors not readily apparent at this time.

It should be noted that in the sandy materials on site, some groundwater conditions might be encountered due to the variability in the soil profile. Isolated sand and gravel layers within the soils, sometimes only a few feet in thickness and width, can carry water in the subsurface. Groundwater may also flow on top of the underlying bedrock. Builders and planners should be cognizant of the potential for the occurrence of such subsurface water features during construction on-site and deal with each individual problem as necessary at the time of construction.

### **Floodplain and Drainage Areas**

The site is not mapped within a floodplain zone according to the FEMA Map No. 08041CO53G, dated December 7, 2018 (Figure 7, Reference 6). Some small areas of stagnant water were observed on this site. Additionally, areas of seasonal and potentially seasonal shallow groundwater were observed across the site. In these areas, we would anticipate the potential for periodically high subsurface moisture conditions and frost heave potential. These areas lie within low-lying areas along the drainage in the central portions of the subdivision and in the low-lying areas and minor drainages across the site. Water was not observed in any of the minor drainages at the time of our site investigation. These areas can likely be avoided or properly mitigated by development. Specific floodplain locations and drainage studies are beyond the scope of this report. Regrading can also mitigate some minor drainage swales on some of the lots. Structures should not block drainages. Any site grading should be done in such a manner as to not create areas of ponded water around structures or septic fields. Septic fields should not be located within 25 feet of drainage areas due to the potential for periodic high groundwater conditions.

In summary, development of the site can be achieved if the items mentioned above are mitigated. These items can be mitigated through proper design and construction or through avoidance. Investigation on each lot is recommended prior to construction.

*psw    Potentially Seasonal High Groundwater Area (Constraint)*

In these areas, we would anticipate the potential for periodically high subsurface moisture conditions, frost heave potential and highly organic soils. The majority of these areas lie within minor drainage areas which can be avoided by the proposed development. Construction in any portions of these areas, if required, should follow these precautions.

Mitigation: In these locations, foundation in areas subject to severe frost heave potential should penetrate to a sufficient depth so as to discourage the formation of ice lenses beneath foundations. At this location and elevation, a foundation depth of 30 inches is recommended for frost protection. In areas where high subsurface moisture conditions are anticipated periodically, a subsurface perimeter drain will be necessary to help prevent the seepage of water into areas located below grade. Any grading in these areas should be done in a manner that directs surface flow around construction to avoid areas of ponded water. Areas of organic material will require removal before any filling is done. Additionally, septic systems are not recommended in these areas unless analyzed for the hazard of high groundwater. Groundwater may be at sufficient depth as to not affect septic fields or foundations. Further investigation is recommended prior to construction in these areas to further delineate groundwater depths.

*sw    Seasonal High Groundwater Area (Constraint)*

These are areas within the main drainages on site and behind earthen dams. Water was not observed flowing in these drainages at the time of this investigation, however, vegetation and soils observed indicate water is near or at the surface during periods of high moisture. Small areas of ponded water were observed in some of the drainages. These areas also contain frost heave potential and highly organic soils.

Mitigation: Because the majority of the areas mapped as seasonally wet lie within defined drainages, we do not recommend structures be built within these areas. Lots are of sufficient size that these areas can be avoided as building sites. Should foundations encroach on seasonal shallow groundwater areas, the recommendations for potentially seasonal shallow groundwater mitigation should be followed. Additionally, basements or septic fields would not be recommended in these areas. Septic fields should be located a minimum of 25 feet away from the drainage areas. Any construction in these areas should be done in a manner that does not create ponded water. Where roadways or driveways cross drainages, adequately sized culverts should be installed. No areas of the site are mapped within any floodplain zones according to the FEMA Map No. 08041CO535G, Figure 10 (Reference 5). A 100-year floodplain along the main drainage on-site is indicated on Figures 3 and 9. Finished floors must be a minimum of one foot above the 100-year floodplain level. Specific floodplain locations and drainage studies are beyond the scope of this report.

## **6.0 ON-SITE WASTEWATER TREATMENT**

The site was evaluated for individual on-site wastewater treatment systems in accordance with El Paso Land Development Code. Eight (8) tactile test pits were recently excavated on the property for the Eagle View Subdivision. The test pits were located in potential locations of future systems. The approximate locations of the Test Pits are indicated on Figure 3, on the Geology/Engineering Geology Map, Figure 6, and on the Septic Suitability Map, Figure 8. A table showing the results of the Tactile Test Pits is presented in Table 2. Test Pit Logs are included in Appendix B. A Summary of Laboratory Test Results, and Test Pit Logs from the previous investigation are included in Appendix D.

The Natural Resource Conservation Service (Reference 2), previously the Soil Conservation Service (Reference 3) has been mapped with two soil descriptions. The Soil Survey Map (Reference 2) is presented in Figure 4, and the Soil Survey Descriptions are presented in Appendix D. The soils are described as having slow to rapid percolation rates. The majority of the soils have been described with moderate permeabilities.

Soils encountered in the tactile test pits consisted of sandy loam, sandy clay loam, and sandy clay with underlying weathered to formational silty to clayey sandstone. Bedrock was encountered

in the test pits at depths ranging from 1.5 to greater than 8 feet. Shallow bedrock (less than 5 feet) was encountered in two of the test pits. The limiting layers encountered in the test pits are the sandy loam (Soil Type 2 and 2A), sandy clay loam (Soil Type 3 and 3A), sandy clay (Soil Type 4 and 4A) and silty to clayey sandstone (Soil Types 3A and 4A) which corresponds to LTAR values ranging from 0.50 to 0.15 gallons per day per square foot.

In summary, it is our opinion the site is suitable for individual on-site wastewater treatment systems (OWTS) and that contamination of surface and subsurface water resources should not occur provided the OWTS sites are evaluated and installed according to El Paso County and State Guidelines and properly maintained. Based on the testing performed as part of this investigation designed systems will likely be required for the majority of the lots. Additional investigation may identify areas where suitable conventional systems could be used on the lots. A Septic Suitability Map is presented in Figure 8. OWTS sites should not be located within drainages. **Individual soil testing is required on the lots prior to construction.** Absorption fields must be located a minimum of 100 feet from any well, including those on adjacent properties. A drainage area in the eastern portion of the site is designated as open space and will be avoided by development. Absorption fields must also be located a minimum of 50 feet from any drainages, floodplains or ponded areas and 25 feet from dry gulches.

## **7.0 CLOSURE**

It is our opinion that the existing geologic engineering and geologic conditions will impose some minor constraints on development and construction of the site. The majority of these conditions can be avoided by construction. Others can be mitigated through proper engineering design and construction practices. The proposed development and use is consistent with anticipated geologic and engineering geologic conditions.

It should be pointed out that because of the nature of data obtained by random sampling of such variable and non-homogeneous materials as soil and rock, it is important that we be informed of any differences observed between surface and subsurface conditions encountered in construction and those assumed in the body of this report. Individual investigations for building sites and septic systems will be required prior to construction. Construction and design personnel should be made familiar with the contents of this report. Reporting such discrepancies to Entech



Engineering, Inc. soon after they are discovered would be greatly appreciated and could possibly help avoid construction and development problems.

This report has been prepared for PT Eagleview, LLC for application to the proposed project in accordance with generally accepted geologic soil and engineering practices. No other warranty expressed or implied is made.

We trust that this report has provided you with all the information that you required. Should you require additional information, please do not hesitate to contact Entech Engineering, Inc.

## BIBLIOGRAPHY

1. Entech Engineering, Inc. July 8, 2003, Revised November 11, 2005. *Soil, Geology, and Geologic Hazard Study, Eagleview Subdivision, El Paso County Colorado. Job No. 80503.*
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8. El Paso County Planning Development. December 1995. *El Paso County Aggregate Resource Evaluation Maps.*
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10. Keller, John W.; TerBest, Harry and Garrison, Rachel E. 2003. *Evaluation of Mineral and Mineral Fuel Potential of El Paso County State Mineral Lands Administered by the Colorado State Land Board.* Colorado Geological Survey. Open-File Report 03-07.

TABLES

**TABLE 1**  
**SUMMARY OF LABORATORY TEST RESULTS**

CLIENT PT EAGLEVIEW  
PROJECT ARROYA AND RAYGOR  
JOB NO. 212684

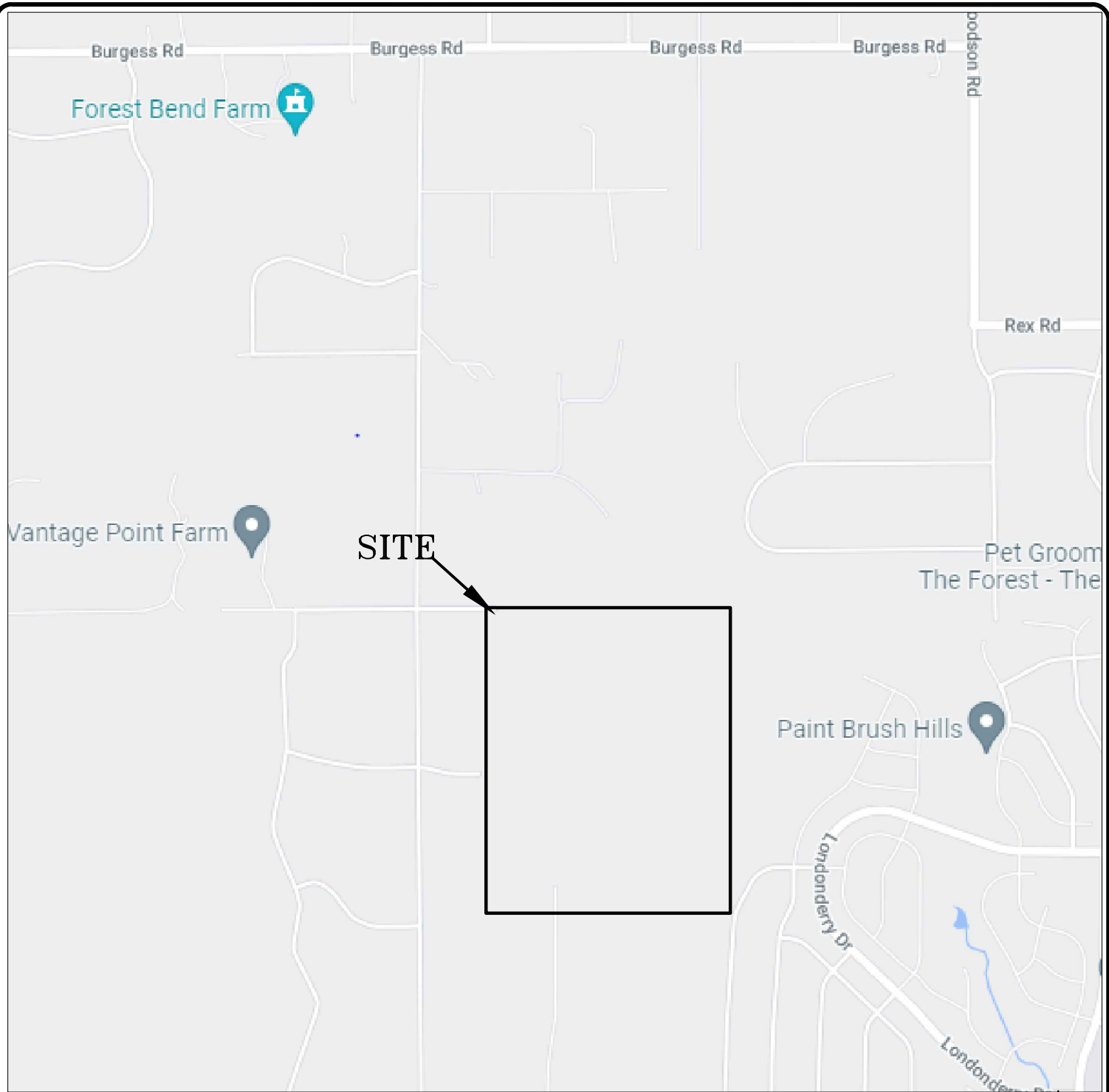
SOIL TYPE	TEST BORING NO.	DEPTH (FT)	WATER (%)	DRY DENSITY (PCF)	PASSING NO. 200 SIEVE (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	SULFATE (WT %)	FHA SWELL (PSF)	SWELL/CONSOL (%)	UNIFIED CLASSIFICATION	SOIL DESCRIPTION
1	TP-1	3			1.4						SW	SAND
1	TP-2	1-2			12.6						SM	SAND, SILTY
1	TP-4	3-4			11.2						SM-SW	SAND, SLIGHTLY SILTY
1	TP-5	1-2			10.5						SM-SW	SAND, SLIGHTLY SILTY
1	TP-5	3			7.3						SM-SW	SAND, SLIGHTLY SILTY
1	TP-6	1.5			35.1						SC	SAND, CLAYEY
1	TP-6	2.5			6.3						SM-SW	SAND, SLIGHTLY SILTY
1	TP-7	2			3.1						SW	SAND
1	TP-7	5			9.6						SM-SW	SAND, SLIGHTLY SILTY
1	TP-8	4			15.8						SM	SAND, SILTY
2	TP-3	2-3			78.3						CL	CLAY, SANDY

**Table 2: Summary Tactile Test Pit Results**

<b>Test Pit No.</b>	<b>USDA Soil Type</b>	<b>LTAR Value</b>	<b>Depth to Bedrock (ft.)</b>	<b>Depth to Seasonally Occurring Groundwater (ft.)</b>
1	2A	0.50	N/A	N/A
2*	3A	0.30	2.5	N/A
3*	4A	0.15	N/A	4.5
4*	3A	0.30	N/A	N/A
5	2A	0.50	N/A	N/A
6*	4A	0.15	N/A	3
7	3	0.30	N/A	N/A
8*	4A	0.15	5	N/A

\*- Conditions that will require an engineered OWTS

## FIGURES



**ENTECH**  
ENGINEERING, INC.  
505 ELKTON DRIVE  
COLORADO SPRINGS, CO. 80907 (719) 531-5599

VICINITY MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

DRAWN:  
JHR

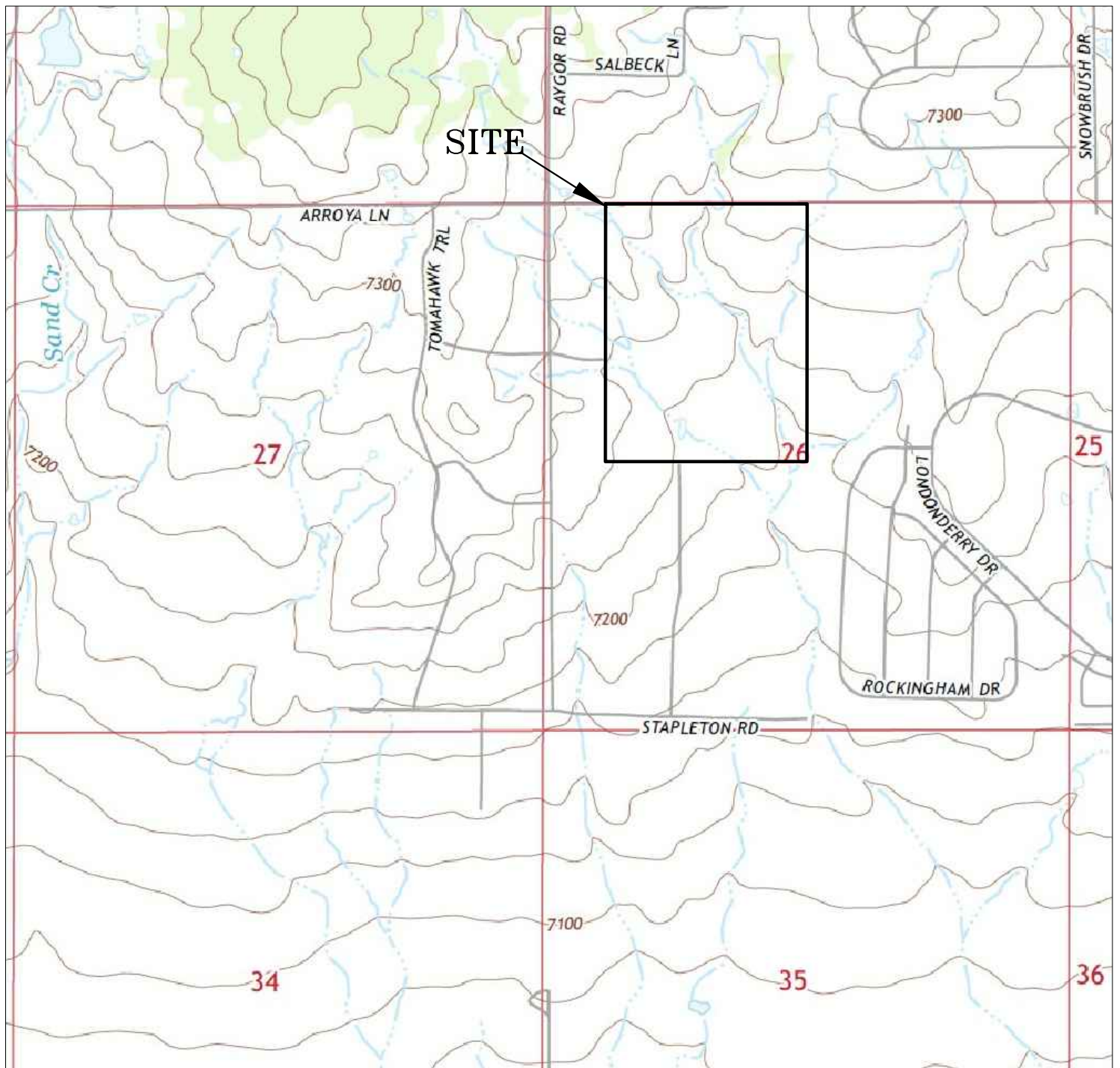
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11/8/21

CHECKED:  
LLL

DATE:

JOB NO.:  
212684

FIG NO.:  
1



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USGS MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

DRAWN:  
JHR

DATE:  
11/8/21

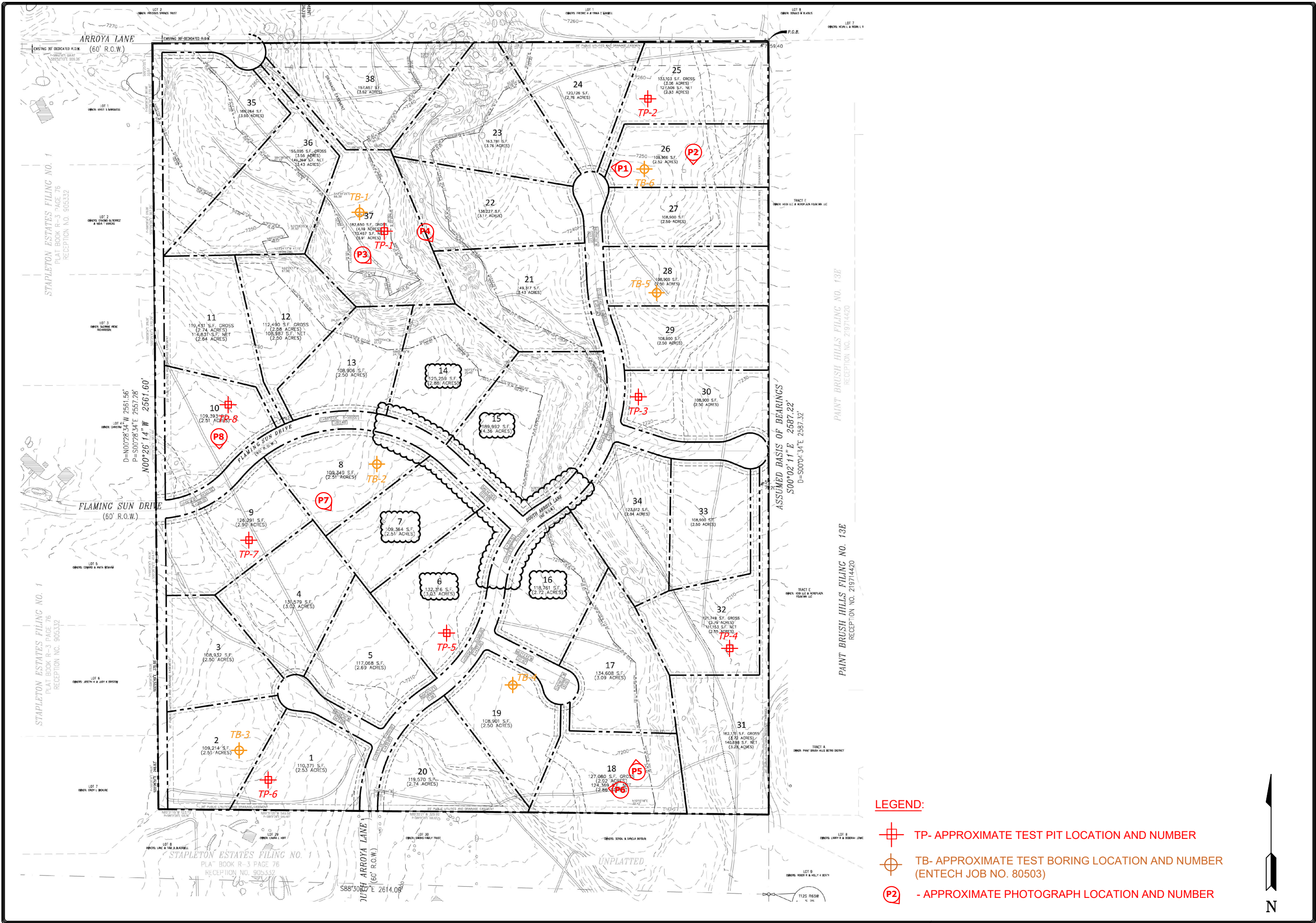
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JOB NO.:  
212684

FIG NO.:  
2





LEGEND:

- TP- APPROXIMATE TEST PIT LOCATION AND NUMBER
- TB- APPROXIMATE TEST BORING LOCATION AND NUMBER (ENTECH JOB NO. 80503)
- P- APPROXIMATE PHOTOGRAPH LOCATION AND NUMBER

REVISION	BY

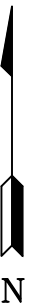
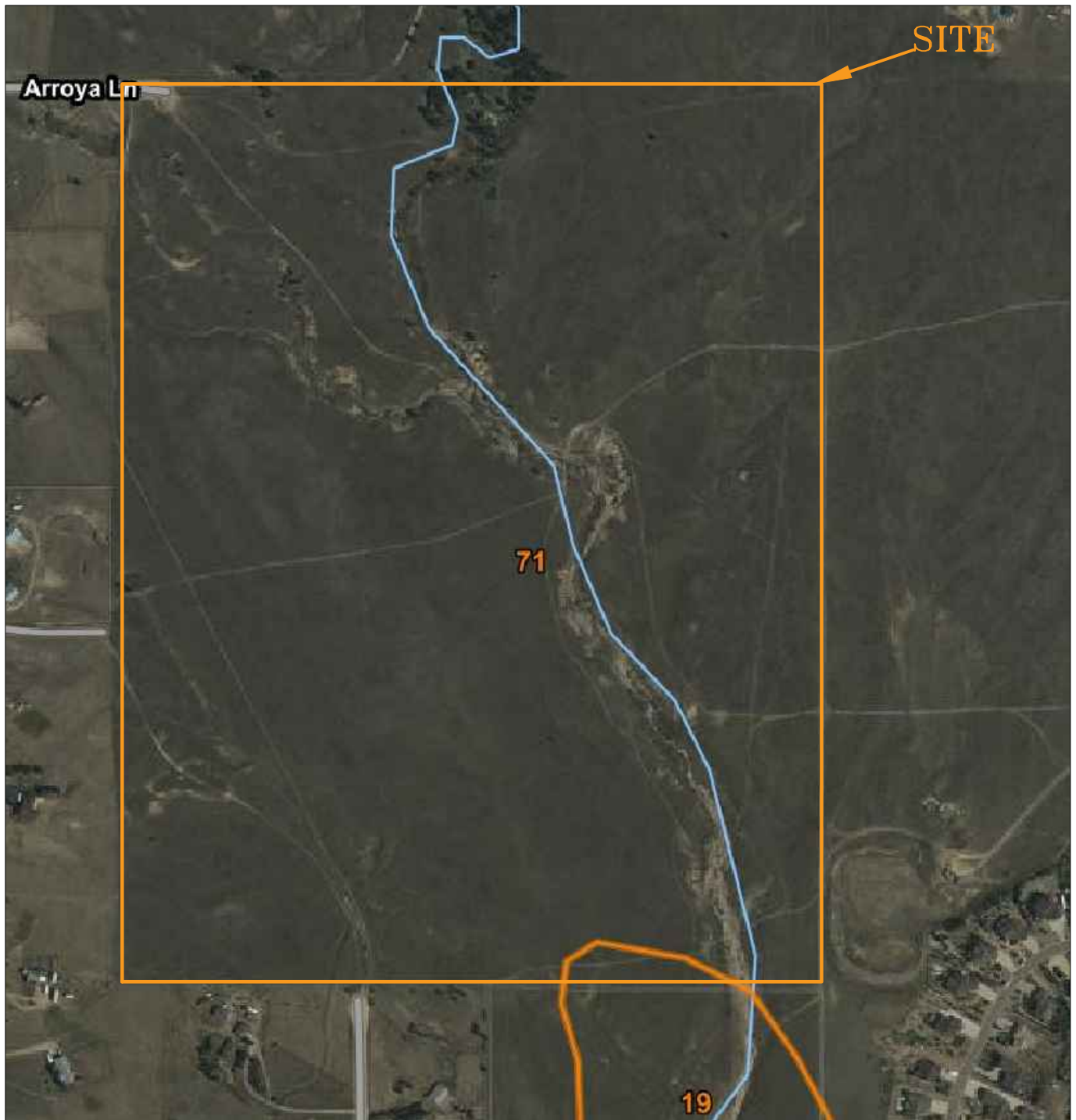
**ENTECH**  
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COLORADO SPRINGS, CO. 80907  
(719) 531-5599

SITE PLAN/TEST BORING LOCATION MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

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DATE 11/4/21
SCALE AS SHOWN
JOB NO. 212684
FIGURE No. 3





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SOIL SURVEY MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
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JHR

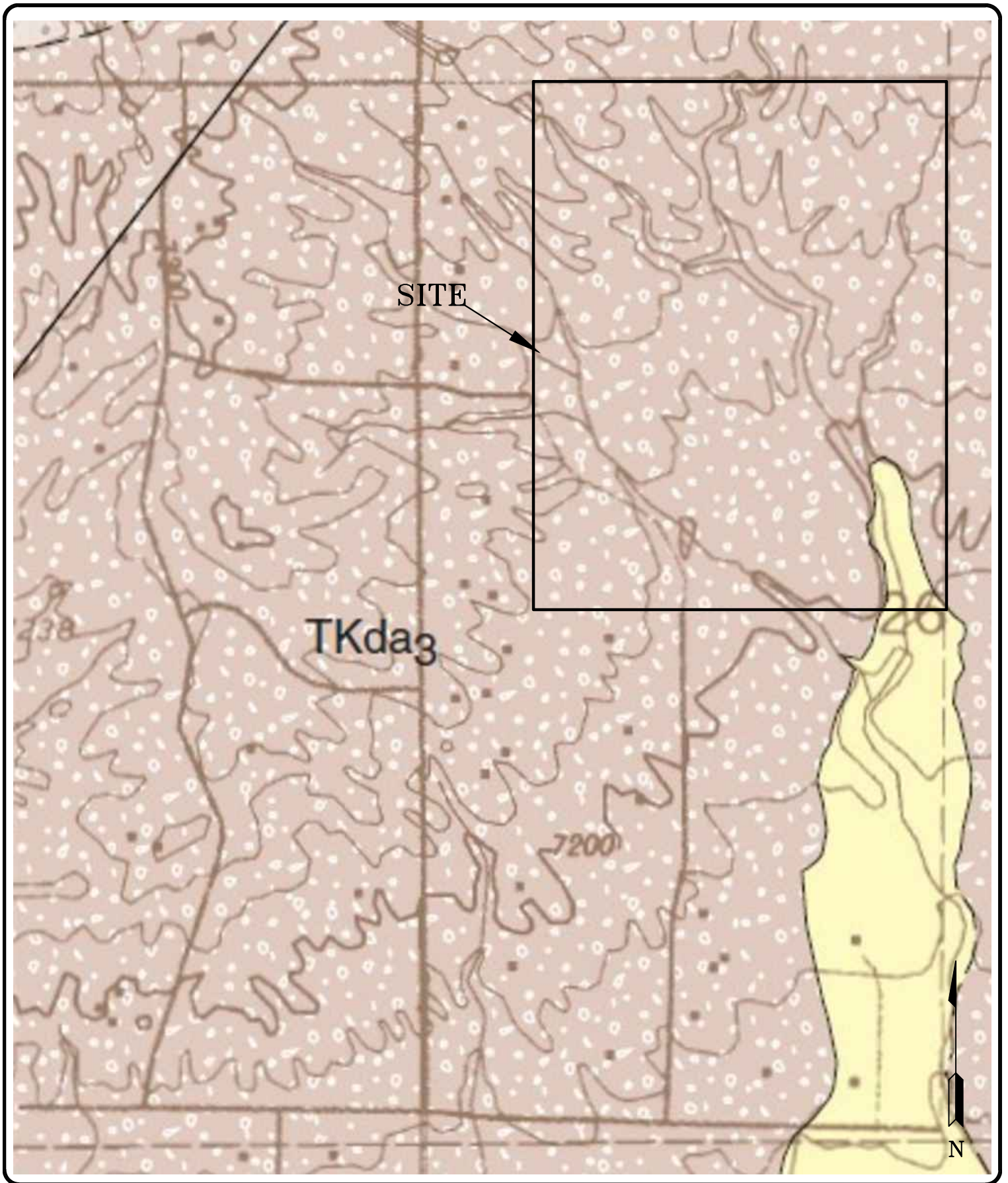
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11/8/21

CHECKED:  
LLL

DATE:

JOB NO.:  
212684

FIG NO.:  
4



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NW FALCON QUADRANGLE GEOLOGIC MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

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JHR

DATE:  
11/8/21

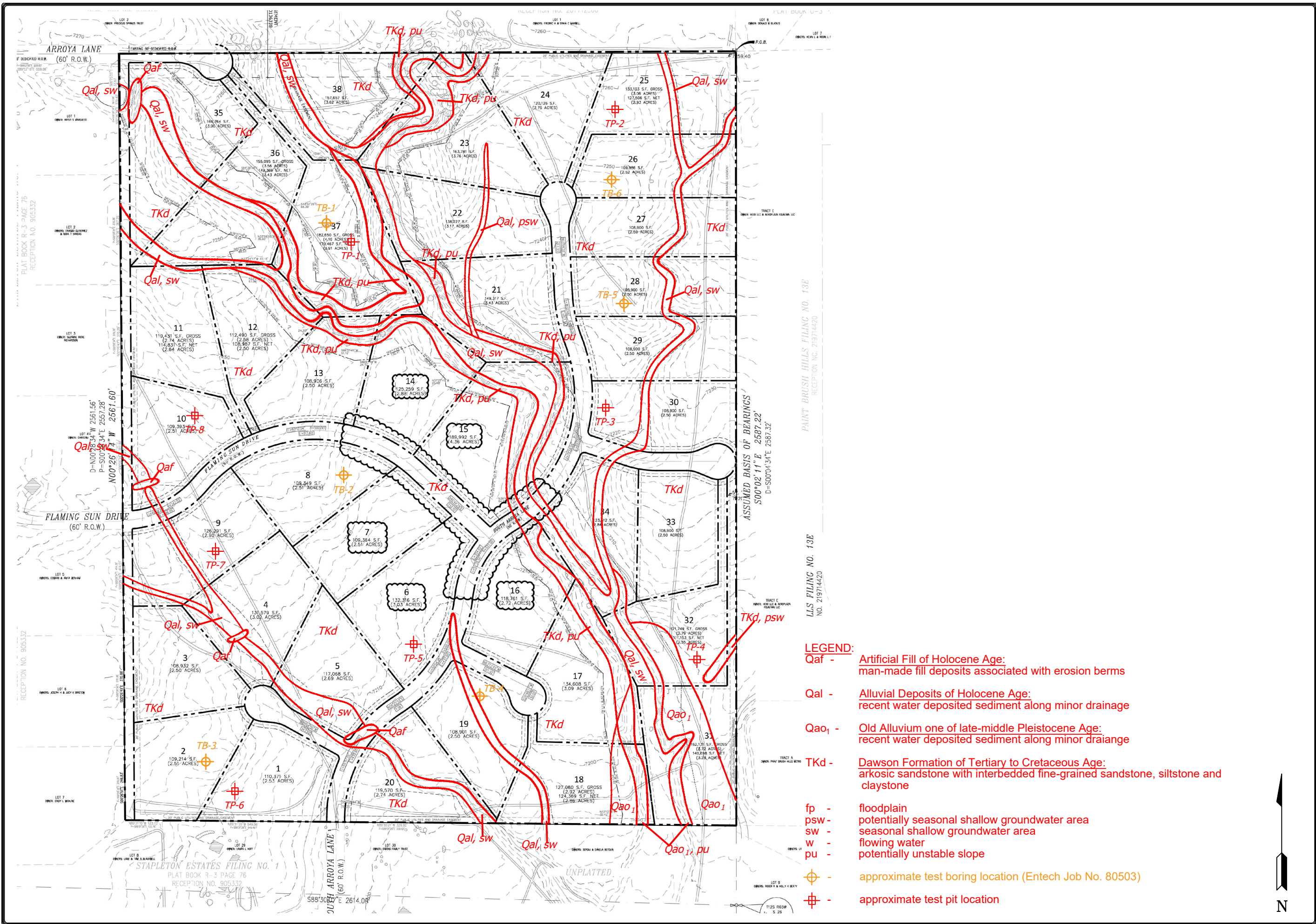
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DATE:

JOB NO.:  
212684

FIG NO.:  
5





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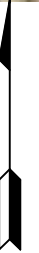
ENGINEERING, INC.

505 ELKTON DRIVE  
COLORADO SPRINGS, CO. 80907

GEOLOGY/ENGINEERING GEOLOGY MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

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DATE 11/17/21
SCALE AS SHOWN
JOB NO. 212684
FIGURE No. 6





N



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COLORADO SPRINGS, CO. 80907 (719) 531-5599

FLOODPLAIN MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

DRAWN:  
JHR

DATE:  
11/8/21

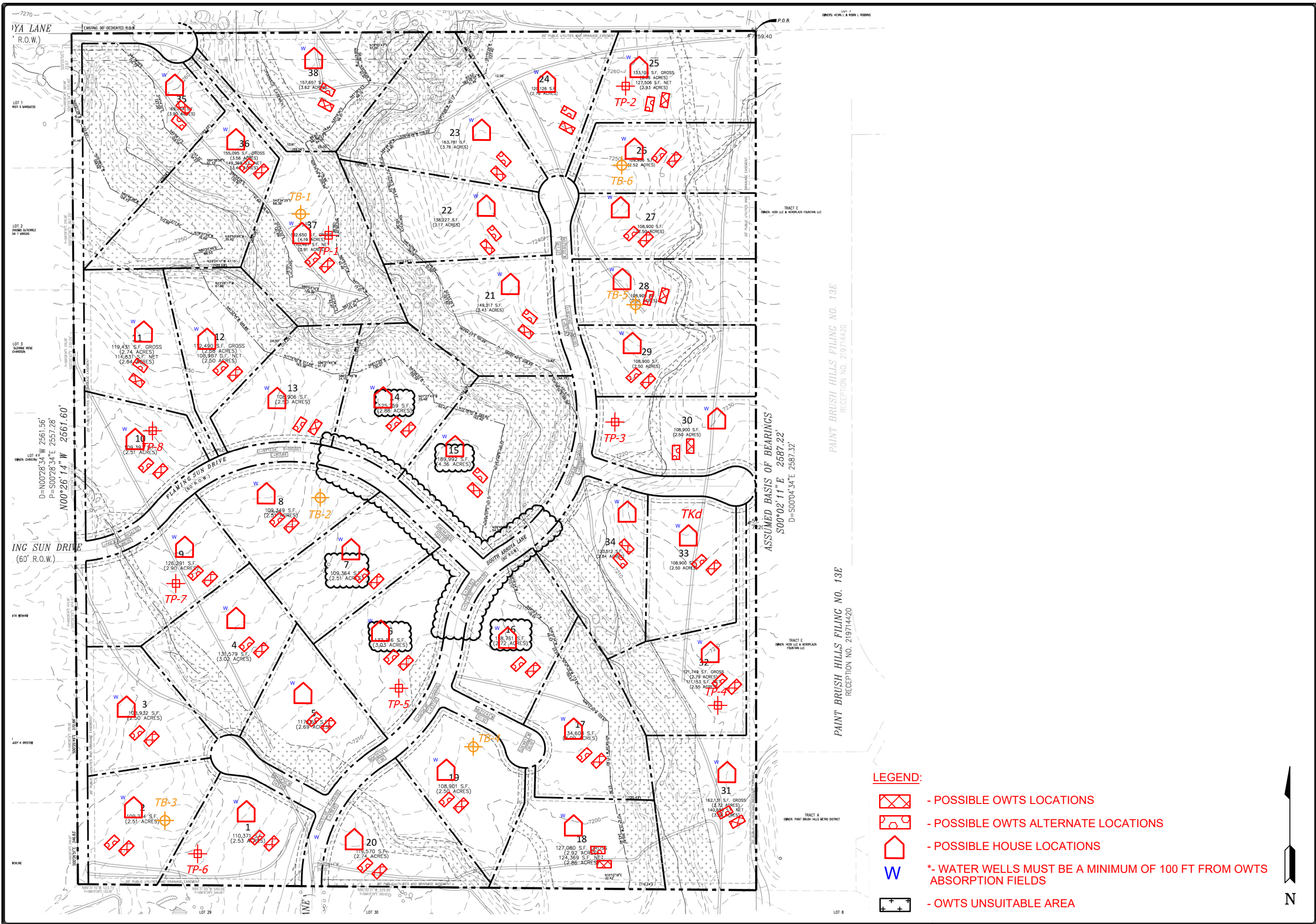
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JOB NO.:  
212684

FIG NO.:  
7





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505 ELKTON DRIVE  
COLORADO SPRINGS, CO. 80907  
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SEPTIC SUITABILITY MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

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CHECKED LLL
DATE 11/17/21
SCALE AS SHOWN
JOB NO. 212684
FIGURE No. 8

## **APPENDIX A: Site Photographs**





**Looking west from the  
northeastern portion of  
the site.**

October 12, 2021



**Looking south from  
northeastern portion of  
the site.**

October 12, 2021





**Looking southeast  
from the north-central  
portion of the site.**

October 12, 2021



**Looking southeast  
along drainage with  
potentially unstable  
slopes.**

October 12, 2021





**Looking north from the  
southeastern portion  
of the site.**

October 12, 2021



**Looking west from  
the southeastern  
portion of the site.**

October 12, 2021





**Looking east from the  
west-central portion of  
the site.**

October 12, 2021



**Looking south from  
the western portion of  
the site.**

October 12, 2021

## APPENDIX B: Test Pit Logs

TEST PIT NO. 1  
DATE EXCAVATED 10/13/2021  
Job # 212684

TEST PIT NO. 2  
DATE EXCAVATED 10/13/2021  
CLIENT PT Eagle View  
LOCATION Arroya Lane and Raygor Road

REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
topsoil, 0-8 inches	1			bl	m	4
sandy clay, Fine to medium grained, brown, moist	2					
	3			gr	w	2A
sandy loam, fine to coarse grained, brown, moist	4					
	5					
	6			gr	s	2
sandy loam, fine to coarse grained, brown, moist	7					
	8					
	9					
	10					

REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
Refusal @ 5 feet						
topsoil, 0-12 inches	1					
	2			gr	m	2
sandy loam, fine to coarse grained, brown, moist	3					
	4			ma		3A
sandy clay loam, sandstone fine to coarse grained, brown moist	5					
	6					
	7					
	8					
	9					
	10					

#### Soil Structure Shape

granular - gr  
platy - pl  
blocky - bl  
prismatic - pr  
single grain - sg  
massive - ma

#### Soil Structure Grade

weak - w  
moderate - m  
strong - s  
loose - l



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**ENGINEERING, INC.**

505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

#### TEST PIT LOG

DRAWN:

DATE:

CHECKED:

DATE:

JOB NO.:  
212684

FIG NO.:  
B-1

TEST PIT NO. 3  
DATE EXCAVATED 10/13/2021  
Job # 212684

TEST PIT NO. 4  
DATE EXCAVATED 10/13/2021  
CLIENT PT Eagle View  
LOCATION Arroya Lane and Raygor Road

REMARKS

Redoxomorphic  
Features @  
~4.5feet

topsoil 0-6 inches

sandy loam, fine to coarse  
grained, brown, moist

sandy clay, fine to medium  
grained, brown, moist

sandy clay loam, fine to  
coarse grained, brown, moist

Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
1			gr	m	2
2					
3			ma		4A
4					
5			gr	m	3
6					
7					
8					
9					
10					

REMARKS

topsoil 0-12 inches

sandy loam, fine to coarse  
grained, brown, moist

sandy clay loam, fine to  
coarse grained, brown, moist

Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
1					
2			gr	m	2
3					
4					
5			ma		3A
6					
7					
8					
9					
10					

Soil Structure Shape

granular - gr  
platy - pl  
blocky - bl  
prismatic - pr  
single grain - sg  
massive - ma

Soil Structure Grade

weak - w  
moderate - m  
strong - s  
loose - l



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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

TEST PIT LOG

DRAWN:

DATE:

CHECKED:  
LL

DATE:

JOB NO.:  
212684

FIG NO.:

B-2

TEST PIT NO. 5  
DATE EXCAVATED 10/13/2021  
Job # 212684

TEST PIT NO. 6  
DATE EXCAVATED 10/13/2021  
CLIENT PT Eagle View  
LOCATION Arroya Lane and Raygor Road

REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type	REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
topsoil 0-18 inches	1						Redoxomorphic Features @ ~3 feet	1					
sandy loam, fine to coarse grained, brown, moist	2			ma		2A	sandy clay, fine to medium grained, brown, moist	2			gr	s	4
sandy loam, fine to coarse grained, brown, moist	3						sandy loam, fine to coarse grained, brown, moist	3			gr	m	2
	4			gr	s	2	sandy clay, fine to medium grained, brown, moist	4			bl	m	4
	5			gr	m	2		5					
	6						sandy clay, fine to medium grained, light brown, moist	6			ma		4A
	7							7					
	8							8					
	9							9					
	10							10					

Soil Structure Shape

granular - gr  
platy - pl  
blocky - bl  
prismatic - pr  
single grain - sg  
massive - ma

Soil Structure Grade

weak - w  
moderate - m  
strong - s  
loose - l



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COLORADO SPRINGS, COLORADO 80907

**TEST PIT LOG**

DRAWN:

DATE:

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DATE:

LLL

JOB NO.:  
212684

FIG NO.:

B-3



TEST PIT NO. 7  
DATE EXCAVATED 10/13/2021  
Job # 212684

TEST PIT NO. 8  
DATE EXCAVATED 10/13/2021  
CLIENT PT Eagle View  
LOCATION Arroya Lane and Raygor Road

REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type	REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
topsoil 0-6 inches	1						Refusal @ 7 feet	1					
sandy clay loam, fine to coarse grained, brown, moist	2			gr	m	3	topsoil 0-6 inches	2			gr	m	2
sandy loam, fine to coarse grained, brown, moist	3			gr	m	2	sandy loam, fine to coarse grained, brown, moist	3					
	4							4			ma		4A
sandy clay loam, fine to coarse grained, brown, moist	5			gr	m	3	sandy clay, fine to medium grained, brown, moist	5					
	6						Bedrock @ 5 feet	6					
	7							7					
	8							8					
	9							9					
	10							10					

Soil Structure Shape

granular - gr  
platy - pl  
blocky - bl  
prismatic - pr  
single grain - sg  
massive - ma

Soil Structure Grade

weak - w  
moderate - m  
strong - s  
loose - l



**ENTECH  
ENGINEERING, INC.**

505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

**TEST PIT LOG**

DRAWN:

DATE:

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LLC

DATE:

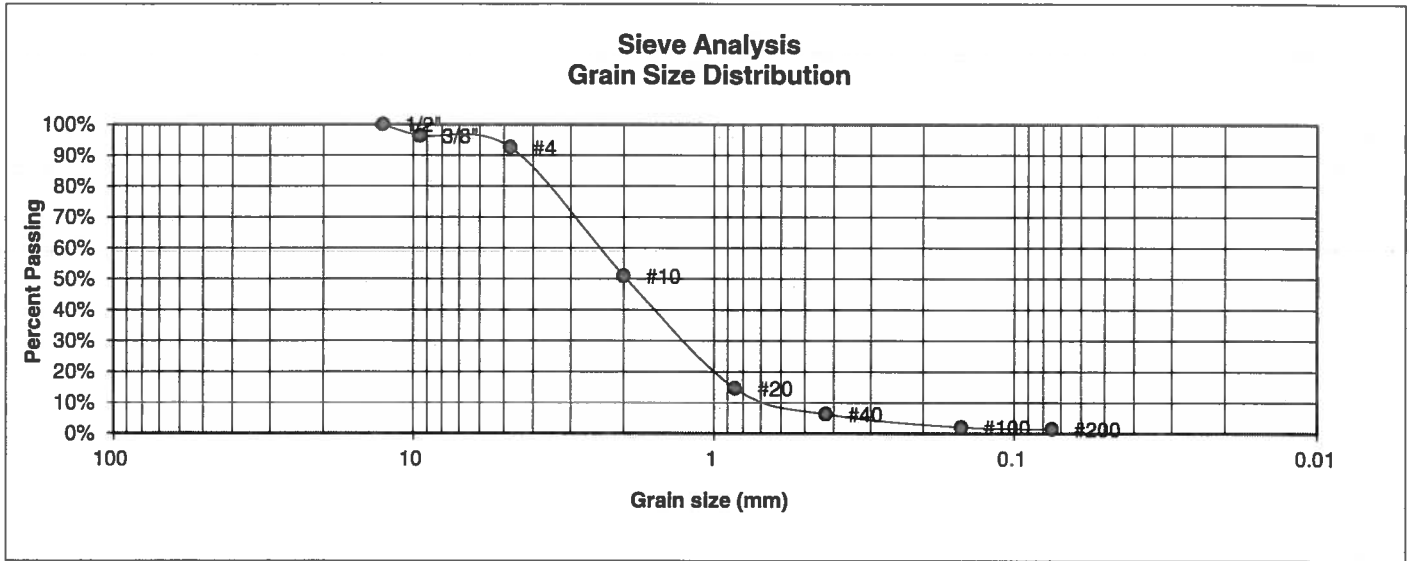
JOB NO.:  
212684

FIG NO.:  
B-4



## APPENDIX C: Laboratory Test Results

<u>UNIFIED CLASSIFICATION</u>	SW	<u>CLIENT</u>	PT EAGLEVIEW
<u>SOIL TYPE #</u>	1	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	TP-1	<u>JOB NO.</u>	212684
<u>DEPTH (FT)</u>	3	<u>TEST BY</u>	BL



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	96.3%
4	92.5%
10	51.0%
20	14.6%
40	6.3%
100	2.1%
200	1.4%

<u>Atterberg Limits</u>
Plastic Limit
Liquid Limit
Plastic Index
<u>Swell</u>
Moisture at start
Moisture at finish
Moisture increase
Initial dry density (pcf)
Swell (psf)



**ENTECH  
ENGINEERING, INC.**  
505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

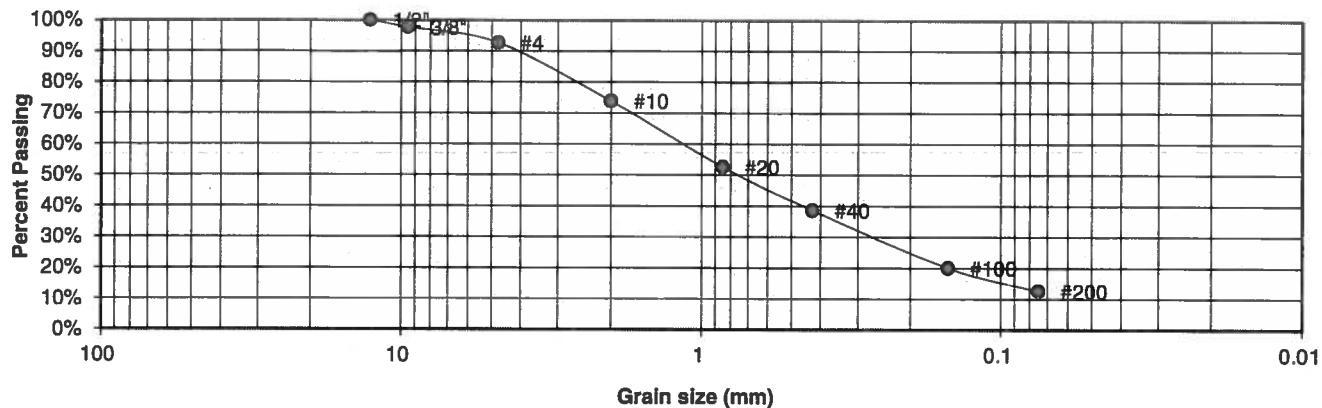
**LABORATORY TEST  
RESULTS**

DRAWN:	DATE:	CHECKED: LLC	DATE:
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JOB NO.:  
212684  
  
FIG NO.:  
C-1

<u>UNIFIED CLASSIFICATION</u>	SM	<u>CLIENT</u>	PT EAGLEVIEW
<u>SOIL TYPE #</u>	1	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	TP-2	<u>JOB NO.</u>	212684
<u>DEPTH (FT)</u>	1-2	<u>TEST BY</u>	BL

### Sieve Analysis Grain Size Distribution



U.S.  
Sieve #

Percent  
Finer

3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	97.7%
4	92.7%
10	73.8%
20	52.6%
40	38.6%
100	20.0%
200	12.6%

Atterberg

Limits

Plastic Limit

Liquid Limit

Plastic Index

Swell

Moisture at start

Moisture at finish

Moisture increase

Initial dry density (pcf)

Swell (psf)



**ENTECH  
ENGINEERING, INC.**

505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:

DATE:

LL

JOB NO.:  
212684

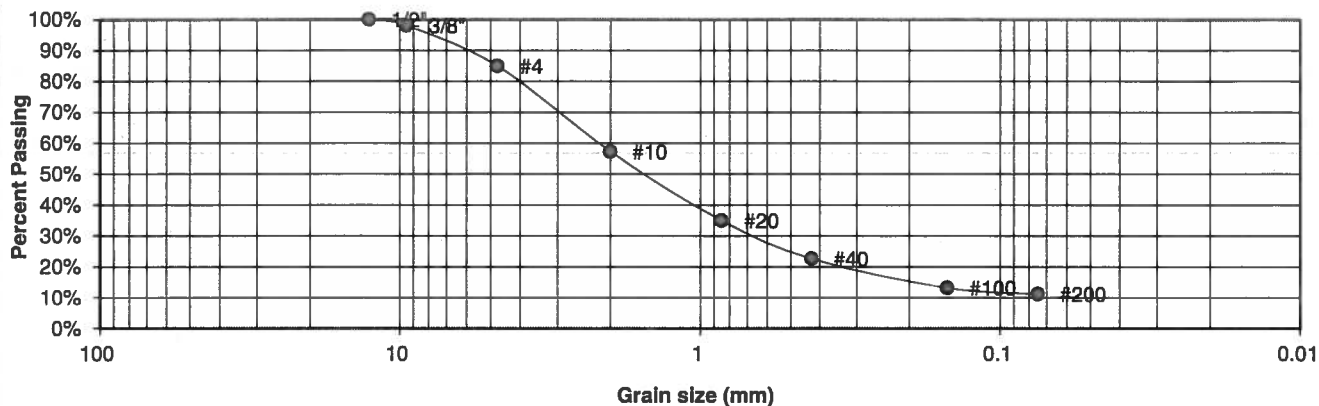
FIG NO.:

C-2

<b>UNIFIED CLASSIFICATION</b>	SM-SW
<b>SOIL TYPE #</b>	1
<b>TEST BORING #</b>	TP-4
<b>DEPTH (FT)</b>	3-4

<b>CLIENT</b>	PT EAGLEVIEW
<b>PROJECT</b>	ARROYA AND RAYGOR
<b>JOB NO.</b>	212684
<b>TEST BY</b>	BL

### Sieve Analysis Grain Size Distribution



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	98.1%
4	85.0%
10	57.3%
20	35.0%
40	22.6%
100	13.2%
200	11.2%

**Atterberg  
Limits**  
Plastic Limit  
Liquid Limit  
Plastic Index

**Swell**  
Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



**ENTECH  
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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:  
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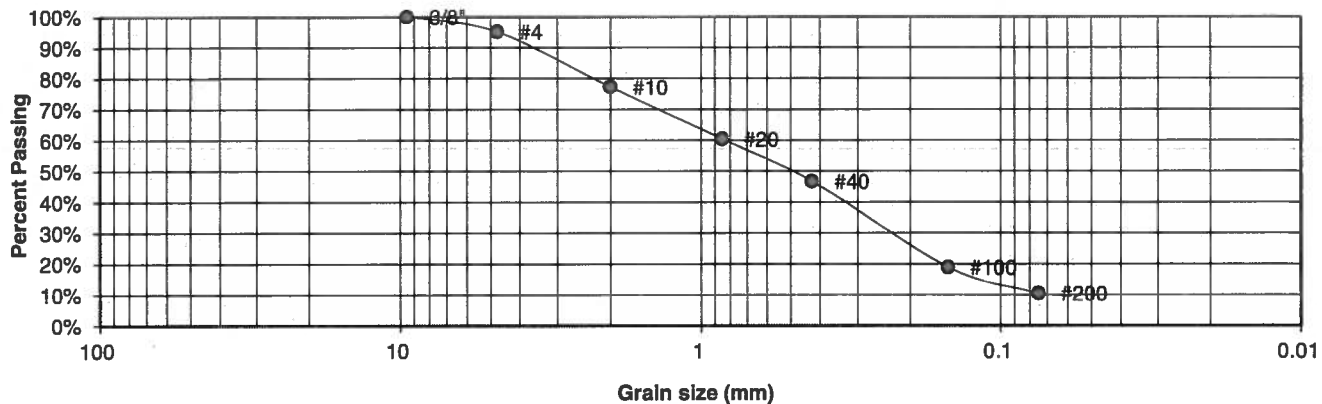
DATE:

JOB NO.:  
212684

FIG NO.:  
C-3

UNIFIED CLASSIFICATION	SM-SW	CLIENT	PT EAGLEVIEW
SOIL TYPE #	1	PROJECT	ARROYA AND RAYGOR
TEST BORING #	TP-5	JOB NO.	212684
DEPTH (FT)	1-2	TEST BY	BL

### Sieve Analysis Grain Size Distribution



U.S.  
Sieve #

3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

Percent  
Finer

100.0%  
95.1%  
77.2%  
60.4%  
46.7%  
18.9%  
10.5%

Atterberg

Limits

Plastic Limit

Liquid Limit

Plastic Index

Swell

Moisture at start

Moisture at finish

Moisture increase

Initial dry density (pcf)

Swell (psf)



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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:

LLL

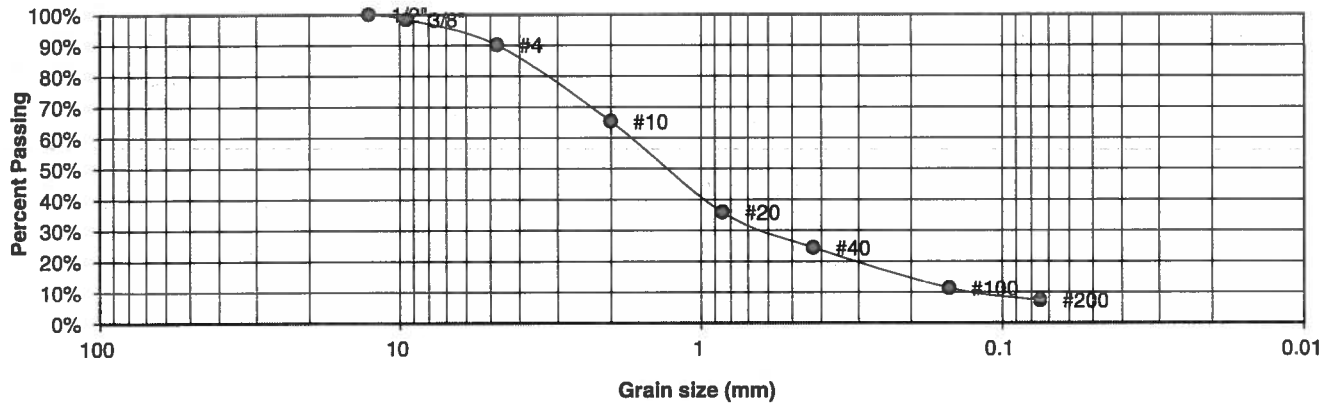
DATE:

JOB NO.:  
212684

FIG NO.:  
C-4

UNIFIED CLASSIFICATION	SM-SW	CLIENT	PT EAGLEVIEW
SOIL TYPE #	1	PROJECT	ARROYA AND RAYGOR
TEST BORING #	TP-5	JOB NO.	212684
DEPTH (FT)	3	TEST BY	BL

### Sieve Analysis Grain Size Distribution



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	98.4%
4	90.1%
10	65.3%
20	35.9%
40	24.4%
100	11.3%
200	7.3%

Atterberg  
Limits  
Plastic Limit  
Liquid Limit  
Plastic Index

Swell  
Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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### LABORATORY TEST RESULTS

DRAWN:

DATE:

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LL

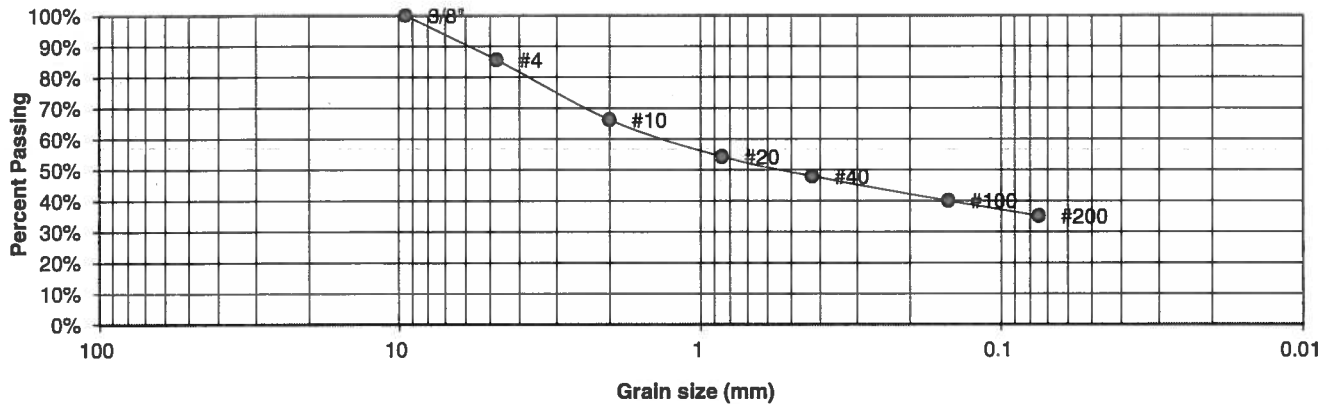
DATE:

JOB NO.:  
212684

FIG NO.:  
C-5

UNIFIED CLASSIFICATION	SC	CLIENT	PT EAGLEVIEW
SOIL TYPE #	1	PROJECT	ARROYA AND RAYGOR
TEST BORING #	TP-6	JOB NO.	212684
DEPTH (FT)	1.5	TEST BY	BL

**Sieve Analysis  
Grain Size Distribution**



U.S.  
Sieve #

Percent  
Finer

3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	85.6%
10	66.2%
20	54.3%
40	47.9%
100	40.0%
200	35.1%

Atterberg

Limits

Plastic Limit

Liquid Limit

Plastic Index

Swell

Moisture at start

Moisture at finish

Moisture increase

Initial dry density (pcf)

Swell (psf)



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**LABORATORY TEST  
RESULTS**

DRAWN:

DATE:

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DATE:

LLL

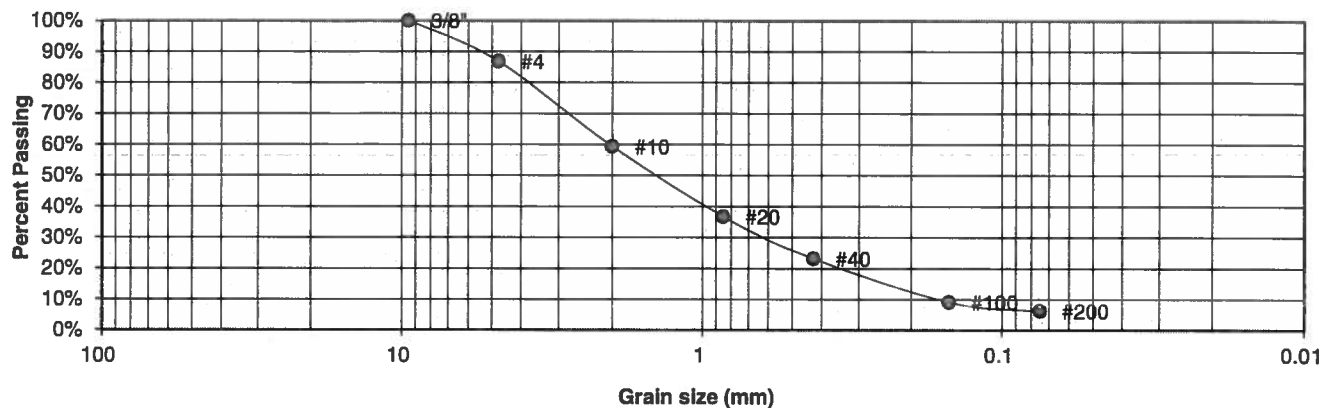
JOB NO.:  
212684

FIG NO.:

C-6

<u>UNIFIED CLASSIFICATION</u>	SM-SW	<u>CLIENT</u>	PT EAGLEVIEW
<u>SOIL TYPE #</u>	1	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	TP-6	<u>JOB NO.</u>	212684
<u>DEPTH (FT)</u>	2.5	<u>TEST BY</u>	BL

### Sieve Analysis Grain Size Distribution



U.S.  
Sieve #

3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

Percent  
Finer

100.0%  
86.8%  
59.4%  
36.7%  
23.1%  
9.1%  
6.3%

Atterberg  
Limits

Plastic Limit  
Liquid Limit  
Plastic Index

Swell

Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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### LABORATORY TEST RESULTS

DRAWN:

DATE:

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DATE:

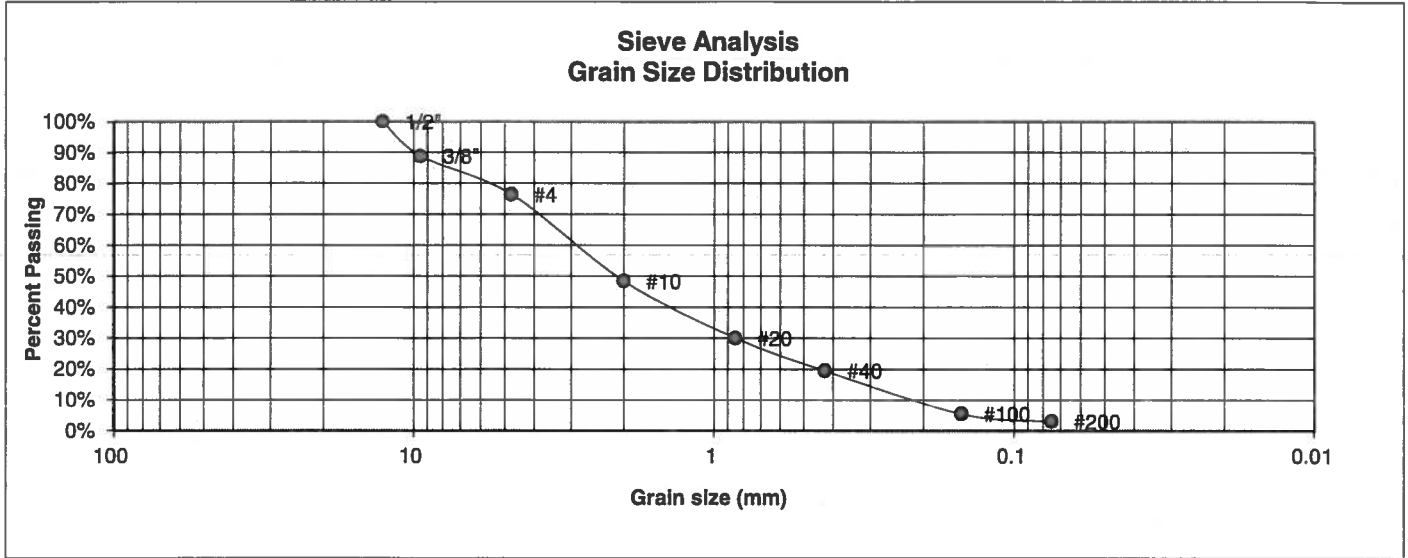
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JOB NO.:  
212684

FIG NO.:  
C-7



UNIFIED CLASSIFICATION	SW	CLIENT	PT EAGLEVIEW
SOIL TYPE #	1	PROJECT	ARROYA AND RAYGOR
TEST BORING #	TP-7	JOB NO.	212684
DEPTH (FT)	2	TEST BY	BL



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	88.8%
4	76.4%
10	48.4%
20	30.0%
40	19.5%
100	5.6%
200	3.1%

Atterberg  
Limits  
Plastic Limit  
Liquid Limit  
Plastic Index

Swell  
Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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**LABORATORY TEST  
RESULTS**

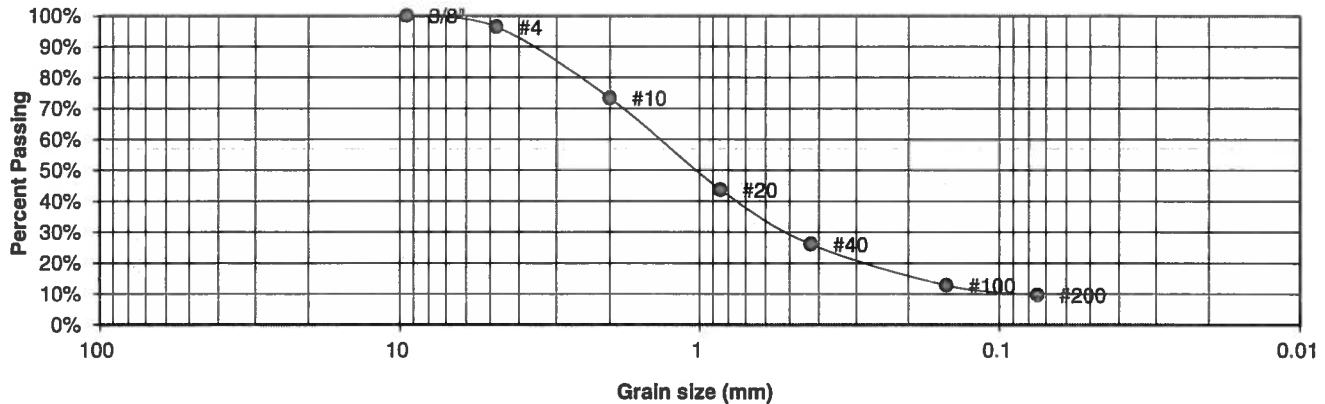
DRAWN:	DATE:	CHECKED: <i>LL</i>	DATE:
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JOB NO.:  
212684

FIG NO.:  
C-8

UNIFIED CLASSIFICATION	SM-SW	CLIENT	PT EAGLEVIEW
SOIL TYPE #	1	PROJECT	ARROYA AND RAYGOR
TEST BORING #	TP-7	JOB NO.	212684
DEPTH (FT)	5	TEST BY	BL

### Sieve Analysis Grain Size Distribution



U.S.  
Sieve #

3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

Percent  
Finer

100.0%  
96.3%  
73.3%  
43.7%  
26.1%  
12.8%  
9.6%

Atterberg  
Limits

Plastic Limit  
Liquid Limit  
Plastic Index

Swell

Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

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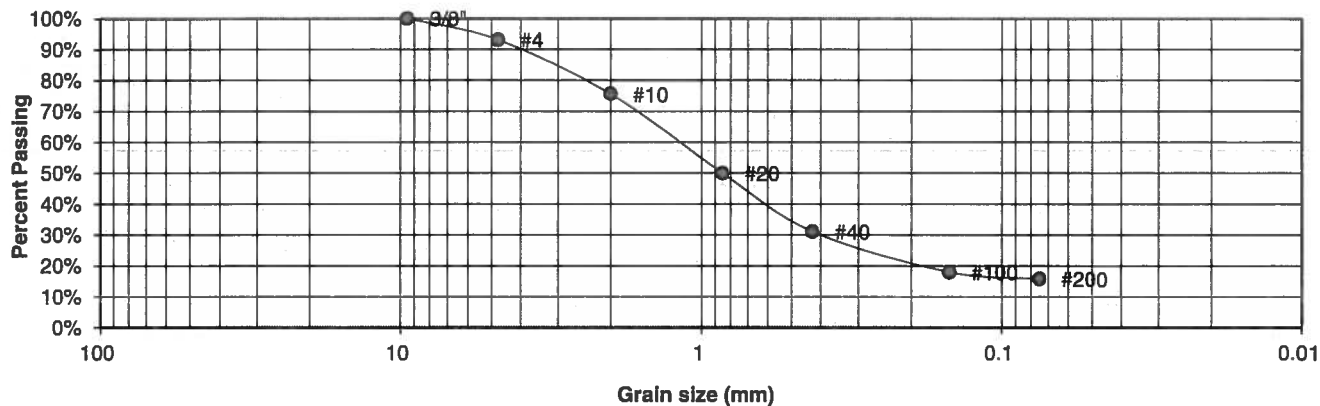
DATE:

JOB NO.:  
212684

FIG NO.:  
C-9

<u>UNIFIED CLASSIFICATION</u>	SM	<u>CLIENT</u>	PT EAGLEVIEW
<u>SOIL TYPE #</u>	1	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	TP-8	<u>JOB NO.</u>	212684
<u>DEPTH (FT)</u>	4	<u>TEST BY</u>	BL

### Sieve Analysis Grain Size Distribution



U.S.  
Sieve #

3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

Percent  
Finer

100.0%  
93.1%  
75.6%  
49.9%  
31.0%  
18.0%  
15.8%

Atterberg

Limits

Plastic Limit

Liquid Limit

Plastic Index

Swell

Moisture at start

Moisture at finish

Moisture increase

Initial dry density (pcf)

Swell (psf)



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COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:

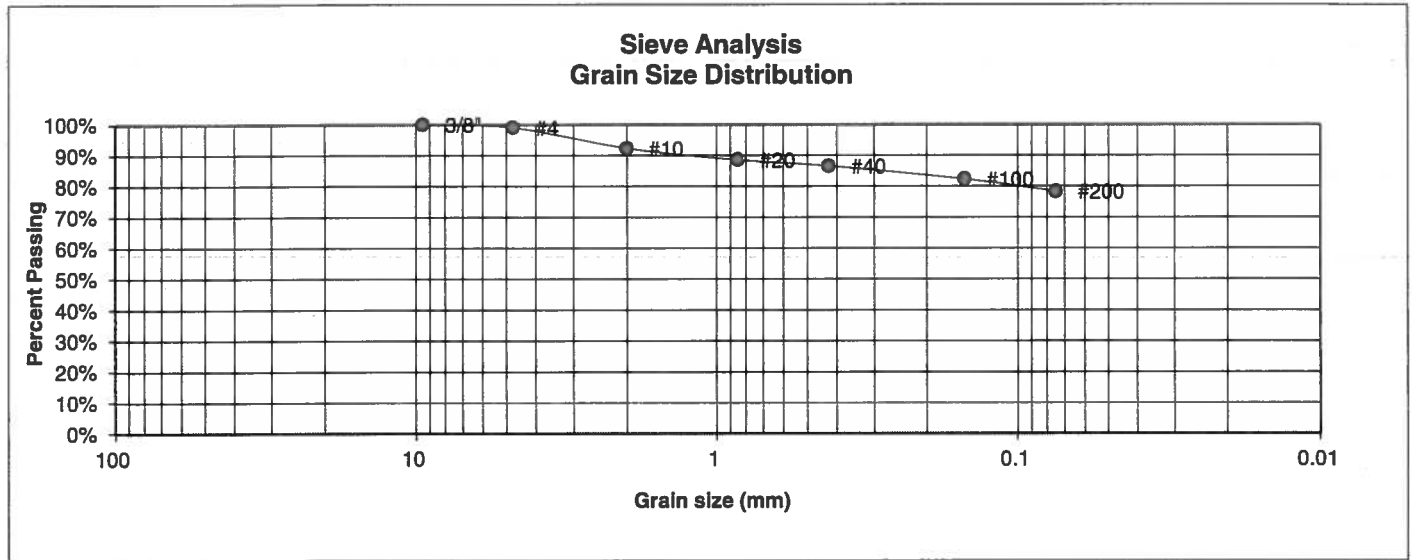
DATE:

JOB NO.:  
212684

FIG NO.:

C-10

<u>UNIFIED CLASSIFICATION</u>	CL	<u>CLIENT</u>	PT EAGLEVIEW
<u>SOIL TYPE #</u>	2	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	TP-3	<u>JOB NO.</u>	212684
<u>DEPTH (FT)</u>	2-3	<u>TEST BY</u>	BL



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.0%
10	92.3%
20	88.6%
40	86.5%
100	82.3%
200	78.3%

Atterberg  
Limits  
Plastic Limit  
Liquid Limit  
Plastic Index

Swell  
Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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### LABORATORY TEST RESULTS

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DATE:

JOB NO.:  
212684

FIG NO.:  
*C-4*

## **APPENDIX D: Soil Survey Descriptions**

## El Paso County Area, Colorado

### 19—Columbine gravelly sandy loam, 0 to 3 percent slopes

#### Map Unit Setting

*National map unit symbol:* 367p

*Elevation:* 6,500 to 7,300 feet

*Mean annual precipitation:* 14 to 16 inches

*Mean annual air temperature:* 46 to 50 degrees F

*Frost-free period:* 125 to 145 days

*Farmland classification:* Not prime farmland

#### Map Unit Composition

*Columbine and similar soils:* 97 percent

*Minor components:* 3 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Columbine

##### Setting

*Landform:* Flood plains, fan terraces, fans

*Down-slope shape:* Linear

*Across-slope shape:* Linear

*Parent material:* Alluvium

##### Typical profile

*A - 0 to 14 inches:* gravelly sandy loam

*C - 14 to 60 inches:* very gravelly loamy sand

##### Properties and qualities

*Slope:* 0 to 3 percent

*Depth to restrictive feature:* More than 80 inches

*Drainage class:* Well drained

*Runoff class:* Very low

*Capacity of the most limiting layer to transmit water (Ksat):* High to very high (5.95 to 19.98 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water supply, 0 to 60 inches:* Very low (about 2.5 inches)

##### Interpretive groups

*Land capability classification (irrigated):* 4e

*Land capability classification (nonirrigated):* 6e

*Hydrologic Soil Group:* A

*Ecological site:* R049XY214CO - Gravelly Foothill

*Hydric soil rating:* No

#### Minor Components

##### Fluvaquentic haplaquolls

*Percent of map unit:* 1 percent

*Landform:* Swales  
*Hydric soil rating:* Yes

**Other soils**

*Percent of map unit:* 1 percent  
*Hydric soil rating:* No

**Pleasant**

*Percent of map unit:* 1 percent  
*Landform:* Depressions  
*Hydric soil rating:* Yes

## Data Source Information

Soil Survey Area: El Paso County Area, Colorado  
Survey Area Data: Version 19, Aug 31, 2021

## El Paso County Area, Colorado

### 71—Pring coarse sandy loam, 3 to 8 percent slopes

#### Map Unit Setting

*National map unit symbol:* 369k

*Elevation:* 6,800 to 7,600 feet

*Farmland classification:* Not prime farmland

#### Map Unit Composition

*Pring and similar soils:* 85 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Pring

##### Setting

*Landform:* Hills

*Landform position (three-dimensional):* Side slope

*Down-slope shape:* Linear

*Across-slope shape:* Linear

*Parent material:* Arkosic alluvium derived from sedimentary rock

##### Typical profile

*A - 0 to 14 inches:* coarse sandy loam

*C - 14 to 60 inches:* gravelly sandy loam

##### Properties and qualities

*Slope:* 3 to 8 percent

*Depth to restrictive feature:* More than 80 inches

*Drainage class:* Well drained

*Runoff class:* Low

*Capacity of the most limiting layer to transmit water (Ksat):* High  
(2.00 to 6.00 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water supply, 0 to 60 inches:* Low (about 6.0 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 3e

*Hydrologic Soil Group:* B

*Ecological site:* R048AY222CO - Loamy Park

*Hydric soil rating:* No

#### Minor Components

##### Pleasant

*Percent of map unit:*

*Landform:* Depressions

*Hydric soil rating:* Yes



**Other soils**

*Percent of map unit:*

*Hydric soil rating:* No

**Data Source Information**

Soil Survey Area: El Paso County Area, Colorado

Survey Area Data: Version 19, Aug 31, 2021

## *Appendix C*



**ENTECH**  
ENGINEERING, INC.

505 ELKTON DRIVE  
COLORADO SPRINGS, CO 80907  
PHONE (719) 531-5599  
FAX (719) 531-5238

**SOIL, GEOLOGY, AND GEOLOGIC HAZARD STUDY  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYOR ROAD  
PARCEL NO. 52260-00-001 & 52260-00-002  
EL PASO COUNTY, COLORADO**

Prepared for

**PT Eagleview, LLC**  
1865 Woodmoor Drive, Suite 100  
Monument, Colorado 80132

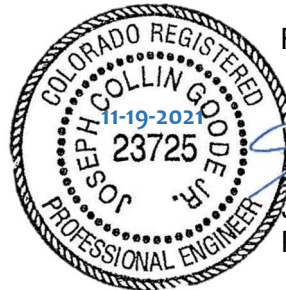
Attn: Joseph DesJardin

November 19, 2021

Respectfully Submitted,

ENTECH ENGINEERING, INC.

Logan L. Langford, P.G.  
Geologist



Reviewed by:

Joseph C. Goode, Jr., P.E.  
President

LLL/jhr

Encl.

Entech Job No. 212684  
AAprojects/2021/212684 countysoil/geo

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Table 2: Summary of Tactile Test Pits Results

### FIGURES

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Figure 2: USGS Map

Figure 3: Site Plan/Testing Location Map

Figure 4: Soil Survey Map

Figure 5: Falcon NW Quadrangle Geology Map

Figure 6: Geology Map/Engineering Geology

Figure 7: Floodplain Map

Figure 8: Typical Perimeter Drain Details

APPENDIX A: Site Photographs

APPENDIX B: Test Pit Logs

APPENDIX C: Laboratory Test Results

APPENDIX D: Test Boring Logs & Laboratory Testing Job No. 80503

APPENDIX E: Soil Survey Descriptions

## **1.0 SUMMARY**

### ***Project Location***

The project site lies in portions of the NW $\frac{1}{4}$  and SW $\frac{1}{4}$  of Section 26, Township 12 South, Range 65 West of the 6<sup>th</sup> Principal Meridian in the northern portion of El Paso County, Colorado. The site is located approximately 14 miles southeast of Monument, Colorado, southwest of Burgess Road and Goodson Road.

### ***Project Description***

Total acreage involved in the subdivision is approximately 124 acres. The proposed development consists of thirty-eight rural residential lots. The development will utilize individual wells and on-site wastewater treatment systems.

### ***Scope of Report***

This report presents the results of our geologic evaluation, and treatment of engineering geologic hazard study.

### ***Land Use and Engineering Geology***

This site was found to be suitable for the proposed development. Areas were encountered where the geologic conditions will impose some constraints on development and land use. These include areas of seasonally shallow groundwater, potential seasonally shallow groundwater, drainage areas, artificial fill, expansive soils, and potentially unstable slopes along the main drainages on site. Based on the proposed development plan, it appears that these areas will have some impact on the development. These conditions will be discussed in greater detail in the report.

In general, it is our opinion that the development can be achieved if the observed geologic conditions on site are either avoided or properly mitigated. All recommendations are subject to the limitations discussed in the report.

## **2.0 GENERAL SITE CONDITIONS AND PROJECT DESCRIPTION**

The site is located in portions of the NW¼ and SW¼ of Section 26, Township 12 South, Range 65 West of the 6th Principal Meridian in the northern portion of El Paso County, Colorado. The site is located approximately 14 miles southeast of Monument, Colorado, southwest of Goodson Road and Burgess Road. The location of the site is as shown on the Vicinity Map, Figure 1.

The topography of the site varies from gently to moderately sloping generally to the south and southeast with some steeper slopes along the drainages on-site. The drainages on site flow in southeasterly and southerly directions through the property. No water was observed flowing in any of the drainages at the time of this investigation. The site boundaries are indicated on the USGS Map, Figure 2. Previous land uses have included grazing and pasture land. The site contains primarily field grasses, and weeds with areas of scattered pine trees and willow along the drainages. Site photographs are included in Appendix D. The approximate locations and directions of the photographs are indicated on Figure 3.

Total acreage involved in the proposed development is approximately 124 acres with thirty-eight rural residential lots. The proposed residential lots are approximately 2.5 to 4.2 acres each. The area will be serviced by individual wells and on-site wastewater treatment systems. The proposed Site Plan/Testing Location Map is presented in Figure 3.

The site was previously investigated as part of a Soil, Geology, and Wastewater Study, performed by Entech Job No. 80503 (Reference 1). Six (6) previously drilled test borings, and eight (8) percolation test were performed on the site to determine general suitability of the site for the use of on-site wastewater treatment systems. The previous report/investigation was utilized to evaluate the site as part of this investigation.

### **3.0 SCOPE OF THE REPORT**

The scope of this report includes:

- A general geologic analysis utilizing published geologic data. Detailed site-specific mapping will be conducted to obtain general information in respect to major geographic and geologic features, geologic descriptions and their effects on the development of the property.

### **4.0 FIELD INVESTIGATION**

Our field investigation consisted of the preparation of a geologic map of any bedrock features and significant surficial deposits. The Natural Resource Conservation Service (NRCS), previously the Soil Conservation Service (SCS) survey was also reviewed to evaluate the site. The position of mappable units within the subject property are shown on the Geologic Map. Our mapping procedures involved both field reconnaissance and measurements and air photo reconnaissance and interpretation. The same mapping procedures have also been utilized to produce the Geology/Engineering Geology Map which identified pertinent geologic conditions affecting development. The field mapping was performed by personnel of Entech Engineering, Inc. for the previous report referenced above (Reference, 1) and verified on October 13, 2021.

Test borings from the Soil, Geology, and Wastewater Study, performed by Entech Job No. 80503, were used as part of the investigation. Six (6) test borings, and eight (8) percolation test were previously performed on the site to determine general suitability of the site for residential construction and the use of on-site wastewater treatment systems respectively. Eight (8) tactile test pits were recently excavated and evaluated by Entech Engineering, Inc. personnel due to the current on-site wastewater regulations. The locations of the test borings, and test pits are indicated on the Site Plan/Testing Location Map, Figure 3. The Test Pit and Test Boring Logs are presented in Appendix B. Results of this testing will be discussed later in this report.

Laboratory testing was also performed on some of the soils to classify and determine the soils engineering characteristics. Laboratory tests included grain-size analysis, ASTM D-422, and Atterberg Limits, ASTM D-4318. Results of the laboratory testing are included in Appendix C. A

Summary of Laboratory Test Results is presented in Table 1. A Summary of Laboratory Test Results, Test Boring Logs from the previous investigation are included in Appendix D.

## **5.0 SOIL, GEOLOGY AND ENGINEERING GEOLOGY**

### **5.1 General Geology**

Physiographically, the site lies in the western portion of the Great Plains Physiographic Province. Approximately 13 miles to the west is a major structural feature known as the Rampart Range Fault. This fault marks the boundary between the Great Plains Physiographic Province and the Southern Rocky Mountain Province. The site exists within the southeastern edge of a large structural feature known as the Denver Basin. Bedrock in the area tends to be very gently dipping in an easterly direction. The rocks in the area of the site are sedimentary in nature, and typically Tertiary to Upper Cretaceous in age. The bedrock underlying the site consists of the Dawson Arkose Formation. Overlying this formation are unconsolidated deposits of residual, colluvial, man-made, and alluvial soils of the Quaternary Age. The residual soils are produced by the in-situ action of weathering of the bedrock on site. Some colluvial soils exist which are deposited by gravity and sheetwash. The alluvial soils were deposited by water in the major drainages on site. Man-made soils exist as fill berms and earthen dams. The site's stratigraphy will be discussed in more detail in Section 5.3.

### **5.2 Soil Conservation Survey**

The Natural Resource Conservation Service (Reference 2), previously the Soil Conservation Service (Reference 3) has mapped two soil types on the site (Figure 4). In general, they vary from coarse sandy loam to gravelly sandy loam. The soils are described as follows:

<u>Type</u>	<u>Description</u>
19	Columbine Gravelly sandy loam, 0 to 3% slopes
71	Pring Coarse Sandy Loam, 3-8% slopes

Complete descriptions of each soil type are presented in Appendix D. The soils have generally been described to typically have slow to rapid permeabilities. The majority of the soils have



moderate to rapid permeabilities. Limitations described for local soils include shrink-swell potential. Roads may need to be designed to minimize frost-heave potential. Possible hazards with soil erosion are present on the site. The erosion potential can be controlled with vegetation, erosion control blankets, and waddles. The majority of the soils have been described to have moderate erosion hazards.

### **5.3 Site Stratigraphy**

The Falcon NW Quadrangle Geology Map is presented in Figure 5 (Reference 4). These maps in conjunction with site specific mapping were used to prepare the site Geology Map. The Geology Map prepared for the site is presented in Figure 6. Four mappable units were identified on this site which are identified as follows:

**Qaf Artificial Fill of Quaternary Age:** These are man-made fill deposits associated with berms and small earth dam embankments on-site.

**Qal Recent Alluvium of Quaternary Age:** These are recent stream deposits in the channels of the main drainages on site.

**Qao, Old Alluvium one of late middle - Pleistocene Age:** These deposits are light brown silty sands which contain an abundance of gravels. They commonly occur as stream terrace deposits above the valley floors. Old alluvium one may correlate with the Louviers Alluvium in the Denver area.

**Tkd Dawson Arkose of Tertiary to Cretaceous Age:** The Dawson formation consists of arkosic sandstone with interbedded lenses of ***fine-grained*** sandstone, claystone or siltstone. Typically, it is buff to light brown and light gray in color. Overlying the Dawson is a variable layer of colluvial and residual soil. These materials consist of silty to clayey sands and gravels deposited by the action of sheetwash and gravity. Some residual soils derived from the in-situ weathering of the bedrock on site exist in this mapping. These soils are overlying the Dawson Formation in many places on this site.

The soils listed above were mapped from site-specific mapping, the *Geologic Map of the NW Falcon Quadrangle* distributed by the Colorado Geological Survey in 2012 (Reference 4), and the *Geologic Map of the Denver 1° x 2° Quadrangle*, distributed by the US Geological Survey in 1981 (Reference 5). The Test Pits and Test Borings were also used in evaluating the site and are included in Appendices B and D. The Geology Map prepared for the site is presented in Figure 6.

#### **5.4 Soil Conditions**

The soils encountered in the test pits, test borings and the profile holes of the percolation tests can be grouped into three general soil types. The soils were classified using the Unified Soil Classification System (USCS). Several soil descriptors are results of the previously studied Soils, Geology, and Waste Water Study, performed by Entech Engineering, Inc. (Reference 1). Descriptions of the soil types are discussed as follows:

Soil Type 1: consists of slightly silty to silty and clayey sand (SW-SM, SC). The sands were encountered in the upper soil profile of nearly all of the profile holes and test borings. These soils were encountered at loose to dense states and moist conditions. The clayey sands are slightly to highly expansive. Swell pressures of 694 psf and 2233 psf were measured on the clayey sands. The slightly silty and silty sands are generally considered non-expansive.

Soil Type 2: is a sandy to silty clay soil (CL), encountered in the upper soil profile in Test Boring No. 2 and Profile Hole Nos. 2 and 4. The clays were encountered at soft to stiff consistencies and moist conditions. A swell pressure of 2060 psf was measured in the FHA Swell Test. This swell is in the high expansion range.

Soil Type 3: consists of silty to clayey sandstone bedrock (SC, SM-SC). The sandstone was encountered in all of the test borings and in Profile Hole Nos. 2, 3, 7 and 8 at depths ranging from the surface to 9 feet below the surface. The sandstone was encountered at very dense states and moist conditions. The clayey sandstone is slightly expansive. FHA Swell pressures of 425 psf and 447 psf were measured on the sandstone.

The Test Pit Logs are presented in Appendix B. Laboratory Test Results are presented in Appendix C. Test Pit Boring Logs and Laboratory Test Results from the previous investigation are presented in Appendix D (Reference, 1).

### **5.5 Groundwater**

Groundwater was not encountered in the test borings which were drilled to 10 to 15 feet. However, seasonally shallow groundwater evidence was encountered in Test Pit Nos. 3 and 6 at depths of 3 to 5 feet below ground surface. The remaining Test Pits which were excavated to 5 to 8 feet did not encounter water or signs of seasonally occurring groundwater. Areas of seasonal and potentially seasonal shallow groundwater have been mapped in low-lying areas and in the drainages on-site. These areas are discussed in the following section. Fluctuation in groundwater conditions may occur due to variations in rainfall and other factors not readily apparent at this time.

It should be noted that in the sandy materials on site, some groundwater conditions might be encountered due to the variability in the soil profile. Isolated sand and gravel layers within the soils, sometimes only a few feet in thickness and width, can carry water in the subsurface. Groundwater may also flow on top of the underlying bedrock. Builders and planners should be cognizant of the potential for the occurrence of such subsurface water features during construction on-site and deal with each individual problem as necessary at the time of construction.

## **6.0 ENGINEERING GEOLOGY – IDENTIFICATION AND MITIGATION OF GEOLOGIC HAZARDS**

As mentioned previously, detailed mapping has been performed on this site to produce a Geology/Engineering Geology Map (Figure 6). This map shows the location of various geologic conditions of which the developers should be cognizant during the planning, design and construction stages of the project. These hazards and the recommended mitigation techniques are as follows:

*psw    Potentially Seasonal High Groundwater Area (Constraint)*

In these areas, we would anticipate the potential for periodically high subsurface moisture conditions, frost heave potential and highly organic soils. The majority of these areas lie within minor drainage areas which can be avoided by the proposed development. Construction in any portions of these areas, if required, should follow these precautions.

Mitigation: In these locations, foundation in areas subject to severe frost heave potential should penetrate to a sufficient depth so as to discourage the formation of ice lenses beneath foundations. At this location and elevation, a foundation depth of 30 inches is recommended for frost protection. In areas where high subsurface moisture conditions are anticipated periodically, a subsurface perimeter drain will be necessary to help prevent the seepage of water into areas located below grade. Any grading in these areas should be done in a manner that directs surface flow around construction to avoid areas of ponded water. Areas of organic material will require removal before any filling is done. Additionally, septic systems are not recommended in these areas unless analyzed for the hazard of high groundwater. Groundwater may be at sufficient depth as to not affect septic fields or foundations. Further investigation is recommended prior to construction in these areas to further delineate groundwater depths.

*sw    Seasonal High Groundwater Area (Constraint)*

These are areas within the main drainages on site and behind earthen dams. Water was not observed flowing in these drainages at the time of this investigation, however, vegetation and soils observed indicate water is near or at the surface during periods of high moisture. Small areas of ponded water were observed in some of the drainages. These areas also contain frost heave potential and highly organic soils.

Mitigation: Because the majority of the areas mapped as seasonally wet lie within defined drainages, we do not recommend structures be built within these areas. Lots are of sufficient size that these areas can be avoided as building sites. Should foundations encroach on seasonal shallow groundwater areas, the recommendations for potentially seasonal shallow groundwater mitigation should be followed. Additionally, basements or septic fields would not be recommended in these areas. Septic fields should be located

a minimum of 25 feet away from the drainage areas. Any construction in these areas should be done in a manner that does not create ponded water. Where roadways or driveways cross drainages, adequately sized culverts should be installed. No areas of the site are mapped within any floodplain zones according to the FEMA Map No. 08041CO535G, Figure 10 (Reference 5). A 100-year floodplain along the main drainage on-site is indicated on Figures 3 and 9. Finished floors must be a minimum of one foot above the 100-year floodplain level. Specific floodplain locations and drainage studies are beyond the scope of this report.

ex *Expansive Soils (Constraint)*

Expansive soils were encountered in some of the test borings and profile holes of some of the percolation tests drilled on site. Additionally, the area of the site has been mapped as having moderate swell potential according to the *Map of Potentially Swelling Soil and Rock in the Front Range Urban Corridor, Colorado* distributed by the Colorado Geological Survey in 1974 (Reference 6). The expansive soils encountered on site are highly sporadic, therefore, none have been indicated on the map. The soils are slightly to highly expansive and can cause differential movement in the structure foundations.

Mitigation: Should expansive soils be encountered beneath the foundation, mitigation will be necessary. Mitigation of expansive soils may include overexcavation and replacement with non-expansive structural fill at 95% of its maximum Modified Proctor Dry Density, ASTM D-1557. Drilled pier foundation systems are another option in areas of highly expansive soils. Floor slabs on expansive soils should be expected to experience movement. Overexcavation and replacement with compacted non-expansive soils has been successful in minimizing slab movements. Final recommendations should be determined after additional investigation of each building site.

pu *Potentially Unstable Slopes (Hazard)*

These are in areas of steep slopes due to erosion along some of the drainages on site. Due to lot sizes, these slopes can be avoided by construction. A minimum setback of 30 feet should be maintained between buildings and the crest of the slopes. In areas where

construction encroaches on potentially unstable slopes, regrading and erosion protection may be necessary. Erosion protection may also be necessary in areas where high water is actively cutting the toe of the slopes to prevent further erosion.

af Artificial Fill (Constraint)

These are areas of man-made fill associated with earthen dams on-site.

Mitigation: The earthen dams lie within defined drainages and should be avoided as building sites. Foundations may penetrate smaller berms on site. Should any uncontrolled fill be encountered in other portions of the site beneath foundations, removal and recompaction at 95% of its maximum Modified Procter Dry Density, ASTM D-1557 will be required.

## **6.1 Relevance of Geologic Conditions to Land Use Planning**

The development will consist of rural residential lots. It is our opinion that the existing geologic and engineering geologic conditions will impose some constraints on the proposed development and construction. The most significant problems affecting development will be those associated with the drainages on site that can be avoided or properly mitigated during construction on each lot. Other hazards on site may be satisfactorily mitigated through proper engineering design and construction practices or avoidance.

The upper materials are typically at medium dense to dense states. Areas of loose soils were encountered that may require recompaction. The medium dense to dense granular soils encountered in the upper soil profiles of the test borings and test pits should provide good support for foundations. Loose soils, if encountered beneath foundations or slabs, will require removal of the upper 2 to 3 feet of loose material and recompaction. Expansive soils, although sporadic, were encountered. Shallow bedrock was encountered in portions of the site. Expansive clayey sandstone and claystone are common in the Dawson Formation, which may require mitigation.

Foundations anticipated for the site are standard spread footings being on granular site soils or sandstone. Overexcavation in areas of expansive soils or loose soils may be required. Areas of

artificial fill, if encountered beneath foundations will require penetration or recompaction. Areas containing arkosic sandstone will have high allowable bearing conditions. Expansive layers may also be encountered in the soil and bedrock on this site. Expansive soils, if encountered, will require special foundation design and/or overexcavation. These soils will not prohibit development.

Areas of seasonal shallow groundwater and potentially seasonal shallow groundwater were encountered on site. The site is not mapped in a floodplain zone (Figure 7, Reference 7). Areas of seasonal and potentially seasonal shallow groundwater were observed across the site. In these areas, we would anticipate the potential for periodically high subsurface moisture conditions and frost heave potential. These areas lie within low-lying areas along the drainage in the central portion of the subdivision and in the low-lying areas and minor drainages across the site. Water was not observed in any of the minor drainages at the time of our site investigation. Due to the size of the lots and the proposed development, the majority of these areas can be avoided by construction on the lots. Regrading can also mitigate some minor drainage swales on some of the lots. Structures should not block drainages. Any site grading should be done in such a manner as to not create areas of ponded water around structures or septic fields. Finished floor levels must be a minimum of one foot above the floodplain level. Septic fields should not be located in drainage areas due to the potential for periodic high groundwater conditions. Specific floodplain locations and drainage studies are beyond the scope of this report.

In summary, development of the site can be achieved if the items mentioned above are mitigated. These items can be mitigated through proper design and construction or through avoidance. Investigation on each lot is recommended prior to construction.

## **7.0 ECONOMIC MINERAL RESOURCES**

Some of the sandy materials on-site could be considered a low-grade sand resource. According to the *El Paso County Aggregate Resource Evaluation Map* (Reference 7), the area is mapped with floodplain, valley fill and upland deposits. According to the *Atlas of Sand, Gravel and Quarry Aggregate Resources, Colorado Front Range Counties* distributed by the Colorado Geological Survey (Reference 8), areas of the site are mapped with Alluvial Fan deposits: sand and probable aggregate resource (A3). According to the *Evaluation of Mineral and Mineral Fuel Potential* (Reference 9), the area of the site has been mapped as “Good” for industrial minerals. However, considering the abundance of similar materials through the region and the close proximity to developed land, they would be considered to have little significance as an economic resource. According to the *Evaluation of Mineral and Mineral Fuel Potential of El Paso County State Mineral Lands* (Reference 9), the site is mapped within the Denver Basin Coal Region. However, the area of the site has been mapped as “Poor” for coal resources. No active or inactive mines have been mapped in the area of the site. No metallic mineral resources have been mapped on the site (Reference 9).

The site has been mapped as “Fair” for oil and gas resources (Reference 9). No oil or gas fields have been discovered in the area of the site. The sedimentary rocks in the area may lack the geologic structure for trapping oil or gas; therefore, it may not be considered a significant resource. Hydraulic fracturing is a new method that is being used to extract oil and gas from rocks. It utilizes pressurized fluid to extract oil and gas from rocks that would not normally be productive. The area of the site has not been explored to determine if the rocks underlying the site would be commercially viable utilizing hydraulic fracturing. The practice of hydraulic fracturing has come under review due to concerns about environmental impacts, health and safety.



## **8.0 EROSION CONTROL**

The soil types observed on the site are mildly to highly susceptible to wind erosion, and moderately to highly susceptible to water erosion. A minor wind erosion and dust problem may be created for a short time during and immediately after construction. Should the problem be considered severe enough during this time, watering of the cut areas or the use of chemical palliative may be required to control dust. However, once construction has been completed and vegetation re-established, the potential for wind erosion should be considerably reduced.

With regard to water erosion, loosely compacted soils will be the most susceptible to water erosion, residually weathered soils and weathered bedrock materials become increasingly less susceptible to water erosion. For the typical soils observed on site, allowable velocities or unvegetated and unlined earth channels would be on the order of 3 to 4 feet/second, depending upon the sediment load carried by the water. Permissible velocities may be increased through the use of vegetation to something on the order of 4 to 7 feet/second, depending upon the type of vegetation established. Should the anticipated velocities exceed these values, some form of channel lining material may be required to reduce erosion potential. These might consist of some of the synthetic channel lining materials on the market or conventional riprap. In cases where ditch-lining materials are still insufficient to control erosion, small check dams or sediment traps may be required. The check dams will serve to reduce flow velocities, as well as provide small traps for containing sediment. The determination of the amount, location and placement of ditch linings, check dams and of the special erosion control features should be performed by or in conjunction with the drainage engineer who is more familiar with the flow quantities and velocities.

Cut and fill slope areas will be subjected primarily to sheetwash and rill erosion. Unchecked rill erosion can eventually lead to concentrated flows of water and gully erosion. The best means to combat this type of erosion is, where possible, the adequate re-vegetation of cut and fill slopes. Cut and fill slopes having gradients more than three (3) horizontal to one (1) vertical become increasingly more difficult to revegetate successfully. Therefore, recommendations pertaining to the vegetation of the cut and fill slopes may require input from a qualified landscape architect and/or the Soil Conservation Service.

## **9.0 EMBANKMENT CONSTRUCTION RECOMMENDATIONS**

In general, the site soils are suitable for embankment construction. Groundwater may be encountered in cuts for the proposed detention ponds across the site. If excavations encroach on the groundwater level unstable soil conditions may be encountered.

Any areas to receive fill should have all topsoil, organic material or debris removed. Fill must be properly benched and compacted to minimize potentially unstable conditions in slope areas. Fill slopes should be 3:1 or flatter on the upstream faces or 2.5:1 or flatter on the downstream face. The subgrade should be scarified and moisture conditioned to within 2% of optimum moisture content and compacted to a minimum of 95% of its maximum Modified Proctor Dry Density, ASTM D-1557, prior to placing new fill. Areas receiving fill may require stabilization with rock or fabric if shallow groundwater conditions are encountered.

New fill should be placed in thin lifts not to exceed 6 inches after compaction while maintaining at least 95% of its maximum Modified Proctor Dry Density, ASTM D-1557. These materials should be placed at a moisture content conducive to compaction, usually 0 to  $\pm 2\%$  of Proctor optimum moisture content. The placement and compaction of fill should be observed and tested by Entech during construction. Entech should approve any import materials prior to placing or hauling them to the site.

## **10.0 CLOSURE**

It is our opinion that the existing geologic engineering and geologic conditions will impose some minor constraints on development and construction of the site. The majority of these conditions can be avoided by construction. Others can be mitigated through proper engineering design and construction practices. The proposed development and use is consistent with anticipated geologic and engineering geologic conditions.

It should be pointed out that because of the nature of data obtained by random sampling of such variable and non-homogeneous materials as soil and rock, it is important that we be informed of any differences observed between surface and subsurface conditions encountered in construction and those assumed in the body of this report. Individual investigations for building

sites and septic systems will be required prior to construction. Construction and design personnel should be made familiar with the contents of this report. Reporting such discrepancies to Entech Engineering, Inc. soon after they are discovered would be greatly appreciated and could possibly help avoid construction and development problems.

This report has been prepared for PT Eagleview, LLC for application to the proposed project in accordance with generally accepted geologic soil and engineering practices. No other warranty expressed or implied is made.

We trust that this report has provided you with all the information that you required. Should you require additional information, please do not hesitate to contact Entech Engineering, Inc.

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**TABLES**

**TABLE 1**  
**SUMMARY OF LABORATORY TEST RESULTS**

CLIENT PT EAGLEVIEW  
PROJECT ARROYA AND RAYGOR  
JOB NO. 212684

SOIL TYPE	TEST BORING NO.	DEPTH (FT)	WATER (%)	DRY DENSITY (PCF)	PASSING NO. 200 SIEVE (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	SULFATE (WT %)	FHA SWELL (PSF)	SWELL/CONSOL (%)	UNIFIED CLASSIFICATION	SOIL DESCRIPTION
1	TP-1	3			1.4						SW	SAND
1	TP-2	1-2			12.6						SM	SAND, SILTY
1	TP-4	3-4			11.2						SM-SW	SAND, SLIGHTLY SILTY
1	TP-5	1-2			10.5						SM-SW	SAND, SLIGHTLY SILTY
1	TP-5	3			7.3						SM-SW	SAND, SLIGHTLY SILTY
1	TP-6	1.5			35.1						SC	SAND, CLAYEY
1	TP-6	2.5			6.3						SM-SW	SAND, SLIGHTLY SILTY
1	TP-7	2			3.1						SW	SAND
1	TP-7	5			9.6						SM-SW	SAND, SLIGHTLY SILTY
1	TP-8	4			15.8						SM	SAND, SILTY
2	TP-3	2-3			78.3						CL	CLAY, SANDY



**Table 2: Summary Test Boring Results**

<b>Test Boring No.</b>	<b>Depth to Bedrock (ft.)</b>
1	7
2	9
3	0
4	5
5	1
6	4

Soils, Geology, and Wastewater Study prepared by Entech Job No. 80503

## FIGURES



N



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505 ELKTON DRIVE  
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VICINITY MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

DRAWN:  
JHR

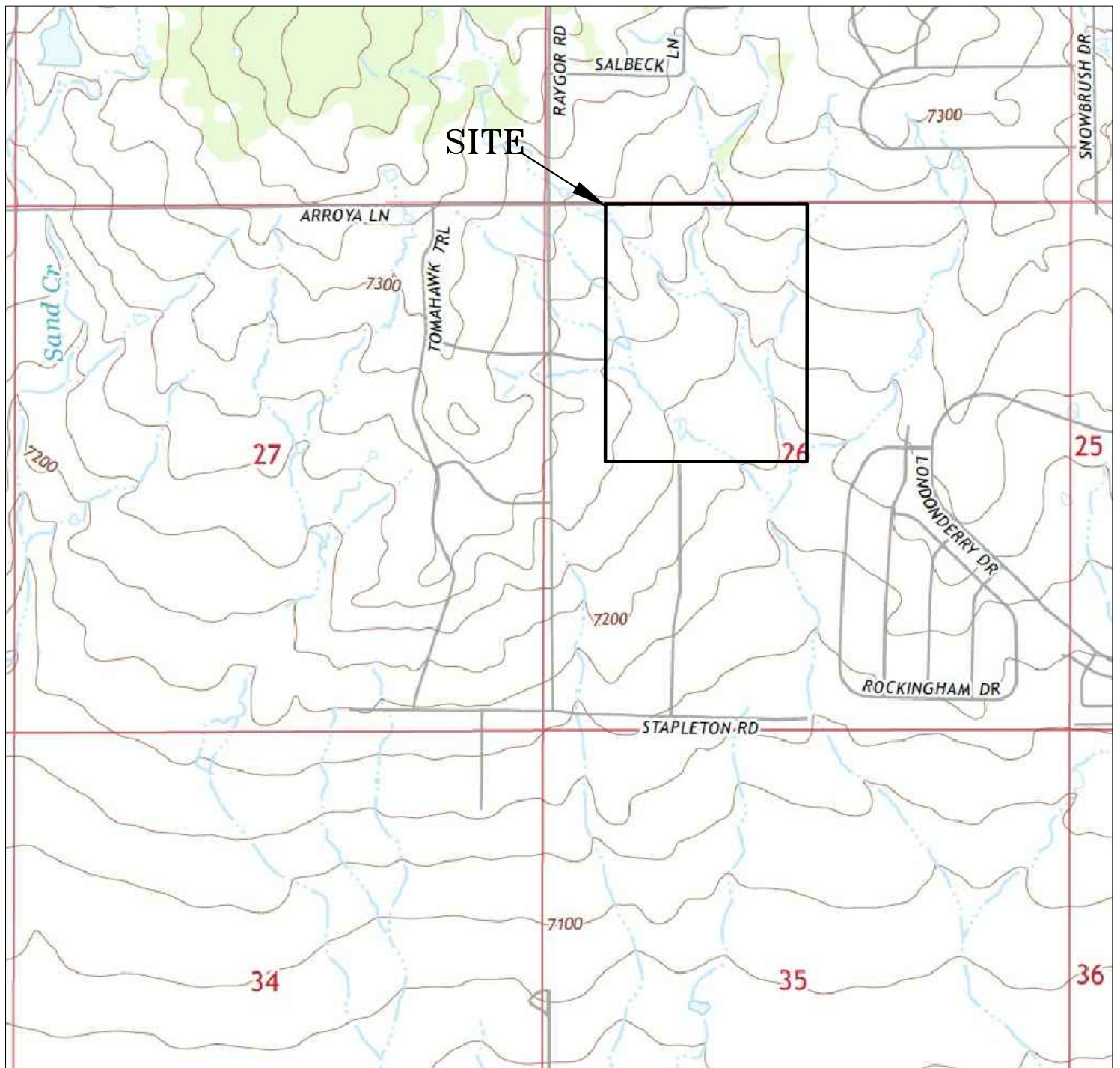
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11/8/21

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DATE:

JOB NO.:  
212684

FIG NO.:  
1



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USGS MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

DRAWN:  
JHR

DATE:  
11/8/21

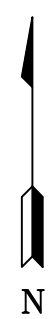
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JOB NO.:  
212684

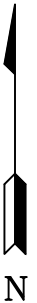
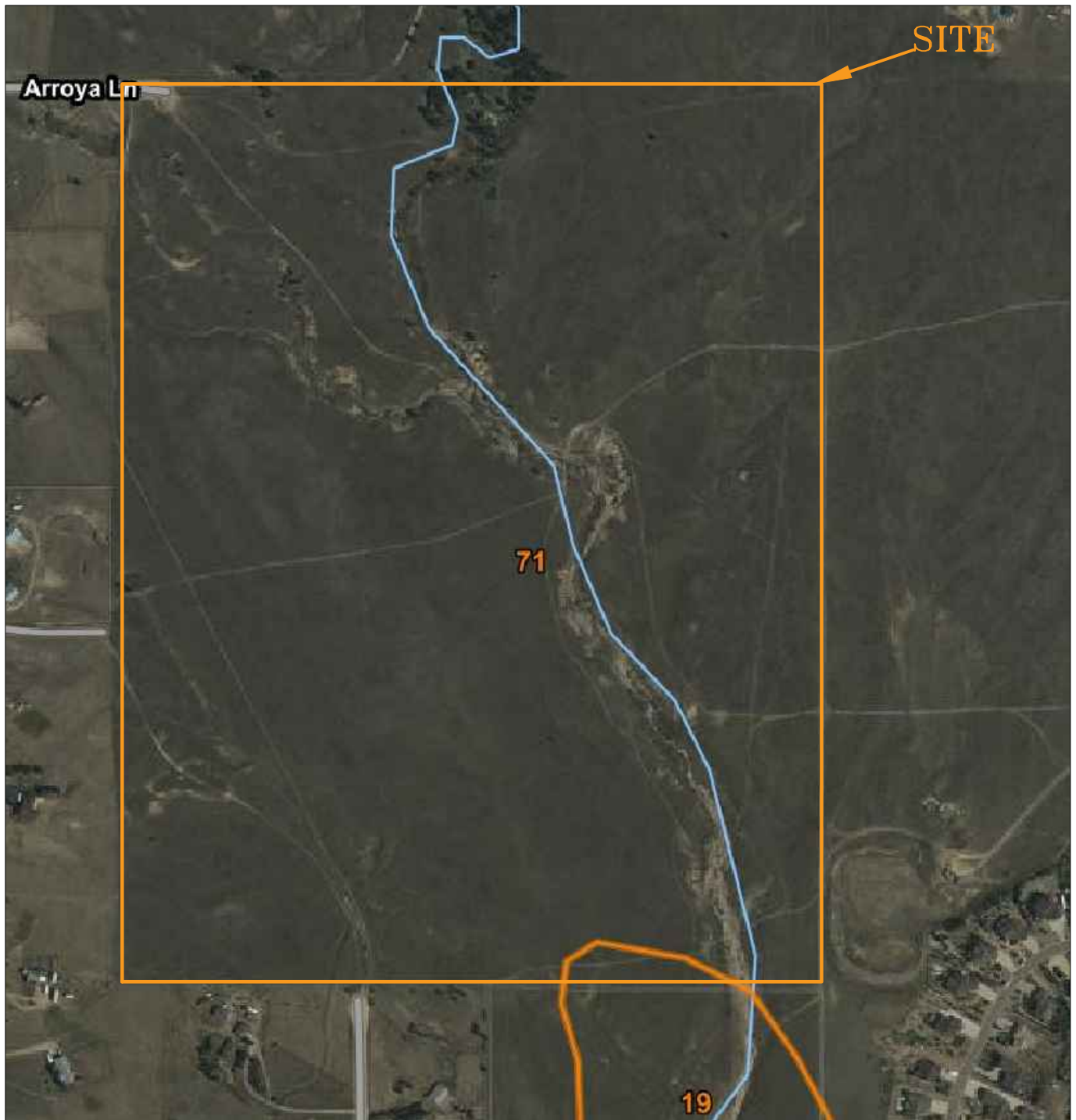
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2





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DATE  
11/4/21  
SCALE  
AS SHOWN  
JOB NO.  
212684  
FIGURE No.  
3





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SOIL SURVEY MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

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JHR

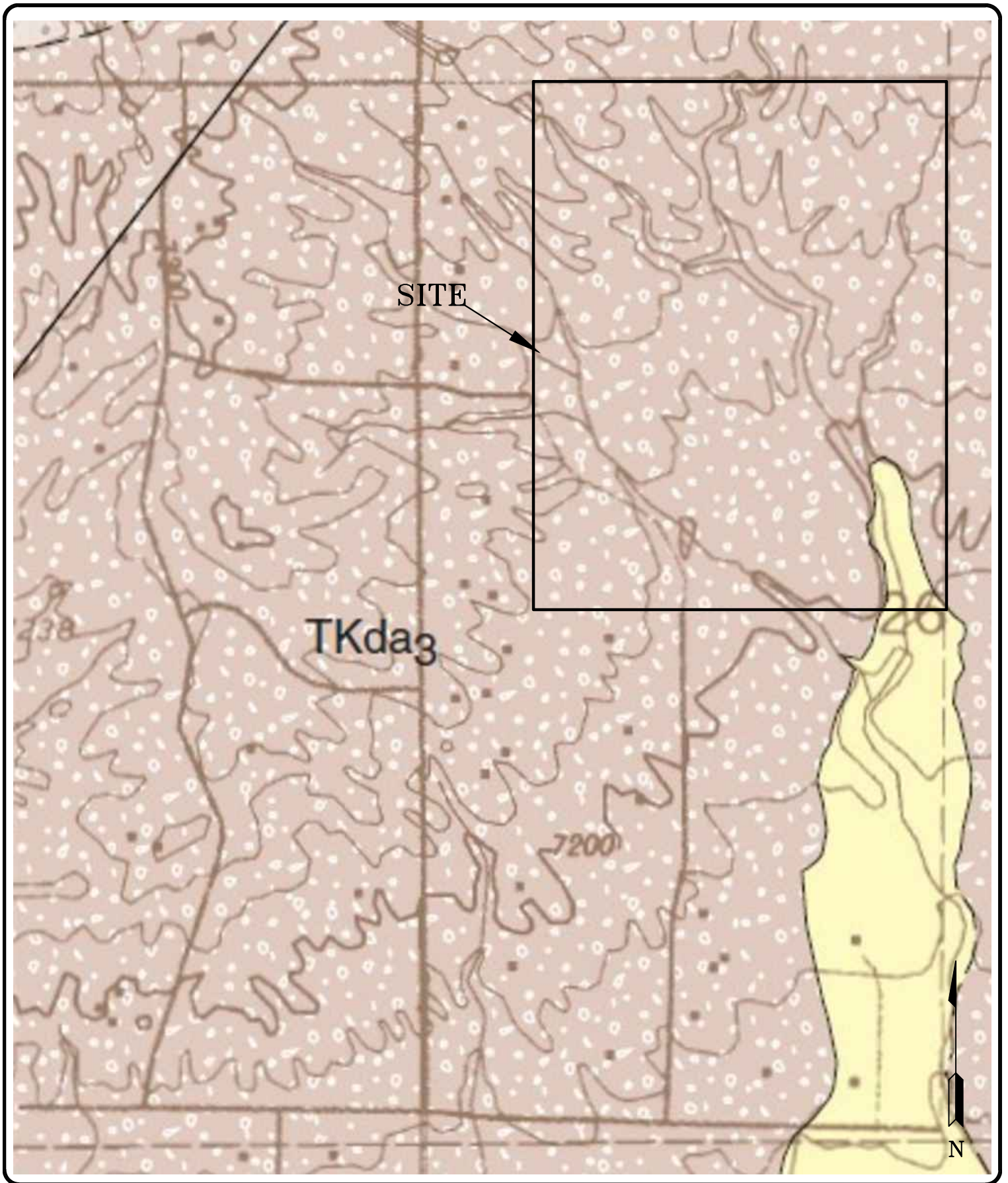
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JOB NO.:  
212684

FIG NO.:  
4



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NW FALCON QUADRANGLE GEOLOGIC MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

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JHR

DATE:  
11/8/21

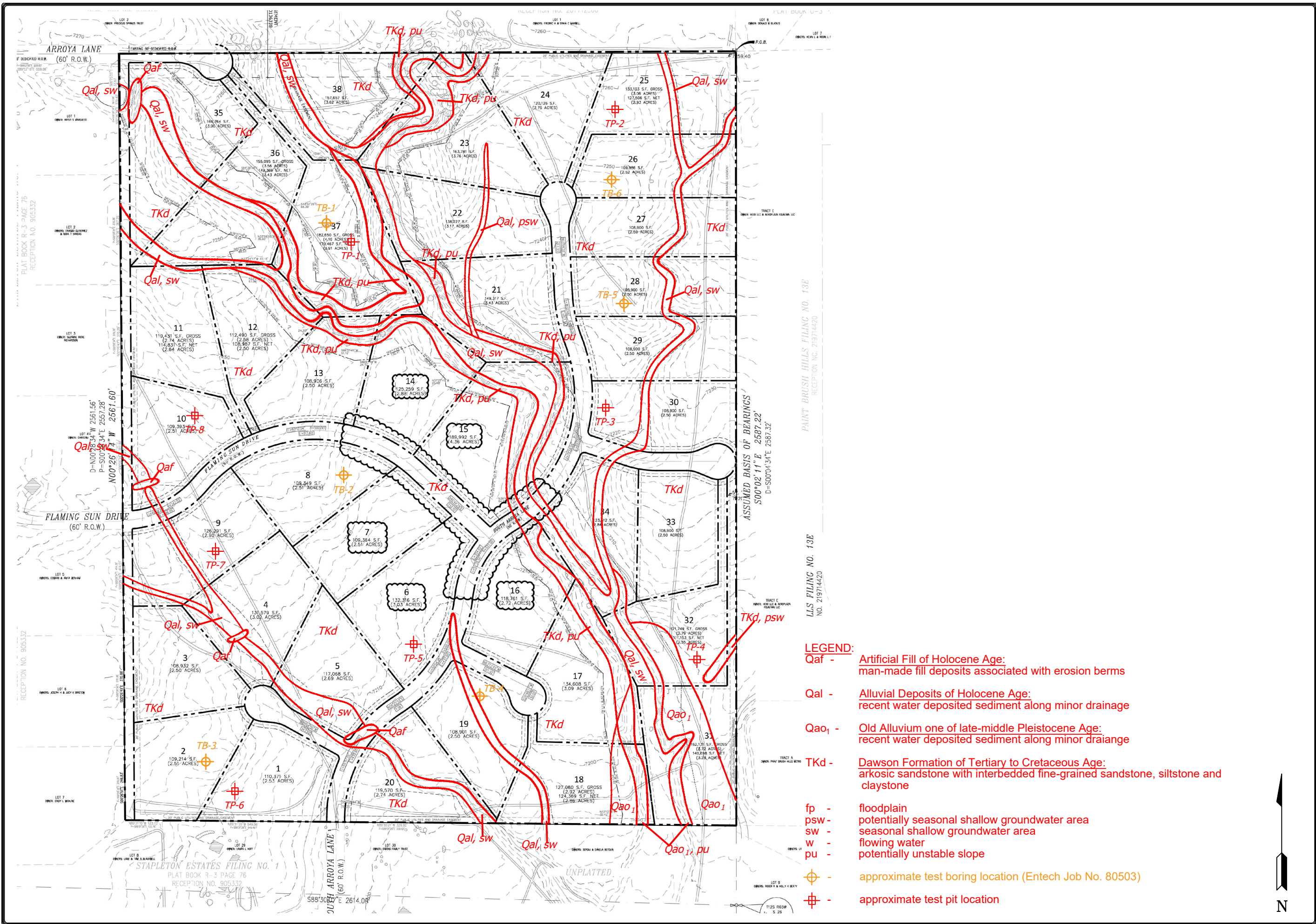
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JOB NO.:  
212684

FIG NO.:  
5





REVISION	BY

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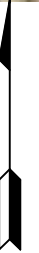
ENGINEERING, INC.

505 ELKTON DRIVE  
COLORADO SPRINGS, CO. 80907

GEOLOGY/ENGINEERING GEOLOGY MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

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DATE 11/17/21
SCALE AS SHOWN
JOB NO. 212684
FIGURE No. 6





N



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FLOODPLAIN MAP  
EAGLEVIEW SUBDIVISION  
ARROYA LANE & RAYGOR ROAD  
EL PASO COUNTY, CO.  
FOR: PT EAGLEVIEW, LLC

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JHR

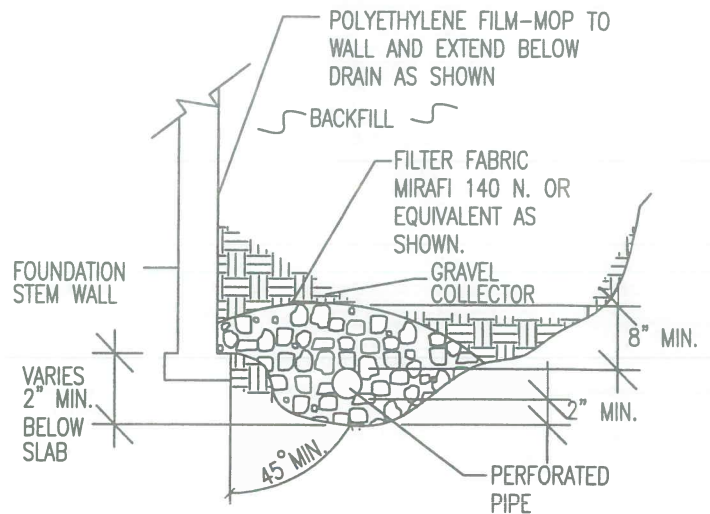
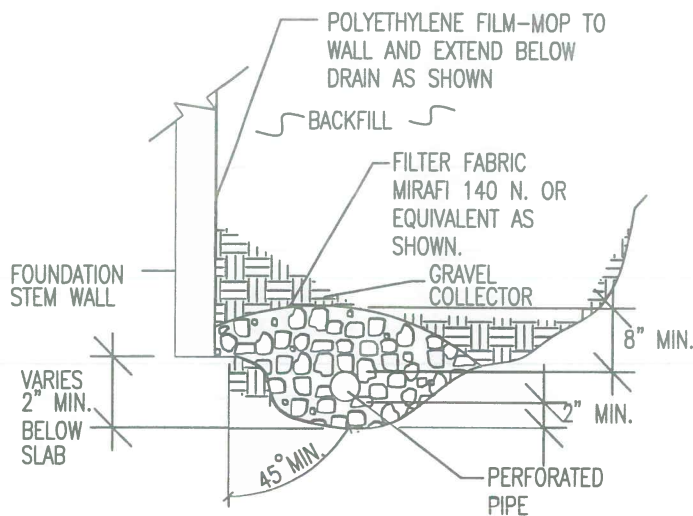
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FIG NO.:  
7



#### NOTES:

-GRAVEL SIZE IS RELATED TO DIAMETER OF PIPE PERFORATIONS-85% GRAVEL GREATER THAN 2x PERFORATION DIAMETER.

-PIPE DIAMETER DEPENDS UPON EXPECTED SEEPAGE. 4-INCH DIAMETER IS MOST OFTEN USED.

-ALL PIPE SHALL BE PERFORATED PLASTIC. THE DISCHARGE PORTION OF THE PIPE SHOULD BE NON-PERFORATED PIPE.

-FLEXIBLE PIPE MAY BE USED UP TO 8 FEET IN DEPTH, IF SUCH PIPE IS DESIGNED TO WITHSTAND THE PRESSURES. RIGID PLASTIC PIPE WOULD OTHERWISE BE REQUIRED.

-MINIMUM GRADE FOR DRAIN PIPE TO BE 1% OR 3 INCHES OF FALL IN 25 FEET.

-DRAIN TO BE PROVIDED WITH A FREE GRAVITY OUTFALL, IF POSSIBLE. A SUMP AND PUMP MAY BE USED IF GRAVITY OUT FALL IS NOT AVAILABLE.



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#### PERIMETER DRAIN DETAIL

DRAWN:

DATE:

DESIGNED:

CHECKED:

JOB NO.:

212684

FIG NO.:

8

**APPENDIX A: Site Photographs**





**Looking west from the  
northeastern portion of  
the site.**

October 12, 2021



**Looking south from  
northeastern portion of  
the site.**

October 12, 2021





**Looking southeast  
from the north-central  
portion of the site.**

October 12, 2021



**Looking southeast  
along drainage with  
potentially unstable  
slopes.**

October 12, 2021





**Looking north from the  
southeastern portion  
of the site.**

October 12, 2021



**Looking west from  
the southeastern  
portion of the site.**

October 12, 2021





**Looking east from the  
west-central portion of  
the site.**

October 12, 2021



**Looking south from  
the western portion of  
the site.**

October 12, 2021

## APPENDIX B: Test Pit Logs

TEST PIT NO. 1  
DATE EXCAVATED 10/13/2021  
Job # 212684

TEST PIT NO. 2  
DATE EXCAVATED 10/13/2021  
CLIENT PT Eagle View  
LOCATION Arroya Lane and Raygor Road

REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
topsoil, 0-8 inches	1			bl	m	4
sandy clay, Fine to medium grained, brown, moist	2					
	3			gr	w	2A
sandy loam, fine to coarse grained, brown, moist	4					
	5					
	6			gr	s	2
sandy loam, fine to coarse grained, brown, moist	7					
	8					
	9					
	10					

REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
Refusal @ 5 feet						
topsoil, 0-12 inches	1					
	2			gr	m	2
sandy loam, fine to coarse grained, brown, moist	3					
	4			ma		3A
sandy clay loam, sandstone fine to coarse grained, brown moist	5					
	6					
	7					
	8					
	9					
	10					

#### Soil Structure Shape

granular - gr  
platy - pl  
blocky - bl  
prismatic - pr  
single grain - sg  
massive - ma

#### Soil Structure Grade

weak - w  
moderate - m  
strong - s  
loose - l



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COLORADO SPRINGS, COLORADO 80907

#### TEST PIT LOG

DRAWN:

DATE:

CHECKED:

DATE:

JOB NO.:  
212684

FIG NO.:  
B-1

TEST PIT NO. 3  
DATE EXCAVATED 10/13/2021  
Job # 212684

TEST PIT NO. 4  
DATE EXCAVATED 10/13/2021  
CLIENT PT Eagle View  
LOCATION Arroya Lane and Raygor Road

REMARKS

Redoxomorphic  
Features @  
~4.5feet

topsoil 0-6 inches

sandy loam, fine to coarse  
grained, brown, moist

sandy clay, fine to medium  
grained, brown, moist

sandy clay loam, fine to  
coarse grained, brown, moist

Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
1			gr	m	2
2					
3			ma		4A
4					
5			gr	m	3
6					
7					
8					
9					
10					

REMARKS

topsoil 0-12 inches

sandy loam, fine to coarse  
grained, brown, moist

sandy clay loam, fine to  
coarse grained, brown, moist

Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
1					
2			gr	m	2
3					
4					
5			ma		3A
6					
7					
8					
9					
10					

Soil Structure Shape

granular - gr  
platy - pl  
blocky - bl  
prismatic - pr  
single grain - sg  
massive - ma

Soil Structure Grade

weak - w  
moderate - m  
strong - s  
loose - l



**ENTECH  
ENGINEERING, INC.**

505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

TEST PIT LOG

DRAWN:

DATE:

CHECKED:  
LL

DATE:

JOB NO.:  
212684

FIG NO.:

B-2

TEST PIT NO. 5  
DATE EXCAVATED 10/13/2021  
Job # 212684

TEST PIT NO. 6  
DATE EXCAVATED 10/13/2021  
CLIENT PT Eagle View  
LOCATION Arroya Lane and Raygor Road

REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type	REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
topsoil 0-18 inches	1						Redoxomorphic Features @ ~3 feet	1					
sandy loam, fine to coarse grained, brown, moist	2			ma		2A	sandy clay, fine to medium grained, brown, moist	2			gr	s	4
sandy loam, fine to coarse grained, brown, moist	3						sandy loam, fine to coarse grained, brown, moist	3			gr	m	2
	4			gr	s	2	sandy clay, fine to medium grained, brown, moist	4			bl	m	4
	5							5					
	6			gr	m	2		6			ma		4A
	7						sandy clay, fine to medium grained, light brown, moist	7					
	8							8					
	9							9					
	10							10					

Soil Structure Shape

granular - gr  
platy - pl  
blocky - bl  
prismatic - pr  
single grain - sg  
massive - ma

Soil Structure Grade

weak - w  
moderate - m  
strong - s  
loose - l



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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

**TEST PIT LOG**

DRAWN:

DATE:

CHECKED:

DATE:

LLL

JOB NO.:  
212684

FIG NO.:

B-3



TEST PIT NO. 7  
DATE EXCAVATED 10/13/2021  
Job # 212684

TEST PIT NO. 8  
DATE EXCAVATED 10/13/2021  
CLIENT PT Eagle View  
LOCATION Arroya Lane and Raygor Road

REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type	REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
topsoil 0-6 inches	1						Refusal @ 7 feet	1					
sandy clay loam, fine to coarse grained, brown, moist	2			gr	m	3	topsoil 0-6 inches	2			gr	m	2
sandy loam, fine to coarse grained, brown, moist	3			gr	m	2	sandy loam, fine to coarse grained, brown, moist	3					
	4							4			ma		4A
sandy clay loam, fine to coarse grained, brown, moist	5			gr	m	3	sandy clay, fine to medium grained, brown, moist	5					
	6						Bedrock @ 5 feet	6					
	7							7					
	8							8					
	9							9					
	10							10					

Soil Structure Shape

granular - gr  
platy - pl  
blocky - bl  
prismatic - pr  
single grain - sg  
massive - ma

Soil Structure Grade

weak - w  
moderate - m  
strong - s  
loose - l



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**TEST PIT LOG**

DRAWN:

DATE:

CHECKED:  
LLC

DATE:

JOB NO.:  
212684

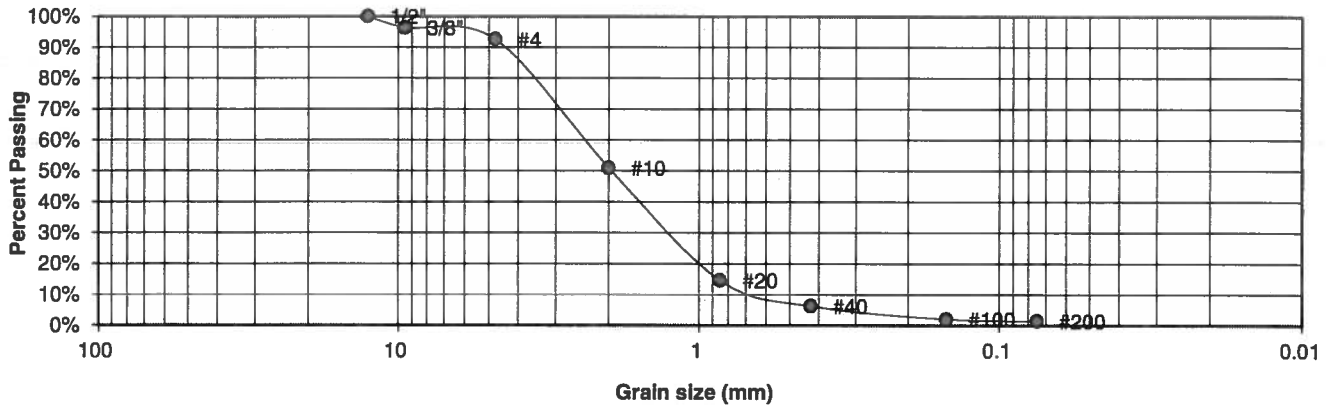
FIG NO.:  
B-4

## APPENDIX C: Laboratory Test Results



<u>UNIFIED CLASSIFICATION</u>	SW	<u>CLIENT</u>	PT EAGLEVIEW
<u>SOIL TYPE #</u>	1	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	TP-1	<u>JOB NO.</u>	212684
<u>DEPTH (FT)</u>	3	<u>TEST BY</u>	BL

**Sieve Analysis  
Grain Size Distribution**



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	96.3%
4	92.5%
10	51.0%
20	14.6%
40	6.3%
100	2.1%
200	1.4%

Atterberg  
Limits  
Plastic Limit  
Liquid Limit  
Plastic Index

Swell  
Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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COLORADO SPRINGS, COLORADO 80907

**LABORATORY TEST  
RESULTS**

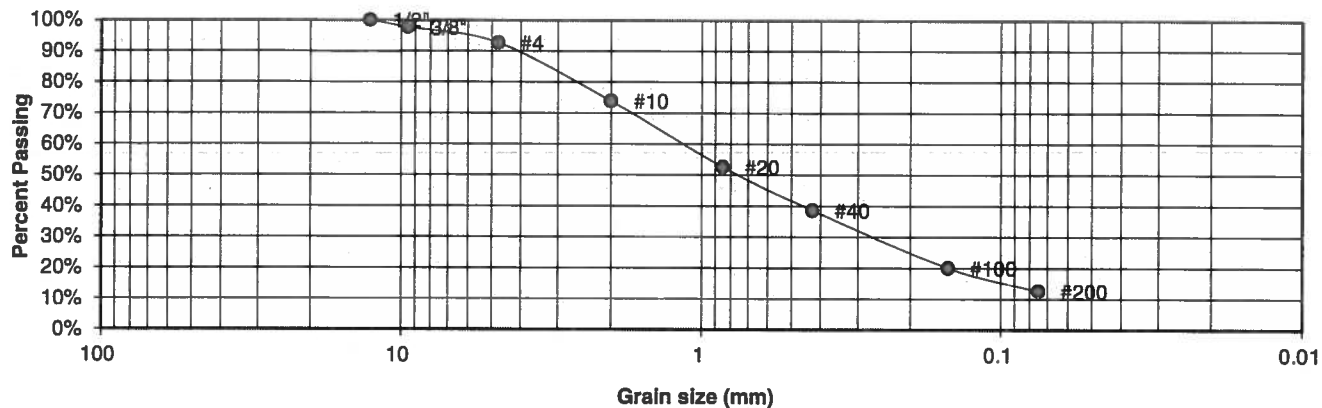
DRAWN:	DATE:	CHECKED: LLC	DATE:
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JOB NO.:  
212684

FIG NO.:  
C-1

<u>UNIFIED CLASSIFICATION</u>	SM	<u>CLIENT</u>	PT EAGLEVIEW
<u>SOIL TYPE #</u>	1	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	TP-2	<u>JOB NO.</u>	212684
<u>DEPTH (FT)</u>	1-2	<u>TEST BY</u>	BL

### Sieve Analysis Grain Size Distribution



U.S.  
Sieve #

3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

Percent  
Finer

100.0%  
97.7%  
92.7%  
73.8%  
52.6%  
38.6%  
20.0%  
12.6%

Atterberg

Limits

Plastic Limit

Liquid Limit

Plastic Index

Swell

Moisture at start

Moisture at finish

Moisture increase

Initial dry density (pcf)

Swell (psf)



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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:

DATE:

LL

JOB NO.:  
212684

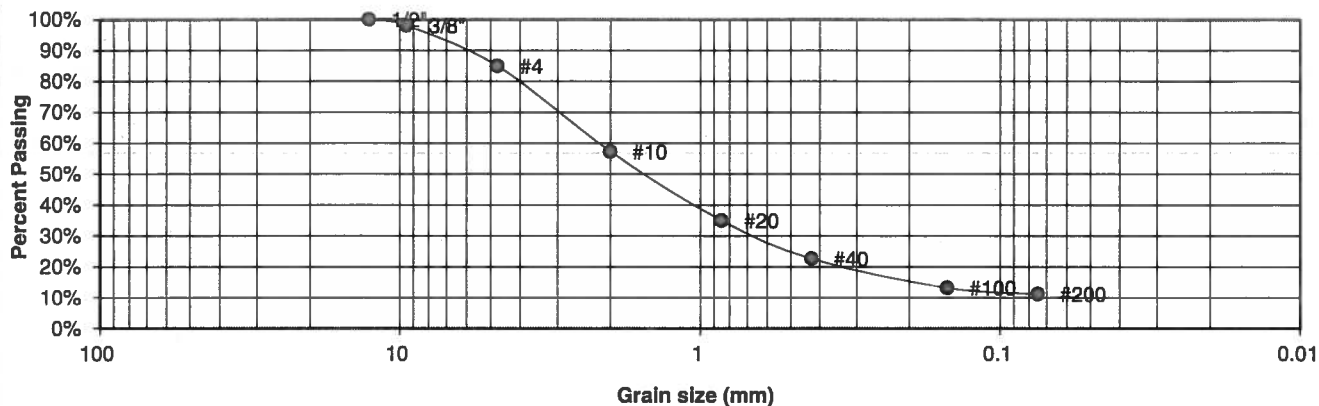
FIG NO.:

C-2

<b>UNIFIED CLASSIFICATION</b>	SM-SW
<b>SOIL TYPE #</b>	1
<b>TEST BORING #</b>	TP-4
<b>DEPTH (FT)</b>	3-4

<b>CLIENT</b>	PT EAGLEVIEW
<b>PROJECT</b>	ARROYA AND RAYGOR
<b>JOB NO.</b>	212684
<b>TEST BY</b>	BL

### Sieve Analysis Grain Size Distribution



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	98.1%
4	85.0%
10	57.3%
20	35.0%
40	22.6%
100	13.2%
200	11.2%

**Atterberg  
Limits**  
Plastic Limit  
Liquid Limit  
Plastic Index

**Swell**  
Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:  
LLL

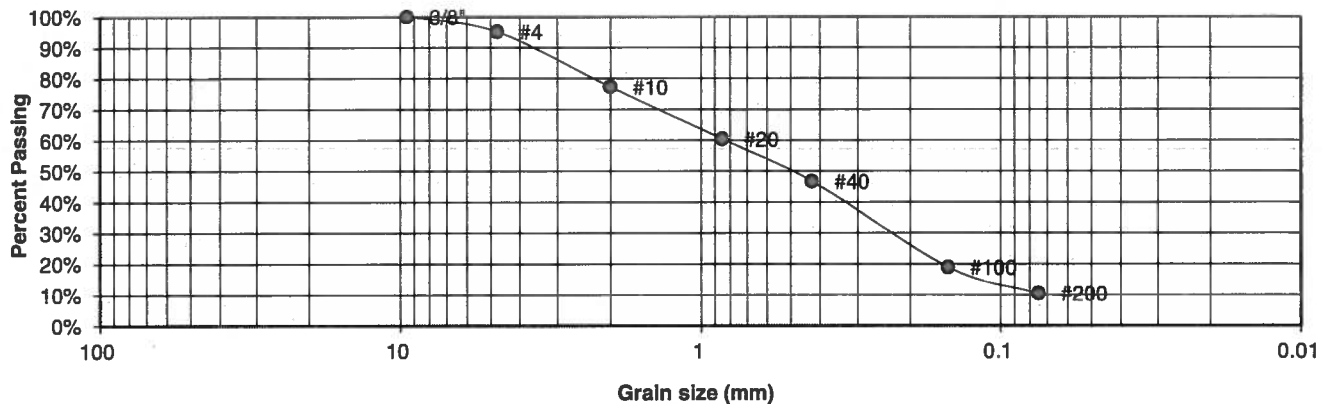
DATE:

JOB NO.:  
212684

FIG NO.:  
C-3

UNIFIED CLASSIFICATION	SM-SW	CLIENT	PT EAGLEVIEW
SOIL TYPE #	1	PROJECT	ARROYA AND RAYGOR
TEST BORING #	TP-5	JOB NO.	212684
DEPTH (FT)	1-2	TEST BY	BL

### Sieve Analysis Grain Size Distribution



U.S.  
Sieve #

3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

Percent  
Finer

100.0%  
95.1%  
77.2%  
60.4%  
46.7%  
18.9%  
10.5%

Atterberg

Limits

Plastic Limit

Liquid Limit

Plastic Index

Swell

Moisture at start

Moisture at finish

Moisture increase

Initial dry density (pcf)

Swell (psf)



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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:  
LLL

DATE:

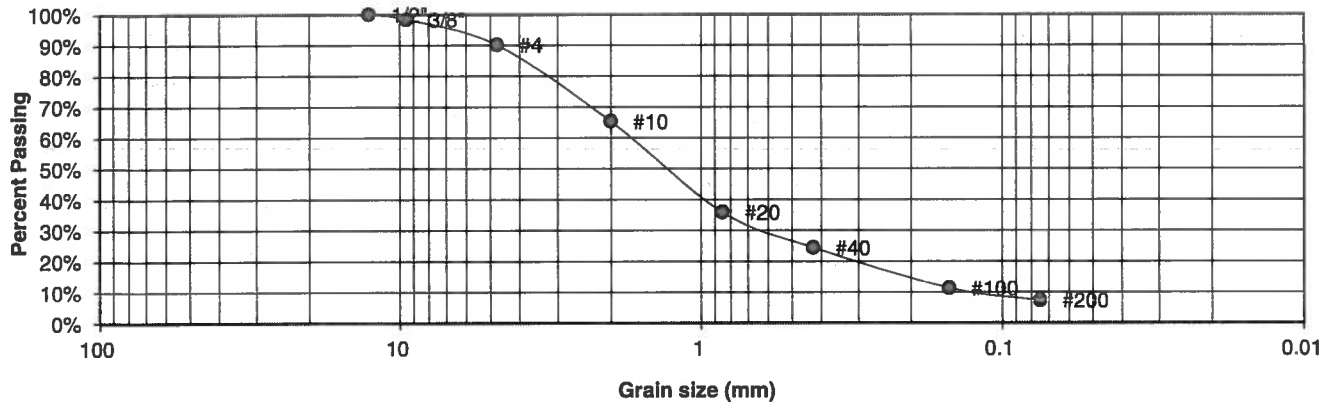
JOB NO.:  
212684

FIG NO.:  
C-4

<b>UNIFIED CLASSIFICATION</b>	SM-SW
<b>SOIL TYPE #</b>	1
<b>TEST BORING #</b>	TP-5
<b>DEPTH (FT)</b>	3

<b>CLIENT</b>	PT EAGLEVIEW
<b>PROJECT</b>	ARROYA AND RAYGOR
<b>JOB NO.</b>	212684
<b>TEST BY</b>	BL

### Sieve Analysis Grain Size Distribution



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	98.4%
4	90.1%
10	65.3%
20	35.9%
40	24.4%
100	11.3%
200	7.3%

**Atterberg  
Limits**  
Plastic Limit  
Liquid Limit  
Plastic Index

**Swell**  
Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



**ENTECH  
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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:  
LL

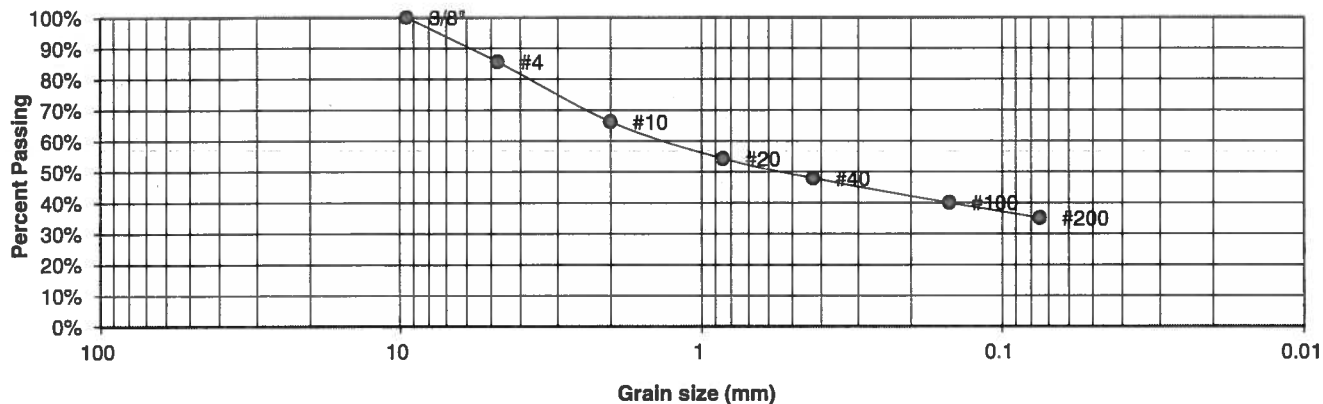
DATE:

JOB NO.:  
212684

FIG NO.:  
C-5

UNIFIED CLASSIFICATION	SC	CLIENT	PT EAGLEVIEW
SOIL TYPE #	1	PROJECT	ARROYA AND RAYGOR
TEST BORING #	TP-6	JOB NO.	212684
DEPTH (FT)	1.5	TEST BY	BL

### Sieve Analysis Grain Size Distribution



U.S.  
Sieve #

Percent  
Finer

3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

100.0%  
85.6%  
66.2%  
54.3%  
47.9%  
40.0%  
35.1%

Atterberg

Limits

Plastic Limit

Liquid Limit

Plastic Index

Swell

Moisture at start

Moisture at finish

Moisture increase

Initial dry density (pcf)

Swell (psf)



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COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:

DATE:

LLL

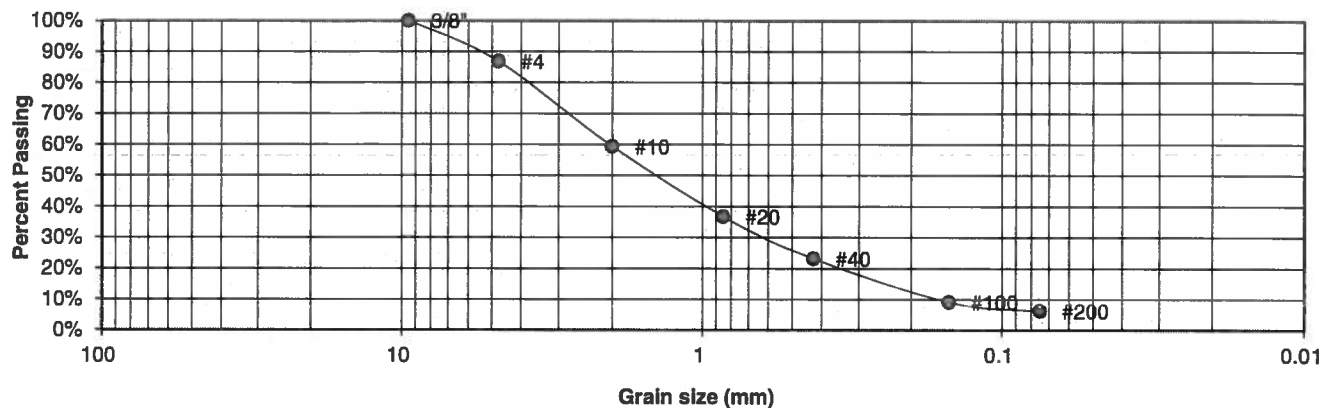
JOB NO.:  
212684

FIG NO.:

C-6

<b>UNIFIED CLASSIFICATION</b>	SM-SW	<b>CLIENT</b>	PT EAGLEVIEW
<b>SOIL TYPE #</b>	1	<b>PROJECT</b>	ARROYA AND RAYGOR
<b>TEST BORING #</b>	TP-6	<b>JOB NO.</b>	212684
<b>DEPTH (FT)</b>	2.5	<b>TEST BY</b>	BL

### Sieve Analysis Grain Size Distribution



**U.S.  
Sieve #**

3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

**Percent  
Finer**

100.0%  
86.8%  
59.4%  
36.7%  
23.1%  
9.1%  
6.3%

**Atterberg  
Limits**

Plastic Limit  
Liquid Limit  
Plastic Index

**Swell**

Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:

LLC

DATE:

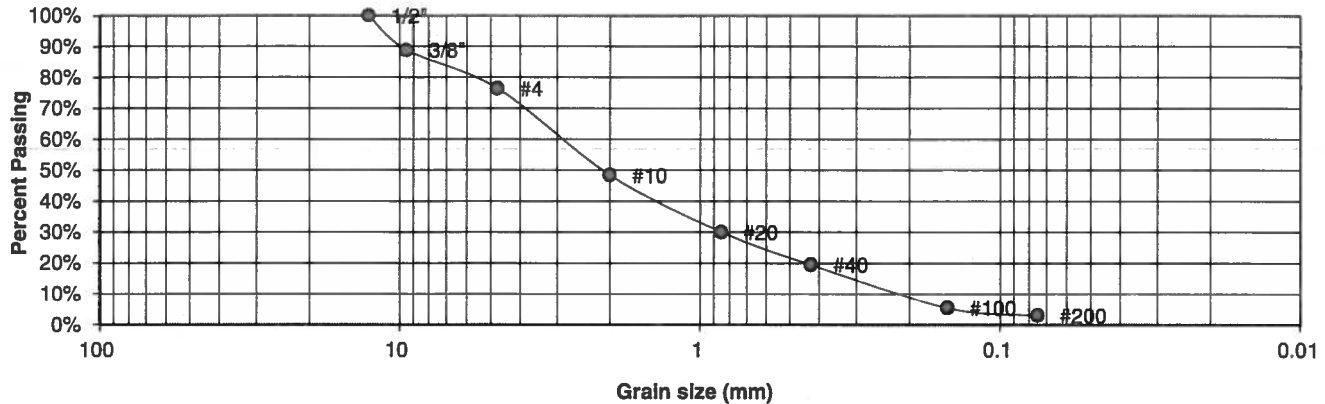
JOB NO.:  
212684

FIG NO.:  
C-7



UNIFIED CLASSIFICATION	SW	CLIENT	PT EAGLEVIEW
SOIL TYPE #	1	PROJECT	ARROYA AND RAYGOR
TEST BORING #	TP-7	JOB NO.	212684
DEPTH (FT)	2	TEST BY	BL

### Sieve Analysis Grain Size Distribution



U.S.  
Sieve #

3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

Percent  
Finer

100.0%  
88.8%  
76.4%  
48.4%  
30.0%  
19.5%  
5.6%  
3.1%

Atterberg  
Limits

Plastic Limit  
Liquid Limit  
Plastic Index

Swell

Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:

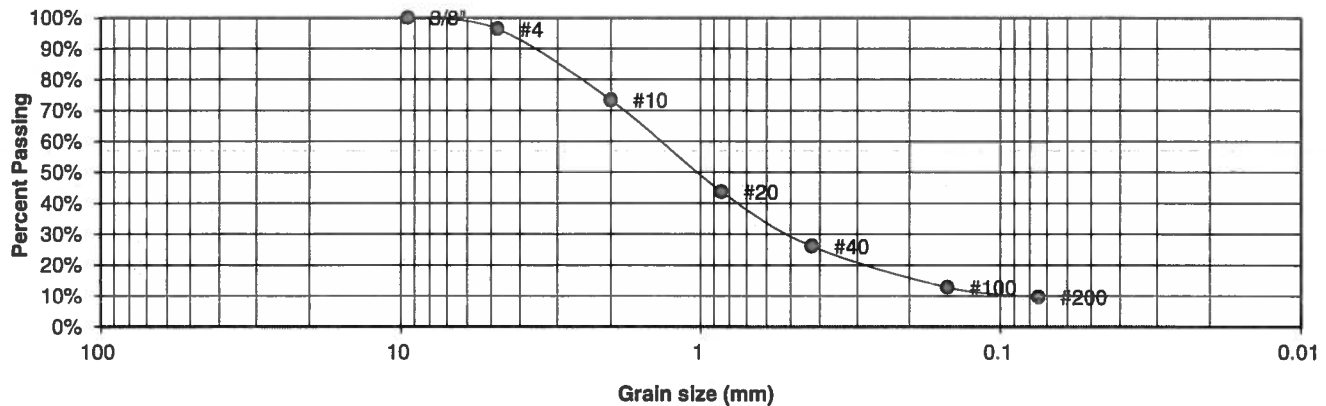
DATE:

JOB NO.:  
212684

FIG NO.:  
C-8

UNIFIED CLASSIFICATION	SM-SW	CLIENT	PT EAGLEVIEW
SOIL TYPE #	1	PROJECT	ARROYA AND RAYGOR
TEST BORING #	TP-7	JOB NO.	212684
DEPTH (FT)	5	TEST BY	BL

### Sieve Analysis Grain Size Distribution



U.S.  
Sieve #

3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

Percent  
Finer

100.0%  
96.3%  
73.3%  
43.7%  
26.1%  
12.8%  
9.6%

Atterberg  
Limits

Plastic Limit  
Liquid Limit  
Plastic Index

Swell

Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

### LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:

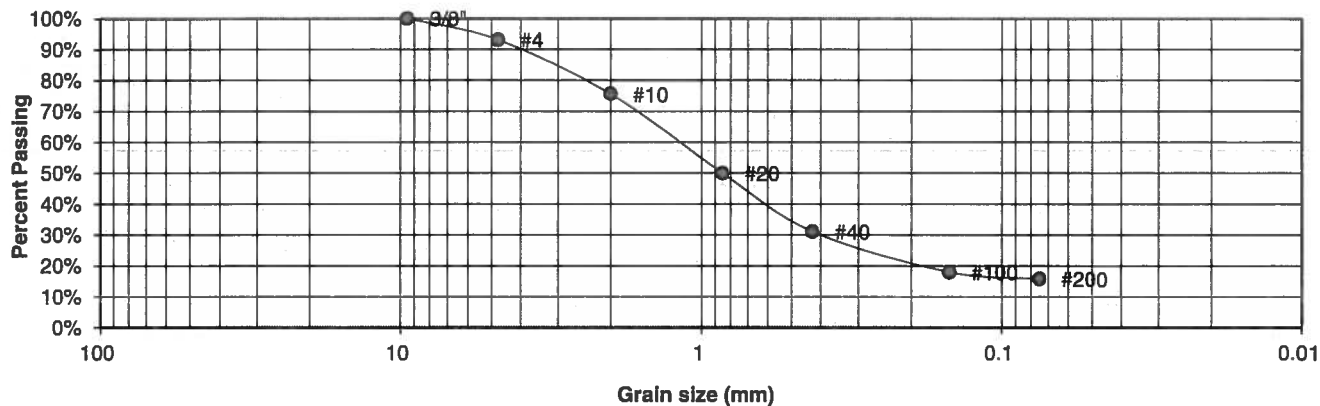
DATE:

JOB NO.:  
212684

FIG NO.:  
C-9

<u>UNIFIED CLASSIFICATION</u>	SM	<u>CLIENT</u>	PT EAGLEVIEW
<u>SOIL TYPE #</u>	1	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	TP-8	<u>JOB NO.</u>	212684
<u>DEPTH (FT)</u>	4	<u>TEST BY</u>	BL

**Sieve Analysis  
Grain Size Distribution**



U.S.  
Sieve #

Percent  
Finer

3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

100.0%  
93.1%  
75.6%  
49.9%  
31.0%  
18.0%  
15.8%

Atterberg

Limits

Plastic Limit

Liquid Limit

Plastic Index

Swell

Moisture at start

Moisture at finish

Moisture increase

Initial dry density (pcf)

Swell (psf)



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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

**LABORATORY TEST  
RESULTS**

DRAWN:

DATE:

CHECKED: *CEL*

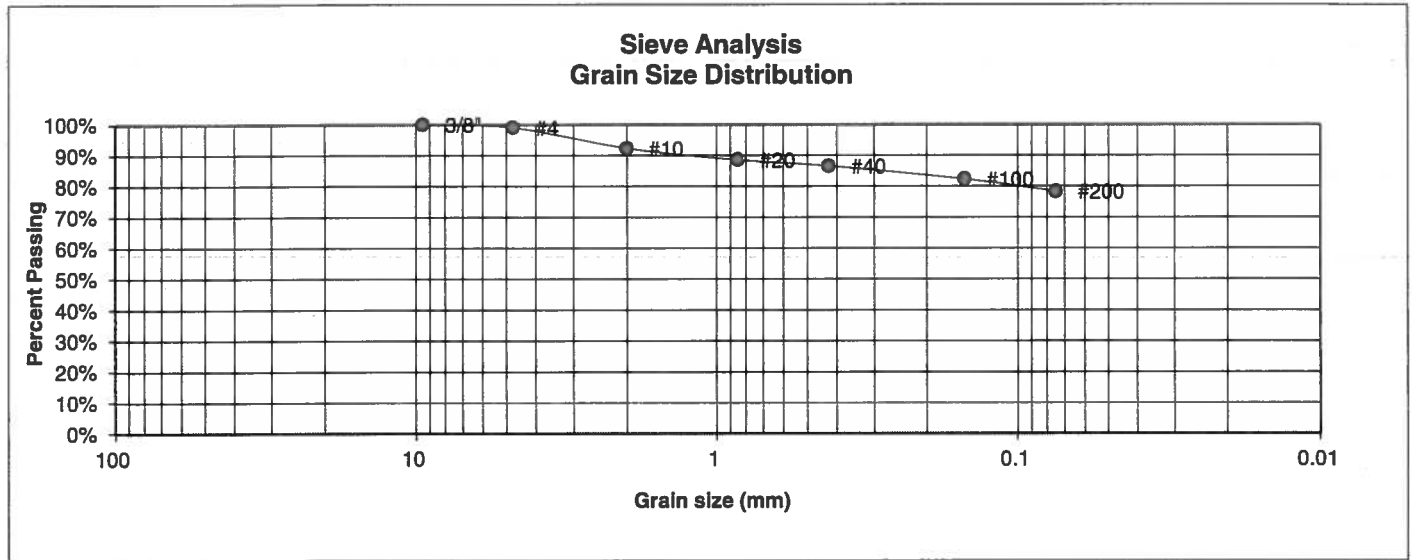
DATE:

JOB NO.:  
212684

FIG NO.:

*C-10*

<u>UNIFIED CLASSIFICATION</u>	CL	<u>CLIENT</u>	PT EAGLEVIEW
<u>SOIL TYPE #</u>	2	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	TP-3	<u>JOB NO.</u>	212684
<u>DEPTH (FT)</u>	2-3	<u>TEST BY</u>	BL



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.0%
10	92.3%
20	88.6%
40	86.5%
100	82.3%
200	78.3%

Atterberg  
Limits  
Plastic Limit  
Liquid Limit  
Plastic Index

Swell  
Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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505 ELKTON DRIVE  
COLORADO SPRINGS, COLORADO 80907

**LABORATORY TEST  
RESULTS**

DRAWN:	DATE:	CHECKED: <i>LL</i>	DATE:
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JOB NO.:  
212684

FIG NO.:  
*C-4*

**APPENDIX D: Test Boring Logs & Laboratory Test Results Previous  
Investigation Entech Job No. 80503**

**TABLE 1**  
**SUMMARY OF LABORATORY TEST RESULTS**

CLIENT    PROLAND GROUP, LLC  
PROJECT    ARROYA AND RAYGOR  
JOB NO.    80503

SOIL TYPE	TEST BORING NO.	DEPTH (FT)	WATER (%)	DRY DENSITY (PCF)	PASSING NO. 200 SIEVE (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	SULFATE (WT %)	FHA SWELL (PSF)	SWELL/CONSOL (%)	UNIFIED CLASSIFICATION	SOIL DESCRIPTION
1	PH-5	5			5.8						SM-SW	SAND, SLIGHTLY SILTY
1	PH-7	2-5			48.8	41	22		2230		SC	SAND, VERY CLAYEY
1	1	2-3			8.0						SM-SW	SAND, SLIGHTLY SILTY
1	4	2-5				28	14		690		SC	SAND, CLAYEY
2	PH-2	5			71.9	40	18				CL	CLAY, SANDY, VERY SILTY
2	2	3							2060		CL	CLAY
3	2	10			19.7				430		SC	SANDSTONE, CLAYEY
3	3	5-10				28	10				SC	SANDSTONE, CLAYEY
3	6	5			35.2				450		SC-SM	SANDSTONE, SILTY, CLAYEY



TEST BORING NO. 1  
 DATE DRILLED 9/12/2002  
 Job # 80503

TEST BORING NO. 2  
 DATE DRILLED 9/12/2002  
 CLIENT PROLAND GROUP, LLC  
 LOCATION ARROYA AND RAYGOR

REMARKS

DRY TO 10', 09/17/02

SAND, SLIGHTLY SILTY, FINE TO  
 COARSE GRAINED, TAN TO  
 BROWN, DENSE TO MEDIUM  
 DENSE, DRY TO MOIST

SANDSTONE, CLAYEY, FINE TO  
 COARSE GRAINED, OLIVE-TAN  
 VERY DENSE, MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
5			32	2.7	1
			18	7.7	1
10			50 8"	8.8	3
15					
20					

REMARKS

DRY TO 15', 09/17/02

SAND, SILTY, FINE GRAINED,  
 BROWN, MEDIUM DENSE,  
 MOIST  
 CLAY LENSE  
 SAND, CLAYEY, FINE TO  
 MEDIUM DENSE, TAN, MEDIUM  
 DENSE, MOIST

SANDSTONE, CLAYEY, FINE TO  
 COARSE GRAINED, OLIVE,  
 VERY DENSE, MOIST

SANDSTONE, SILTY, FINE  
 GRAINED, TAN, VERY DENSE,  
 MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
5			11	6.8	1
			29	7.2	1
10			50 11"	9.9	3
15			50 5"	10.5	3
20					



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505 ELKTON DRIVE  
 COLORADO SPRINGS, COLORADO 80907

TEST BORING LOG

DRAWN:

DATE:

CHECKED:

DATE:

JOB NO.:  
 80503

FIG NO.:

0-1

TEST BORING NO. 3  
 DATE DRILLED 9/12/2002  
 Job # 80503

TEST BORING NO. 4  
 DATE DRILLED 9/12/2002  
 CLIENT PROLAND GROUP, LLC  
 LOCATION ARROYA AND RAYGOR

REMARKS

DRY TO 10', 09/17/02

SAND, SILTY, BROWN  
 SANDSTONE, CLAYEY TO  
 SILTY, FINE TO COARSE GRAINED,  
 TAN-OLIVE, VERY DENSE, MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
			50	5.7	1
			7"		3
5			50	6.7	3
			5"		
10			50	7.2	3
			4"		
15					
20					

REMARKS

DRY TO 10', 09/17/02

SAND, CLAYEY, FINE TO MEDIUM  
 GRAINED, OLIVE BROWN, MEDIUM  
 DENSE TO DENSE, MOIST

SANDSTONE, CLAYEY, FINE TO  
 COARSE GRAINED, TAN TO  
 OLIVE, VERY DENSE, MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
			20	5.7	1
5			41	7.5	1
10			50	8.6	3
			6"		
15					
20					



**ENTECH**  
**ENGINEERING, INC.**

505 ELKTON DRIVE  
 COLORADO SPRINGS, COLORADO 80907

TEST BORING LOG

DRAWN:

DATE:

CHECKED:

DATE:

JOB NO:  
 80503

FIG NO:

D-2

TEST BORING NO. 5  
 DATE DRILLED 9/12/2002  
 Job # 80503

TEST BORING NO. 6  
 DATE DRILLED 9/12/2002  
 CLIENT PROLAND GROUP, LLC  
 LOCATION ARROYA AND RAYGOR

REMARKS

DRY TO 10', 09/12/02

SAND, SILTY, BROWN  
 SANDSTONE, SLIGHTLY SILTY  
 TO SLIGHTLY CLAYEY, FINE TO  
 COARSE GRAINED, TAN TO  
 OLIVE, VERY DENSE, MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
1					1
5			50 9"	5.4	3
10			50 8"	10.1	3
15					
20			50 7"	10.1	3

REMARKS

DRY TO 10', 09/12/02

SAND, VERY SILTY, FINE GRAINED,  
 OLIVE-TAN, DENSE, MOIST

SANDSTONE, SILTY, CLAYEY,  
 FINE GRAINED, OLIVE-TAN TO  
 BROWN, VERY DENSE,  
 MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
1					
5			37 6"	7.1	1
10			50 2"	14.5	3
15					
20				11.4	3



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TEST BORING LOG

DRAWN:

DATE:

CHECKED:

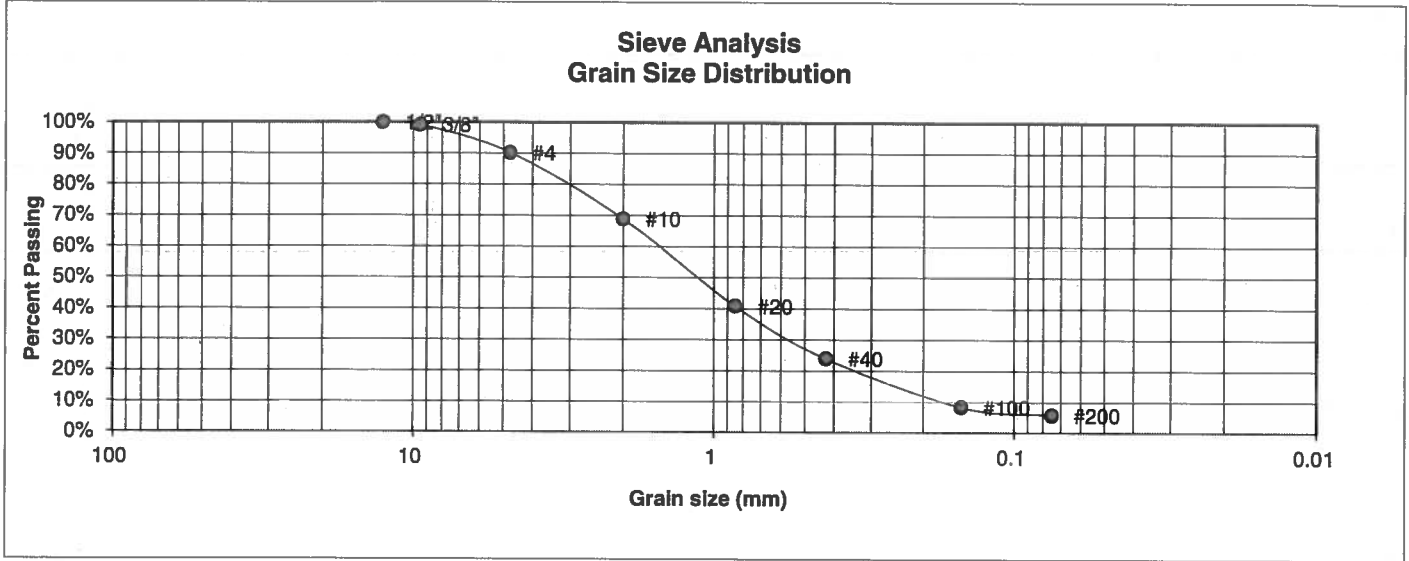
DATE:

JOB NO:  
 80503

FIG NO:

0-3

<b>UNIFIED CLASSIFICATION</b>	SM-SW	<b>CLIENT</b>	PROLAND GROUP, LLC
<b>SOIL TYPE #</b>	1	<b>PROJECT</b>	ARROYA AND RAYGOR
<b>TEST BORING #</b>	PH-5	<b>JOB NO.</b>	80503
<b>DEPTH (FT)</b>	5	<b>TEST BY</b>	BL



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	99.2%
4	90.2%
10	68.9%
20	40.8%
40	23.9%
100	8.4%
200	5.8%

**Atterberg  
Limits**  
 Plastic Limit  
 Liquid Limit  
 Plastic Index

**Swell**  
 Moisture at start  
 Moisture at finish  
 Moisture increase  
 Initial dry density (pcf)  
 Swell (psf)



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**LABORATORY TEST  
RESULTS**

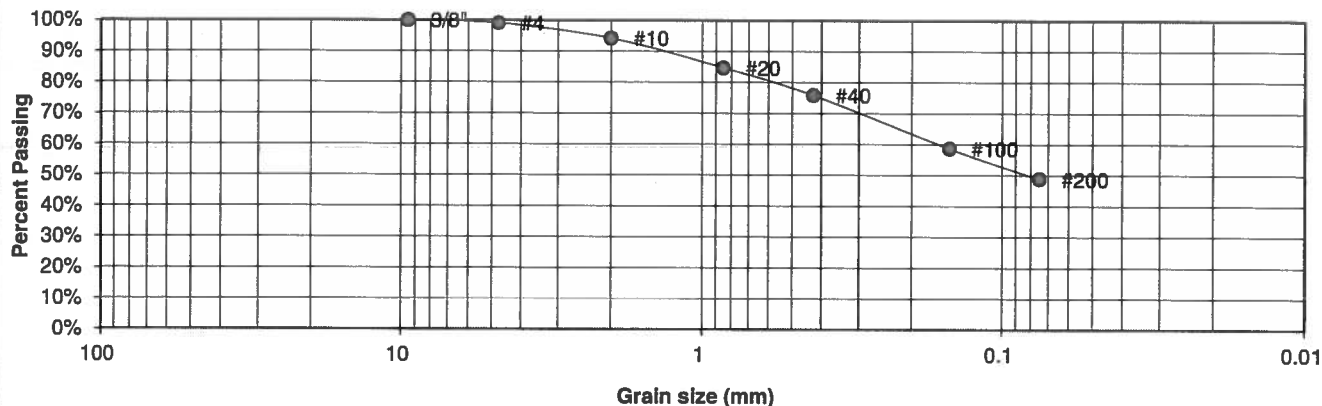
DRAWN:	DATE:	CHECKED:	DATE:
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JOB NO.:  
80503

FIG NO.:  
D-4

<b>UNIFIED CLASSIFICATION</b>	SC	<b>CLIENT</b>	PROLAND GROUP, LLC
<b>SOIL TYPE #</b>	1	<b>PROJECT</b>	ARROYA AND RAYGOR
<b>TEST BORING #</b>	PH-7	<b>JOB NO.</b>	80503
<b>DEPTH (FT)</b>	2-5	<b>TEST BY</b>	BL

**Sieve Analysis  
Grain Size Distribution**



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.1%
10	94.2%
20	84.7%
40	75.8%
100	58.6%
200	48.8%

<u>Atterberg Limits</u>	
Plastic Limit	19
Liquid Limit	41
Plastic Index	22

<u>Swell</u>	
Moisture at start	12.5%
Moisture at finish	20.8%
Moisture increase	8.3%
Initial dry density (pcf)	105
Swell (psf)	2230



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**LABORATORY TEST  
RESULTS**

DRAWN:	DATE:	CHECKED:	DATE:
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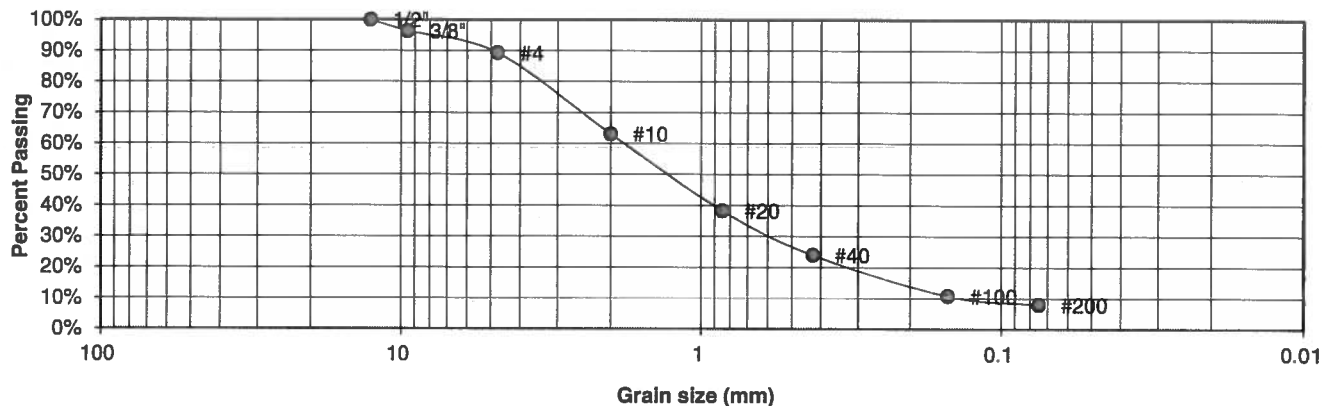
JOB NO.:  
80503

FIG NO.:

0-5

<u>UNIFIED CLASSIFICATION</u>	SM-SW	<u>CLIENT</u>	PROLAND GROUP, LLC
<u>SOIL TYPE #</u>	1	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	1	<u>JOB NO.</u>	80503
<u>DEPTH (FT)</u>	2-3	<u>TEST BY</u>	BL

**Sieve Analysis  
Grain Size Distribution**



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	96.4%
4	89.3%
10	63.0%
20	38.3%
40	23.9%
100	10.8%
200	8.0%

Atterberg  
Limits  
Plastic Limit  
Liquid Limit  
Plastic Index

Swell  
Moisture at start  
Moisture at finish  
Moisture increase  
Initial dry density (pcf)  
Swell (psf)



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**LABORATORY TEST  
RESULTS**

DRAWN:

DATE:

CHECKED:

DATE:

JOB NO.:  
80503

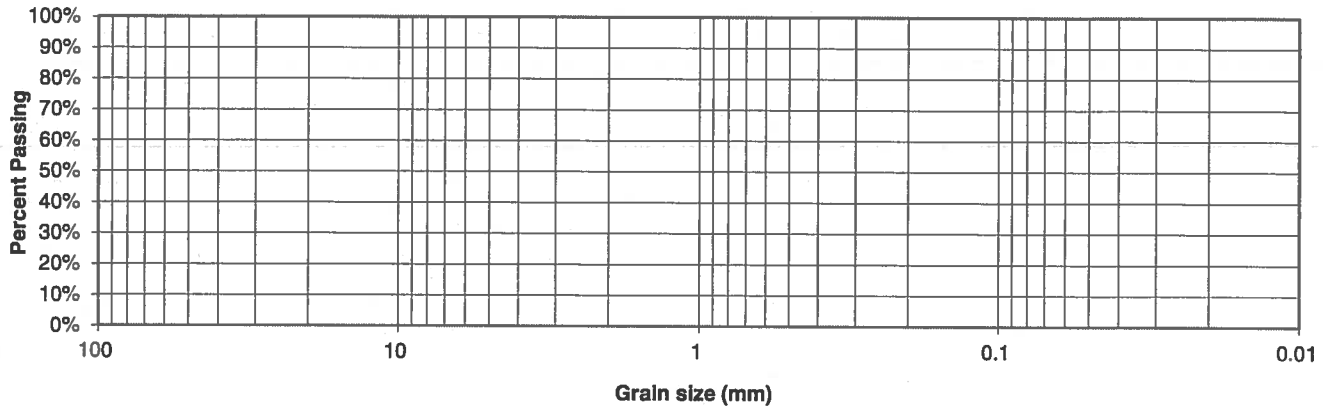
FIG NO.:

D-6



<u>UNIFIED CLASSIFICATION</u>	SC	<u>CLIENT</u>	PROLAND GROUP, LLC
<u>SOIL TYPE #</u>	1	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	4	<u>JOB NO.</u>	80503
<u>DEPTH (FT)</u>	2-5	<u>TEST BY</u>	BL

**Sieve Analysis  
Grain Size Distribution**



U.S.  
Sieve #  
3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

Percent  
Finer

Atterberg  
Limits

Plastic Limit	14
Liquid Limit	28
Plastic Index	14

Swell

Moisture at start	6.7%
Moisture at finish	17.6%
Moisture increase	10.9%
Initial dry density (pcf)	106
Swell (psf)	690



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**LABORATORY TEST  
RESULTS**

DRAWN:

DATE:

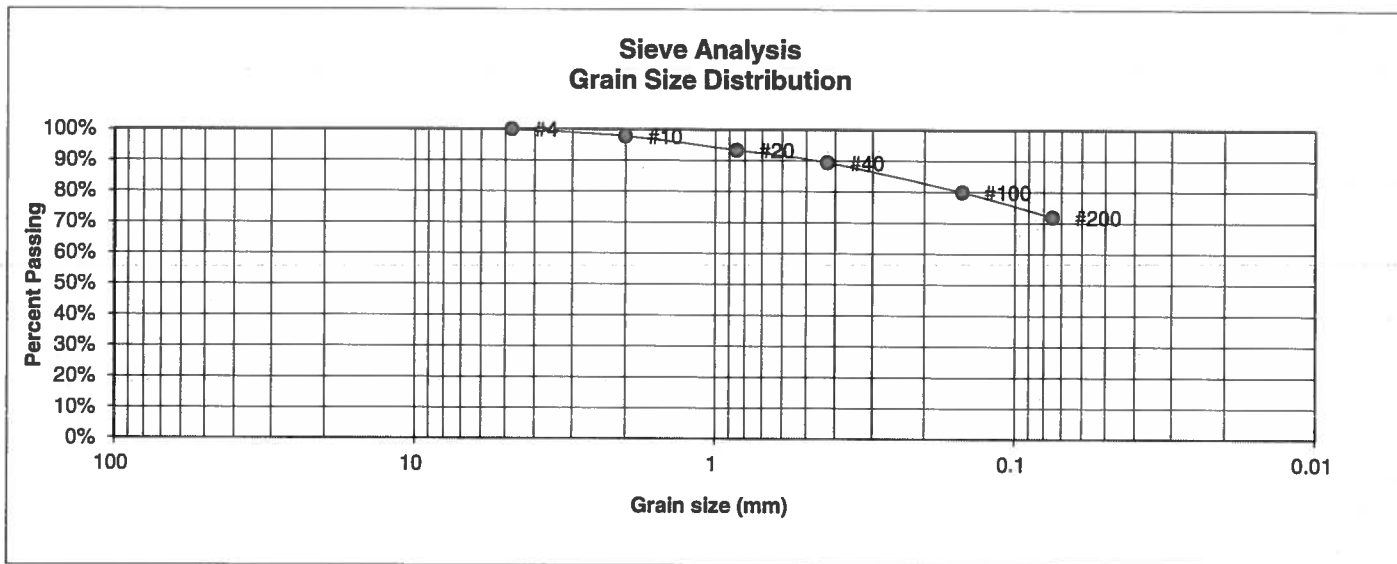
CHECKED:

DATE:

JOB NO.:  
80503

FIG NO.:  
D-7

<u>UNIFIED CLASSIFICATION</u>	CL	<u>CLIENT</u>	PROLAND GROUP, LLC
<u>SOIL TYPE #</u>	2	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	PH-2	<u>JOB NO.</u>	80503
<u>DEPTH (FT)</u>	5	<u>TEST BY</u>	BL



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	97.7%
20	93.4%
40	89.4%
100	80.0%
200	71.9%

<u>Atterberg Limits</u>	
Plastic Limit	22
Liquid Limit	40
Plastic Index	18

<u>Swell</u>	
Moisture at start	
Moisture at finish	
Moisture increase	
Initial dry density (pcf)	
Swell (psf)	



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## LABORATORY TEST RESULTS

DRAWN:

DATE:

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DATE:

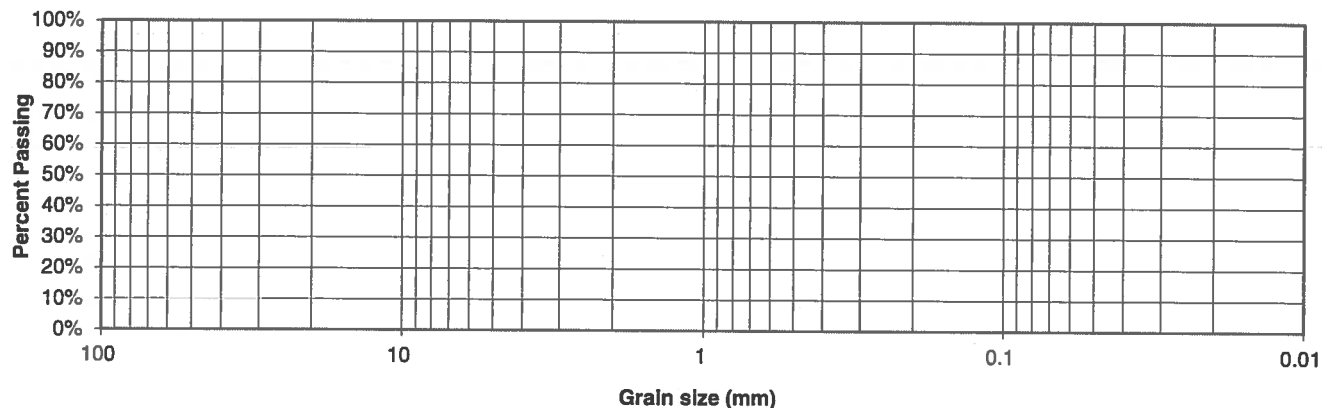
JOB NO.:  
80503

FIG NO.:

08

<u>UNIFIED CLASSIFICATION</u>	CL	<u>CLIENT</u>	PROLAND GROUP, LLC
<u>SOIL TYPE #</u>	2	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	2	<u>JOB NO.</u>	80503
<u>DEPTH (FT)</u>	3	<u>TEST BY</u>	BL

**Sieve Analysis  
Grain Size Distribution**



U.S.  
Sieve #  
3"  
1 1/2"  
3/4"  
1/2"  
3/8"  
4  
10  
20  
40  
100  
200

Percent  
Finer

Atterberg  
Limits  
Plastic Limit  
Liquid Limit  
Plastic Index

Swell  
Moisture at start 8.7%  
Moisture at finish 19.2%  
Moisture increase 10.6%  
Initial dry density (pcf) 106  
Swell (psf) 2060



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**LABORATORY TEST  
RESULTS**

DRAWN:

DATE:

CHECKED:

DATE:

JOB NO.:  
80503

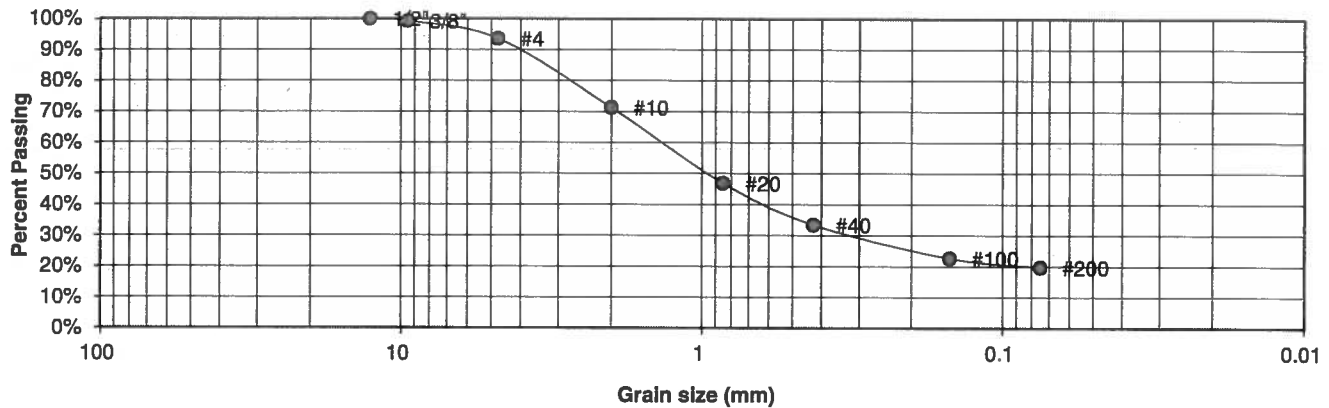
FIG NO.:

09

<b>UNIFIED CLASSIFICATION</b>	SC
<b>SOIL TYPE #</b>	3
<b>TEST BORING #</b>	2
<b>DEPTH (FT)</b>	10

<b>CLIENT</b>	PROLAND GROUP, LLC
<b>PROJECT</b>	ARROYA AND RAYGOR
<b>JOB NO.</b>	80503
<b>TEST BY</b>	BL

**Sieve Analysis  
Grain Size Distribution**



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	99.2%
4	93.5%
10	71.2%
20	46.8%
40	33.4%
100	22.6%
200	19.7%

**Atterberg  
Limits**  
Plastic Limit  
Liquid Limit  
Plastic Index

**Swell**  
Moisture at start 9.3%  
Moisture at finish 18.3%  
Moisture increase 9.0%  
Initial dry density (pcf) 111  
Swell (psf) 430



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**LABORATORY TEST  
RESULTS**

DRAWN:

DATE:

CHECKED:

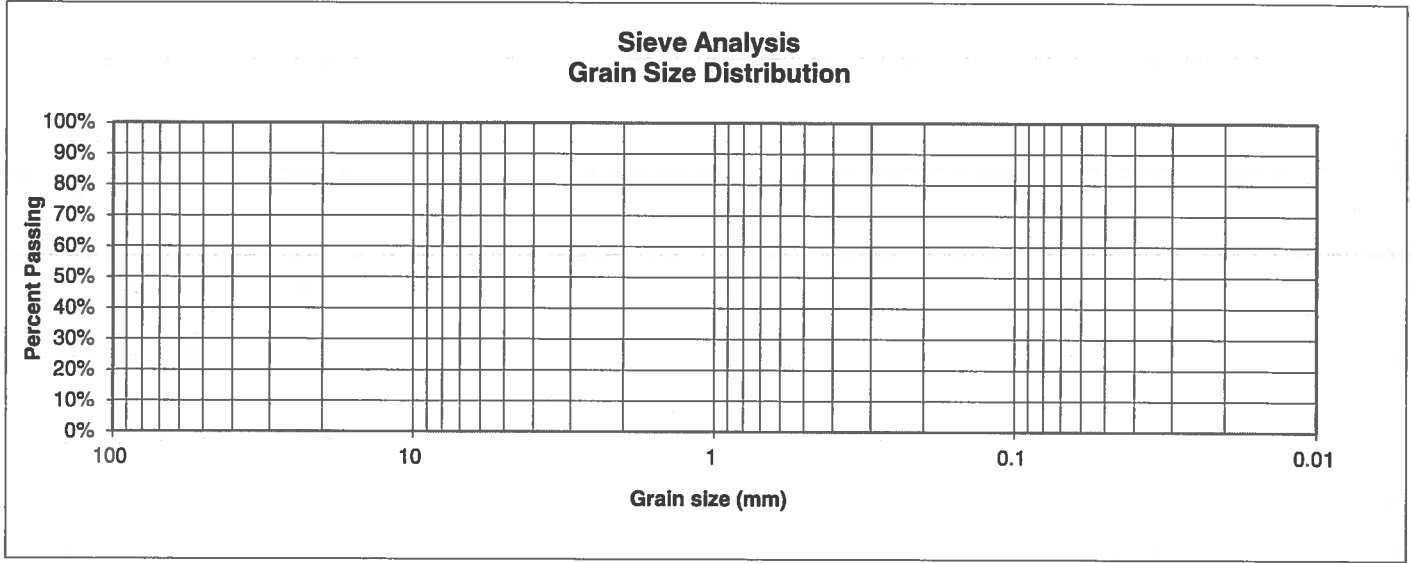
DATE:

JOB NO.:  
80503

FIG NO.:

D-10

<u>UNIFIED CLASSIFICATION</u>	SC	<u>CLIENT</u>	PROLAND GROUP, LLC
<u>SOIL TYPE #</u>	3	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	3	<u>JOB NO.</u>	80503
<u>DEPTH (FT)</u>	5-10	<u>TEST BY</u>	BL



<u>U.S. Sieve #</u>	<u>Percent Finer</u>	<u>Atterberg Limits</u>	
3"		Plastic Limit	18
1 1/2"		Liquid Limit	28
3/4"		Plastic Index	10
1/2"			
3/8"		<u>Swell</u>	
4		Moisture at start	
10		Moisture at finish	
20		Moisture increase	
40		Initial dry density (pcf)	
100		Swell (psf)	
200			



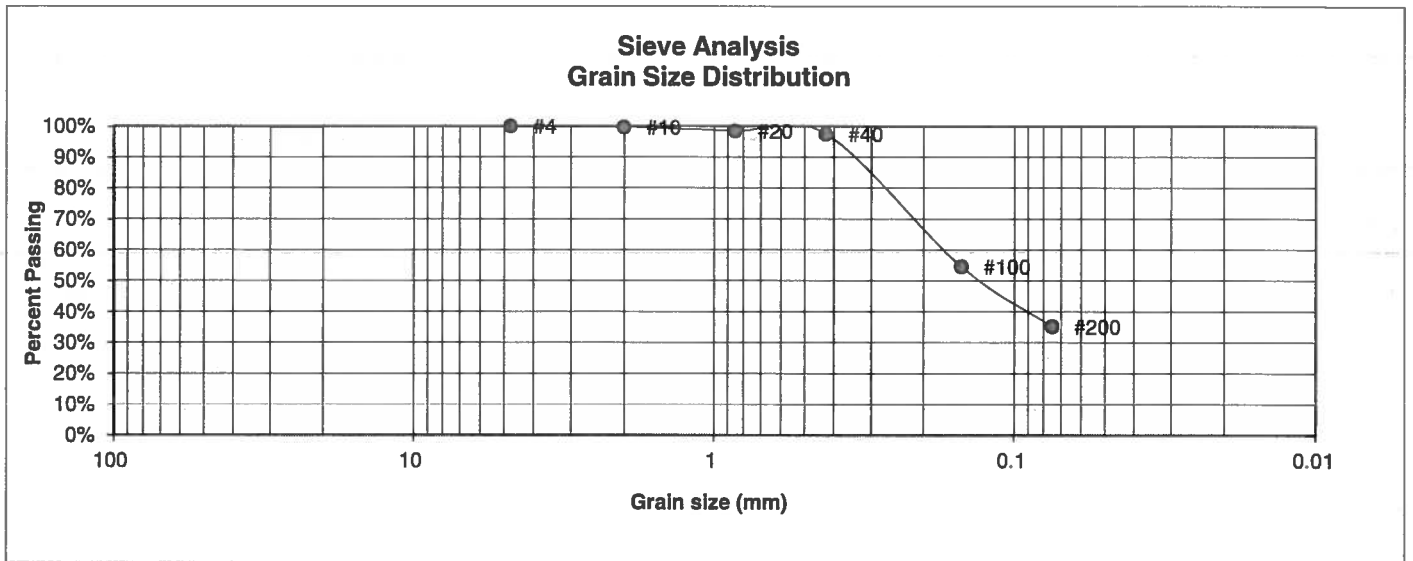
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**LABORATORY TEST  
RESULTS**

DRAWN:	DATE:	CHECKED:	DATE:
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JOB NO.:  
80503  
  
FIG NO.:  
D-11

<u>UNIFIED CLASSIFICATION</u>	SC-SM	<u>CLIENT</u>	PROLAND GROUP, LLC
<u>SOIL TYPE #</u>	3	<u>PROJECT</u>	ARROYA AND RAYGOR
<u>TEST BORING #</u>	6	<u>JOB NO.</u>	80503
<u>DEPTH (FT)</u>	5	<u>TEST BY</u>	BL



U.S.  
Sieve #

Percent  
Finer

Atterberg  
Limits

Plastic Limit  
Liquid Limit  
Plastic Index

3"

1 1/2"

3/4"

1/2"

3/8"

4

10

20

40

100

200

100.0%

99.7%

98.5%

97.4%

54.6%

35.2%

Swell

Moisture at start

Moisture at finish

Moisture increase

Initial dry density (pcf)

Swell (psf)

18.9%

22.5%

3.6%

102

450



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## LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:

DATE:

JOB NO.:  
80503

FIG NO.:

D-12



## APPENDIX E: Soil Survey Descriptions

## El Paso County Area, Colorado

### 19—Columbine gravelly sandy loam, 0 to 3 percent slopes

#### Map Unit Setting

*National map unit symbol:* 367p

*Elevation:* 6,500 to 7,300 feet

*Mean annual precipitation:* 14 to 16 inches

*Mean annual air temperature:* 46 to 50 degrees F

*Frost-free period:* 125 to 145 days

*Farmland classification:* Not prime farmland

#### Map Unit Composition

*Columbine and similar soils:* 97 percent

*Minor components:* 3 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Columbine

##### Setting

*Landform:* Flood plains, fan terraces, fans

*Down-slope shape:* Linear

*Across-slope shape:* Linear

*Parent material:* Alluvium

##### Typical profile

*A - 0 to 14 inches:* gravelly sandy loam

*C - 14 to 60 inches:* very gravelly loamy sand

##### Properties and qualities

*Slope:* 0 to 3 percent

*Depth to restrictive feature:* More than 80 inches

*Drainage class:* Well drained

*Runoff class:* Very low

*Capacity of the most limiting layer to transmit water (Ksat):* High to very high (5.95 to 19.98 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water supply, 0 to 60 inches:* Very low (about 2.5 inches)

##### Interpretive groups

*Land capability classification (irrigated):* 4e

*Land capability classification (nonirrigated):* 6e

*Hydrologic Soil Group:* A

*Ecological site:* R049XY214CO - Gravelly Foothill

*Hydric soil rating:* No

#### Minor Components

##### Fluvaquentic haplaquolls

*Percent of map unit:* 1 percent

*Landform:* Swales  
*Hydric soil rating:* Yes

**Other soils**

*Percent of map unit:* 1 percent  
*Hydric soil rating:* No

**Pleasant**

*Percent of map unit:* 1 percent  
*Landform:* Depressions  
*Hydric soil rating:* Yes

## Data Source Information

Soil Survey Area: El Paso County Area, Colorado  
Survey Area Data: Version 19, Aug 31, 2021

## El Paso County Area, Colorado

### 71—Pring coarse sandy loam, 3 to 8 percent slopes

#### Map Unit Setting

*National map unit symbol:* 369k

*Elevation:* 6,800 to 7,600 feet

*Farmland classification:* Not prime farmland

#### Map Unit Composition

*Pring and similar soils:* 85 percent

*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Pring

##### Setting

*Landform:* Hills

*Landform position (three-dimensional):* Side slope

*Down-slope shape:* Linear

*Across-slope shape:* Linear

*Parent material:* Arkosic alluvium derived from sedimentary rock

##### Typical profile

*A - 0 to 14 inches:* coarse sandy loam

*C - 14 to 60 inches:* gravelly sandy loam

##### Properties and qualities

*Slope:* 3 to 8 percent

*Depth to restrictive feature:* More than 80 inches

*Drainage class:* Well drained

*Runoff class:* Low

*Capacity of the most limiting layer to transmit water (Ksat):* High  
(2.00 to 6.00 in/hr)

*Depth to water table:* More than 80 inches

*Frequency of flooding:* None

*Frequency of ponding:* None

*Available water supply, 0 to 60 inches:* Low (about 6.0 inches)

##### Interpretive groups

*Land capability classification (irrigated):* None specified

*Land capability classification (nonirrigated):* 3e

*Hydrologic Soil Group:* B

*Ecological site:* R048AY222CO - Loamy Park

*Hydric soil rating:* No

#### Minor Components

##### Pleasant

*Percent of map unit:*

*Landform:* Depressions

*Hydric soil rating:* Yes

**Other soils**

*Percent of map unit:*

*Hydric soil rating:* No

**Data Source Information**

Soil Survey Area: El Paso County Area, Colorado

Survey Area Data: Version 19, Aug 31, 2021