



FINAL DRAINAGE REPORT

ELK VIEW ESTATES

El Paso County, CO

PREPARED FOR:

**Elk Ridge Development, LLC
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PREPARED BY:

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DATE:

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ENGINEERS CERTIFICATION STATEMENT:

"The attached drainage plan and report were prepared under my direct supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report."

SIGNATURE: _____

(Affix Seal)

Treven Edwards

(For and on behalf of Galloway & Company)

DEVELOPERS CERTIFICATION STATEMENT:

"I, _____ the developer have read and will comply with all of the requirements in this drainage report and plan."

Name of Developer

Authorized Signature

Date

Printed Name

Title

Address

City, State Zip

EL PASO COUNTY ONLY:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volume 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

Joshua J. Palmer, P.E.
County Engineer / ECM Administrator

Date

Conditions:

I. INTRODUCTION

Purpose:

This document is to be the Final Drainage Report (FDR) for the proposed Elk View Estates Subdivision, hereon referred to as "Site." The purpose of this report is to identify on and offsite drainage patterns, analyze existing and proposed hydraulic improvements required by this development, and ensure there are no significant changes to existing drainage patterns in compliance with the El Paso County Land Development Code (LDC) and Drainage Criteria Manual (DCM).

Location:

The project site is located entirely within the boundaries of Parcel 6223000044 as shown in the El Paso County Assessor records. The Site is located on the north side of Old Ranch Rd., approximately 1,300-ft west from the Milam Rd./Union Blvd. intersection with Old Ranch Rd. to the southeast property corner. More specifically, the Site is located in a portion of the Southeast One-Quarter of Section 23, Township 12 South, Range 66 West of the 6th Principal Meridian, County of El Paso, State of Colorado.

The site is bounded by single-family residential properties on the north, east and west, and by Old Ranch Road public Right-of-Way to the south. A vicinity map is provided in **Figure 1** below for reference.



Figure 1 – Vicinity Map

Description of Property:

The Site occupies approximately 17.10 acres of undeveloped land, consisting of natural vegetation and large trees, densely concentrated on the north side of the Site and a small dense portion on the south, adjacent to Old Ranch Rd. The site is currently zoned as RR-5 and has been rezoned to RR-2.5. The proposed development will be subdivided into (6) 2.5-acre or larger lots and the addition of private access road along the eastern property boundary, providing access to the future lots from Old Ranch Rd.

Description of Floodplain:

The site is located within Zone X – Areas determined to be outside of the 0.2% annual chance floodplain, according to the FEMA Flood Insurance Rate Map (FIRM) panel #08041C0526G, effective December 7, 2018. A copy of the FEMA FIRM panel can be found in **Appendix A** for reference.

Description of Soils:

Soils can be classified in four different hydrologic groups, A, B, C, or D to help predict stormwater runoff rates. Hydrologic group “A” is characterized by deep, well-drained coarse-grained soils with a rapid infiltration rate when thoroughly wet and having a low runoff potential. Group “D” typically has a clay layer at or near to the surface, or very shallow depth to impervious bedrock and has a very slow infiltration rate and a high runoff potential. According to the Natural Resources Conservation Service (NRCS) Web Soil Survey, the project site consists of a mix of soil types and Hydrologic Soil Groups (HSGs) shown in **Table 1** below.

Table 1 – USDA NRCS Soil Data

Soil Name	HSG	Percent of Site
Kettle Fravelly Loam Sand	B	44.6%
Peyton-Pring Complex (3 to 8 % slopes)	B	11.6%
Peyton-Pring Complex (8 to 15 % Slopes)	B	43.8%

The predominant HSG for the Site is type ‘B’. A copy of the NRCS Web Soil Survey report can be found in **Appendix A** for reference.

Other Site Considerations:

- The Site is not located within a Streamside Overlay zone.
- The Site is not adjacent, nor contain, waters of the State or Federal Waters.
- An environmental study was not conducted, however, there does not appear to be any wetlands on-site that would be impacted by the Site development.
- According to the OTWS report (prepared by D&D Engineering, LLC, dated April 20, 2023), shallow seasonal ground water was encountered in test pits ranging from 6-inches to 7-feet deep.

II. DESIGN CRITERIA

Required Criteria:

The analysis and design of the drainage for the Site were prepared in accordance with the criteria set forth from the following resources:

- El Paso Drainage Criteria Manual (DCM), Volumes 1 & 2, (With latest updates); hereon referred to as DCM.
- City of Colorado Springs Drainage Criteria Manual 2014 Update: Chapter 6 and Section 3.2.1 of Chapter 13; heron referred to as COSDCM.
- Mile High Flood District (MHFD) Urban Storm Drainage Criteria Manual (USDCM), Volumes 1-3 latest updates; hereon referred to as USDCM.

HYDROLOGIC CRITERIA:

Rainfall:

The drainage analysis are based on rainfall depths for the Site as provided by Table 6-2 of the DCM as shown below in **Table 2**.

Table 2 – Rainfall Data per Table 6-2 of DCM, Vol. 1

Return Period	1-Hr. Depth (in.)	6-Hr. Depth (in.)	24-Hr. Depth (in.)
2-year	1.19	1.70	2.10
5-year	1.50	2.10	2.70
10-year	1.75	2.40	3.20
25-year	2.00	2.90	3.60
50-year	2.25	3.20	4.20
100-year	2.52	3.50	4.60

Based on elevation Z = 6,840 ft / 100

This table is based on the City of Colorado Springs Intensity Duration Frequency (IDF) curve created from NOAA Atlas 2, Volume III, Regional 1 rainfall intensity data for an elevation of 6,840-ft.

The above rainfall methodology is not appropriate in determining the Water Quality Capture Volume (WQCV). Therefore, the MHFD USDCM methods for the WQCV are utilized, as approved by the DCM, Vol. 1, Section 2.3.

Runoff:

Because the drainage basins for the site are less than 130-acres, the Rational Method is utilized to determine the runoff peak discharges, which is calculated by the following formula from the DCM, Vol. 1, Sec. 3.0:

$$Q = C \cdot i \cdot A \quad \text{(Equation 6-5)}$$

Where:

Q = maximum rate of runoff (cubic feet per second [cfs])

C = runoff coefficient (discussed below)

I = intensity of rainfall for a duration equal to the time of concentration (inches per hour [in/hr])

Runoff peak discharges were calculated for the 5-year and 100-year return periods, which are hereon referred to as the minor and major storm events, respectively.

Runoff Coefficient:

The rational method runoff coefficient 'C' is calculated for each drainage basin by using a weighted average of the land use and/or surface characteristics within the particular drainage basin, which follows the following formula from the DCM, Vol. 1, Sec. 3.0:

$$C = (C_1A_1 + C_2A_2 + C_3A_3 + \dots C_iA_i)/A_t \quad \text{(Equation 6-6)}$$

Where:

C = Composite runoff coefficient for the drainage basin

C_i = runoff coefficient for subarea corresponding to surface type or land use

A_i = area of surface type corresponding to C_i (Acres)

A_t = total area of all subareas for which composite runoff coefficient applies (Acres)

i = number of surface types in the drainage area

The sub-area runoff coefficient is determined from Table 6-6 of the DCM which provides values based on land use/surface characteristic, percent impervious, return period, and hydrologic soil group. A copy of the table can be found in **Appendix A** for reference.

Time of Concentration:

The Time of Concentration was calculated using the equations in the DCM, Vol.1, Sec. 3.2 as follows:

$$t_c = t_i + t_t \quad \text{(Equation 6-7)}$$

Where:

t_c = time of concentration (min)

t_i = overland (initial) flow time (min)

t_t = travel time of concentrated flow (i.e. ditch, curb/gutter, etc. [min])

The overland flow time, t_i, is calculated by the following equation:

$$t_i = \frac{0.395(1.1 - C_s)\sqrt{L}}{S^{0.33}} \quad \text{(Equation 6-8)}$$

Where:

t_i = overland (initial) flow time (min)

C_5 = runoff coefficient for a 5-year return period

L = length of overland flow (300-ft max for non-urban, 100-ft max for urban)

S = average drainage basin slope (ft/ft)

The concentrated flow (or channelized flow) is calculated by the following equations:

$$V = C_v S_w^{0.5} \quad \text{(Equation 6-9)}$$

Where:

V = velocity (feet per second [ft/s or fps])

C_v = conveyance coefficient (see **Table 3** below)

S_w = watercourse slope (ft/ft)

$$t_t = \frac{L}{60 \cdot V}$$

Where:

t_t = travel time of concentrated flow (min)

L = length of concentrated flow (feet)

V = velocity (ft/s)

Table 3 - Conveyance coefficient (C_v) from Table 6-7 of DCM

Type of Land Surface	C_v
Heavy Meadow	2.5
Tillage/field	5
Riprap (not buried)	6.5
Short Pasture and Lawns	7
Nearly Bare Ground	10
Grassed Waterway	15
Paved areas and shallow paved swales	20

The final time of concentration (t_c) is then checked to according the following equation:

$$t_c = \frac{L}{180+10} \quad \text{(Equation 6-10)}$$

Where:

t_c = max time of concentration at 1st design point in urban watershed (min)

L = waterway length (ft)

Where calculations result in a time less than 5-minutes, the minimum time of concentration is analyzed at 5-minutes.

HYDRAULIC CRITERIA:

Street Capacity & Inlets:

Roadside ditches were analyzed utilizing the Federal Highway Administration (FHWA) Hydraulic Toolbox *Version 5.4*. The roadside ditches were sized to convey the minor and major storm events and provide water quality reduction (discussed later in the report). Figure 8-10 (Type C Inlet Capacity Chart) from the City of Colorado Springs DCM, Volume 1 was utilized to ensure adequate inlet capacity. A copy of this chart can be found in **Appendix C**.

Roadside ditch capacity analysis within Hydraulic Toolbox uses Manning's equation, which estimates the ditch flow capacity based on the channel geometry, slope, and land characteristics based on the following equation:

$$Q = \frac{1.49}{n} \cdot A \cdot R^{0.67} \cdot S^{0.5} \quad \text{(Equation 10-1)}$$

Where:

Q = Channel discharge (cfs)

n = Manning's roughness coefficient (based on channel material and/or lining)

A = cross sectional area of channel (ft²)

R = hydraulic radius of channel = A / P (ft)

P = wetted perimeter of channel (ft)

S = slope of energy gradient (ft/ft)

The roadside ditch stability is analyzed within Hydraulic Toolbox by computing the side slope and bottom shear stresses. The channel is deemed to be stable if the computed shear stress is less than the critical shear stress for type of channel material or lining utilized.

Culverts:

Existing and proposed culverts were analyzed using the FHWA HY-8 software. Entrance loss coefficients utilized in the analysis were taken from the DCM, Vol. 1, Ch. 9, Table 9-1. A copy of the table can be found in **Appendix A** for reference.

Water Quality:

The SCM-Design-v4.02 software is utilized in calculating the on-site required Water Quality Capture Volume (WQCV) based on the concepts provided in the MHFD USDCM Volume 3, Chapter 4. This software also is utilized in the analysis of Water Quality Reduction by analyzing sub-areas within the drainage basin, where proposed impervious areas (PIA's) are routed across receiving pervious areas (RPA's) prior to entering the downstream stormwater conveyance system.

III. HISTORIC / EXISTING DRAINAGE

Major Drainage Basin:

The Site is within the *“Kettle Creek Drainage Basin Planning Study”* (DBPS-K), prepared by JR Engineering, LLC., Dated May 5, 2015 and the *“Kettle Creek – Old Ranch Road Tributary Drainage Basin Planning Study”* (DBPS-KO), prepared by JR Engineering, LLC., Dated April 2001. Specifically, the Site is within **sub-basin area S1** of the DBPS-K, and within **sub-basin areas D1 & D2** of the DBPS-KO. The project site is also described as part of the Cordera Master Development Drainage Plan (MDDP) which is located south of Old Ranch Road within the City of Colorado Springs. This MDDP uses excerpts from the DBPS’s described above for drainage patterns for this site.

No additional previous or on-going drainage reports were found applicable to the Site.

Overall Basin Description:

Existing topography generally slopes from the northeast to the southwest at approximately 6.0%. A defined natural channel exists on the site running across the south-central portion of the site, conveying flows to the west. The flows then are conveyed under Studebaker St. via an existing (private) pipe culvert.

Sub-Basin Descriptions:

Drainage sub-basins in their current, existing, condition have been analyzed and denoted as **“K”** for sub-basins within DBPS-K tributary and **“KO”** for sub-basin within the DBPS-KO tributary. Full analysis and results of the sub-basins hydrology can be found in **Appendix B**. An existing conditions drainage basin map has been provided in **Appendix E** and can be used to reference the basins discussed below:

Basin K-1 (2.1 ac, $Q_5 = 0.7$ cfs, $Q_{100} = 4.8$ cfs): a sub-basin defining a portion of the site in the northwest corner of the property consisting of an existing dirt roadway and undeveloped land. Runoff generally sheet flows across the sub-basin, following historic drainage patterns, to the north. Flows are then routed over the existing dirt road and off-site to the northern adjacent parcels eventually being captured and conveyed west in a roadside ditch along Arrowhead Drive.

Basin KO-D10 (41.6 ac, $Q_5 = 11.8$ cfs, $Q_{100} = 50.2$ cfs): a sub-basin defining a large portion of offsite tributary area to the east consisting of undeveloped land, large acreage single family residences, and a portion of Milam Road. This sub-basin was created to match the portion of Basin D1 of the DBPS-KO drainage areas outside of the sites property boundaries. Runoff generally sheet flows to the west, channelizing centrally within the sub-basin. Channelized flows are then routed west, via an existing vegetated natural channel, where it then drains across **Basin KO-D20 at DP D10**.

Basin KO-D11 (8.4 ac, $Q_5 = 4.4$ cfs, $Q_{100} = 16.2$ cfs): a sub-basin defining a large portion of offsite tributary area to the east consisting of undeveloped land, large acreage single family residences, and a portion of Milam Road. This sub-basin was created to match the portion of Basin D1 of the DBPS-KO drainage areas outside of the sites property boundaries. Runoff generally sheet flows to the west, channelizing centrally within the sub-basin. Channelized flows are then routed west, via an existing vegetated natural channel, where it then drains across **Basin KO-D20 at DP D10**.

Basin KO-D20a (13.0 ac, $Q_5 = 3.1$ cfs, $Q_{100} = 20.9$ cfs): a sub-basin defining a large portion of the site, south of **Basin K-1**. This basin consists of native vegetation and is undeveloped. Runoff generally sheet flows south before becoming channelized in the existing vegetated natural channel on the southern portion of the basin. Channelized flows are then routed west, to an existing (private) 18" CMP culvert under the private Studebaker Road at **DP D11**. Flows then discharge, via the existing pipe culvert, to **Basin KO-D23**.

Basin KO-D20b (2.3 ac, $Q_5 = 0.8$ cfs, $Q_{100} = 5.0$ cfs): a sub-basin defining the remaining portion of the site, south of **Basin K-1** and west of **Basin KO-D20a**. This basin consists of native vegetation and is undeveloped. Runoff generated generally flows southwest where it flows off-site at **Design Point D12** at Studebaker St. Flows at this design point are assumed to continue south to **DP D11**.

Basin KO-D21 (2.5 ac, $Q_5 = 2.1$ cfs, $Q_{100} = 7.8$ cfs): a sub-basin located off-site, on the central-western side of the site. This basin consists of a large acreage, single-family, residential lot. Runoff from this basin generally sheet flows south, across **Basin KO-D20** draining to **DP D11**.

Basin KO-D22 (0.6 ac, $Q_5 = 0.5$ cfs, $Q_{100} = 1.7$ cfs): a sub-basin defining a small portion of off-site area adjacent to the southeast corner of the site. This sub-basin was created to match the portion of Basin D2 of the DBPS-KO drainage areas outside of the sites property boundaries. Runoff from this basin generally sheet flows south, before becoming channelized in roadside ditch, along the north of Old Ranch Road. Channelized flows from the roadside ditch continue into **Basin KO-D20** draining to **DP D11**.

Design Point D11 ($Q_5 = 18.2$ cfs, $Q_{100} = 81.2$ cfs): This design point is located at the southwest portion of the site and represents the total runoff upstream to the existing, private, 18" CMP that crosses under the private Studebaker Road. In the existing condition, the culvert does not have capacity for the minor or major storm events. Therefore, excess runoff ponds up on the east side of Studebaker Road until it overtops the roadway and continues west, across **Basin KO-D23**.

Basin KO-D23 (22.4 ac, $Q_5 = 13.8$ cfs, $Q_{100} = 51.0$ cfs): a sub-basin defining the off-site, downstream, area west of the site. This sub-basin was created to match the portion of Basin D2 of the DBPS-KO drainage areas outside of the sites property boundaries. Runoff generally sheet flows to the south, channelizing centrally within the sub-basin. Channelized flows are then routed south, via an existing vegetated natural channel, to an existing, public, 18" CMP pipe culvert under Old Ranch Road, at **DP D1**.

IV. PROPOSED DRAINAGE

General Concept:

Elk View Estates will be developed as a single-family rural development. Runoff will primarily follow existing drainage patterns, across the proposed lots, to the southwest. Runoff from the new access roads will be collected in roadside ditches to provide a runoff reduction for water quality before being discharged to the existing natural flow paths on-site. A new, private, pipe culvert will be constructed under the new access road to allow off-site flows to the east to continue westward, following existing drainage patterns identified in the DBPS-KO.

Sub-Basin Descriptions:

Drainage sub-basins in there, proposed, future condition have been analyzed and denoted as “K” for sub-basins within DBPS-K tributary and “KO” for sub-basin within the DBPS-KO tributary. Off-site sub-basins **KO-D10**, **KO-D11**, **KO-D21**, and **KO-D23** were not changed from the existing condition and therefore are not added to the descriptions below. Full analysis and results of the sub-basins hydrology can be found in **Appendix B**. A proposed conditions drainage basin map has been provided in **Appendix E** and can be used to reference the basins discussed below:

Basin K-12 (2.2 ac, $Q_5 = 1.8$ cfs, $Q_{100} = 6.5$ cfs): Located along the northern property boundary, this sub-basin consists of an existing private access road and a portion of 2.5-acre single-family residential lots. Runoff from this basin generally sheet flows to the north / northwest, ultimately flowing off-site following historic drainage patterns, and are captured and conveyed west in a roadside ditch along Arrowhead Drive

Basin KO-D201a (0.4 ac, $Q_5 = 1.4$ cfs, $Q_{100} = 2.8$ cfs): Located along the northeastern property boundary, this sub-basin consists of a portion of the east side of the north/south running private roadway. Runoff from this basin generally sheet flows east, across the private roadway, to a roadside ditch. Flows are then conveyed via the roadside ditch south, to a private, dual, 30-inch RCP pipe culvert at **DP D10**.

Design Point D10 ($Q_5 = 12.3$ cfs, $Q_{100} = 51.2$ cfs): This design point is located at the central east portion of the property boundary and represents the total flows entering the proposed, private, dual 30-inch RCP pipe culvert that conveys flows under the proposed, private, access road to the west. The total flows conveyed to this design point consist of **KO-D10** and **KO-D201a**.

Basin KO-D201b (0.2 ac, $Q_5 = 0.4$ cfs, $Q_{100} = 0.8$ cfs): Located along the southeastern property boundary, this sub-basin consists of a portion of the east side of the north/south running private roadway. Runoff from this basin generally sheet flows east, across the private roadway, to a roadside ditch. Flows are then conveyed via the roadside ditch to a private, 18-inch RCP pipe culvert at **DP D11**.

Design Point D11 ($Q_5 = 4.6$ cfs, $Q_{100} = 16.6$ cfs): This design point is located at the central east portion of the property boundary and represents the total flows entering the proposed, private, 18-inch RCP pipe culvert that conveys flows under the proposed, private, access road to the west. The total flows conveyed to this design point consist of **KO-D11** and **KO-D201b**.

Basin KO-D202 (1.7 ac, $Q_5 = 1.4$ cfs, $Q_{100} = 5.2$ cfs): Located at the northeast portion of the site, this sub-basin consists entirely of 2.5-acre single-family residential lots. Runoff from this basin generally sheet flows to the south, following historic drainage patterns, to a proposed, private, 18-inch RCP pipe culvert at **DP D12**.

Basin KO-D203 (0.2 ac, $Q_5 = 0.9$ cfs, $Q_{100} = 1.6$ cfs): Located at the northeast portion of the property boundary, this sub-basin consists of a portion of the north side of the east/west running private roadway. Runoff from this basin generally sheet flows north, across the private roadway, to a roadside ditch. Flows are then conveyed via the roadside ditch to a private, 18-inch RCP pipe culvert at **DP D12**.

Design Point D12 ($Q_5 = 2.1$ cfs, $Q_{100} = 6.5$ cfs): This design point is located at the northeast portion of the property boundary and represents the total flows entering the proposed, private, 18-inch RCP pipe culvert that conveys flows under the proposed, private, access road to the south. The total flows conveyed to this design point consist of **KO-D202**, and **KO-D203**.

Basin KO-D204 (0.3 ac, $Q_5 = 1.4$ cfs, $Q_{100} = 2.5$ cfs): Located along the northeastern portion of the site, this sub-basin consists of a portion of the south side of the east/west running private roadway. Runoff from this basin generally sheet flows south, across the private roadway, over a +/- 14-ft wide, native vegetated, landscape buffer. Runoff then continues across **Basin KO-D207**, following historic drainage patterns, until it reaches the existing, natural, drainage swale at **DP D13**.

Basin KO-D205a (0.4 ac, $Q_5 = 1.4$ cfs, $Q_{100} = 2.7$ cfs): Located along the central eastern portion of the site, this sub-basin consists of a portion of the west side of the north/south running private roadway. Runoff from this basin generally sheet flows west, across the private roadway, over a +/- 14-ft wide, native vegetated, landscape buffer. Runoff then continues across **Basin KO-D207**, following historic drainage patterns, until it reaches the existing, natural, drainage swale at **DP D13**.

Basin KO-D205b (0.2 ac, $Q_5 = 0.9$ cfs, $Q_{100} = 1.6$ cfs): Located along the southeastern portion of the site, this sub-basin consists of a portion of the west side of the north/south running private roadway. Runoff from this basin generally sheet flows west, across the private roadway, over a +/- 14-ft wide, native vegetated, landscape buffer. Runoff then continues across **Basin KO-D207**, following historic drainage patterns, until it reaches the existing, natural, drainage swale at **DP D13**.

Design Point D13 ($Q_5 = 17.6$ cfs, $Q_{100} = 67.4$ cfs): This design point is located at the central east portion of the site and represents the total flows entering the existing, natural, drainage swale which conveys flows west, following historic drainage patterns. The total flows conveyed to this design point consist of **DP D10**, **DP D11**, **DP D12**, **KO-D204**, **KO-D205a**, and **KO-D205b**.

Basin KO-D206 (2.3 ac, $Q_5 = 1.7$ cfs, $Q_{100} = 6.3$ cfs): Located at the northwest corner of the site, this sub-basin consists entirely of 2.5-acre single-family residential lots. Runoff from this basin generally sheet flows to the south, ultimately flowing off-site following historic drainage patterns, across **Basin KO-D21**.

Design Point D14a ($Q_5 = 1.7$ cfs, $Q_{100} = 6.3$ cfs): This design point is located at the southwest portion of the site and represents the total flows entering the existing, natural, drainage swale, from the northwest. The total flows conveyed to this design point consists only of flows from **KO-D206**.

Design Point D14 ($Q_5 = 3.6$ cfs, $Q_{100} = 13.2$ cfs): This design point is located at the southwest portion of the site and represents the total flows entering the existing, natural, drainage swale, from the northwest. The total flows conveyed to this design point consist of **KO-D206** and **KO-D21**.

Basin KO-D207 (8.6 ac, $Q_5 = 5.7$ cfs, $Q_{100} = 21.1$ cfs): Located to the east and south portions of the site, this sub-basin consists entirely of 2.5-acre single-family residential lots. Runoff from this basin generally sheet flow either north or south, following historic drainage patterns, to the existing, natural, drainage swale. Flows are then conveyed west, to an existing 18" culvert at **DP D17**.

Basin KO-D221 (0.7 ac, $Q_5 = 0.5$ cfs, $Q_{100} = 2.0$ cfs): Located off-site, adjacent to the southeast property corner, this sub-basin consists of existing vegetation, a portion of roadside ditch along the north side of Old Ranch Road, and a portion of the east side of the proposed, private, roadway. Runoff from this basin generally sheet flows west to the proposed roadside ditch along the new private roadway. Flows are then conveyed to a proposed, private, Type 'C' Sump Inlet. Collected flows are then routed downstream through a proposed, private, 15-inch RCP pipe culvert at **DP D15** which conveys flows under the proposed, private, road, to follow historic drainage patterns along the north side of Old Ranch Road.

Design Point D15 ($Q_5 = 0.5$ cfs, $Q_{100} = 2.0$ cfs): This design point is located at the southeast portion of the property boundary and represents the total flows entering the proposed, private, 15-inch RCP pipe culvert that conveys flows under the proposed, private, driveway access. The total flows conveyed to this design point consist of **KO-D221**.

Basin KO-D222 (0.8 ac, $Q_5 = 0.6$ cfs, $Q_{100} = 2.4$ cfs): Located along the southern property boundary, this sub-basin consist of 2.5-acre single-family residential lots, a portion of the proposed, private, road, and a portion of roadside ditch along the north side of Old Ranch Road. Runoff form this basin generally sheet flows south, to the existing roadside ditch along Old Ranch Road. Flows are then conveyed west, to **DP D16**.

Design Point D16 ($Q_5 = 1.2$ cfs, $Q_{100} = 4.3$ cfs): This design point is located at the southwest portion of the site and represents the total flows entering the existing, natural, drainage swale, from the southeast. The total flows conveyed to this design point consist of **KO-D221** and **KO-D222**.

Design Point D17 ($Q_5 = 22.8$ cfs, $Q_{100} = 86.6$ cfs): This design point is located at the southwest portion of the site and represents the total flow at the existing, private, 18" CMP pipe culvert that conveys flows under the private Studebaker Road. The total flows conveyed to this design point consist of **D13**, **D14**, **D16**, and **KO-D207**. Flows at this design point will continue west, following historic drainage patterns. In both the historic and proposed conditions, the existing, private, 18" CMP does not have capacity for the major storm event. Therefore, flows will pond on the east side of Studebaker Road, until it overtops the roadway, and continues west to **DP D1** (following historic drainage patterns).

Design Point D1 ($Q_5 = 30.6$ cfs, $Q_{100} = 115.5$ cfs): This design point is located at the southwest property boundary and represents the total flow leaving the site. Flows are conveyed though an existing, private, 18" CMP pipe culvert under Old Ranch Road and continues south following historic drainage patterns.

The proposed and existing condition flows at similar design points downstream are compared in the summary table below to show the change in flow.

Flows Leaving Site Comparison									
	Existing				Proposed			% Flow Increase	
Design Point	Area (acres)	Q ₅ (cfs)	Q ₁₀₀ (cfs)	Design Point	Area (acres)	Q ₅ (cfs)	Q ₁₀₀ (cfs)	Q ₅ (cfs)	Q ₁₀₀ (cfs)
D10	50.6	14.9	61.8	D10, D11, D15	51.1	17.4	69.8	16.8%	12.9%
D12	2.3	0.8	5.0	D14a	2.3	1.7	6.3	112.5%	26.0%
D11	68.4	18.2	81.2	D17	68.2	22.8	86.6	25.3%	6.7%
D1	90.8	26.1	110.1	D1	90.6	30.6	115.5	17.2%	4.9%

Based on the comparison shown above, the ultimate flows at Design Point D1 (which is associated with the existing 18" pipe culvert crossing at Old Ranch Road from the DBPS-KO) maintains the same drainage area but has a minor increase in flow. This is likely due largely to the DBPS-KO assuming that the proposed site imperviousness would be 5% for >1 ac/du, which with the new criteria, is at 11% for 2.5-acre Single-Family Residential. The intended land use between the DBPS-KO and this proposed development remains the same. The two existing 18" culverts were analyzed in the existing and proposed conditions and were found to function similarly between the two conditions with most of the flow being passed over the street section and continuing downstream.

Detention is not being provided for this development as the increase in flows is minor and the increase in flow will not have an adverse impact on infrastructure downstream. As shown in the table above, Flows at **Design Point D1**, the furthest downstream design point analyzed as part of this report, shows only an increase of 4.5 cfs and 5.4 cfs in the minor and major storms, respectively. The analysis of the existing culverts crossing Old Ranch Rd. and Studebaker Rd. shows that these culverts will continue to function as they had historically and will not be adversely affected by the increase in flow caused by this development.

4-Step Process:

1. Volume Reduction Practices

Step 1 requires the site to evaluate Low Impact Development (LID) measures to reduce pollutants through infiltration measures, prior to entering the facilities required for Step 2. This is done by routing impervious areas across pervious areas prior to the capture and conveyance of runoff downstream to the maximum extent practicable.

Runoff Reduction Calculations: Runoff reduction has been calculated using the MHFD "UD-BMP_v3.07" software package. Print outs from this software are provided in **Appendix D**. Additionally, a water quality map showing the Upstream Impervious Areas (UIA) in translucent blue & Receiving Pervious Areas (RPA) in translucent green is provided in **Appendix E**.

Runoff Reduction Calculations are summarized below. The values shown are based only on the disturbed areas analyzed for water quality treatment and does not include the exclusion areas outlined in **Step 2**.

Table 4 – Runoff Reduction Summary

Downstream Design Point	Total Area, SF	DCIA, SF	Separate Pervious Area, (SPA), SF	Upstream Impervious Area (UIA), SF	RPA, SF	Volume Reduction, %
KO-Basins	79,633	391	4,575	44,613	30,054	91%

2. Volume Treatment

Step 2 requires the implementation of Permanent Control Measures (PCM's) which seek to address water quality impacts from the development site.

The proposed site utilizes the runoff reduction base method, following the standards outlined in the MHFD USDCM, Vol. 3. However, some disturbed areas are excluded from this analysis in accordance with the ECM, App. I.7.1.B. A summary table is provided below showing the total disturbed area, runoff reduction, and excluded areas.

Additionally, supporting calculations can be found in **Appendix D** and a water quality map in **Appendix E** for reference.

Note, according to ECM App I.7.1.B.5, the lots must have a lot impervious area of less than 10 percent. A total lot imperviousness greater than 10 percent is allowed when a study specific to the watershed and/or MS4 shows that expected soil and vegetation conditions are suitable for infiltration/filtration of the WQCV for a typical site, and the permittee accepts such study as applicable within its MS4 boundaries. The maximum total lot imperviousness covered under this exclusion shall be 20 percent.

Table 5 – Water Quality Summary Table

Basin ID	Total Area (Acre)	Total Disturbed Area (Acre)	Runoff Reduction Area (Acre)	Untreated Area (Acre)	Disturbed Area Excluded (Acre)	Applicable WQ Exclusions
K-12	2.2	2.2	0.00		1.8	ECM App. I.7.1.B.5
KO-D20X	14.3	14.3	0.63	0.01	12.3	ECM App. I.7.1.B.5
KO-D22X	1.5	0.8	0.06		0.4	ECM App. I.7.1.B.5
Total	18.0	17.3	0.69	0.01	14.5	

3. Stabilize Drainageways

Step 3 requires channel stabilization measures to protect open channels from erosion resulting from increases in frequency, duration rate, and volume of runoff associated with development.

The Site is not adjacent to Kettle Creek or any of the tributaries identified in the DBPS. Additionally, the site is being subdivided and is therefore required to participate in the drainage basin fee program. **Table 4** below reflects the El Paso County drainage basin fees for 2026. Drainage basin fees are paid at the time of recording of the plat.

Table 6 – Drainage Basin Fee

Drainage Basin	2026 Drainage Fee (per impervious acre)	2025 Bridge Fee (per impervious acre)	Impervious Area (acres)	Total Drainage Fees
Kettle Creek	\$14,733	\$0	3.335	\$49,134.55

A copy of the 2026 El Paso County Drainage Basin Fees schedule has been included in **Appendix A** for reference. A total imperviousness summary for the basins within the property boundaries is included in **Appendix B** for reference.

4. Source Controls

Step 4 requires targeted source control measures based on site specific needs. Source control can include material storage, secondary containment, good housekeeping practices, and other source control measures.

Source control measures will be implemented during construction and adhere to County and State requirements. A Grading & Erosion Control Plan (GEC) and associated Stormwater Management Plan (SWMP) will be provided for the Site to show that source control measures during construction are met.

There are no plans for outdoor stockpiling of materials, or chemical and/or pollutant storage, onsite after construction has been completed, therefore, no other source control measures are anticipated at this time.

PROPOSED INFRASTRUCTURE

Culverts:

The Site is graded to maintain as much of the existing drainage patterns as possible; therefore, pipe culverts are proposed to allow existing drainage ways cross the proposed roads where they intersect. The proposed culverts are designed to adhere to the ECM for culvert sizing; an existing, private, 18-inch CMP pipe culvert at **Design Point D17** conveys flows under Studebaker St. was analyzed to compare capacity in both the existing and proposed conditions. Calculations and results can be found in **Appendix C**.

Roadside Ditches:

Roadside ditches are typical for stormwater conveyance along rural roadways as shown in the EPC Standard Drawings for standard roadway cross sections. The ditches were analyzed utilizing the Federal Highway Administration (FHWA) Hydraulic Toolbox *Version 5.4* to ensure adequate capacity to convey the minor and major storm events. The vegetated ditches will also provide stormwater runoff reduction. The results of the hydraulic analysis can be found in **Appendix C**. Only one calculation is provided for the typical ditch section and corresponds to the highest flow rate for all the ditch sections.

Where the roadside ditches transition to sheet flow, riprap pads are proposed. Velocities within the roadside ditches are low, below 2 fps, and calculations for these pads are not provided as velocities are already non-erosive. The pads are proposed to prevent rilling at the transition point.

Ownership & Maintenance:

All proposed internal roadways & stormwater infrastructure are to be privately owned and maintained by the developer and its successors.

VARIANCES

There are no proposed stormwater variances required by this study.

V. CONCLUSION

This Final Drainage Report for Elk View Estates was prepared using the methods and procedures described, and in conformance with, the El Paso County Drainage Criteria Manual (DCM) & Engineering Criteria Manual (ECM). Additionally, it was prepared in accordance with the applicable chapters of the Mile High Flood District (MHFD) Urban Stormwater Drainage Criteria Manual (USDCM), Volumes 1-3; and in accordance with the applicable chapters and sections of the City of Colorado Springs Drainage Criteria Manual (COS DCM), Volumes 1-2.

This report demonstrates that the proposed Site is in general conformance with the DBPS and does not adversely affect downstream facilities and surrounding developments.

VI. REFERENCES

1. [Drainage Criteria Manual Volume 1 & 2](#), County of El Paso, Colorado, October 31, 2018 (with current revisions).
2. [Engineering Criteria Manual](#), County of El Paso, Colorado, January 9, 2025 (with current revisions).
3. [Drainage Criteria Manual Volume 1](#), City of Colorado Springs (January 2021)
4. [Urban Storm Drainage Criteria Manual, Vol. 1-3](#), Mile High Flood District, March 2024 (with current revisions).
5. [Kettle Creek Drainage Basin – Old Ranch Road Tributary Drainage Basin Planning Study & Master Development Drainage Plan](#), JR Engineering, April 2001 (Revised October 2002).
6. [Drainage Basin Planning Study for Kettle Creek Basin](#), JR Engineering, May 5, 2015.
7. Flood Insurance Rate Map – El Paso County, Colorado and Incorporated Areas Community Panel No. 08041C0526G, Effective December 7th, 2018.
8. Soil Map – El Paso County Area, Colorado as available through the Natural Resources Conservation Service National Cooperative Soil Survey web site via Web Soil Survey 2.0.

VII. APPENDIX

APPENDIX A – REFERENCE DOCUMENTATION

APPENDIX B – HYDROLOGIC COMPUTATIONS

APPENDIX C – HYDRAULIC COMPUTATIONS

APPENDIX D – WATER QUALITY COMPUTATIONS

APPENDIX E – DRAINAGE MAPS

APPENDIX A

REFERENCE DOCUMENTATION

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations (BFEs)** and/or **floodways** have been determined, users are encouraged to consult the **Flood Profiles and Floodway Data** and/or **Summary of Stillwater Elevations** tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988 (NAVD88)**. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov/> or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, NNGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at <http://www.ngs.noaa.gov/>.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, and Anderson Consulting Engineers, Inc. These data are current as of 2008.

This map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

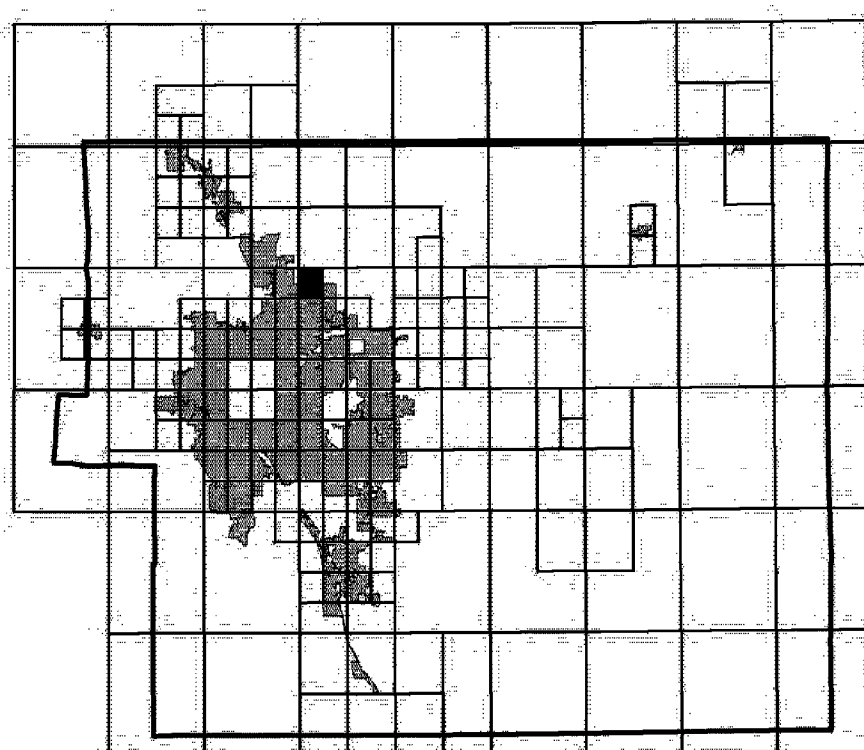
Contact **FEMA Map Service Center (MSC)** via the FEMA Map Information eXchange (FIMAX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at <http://www.msc.fema.gov/>.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2627) or visit the FEMA website at <http://www.fema.gov/business/nfip>.

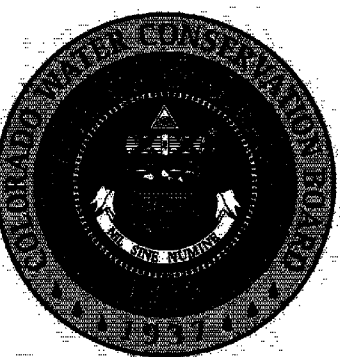
El Paso County Vertical Datum Offset Table

Flooding Source	Vertical Datum Offset (ft)
REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION	

Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).



Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 12 SOUTH, RANGE 65 WEST, AND TOWNSHIP 12 SOUTH, RANGE 66 WEST.

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A No Base Flood Elevations determined.
ZONE AE Base Flood Elevations determined.
ZONE AH Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99 Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

— Floodplain boundary
— Floodway boundary
— Zone D boundary
— CBRS and OPA boundary

— Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
— Base Flood Elevation line and value; elevation in feet*
— Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

— Cross section line

— Transect line

97° 07' 30.00" 32° 22' 30.00" 4750000N
1000-meter Universal Transverse Mercator grid ticks, zone 13

6000000 FT
5000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0502), Lambert Conformal Conic Projection

DX5510
Bench mark (see explanation in Notes to Users section of this FIRM panel)

M1.5
River Mile

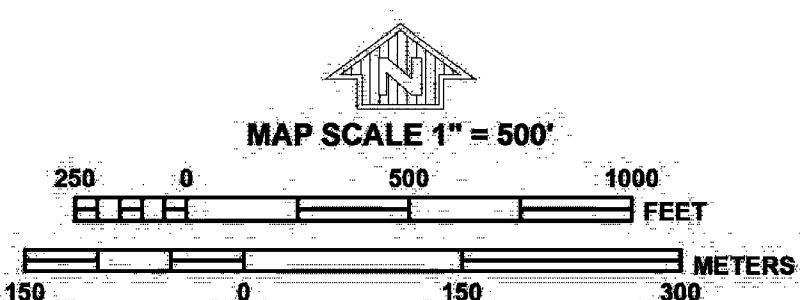
MAP REPOSITORIES
Refer to Map Repositories list on Map Index.

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
MARCH 17, 1997

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.



PANEL 0526G

FIRM

FLOOD INSURANCE RATE MAP

EL PASO COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 526 OF 1300
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
COLORADO SPRINGS, CITY OF	080060	0526	G
EL PASO COUNTY	080059	0526	G

Notice to User: The Map Number shown below should be used when placing map orders. The Community Number shown above should be used on insurance applications for the subject community.

MAP NUMBER
08041C0526G

MAP REVISED
DECEMBER 7, 2018

Federal Emergency Management Agency



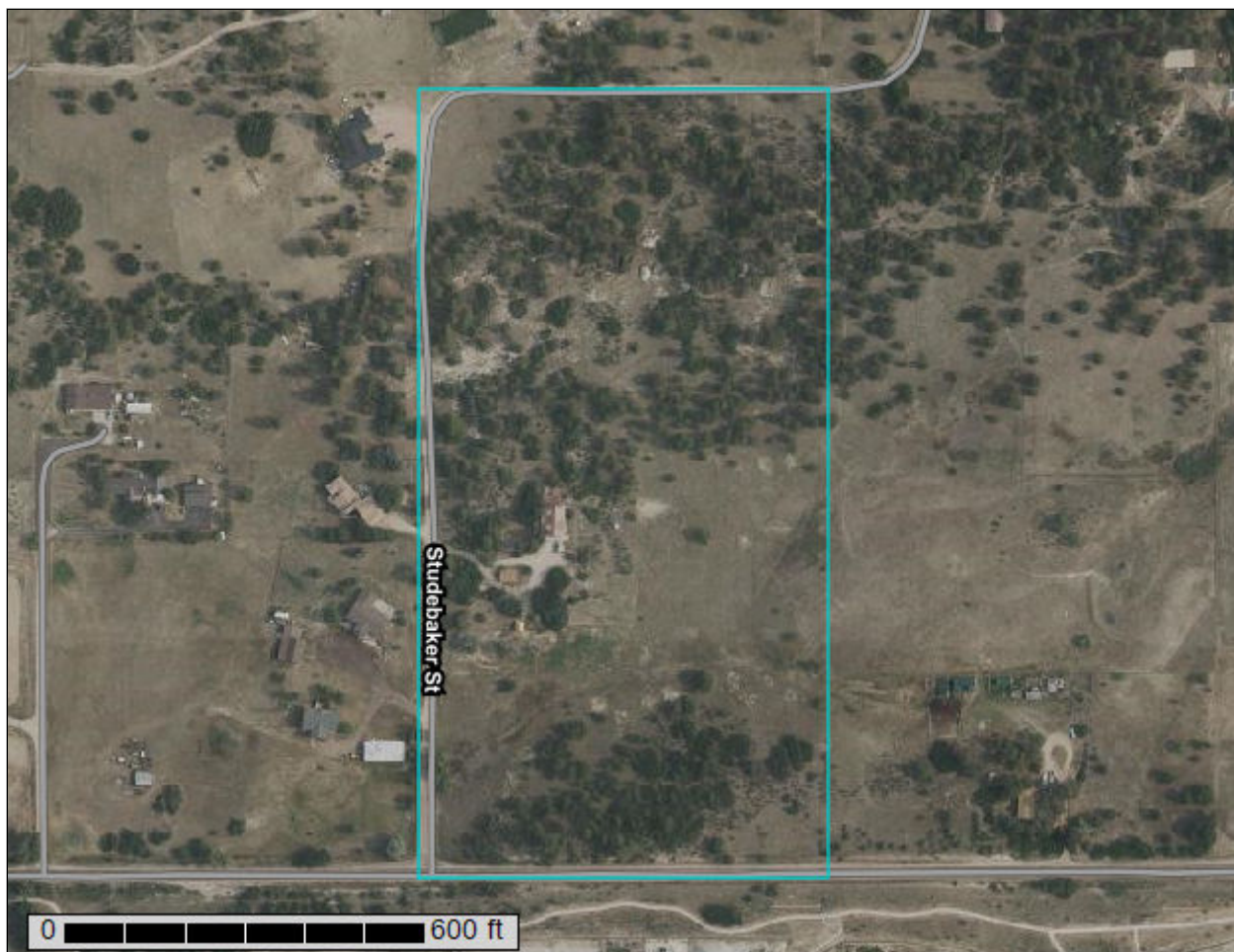
United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for **El Paso County Area, Colorado**



June 6, 2025

Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

Custom Soil Resource Report

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map



Custom Soil Resource Report

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

Special Point Features

 Blowout

 Borrow Pit

 Clay Spot

 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water

 Perennial Water

 Rock Outcrop

 Saline Spot

 Sandy Spot

 Severely Eroded Spot

 Sinkhole

 Slide or Slip

 Sodic Spot

 Spoil Area

 Stony Spot

 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

Water Features

 Streams and Canals

Transportation

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 22, Sep 3, 2024

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 23, 2024—Aug 4, 2024

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
41	Kettle gravelly loamy sand, 8 to 40 percent slopes	9.2	44.6%
68	Peyton-Pring complex, 3 to 8 percent slopes	2.4	11.6%
69	Peyton-Pring complex, 8 to 15 percent slopes	9.1	43.8%
Totals for Area of Interest		20.7	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or

landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

El Paso County Area, Colorado

41—Kettle gravelly loamy sand, 8 to 40 percent slopes

Map Unit Setting

National map unit symbol: 368h

Elevation: 7,000 to 7,700 feet

Farmland classification: Not prime farmland

Map Unit Composition

Kettle and similar soils: 85 percent

Minor components: 5 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Kettle

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Sandy alluvium derived from arkose

Typical profile

E - 0 to 16 inches: gravelly loamy sand

Bt - 16 to 40 inches: gravelly sandy loam

C - 40 to 60 inches: extremely gravelly loamy sand

Properties and qualities

Slope: 8 to 40 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Somewhat excessively drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 3.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7e

Hydrologic Soil Group: B

Ecological site: F048AY908CO - Mixed Conifer

Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit: 5 percent

Landform: Depressions

Hydric soil rating: Yes

Other soils

Percent of map unit:

Hydric soil rating: No

68—Peyton-Pring complex, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 369f

Elevation: 6,800 to 7,600 feet

Farmland classification: Not prime farmland

Map Unit Composition

Peyton and similar soils: 40 percent

Pring and similar soils: 30 percent

Minor components: 5 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Peyton

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Arkosic alluvium derived from sedimentary rock and/or arkosic residuum weathered from sedimentary rock

Typical profile

A - 0 to 12 inches: sandy loam

Bt - 12 to 25 inches: sandy clay loam

BC - 25 to 35 inches: sandy loam

C - 35 to 60 inches: sandy loam

Properties and qualities

Slope: 3 to 5 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 7.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4c

Hydrologic Soil Group: B

Ecological site: R049XY216CO - Sandy Divide

Hydric soil rating: No

Description of Pring

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Arkosic alluvium derived from sedimentary rock

Typical profile

A - 0 to 14 inches: coarse sandy loam

C - 14 to 60 inches: gravelly sandy loam

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 6.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: B

Ecological site: R048AY222CO - Loamy Park

Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit: 5 percent

Landform: Depressions

Hydric soil rating: Yes

Other soils

Percent of map unit:

Hydric soil rating: No

69—Peyton-Pring complex, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 369g

Elevation: 6,800 to 7,600 feet

Farmland classification: Not prime farmland

Map Unit Composition

Peyton and similar soils: 40 percent

Pring and similar soils: 30 percent

Minor components: 5 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Peyton

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Arkosic alluvium derived from sedimentary rock and/or arkosic residuum weathered from sedimentary rock

Typical profile

A - 0 to 12 inches: sandy loam

Bt - 12 to 25 inches: sandy clay loam

BC - 25 to 35 inches: sandy clay loam

C - 35 to 60 inches: sandy loam

Properties and qualities

Slope: 8 to 9 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 7.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4e

Hydrologic Soil Group: B

Ecological site: R049XY216CO - Sandy Divide

Hydric soil rating: No

Description of Pring

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Arkosic alluvium derived from sedimentary rock

Typical profile

A - 0 to 14 inches: coarse sandy loam

C - 14 to 60 inches: gravelly sandy loam

Properties and qualities

Slope: 8 to 15 percent

Depth to restrictive feature: More than 80 inches

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Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 6.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6e

Hydrologic Soil Group: B

Ecological site: R048AY222CO - Loamy Park

Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit: 5 percent

Landform: Depressions

Hydric soil rating: Yes

Other soils

Percent of map unit:

Hydric soil rating: No

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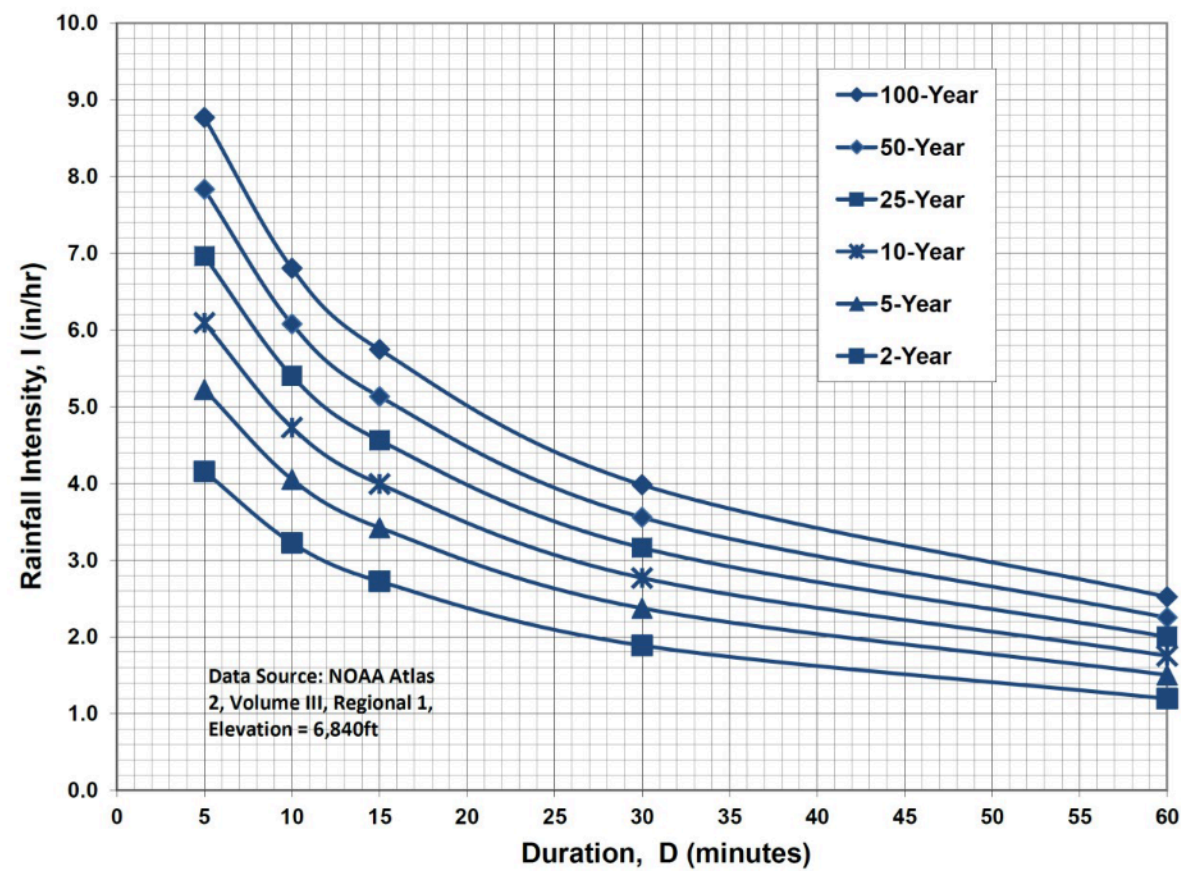
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Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency



IDF Equations

$$I_{100} = -2.52 \ln(D) + 12.735$$

$$I_{50} = -2.25 \ln(D) + 11.375$$

$$I_{25} = -2.00 \ln(D) + 10.111$$

$$I_{10} = -1.75 \ln(D) + 8.847$$

$$I_5 = -1.50 \ln(D) + 7.583$$

$$I_2 = -1.19 \ln(D) + 6.035$$

Note: Values calculated by equations may not precisely duplicate values read from figure.

Table 6-6. Runoff Coefficients for Rational Method
(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients											
		2-year		5-year		10-year		25-year		50-year		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis-- Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

Type of Structure and Design of Entrance	Coefficient k_e
Pipe, Concrete	
Projecting from fill, socket end (groove end)	0.2
Projecting from fill, square-cut end	0.5
Headwall or headwall and wingwalls	
Socket end of pipe (groove end)	0.2
Square-edged	0.5
Rounded (radius = $1/12D$)	0.2
Mitered to conform to fill slope	0.7
*End section conforming to fill slope	0.5
Beveled edges, 33.7-degree to 45-degree bevels	0.2
Side-tapered or slope-tapered inlet	0.2
Pipe, or Pipe-Arch, Corrugated Metal	
Projecting from fill (no headwall)	0.9
Headwall or headwall and wingwalls square-edge	0.5
Mitered to conform to fill slope, paved or unpaved slope	0.7
*End section conforming to fill slope	0.5
Beveled edges, 33.7-degree to 45-degree bevels	0.2
Side-tapered or slope-tapered inlet	0.2

Type of Structure and Design of Entrance	Coefficient k_e
Box, Reinforced Concrete	
Headwall parallel to embankment (no wingwalls) Square-edged on 3 edges	0.5
Rounded on 3 edges to radius of 1/12 barrel dimension, or beveled edges on 3 sides	0.2
Wingwalls at 30 degrees to 75 degrees to barrel Square-edged at crown	0.4
Crown edge rounded to radius of 1/12 barrel dimension, or beveled top edge	0.2
Wingwall at 10 degrees to 25 degrees to barrel Square-edged at crown	0.5
Wingwall parallel (extension of sides) Square-edged at crown	0.7
Side-tapered or slope-tapered inlet	0.2
*Note: End sections conforming to fill slope are the sections commonly available from manufacturers. From limited hydraulic tests, they are equivalent in operation to a headwall in both inlet and outlet control. Some end sections incorporating a closed taper in their design have a superior hydraulic performance.	
Federal Highway Administration, Hec No. 13	

El Paso County Drainage Basin Fees

Resolution No. 25-302_____

Basin Number	Receiving Waters	Year Studied	Drainage Basin Name	2026 Drainage Fee (per Impervious Acre)	2026 Bridge Fee (per Impervious Acre)
<u>Drainage Basins with DBPS's:</u>					
CHMS0200	Chico Creek	2013	Haegler Ranch	\$15,350	\$2,265
CHWS1200	Chico Creek	2001	Bennett Ranch	\$17,184	\$6,592
CHWS1400	Chico Creek	2013	Falcon	\$44,042	\$6,050
FOFO2000	Fountain Creek	2025	Jimmy Camp Creek ³	\$58,638	\$0
FOFO2600	Fountain Creek	1991*	Big Johnson / Crews Gulch	\$27,282	\$3,523
FOFO2800	Fountain Creek	1988*	Widefield	\$27,282	\$0
FOFO2900	Fountain Creek	1988*	Security	\$27,282	\$0
FOFO3000	Fountain Creek	1991*	Windmill Gulch	\$27,282	\$409
FOFO3100 / FOFO3200	Fountain Creek	1988*	Carson Street / Little Johnson	\$16,641	\$0
FOFO3400	Fountain Creek	1984*	Peterson Field	\$19,678	\$1,492
FOFO3600	Fountain Creek	1991*	Fisher's Canyon	\$27,282	\$0
FOFO4000	Fountain Creek	1996	Sand Creek	\$28,160	\$11,518
FOFO4200	Fountain Creek	1977	Spring Creek	\$14,149	\$0
FOFO4600	Fountain Creek	1984*	Southwest Area	\$27,282	\$0
FOFO4800	Fountain Creek	1991	Bear Creek	\$27,282	\$1,492
FOFO5800	Fountain Creek	1964	Camp Creek	\$3,023	\$0
FOMO1000	Monument Creek	1981	Douglas Creek	\$17,157	\$379
FOMO1200	Monument Creek	1977	Templeton Gap	\$17,614	\$409
FOMO2000	Monument Creek	1971	Pulpit Rock	\$9,047	\$0
FOMO2200	Monument Creek	1994	Cottonwood Creek / S. Pine	\$27,282	\$1,492
FOMO2400	Monument Creek	1966	Dry Creek	\$21,537	\$780
FOMO3600	Monument Creek	1989*	Black Squirrel Creek	\$12,387	\$780
FOMO3700	Monument Creek	1987*	Middle Tributary	\$22,766	\$0
FOMO3800	Monument Creek	1987*	Monument Branch	\$27,282	\$0
FOMO4000	Monument Creek	1996	Smith Creek	\$11,122	\$1,492
FOMO4200	Monument Creek	1989*	Black Forest	\$27,282	\$743
FOMO5200	Monument Creek	1993*	Dirty Woman Creek	\$27,282	\$1,492
FOMO5300	Fountain Creek	1993*	Crystal Creek	\$27,282	\$1,492
<u>Miscellaneous Drainage Basins: ¹</u>					
CHBS0800	Chico Creek		Book Ranch	\$25,599	\$3,706
CHEC0400	Chico Creek		Upper East Chico	\$13,946	\$404
CHWS0200	Chico Creek		Telephone Exchange	\$15,323	\$359
CHWS0400	Chico Creek		Livestock Company	\$25,240	\$300
CHWS0600	Chico Creek		West Squirrel	\$13,156	\$5,460
CHWS0800	Chico Creek		Solberg Ranch	\$27,282	\$0
FOFO1200	Fountain Creek		Crooked Canyon	\$8,237	\$0
FOFO1400	Fountain Creek		Calhan Reservoir	\$6,877	\$401
FOFO1600	Fountain Creek		Sand Canyon	\$4,968	\$0
FOFO2200	Fountain Creek		Fort Carson	\$21,537	\$780
FOFO2700	Fountain Creek		West Little Johnson	\$1,798	\$0
FOFO3800	Fountain Creek		Stratton	\$13,086	\$585
FOFO5000	Fountain Creek		Midland	\$21,537	\$780
FOFO6000	Fountain Creek		Palmer Trail	\$21,537	\$780
FOFO6800	Fountain Creek		Black Canyon	\$21,537	\$780
FOMO4600	Monument Creek		Beaver Creek	\$16,311	\$0
FOMO3000	Monument Creek		Kettle Creek	\$14,733	\$0
FOMO3400	Monument Creek		Elkhorn	\$2,475	\$0
FOMO5000	Monument Creek		Monument Rock	\$11,825	\$0
FOMO5400	Monument Creek		Palmer Lake	\$18,908	\$0
FOMO5600	Monument Creek		Raspberry Mountain	\$6,360	\$0
PLPL0200	Monument Creek		Bald Mountain	\$13,554	\$0
<u>Interim Drainage Basins: ²</u>					
FOFO1800	Fountain Creek		Little Fountain Creek	\$3,488	\$0
FOMO4400	Monument Creek		Jackson Creek	\$10,799	\$0
FOMO4800	Monument Creek		Teachout Creek	\$7,499	\$1,127

1. The miscellaneous drainage fee previous to September 1999 resolution was the average of all drainage fees for basins with Basin Planning Studies performed within the previous 14 years.

2. Interim Drainage Fees are based upon draft Drainage Basin Planning Studies or the Drainage Basin Identification and Fee Estimation Report. (Best available information suitable for setting a fee.)

3. Per Resolutions 25-95 and 25-96, the Bull-Hill/Rolling Meadows development and the West Fork Jimmy Camp Creek are closed for fees.

**KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
DRAINAGE BASIN PLANNING STUDY
AND
MASTER DEVELOPMENT DRAINAGE PLAN**

April 2001
(Minor Text Revisions, October 2002)

Prepared For:

**LP47, LLC dba
LA PLATA INVESTMENTS**
2315 Briargate Parkway, Suite 100
Colorado Springs, CO 80920
(719) 260-7477

Prepared By:

JR ENGINEERING
4310 ArrowsWest Drive
Colorado Springs, CO 80907
(719) 593-2593

Job No. 8877.10

**KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
DRAINAGE BASIN PLANNING STUDY
AND
MASTER DEVELOPMENT DRAINAGE PLAN**



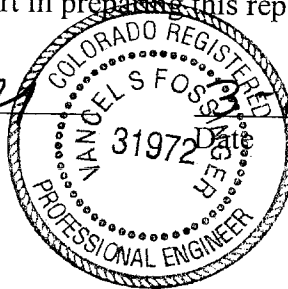
DRAINAGE REPORT STATEMENT

ENGINEER'S STATEMENT:

The attached drainage report was prepared under my direction and supervision and is correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the City for drainage reports. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

Vancel Fossinger

Vancel S. Fossinger, Colorado P.E. #31972
For and On Behalf of JR Engineering



3-13-03

CITY OF COLORADO SPRINGS ONLY:

Filed in accordance with Section 15-3-906 of the Code of the City of Colorado Springs, 1980, as amended.

Tom Minto Jr.

City Engineer

March 18, 2003

Date

Conditions:

The proposed detention facilities are distributed throughout the watershed in order to mitigate high peak flow rates throughout the conveyance system and thus, limit the size of the required storm drains. The South Tributary will be utilized as an outfall for major flows from the area above proposed Regional Detention Facility “E”, but the frequent flows from this area will be routed to Kettle Creek through a storm drain in Old Ranch Road in order to minimize impact to the natural channel of the South Tributary.

Due to concerns about preservation of downstream natural channels in their existing condition, this plan has been structured to regulate peaks release rates from the study area in the 2, 5, 10, 25, 50 and 100-year design storms. Generally accepted practice in this area has been to only regulate flows from large events. Recent regulatory actions such as the listing of the Prebles Meadow Jumping Mouse (PMJM) as a “threatened species” under the Endangered Species Act of 1973, has made it very difficult to modify or make improvements to natural channels that may contain habitat used by the Prebles Meadow Jumping Mouse. Thus, it appears to be prudent to take additional precautions in order to minimize impact to potentially sensitive areas located downstream of proposed development.

B. Fully Developed Condition Plan

1. Sub-Basins D1 through D9A

The study area begins east of future Powers Boulevard. Current land planning is very general for this area. Sub-Basins D1, D2 and D4 are assumed to remain at their current level of development. It is assumed that runoff patterns from this area will remain unchanged with the exception of the upsizing of the culverts under Old Ranch Road to pass the 100-year design storm. Sub-Basins D3 and D5 are planned for very low density residential development. The future street to be constructed along the southern boundary of these basins should be graded to provide the embankments required to detain water in the small proposed ponds labeled as Detention Facilities “A” and “B”. Runoff from Sub-Basins D1 through D3 will be routed to and detained in Detention Facility “A” and runoff from Sub-Basins D4 and D5 will be routed to and detained in Detention Facility “B”.

Proposed Regional Detention Facility "A" is planned to have a 100-year peak inflow of 165 cfs, a 100-year peak outflow of 65 cfs, and a 100-year storage volume requirement of 4-acre feet.

Proposed Regional Detention Facility "B" is planned to have a 100-year peak inflow of 103 cfs, a 100-year peak outflow of 57 cfs, and a 100-year storage volume requirement of 1-acre feet.

The outflow from Detention Facilities "A" and "B" will be conveyed in proposed storm drains along with runoff collected from Sub-Basins D6 and D7 to proposed Regional Detention Facility "C". Runoff from adjacent Sub-Basin D8 will also be routed to proposed Regional Detention Facility "C". Proposed Regional Detention Facility "C" is planned to have a 100-year peak inflow of 524 cfs, a 100-year peak outflow of 86 cfs, and a 100-year storage volume requirement of 21-acre feet. Outflow from proposed Regional Detention Facility "C" will be conveyed under Powers Boulevard and then to Analysis Point D4 in a proposed storm drain along with runoff from Sub-Basin D9A.

Detention Facility C will be configured such that the overflow that would occur from the 100-year flood being routed through the proposed pond (assuming the pond empty and the normal outlet is clogged at the beginning of the storm) will be routed under Powers Boulevard to Royal Pine Drive a 72" diameter culvert to be constructed with Powers Boulevard. In an overflow event, flow in excess of the capacity of the proposed storm sewer to be constructed in Royal Pine Drive will be conveyed in the street section of Royal Pine Drive to its low point, then across Basin D37 to Old Ranch Road near Chapel Hills Drive then west in Old Ranch Road to Kettle Creek. The excess stormwater will be diverted from the Royal Pine Drive storm drain via a proposed Junction 1 overflow box to be located on the storm drain between proposed Powers Boulevard and Royal Pine Drive. A concept detail of the proposed structure is contained in the appendix of this report. The proposed lots adjacent to proposed Royal Pine Drive should be graded to be a minimum of 1.5 feet above the flow line of adjacent Royal Pine Drive or have a solid wall separating them from Royal Pine

modified dams. The primary development impact mitigation measures proposed in this plan is the diversion of runoff from frequent storms to a storm drain system rather than allowing it to be conveyed down this relatively steep natural channel while allowing the perennial flow that supports the vegetation growing in the channel to continue flowing. The proposed outflow structures for the proposed Regional Detention Pond “E” are planned to limit both peak flow rates and volumes that will be released to the natural channel and thus, should minimize potential impacts that development of the study area may have on the channel.

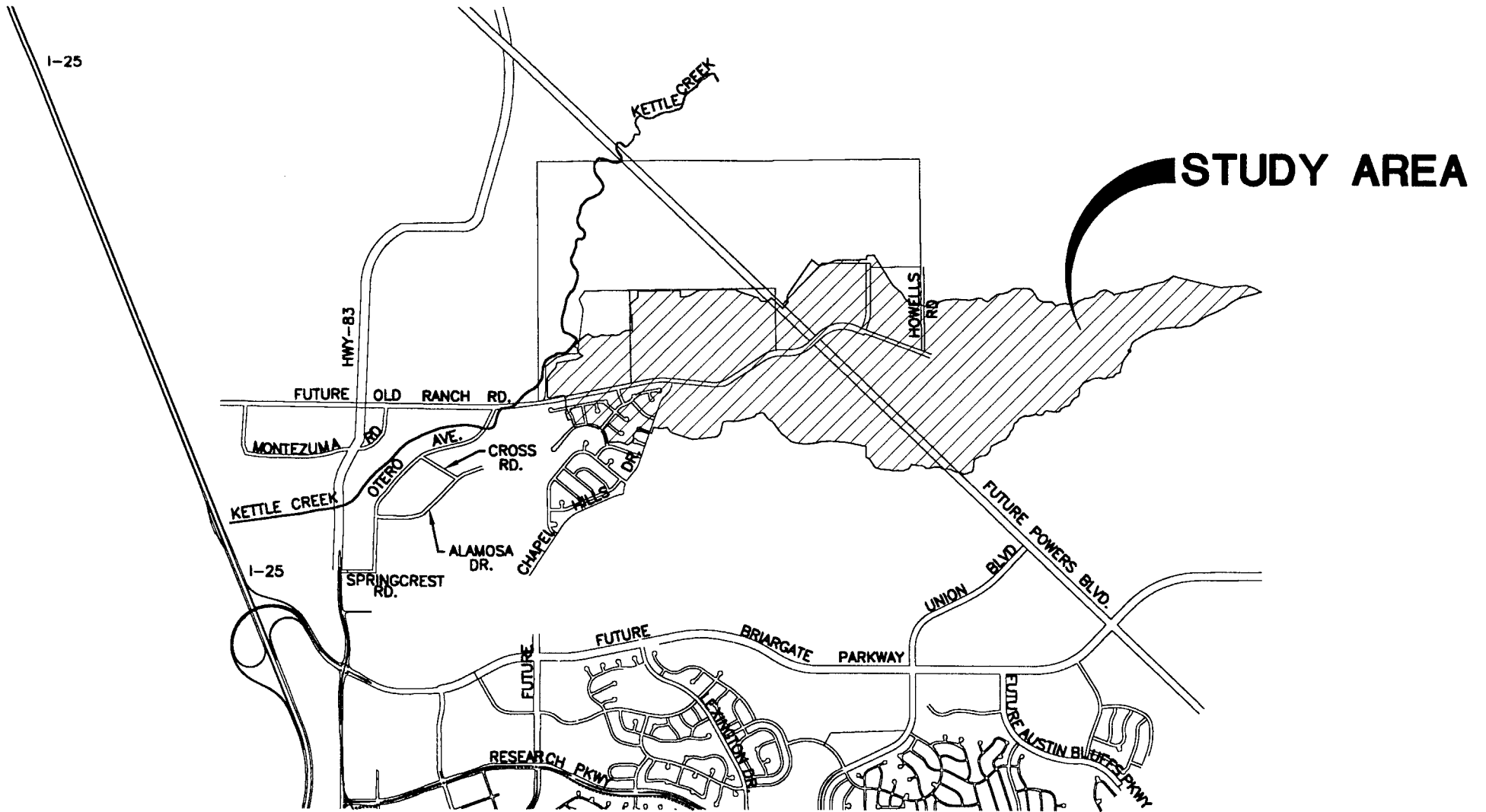
4. Proposed Constraints and Recommendations

a. The following discharge constraints are proposed for the study area:

- Free discharge of developed condition drainage from the study area will be allowed provided that the following criteria is followed.
- Adequate down stream conveyance facilities must exist or be provided in accordance with City of Colorado Springs policy and criteria.
- Runoff must be routed through the regional detention facilities as proposed in this study unless a detailed drainage study demonstrates the adequacy of alternative routing to achieve the discharge goals of this study.
- Land uses must be similar or less intensive than the land uses assumed for the purpose of this study unless a detailed drainage analysis indicates that free discharge from the more intensive land use will not have an adverse affect on the downstream drainage facilities.

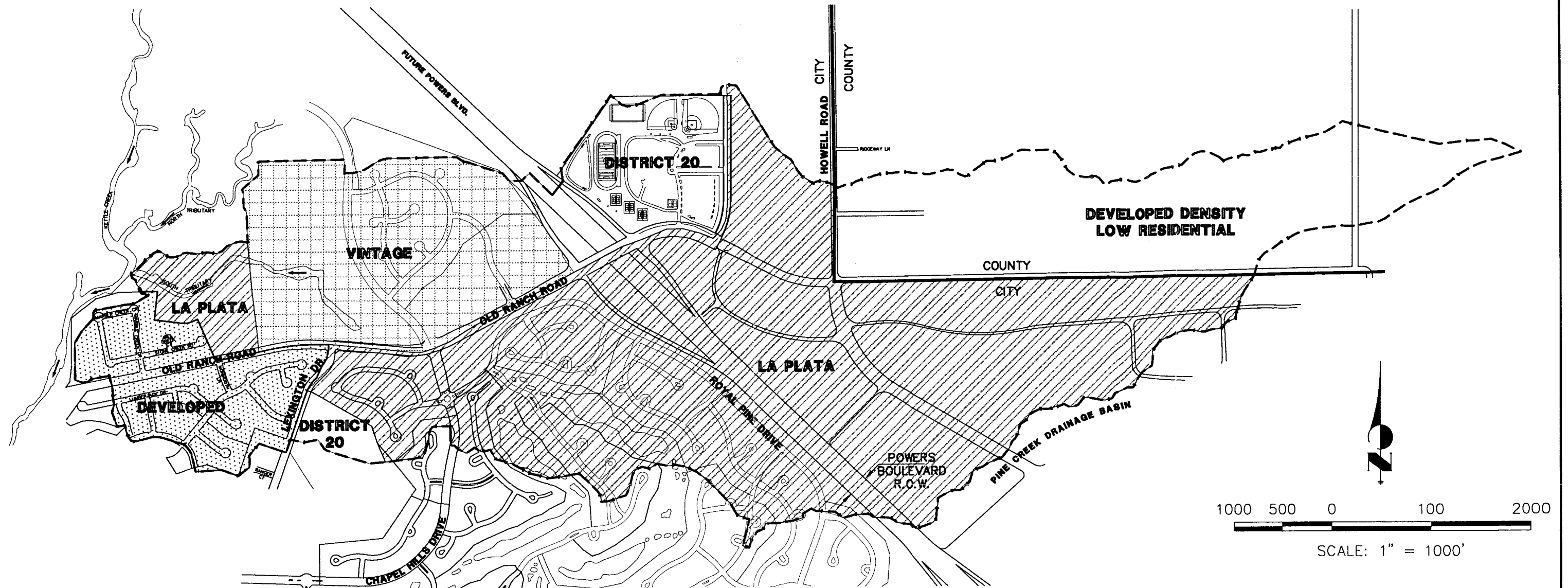
b. The following recommendations are made with regard to facilities and sites that emergency overflow from detention ponds will be diverted to:

- Lots adjacent to proposed Royal Pine Drive located downstream or adjacent to the outfall from proposed Detention Facility “C” should be graded to be a minimum of 1.5 feet above the adjacent flow line of the street or be hydraulically isolated from the street by a wall or berm or combination of the two.
- An overflow route should be planned and an easement recorded for the same across Basin D37 between the low point of proposed Royal Pine Drive and



VICINITY MAP
1" = 3000'





LEGEND

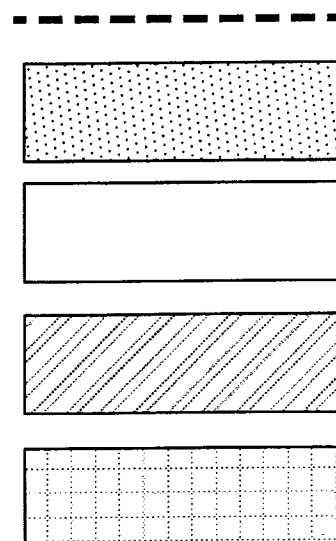
BASIN BOUNDARY

DEVELOPED AREA

DISTRICT 20 SCHOOL DISTRICT

LP47, LLC dba La Plata Investments

VINTAGE COMMUNITIES



KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
DRAINAGE BASIN PLANNING STUDY AND
MASTER DEVELOPMENT DRAINAGE PLAN
GENERAL LAND OWNERSHIP IN STUDY AREA

JOB NO. 8877.10
04/18/01
SHEET 1 OF 1

 **J-R ENGINEERING**
A Subsidiary of Westrian

4310 ArrowsWest Drive • Colorado Springs, CO 80907
719-593-2583 • Fax: 719-528-6613 • www.jrengineering.com

KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
MASTER DEVELOPMENT DRAINAGE PLAN

DEVELOPED CONDITION
ESTIMATED CURVE NUMBERS
4/3/2001

SUB-BASIN LABEL	SUB AREA ONE				SUB AREA TWO				SUB AREA THREE				SUB AREA FOUR				TOTAL AREA AC.	TOTAL AREA S.M.	WEIGHTED CN	WEIGHTED IMPERV.
	ASSUMED LAND USE	ESTIMATED PERCENT IMPERVIOUS	ESTIMATED CN	AREA AC.	ASSUMED LAND USE	ESTIMATED PERCENT IMPERVIOUS	ESTIMATED CN	AREA AC.	ASSUMED LAND USE	ESTIMATED PERCENT IMPERVIOUS	ESTIMATED CN	AREA AC.	ASSUMED LAND USE	ESTIMATED PERCENT IMPERVIOUS	ESTIMATED CN	AREA AC.				
D1	<1 DU/AC	5.0	69.0	54.2													54.2	0.085	69.0	5.0
D2	<1 DU/AC	5.0	69.0	34.3	ART. STREET	85.0	93.0	2.3									36.6	0.057	70.5	10.0
D3	2DU/AC	25.0	70.0	14.3	ART. STREET	85.0	93.0	2.3									16.6	0.026	73.2	33.3
D4	<1 DU/AC	5.0	69.0	28.9	ART. STREET	85.0	93.0	1.6									30.5	0.048	70.3	9.2
D5	2DU/AC	25.0	70.0	17.4	ART. STREET	85.0	93.0	1.7									19.1	0.030	72.0	30.3
D6	4DU/AC	40.0	76.0	18.4	SCHOOL	50.0	84.0	6.7									25.1	0.039	78.1	42.7
D7	4DU/AC	40.0	76.0	31.3	ART. STREET	85.0	93.0	8.7	2DU/AC	25.0	70.0	5.5					45.5	0.071	78.5	46.8
D8	MULT FAM	70.0	87.0	6.0	ART. STREET	85.0	93.0	3.4	L/O	83.0	92.0	25.6	OPEN SPACE	5.0	69.0	4.5	39.5	0.062	88.7	72.3
D9*					ART. ST. PAV.	100.0	99.0	4.9	OPEN SPACE	9.0	69.0	9.0					13.9	0.022	79.6	41.1
D9A	3 DU/AC	30.0	70.0	9.2	STREET PAV.	100.0	99.0	0.8	OPEN SPACE	5.0	69.0	1.4					11.4	0.018	71.9	31.8
D10	<1 DU/AC	5.0	69.0	16.5	ART. STREET	85.0	93.0	0.7									17.2	0.027	70.0	8.3
D11	2DU/AC	25.0	70.0	30.2	ART. STREET	85.0	93.0	2.5									32.7	0.051	71.8	29.6
D12	<1 DU/AC	25.0	69.0	11.2	ART. STREET	85.0	93.0	2.1									13.3	0.021	72.8	34.5
D13*	COMMERCIAL	95.0	95.0	10.0	ART. ST. PAV.	100.0	99	4	MULT FAM	70.0	87.0	24.9	OPEN SPACE	5.0	69.0	4.0	42.9	0.067	88.3	72.6
D14*	INTERCHANGE	100.0	99.0	5.0	OPEN SPACE	5.0	69.0	10.0									15.0	0.023	79.0	36.7
D15*	3 DU/AC	30.0	72.0	26.5	OPEN SPACE	5.0	69.0	0.4	ART. ST. PAV.	85.0	93.0	0.7					27.6	0.043	72.5	31.0
D16*	3 DU/AC	30.0	72.0	26.2	GOLF CRS	0.0	61.0	13.1	ART. ST. PAV.	100.0	99.0	0.5					39.8	0.062	68.7	21.0
D16A	3 DU/AC	30.0	70.0	7.5	ART. ST. PAV.	100.0	99.0	0.7									8.2	0.013	72.5	36.0
D17*					ART. ST. PAV.	100.0	99.0	1.9	OPEN SPACE	5.0	69.0	4.6					6.5	0.010	77.8	32.8
D17A*					ART. ST. PAV.	100.0	99.0	3.5	COMMERCIAL	95.0	95.0	2.0	OPEN SPACE	5.0	69.0	1.3	6.8	0.011	92.3	80.9
D18	SCHOOL	38.0	80.0	41.0													41.0	0.064	80.0	38.0
D19	INTERCHANGE	40.0	80.0	15.1													15.1	0.024	80.0	40.0
D20	COMMERCIAL	95.0	95.0	9.2	OPEN SPACE	5.0	69.0	0.7	MULTI FAM	70.0	87.0	8.2	ART. STREET	85.0	93.0	0.8	18.9	0.030	90.5	80.4
D21*	COMMERCIAL	95.0	95.0	12.5	OPEN SPACE	5.0	69.0	4.5	4DU/AC	40.0	76.0	8.0	ART. STREET	85.0	93.0	1.4	26.4	0.041	84.7	62.5
D22*	3 DU/AC	30.0	72.0	19.0					ART STREET	85.0	93.0	4.7					23.7	0.037	76.2	40.9
D23	3 DU/AC	30.0	72.0	1.8	ART. STREET	85.0	93.0	1.7									3.5	0.005	82.2	56.7
D24	3 DU/AC	30.0	72.0	13.3	OPEN SPACE	5.0	69.0	2.8									16.1	0.025	71.5	25.7
D25*	4 DU/AC	40.0	76.0	0.7	SFA 5DU/AC	70.0	87.0	1.4	SCHOOL	50.0	80.0	8.3	ART STREET	85.0	93.0	0.4	10.8	0.017	81.1	53.2
D26	3 DU/AC	30.0	72.0	20.9													20.9	0.033	72.0	30.0
D27	3 DU/AC	30.0	72.0	3.5	ART. STREET	85.0	93.0	1.5									5.0	0.008	78.3	46.5
D28	3 DU/AC	30.0	72.0	1.8	ART. STREET	85.0	93.0	1.0									2.8	0.004	79.5	49.6
D29	3 DU/AC	30.0	72.0	2.5	4 DU/AC	40.0	76.0	9.1									11.6	0.018	75.1	37.8
D30	3 DU/AC	30.0	72.0	9.0	OPEN SPACE	5.0	69.0	2.0									11.0	0.017	71.5	25.5
D31	4 DU/AC	40.0	76.0	4.5													4.5	0.007	76.0	40.0
D32	OPEN SPACE	5.0	69.0	3.7													3.7	0.006	69.0	5.0
D33*	3 DU/AC	30.0	72.0	5.0	OPEN SPACE	5.0	76.0	18.3									23.3	0.036	75.1	10.4
D34	3 DU/AC	30.0	72.0	21.8	ART. STREET	85.0	93.0	2.9									24.7	0.039	74.5	36.5
D35	4 DU/AC	40.0	76.0	24.7													24.7	0.039	76.0	40.0
D36*	3 DU/AC	30.0	72.0	6.5	ART STREET	85.0	93.0	1.2	GOLF CRS	0.0	61.0	7.5					15.2	0.024	68.2	19.5
D37*	COMMERCIAL	95.0	95.0	5.5	ART STREET	85.0	93.0	1.3									6.8	0.011	94.6	93.1
																	812.0	1.3		

KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
MASTER DEVELOPMENT DRAINAGE PLAN
FULLY DEVELOPED CONDITION OUTPUT SUMMARY AND CURVE NUMBER ADJUSTMENT
3/27/2001

TYPE IIa 24HR STRM @3 MIN. TIME STEP

TYPE IIa 24HR STRM @3 MIN. TIME STEP									HEC1 MODEL					RATIONAL METHOD	COMPUTED HEC1 VS.	ADJUSTED HEC1 VS.	COMPUTED
									W/ COMPUTED CN		W/ ADJUSTED CN						
SUB BASIN I.D.	AREA (sq miles)	AREA (acres)	IMPERVIOUS PERCENT	COMPUTED CN	ADJUSTED CN	COMPUTED C ₁₀₀	TC (min)	LAG (hours)	Q ₁₀₀ (cfs)	Q ₁₀₀ /ACRE (cfs)	Q ₁₀₀ (cfs)	Q ₁₀₀ /ACRE (cfs)	I ₁₀₀ (in/hr)	Q ₁₀₀ (cfs/AC)	RATIONAL PERCENT	RATIONAL PERCENT	RATIONAL Q100
D1	0.085	54.2	5.0	69.0	67.5	0.38	38.34	0.383	78	1.44	72	1.33	3.49	1.33	8	0	72
D2	0.057	36.6	10.0	70.5	69.0	0.41	22.41	0.224	78	2.13	72	1.97	4.79	1.96	8	0	72
D3	0.026	16.6	33.3	73.2	75.0	0.55	26.70	0.267	37	2.23	40	2.41	4.34	2.38	-7	1	40
D4	0.048	30.5	9.2	70.3	68.2	0.41	24.46	0.245	62	2.03	56	1.84	4.56	1.85	9	-1	56
D5	0.030	19.1	30.3	72.0	74.5	0.53	23.90	0.239	42	2.20	47	2.46	4.62	2.46	-12	0	47
D6	0.039	25.1	42.7	78.1	79.2	0.61	16.43	0.164	82	3.27	86	3.43	5.63	3.41	-5	0	86
D7	0.071	45.5	46.8	78.5	79.8	0.63	17.31	0.173	149	3.27	157	3.45	5.49	3.46	-6	0	157
D8	0.062	39.5	72.3	88.7	92.2	0.78	11.12	0.111	193	4.89	208	5.27	6.76	5.30	-8	-1	209
D9*	0.022	13.9	41.1	79.6	77.0	0.60	25.09	0.251	42	3.02	37	2.66	4.49	2.68	11	-1	37
D9A*	0.018	11.4	31.8	71.9	74.5	0.54	21.27	0.213	27	2.37	30	2.63	4.92	2.66	-12	-1	30
D10	0.027	17.2	8.3	70.0	68.0	0.40	30.69	0.307	30	1.74	27	1.57	4.00	1.60	8	-2	27
D11	0.051	32.7	29.6	71.8	74.2	0.53	23.11	0.231	73	2.23	81	2.48	4.70	2.48	-11	0	81
D12	0.021	13.3	34.5	72.8	74.6	0.56	28.05	0.281	28	2.11	31	2.33	4.21	2.35	-11	-1	31
D13*	0.067	42.9	72.6	88.3	91.5	0.79	12.33	0.123	204	4.76	219	5.10	6.46	5.07	-7	1	218
D14*	0.023	15.0	36.7	79.0	78.0	0.57	15.83	0.158	51	3.40	49	3.27	5.74	3.27	4	0	49
D15*	0.043	27.6	31.0	72.5	75.0	0.54	19.99	0.200	68	2.46	75	2.72	5.09	2.73	-11	0	75
D16*	0.062	39.8	21.0	68.7	72.5	0.48	19.69	0.197	83	2.09	98	2.46	5.13	2.44	-17	1	97
D16A*	0.013	8.2	36.0	72.5	78.0	0.57	14.31	0.143	23	2.80	28	3.41	6.03	3.41	-22	0	28
D17*	0.010	6.5	32.8	77.8	76.5	0.55	11.65	0.117	22	3.38	25	3.85	6.63	3.62	-7	6	24
D17A	0.010	6.7	80.9	92.3	99.0	0.84	11.95	0.120	33	4.93	36	5.37	6.55	5.47	-11	-2	37
D18	0.064	41.0	38.0	80.0	76.3	0.58	25.24	0.252	123	3.00	106	2.59	4.48	2.59	14	0	106
D19	0.024	15.1	40.0	80.0	80.9	0.59	12.16	0.122	50	3.31	59	3.91	6.50	3.83	-16	2	58
D20	0.030	18.9	80.4	90.5	96.5	0.83	10.83	0.108	98	5.19	107	5.66	6.84	5.69	-10	-1	108
D21*	0.041	26.4	62.5	84.7	86.5	0.73	13.68	0.137	111	4.20	117	4.43	6.16	4.46	-6	-1	118
D22*	0.037	23.7	40.9	76.2	78.5	0.60	15.64	0.156	74	3.12	81	3.42	5.77	3.44	-10	-1	81
D23	0.005	3.5	56.7	82.2	88.0	0.69	11.05	0.111	13	3.71	15	4.29	6.78	4.68	-26	-9	16
D24	0.025	16.1	25.7	71.5	74.5	0.50	15.06	0.151	42	2.61	48	2.98	5.88	2.97	-14	1	48
D25	0.017	10.8	53.2	81.1	82.2	0.67	15.60	0.156	40	3.70	42	3.89	5.78	3.87	-4	1	42
D26	0.033	20.9	30.0	72.0	75.5	0.53	14.46	0.145	57	2.73	66	3.16	6.00	3.18	-17	-1	66
D27	0.008	5.0	46.5	78.3	80.0	0.63	14.35	0.144	18	3.60	19	3.80	6.02	3.79	-5	0	19
D28	0.004	2.8	49.6	79.5	84.0	0.65	13.71	0.137	9	3.21	11	3.93	6.15	3.98	-24	-1	11
D29	0.018	11.6	37.8	75.1	78.2	0.58	14.57	0.146	35	3.02	40	3.45	5.98	3.45	-14	0	40
D30	0.017	11.0	25.5	71.5	74.4	0.50	15.72	0.157	28	2.55	32	2.91	5.76	2.90	-14	0	32
D31	0.007	4.5	40.0	76.0	78.5	0.59	14.55	0.146	14	3.11	16	3.56	5.98	3.53	-13	1	16
D32	0.006	3.7	5.0	69.0	68.0	0.38	11.45	0.115	10	2.70	9	2.43	6.68	2.54	6	-4	9
D33*	0.036	23.3	10.4	75.1	70.2	0.41	13.83	0.138	72	3.09	59	2.53	6.13	2.53	18	0	59
D34	0.039	24.7	36.5	74.5	77.0	0.57	15.92	0.159	73	2.96	80	3.24	5.72	3.26	-10	-1	80
D35	0.039	24.7	40.0	76.0	78.0	0.59	16.11	0.161	77	3.12	83	3.36	5.69	3.36	-8	0	83
D36*	0.024	15.2	19.5	68.2	72.0	0.47	23.74	0.237	28	1.84	34	2.24	4.63	2.16	-17	3	33
D37	0.011	6.8	93.1	94.6	99.0	0.91	12.41	0.124	36	5.29	40	5.88	6.44	5.85	-11	1	40
TOTAL	1.258	812.000															

**KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
MASTER DEVELOPMENT DRAINAGE PLAN**

HISTORIC (UNDEVELOPED) CONDITION LAG TIME ESTIMATE

5/6/1999

BASIN ID.	OVERLAND FLOW				GRASS LINED SWALE				NATURAL CHANNEL				TOTAL	TOTAL	TOTAL	
	L (ft)	C(10YR)	S (%)	TC(min)	L (ft)	S (%)	V (fps)	TC(min)	TYPE	L (ft)	S(%)	V (fps)	TC(min)	TC(min)	LAG(min.)	LAG(hrs)
H1	500	0.25	2.2	26.37	2060	4.1	3.0	11.44	1	1260	2.9	5.0	4.20	42.02	25.21	0.420
H2	500	0.25	5.6	19.38	1350	4.6	3.2	7.03	2	1510	2.4	6.5	3.87	30.28	18.17	0.303
H3	500	0.25	5.2	19.86	550	4.7	3.2	2.86	1	2350	3.1	4.5	8.70	31.42	18.85	0.314
H4	350	0.25	8.6	14.07				0.00	1	1900	2.9	6.7	4.73	18.80	11.28	0.188
H5	500	0.25	6.2	18.74	930	4.8	3.3	4.70	1	1540	4.5	5	5.13	28.57	17.14	0.286
H6	500	0.25	4.2	21.31	3330	4.3	3.1	17.90					0.00	39.21	23.53	0.392
H7	500	0.25	3.4	22.84	2040	3.1	2.6	13.08	3				0.00	35.92	21.55	0.359
H8	500	0.25	5	20.11	760	2.9	2.6	4.87	3	2140	3	3.7	9.64	34.63	20.78	0.346
H9	500	0.25	6.4	18.54	590	5.8	3.7	2.66	3	3230	2.9	6.3	8.54	29.74	17.85	0.297
H10	500	0.25	10	16.00	320	13	5.2	1.03	2	2170	4.6	8	4.52	21.55	12.93	0.215
H11	500	0.25	2.2	26.37	800	2.4	2.4	5.56	2	3750	5.6	6.8	9.19	41.12	24.67	0.411
H12	500	0.25	4.4	20.98	3250	3.8	3.1	17.47	2	600	8.3	8	1.25	39.70	23.82	0.397

UPPER OVERLAND FLOW (TC=1.8*(1.1-C10)*(L^0.5)*S^-0.33)

GRASS LINED SWALE VELOCITIES BASED ON SCS TR 55 CHART
FLOW RATE

NATURAL CHANNEL VELOCITIES BASED ON MANNINGS SOLUTION FOR AN APPROXIMATE AVERAGE
SECTION CARRYING AN ESTIMATED FLOW RATE "n" VARIES FROM .040 TO .070 DEPENDING ON LOCATION

**KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
MASTER DEVELOPMENT DRAINAGE PLAN
DEVELOPED CONDITION LAG TIME ESTIMATE
3/8/2001**

BASIN ID.	OVERLAND FLOW				SWALE OR STREET					CHANNEL OR STORM DRAIN				TOTAL TC(min)	TOTAL LAG(min.)	TOTAL LAG(hrs)	
	L (ft)	C(10YR)	S (%)	TC(min)	TYPE	L (ft)	S (%)	V (fps)	TC(min)	TYPE	L (ft)	S(%)	V (fps)				TC(min)
D1	500	0.25	2.2	27.40	SWALE	2100	4.1	3.2	10.94					0.00	38.34	23.00	0.383
D2	500	0.25	7.8	18.04	SWALE	970	4.9	3.7	4.37					0.00	22.41	13.45	0.224
D3	300	0.25	2.7	19.84	SWALE	1400	4.4	3.4	6.86					0.00	26.70	16.02	0.267
D4	500	0.25	8	17.89	SWALE	1260	4.1	3.2	6.56					0.00	24.46	14.67	0.245
D5	300	0.25	3.3	18.57	SWALE	400	3.5	2.7	2.47	CHAN	1030	3.7	6	2.86	23.90	14.34	0.239
D6	100	0.25	2	12.65	STREET	1550	3.8	6.8	3.79					0.00	16.43	9.86	0.164
D7	100	0.25	2	12.65	STREET	1550	2.5	5.5	4.67					0.00	17.31	10.39	0.173
D8	300	0.75	3	7.89	STREET	830	3	6.1	2.28	SD	800	3	14	0.95	11.12	6.67	0.111
D9*	400	0.25	5	18.69	STREET	1900	2	4.9	6.40					0.00	25.09	15.05	0.251
D9A	150	0.25	2	15.49	STREET	1800	2.2	5.2	5.78					0.00	21.27	12.76	0.213
D10	500	0.25	4	22.49	SWALE	1230	2.6	2.5	8.20					0.00	30.69	18.42	0.307
D11	200	0.25	2	17.88	STREET	1300	1.4	4.1	5.23					0.00	23.11	13.87	0.231
D12	500	0.25	4	22.49	SWALE	900	3.4	2.7	5.56					0.00	28.05	16.83	0.280
D13*	300	0.75	2	9.02	STREET	1000	2.7	5.8	2.90	SD	300	2	12	0.42	12.33	7.40	0.123
D14*	100	0.25	2	12.65	STREET	780	2	4.9	2.63	SD	400	2	12	0.56	15.83	9.50	0.158
D15*	120	0.25	2	13.85	STREET	2000	2.5	5.5	6.02	SD	50	1	7	0.12	19.99	12.00	0.200
D16	120	0.25	2	13.85	SWALE	2100	3	6.0	5.83					0.00	19.69	11.81	0.197
D16A	100	0.25	2	12.65	SWALE	600	3	6.0	1.67					0.00	14.31	8.59	0.143
D17	50	0.25	10	5.26	STREET	1900	2	4.9	6.40					0.00	11.65	6.99	0.117
D17A	50	0.25	2	8.94	STREET	1000	2.5	5.5	3.01					0.00	11.95	7.17	0.120
D18	300	0.25	2.0	21.90						CHAN	1200	3	6.00	3.33	25.24	15.14	0.252
D19	300	0.8	1.7	8.16	STREET	1500	3.6	6.6	3.76	SD	200	3	14	0.24	12.16	7.30	0.122
D20	300	0.75	2	9.02	STREET	600	2.5	5.5	1.81					0.00	10.83	6.50	0.108
D21	300	0.68	2	10.82	STREET	950	2.5	5.5	2.86					0.00	13.68	8.21	0.137
D22	100	0.25	2	12.65	STREET	1100	3.6	6.6	2.76	SD	200	3	14	0.24	15.64	9.39	0.156
D23	100	0.25	5	9.35	STREET	800	5	7.8	1.70					0.00	11.05	6.63	0.110
D24	100	0.25	2	12.65	STREET	950	3.5	6.5	2.42					0.00	15.06	9.04	0.151
D25*	100	0.25	2	12.65	STREET	900	3	6.1	2.47	SD	400	2.5	14	0.48	15.60	9.36	0.156

F.
HEC-1 MODEL OUTPUT
FULLY DEVELOPED CONDITION
• 100-YEAR STORM

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   MAY 1991
*   VERSION 4.0.1E
*
* RUN DATE 10/21/2002 TIME 15:02:17
*
*****
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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
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X   X   XXXXXX   XXXXX   X
X   X   X       X   X   XX
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XXXXXXX XXXX   X       XXXXX X
X   X   X       X       X
X   X   X       X   X   X
X   X   XXXXXX   XXXXX   XXX

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::::::::::::::::::::::::::::::::::::::::::::::::::::
::: Full Microcomputer Implementation :::
::: by :::
::: Haestad Methods, Inc. :::
::: :::
::::::::::::::::::::::::::::::::::::::::::::::::::::
::::::::::::::::::::::::::::::::::::::::::::::::::::

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37 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

PAGE 1

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1          ID   KETTLE CREEK OLD RANCH RD. TRIBUTARY WATERSHED IN PROJECTED FULLY DEVELOPED
2          ID   CONDITION (100 YEAR 24 HOUR RAINFALL, TYPE IIa SCS DISTRIBUTION)
3          ID   FILE NAME:KCD500.DAT
4          ID   3 MINUTE TIME STEP USED DUE TO SMALL SIZE OF BASINS, THIS LIMITS OUTPUT TO
5          ID   FIRST 15 HOURS OF DESIGN STORM
6          ID   *****
7          ID   BEGIN CALCULATIONS IN THE SOUTH TRIBUTARY WATERSHED
8          ID   *****

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*** FREE ***

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*DIAGRAM
9          IT      3      0      0      300
10         IO      5
11         PG      2      4.4
12         PG      1      0
13         IN      15
14         PC      0000   .0005   .0015   .0030   .0045   .0060   .0080   .0100   .0120   .0143
15         PC      .0165   .0188   .0210   .0233   .0255   .0278   .0320   .0390   .0460   .0530
16         PC      .0600   .0750   .1000   .4000   .7000   .7250   .7500   .7650   .7800   .7900
17         PC      .8000   .8100   .8200   .8250   .8300   .8350   .8400   .8450   .8500   .8550
18         PC      .8600   .8638   .8675   .8713   .8750   .8788   .8825   .8863   .8900   .8938

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19	PC	.8975	.9013	.9050	.9083	.9115	.9148	.9180	.9210	.9240	.9270
20	PC	.9300	.9325	.9350	.9375	.9400	.9425	.9450	.9475	.9500	.9525
21	PC	.9550	.9575	.9600	.9625	.9650	.9675	.9700	.9725	.9750	.9775
22	PC	.9800	.9813	.9825	.9838	.9850	.9863	.9875	.9888	.9900	.9913
23	PC	.9925	.9938	.9950	.9963	.9975	.9988	1.000			

24	KK	SB-D1									
25	KM	COMPUTE HYDROGRAPH FOR BASIN D1									
26	BA	.085									
27	PR	1									
28	PW	1									
29	PT	2									
30	PW	1									
31	LS	0	67.5								
32	UD	.383									

33	KK	RT-SBD1									
34	KM	ROUTE THE FLOW FROM SB-D1 TO AP-D1									
35	RD	1200	.045	.029		TRAP	10	10			

36	KK	SB-D2									
37	KM	COMPUTE HYDROGRAPH FOR BASIN D2									
38	BA	0.057									
39	PR	1									
40	PW	1									
41	PT	2									
42	PW	1									
43	LS	0	69.0								
44	UD	.224									

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

45	KK	AP-D1									
46	KM	COMBINE THE FLOW FROM BASIN D2 WITH THE ROUTED FLOW FROM BASIN D1 AT AP-D1									
47	HC	2									
48	KK	SB-D3									
49	KM	COMPUTE HYDROGRAPH FOR BASIN D3									
50	BA	0.026									
51	PR	1									
52	PW	1									
53	PT	2									
54	PW	1									
55	LS	0	75								
56	UD	.267									
57	KK	AP-DDA									
58	KM	COMBINE THE FLOW FROM BASIN D3 WITH THE FLOW FROM AP-D1									
59	HC	2									
60	KK	RR-DFA									
61	KM	ROUTE FLOW THROUGH A PROPOSED SMALL DETENTION FACILITY									
62	KM	ASSUME A 30" DIA OUTLET WITH INVERT AT EL. 54. OUTLET Q ESTIMATED WITH									
63	KM	BUREAU OF PUBLIC ROADS NOMOGRAPH FOR INLET CONTROL OF CULVERTS. VOLUME									
64	KM	BASED ON VERY CONCEPTUAL PLAN THAT ASSUMES A ROAD EMBANKMENT WILL SERVE									
65	KM	AS A DAM AND THE GRADING BEHIND THE EMBANKMENT WILL REMAIN AS IS.									
66	KO	1	1								
67	RS	1	STOR	0							
68	SV	0	.08	0.44	1.23	2.59	4.64	7.4			
69	SE	54	56	58	60	62	64	66			
70	SQ	0	37	43	50	60	68	75			
71	KKRT	APDFA									
72	KM	ROUTE THE FLOW FROM AP-DFA TO AP-D2									
73	RD	1000	.02	.013		CIRC	2.5				
74	KK	SB-D6									
75	KM	COMPUTE HYDROGRAPH FOR BASIN D6									
76	BA	0.039									
77	PR	1									
78	PW	1									
79	PT	2									
80	PW	1									
81	LS	0	79.2								
82	UD	.164									

83 KK AP-D2
 84 KM COMBINE THE FLOW FROM BASIN D6 WITH THE ROUTED FLOW FROM DFA
 85 HC 2

86 KK RT-APD2
 87 KM ROUTE THE FLOW FROM AP-D2 TO AP-D3
 88 RD 1600 .02 .013 CIRC 4
 HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

89 KK SB-D7
 90 KM COMPUTE HYDROGRAPH FOR BASIN D7
 91 BA 0.071
 92 PR 1
 93 PW 1
 94 PT 2
 95 PW 1
 96 LS 0 79.8
 97 UD .173

98 KK SB-D4
 99 KM COMPUTE HYDROGRAPH FOR BASIN D4
 100 BA 0.048
 101 PR 1
 102 PW 1
 103 PT 2
 104 PW 1
 105 LS 0 68.2
 106 UD .245

107 KK SB-D5
 108 KM COMPUTE HYDROGRAPH FOR BASIN D5
 109 BA 0.030
 110 PR 1
 111 PW 1
 112 PT 2
 113 PW 1
 114 LS 0 74.5
 115 UD .239

116 KK AP-DFB
 117 KM COMBINE THE FLOW FROM BASIN D5 WITH THE FLOW FROM BASIN D4
 118 HC 2

119 KK RR-DFB
 120 KM ROUTE FLOW THROUGH A PROPOSED SMALL DETENTION FACILITY
 121 KM ASSUME A 30" DIA OUTLET WITH INVERT AT EL.19. OUTLET Q ESTIMATED WITH
 122 KM BUREAU OF PUBLIC ROADS NOMOGRAPH FOR INLET CONTROL OF CULVERTS. VOLUME
 123 KM BASED ON VERY CONCEPTUAL PLAN THAT ASSUMES A ROAD EMBANKMENT WILL SERVE
 124 KM AS A DAM AND THE GRADING BEHIND THE EMBANKMENT WILL REMAIN AS IS.
 125 KO 1 1
 126 RS 1 STOR 0
 127 SV 0 .02 0.18 0.69 1.44 3.23
 128 SE 19 20 22 24 26 28
 129 SQ 0 37 43 50 60 68

130 KKRT-APDFB
 131 KM ROUTE THE FLOW FROM DFB TO AP3
 132 RD 800 .02 .013 CIRC 2.5
 HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

133 KK AP-D3
 134 KM COMBINE THE ROUTED FLOW FROM DF-B WITH THE ROUTED FLOW FROM AP-D2 AND
 135 KM BASIN D7
 136 HC 3

137 KK RT-APD3
 138 KM ROUTE THE FLOW FROM AP-D3 TO AP-DFC
 139 RD 600 .02 .013 CIRC 5.5

140 KK SB-D8

141 KM COMPUTE HYDROGRAPH FOR BASIN D8
 142 BA 0.062
 143 PR 1
 144 PW 1
 145 PT 2
 146 PW 1
 147 LS 0 92.2
 148 UD .111

 149 KK AP-DFC
 150 KM COMBINE THE ROUTED FLOW FROM AP-3 WITH THE FLOW FROM BASIN D8
 151 HC 2

 152 KK RR-DFC
 153 KM ROUTE FLOW THROUGH A PROPOSED REGIONAL DETENTION FACILITY ADJACENT TO POWERS
 154 KM BLVD. ASSUME A 30" DIA OUTLET WITH INVERT AT EL. 51. OUTLET Q ESTIMATED WITH
 155 KM BUREAU OF PUBLIC ROADS NOMOGRAPH FOR INLET CONTROL OF CULVERTS. VOLUME
 156 KM BASED ON VERY CONCEPTUAL GRADING PLAN.
 157 KO 1 1
 158 RS 1 STOR 0
 159 SV 0 .9 2.1 3.4 6.3 9.7 13.6 17.8 22.6 27.8
 160 SE 52 54 55 56 58 60 62 64 66 68
 161 SQ 0 28 37 45 58 66 74 81 88 94

 162 KK RT-DFC
 163 KM ROUTE THE FLOW FROM DFC TO AP-D4
 164 RD 950 0.01 0.013 CIRC 3.5

 165 KK SB-D9A
 166 KM COMPUTE HYDROGRAPH FOR BASIN D9A
 167 BA 0.018
 168 PR 1
 169 PW 1
 170 PT 2
 171 PW 1
 172 LS 0 74.5
 173 UD 0.213

 174 KK AP-D4
 175 KM COMBINE THE FLOW FROM BASIN D9A WITH THE ROUTED FLOW FROM AP-DFC
 176 HC 2

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

177 KK RT-APD4
 178 KM ROUTE THE FLOW FROM AP-D4 TO THE DOWNSTREAM END OF BASIN D9
 179 RD 700 .027 .013 CIRC 3.5

 180 KK SB-D9
 181 KM COMPUTE HYDROGRAPH FOR BASIN D9
 182 BA 0.022
 183 PR 1
 184 PW 1
 185 PT 2
 186 PW 1
 187 LS 0 77
 188 UD 0.251

 189 KK AP-D4a
 190 KM COMBINE THE FLOW FROM BASIN D9 WITH THE ROUTED FLOW FROM AP-D4
 191 HC 2

 192 KKRT-APD4a
 193 KM ROUTE THE FLOW FROM THE DOWNSTREAM END OF BASIN D9 TO THE DOWNSTREAM END OF
 194 KM BASIN D15
 195 RD 500 .027 .013 CIRC 3.5

 196 KK SB-D15
 197 KM COMPUTE HYDROGRAPH FOR BASIN D15
 198 BA 0.043
 199 PR 1
 200 PW 1
 201 PT 2
 202 PW 1
 203 LS 0 75

204 UD 0.200

205 KK AP-D4b

206 KM COMBINE THE FLOW FROM BASIN D15 WITH THE ROUTED FLOW FROM AP4 AND BASIN D9

207 HC 2

208 KKRT-APD4b

209 KM ROUTE THE FLOW FROM THE DOWNSTREAM END OF BASIN D15 TO AP7

210 RD 500 .027 .013 CIRC 6.0

211 KK SB-D10

212 KM COMPUTE HYDROGRAPH FOR BASIN D10

213 BA 0.027

214 PR 1

215 PW 1

216 PT 2

217 PW 1

218 LS 0 68

219 UD .307

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

220 KKRT-SBD10

221 KM ROUTE THE FLOW FROM SB-D10 TO AP-D5 ASSUME FLOW IS ROUTED IN A RESIDENTIAL

222 KM STREET SECTION USE A ROUGH EQUIVILANT TRAPAZOIDAL SECTION TO MODEL

223 RD 1200 .04 .017 TRAP 0 50

224 KK SB-D11

225 KM COMPUTE HYDROGRAPH FOR BASIN D11

226 BA 0.051

227 PR 1

228 PW 1

229 PT 2

230 PW 1

231 LS 0 74.2

232 UD .231

233 KK SB-D12

234 KM COMPUTE HYDROGRAPH FOR BASIN D12

235 BA 0.021

236 PR 1

237 PW 1

238 PT 2

239 PW 1

240 LS 0 74.6

241 UD .281

242 KKRT-SBD12

243 KM ROUTE THE FLOW FROM SB-D12 TO AP-D5 ASSUME FLOW IS ROUTED IN THE NORTH SIDE

244 KM OF THE OLD RANCH ROAD STREET SECTION USE A ROUGH EQUIVILANT TRAPAZOIDAL

245 KM SECTION TO MODEL

246 RD 1200 .038 .017 TRAP 0 25

247 KK AP-D5

248 KM COMBINE THE FLOW FROM BASIN D11 WITH THE ROUTED FLOW FROM BASINS D10 AND D12

249 KM AT THE INTERSECTION OF OLD RANCH RD. AND THE STREET TO THE HIGH SCHOOL

250 HC 3

251 KK RT-APD5

252 KM ROUTE THE FLOW FROM AP-D5 TO AP-D6

253 RD 700 .03 .013 CIRC 5.0

254 KK SB-D13

255 KM COMPUTE HYDROGRAPH FOR BASIN D13

256 BA 0.067

257 PR 1

258 PW 1

259 PT 2

260 PW 1

261 LS 0 91.5

262 UD .123

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

263	KK	AP-D6				
264	KM	COMBINE THE ROUTED FLOW FROM AP-D5 WITH THE FLOW FROM BASIN D13 AT AP-D6				
265	HC	2				
266	KK	RT-APD6				
267	KM	ROUTE THE FLOW FROM AP-D6 TO AP-D7				
268	RD	1100	0.02	0.013	CIRC	6.0
269	KK	SB-D14				
270	KM	COMPUTE HYDROGRAPH FOR BASIN D14				
271	BA	0.023				
272	PR	1				
273	PW	1				
274	PT	2				
275	PW	1				
276	LS	0	78			
277	UD	0.158				
278	KKRT-SBD14					
279	KM	ROUTE THE FLOW FROM BASIN D14 TO AP-D7				
280	RD	300	.02	.013	CIRC	2
281	KK	AP-D7				
282	KM	COMBINE THE ROUTED FLOW FROM BASIN D14 WITH THE ROUTED FLOW AT AP-D7				
283	HC	3				
284	KK	RT-APD7				
285	KM	ROUTE THE FLOW FROM AP-D7 TO AP-D7A				
286	RD	850	0.02	0.013	CIRC	7
287	KK	SB-D17				
288	KM	COMPUTE HYDROGRAPH FOR BASIN D17				
289	BA	0.010				
290	PR	1				
291	PW	1				
292	PT	2				
293	PW	1				
294	LS	0	80.5			
295	UD	0.117				
296	KK	AP-D7A				
297	KM	COMBINE THE FLOW FROM BASIN D17 WITH THE ROUTED FLOW AT AP-D7A				
298	HC	2				
299	KK	SB-D16A				
300	KM	COMPUTE HYDROGRAPH FOR BASIN D16A				
301	BA	0.013				
302	PR	1				
303	PW	1				
304	PT	2				
305	PW	1				
306	LS	0	78			
307	UD	0.143				
		HEC-1 INPUT				
LINE	ID.....	1.....	2.....	3.....	4.....	5.....6.....7.....8.....9.....10
308	KK	AP-D7A				
309	KM	COMBINE THE ROUTED FLOW FROM BASIN D16A WITH THE ROUTED FLOW AT AP-D7A				
310	HC	2				
311	KK	RT-APD8				
312	KM	ROUTE THE FLOW FROM AP-D7A TO AP-D8				
313	RD	500	0.015	0.013	CIRC	7
314	KK	SB-D17A				
315	KM	COMPUTE HYDROGRAPH FOR BASIN D17A				
316	BA	0.010				
317	PR	1				
318	PW	1				
319	PT	2				
320	PW	1				
321	LS	0	99			
322	UD	0.120				

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323      KK  AP-D8
324      KM  COMBINE THE FLOW FROM BASIN D17A WITH THE ROUTED FLOW AT AP-D8
325      HC      2

326      KK  SB-D16
327      KM  COMPUTE HYDROGRAPH FOR BASIN D16
328      BA  0.062
329      PR      1
330      PW      1
331      PT      2
332      PW      1
333      LS      0      72.5
334      UD  0.197

335      KKRT-SBD16
336      KM  ROUTE THE FLOW FROM BASIN D16 TO AP-D8
337      RD  400      0.02      0.013      CIRC      3

338      KK  AP-D8
339      KM  COMBINE THE ROUTED FLOW FROM BASIN D16 WITH THE ROUTED FLOW AT AP-D8
340      HC      2

341      KK  RT-APD9
342      KM  ROUTE THE FLOW FROM AP-D8 TO AP-D9
343      RD  430      0.02      0.013      CIRC      7

344      KK  SB-D36
345      KM  COMPUTE HYDROGRAPH FOR BASIN D36
346      BA  0.024
347      PR      1
348      PW      1
349      PT      2
350      PW      1
351      LS      0      72
352      UD  0.237

                                     HEC-1 INPUT

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

353      KKRT-SBD36
354      KM  ROUTE THE FLOW FROM BASIN D36 TO AP-D9
355      RD  750      0.02      0.013      CIRC      2.5

356      KK  AP-D9
357      KM  COMBINE THE ROUTED FLOW FROM BASIN D36 WITH THE ROUTED FLOW AT AP-D9
358      HC      2

359      KK  SB-D37
360      KM  COMPUTE HYDROGRAPH FOR BASIN D37
361      BA  0.011
362      PR      1
363      PW      1
364      PT      2
365      PW      1
366      LS      0      99
367      UD  0.124

368      KKRT-SBD37
369      KM  ROUTE THE FLOW FROM BASIN D37 TO AP-D9
370      RD  200      0.02      0.013      CIRC      2.5

371      KK  AP-D9
372      KM  COMBINE THE ROUTED FLOW FROM BASIN D37 WITH THE ROUTED FLOW AT AP-D9
373      HC      2

374      KKRT-APDFE
375      KM  ROUTE THE FLOW FROM AP-D9 TO DFE, ASSUME 96"PIPE
376      RD  350      0.01      .013      CIRC      8

377      KK  SB-D18
378      KM  COMPUTE HYDROGRAPH FOR BASIN D18
379      BA  0.064
380      PR      1
381      PW      1
382      PT      2
383      PW      1

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384 LS 0 76.3
385 UD .252

386 KKRR-DFPCHS
387 KM ROUTE FLOW THROUGH THE EXISTING PINE CREEK HIGH SCHOOL DETENTION POND
388 KM STORAGE IS BASED ON A TAKEOFF MADE FROM THE GRADING PLAN FOR THE SITE
389 KM OUTLET DISCHARGE IS ESTIMATED BASED ON ON A VERY CRUDE FIELD SURVEY
390 KM OF THE POND OUTLET STRUCTURE
391 KO 1 1
392 RS 1 STOR 0
393 SV 0 .2 .44 .92 1.47 2.17 3.73
394 SE 16 18 19 20 21 22 24
395 SQ 0 0 0 0 7.9 30 40
HEC-1 INPUT

PAGE 10

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

396 KKRT-RRDFPCHS
397 KM ROUTE THE FLOW FROM DFPCHS IN A STORM DRAIN TO AP-D10
398 RD 400 .04 .013 CIRC 2.0

399 KK SB-D19
400 KM COMPUTE HYDROGRAPH FOR BASIN D19
401 BA 0.024
402 PR 1
403 PW 1
404 PT 2
405 PW 1
406 LS 0 80.9
407 UD .122

408 KK AP-D10
409 KM COMBINE THE ROUTED FLOW FROM THE HIGH SCHOOL DETENTION POND WITH THE FLOW
410 KM FROM SB-D10
411 HC 2

412 KKRT-APD10
413 KM ROUTE THE FLOW FROM AP-D10 TO AP-D11
414 RD 800 .02 .013 CIRC 3.0

415 KK SB-D20
416 KM COMPUTE HYDROGRAPH FOR BASIN D20
417 BA 0.030
418 PR 1
419 PW 1
420 PT 2
421 PW 1
422 LS 0 96.5
423 UD .108

424 KK AP-D11
425 KM COMBINE THE ROUTED FLOW FROM AP-D10 WITH THE FLOW FROM SB-D20
426 HC 2

427 KKRT-APD11
428 KM ROUTE THE FLOW FROM AP-D11 TO PROPOSED DETENTION FACILITY "E"
429 RD 1300 .02 .013 CIRC 5.0

430 KK SB-D21
431 KM COMPUTE HYDROGRAPH FOR BASIN D21
432 BA 0.041
433 PR 1
434 PW 1
435 PT 2
436 PW 1
437 LS 0 86.5
438 UD .137

HEC-1 INPUT

PAGE 11

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

439 KK AP-DFE
440 KM COMBINE THE ROUTED FLOW FROM AP-D11 WITH THE FLOW FROM SB-D21 AND AP-D8A
441 KM THIS IS THE TOTAL FLOW TO PROPOSED
442 KM DETENTION FACILITY "E"

443 HC 3

444 KK RR-DFE

445 KM ROUTE FLOW THROUGH THE PROPOSED DETENTION FACILITY "E". STORAGE IS BASED ON

446 KM A CONCEPTUAL GRADING PLAN DATED 11-21-00 WITH THE POND LOCATED AT THE NW

447 KM CORNER OF THE INTERSECTION OF OLD RANCH RD. AND CHAPEL HILLS DRIVE. OUTLET

448 KM CAPACITY IS BASED ON A 6" DIA ORIFICE OUTLET WITH INV AT EL 22.25, A 27" DIA

449 KM ORIFICE OUTLET AT INVERT ELEV. 24.00.

450 KM 90 DEGREE V-NOTCH WEIR INVERT ELEV. 31.0, TRIMMED VERT AT 10' WIDTH ELEV. 36.0

451 KM 6" DIA. OUTLET ALLOWS LOW FLOW TO CONTINUE DOWN THE HISTORIC NATURAL CHANNEL

452 KM 90 DEGREE V-NOTCH OUTFALLS TO THE HISTORIC NATURAL CHANNEL TO DIRECT SOME OF

453 KM THE PEAK FLOW FROM LARGE STORMS TO THE NATURAL CHANNEL

454 KM THE 27" DIA ORIFICE OUTLET OUTFALLS TO A PROPOSED STORM DRAIN IN OLD RANCH RD

455 KO 1 1

456 RS 1 STOR 0

457 SV 0 0.09 .74 2.79 5.13 7.78 10.82 12.47 14.21 16.03

458 SV 17.95 19.92 21.98 24.14 26.39 28.75 31.20 33.76 36.42

459 SE 22.5 23.0 24.0 26.0 28.0 30.0 32.0 33.0 34.0 35.0

460 SE 36 37.0 38.0 39.0 40.0 41.0 42.0 43.0 44.0

461 SQ 0.0 0.7 1.20 19.7 34.7 44.9 55.6 70.6 98 141

462 SQ 201 236 309 435 530 632 750 877 1016

463 KK AP-DFE

464 KM DIVERT OUT FLOW THAT PASSES THROUGH THE V-NOTCH AND THE 6" DIA OUTLET TO THE

465 KM NATURAL CHANNEL

466 KO 1 1

467 DT AP-D12

468 DI 0.0 0.7 1.20 19.7 34.7 44.9 55.6 70.6 98 141

469 DI 201 236 309 435 530 632 750 877 1016

470 DQ 0 0.7 1.2 1.8 2.2 2.6 5.4 16.9 41 80.5

471 DQ 137.7 169.6 240.7 363.8 456 556 671 796 932.4

472 KKRT-APD13

473 KM ROUTE THE FLOW FROM THE 24" DIA OUTLET IN DETENTION FACILITY "E" DOWN TO

474 KM AP-D13 IN THE PROPOSED OLD RANCH ROAD STORM DRAIN

475 RD 580 .02 .013 CIRC 3.5

476 KK SB-D22

477 KM COMPUTE HYDROGRAPH FOR BASIN D22

478 BA 0.037

479 PR 1

480 PW 1

481 PT 2

482 PW 1

483 LS 0 78.5

484 UD 0.156

HEC-1 INPUT

PAGE 12

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

485 KK AP-D13

486 KM COMBINE FLOW FROM BASIN D22 WITH ROUTED FLOW TO AP D13

487 HC 2

488 KKRT-APD13

489 KM ROUTE THE FLOW FROM AP-13 TO AP D14

490 RD 850 .03 .013 CIRC 4

491 KK SB-D23

492 KM COMPUTE HYDROGRAPH FOR BASIN D23

493 BA 0.005

494 PR 1

495 PW 1

496 PT 2

497 PW 1

498 LS 0 88.0

499 UD .111

500 KK AP-D14

501 KM COMBINE ROUTED FLOW FROM AP-D13 WITH THE FLOW FROM BASIN D23

502 HC 2

503 KK AP-DFF

504 KM ROUTE THE FLOW FROM AP-14 TO PROPOSED DETENTION FACILITY "F"

505 RD 200 .03 .013 CIRC 4.5

506 KK SB-D24
 507 KM COMPUTE HYDROGRAPH FOR BASIN D24
 508 BA 0.025
 509 PR 1
 510 PW 1
 511 PT 2
 512 PW 1
 513 LS 0 74.5
 514 UD .151

 515 KK AP-DFF
 516 KM COMBINE ROUTED FLOW FROM AP-D14 WITH THE FLOW FROM BASIN D24
 517 HC 2

 518 KK RR-DFF
 519 KM KM ROUTE FLOW THROUGH A POND EAST OF THE EXISTING CREEKSIDE FILING 3 AREA
 520 KM STORAGE IS BASED ON THE CONSTRUCTION PLAN CONTOURS
 521 KM OUTLET CAPACITY IS BASED ON A 12" DIA OUTLET PIPE WITH INVERT AT ELEV. 58
 522 KM AND A SECOND OUTLET A 48" DIA STAND PIPE RIM AT ELEVATION 68
 523 KM THE PROPOSED OUTLET FOR THIS POND IS DESIGNED TO SIGNIFICANTLY
 524 KM LAG THE PEAK FLOWS TO THE DOWNSTREAM STORM DRAIN SYSTEM THUS OUTFLOW
 525 KM IS VERY RESTRICTED UNTIL THE POND IS NEARLY FULL
 526 KO 1 1
 527 RS 1 STOR 0
 528 SV 0 0.0 0.40 1.60 3.04 4.76 6.78 9.13 11.81
 529 SE 58 58.5 60 62 64 66 68 70 72
 530 SQ 0 0.5 4.6 7.1 8.9 10.4 11.7 98.4 135

 HEC-1 INPUT

PAGE 13

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

531 KK SB-D25
 532 KM COMPUTE HYDROGRAPH FOR BASIN D25
 533 BA 0.017
 534 PR 1
 535 PW 1
 536 PT 2
 537 PW 1
 538 LS 0 82.2
 539 UD .156

 540 KKRR-SBD25
 541 KM ROUTE FLOW FROM SB-D25 DOWN THE EXISTING STORM DRAIN FROM THE LOW POINT IN
 542 KM LEXINGTON DR. TO THE LOW POINT IN MONMOUTH LANE AT AP-D16
 543 RD 800 .05 .013 CIRC 3.0

 544 KK SB-D26
 545 KM COMPUTE HYDROGRAPH FOR BASIN D26
 546 BA 0.033
 547 PR 1
 548 PW 1
 549 PT 2
 550 PW 1
 551 LS 0 75.5
 552 UD .145

 553 KK AP-D16
 554 KM COMBINE THE ROUTED FLOW FROM BASIN D26 WITH THE ROUTED FLOW FROM BASIN D25
 555 HC 2

 556 KKRT-APD16
 557 KM ROUTE THE FLOW IN THE EXISTING STORM DRAIN FROM AP-D16 TO AP-D17 IN OLD
 558 KM RANCH ROAD AND MAMOUTH LANE
 559 KO 1 1
 560 RD 400 .05 .013 CIRC 3.5

 561 KK AP-D17
 562 KM COMBINE THE ROUTED FLOW FROM AP-D16 WITH THE ROUTED FLOW FROM DETENTION
 563 KM FACILITY F
 564 KO 1 1
 565 HC 2

 566 KKRT-APD17
 567 KM ROUTE THE COMBINED FLOW AT AP-D17 DOWN THE EXISTING OLD RANCH ROAD STORM
 568 KM TO AP-D18
 569 RD 500 .05 .013 CIRC 3.5

570 KK SB-D27
 571 KM COMPUTE HYDROGRAPH FOR BASIN D27
 572 BA 0.008
 573 PR 1
 574 PW 1
 575 PT 2
 576 PW 1
 577 LS 0 80

HEC-1 INPUT

PAGE 14

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

578 UD .144

579 KK AP-D18
 580 KM COMBINE THE ROUTED FLOW FROM AP-D17 WITH THE FLOW FROM BASIN D27
 581 HC 2

582 KKRT-APD18
 583 KM ROUTE THE COMBINED FLOW AT AP-D18 DOWN THE EXISTING OLD RANCH ROAD STORM
 584 KM TO AP-D20
 585 RD 500 .05 .013 CIRC 3.5

586 KK SB-D29
 587 KM COMPUTE HYDROGRAPH FOR BASIN D29
 588 BA 0.018
 589 PR 1
 590 PW 1
 591 PT 2
 592 PW 1
 593 LS 0 78.2
 594 UD .146

595 KK AP-D19
 596 KM THE EXISTING STORM DRAIN THAT CONNECTS BASIN D29 TO THE EXISTING OLD RANCH
 597 KM ROAD STORM HAS LESS CAPACITY (DUE TO INLET CONTROL) THAN THE PREDICTED 100
 598 KM YEAR PEAK FLOW FROM BASIN D29. THE MAXIMUM CAPACITY OF THE STORM DRAIN IS
 599 KM 20 CFS. DIVERT OUT FLOW THAT EXCEEDS 20 CFS. THE EXCESS FLOW WILL CONTINUE
 600 KM DOWN LUMBERJACK DRIVE AND WILL NOT BE ROUTED THROUGH THE CREEKSIDE DETENTION
 601 KM POND
 602 DT AP-D19a
 603 DI 0 5 10 15 20 25 30 35 40 45
 604 DI 50 55
 605 DQ 0 0 0 0 0 5 10 15 20 25
 606 DQ 30 35

607 KKRT-APD19
 608 KM ROUTE FLOW COLLECTED IN THE STORM DRAIN AT D19 DOWN THE EXISTING STORM DRAIN
 609 KM TO AP-D20 IN OLD RANCH ROAD
 610 RD 150 .13 .013 CIRC 1.5

611 KK SB-D28
 612 KM COMPUTE HYDROGRAPH FOR BASIN D28
 613 BA 0.004
 614 PR 1
 615 PW 1
 616 PT 2
 617 PW 1
 618 LS 0 84.0
 619 UD .137

620 KK AP-D20
 621 KM COMBINE THE ROUTED FLOW FROM AP-D18 AND AP-D20 WITH THE FLOW FROM BASIN D28
 622 HC 3

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

623 KKRT-APD20
 624 KM ROUTE THE FLOW FROM AP-D20 TO AP-D21 IN THE EXISTING MARBLE CREEK STORM DRAIN
 625 RD 420 .025 .013 CIRC 4.0

626 KK SB-D30
 627 KM COMPUTE HYDROGRAPH FOR BASIN D30
 628 BA 0.017

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629      PR      1
630      PW      1
631      PT      2
632      PW      1
633      LS      0      74.4
634      UD      .157

635      KKRT-SBD30
636      KM      ROUTE THE FLOW FROM BASIN D30 TO AP-D21 IN THE EXISTING EASEMENT STORM DRAIN
637      RD      330      .065      .013      CIRC      2.0

638      KK      AP-D21
639      KM      COMBINE THE ROUTED FLOW FROM SB-D30 WITH THE ROUTED FLOW FROM AP-D20
640      KO      1      1
641      HC      2

642      KK      SB-D31
643      KM      COMPUTE HYDROGRAPH FOR BASIN D31
644      BA      0.007
645      PR      1
646      PW      1
647      PT      2
648      PW      1
649      LS      0      78.5
650      UD      .146

651      KK      AP-D22
652      KM      COMBINE THE ROUTED FLOW FROM SB-D31 WITH THE ROUTED FLOW FROM AP-D21
653      HC      2

654      KK      SB-D32
655      KM      COMPUTE HYDROGRAPH FOR BASIN D32
656      BA      0.006
657      PR      1
658      PW      1
659      PT      2
660      PW      1
661      LS      0      68.0
662      UD      .115

663      KK      AP-DFCS
664      KM      COMBINE THE ROUTED FLOW FROM SB-D32 WITH THE ROUTED FLOW FROM AP-D22
665      KM      THIS IS THE TOTAL FLOW INTO THE EXISTING CREEKSIDE DETENTION FACILITY
666      KO      1      1
667      HC      2

                                HEC-1 INPUT

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

668      KK      RR-DFCS
669      KM      ROUTE FLOW THROUGH THE EXISTING CREEKSIDE POND (DF*CS*). STORAGE IS BASED
670      KM      ON THE STORAGE DATA PRESENTED IN THE FINAL DRAINAGE REPORT FOR CREEKSIDE
671      KM      FILING No.3. OUTFLOW IS BASED ON A PROPOSED OUTLET MODIFICATION TO INCLUDE
672      KM      A 11' LONG HORZ. WEIR AT ELEV 79 AND A 24" DIA OUTLET WITH AN INVERT
673      KM      ELEVATION OF 72.
674      KO      1      1
675      RS      1      STOR      0
676      SV      0      .050      .398      1.026      1.723      2.488      3.325      4.235      5.221      6.283
677      SV      7.423      8.643      9.946
678      SE      72      73      74      75      76      77      78      79      80      81
679      SE      82      83      84
680      SQ      0      4      15      28      32      35      38      41      78      143
681      SQ      226      324      435

682      KKDR-APD12
683      KM      RETRIEVE THE FLOW THAT IS DIVERTED TO THE HISTORIC CHANNEL AT DETENTION
684      KM      FACILITY "E"
685      DR      AP-D12

686      KKRT-APD12
687      KM      ROUTE THE FLOW FROM DETENTION FACILITY "E" TO KETTLE CREEK IN THE EXISTING
688      KM      NATURAL CHANNEL. USE GENERALIZED SECTION AND AVERAGE SLOPE
689      RD      3700      .040      .050      TRAP      5      5

690      KK      SB-D33
691      KM      COMPUTE HYDROGRAPH FOR BASIN D33

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692      BA    0.036
693      PR      1
694      PW      1
695      PT      2
696      PW      1
697      LS      0    70.2
698      UD      .138

699      KK    AP-D23
700      KM    COMBINE THE ROUTED FLOW FROM SB-D33 WITH THE ROUTED FLOW FROM AP-D12
701      HC      2

702      KK    AP-D24
703      KM    COMBINE THE FLOW AT AP-D23 WITH THE OUTFLOW FROM THE MODIFIED CREEKSIDE
704      KM    DETENTION FACILITY FOR THE PURPOSE OF COMPARISON TO HISTORIC FLOW RATES
705      KO      1      1
706      HC      2
707      KM    *****
708      KM    BEGIN CALCULATIONS IN THE NORTH TRIBUTARY WATERSHED
709      KM    *****

710      KK    SB-D34
711      KM    COMPUTE HYDROGRAPH FOR BASIN D34
712      BA    0.039
713      PR      1
714      PW      1
715      PT      2

```

HEC-1 INPUT

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

716      PW      1
717      LS      0    77.0
718      UD      .159

719      KKRT-SBD34
720      KM    ROUTE FLOW FROM BASIN D34 IN A STORM DRAIN TO PROPOSED DETENTION FACILITY "G"
721      RD      900    .03    .013    CIRC    3.5

722      KK    SB-D35
723      KM    COMPUTE HYDROGRAPH FOR BASIN D35
724      BA    0.039
725      PR      1
726      PW      1
727      PT      2
728      PW      1
729      LS      0    78.0
730      UD      .161

731      KK    AP-DFG
732      KM    COMBINE THE ROUTED FLOW FROM AP-D34 WITH THE FLOW FROM BASIN D35
733      HC      2

734      KK    RR-DFG
735      KM    ROUTE FLOW THROUGH PROPOSED DETENTION FACILITY "G". STORAGE IS BASED ON A
736      KM    PRELIMINARY GRADING PLAN AND A TWO STAGE OUTLET WITH A 6" DIA OUTLET AT
737      KM    INVERT ELEV. 60 AND A 24" DIA STANDPIPE WITH A TOP ELEV OF 66
738      KO      1      1
739      RS      1    STOR      0
740      SV      0    .081    .262    .732    1.366    2.179    3.195    4.211    5.63
741      SE      60    61    62    64    66    68    70    72    74
742      SQ      0    0.8    1.3    1.8    2.3    24.0    33.2    40.3    46.3
743      ZZ

```

SCHEMATIC DIAGRAM OF STREAM NETWORK

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INPUT
LINE      (V) ROUTING      (--->) DIVERSION OR PUMP FLOW

NO.      (.) CONNECTOR      (<---) RETURN OF DIVERTED OR PUMPED FLOW

24      SB-D1
        V
        V
33      RT-SBD1
        .
        .
36      .      SB-D2

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45 AP-D1.....

48 SB-D3

57 AP-DDA.....

V

V

60 RR-DFA.....

V

V

71 RT-APDFA

74 SB-D6

83 AP-D2.....

V

V

86 RT-APD2

89 SB-D7

98 SB-D4

107 SB-D5

116 AP-DFB.....

V

V

119 RR-DFB.....

V

V

130 RT-APDFB

133 AP-D3.....

V

V

137 RT-APD3

140 SB-D8

149 AP-DFC.....

V

V

152 RR-DFC.....

V

V

162 RT-DFC

165 SB-D9A

174 AP-D4.....

V

V

177 RT-APD4

180 SB-D9

189 AP-D4a.....

V

V

192	RT-APD4a	.	.	.
		.	.	.
196		.	SB-D15	.
		.	.	.
205	AP-D4b		.	.
		V	.	.
		V	.	.
208	RT-APD4b	.	.	.
		.	.	.
211		.	SB-D10	.
		.	V	.
		.	V	.
220		.	RT-SBD10	.
		.	.	.
224		.	.	SB-D11
		.	.	.
233		.	.	SB-D12
		.	.	V
		.	.	V
242		.	.	RT-SBD12
		.	.	.
247		.	AP-D5	
		.	V	.
		.	V	.
251		.	RT-APD5	.
		.	.	.
254		.	.	SB-D13
		.	.	.
263		.	AP-D6	
		.	V	.
		.	V	.
266		.	RT-APD6	.
		.	.	.
269		.	.	SB-D14
		.	.	V
		.	.	V
278		.	.	RT-SBD14
		.	.	.
281	AP-D7		.	.
		V	.	.
		V	.	.
284	RT-APD7	.	.	.
		.	.	.
287		.	SB-D17	.
		.	.	.
296	AP-D7A		.	.
		.	.	.
299		.	SB-D16A	.
		.	.	.
308	AP-D7A		.	.
		V	.	.
		V	.	.
311	RT-APD8	.	.	.
		.	.	.
314		.	SB-D17A	.
		.	.	.
323	AP-D8		.	.
		.	.	.
326		.	SB-D16	.
		.	V	.

```

.      V
335      .      RT-SBD16
.
.
338      AP-D8.....
.      V
.      V
341      RT-APD9
.
.
344      .      SB-D36
.      V
.      V
353      .      RT-SBD36
.
.
356      AP-D9.....
.
.
359      .      SB-D37
.      V
.      V
368      .      RT-SBD37
.
.
371      AP-D9.....
.      V
.      V
374      RT-APDFE
.
.
377      .      SB-D18
.      V
.      V
386      .      RR-DFPCH
.      V
.      V
396      .      RT-RRDFP
.
.
399      .      .      SB-D19
.      .
.      .
408      .      AP-D10.....
.      V
.      V
412      .      RT-APD10
.
.
415      .      .      SB-D20
.      .
.      .
424      .      AP-D11.....
.      V
.      V
427      .      RT-APD11
.
.
430      .      .      SB-D21
.      .
.      .
439      AP-DFE.....
.      V
.      V
444      RR-DFE
.
.
467      -----> AP-D12
463      AP-DFE
.      V
.      V
472      RT-APD13
.
.
476      .      SB-D22
.
.

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485     AP-D13.....
      V
      V
488     RT-APD13
      .
      .
491     .          SB-D23
      .
      .
500     AP-D14.....
      V
      V
503     AP-DFF
      .
      .
506     .          SB-D24
      .
      .
515     AP-DFF.....
      V
      V
518     RR-DFF
      .
      .
531     .          SB-D25
      .          V
      .          V
540     .          RR-SBD25
      .
      .
544     .          .          SB-D26
      .          .
      .          .
553     .          AP-D16.....
      .          V
      .          V
556     .          RT-APD16
      .
      .
561     AP-D17.....
      V
      V
566     RT-APD17
      .
      .
570     .          SB-D27
      .
      .
579     AP-D18.....
      V
      V
582     RT-APD18
      .
      .
586     .          SB-D29
      .
      .
602     .          .-----> AP-D19a
595     .          AP-D19
      .          V
      .          V
607     .          RT-APD19
      .
      .
611     .          .          SB-D28
      .          .
      .          .
620     AP-D20.....
      V
      V
623     RT-APD20
      .
      .
626     .          SB-D30
      .          V
      .          V
635     .          RT-SBD30

```

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638      AP-D21.....
642      .          SB-D31
651      AP-D22.....
654      .          SB-D32
663      AP-DFCS.....
        V
        V
668      RR-DFCS
        .
685      .          .<----- AP-D12
682      .          DR-APD12
        .          V
        .          V
686      .          RT-APD12
        .          .
690      .          .          SB-D33
        .          .
699      .          AP-D23.....
        .          .
702      AP-D24.....
        .
710      .          SB-D34
        .          V
        .          V
719      .          RT-SBD34
        .          .
722      .          .          SB-D35
        .          .
731      .          AP-DFG.....
        .          V
        .          V
734      .          RR-DFG

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION
 HEC1 S/N: 1343000062 HMVersion: 6.33 Data File: KCD100.DAT

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 10/21/2002 TIME 15:02:17 *
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****

```

KETTLE CREEK OLD RANCH RD. TRIBUTARY WATERSHED IN PROJECTED FULLY DEVELOPED
 CONDITION (100 YEAR 24 HOUR RAINFALL, TYPE IIa SCS DISTRIBUTION)
 FILE NAME:KCD500.DAT
 3 MINUTE TIME STEP USED DUE TO SMALL SIZE OF BASINS, THIS LIMITS OUTPUT TO
 FIRST 15 HOURS OF DESIGN STORM

 BEGIN CALCULATIONS IN THE SOUTH TRIBUTARY WATERSHED

10 IO

OUTPUT CONTROL VARIABLES

IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT

HYDROGRAPH TIME DATA

NMIN 3 MINUTES IN COMPUTATION INTERVAL
 IDATE 1 0 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 300 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 1 0 ENDING DATE
 NDTIME 1457 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.05 HOURS
 TOTAL TIME BASE 14.95 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-Feet
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

60 KK

 * *
 * RR-DFA *
 * *

66 KO

OUTPUT CONTROL VARIABLES

IPRNT 1 PRINT CONTROL
 IPLOT 1 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

67 RS

STORAGE ROUTING

NSTPS 1 NUMBER OF SUBREACHES
 ITYP STOR TYPE OF INITIAL CONDITION
 RSVRIC 0.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

		0.0	0.1	0.4	1.2	2.6	4.6	7.4
68 SV	STORAGE	0.0	0.1	0.4	1.2	2.6	4.6	7.4
69 SE	ELEVATION	54.00	56.00	58.00	60.00	62.00	64.00	66.00
70 SQ	DISCHARGE	0.	37.	43.	50.	60.	68.	75.

HYDROGRAPH AT STATION RR-DFA

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
1	0000	1	0.	0.0	54.0	1	0500	101	0.	0.0	54.0	1	1000	201	7.	0.0	54.4			
1	0003	2	0.	0.0	54.0	1	0503	102	0.	0.0	54.0	1	1003	202	7.	0.0	54.4			
1	0006	3	0.	0.0	54.0	1	0506	103	0.	0.0	54.0	1	1006	203	7.	0.0	54.4			
1	0009	4	0.	0.0	54.0	1	0509	104	0.	0.0	54.0	1	1009	204	7.	0.0	54.4			
1	0012	5	0.	0.0	54.0	1	0512	105	0.	0.0	54.0	1	1012	205	7.	0.0	54.4			
1	0015	6	0.	0.0	54.0	1	0515	106	0.	0.0	54.0	1	1015	206	6.	0.0	54.3			
1	0018	7	0.	0.0	54.0	1	0518	107	0.	0.0	54.0	1	1018	207	6.	0.0	54.3			
1	0021	8	0.	0.0	54.0	1	0521	108	0.	0.0	54.0	1	1021	208	6.	0.0	54.3			
1	0024	9	0.	0.0	54.0	1	0524	109	0.	0.0	54.0	1	1024	209	6.	0.0	54.3			
1	0027	10	0.	0.0	54.0	1	0527	110	0.	0.0	54.0	1	1027	210	6.	0.0	54.3			
1	0030	11	0.	0.0	54.0	1	0530	111	0.	0.0	54.0	1	1030	211	6.	0.0	54.3			

1	0033	12	0.	0.0	54.0 *	1	0533	112	0.	0.0	54.0 *	1	1033	212	6.	0.0	54.3
1	0036	13	0.	0.0	54.0 *	1	0536	113	0.	0.0	54.0 *	1	1036	213	6.	0.0	54.3
1	0039	14	0.	0.0	54.0 *	1	0539	114	1.	0.0	54.0 *	1	1039	214	5.	0.0	54.3
1	0042	15	0.	0.0	54.0 *	1	0542	115	3.	0.0	54.1 *	1	1042	215	5.	0.0	54.3
1	0045	16	0.	0.0	54.0 *	1	0545	116	7.	0.0	54.4 *	1	1045	216	5.	0.0	54.3
1	0048	17	0.	0.0	54.0 *	1	0548	117	15.	0.0	54.8 *	1	1048	217	5.	0.0	54.3
1	0051	18	0.	0.0	54.0 *	1	0551	118	28.	0.1	55.5 *	1	1051	218	5.	0.0	54.3
1	0054	19	0.	0.0	54.0 *	1	0554	119	38.	0.1	56.2 *	1	1054	219	5.	0.0	54.3
1	0057	20	0.	0.0	54.0 *	1	0557	120	40.	0.2	56.9 *	1	1057	220	5.	0.0	54.3
1	0100	21	0.	0.0	54.0 *	1	0600	121	43.	0.5	58.1 *	1	1100	221	5.	0.0	54.3
1	0103	22	0.	0.0	54.0 *	1	0603	122	46.	0.8	58.9 *	1	1103	222	5.	0.0	54.3
1	0106	23	0.	0.0	54.0 *	1	0606	123	50.	1.2	59.9 *	1	1106	223	5.	0.0	54.3
1	0109	24	0.	0.0	54.0 *	1	0609	124	53.	1.6	60.6 *	1	1109	224	5.	0.0	54.3
1	0112	25	0.	0.0	54.0 *	1	0612	125	56.	2.1	61.3 *	1	1112	225	5.	0.0	54.3
1	0115	26	0.	0.0	54.0 *	1	0615	126	59.	2.5	61.9 *	1	1115	226	5.	0.0	54.3
1	0118	27	0.	0.0	54.0 *	1	0618	127	61.	2.9	62.3 *	1	1118	227	5.	0.0	54.3
1	0121	28	0.	0.0	54.0 *	1	0621	128	62.	3.2	62.6 *	1	1121	228	5.	0.0	54.3
1	0124	29	0.	0.0	54.0 *	1	0624	129	63.	3.4	62.8 *	1	1124	229	5.	0.0	54.3
1	0127	30	0.	0.0	54.0 *	1	0627	130	64.	3.6	63.0 *	1	1127	230	5.	0.0	54.3
1	0130	31	0.	0.0	54.0 *	1	0630	131	64.	3.7	63.1 *	1	1130	231	5.	0.0	54.3
1	0133	32	0.	0.0	54.0 *	1	0633	132	65.	3.8	63.2 *	1	1133	232	5.	0.0	54.3
1	0136	33	0.	0.0	54.0 *	1	0636	133	65.	3.8	63.2 *	1	1136	233	5.	0.0	54.3
1	0139	34	0.	0.0	54.0 *	1	0639	134	65.	3.8	63.2 *	1	1139	234	5.	0.0	54.3
1	0142	35	0.	0.0	54.0 *	1	0642	135	65.	3.8	63.2 *	1	1142	235	5.	0.0	54.3
1	0145	36	0.	0.0	54.0 *	1	0645	136	65.	3.7	63.1 *	1	1145	236	5.	0.0	54.3
1	0148	37	0.	0.0	54.0 *	1	0648	137	64.	3.7	63.0 *	1	1148	237	5.	0.0	54.3
1	0151	38	0.	0.0	54.0 *	1	0651	138	64.	3.6	63.0 *	1	1151	238	5.	0.0	54.3
1	0154	39	0.	0.0	54.0 *	1	0654	139	63.	3.5	62.8 *	1	1154	239	5.	0.0	54.3
1	0157	40	0.	0.0	54.0 *	1	0657	140	63.	3.3	62.7 *	1	1157	240	5.	0.0	54.3
1	0200	41	0.	0.0	54.0 *	1	0700	141	62.	3.2	62.6 *	1	1200	241	5.	0.0	54.3
1	0203	42	0.	0.0	54.0 *	1	0703	142	62.	3.1	62.5 *	1	1203	242	5.	0.0	54.3
1	0206	43	0.	0.0	54.0 *	1	0706	143	61.	2.9	62.3 *	1	1206	243	5.	0.0	54.3
1	0209	44	0.	0.0	54.0 *	1	0709	144	61.	2.8	62.2 *	1	1209	244	5.	0.0	54.3
1	0212	45	0.	0.0	54.0 *	1	0712	145	60.	2.6	62.0 *	1	1212	245	5.	0.0	54.3
1	0215	46	0.	0.0	54.0 *	1	0715	146	59.	2.5	61.8 *	1	1215	246	5.	0.0	54.3
1	0218	47	0.	0.0	54.0 *	1	0718	147	58.	2.3	61.6 *	1	1218	247	5.	0.0	54.3
1	0221	48	0.	0.0	54.0 *	1	0721	148	57.	2.1	61.3 *	1	1221	248	5.	0.0	54.3
1	0224	49	0.	0.0	54.0 *	1	0724	149	56.	2.0	61.1 *	1	1224	249	5.	0.0	54.3
1	0227	50	0.	0.0	54.0 *	1	0727	150	54.	1.8	60.9 *	1	1227	250	5.	0.0	54.3
1	0230	51	0.	0.0	54.0 *	1	0730	151	53.	1.7	60.7 *	1	1230	251	5.	0.0	54.3
1	0233	52	0.	0.0	54.0 *	1	0733	152	52.	1.5	60.4 *	1	1233	252	5.	0.0	54.3
1	0236	53	0.	0.0	54.0 *	1	0736	153	51.	1.4	60.2 *	1	1236	253	5.	0.0	54.3
1	0239	54	0.	0.0	54.0 *	1	0739	154	50.	1.2	60.0 *	1	1239	254	5.	0.0	54.3
1	0242	55	0.	0.0	54.0 *	1	0742	155	49.	1.1	59.6 *	1	1242	255	5.	0.0	54.3
1	0245	56	0.	0.0	54.0 *	1	0745	156	47.	0.9	59.3 *	1	1245	256	5.	0.0	54.3
1	0248	57	0.	0.0	54.0 *	1	0748	157	46.	0.8	58.9 *	1	1248	257	5.	0.0	54.3
1	0251	58	0.	0.0	54.0 *	1	0751	158	45.	0.7	58.6 *	1	1251	258	5.	0.0	54.3
1	0254	59	0.	0.0	54.0 *	1	0754	159	44.	0.5	58.3 *	1	1254	259	5.	0.0	54.3
1	0257	60	0.	0.0	54.0 *	1	0757	160	43.	0.4	57.9 *	1	1257	260	5.	0.0	54.3
1	0300	61	0.	0.0	54.0 *	1	0800	161	41.	0.3	57.2 *	1	1300	261	5.	0.0	54.3
1	0303	62	0.	0.0	54.0 *	1	0803	162	39.	0.2	56.6 *	1	1303	262	5.	0.0	54.3
1	0306	63	0.	0.0	54.0 *	1	0806	163	37.	0.1	56.1 *	1	1306	263	5.	0.0	54.3
1	0309	64	0.	0.0	54.0 *	1	0809	164	16.	0.0	54.9 *	1	1309	264	5.	0.0	54.3
1	0312	65	0.	0.0	54.0 *	1	0812	165	13.	0.0	54.7 *	1	1312	265	5.	0.0	54.3
1	0315	66	0.	0.0	54.0 *	1	0815	166	12.	0.0	54.6 *	1	1315	266	5.	0.0	54.3
1	0318	67	0.	0.0	54.0 *	1	0818	167	11.	0.0	54.6 *	1	1318	267	5.	0.0	54.3
1	0321	68	0.	0.0	54.0 *	1	0821	168	11.	0.0	54.6 *	1	1321	268	5.	0.0	54.3
1	0324	69	0.	0.0	54.0 *	1	0824	169	10.	0.0	54.5 *	1	1324	269	5.	0.0	54.3
1	0327	70	0.	0.0	54.0 *	1	0827	170	10.	0.0	54.5 *	1	1327	270	5.	0.0	54.3
1	0330	71	0.	0.0	54.0 *	1	0830	171	9.	0.0	54.5 *	1	1330	271	5.	0.0	54.3
1	0333	72	0.	0.0	54.0 *	1	0833	172	9.	0.0	54.5 *	1	1333	272	5.	0.0	54.3
1	0336	73	0.	0.0	54.0 *	1	0836	173	8.	0.0	54.4 *	1	1336	273	5.	0.0	54.3
1	0339	74	0.	0.0	54.0 *	1	0839	174	8.	0.0	54.4 *	1	1339	274	5.	0.0	54.3
1	0342	75	0.	0.0	54.0 *	1	0842	175	8.	0.0	54.4 *	1	1342	275	5.	0.0	54.3
1	0345	76	0.	0.0	54.0 *	1	0845	176	7.	0.0	54.4 *	1	1345	276	5.	0.0	54.2
1	0348	77	0.	0.0	54.0 *	1	0848	177	7.	0.0	54.4 *	1	1348	277	5.	0.0	54.2
1	0351	78	0.	0.0	54.0 *	1	0851	178	7.	0.0	54.4 *	1	1351	278	5.	0.0	54.2
1	0354	79	0.	0.0	54.0 *	1	0854	179	7.	0.0	54.4 *	1	1354	279	5.	0.0	54.2
1	0357	80	0.	0.0	54.0 *	1	0857	180	7.	0.0	54.4 *	1	1357	280	5.	0.0	54.2
1	0400	81	0.	0.0	54.0 *	1	0900	181	7.	0.0	54.4 *	1	1400	281	5.	0.0	54.2
1	0403	82	0.	0.0	54.0 *	1	0903	182	7.	0.0	54.4 *	1	1403	282	5.	0.0	54.2
1	0406	83	0.	0.0	54.0 *	1	0906	183	7.	0.0	54.4 *	1	1406	283	5.	0.0	54.2
1	0409	84	0.	0.0	54.0 *	1	0909	184	7.	0.0	54.4 *	1	1409	284	5.	0.0	54.2
1	0412	85	0.	0.0	54.0 *	1	0912	185	7.	0.0	54.4 *	1	1412	285	4.	0.0	54.2
1	0415	86	0.	0.0	54.0 *	1	0915	186	7.	0.0	54.4 *	1	1415	286	4.	0.0	54.2
1	0418	87	0.	0.0	54.0 *	1	0918	187	7.	0.0	54.4 *	1	1418	287	4.	0.0	54.2
1	0421	88	0.	0.0	54.0 *	1	0921	188	7.	0.0	54.4 *	1	1421	288	4.	0.0	54.2

1	0424	89	0.	0.0	54.0	*	1	0924	189	7.	0.0	54.4	*	1	1424	289	4.	0.0	54.2
1	0427	90	0.	0.0	54.0	*	1	0927	190	7.	0.0	54.4	*	1	1427	290	4.	0.0	54.2
1	0430	91	0.	0.0	54.0	*	1	0930	191	7.	0.0	54.4	*	1	1430	291	4.	0.0	54.2
1	0433	92	0.	0.0	54.0	*	1	0933	192	7.	0.0	54.4	*	1	1433	292	4.	0.0	54.2
1	0436	93	0.	0.0	54.0	*	1	0936	193	7.	0.0	54.4	*	1	1436	293	4.	0.0	54.2
1	0439	94	0.	0.0	54.0	*	1	0939	194	7.	0.0	54.4	*	1	1439	294	4.	0.0	54.2
1	0442	95	0.	0.0	54.0	*	1	0942	195	7.	0.0	54.4	*	1	1442	295	4.	0.0	54.2
1	0445	96	0.	0.0	54.0	*	1	0945	196	7.	0.0	54.4	*	1	1445	296	4.	0.0	54.2
1	0448	97	0.	0.0	54.0	*	1	0948	197	7.	0.0	54.4	*	1	1448	297	4.	0.0	54.2
1	0451	98	0.	0.0	54.0	*	1	0951	198	7.	0.0	54.4	*	1	1451	298	4.	0.0	54.2
1	0454	99	0.	0.0	54.0	*	1	0954	199	7.	0.0	54.4	*	1	1454	299	4.	0.0	54.2
1	0457	100	0.	0.0	54.0	*	1	0957	200	7.	0.0	54.4	*	1	1457	300	4.	0.0	54.2

```

*****
PEAK FLOW      TIME      MAXIMUM AVERAGE FLOW
(CFS)          (HR)          6-HR      24-HR      72-HR      14.95-HR
      65.      6.60      (CFS)      25.      11.      11.      11.
                        (INCHES) 1.385    1.531    1.531    1.531
                        (AC-FT)  12.      14.      14.      14.

PEAK STORAGE   TIME      MAXIMUM AVERAGE STORAGE
(AC-FT)        (HR)          6-HR      24-HR      72-HR      14.95-HR
      4.      6.60          1.      0.      0.      0.

PEAK STAGE     TIME      MAXIMUM AVERAGE STAGE
(FEET)         (HR)          6-HR      24-HR      72-HR      14.95-HR
      63.22    6.60      56.80    55.18    55.18    55.18

CUMULATIVE AREA = 0.17 SQ MI

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119 KK

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*****
*      *
* RR-DFB *
*      *
*****

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125 KO

OUTPUT CONTROL VARIABLES

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IPRNT      1  PRINT CONTROL
IPLOT      1  PLOT CONTROL
QSCAL      0.  HYDROGRAPH PLOT SCALE

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HYDROGRAPH ROUTING DATA

126 RS

STORAGE ROUTING

```

NSTPS      1  NUMBER OF SUBREACHES
ITYP      STOR TYPE OF INITIAL CONDITION
RSVRIC     0.00 INITIAL CONDITION
X          0.00 WORKING R AND D COEFFICIENT

```

127 SV	STORAGE	0.0	0.0	0.2	0.7	1.4	3.2
128 SE	ELEVATION	19.00	20.00	22.00	24.00	26.00	28.00
129 SQ	DISCHARGE	0.	37.	43.	50.	60.	68.

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 0. TO 37.
THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

HYDROGRAPH AT STATION RR-DFB

G.

HEC-1 MODEL OUTPUT

FULLY DEVELOPED CONDITION

SUMMARY SHEETS FOR 2, 5, 10, 25 AND 50-YEAR STORMS

2 Year, 24 Hour, Developed Condition

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6 HOUR	24 HOUR	72 HOUR			
HYDROGRAPH AT	SB-D1	6.	6.35	1.	1.	1.	0.09		
ROUTED TO	RT-SBD1	6.	6.45	1.	1.	1.	0.09		
HYDROGRAPH AT	SB-D2	8.	6.15	1.	1.	1.	0.06		
2 COMBINED AT	AP-D1	11.	6.25	2.	1.	1.	0.14		
HYDROGRAPH AT	SB-D3	7.	6.20	1.	0.	0.	0.03		
2 COMBINED AT	AP-DDA	17.	6.20	3.	2.	2.	0.17		
ROUTED TO	RR-DFA	17.	6.25	3.	2.	2.	0.17	54.92	6.25
ROUTED TO	RT-APDFA	17.	6.25	3.	2.	2.	0.17		
HYDROGRAPH AT	SB-D6	19.	6.10	2.	1.	1.	0.04		
2 COMBINED AT	AP-D2	30.	6.15	5.	2.	2.	0.21		
ROUTED TO	RT-APD2	29.	6.15	5.	2.	2.	0.21		
HYDROGRAPH AT	SB-D7	36.	6.10	4.	2.	2.	0.07		
HYDROGRAPH AT	SB-D4	5.	6.20	1.	0.	0.	0.05		
HYDROGRAPH AT	SB-D5	8.	6.15	1.	0.	0.	0.03		
2 COMBINED AT	AP-DFB	13.	6.15	2.	1.	1.	0.08		
ROUTED TO	RR-DFB	13.	6.20	2.	1.	1.	0.08	19.35	6.20
ROUTED TO	RT-APDFB	13.	6.20	2.	1.	1.	0.08		
3 COMBINED AT	AP-D3	73.	6.10	11.	5.	5.	0.36		
ROUTED TO	RT-APD3	73.	6.15	11.	5.	5.	0.36		
HYDROGRAPH AT	SB-D8	79.	6.00	8.	3.	3.	0.06		
2 COMBINED AT	AP-DFC	139.	6.05	19.	8.	8.	0.42		
ROUTED TO	RR-DFC	45.	6.40	18.	8.	8.	0.42	55.96	6.40

5 Year, 24 Hour, Developed Condition

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-D1	17.	6.30	3.	1.	1.	0.09		
ROUTED TO	RT-SBD1	17.	6.35	3.	1.	1.	0.09		
HYDROGRAPH AT	SB-D2	20.	6.15	2.	1.	1.	0.06		
2 COMBINED AT	AP-D1	31.	6.25	6.	3.	3.	0.14		
HYDROGRAPH AT	SB-D3	13.	6.15	2.	1.	1.	0.03		
2 COMBINED AT	AP-DDA	44.	6.20	7.	3.	3.	0.17		
ROUTED TO	RR-DFA	38.	6.35	7.	3.	3.	0.17	56.38	6.35
ROUTED TO	RT-APDFA	38.	6.35	7.	3.	3.	0.17		
HYDROGRAPH AT	SB-D6	34.	6.05	3.	2.	2.	0.04		
2 COMBINED AT	AP-D2	64.	6.15	11.	5.	5.	0.21		
ROUTED TO	RT-APD2	64.	6.15	11.	5.	5.	0.21		
HYDROGRAPH AT	SB-D7	62.	6.05	7.	3.	3.	0.07		
HYDROGRAPH AT	SB-D4	14.	6.15	2.	1.	1.	0.05		
HYDROGRAPH AT	SB-D5	16.	6.15	2.	1.	1.	0.03		
2 COMBINED AT	AP-DFB	30.	6.15	4.	2.	2.	0.08		
ROUTED TO	RR-DFB	30.	6.15	4.	2.	2.	0.08	19.82	6.15
ROUTED TO	RT-APDFB	30.	6.15	4.	2.	2.	0.08		
3 COMBINED AT	AP-D3	148.	6.10	21.	10.	10.	0.36		
ROUTED TO	RT-APD3	147.	6.15	21.	10.	10.	0.36		
HYDROGRAPH AT	SB-D8	111.	6.00	11.	5.	5.	0.06		
2 COMBINED AT	AP-DFC	237.	6.05	32.	14.	14.	0.42		
ROUTED TO	RR-DFC	60.	6.60	32.	14.	14.	0.42	58.40	6.60

10 Year, 24 Hour, Developed Condition

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-D1	27.	6.30	5.	2.	2.	0.09		
ROUTED TO	RT-SB01	27.	6.35	5.	2.	2.	0.09		
HYDROGRAPH AT	SB-D2	30.	6.15	4.	2.	2.	0.06		
2 COMBINED AT	AP-D1	48.	6.20	8.	4.	4.	0.14		
HYDROGRAPH AT	SB-D3	19.	6.15	2.	1.	1.	0.03		
2 COMBINED AT	AP-DDA	66.	6.20	11.	5.	5.	0.17		
ROUTED TO	RR-DFA	45.	6.45	11.	5.	5.	0.17	58.43	6.45
ROUTED TO	RT-APDFA	44.	6.45	11.	5.	5.	0.17		
HYDROGRAPH AT	SB-D6	45.	6.05	5.	2.	2.	0.04		
2 COMBINED AT	AP-D2	81.	6.10	15.	7.	7.	0.21		
ROUTED TO	RT-APD2	80.	6.10	15.	7.	7.	0.21		
HYDROGRAPH AT	SB-D7	82.	6.05	9.	4.	4.	0.07		
HYDROGRAPH AT	SB-D4	22.	6.15	3.	1.	1.	0.05		
HYDROGRAPH AT	SB-D5	22.	6.15	3.	1.	1.	0.03		
2 COMBINED AT	AP-DFB	44.	6.15	6.	3.	3.	0.08		
ROUTED TO	RR-DFB	39.	6.25	6.	3.	3.	0.08	20.69	6.25
ROUTED TO	RT-APDFB	39.	6.25	6.	3.	3.	0.08		
3 COMBINED AT	AP-D3	198.	6.10	29.	13.	13.	0.36		
ROUTED TO	RT-APD3	196.	6.10	29.	13.	13.	0.36		
HYDROGRAPH AT	SB-D8	133.	6.00	13.	6.	6.	0.06		
2 COMBINED AT	AP-DFC	311.	6.05	43.	19.	19.	0.42		
ROUTED TO	RR-DFC	66.	6.80	42.	19.	19.	0.42	60.06	6.80

25 Year, 24 Hour, Developed Condition

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-D1	45.	6.30	8.	3.	3.	0.09		
ROUTED TO	RT-SBD1	45.	6.35	8.	3.	3.	0.09		
HYDROGRAPH AT	SB-D2	46.	6.10	6.	2.	2.	0.06		
2 COMBINED AT	AP-D1	79.	6.20	13.	6.	6.	0.14		
HYDROGRAPH AT	SB-D3	27.	6.15	3.	2.	2.	0.03		
2 COMBINED AT	AP-DDA	105.	6.20	16.	7.	7.	0.17		
ROUTED TO	RR-DFA	54.	6.55	16.	7.	7.	0.17	60.75	6.55
ROUTED TO	RT-APDFA	54.	6.55	16.	7.	7.	0.17		
HYDROGRAPH AT	SB-D6	62.	6.05	6.	3.	3.	0.04		
2 COMBINED AT	AP-D2	101.	6.10	23.	10.	10.	0.21		
ROUTED TO	RT-APD2	101.	6.10	23.	10.	10.	0.21		
HYDROGRAPH AT	SB-D7	113.	6.05	12.	5.	5.	0.07		
HYDROGRAPH AT	SB-D4	36.	6.15	4.	2.	2.	0.05		
HYDROGRAPH AT	SB-D5	32.	6.15	4.	2.	2.	0.03		
2 COMBINED AT	AP-DFB	68.	6.15	8.	4.	4.	0.08		
ROUTED TO	RR-DFB	47.	6.30	8.	4.	4.	0.08	23.03	6.30
ROUTED TO	RT-APDFB	47.	6.30	8.	4.	4.	0.08		
3 COMBINED AT	AP-D3	254.	6.10	43.	19.	19.	0.36		
ROUTED TO	RT-APD3	253.	6.10	43.	19.	19.	0.36		
HYDROGRAPH AT	SB-D8	165.	6.00	17.	7.	7.	0.06		
2 COMBINED AT	AP-DFC	405.	6.05	60.	26.	26.	0.42		
ROUTED TO	RR-DFC	75.	7.05	58.	26.	26.	0.42	62.40	7.10

50 Year, 24 Hour, Developed Condition

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-D1	58.	6.25	10.	4.	4.	0.09		
ROUTED TO	RT-SBD1	58.	6.30	10.	4.	4.	0.09		
HYDROGRAPH AT	SB-D2	59.	6.10	7.	3.	3.	0.06		
2 COMBINED AT	AP-D1	102.	6.20	16.	7.	7.	0.14		
HYDROGRAPH AT	SB-D3	33.	6.15	4.	2.	2.	0.03		
2 COMBINED AT	AP-DDA	134.	6.20	21.	9.	9.	0.17		
ROUTED TO	RR-DFA	60.	6.60	21.	9.	9.	0.17	62.11	6.60
ROUTED TO	RT-APDFA	60.	6.60	21.	9.	9.	0.17		
HYDROGRAPH AT	SB-D6	74.	6.05	8.	3.	3.	0.04		
2 COMBINED AT	AP-D2	116.	6.05	28.	12.	12.	0.21		
ROUTED TO	RT-APD2	116.	6.10	28.	12.	12.	0.21		
HYDROGRAPH AT	SB-D7	134.	6.05	14.	6.	6.	0.07		
HYDROGRAPH AT	SB-D4	45.	6.15	6.	2.	2.	0.05		
HYDROGRAPH AT	SB-D5	40.	6.10	5.	2.	2.	0.03		
2 COMBINED AT	AP-DFB	85.	6.15	10.	5.	5.	0.08		
ROUTED TO	RR-DFB	52.	6.30	10.	5.	5.	0.08	24.31	6.30
ROUTED TO	RT-APDFB	52.	6.35	10.	5.	5.	0.08		
3 COMBINED AT	AP-D3	291.	6.10	53.	23.	23.	0.36		
ROUTED TO	RT-APD3	291.	6.10	53.	23.	23.	0.36		
HYDROGRAPH AT	SB-D8	186.	6.00	19.	8.	8.	0.06		
2 COMBINED AT	AP-DFC	465.	6.05	72.	32.	32.	0.42		
ROUTED TO	RR-DFC	81.	7.20	67.	31.	31.	0.42	64.02	7.20

H.
HEC-1 MODEL OUTPUT
HISTORIC (UNDEVELOPED) CONDITION
• 100-YEAR STORM

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* MAY 1991
* VERSION 4.0.1E
*
* RUN DATE 05/06/1999 TIME 19:40:18
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*****
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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
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X   X  XXXXXXX  XXXXX      X
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::::::::::::::::::::::::::::::::::::
:::
::: Full Microcomputer Implementation :::
::: by :::
::: Haestad Methods, Inc. :::
:::
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37 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1         ID   KETTLE CREEK DRAINAGE BASIN - 24HR, HISTORIC CONDITION (TYPE IIa100 YEAR)
2         ID   WATERSHED IN PREDEVELOPMENT CONDITION WITH NO CONSERVATION TREATMENT
3         ID   FILE:KCU100.DAT
4         ID   UNDEVELOPED CONDITION MODEL
5         ID   *****
6         ID   BEGIN CALCULATIONS IN THE SOUTH TRIBUTARY WATERSHED
7         ID   *****

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*** FREE ***

*DIAGRAM

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8         IT      3      0      0      300
9         IO      5

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10        KK   SB-H1
11        KM   COMPUTE HYDROGRAPH FOR BASIN H1
12        BA   .1347
13        IN   15
14        PB   4.4    4.0
15        PC   .0000 .0005 .0015 .0030 .0045 .0060 .0080 .0100 .0120 .0143
16        PC   .0165 .0188 .0210 .0233 .0255 .0278 .0320 .0390 .0460 .0530
17        PC   .0600 .0750 .1000 .4000 .7000 .7250 .7500 .7650 .7800 .7900
18        PC   .8000 .8100 .8200 .8250 .8300 .8350 .8400 .8450 .8500 .8550
19        PC   .8600 .8638 .8675 .8713 .8750 .8788 .8825 .8863 .8900 .8938
20        PC   .8975 .9013 .9050 .9083 .9115 .9148 .9180 .9210 .9240 .9270
21        PC   .9300 .9325 .9350 .9375 .9400 .9425 .9450 .9475 .9500 .9525
22        PC   .9550 .9575 .9600 .9625 .9650 .9675 .9700 .9725 .9750 .9775
23        PC   .9800 .9813 .9825 .9838 .9850 .9863 .9875 .9888 .9900 .9913
24        PC   .9925 .9938 .9950 .9963 .9975 .9988 1.000
25        LS   0      66
26        UD   .420

27        KK   RT-SBH1
28        KM   ROUTE THE RUNOFF FROM SB-H1 THROUGH SB-H2 TO AP-H1
29        RD   3500 .027 .035          TRAP   15    10

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30        KK   SB-H2
31        KM   COMPUTE HYDROGRAPH FOR BASIN H2
32        BA   0.1288
33        LS   0      66
34        UD   .303

35        KK   AP-H1
36        KM   COMBINE THE FLOW FROM BASIN H2 TO THE ROUTED FLOW FROM BASIN H1 AT AP-H1
37        HC   2

38        KK   RT-APH1
39        KM   ROUTE THE RUNOFF FROM AP1 TO AP-H2
40        RD   590 .015 .035          TRAP   30    5

41        KK   SB-H3
42        KM   COMPUTE HYDROGRAPH FOR BASIN H3
43        BA   0.0853
44        LS   0      66
45        UD   .314

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SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT

LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW

NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

10 SB-H1

V

V

27 RT-SBH1

.

30 . SB-H2

.

.

35 AP-H1.....

V

V

38 RT-APH1

.

.

41 . SB-H3

.

V

.

V

46 . RT-H3

.

.

49 AP-H2.....

V

V

52 RT-AP2

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55 . SB-H4

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60 . SB-H5

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65 AP-H3.....

V

V

68 RT-APH3

.

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71 . SB-H6

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76 AP-H4.....

V

V

79 RT-APH4

V

V

82 RT-APH5

V

V

85 RT-APH6

.

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-H1	99.	6.30	17.	8.	8.	0.13		
ROUTED TO	RT-SBH1	98.	6.45	17.	8.	8.	0.13		
HYDROGRAPH AT	SB-H2	118.	6.20	16.	7.	7.	0.13		
2 COMBINED AT	AP-H1	179.	6.30	33.	15.	15.	0.26		
ROUTED TO	RT-APH1	178.	6.35	33.	15.	15.	0.26		
HYDROGRAPH AT	SB-H3	76.	6.20	11.	5.	5.	0.09		
ROUTED TO	RT-H3	75.	6.25	11.	5.	5.	0.09		
2 COMBINED AT	AP-H2	247.	6.30	44.	20.	20.	0.35		
ROUTED TO	RT-AP2	245.	6.35	44.	20.	20.	0.35		
HYDROGRAPH AT	SB-H4	51.	6.10	5.	2.	2.	0.04		
HYDROGRAPH AT	SB-H5	72.	6.20	10.	4.	4.	0.08		
3 COMBINED AT	AP-H3	317.	6.30	59.	26.	26.	0.47		
ROUTED TO	RT-APH3	314.	6.35	59.	26.	26.	0.47		
HYDROGRAPH AT	SB-H6	62.	6.30	10.	5.	5.	0.08		
2 COMBINED AT	AP-H4	376.	6.30	69.	31.	31.	0.55		
ROUTED TO	RT-APH4	376.	6.35	69.	31.	31.	0.55		
ROUTED TO	RT-APH5	376.	6.40	69.	31.	31.	0.55		
ROUTED TO	RT-APH6	374.	6.40	69.	31.	31.	0.55		
HYDROGRAPH AT	SB-H7	48.	6.25	8.	3.	3.	0.06		
ROUTED TO	RT-SBH7	48.	6.45	8.	3.	3.	0.06		
HYDROGRAPH AT	SB-H8	67.	6.25	10.	5.	5.	0.08		
2 COMBINED AT	AP-H7	103.	6.35	18.	8.	8.	0.14		
ROUTED TO	RT-APH7	102.	6.40	18.	8.	8.	0.14		
2 COMBINED AT	AP-H8	477.	6.40	87.	39.	39.	0.69		
ROUTED TO	RT-APH8	474.	6.40	87.	39.	39.	0.69		
HYDROGRAPH AT	SB-H9	130.	6.20	18.	8.	8.	0.14		

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING

(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

INTERPOLATED TO
COMPUTATION INTERVAL

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	DT	PEAK	TIME TO PEAK	VOLUME
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		(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CFS)	(MIN)	(IN)
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RT-SBH1	MANE	2.25	98.35	387.00	1.30	3.00	98.35	387.00	1.30
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.9429E+01 EXCESS=0.0000E+00 OUTFLOW=0.9338E+01 BASIN STORAGE=0.1320E+00 PERCENT ERROR= -0.4

RT-APH1	MANE	1.79	178.47	381.10	1.31	3.00	178.46	381.00	1.31
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1840E+02 EXCESS=0.0000E+00 OUTFLOW=0.1836E+02 BASIN STORAGE=0.5273E-01 PERCENT ERROR= -0.1

RT-H3	MANE	1.95	75.64	374.40	1.32	3.00	75.26	375.00	1.31
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5996E+01 EXCESS=0.0000E+00 OUTFLOW=0.5985E+01 BASIN STORAGE=0.1681E-01 PERCENT ERROR= -0.1

RT-AP2	MANE	2.40	245.53	381.60	1.30	3.00	244.54	381.00	1.30
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2434E+02 EXCESS=0.0000E+00 OUTFLOW=0.2423E+02 BASIN STORAGE=0.1467E+00 PERCENT ERROR= -0.1

RT-APH3	MANE	1.00	316.88	379.09	1.31	3.00	314.46	381.00	1.31
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3266E+02 EXCESS=0.0000E+00 OUTFLOW=0.3263E+02 BASIN STORAGE=0.5039E-01 PERCENT ERROR= 0.0

RT-APH4	MANE	1.51	376.34	381.05	1.30	3.00	376.31	381.00	1.30
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3833E+02 EXCESS=0.0000E+00 OUTFLOW=0.3827E+02 BASIN STORAGE=0.8959E-01 PERCENT ERROR= -0.1

RT-APH5	MANE	3.00	376.45	384.00	1.30	3.00	376.45	384.00	1.30
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3830E+02 EXCESS=0.0000E+00 OUTFLOW=0.3818E+02 BASIN STORAGE=0.1874E+00 PERCENT ERROR= -0.2

RT-APH6	MANE	1.09	375.92	385.14	1.30	3.00	374.36	384.00	1.30
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CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3815E+02 EXCESS=0.0000E+00 OUTFLOW=0.3811E+02 BASIN STORAGE=0.5767E-01 PERCENT ERROR= 0.0

RT-SBH7	MANE	2.10	48.07	386.40	1.30	3.00	47.88	387.00	1.30
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HEC-1 MODEL OUTPUT
HISTORIC (UNDEVELOPED) CONDITION
SUMMARY SHEETS FOR 2, 5, 10, 25, AND 50 YEAR STORMS

2 YEAR 24 HOUR STORM

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-H1	7.	6.40	2.	1.	1.	0.13		
ROUTED TO	RT-SBH1	7.	6.65	2.	1.	1.	0.13		
HYDROGRAPH AT	SB-H2	8.	6.25	2.	1.	1.	0.13		
2 COMBINED AT	AP-H1	11.	6.65	3.	2.	2.	0.26		
ROUTED TO	RT-APH1	11.	6.75	3.	2.	2.	0.26		
HYDROGRAPH AT	SB-H3	5.	6.30	1.	1.	1.	0.09		
ROUTED TO	RT-H3	5.	6.35	1.	1.	1.	0.09		
2 COMBINED AT	AP-H2	14.	6.75	5.	2.	2.	0.35		
ROUTED TO	RT-AP2	14.	6.85	5.	2.	2.	0.35		
HYDROGRAPH AT	SB-H4	4.	6.15	1.	0.	0.	0.04		
HYDROGRAPH AT	SB-H5	5.	6.25	1.	1.	1.	0.08		
3 COMBINED AT	AP-H3	18.	6.45	6.	3.	3.	0.47		
ROUTED TO	RT-APH3	18.	6.50	6.	3.	3.	0.47		
HYDROGRAPH AT	SB-H6	4.	6.40	1.	1.	1.	0.08		
2 COMBINED AT	AP-H4	22.	6.50	7.	3.	3.	0.55		
ROUTED TO	RT-APH4	22.	6.50	7.	3.	3.	0.55		
ROUTED TO	RT-APH5	23.	6.65	7.	3.	3.	0.55		
ROUTED TO	RT-APH6	22.	6.70	7.	3.	3.	0.55		
HYDROGRAPH AT	SB-H7	3.	6.35	1.	0.	0.	0.06		
ROUTED TO	RT-SBH7	4.	6.65	1.	0.	0.	0.06		
HYDROGRAPH AT	SB-H8	5.	6.30	1.	1.	1.	0.08		
2 COMBINED AT	AP-H7	7.	6.65	2.	1.	1.	0.14		
ROUTED TO	RT-APH7	6.	6.75	2.	1.	1.	0.14		
2 COMBINED AT	AP-H8	27.	6.75	9.	4.	4.	0.69		
ROUTED TO	RT-APH8	27.	6.75	9.	4.	4.	0.69		
HYDROGRAPH AT	SB-H9	9.	6.25	2.	1.	1.	0.14		

5 YEAR 24 HOUR STORM

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-H1	22.	6.35	5.	2.	2.	0.13		
ROUTED TO	RT-SBH1	22.	6.55	4.	2.	2.	0.13		
HYDROGRAPH AT	SB-H2	26.	6.25	4.	2.	2.	0.13		
2 COMBINED AT	AP-H1	36.	6.40	9.	4.	4.	0.26		
ROUTED TO	RT-APH1	35.	6.50	9.	4.	4.	0.26		
HYDROGRAPH AT	SB-H3	17.	6.25	3.	1.	1.	0.09		
ROUTED TO	RT-H3	17.	6.30	3.	1.	1.	0.09		
2 COMBINED AT	AP-H2	48.	6.45	12.	5.	5.	0.35		
ROUTED TO	RT-AP2	47.	6.60	12.	5.	5.	0.35		
HYDROGRAPH AT	SB-H4	12.	6.10	1.	1.	1.	0.04		
HYDROGRAPH AT	SB-H5	16.	6.20	3.	1.	1.	0.08		
3 COMBINED AT	AP-H3	60.	6.35	15.	7.	7.	0.47		
ROUTED TO	RT-APH3	59.	6.35	15.	7.	7.	0.47		
HYDROGRAPH AT	SB-H6	14.	6.35	3.	1.	1.	0.08		
2 COMBINED AT	AP-H4	73.	6.35	18.	8.	8.	0.55		
ROUTED TO	RT-APH4	73.	6.40	18.	8.	8.	0.55		
ROUTED TO	RT-APH5	72.	6.50	18.	8.	8.	0.55		
ROUTED TO	RT-APH6	73.	6.50	18.	8.	8.	0.55		
HYDROGRAPH AT	SB-H7	11.	6.30	2.	1.	1.	0.06		
ROUTED TO	RT-SBH7	11.	6.55	2.	1.	1.	0.06		
HYDROGRAPH AT	SB-H8	15.	6.30	3.	1.	1.	0.08		
2 COMBINED AT	AP-H7	23.	6.45	5.	2.	2.	0.14		
ROUTED TO	RT-APH7	22.	6.55	5.	2.	2.	0.14		
2 COMBINED AT	AP-H8	94.	6.55	23.	10.	10.	0.69		
ROUTED TO	RT-APH8	93.	6.55	23.	10.	10.	0.69		
HYDROGRAPH AT	SB-H9	29.	6.25	5.	2.	2.	0.14		

10 YEAR 24 HOUR STORM

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND

TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-H1	35.	6.35	7.	3.	3.	0.13		
ROUTED TO	RT-SBH1	35.	6.55	7.	3.	3.	0.13		
HYDROGRAPH AT	SB-H2	43.	6.20	7.	3.	3.	0.13		
2 COMBINED AT	AP-H1	60.	6.40	13.	6.	6.	0.26		
ROUTED TO	RT-APH1	60.	6.45	13.	6.	6.	0.26		
HYDROGRAPH AT	SB-H3	27.	6.25	4.	2.	2.	0.09		
ROUTED TO	RT-H3	27.	6.25	4.	2.	2.	0.09		
2 COMBINED AT	AP-H2	81.	6.40	18.	8.	8.	0.35		
ROUTED TO	RT-AP2	81.	6.50	18.	8.	8.	0.35		
HYDROGRAPH AT	SB-H4	19.	6.10	2.	1.	1.	0.04		
HYDROGRAPH AT	SB-H5	27.	6.20	4.	2.	2.	0.08		
3 COMBINED AT	AP-H3	99.	6.30	23.	11.	11.	0.47		
ROUTED TO	RT-APH3	99.	6.35	23.	11.	11.	0.47		
HYDROGRAPH AT	SB-H6	22.	6.30	4.	2.	2.	0.08		
2 COMBINED AT	AP-H4	121.	6.35	28.	13.	13.	0.55		
ROUTED TO	RT-APH4	121.	6.35	28.	13.	13.	0.55		
ROUTED TO	RT-APH5	121.	6.45	28.	12.	12.	0.55		
ROUTED TO	RT-APH6	120.	6.50	27.	12.	12.	0.55		
HYDROGRAPH AT	SB-H7	17.	6.30	3.	1.	1.	0.06		
ROUTED TO	RT-SBH7	17.	6.55	3.	1.	1.	0.06		
HYDROGRAPH AT	SB-H8	24.	6.25	4.	2.	2.	0.08		
2 COMBINED AT	AP-H7	35.	6.40	7.	3.	3.	0.14		
ROUTED TO	RT-APH7	35.	6.50	7.	3.	3.	0.14		
2 COMBINED AT	AP-H8	155.	6.50	34.	16.	16.	0.69		
ROUTED TO	RT-APH8	154.	6.50	34.	16.	16.	0.69		
HYDROGRAPH AT	SB-H9	47.	6.20	7.	3.	3.	0.14		

25 YEAR 24 HOUR STORM

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-H1	59.	6.35	11.	5.	5.	0.13		
ROUTED TO	RT-SBH1	60.	6.50	11.	5.	5.	0.13		
HYDROGRAPH AT	SB-H2	72.	6.20	10.	5.	5.	0.13		
2 COMBINED AT	AP-H1	105.	6.35	21.	10.	10.	0.26		
ROUTED TO	RT-APH1	105.	6.40	21.	10.	10.	0.26		
HYDROGRAPH AT	SB-H3	46.	6.20	7.	3.	3.	0.09		
ROUTED TO	RT-H3	46.	6.25	7.	3.	3.	0.09		
2 COMBINED AT	AP-H2	144.	6.35	28.	13.	13.	0.35		
ROUTED TO	RT-AP2	143.	6.40	28.	13.	13.	0.35		
HYDROGRAPH AT	SB-H4	32.	6.10	3.	2.	2.	0.04		
HYDROGRAPH AT	SB-H5	45.	6.20	6.	3.	3.	0.08		
3 COMBINED AT	AP-H3	181.	6.35	37.	17.	17.	0.47		
ROUTED TO	RT-APH3	180.	6.40	37.	17.	17.	0.47		
HYDROGRAPH AT	SB-H6	38.	6.30	7.	3.	3.	0.08		
2 COMBINED AT	AP-H4	217.	6.35	44.	20.	20.	0.55		
ROUTED TO	RT-APH4	216.	6.40	44.	20.	20.	0.55		
ROUTED TO	RT-APH5	216.	6.45	44.	20.	20.	0.55		
ROUTED TO	RT-APH6	215.	6.45	44.	20.	20.	0.55		
HYDROGRAPH AT	SB-H7	29.	6.25	5.	2.	2.	0.06		
ROUTED TO	RT-SBH7	29.	6.45	5.	2.	2.	0.06		
HYDROGRAPH AT	SB-H8	41.	6.25	6.	3.	3.	0.08		
2 COMBINED AT	AP-H7	60.	6.40	11.	5.	5.	0.14		
ROUTED TO	RT-APH7	60.	6.40	11.	5.	5.	0.14		
2 COMBINED AT	AP-H8	274.	6.45	55.	25.	25.	0.69		
ROUTED TO	RT-APH8	274.	6.45	55.	25.	25.	0.69		
HYDROGRAPH AT	SB-H9	80.	6.20	11.	5.	5.	0.14		

50 YEAR 24 HOUR STORM

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-H1	78.	6.30	14.	6.	6.	0.13		
ROUTED TO	RT-SBH1	78.	6.45	14.	6.	6.	0.13		
HYDROGRAPH AT	SB-H2	94.	6.20	13.	6.	6.	0.13		
2 COMBINED AT	AP-H1	140.	6.35	27.	12.	12.	0.26		
ROUTED TO	RT-APH1	139.	6.35	27.	12.	12.	0.26		
HYDROGRAPH AT	SB-H3	61.	6.20	9.	4.	4.	0.09		
ROUTED TO	RT-H3	60.	6.25	9.	4.	4.	0.09		
2 COMBINED AT	AP-H2	193.	6.30	36.	16.	16.	0.35		
ROUTED TO	RT-AP2	192.	6.40	36.	16.	16.	0.35		
HYDROGRAPH AT	SB-H4	41.	6.10	4.	2.	2.	0.04		
HYDROGRAPH AT	SB-H5	58.	6.20	8.	4.	4.	0.08		
3 COMBINED AT	AP-H3	246.	6.35	48.	22.	22.	0.47		
ROUTED TO	RT-APH3	246.	6.35	48.	21.	21.	0.47		
HYDROGRAPH AT	SB-H6	50.	6.30	8.	4.	4.	0.08		
2 COMBINED AT	AP-H4	294.	6.35	56.	25.	25.	0.55		
ROUTED TO	RT-APH4	292.	6.35	56.	25.	25.	0.55		
ROUTED TO	RT-APH5	291.	6.40	56.	25.	25.	0.55		
ROUTED TO	RT-APH6	291.	6.45	56.	25.	25.	0.55		
HYDROGRAPH AT	SB-H7	38.	6.25	6.	3.	3.	0.06		
ROUTED TO	RT-SBH7	38.	6.45	6.	3.	3.	0.06		
HYDROGRAPH AT	SB-H8	54.	6.25	8.	4.	4.	0.08		
2 COMBINED AT	AP-H7	81.	6.35	14.	6.	6.	0.14		
ROUTED TO	RT-APH7	81.	6.45	14.	6.	6.	0.14		
2 COMBINED AT	AP-H8	371.	6.45	70.	31.	31.	0.69		
ROUTED TO	RT-APH8	370.	6.45	70.	31.	31.	0.69		
HYDROGRAPH AT	SB-H9	104.	6.20	15.	7.	7.	0.14		

I.
HEC-1 MODEL OUTPUT
HISTORIC (UNDEVELOPED) CONDITION
SUMMARY SHEETS FOR 2, 5, 10, 25, AND 50-YEAR STORMS

2 YEAR 24 HOUR STORM

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
6-HOUR	24-HOUR	72-HOUR							
HYDROGRAPH AT	SB-D1	6.	6.35	1.	1.	1.	0.09		
ROUTED TO	RT-SBD1	6.	6.45	1.	1.	1.	0.09		
HYDROGRAPH AT	SB-D2	8.	6.15	1.	1.	1.	0.06		
2 COMBINED AT	AP-D1	11.	6.25	2.	1.	1.	0.14		
HYDROGRAPH AT	SB-D3	7.	6.20	1.	0.	0.	0.03		
2 COMBINED AT	AP-DDA	17.	6.20	3.	2.	2.	0.17		
ROUTED TO	RR-DFA	17.	6.25	3.	2.	2.	0.17	54.92	6.25
ROUTED TO	RT-APDFA	17.	6.25	3.	2.	2.	0.17		
HYDROGRAPH AT	SB-D6	19.	6.10	2.	1.	1.	0.04		
2 COMBINED AT	AP-D2	30.	6.15	5.	2.	2.	0.21		
ROUTED TO	RT-APD2	29.	6.15	5.	2.	2.	0.21		
HYDROGRAPH AT	SB-D7	36.	6.10	4.	2.	2.	0.07		
HYDROGRAPH AT	SB-D4	5.	6.20	1.	0.	0.	0.05		
HYDROGRAPH AT	SB-D5	8.	6.15	1.	0.	0.	0.03		
2 COMBINED AT	AP-DFB	13.	6.15	2.	1.	1.	0.08		
ROUTED TO	RR-DFB	13.	6.20	2.	1.	1.	0.08	19.35	6.20
ROUTED TO	RT-APDFB	13.	6.20	2.	1.	1.	0.08		
3 COMBINED AT	AP-D3	73.	6.10	11.	5.	5.	0.36		
ROUTED TO	RT-APD3	73.	6.15	11.	5.	5.	0.36		
HYDROGRAPH AT	SB-D8	79.	6.00	8.	3.	3.	0.06		
2 COMBINED AT	AP-DFC	139.	6.05	19.	8.	8.	0.42		
ROUTED TO	RR-DFC	45.	6.40	18.	8.	8.	0.42	55.96	6.40
ROUTED TO	RT-RRDFC	45.	6.45	18.	8.	8.	0.42		
HYDROGRAPH AT	SB-D9	41.	6.05	4.	2.	2.	0.06		
2 COMBINED AT	AP-D4	78.	6.10	23.	10.	10.	0.47		
ROUTED TO	RT-APD4	78.	6.10	23.	10.	10.	0.47		

5 YEAR 24 HOUR STORM

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-D1	17.	6.30	3.	1.	1.	0.09		
ROUTED TO	RT-SBD1	17.	6.35	3.	1.	1.	0.09		
HYDROGRAPH AT	SB-D2	20.	6.15	2.	1.	1.	0.06		
2 COMBINED AT	AP-D1	31.	6.25	6.	3.	3.	0.14		
HYDROGRAPH AT	SB-D3	13.	6.15	2.	1.	1.	0.03		
2 COMBINED AT	AP-DDA	44.	6.20	7.	3.	3.	0.17		
ROUTED TO	RR-DFA	38.	6.35	7.	3.	3.	0.17	56.38	6.35
ROUTED TO	RT-APDFA	38.	6.35	7.	3.	3.	0.17		
HYDROGRAPH AT	SB-D6	34.	6.05	3.	2.	2.	0.04		
2 COMBINED AT	AP-D2	64.	6.15	11.	5.	5.	0.21		
ROUTED TO	RT-APD2	64.	6.15	11.	5.	5.	0.21		
HYDROGRAPH AT	SB-D7	62.	6.05	7.	3.	3.	0.07		
HYDROGRAPH AT	SB-D4	14.	6.15	2.	1.	1.	0.05		
HYDROGRAPH AT	SB-D5	16.	6.15	2.	1.	1.	0.03		
2 COMBINED AT	AP-DFB	30.	6.15	4.	2.	2.	0.08		
ROUTED TO	RR-DFB	30.	6.15	4.	2.	2.	0.08	19.82	6.15
ROUTED TO	RT-APDFB	30.	6.15	4.	2.	2.	0.08		
3 COMBINED AT	AP-D3	148.	6.10	21.	10.	10.	0.36		
ROUTED TO	RT-APD3	147.	6.15	21.	10.	10.	0.36		
HYDROGRAPH AT	SB-D8	111.	6.00	11.	5.	5.	0.06		
2 COMBINED AT	AP-DFC	237.	6.05	32.	14.	14.	0.42		
ROUTED TO	RR-DFC	60.	6.60	32.	14.	14.	0.42	58.40	6.60
ROUTED TO	RT-RRDFC	60.	6.60	32.	14.	14.	0.42		
HYDROGRAPH AT	SB-D9	66.	6.05	7.	3.	3.	0.06		
2 COMBINED AT	AP-D4	112.	6.10	39.	17.	17.	0.47		
ROUTED TO	RT-APD4	112.	6.10	39.	17.	17.	0.47		

10YEAR 24 HOUR STORM

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
HYDROGRAPH AT	SB-D1	27.	6.30	5.	2.	2.	0.09		
ROUTED TO	RT-SBD1	27.	6.35	5.	2.	2.	0.09		
HYDROGRAPH AT	SB-D2	30.	6.15	4.	2.	2.	0.06		
2 COMBINED AT	AP-D1	48.	6.20	8.	4.	4.	0.14		
HYDROGRAPH AT	SB-D3	19.	6.15	2.	1.	1.	0.03		
2 COMBINED AT	AP-DDA	66.	6.20	11.	5.	5.	0.17		
ROUTED TO	RR-DFA	45.	6.45	11.	5.	5.	0.17	58.43	6.45
ROUTED TO	RT-APDFA	44.	6.45	11.	5.	5.	0.17		
HYDROGRAPH AT	SB-D6	45.	6.05	5.	2.	2.	0.04		
2 COMBINED AT	AP-D2	81.	6.10	15.	7.	7.	0.21		
ROUTED TO	RT-APD2	80.	6.10	15.	7.	7.	0.21		
HYDROGRAPH AT	SB-D7	82.	6.05	9.	4.	4.	0.07		
HYDROGRAPH AT	SB-D4	22.	6.15	3.	1.	1.	0.05		
HYDROGRAPH AT	SB-D5	22.	6.15	3.	1.	1.	0.03		
2 COMBINED AT	AP-DFB	44.	6.15	6.	3.	3.	0.08		
ROUTED TO	RR-DFB	39.	6.25	6.	3.	3.	0.08	20.69	6.25
ROUTED TO	RT-APDFB	39.	6.25	6.	3.	3.	0.08		
3 COMBINED AT	AP-D3	198.	6.10	29.	13.	13.	0.36		
ROUTED TO	RT-APD3	196.	6.10	29.	13.	13.	0.36		
HYDROGRAPH AT	SB-D8	133.	6.00	13.	6.	6.	0.06		
2 COMBINED AT	AP-DFC	311.	6.05	43.	19.	19.	0.42		
ROUTED TO	RR-DFC	66.	6.80	42.	19.	19.	0.42	60.06	6.80
ROUTED TO	RT-RRDFC	66.	6.85	42.	19.	19.	0.42		
HYDROGRAPH AT	SB-D9	83.	6.05	9.	4.	4.	0.06		
2 COMBINED AT	AP-D4	135.	6.10	51.	23.	23.	0.47		
ROUTED TO	RT-APD4	135.	6.10	51.	23.	23.	0.47		

25 YEAR 24 HOUR STORM

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR 6-HOUR	24-HOUR	72-HOUR	BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
HYDROGRAPH AT	SB-D1	45.	6.30	8.	3.	3.	0.09		
ROUTED TO	RT-SBD1	45.	6.35	8.	3.	3.	0.09		
HYDROGRAPH AT	SB-D2	46.	6.10	6.	2.	2.	0.06		
2 COMBINED AT	AP-D1	79.	6.20	13.	6.	6.	0.14		
HYDROGRAPH AT	SB-D3	27.	6.15	3.	2.	2.	0.03		
2 COMBINED AT	AP-DDA	105.	6.20	16.	7.	7.	0.17		
ROUTED TO	RR-DFA	54.	6.55	16.	7.	7.	0.17	60.75	6.55
ROUTED TO	RT-APDFA	54.	6.55	16.	7.	7.	0.17		
HYDROGRAPH AT	SB-D6	62.	6.05	6.	3.	3.	0.04		
2 COMBINED AT	AP-D2	101.	6.10	23.	10.	10.	0.21		
ROUTED TO	RT-APD2	101.	6.10	23.	10.	10.	0.21		
HYDROGRAPH AT	SB-D7	113.	6.05	12.	5.	5.	0.07		
HYDROGRAPH AT	SB-D4	36.	6.15	4.	2.	2.	0.05		
HYDROGRAPH AT	SB-D5	32.	6.15	4.	2.	2.	0.03		
2 COMBINED AT	AP-DFB	68.	6.15	8.	4.	4.	0.08		
ROUTED TO	RR-DFB	47.	6.30	8.	4.	4.	0.08	23.03	6.30
ROUTED TO	RT-APDFB	47.	6.30	8.	4.	4.	0.08		
3 COMBINED AT	AP-D3	254.	6.10	43.	19.	19.	0.36		
ROUTED TO	RT-APD3	253.	6.10	43.	19.	19.	0.36		
HYDROGRAPH AT	SB-D8	165.	6.00	17.	7.	7.	0.06		
2 COMBINED AT	AP-DFC	405.	6.05	60.	26.	26.	0.42		
ROUTED TO	RR-DFC	75.	7.05	58.	26.	26.	0.42	62.40	7.10
ROUTED TO	RT-RRDFC	75.	7.10	58.	26.	26.	0.42		
HYDROGRAPH AT	SB-D9	109.	6.05	12.	5.	5.	0.06		
2 COMBINED AT	AP-D4	168.	6.05	70.	31.	31.	0.47		
ROUTED TO	RT-APD4	167.	6.10	70.	31.	31.	0.47		

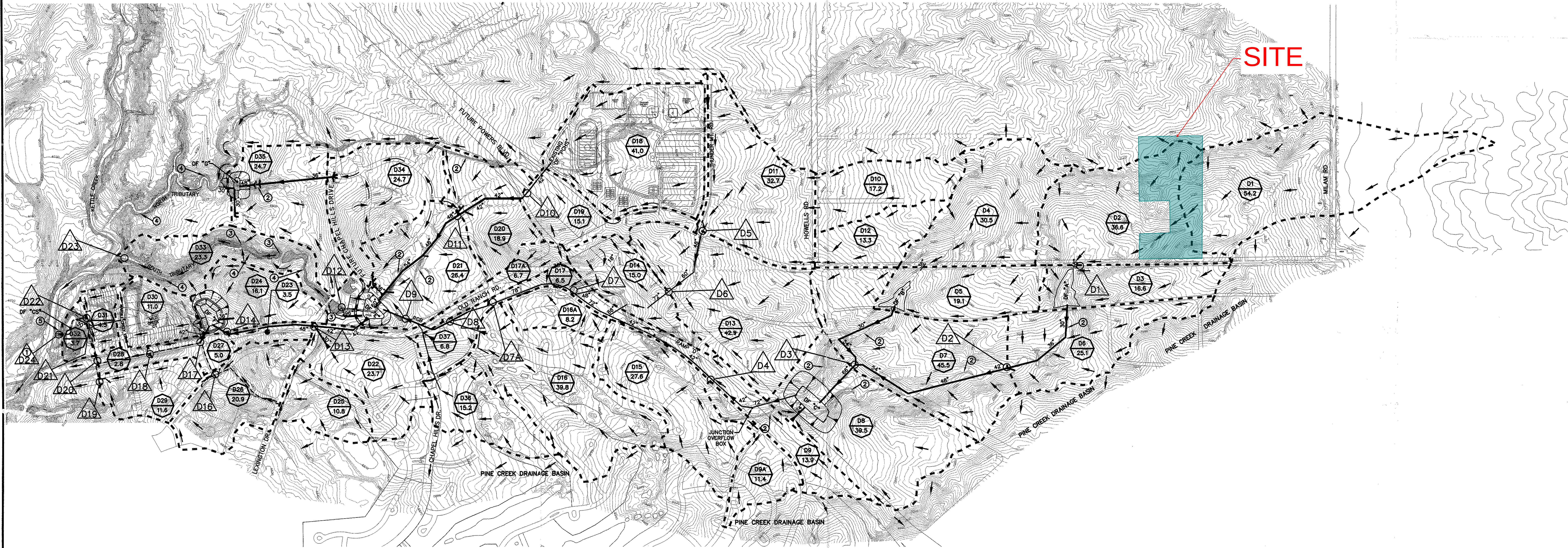
50 YEAR 24 HOUR STORM

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB-D1	58.	6.25	10.	4.	4.	0.09		
ROUTED TO	RT-SBD1	58.	6.30	10.	4.	4.	0.09		
HYDROGRAPH AT	SB-D2	59.	6.10	7.	3.	3.	0.06		
2 COMBINED AT	AP-D1	102.	6.20	16.	7.	7.	0.14		
HYDROGRAPH AT	SB-D3	33.	6.15	4.	2.	2.	0.03		
2 COMBINED AT	AP-DDA	134.	6.20	21.	9.	9.	0.17		
ROUTED TO	RR-DFA	60.	6.60	21.	9.	9.	0.17		
ROUTED TO	RT-APDFA	60.	6.60	21.	9.	9.	0.17		
HYDROGRAPH AT	SB-D6	74.	6.05	8.	3.	3.	0.04		
2 COMBINED AT	AP-D2	116.	6.05	28.	12.	12.	0.21		
ROUTED TO	RT-APD2	116.	6.10	28.	12.	12.	0.21		
HYDROGRAPH AT	SB-D7	134.	6.05	14.	6.	6.	0.07		
HYDROGRAPH AT	SB-D4	45.	6.15	6.	2.	2.	0.05		
HYDROGRAPH AT	SB-D5	40.	6.10	5.	2.	2.	0.03		
2 COMBINED AT	AP-DFB	85.	6.15	10.	5.	5.	0.08		
ROUTED TO	RR-DFB	52.	6.30	10.	5.	5.	0.08		
ROUTED TO	RT-APDFB	52.	6.35	10.	5.	5.	0.08		
3 COMBINED AT	AP-D3	291.	6.10	53.	23.	23.	0.36		
ROUTED TO	RT-APD3	291.	6.10	53.	23.	23.	0.36		
HYDROGRAPH AT	SB-D8	186.	6.00	19.	8.	8.	0.06		
2 COMBINED AT	AP-DFC	465.	6.05	72.	32.	32.	0.42		
ROUTED TO	RR-DFC	81.	7.20	67.	31.	31.	0.42		
ROUTED TO	RT-RRDFC	81.	7.20	67.	31.	31.	0.42		
HYDROGRAPH AT	SB-D9	127.	6.05	14.	6.	6.	0.06		
2 COMBINED AT	AP-D4	190.	6.05	80.	37.	37.	0.47		
ROUTED TO	RT-APD4	188.	6.05	80.	37.	37.	0.47		

KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
MASTER DEVELOPMENT DRAINAGE PLAN
FULLY DEVELOPED CONDITION BASIN MAP AND MASTER PLAN



KEYED NOTES

- ANALYSIS POINT D24 REPRESENTS A DIRECT ADDITION OF THE HYDROGRAPHS AT ANALYSIS POINT D23 AND THE OUTLET HYDROGRAPH FROM THE CREEKSIDE ESTATES REGIONAL DETENTION FACILITY. IT DOES NOT REFLECT ANY OTHER FLOW IN KETTLE CREEK.
- NATURAL CHANNEL IS PROPOSED TO BE ELIMINATED IN THIS AREA. STORM WATER TO BE CONVEYED IN A PROPOSED STORM DRAIN.
- REMOVE EXISTING EMBANKMENT DOWN TO LEVEL OF PERMANENT POOL. PROTECT REMAINING EMBANKMENT IF PRUDENT TO DO SO IN ORDER TO PROTECT AGAINST EXCESSIVE EROSION.
- NATURAL CHANNEL PROPOSED TO REMAIN UNIMPROVED IN THIS AREA.
- DETENTION FACILITY OUTLET AND SPILLWAY ARE PROPOSED TO BE REVISED.

GENERAL NOTES:

- PROPOSED STORM DRAINS SHOWN ON THIS PLAN ARE ONLY INTENDED TO INDICATE GENERAL LOCATIONS AND APPROXIMATE SIZES OF FUTURE FACILITIES. ACTUAL STORM DRAIN SIZES AND LOCATIONS SHALL BE DETERMINED WITH MORE DETAILED ANALYSIS AT THE TIME OF DETAILED DESIGN OF THE FACILITIES. IT IS LIKELY THAT ADDITIONAL FACILITIES NOT SHOWN ON THIS PLAN WILL BE REQUIRED.
- PROPOSED DETENTION FACILITIES SHOWN ON THIS PLAN ARE ONLY INTENDED TO INDICATE GENERAL LOCATIONS AND LAND AREA REQUIRED FOR THESE FACILITIES. ACTUAL LOCATIONS AND LAND AREA REQUIRED SHALL BE DETERMINED AT THE TIME OF DETAILED DESIGN OF THE FACILITIES.
- EXCEPT AS OTHERWISE NOTED, THIS PLAN SHALL NOT MODIFY THE REQUIREMENTS OF PREVIOUSLY APPROVED MASTER DEVELOPMENT DRAINAGE PLANS AND FINAL DRAINAGE REPORTS.

SUB-BASIN DATA SUMMARY FULLY DEVELOPED CONDITION									
SUB BASIN	AREA	PERCENT	CN	LAG	Q5	Q100			
I.D.	(sq. miles)	(acres)	IMPERVIOUS	(hours)	(cfs)	(cfs)			
D1	0.085	24.7	5.0	67.5	0.383	17			
D2	0.057	38.5	10.0	69.0	0.224	20			
D3	0.028	16.8	33.3	75.0	0.267	13			
D4	0.030	30.5	39.4	68.2	0.235	14			
D5	0.030	18.1	30.4	74.5	0.239	16			
D6	0.039	25.1	42.7	79.2	0.164	34			
D7	0.071	45.5	46.6	79.8	0.173	62			
D8	0.062	39.5	72.9	82.2	0.111	111			
D9	0.022	13.9	41.1	77.0	0.251	14			
D9A	0.018	11.4	31.8	74.5	0.213	10			
D10	0.027	17.2	8.3	68.0	0.307	7			
D11	0.051	32.7	29.6	74.2	0.231	27			
D12	0.021	13.3	34.5	74.6	0.280	10			
D13	0.007	4.3	32.6	84.0	0.137	115			
D14	0.023	15.0	36.7	78.0	0.158	19			
D15	0.043	27.6	31.0	75.0	0.200	26			
D16	0.025	16.1	38.5	72.5	0.197	31			
D16A	0.013	8.9	36.0	78.0	0.143	11			
D17	0.019	6.5	32.8	76.5	0.117	10			
D17A	0.011	6.7	60.9	99.0	0.120	21			
D18	0.064	41.0	76.3	82.6	0.182	38			
D19	0.024	15.1	40.0	80.9	0.122	25			
D20	0.030	18.9	80.4	96.5	0.108	62			
D21	0.009	5.9	33.0	88.0	0.114	11			
D22	0.037	23.7	40.9	78.5	0.156	31			
D23	0.005	3.5	56.7	88.0	0.110	8			
D24	0.025	16.1	65.5	74.5	0.151	16			
D25	0.017	10.8	53.2	82.2	0.156	18			
D26	0.033	20.9	30.0	75.5	0.145	24			
D27	0.008	5.0	46.5	80.0	0.143	8			
D28	0.004	2.8	49.6	84.0	0.137	9			
D29	0.018	11.6	32.8	78.2	0.146	15			
D30	0.017	11.0	25.5	74.4	0.157	11			
D31	0.007	4.5	40.0	78.5	0.146	6			
D32	0.009	5.9	33.0	88.0	0.114	11			
D33	0.036	23.3	10.4	70.2	0.138	18			
D34	0.039	24.7	36.5	77.0	0.159	30			
D35	0.039	24.7	40.0	78.0	0.161	32			
D36	0.024	15.2	19.5	72.0	0.237	10			
D37	0.011	6.8	93.1	99.0	0.124	23			
TOTAL	1.269	812.0							

ANALYSIS POINT DATA SUMMARY FULLY DEVELOPED CONDITION									
ANALYSIS POINT	WATERSHED AREA (acres)	Q2 (cfs)	Q5 (cfs)	Q10 (cfs)	Q25 (cfs)	Q50 (cfs)	Q100 (cfs)	POINT DESCRIPTION	
D1	90.8	0.14	11	31	48	79	102	TOTAL FLOW	
D1A	107.4	0.17	17	44	66	105	134	TOTAL POND INFLOW	
D2	132.5	0.21	30	64	81	101	116	TOTAL FLOW	
D2B	49.6	0.08	13	30	44	68	85	TOTAL POND INFLOW	
D3	227.6	0.36	73	148	198	254	291	TOTAL FLOW	
D3C	287.1	0.42	139	237	311	405	465	TOTAL POND INFLOW	
D4	278.5	0.44	47	64	73	85	102	TOTAL FLOW	
D5	63.2	0.10	19	40	58	88	110	TOTAL FLOW	
D6	106.1	0.17	90	141	178	238	280	TOTAL FLOW	
D7	441.1	0.69	163	244	311	418	492	TOTAL FLOW	
D7A	447.6	0.70	164	263	335	451	532	TOTAL FLOW	
D8	502.3	0.78	191	310	398	538	637	TOTAL FLOW	
D9	524.3	0.82	208	337	433	585	692	TOTAL FLOW	
D10	55.1	0.09	15	29	32	44	55	TOTAL FLOW	
D11	75.0	0.12	60	85	103	129	147	TOTAL FLOW	
D12	625.7	0.98	299	470	596	796	935	TOTAL POND INFLOW	
D13	**	0.98	9	77	149	287	417	OUTFLOW TO NAT. CHANNEL	
D14	* 649.4	1.02	55	79	95	113	132	TOTAL FLOW	
D15	* 652.9	1.02	58	84	101	127	144	TOTAL FLOW	
D16	* 659.0	1.05	66	100	123	159	184	TOTAL POND INFLOW	
D17	* 700.7	1.10	48	59	64	85	101	TOTAL FLOW	
D18	* 705.7	1.10	49	59	72	98	116	TOTAL FLOW	
D19	* 711.6	1.02	9	15	20	20	20	INTERCEPTED FLOW	
D20	* 720.1	1.15	49	74	97	129	145	TOTAL FLOW	
D21	* 731.1	1.14	49	84	111	146	171	TOTAL FLOW IN STORM DRAIN	
D22	* 735.6	1.15	52	90	119	158	184	TOTAL FLOW	
D23	* 739.3	1.15	53	93	123	164	192	TOTAL POND INFLOW	
D24	762.6	1.19	40	111	188	348	501	TOTAL FLOW FROM "DFCS" & D23	
D25	49.4	0.08	33	60	81	115	138	TOTAL POND INFLOW	

* WATERSHED AREAS DO NOT REFLECT THE TRIBUTARY AREA REDUCTION DUE TO THE DIVERSION OF FLOW Q'S ARE COMPUTED BASED ON THE ACTUAL BASIN AREAS.

** PEAK FLOW DIVERTED TO NATURAL CHANNEL DURING INFREQUENT RUNOFF EVENTS.

LEGEND

- 5660 EXISTING CONTOUR 10'
- EXISTING CONTOUR 2'
- DEVELOPED BASIN BOUNDARY
- EXISTING STORM DRAIN
- 48" ESTIMATED STORM DRAIN SIZE PROPOSED STORM DRAIN
- RBC PROPOSED REINFORCED BOX CULVERT
- DRAINAGE DIRECTION
- D8 BASIN IDENTIFIER
- 39.5 BASIN AREA (ACRES)
- D4 ANALYSIS POINT
- KEYED NOTE REFERENCE

DETENTION FACILITY DATA SUMMARY															
DETENTION FACILITY I.D.	PEAK INFLOW (CFS)					PEAK OUTFLOW (CFS)					ESTIMATED PEAK STORAGE (ACRE-Feet)				
	Q2	Q5	Q10	Q25	Q50	Q2	Q5	Q10	Q25	Q50	V2	V5	V10	V25	V50
A	17	44	66	105	134	165	17	38	45	54	60	65	0	0.1	0.4
B	13	30	44	68	85	103	13	30	39	47	52	57	0	0.0	0.1
C	139	237	311	405	465	524	45	60	66	75	81	86	5	7	13
D	299	470	596	796	935	1079	60	137	213	366	489	600	11	16	23
E	66	100	123	159	184	208	48	57	61	66	71	76	8	8	8
F	33	60	81	115	138	162	2	11	23	31	36	41	1	2	2
G	53	93	123	164	192	221	37	55	61	73	79	90	3	5	5
CREEKSIDE P.C. HIGH SCHOOL	19	38	51	74	90	106	1	6	14	29	33	36	1	1	2

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PREPARED FOR

JR ENGINEERING
A Subsidiary of Western
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719-533-2583 • Fax 719-533-6633
www.jr-engineering.com

DATE 10/2002
BY VSF
REVISD ROYAL PINE DRIVE STORM SEWER AT POWERS RAMP "D"

No. 1
H-SCALE 1"=400'
V-SCALE N/A
DATE 03/28/01
DESIGNED BY VSF
DRAWN BY ELY
CHECKED BY

KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
MASTER DEVELOPMENT DRAINAGE PLAN
FULLY DEVELOPED CONDITION
BASIN MAP AND MASTER PLAN

SHEET 1 OF 1
JOB NO. 28877.10

KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
DRAINAGE BASIN PLANNING STUDY AND MASTER DEVELOPMENT DRAINAGE PLAN
HISTORIC CONDITION DRAINAGE BASIN MAP

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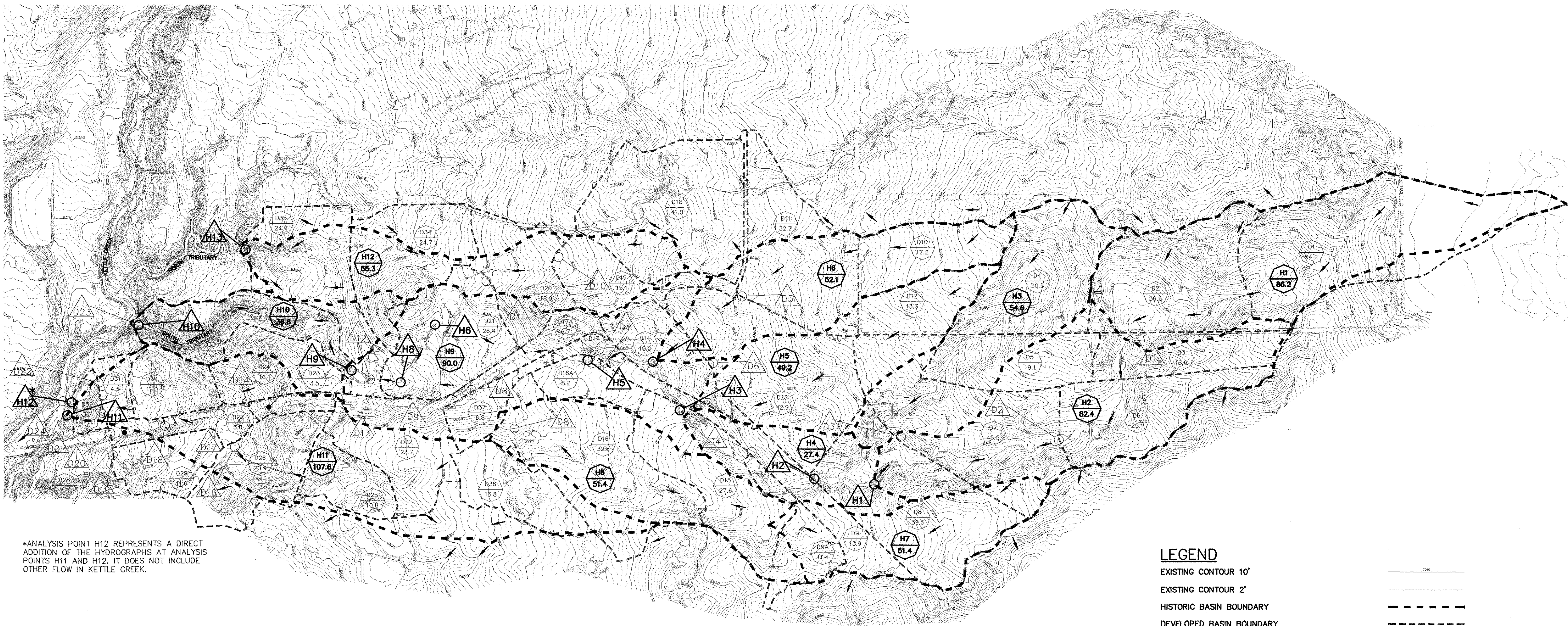
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BY	DATE	REVISION	No.	1" = 500'	H-SCALE	V-SCALE	DATE	DESIGNED BY	DRAWN BY	CHECKED BY
				N/A	04/18/01	VSF			SDG	

KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
DRAINAGE BASIN PLANNING STUDY AND
MASTER DEVELOPMENT DRAINAGE PLAN
HISTORIC DRAINAGE BASIN MAP

SHEET 1 OF 1
JOB NO. 8877.10

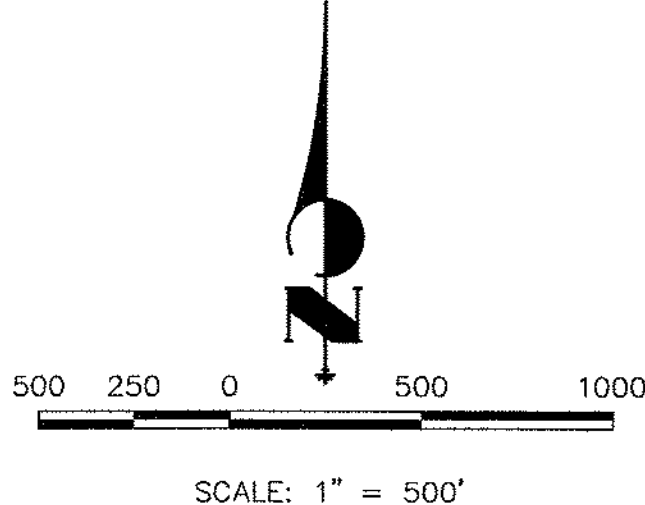


*ANALYSIS POINT H12 REPRESENTS A DIRECT
ADDITION OF THE HYDROGRAPHS AT ANALYSIS
POINTS H11 AND H12. IT DOES NOT INCLUDE
OTHER FLOW IN KETTLE CREEK.

(HISTORIC) UNDEVELOPED CONDITION BASIN DATA AND FLOW RATE SUMMARY									
SUBBASIN ID	AREA (acres)	AREA (sq mi)	CN	LAG (hrs)	Q10 (cfs)	Q10 (cfs/ac)	Q100 (cfs)	Q100 (cfs/ac)	Q100 (cfs/ac)
H1	86.2	0.135	66	0.420	35	0.4	99	1.1	
H2	82.4	0.129	66	0.303	43	0.5	118	1.4	
H3	54.6	0.085	66	0.314	27	0.5	76	1.4	
H4	27.4	0.043	66	0.188	19	0.7	51	1.9	
H5	49.2	0.077	66	0.286	27	0.5	72	1.5	
H6	52.1	0.081	66	0.392	22	0.4	62	1.2	
H7	37.8	0.059	66	0.359	17	0.4	48	1.3	
H8	51.4	0.080	66	0.346	24	0.5	67	1.3	
H9	90.0	0.141	66	0.359	47	0.5	130	1.4	
H10	36.6	0.057	66	0.215	24	0.7	63	1.7	
H11	107.6	0.168	66	0.411	45	0.4	125	1.2	
H12	55.3	0.086	66	0.397	23	0.4	65	1.2	

(HISTORIC) UNDEVELOPED CONDITION ANALYSIS POINT WATERSHED AREA AND FLOW RATE SUMMARY									
ANALYSIS POINT	AREA (acres)	AREA (sq mi)	Q2 (cfs)	Q5 (cfs)	Q10 (cfs)	Q25 (cfs)	Q50 (cfs)	Q100 (cfs)	Q100 (cfs)
H1	166	0.26	11	36	60	105	140	179	
H2	224	0.35	14	48	81	144	193	247	
H3	301	0.47	18	60	99	181	246	317	
H4	352	0.55	22	73	121	217	294	376	
H5	352	0.55	22	73	121	217	294	376	
H6	352	0.55	22	73	120	215	291	374	
H7	90	0.14	7	23	35	60	81	103	
H8	442	0.69	27	94	155	274	371	477	
H9	531	0.83	31	107	178	320	432	557	
H10	570	0.89	32	110	184	331	448	577	
H11	109	0.17	9	28	45	76	99	125	
H12	678	1.06	37	129	219	398	535	693	
H13	58	0.09	5	14	23	40	52	65	

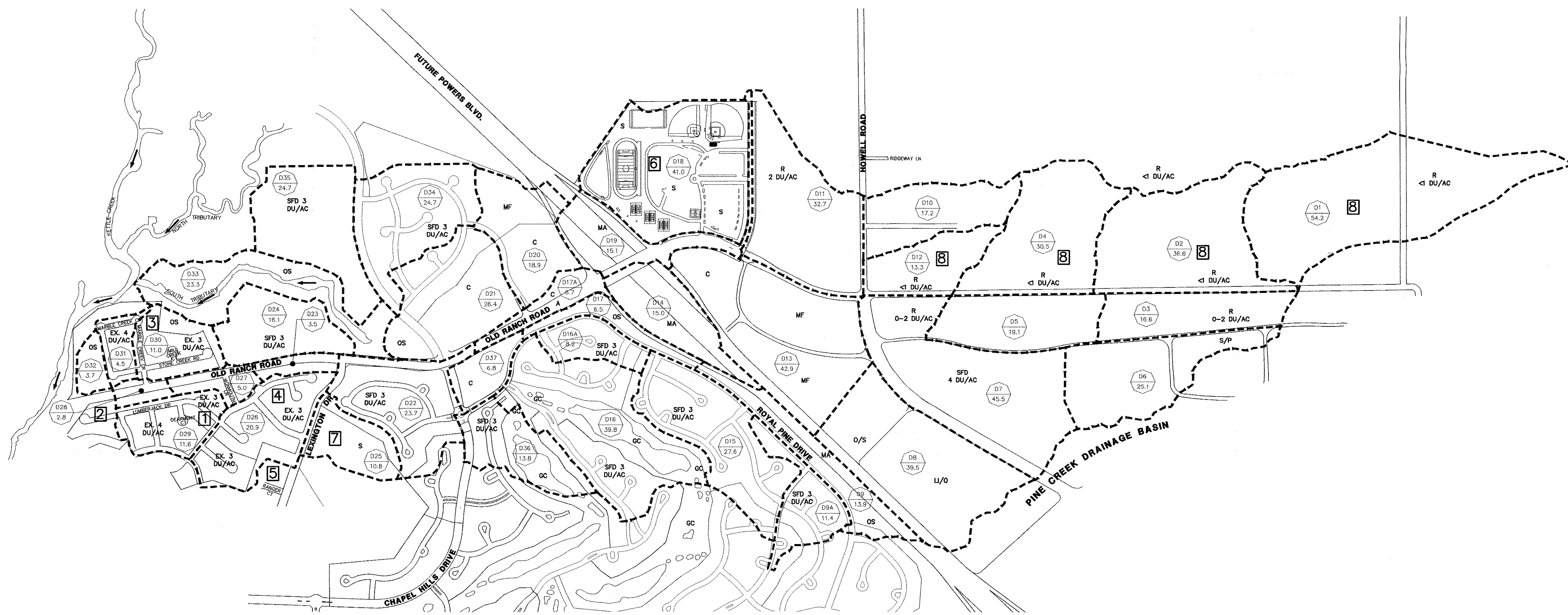
- NOTES:
1. PEAK FLOW RATES WERE ESTIMATED WITH HEC-1
COMPUTER MODEL USING THE SCS UNIT HYDROGRAPH
METHOD.
 2. HISTORIC FLOW RATES WERE ESTIMATED BASED ON AN
ASSUMED CONDITION PRIOR TO ANY DEVELOPMENT IN
THE WATERSHED. TOPO SHOWN ON THIS MAP REFLECTS
SOME CHANGES CAUSED BY DEVELOPMENT. HOWEVER,
FLOW PATHS UTILIZED IN THIS ANALYSIS WERE ESTIMATED
BASED ON PRE-DEVELOPMENT CONDITIONS TO THE
EXTENT POSSIBLE.



LEGEND

EXISTING CONTOUR 10'
EXISTING CONTOUR 2'
HISTORIC BASIN BOUNDARY
DEVELOPED BASIN BOUNDARY
DRAINAGE FLOW ARROW
DEVELOPED BASIN IDENTIFIER
AREA IN ACRES
HISTORIC BASIN IDENTIFIER
AREA IN ACRES
HISTORIC ANALYSIS POINT
DEVELOPED ANALYSIS POINT

DRAINAGE BASIN PLANNING STUDY AND MASTER DEVELOPMENT DRAINAGE PLAN
PLATTED SUBDIVISION I.D. AND LAND USE MAP
KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY

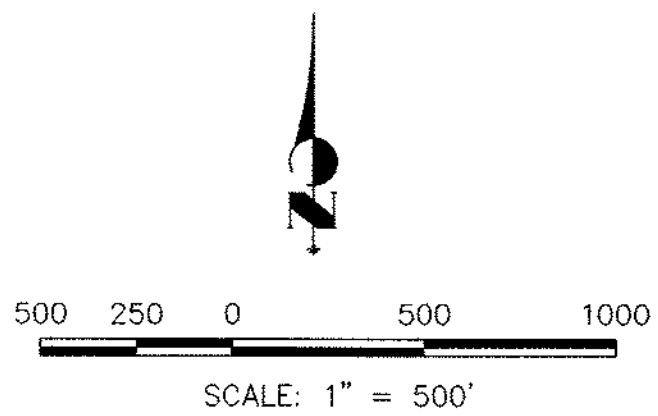


SUBDIVISION INDEX		
CREEKSIDE ESTATES FILING NO. 1	1	
CREEKSIDE ESTATES FILING NO. 2	2	
CREEKSIDE ESTATES FILING NO. 3	3	
CREEKSIDE ESTATES FILING NO. 4	4	
KITTY HAWK VILLAGE IN CHAPEL HILLS	5	
ACADEMY HIGH SCHOOL FILING NO. 5	6	
PINE CREEK FILING NO. 10	7	
MISCELLANEOUS 4 ACRE TO 20 ACRE LOTS	8	

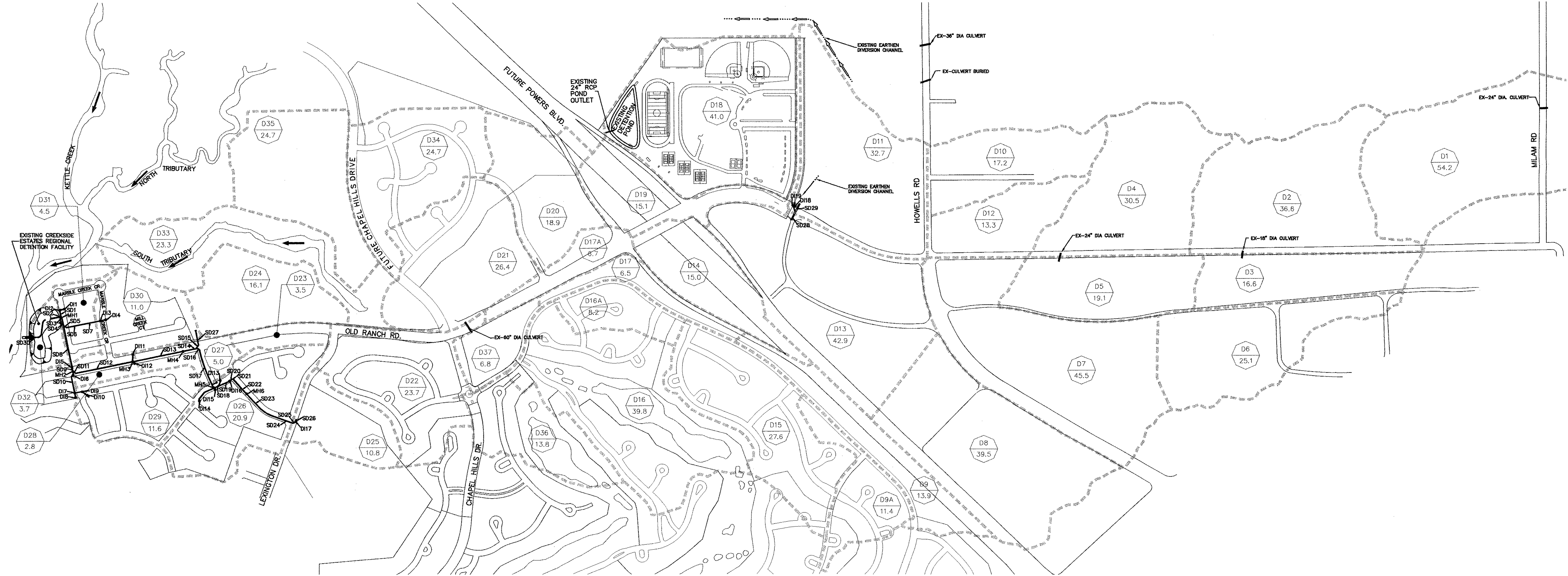
LAND USE LEGEND	
DESCRIPTION	SYMBOL
COMMERCIAL	C
LIGHT INDUSTRIAL/OFFICE	LI/O
OPEN SPACE	OS
RURAL RESIDENTIAL	R
GOLF COURSE	GC
MAJOR ARTERIAL ROADWAY	MA
MULTI FAMILY RESIDENTIAL	MF
SINGLE FAMILY DETACHED	SFD
SCHOOL	S
SCHOOL/PARK	S/P
SINGLE FAMILY ATTACHED	SFA
DWELLING UNITS PER ACRE	XX DU/AC
EXISTING DEVELOPMENT	EX

NOTE:
THE LAND USES SHOWN ON THIS MAP ARE THE LAND USES ASSUMED FOR THE PURPOSE OF PREPARING THIS MDDP AND ASSOCIATED DRAINAGE ANALYSIS. THESE LAND USES ARE PATTERNED AFTER THE MAY 1998 BRIARGATE MASTER PLAN AND MORE SPECIFIC NEIGHBORHOOD PLANS WHERE AVAILABLE.

LEGEND	
SUB-BASIN ID	
SUB-BASIN AREA (ACRES)	
SUB-BASIN BOUNDARY	



KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
DRAINAGE BASIN PLANNING STUDY AND MASTER DEVELOPMENT DRAINAGE PLAN
EXISTING STORM DRAIN FACILITY MAP



STORM DRAIN PIPE DATA TABLE				
I.D. NUMBER	LENGTH	DIAMETER	TYPE	SLOPE
SD1	81'	24"	RCP	1.00%
SD2	26'	48"	RCP	2.20%
SD3	11'	48"	RCP	2.50%
SD4	56'	48"	RCP	2.50%
SD5	30'	24"	RCP	2.00%
SD6	85'	24"	RCP	2.00%
SD7	208'	24"	RCP	9.60%
SD8	364'	48"	RCP	2.50%
SD9	31'	48"	RCP	2.25%
SD10	148'	18"	RCP	13.20%
SD11	27'	42"	RCP	5.14%
SD12	494'	42"	RCP	5.14%
SD13	440'	42"	RCP	5.00%
SD14	114'	42"	RCP	4.00%
SD15	71'	30"	RCP	3.64%
SD16	33'	42"	RCP	4.00%
SD17	336'	42"	RCP	4.00%
SD18	36'	36"	RCP	2.00%
SD19	103'	36"	RCP	2.20%
SD20	21'	36"	RCP	2.20%
SD21	10'	36"	RCP	2.50%
SD22	168'	36"	RCP	4.80%
SD23	296'	30"	RCP	9.60%
SD24	181'	30"	RCP	2.80%
SD25	24'	30"	RCP	2.80%
SD26	8'	30"	RCP	1.20%
SD27	81'	30"	CMP	3.60%
SD28	63'	48"	RCP	1.00%
SD29	92'	48"	RCP	1.00%
SD30	81'	24"	RCP	1.00%

DRAINAGE INLET DATA TABLE				
I.D. NUMBER	LENGTH	TYPE	DEPTH	CONNECTOR STORM DRAIN
D1	6'	D-10-R	4.5'	37', 18" RCP @ 2.00%
D12	4'	D-10-R	5.0'	54', 24" RCP @ 2.00%
D13	4'	D-10-R	5.0'	N/A
D14	10'	D-10-R	4.5'	37', 24" RCP @ 2.00%
D15	20'	D-10-R	6.0'	N/A
D16	20'	D-10-R	4.0'	29', 30" RCP @ 2.00%
D17	14'	D-10-R	3.7'	N/A
D18	20'	D-10-R	3.0'	37', 18" RCP @ 1.00%
D19	20'	D-10-R	3.5'	48', 18" RCP @ 4.00%
D110	20'	D-10-R	3.0'	54', 18" RCP @ 4.00%
D111	20'	D-10-R	4.0'	52', 24" RCP @ 5.00%
D112	20'	D-10-R	5.0'	14', 18" RCP @ 5.00%
D113	10'	D-10-R	3.0'	45', 18" RCP @ 8.80%
D114	10'	D-10-R	4.5'	180', 18" RCP @ 1.50%
D115	18'	D-10-R	8.0'	5', 36" RCP @ 1.00%
D116	10'	D-10-R	8.0'	N/A
D117	16'	D-10-R	9.0'	N/A
D118	15'	D-10-R	4.0'	50', 24" RCP @ 1.00%
D119	10'	D-10-R	6.0'	20', 24" RCP @ 20.00%

MANHOLE DATA TABLE		
I.D. NUMBER	TYPE	DEPTH (FT)
MH1	TYPE 1	9.5'
MH2	TYPE 1	6.0'
MH3	TYPE 1	7.8'
MH4	TYPE 1	11.9'
MH5	TYPE 1	8.7'
MH6	TYPE 1	11.0'

LEGEND

SUB-BASIN ID ———→ D6

SUB-BASIN AREA (ACRES) ———→ 25.1

SUB-BASIN BOUNDARY ———→

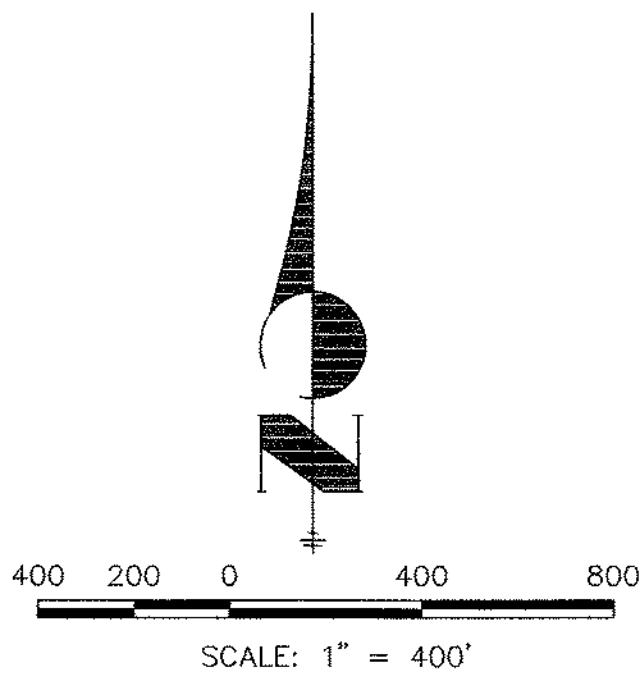
EXISTING STORM DRAIN ———→ SD15

EXISTING STORM MANHOLE ———→ MH1

EXISTING STORM DRAIN INLET ———→ DI15

NOTE:

THE INFORMATION CONTAINED ON THIS MAP WAS OBTAINED IN THE REVIEW OF DRAINAGE PLANS AND CONSTRUCTION PLANS. THE INFORMATION IS INTENDED TO DEMONSTRATE THE GENERAL EXTENT OF EXISTING DRAINAGE FACILITIES AND IS NOT INTENDED OR WARRANTED FOR ANY OTHER USE.



KETTLE CREEK DRAINAGE BASIN
OLD RANCH ROAD TRIBUTARY
DRAINAGE BASIN PLANNING STUDY AND
MASTER DEVELOPMENT DRAINAGE PLAN
EXISTING STORM DRAIN FACILITY MAP
JOB NO. 8877.10
04/18/01
SHEET 1 OF 1



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APPENDIX B

HYDROLOGIC COMPUTATIONS

COMPOSITE % IMPERVIOUS CALCULATIONS

Subdivision: Elk View Estates
Location: CO, Colorado Springs

Project Name: Elk View Estates
Project No.: ERD01.20
Calculated By: EBB
Checked By: TJE
Date: 7/30/26

Add tables with notes

Basin ID	Total Area (ac)	<1 DU/AC *			Undeveloped / Historic			Single-Family (2.5 Acre Lots)			Paved Streets			Basins Total
		% Imp.	Area (ac)	Weighted % Imp.	% Imp.	Area (ac)	Weighted % Imp.	% Imp.	Area (ac)	Weighted % Imp.	% Imp.	Area (ac)	Weighted % Imp.	Weighted % Imp.
EXISTING CONDITIONS														
K-1	2.1	5		0.0	2	2.1	2.0	11		0.0	100		0.00	2.0
KO-D10	41.6	5	27.1	3.3	2	14.5	0.7	11		0.0	100		0.00	4.0
KO-D11	8.4	5	8.4	5.0	2		0.0	11		0.0	100		0.00	5.0
KO-D20a	13.0	5		0.0	2	13.0	2.0	11		0.0	100		0.00	2.0
KO-D20b	2.3	5		0.0	2	2.3	2.0	11		0.0	100		0.00	2.0
KO-D21	2.5	5	2.5	5.0	2		0.0	11		0.0	100		0.00	5.0
KO-D22	0.6	5	0.6	5.0	2		0.0	11		0.0	100		0.00	5.0
KO-D23	22.4	5	22.4	5.0	2		0.0	11		0.0	100		0.00	5.0
PROPOSED CONDITIONS														
K-12	2.2	5		0.0	2		0.0	11	2.2	11.0	100		0.00	11.0
KO-D201a	0.4	5		0.0	2	0.10	0.5	11		0.0	100	0.3	75.00	75.5
KO-D201b	0.2	5		0.0	2		0.0	11		0.0	100	0.1	50.00	50.0
KO-D202	1.7	5		0.0	2		0.0	11	1.7	11.0	100		0.00	11.0
KO-D203	0.2	5		0.0	2		0.0	11		0.0	100	0.2	100.00	100.0
KO-D204	0.3	5		0.0	2		0.0	11		0.0	100	0.3	100.00	100.0
KO-D205a	0.4	5		0.0	2	0.10	0.5	11		0.0	100	0.3	75.00	75.5
KO-D205b	0.2	5		0.0	2		0.0	11		0.0	100	0.2	100.00	100.0
KO-D206	2.3	5		0.0	2		0.0	11	2.3	11.0	100		0.00	11.0
KO-D207	8.6	5		0.0	2		0.0	11	8.6	11.0	100		0.00	11.0
KO-D21	2.5	5		0.0	2		0.0	11		0.0	100		0.00	0.0
KO-D221	0.7	5	0.70	5.0	2		0.0	11		0.0	100		0.00	5.0
KO-D222	0.8	5		0.0	2		0.0	11	0.8	11.0	100		0.00	11.0

* percent impervious taken from the Kettel Creek - Old Ranch Rd. Tributary DBPS for off-site basins adjacent to site to be consistent

369.036
3.9724

TOTAL SITE IMPERVIOUSNESS SUMMARY

Subdivision: Elk View Estates
Location: CO, Colorado Springs

Project Name: Elk View Estates
Project No.: ERD01.20
Calculated By: EBB
Checked By: TJE
Date: 7/30/26

Basin	Area	% Imp
K-12	2.2	11.0
KO-D201a	0.4	75.5
KO-D201b	0.2	50.0
KO-D202	1.7	11.0
KO-D203	0.2	100.0
KO-D204	0.3	100.0
KO-D205a	0.4	75.5
KO-D205b	0.2	100.0
KO-D206	2.3	11.0
KO-D207	8.6	11.0
KO-D221	0.7	5.0
Total	17.2	17.8

COMPOSITE C VALUE CALCULATIONS

Subdivision: Elk View Estates
Location: CO, Colorado Springs

Project Name: Elk View Estates
Project No.: ERD01.20
Calculated By: EBB
Checked By: TJE
Date: 7/30/26

Basin ID	Total Area (ac)	Soil Type	<1 DU/AC *			Undeveloped / Historic			Single-Family (2.5 Acre Lots) **			Paved Streets			Composite C ₅	Composite C ₁₀₀
			C ₅	C ₁₀₀	Area (ac)	C ₅	C ₁₀₀	Area (ac)	C ₅	C ₁₀₀	Area (ac)	C ₅	C ₁₀₀	Area (ac)		
EXISTING CONDITIONS																
K-1	2.1	B	0.20	0.44	0.0	0.09	0.36	2.1	0.20	0.44	0.0	0.90	0.96	0.0	0.09	0.36
KO-D10	41.6	B	0.20	0.44	27.1	0.09	0.36	14.5	0.20	0.44	0.0	0.90	0.96	0.0	0.16	0.41
KO-D11	8.4	B	0.20	0.44	8.4	0.09	0.36	0.0	0.20	0.44	0.0	0.90	0.96	0.0	0.20	0.44
KO-D20a	13.0	B	0.20	0.44	0.0	0.09	0.36	13.0	0.20	0.44	0.0	0.90	0.96	0.0	0.09	0.36
KO-D20b	2.3	B	0.20	0.44	0.0	0.09	0.36	2.3	0.20	0.44	0.0	0.90	0.96	0.0	0.09	0.36
KO-D21	2.5	B	0.20	0.44	2.5	0.09	0.36	0.0	0.20	0.44	0.0	0.90	0.96	0.0	0.20	0.44
KO-D22	0.6	B	0.20	0.44	0.6	0.09	0.36	0.0	0.20	0.44	0.0	0.90	0.96	0.0	0.20	0.44
KO-D23	22.4	B	0.20	0.44	22.4	0.09	0.36	0.0	0.20	0.44	0.0	0.90	0.96	0.0	0.20	0.44
PROPOSED CONDITIONS																
K-12	2.2	B	0.20	0.44	0.00	0.09	0.36	0.00	0.20	0.44	2.20	0.90	0.96	0.00	0.20	0.44
KO-D201a	0.4	B	0.20	0.44	0.00	0.09	0.36	0.10	0.20	0.44	0.00	0.90	0.96	0.30	0.70	0.81
KO-D201b	0.2	B	0.20	0.44	0.00	0.09	0.36	0.00	0.20	0.44	0.00	0.90	0.96	0.10	0.45	0.48
KO-D202	1.7	B	0.20	0.44	0.00	0.09	0.36	0.00	0.20	0.44	1.70	0.90	0.96	0.00	0.20	0.44
KO-D203	0.2	B	0.20	0.44	0.00	0.09	0.36	0.00	0.20	0.44	0.00	0.90	0.96	0.20	0.90	0.96
KO-D204	0.3	B	0.20	0.44	0.00	0.09	0.36	0.00	0.20	0.44	0.00	0.90	0.96	0.30	0.90	0.96
KO-D205a	0.4	B	0.20	0.44	0.00	0.09	0.36	0.10	0.20	0.44	0.00	0.90	0.96	0.30	0.70	0.81
KO-D205b	0.2	B	0.20	0.44	0.00	0.09	0.36	0.00	0.20	0.44	0.00	0.90	0.96	0.20	0.90	0.96
KO-D206	2.3	B	0.20	0.44	0.00	0.09	0.36	0.00	0.20	0.44	2.30	0.90	0.96	0.00	0.20	0.44
KO-D207	8.6	B	0.20	0.44	0.00	0.09	0.36	0.00	0.20	0.44	8.60	0.90	0.96	0.00	0.20	0.44
KO-D21	2.5	B	0.20	0.44	0.00	0.09	0.36	0.00	0.20	0.44	0.00	0.90	0.96	0.00	0.00	0.00
KO-D221	0.7	B	0.20	0.44	0.70	0.09	0.36	0.00	0.20	0.44	0.00	0.90	0.96	0.00	0.20	0.44
KO-D222	0.8	B	0.20	0.44	0.00	0.09	0.36	0.00	0.20	0.44	0.80	0.90	0.96	0.00	0.20	0.44

* Curve Number was utilized in DBPS - Runoff coefficients are based on 1-acre Residential per Table 6-6 of ECM

STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Elk View Estates
Location: CO, Colorado Springs

Project Name: Elk View Estates
Project No.: ERD01.20
Calculated By: EBB
Checked By: TJE
Date: 7/30/26

SUB-BASIN						INITIAL/OVERLAND (Sheet Flow)			Shallow Concentrated Flows					Tc CHECK		FINAL
DATA						(T _i)			(T _t)							
BASIN ID	D.A. (AC)	Hydrologic Soils Group	Impervious (%)	C ₁₀₀	C ₅	L (FT)	S (%)	T _i (MIN)	L (FT)	S (%)	Cv	VEL. (FPS)	T _t (MIN)	COMP. T _c (MIN)	Regional T _c (MIN)	
EXISTING CONDITIONS																
K-1	2.10	B	2.0	0.36	0.09	120	7.2	10.5	171	4.2	7.0	1.4	2.0	12.5	27.2	12.5
KO-D10	41.60	B	4.0	0.41	0.16	300	6.8	15.7	2893	4.3	7.0	1.5	33.2	48.9	49.7	48.9
KO-D11	8.40	B	5.0	0.44	0.20	162	5.3	12.0	1352	4.3	7.0	1.5	15.5	27.6	36.4	27.6
KO-D20a	13.00	B	2.0	0.36	0.09	200	9.2	12.5	1237	4.3	7.0	1.5	14.2	26.7	36.4	26.7
KO-D20b	2.30	B	2.0	0.36	0.09	100	5.4	10.5	525	5.4	10.0	2.3	3.8	14.3	29.7	14.3
KO-D21	2.50	B	5.0	0.44	0.20	54	3.8	7.8	288	10.7	10.0	3.3	1.5	9.2	26.7	9.2
KO-D22	0.60	B	5.0	0.44	0.20	123	6.1	10.0	174	3.7	10.0	1.9	1.5	11.5	26.7	11.5
KO-D23	22.40	B	5.0	0.44	0.20	300	9.6	13.4	970	5.9	10.0	2.4	6.7	20.1	32.0	20.1
PROPOSED CONDITIONS																
K-12	2.20	B	11.0	0.44	0.20	120	7.2	9.4	171	4.2	10.0	2.0	1.4	10.7	25.4	10.7
KO-D201a	0.40	B	75.5	0.81	0.70	25	2.0	2.9	485	8.0	15.0	4.2	1.9	4.8	14.6	5.0
KO-D201b	0.20	B	50.0	0.48	0.45	25	2.0	4.7	334	4.0	15.0	3.0	1.9	6.6	19.2	6.6
KO-D202	1.70	B	11.0	0.44	0.20	100	6.5	8.8	160	4.2	10.0	2.0	1.3	10.1	25.4	10.1
KO-D203	0.20	B	100.0	0.96	0.90	25	2.0	1.5	421	6.5	15.0	3.8	1.8	3.3	10.2	5.0
KO-D204	0.30	B	100.0	0.96	0.90	100	2.0	2.9	116	2.0	15.0	2.1	0.9	3.8	9.6	5.0
KO-D205a	0.40	B	75.5	0.81	0.70	25	2.0	2.9	425	4.0	15.0	3.0	2.4	5.3	15.0	5.3
KO-D205b	0.20	B	100.0	0.96	0.90	25	2.0	1.5	322	4.0	15.0	3.0	1.8	3.2	10.2	5.0
KO-D206	2.30	B	11.0	0.44	0.20	100	5.4	9.4	525	5.4	10.0	2.3	3.8	13.2	27.7	13.2
KO-D207	8.60	B	11.0	0.44	0.20	200	9.2	11.1	1237	5.2	15.0	3.4	6.0	17.2	32.7	17.2
KO-D21	2.50	B	0.0	0.00	0.00	54	3.8	9.5	288	10.7	10.0	3.3	1.5	11.0	27.6	11.0
KO-D221	0.70	B	5.0	0.44	0.20	123	6.1	10.0	174	3.7	10.0	1.9	1.5	11.5	26.7	11.5
KO-D222	0.80	B	11.0	0.44	0.20	25	2.0	6.5	663	3.4	15.0	2.8	4.0	10.5	29.8	10.5

NOTES:

$T_i = (0.395 * (1.1 - C_s) * (L)^{0.5}) / ((S)^{0.33})$, S in ft/ft

$T_c = L / 60V$ (Velocity From Fig. 501)

Velocity $V = C_v * S^{0.5}$, S in ft/ft

$T_c \text{ Regional} = (26 - 17i) + (L / (60(14i + 9)S^{0.5}))$, S in ft/ft, i= Imperviousness expressed as decimal

For Urbanized basins a minimum T_c of 5.0 minutes is required.

For non-urbanized basins a minimum T_c of 10.0 minutes is required

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Elk View Estates
Location: CO, Colorado Springs
Design Storm: 5-Year

Project Name: Elk View Estates
Project No.: ERD01.20
Calculated By: EBB
Checked By: TJE
Date: 7/30/26

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE		TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	
EXISTING CONDITIONS																				
		K-1	2.1	0.09	12.5	0.19	3.80	0.7												PORTION OF SITE IN KETTLE CREEK TRIB.
		KO-D10	41.6	0.16	48.9	6.73	1.75	11.8												OFFSITE BASIN - EAST
		KO-D11	8.4	0.20	27.6	1.68	2.61	4.4												OFFSITE BASIN - SOUTHEAST
		KO-D22	0.6	0.20	11.5	0.12	3.92	0.5												OFFSITE BASIN - SOUTHEAST
	D10		50.6						48.9	8.53	1.75	14.9								KO-D10 + KO-D11 + KO-D22
		KO-D20a	13.0	0.09	26.7	1.17	2.66	3.1												PORTION OF SITE IN KETTLE CREEK - ORR TRIB.
	D12	KO-D20b	2.3	0.09	14.3	0.21	3.59	0.8				0.8								
		KO-D21	2.5	0.20	9.2	0.50	4.25	2.1												OFFSITE BASIN - CENTRAL / WEST
	D11		66.1						48.9	10.41	1.75	18.2								DP D10 + KO-D20 + KO-D21 EXISTING 18" CMP CULVERT UNDER STUDEBAKER ST.
		KO-D23	22.4	0.20	20.1	4.48	3.08	13.8												OFFSITE BASIN - WEST
	D1		90.8						48.9	14.89	1.75	26.1								DP D11 + KO-D23+ DP D12 EXISTING 18" CMP CULVERT UNDER ORR

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Elk View Estates
Location: CO, Colorado Springs
Design Storm: 5-Year

Project Name: Elk View Estates
Project No.: ERD01.20
Calculated By: EBB
Checked By: TJE
Date: 7/30/26

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C* A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	
PROPOSED CONDITIONS																					
		K-12	2.2	0.20	10.7	0.44	4.02	1.8													PORTION OF SITE IN KETTLE CREEK TRIB.
	K1								10.7	0.44	4.02	1.8									TOTAL RUNOFF FROM K-11 & K-12
		KO-D10	41.6	0.16	48.9	6.73	1.75	11.8													OFFSITE BASIN - EAST
		KO-D201a	0.4	0.70	5.0	0.28	5.17	1.4													NORTHEAST SIDE OF PROP. PRIVATE ACCESS ROAD
	D10		42.0						48.9	7.01	1.75	12.3									KO-D10 + KO-D201a PR. (2) 30" RCP CULVERT
		KO-D11	8.4	0.20	27.6	1.68	2.61	4.4													OFFSITE BASIN - EAST
		KO-D201b	0.2	0.45	6.6	0.09	4.76	0.4													NORTHEAST SIDE OF PROP. PRIVATE ACCESS ROAD
	D11		8.6						27.6	1.77	2.61	4.6									KO-D11 + KO-D201b PR. 18" RCP CULVERT
		KO-D202	1.7	0.20	10.1	0.34	4.11	1.4													PROP. NORTHEAST SUB-BASIN
		KO-D203	0.2	0.90	5.0	0.18	5.17	0.9													PROP. NORTHEAST SUB-BASIN
	D12		1.9						10.1	0.52	4.11	2.1									KO-D202 + KO-D203 PR. 18" RCP CULVERT
		KO-D204	0.3	0.90	5.0	0.27	5.17	1.4													PROP. NORTHEAST SUB-BASIN
		KO-D205a	0.4	0.70	5.3	0.28	5.09	1.4													NORTHWEST SIDE OF PROP. PRIVATE ACCESS ROAD
		KO-D205b	0.2	0.90	5.0	0.18	5.17	0.9													SOUTHWEST SIDE OF PROP. PRIVATE ACCESS ROAD
	D13		53.4						48.9	10.03	1.75	17.6									DP D10 + DP D11 + DP D12 + KO-D204 + KO-D205a + KO-D205b
	D14a	KO-D206	2.3	0.20	13.2	0.46	3.72	1.7				1.7									PROP. NORTHWEST SUB-BASIN Flows from KO-D206 leaving site
		KO-D21	2.5	0.20	9.2	0.50	4.25	2.1													OFFSITE BASIN - CENTRAL / WEST
	D14		4.8						13.2	0.96	3.72	3.6									KO-D206 + KO-D21

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Elk View Estates
Location: CO, Colorado Springs
Design Storm: 5-Year

Project Name: Elk View Estates
Project No.: ERD01.20
Calculated By: EBB
Checked By: TJE
Date: 7/30/26

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	
		KO-D221	0.7	0.20	11.5	0.14	3.92	0.5													OFF-SITE BASIN - SOUTHEAST
	D15		0.7						11.5	0.14	3.92	0.5									PR. 18" RCP CULVERT
		KO-D222	0.8	0.20	10.5	0.16	4.05	0.6													PROP. SOUTHEAST BASIN
	D16		1.5						11.5	0.30	3.92	1.2									DP14 + KO-D222
		KO-D207	8.6	0.20	17.2	1.72	3.32	5.7													PROP. MAIN SUB-BASIN
	D17		68.3						48.9	13.01	1.75	22.8									DP D13 + DP D14 + DP D16 + KO-D207 EXISTING 18" CMP CULVERT UNDER STUDEBAKER ST.
		KO-D23	22.4	0.20	20.1	4.48	3.08	13.8													OFFSITE BASIN - WEST
	D1		90.7						48.9	17.49	1.75	30.6									DP D17 + KO-D23 EXISTING 18" CMP CULVERT UNDER ORR

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Elk View Estates
Location: CO, Colorado Springs
Design Storm: 100-Year

Project Name: Elk View Estates
Project No.: ERD01.20
Calculated By: EBB
Checked By: TJE
Date: 7/30/26

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE		TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	
EXISTING CONDITIONS																				
		K-1	2.1	0.36	12.5	0.76	6.37	4.8												PORTION OF SITE IN KETTLE CREEK TRIB.
		KO-D10	41.6	0.41	48.9	17.14	2.93	50.2												OFFSITE BASIN - EAST
		KO-D11	8.4	0.44	27.6	3.70	4.38	16.2												OFFSITE BASIN - SOUTHEAST
		KO-D22	0.6	0.44	11.5	0.26	6.58	1.7												OFFSITE BASIN - SOUTHEAST
	D10		50.6						48.9	21.10	2.93	61.8								KO-D10 + KO-D11 + KO-D22
		KO-D20a	13.0	0.36	26.7	4.68	4.46	20.9												PORTION OF SITE IN KETTLE CREEK - ORR TRIB.
	D12	KO-D20b	2.3	0.36	14.3	0.83	6.03	5.0				5.0								
		KO-D21	2.5	0.44	9.2	1.10	7.13	7.8												OFFSITE BASIN - CENTRAL / WEST
	D11		17.8						48.9	27.71	2.93	81.2								DP D10 + KO-D20 + KO-D21 EXISTING 18" CMP CULVERT UNDER STUDEBAKER ST.
		KO-D23	22.4	0.44	20.1	9.86	5.17	51.0												OFFSITE BASIN - WEST
	D1		22.4						48.9	37.57	2.93	110.1								DP D11 + KO-D23 EXISTING 18" CMP CULVERT UNDER ORR

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Elk View Estates
Location: CO, Colorado Springs
Design Storm: 100-Year

Project Name: Elk View Estates
Project No.: ERD01.20
Calculated By: EBB
Checked By: TJE
Date: 7/30/26

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE		TRAVEL TIME			REMARKS	
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)		Tt (min)
PROPOSED CONDITIONS																					
		K-12	2.2	0.44	10.7	0.97	6.75	6.5												PORTION OF SITE IN KETTLE CREEK TRIB.	
	K1								10.7	0.97	6.75	6.5								TOTAL RUNOFF FROM K-11 & K-12	
		KO-D10	41.6	0.41	48.9	17.14	2.93	50.2												OFFSITE BASIN - EAST	
		KO-D201a	0.4	0.81	5.0	0.32	8.68	2.8												NORTHEAST SIDE OF PROP. PRIVATE ACCESS ROAD	
	D10		42.0						48.9	17.46	2.93	51.2								KO-D10 + KO-D201a PR. (2) 30" RCP CULVERT	
		KO-D11	8.4	0.44	27.6	3.70	4.38	16.2												OFFSITE BASIN - EAST	
		KO-D201b	0.2	0.48	6.6	0.10	7.99	0.8												NORTHEAST SIDE OF PROP. PRIVATE ACCESS ROAD	
	D11		8.6						27.6	3.80	4.38	16.6								KO-D11 + KO-D201b PR. 18" RCP CULVERT	
		KO-D202	1.7	0.44	10.1	0.75	6.90	5.2												PROP. NORTHEAST SUB-BASIN	
		KO-D203	0.2	0.96	5.0	0.19	8.68	1.6												PROP. NORTHEAST SUB-BASIN	
	D12		1.9						10.1	0.94	6.90	6.5								KO-D202 + KO-D203 PR. 18" RCP CULVERT	
		KO-D204	0.3	0.96	5.0	0.29	8.68	2.5												PROP. NORTHEAST SUB-BASIN	
		KO-D205a	0.4	0.81	5.3	0.32	8.54	2.7												NORTHWEST SIDE OF PROP. PRIVATE ACCESS ROAD	
		KO-D205b	0.2	0.96	5.0	0.19	8.68	1.6												SOUTHWEST SIDE OF PROP. PRIVATE ACCESS ROAD	
	D13		53.4						48.9	23.00	2.93	67.4								DP D10 + DP D11 + DP D12 + KO-D204 + KO-D205a + KO-D205b	
	D14a	KO-D206	2.3	0.44	13.2	1.01	6.24	6.3				6.3								PROP. NORTHWEST SUB-BASIN	
		KO-D21	2.5	0.44	9.2	1.10	7.13	7.8												OFFSITE BASIN - CENTRAL / WEST	
	D14		4.8						13.2	2.11	6.24	13.2								KO-D206 + KO-D21	

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Elk View Estates
Location: CO, Colorado Springs
Design Storm: 100-Year

Project Name: Elk View Estates
Project No.: ERD01.20
Calculated By: EBB
Checked By: TJE
Date: 7/30/26

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	
		KO-D221	0.7	0.44	11.5	0.31	6.58	2.0													OFF-SITE BASIN - SOUTHEAST
	D15								11.5	0.31	6.58	2.0									PR. 18" RCP CULVERT
		KO-D222	0.8	0.44	10.5	0.35	6.80	2.4													PROP. SOUTHEAST BASIN
	D16		1.5						11.5	0.66	6.58	4.3									DP14 + KO-D222
		KO-D207	8.6	0.44	17.2	3.78	5.57	21.1													PROP. MAIN SUB-BASIN
	D17		68.3						48.9	29.55	2.93	86.6									DP D13 + DP D14 + DP D16 + KO-D207 EXISTING 18" CMP CULVERT UNDER STUDEBAKER ST.
		KO-D23	22.4	0.44	20.1	9.86	5.17	51.0													OFFSITE BASIN - WEST
	D1		90.7						48.9	39.41	2.93	115.5									DP D17 + KO-D23 EXISTING 18" CMP CULVERT UNDER ORR

APPENDIX C

HYDRAULIC COMPUTATIONS

HY-8 Culvert Analysis Report

Table 1 - Project Headwater Table

Crossing Name	Culvert Name	Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	HW / D (ft)	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Outlet Velocity (ft/s)
DP D10	DP D10	55.10	55.10	7117.94	3.18	0.0*	1.27	0.95	1.79	1.03	14.40
DP D11	DP D11	13.30	13.30	7118.01	3.83	0.0*	2.55	0.74	1.36	0.76	14.74
DP D12	DP D12	6.50	6.50	7145.30	1.64	0.0*	1.09	0.53	0.99	0.54	11.24
DP D15	DP D15	2.00	2.00	7123.68	0.83	0.375	0.66	0.55	0.56	0.55	3.80

* Full Flow Headwater elevation is below inlet invert.

Crossing Input: DP D10

Parameter	Value	Units
DISCHARGE DATA		
Discharge Method	Minimum, Design, and Maximum	
Minimum Flow	15.200	cfs
Design Flow	55.100	cfs
Maximum Flow	55.100	cfs
TAILWATER DATA		
Channel Type	Enter Constant Tailwater Elevation	
Channel Invert Elevation	7111.410	ft
Constant Tailwater Elevation	7111.410	ft
Rating Curve	View...	
ROADWAY DATA		
Roadway Profile Shape	Constant Roadway Elevation	
First Roadway Station	1000.000	ft
Crest Length	50.000	ft
Crest Elevation	7119.620	ft
Roadway Surface	Paved	
Top Width	36.000	ft

Culvert Input: DP D10

Parameter	Value	Units
CULVERT DATA		
Name	DP D10	
Shape	Circular	
Material	Concrete	
Diameter	2.500	ft
Embedment Depth	0.000	in
Manning's n	0.012	
Culvert Type	Straight	
Inlet Configuration	Mitered to Conform to Slope (Ke=0.7)	
Inlet Depression?	No	
SITE DATA		
Site Data Input Option	Culvert Invert Data	
Inlet Station	1000.000	ft
Inlet Elevation	7114.760	ft
Outlet Station	1081.430	ft
Outlet Elevation	7111.410	ft
Number of Barrels	2	
Computed Culvert Slope	0.041140	ft/ft

Table 2 - Culvert Summary Table: DP D10

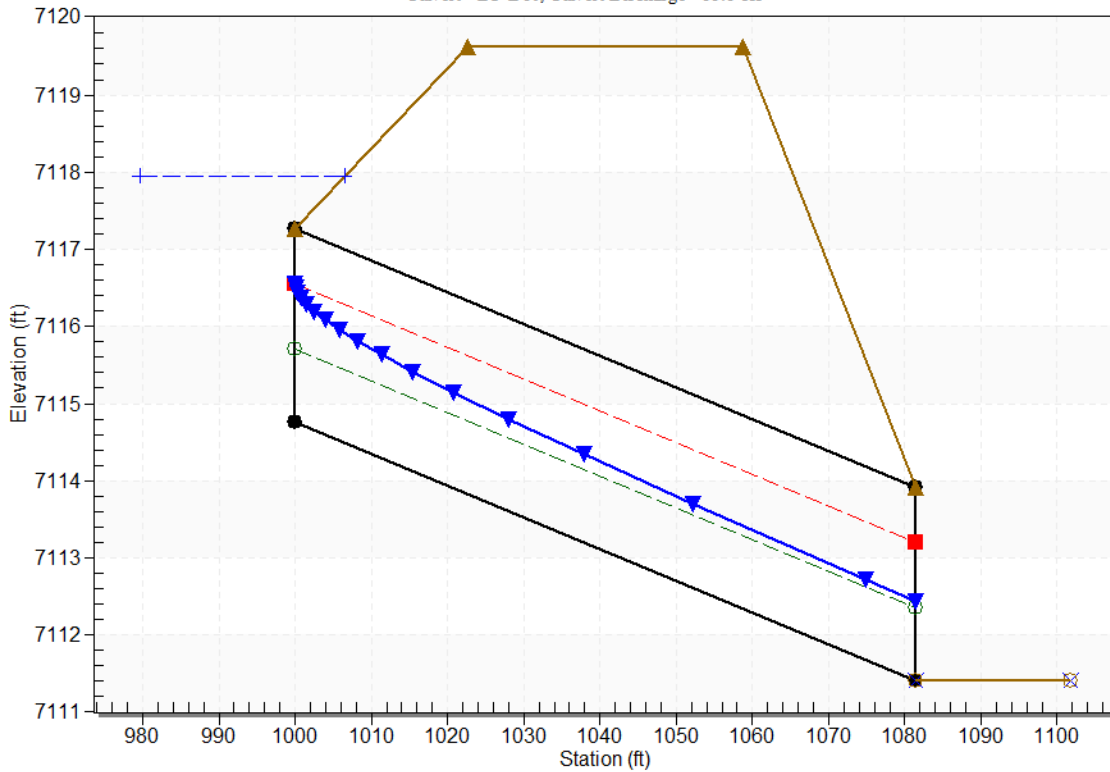
Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	HW / D (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
15.20	15.20	7116.15	1.39	0.0*	0.56	1-S2n	0.49	0.92	0.49	0.00	11.20	0.00
19.19	19.19	7116.34	1.58	0.0*	0.63	1-S2n	0.55	1.03	0.58	0.00	11.12	0.00
23.18	23.18	7116.50	1.74	0.0*	0.70	1-S2n	0.60	1.14	0.64	0.00	11.77	0.00
27.17	27.17	7116.66	1.90	0.0*	0.76	1-S2n	0.65	1.24	0.69	0.00	12.36	0.00
31.16	31.16	7116.82	2.06	0.0*	0.82	1-S2n	0.70	1.33	0.74	0.00	12.82	0.00
35.15	35.15	7116.98	2.22	0.0*	0.89	1-S2n	0.75	1.42	0.80	0.00	13.05	0.00
39.14	39.14	7117.14	2.38	0.0*	0.95	1-S2n	0.79	1.50	0.84	0.00	13.41	0.00
43.13	43.13	7117.32	2.56	0.0*	1.02	5-S2n	0.83	1.58	0.89	0.00	13.69	0.00
47.12	47.12	7117.51	2.75	0.0*	1.10	5-S2n	0.87	1.65	0.94	0.00	13.93	0.00
51.11	51.11	7117.72	2.96	0.0*	1.18	5-S2n	0.91	1.72	0.99	0.00	14.18	0.00
55.10	55.10	7117.94	3.18	0.0*	1.27	5-S2n	0.95	1.79	1.03	0.00	14.40	0.00
85.15	80.03	7119.73	4.97	1.376	1.99	5-S2n	1.17	2.13	1.30	0.00	15.54	0.00

* Full Flow Headwater elevation is below inlet invert.

Water Surface Profile Plot for Culvert: DP D10

Crossing - DP D10, Design Discharge - 55.1 cfs

Culvert - DP D10, Culvert Discharge - 55.1 cfs



Crossing Input: DP D11

Parameter	Value	Units
DISCHARGE DATA		
Discharge Method	Minimum, Design, and Maximum	
Minimum Flow	2.200	cfs
Design Flow	13.300	cfs
Maximum Flow	13.300	cfs
TAILWATER DATA		
Channel Type	Enter Constant Tailwater Elevation	
Channel Invert Elevation	7109.060	ft
Constant Tailwater Elevation	7109.060	ft
Rating Curve	View...	
ROADWAY DATA		
Roadway Profile Shape	Constant Roadway Elevation	
First Roadway Station	1000.000	ft
Crest Length	50.000	ft
Crest Elevation	7120.330	ft
Roadway Surface	Paved	
Top Width	36.000	ft

Culvert Input: DP D11

Parameter	Value	Units
CULVERT DATA		
Name	DP D11	
Shape	Circular	
Material	Concrete	
Diameter	1.500	ft
Embedment Depth	0.000	in
Manning's n	0.012	
Culvert Type	Straight	
Inlet Configuration	Mitered to Conform to Slope (Ke=0.7)	
Inlet Depression?	No	
SITE DATA		
Site Data Input Option	Culvert Invert Data	
Inlet Station	1000.000	ft
Inlet Elevation	7114.180	ft
Outlet Station	1088.040	ft
Outlet Elevation	7109.060	ft
Number of Barrels	1	
Computed Culvert Slope	0.058155	ft/ft

Table 3 - Culvert Summary Table: DP D11

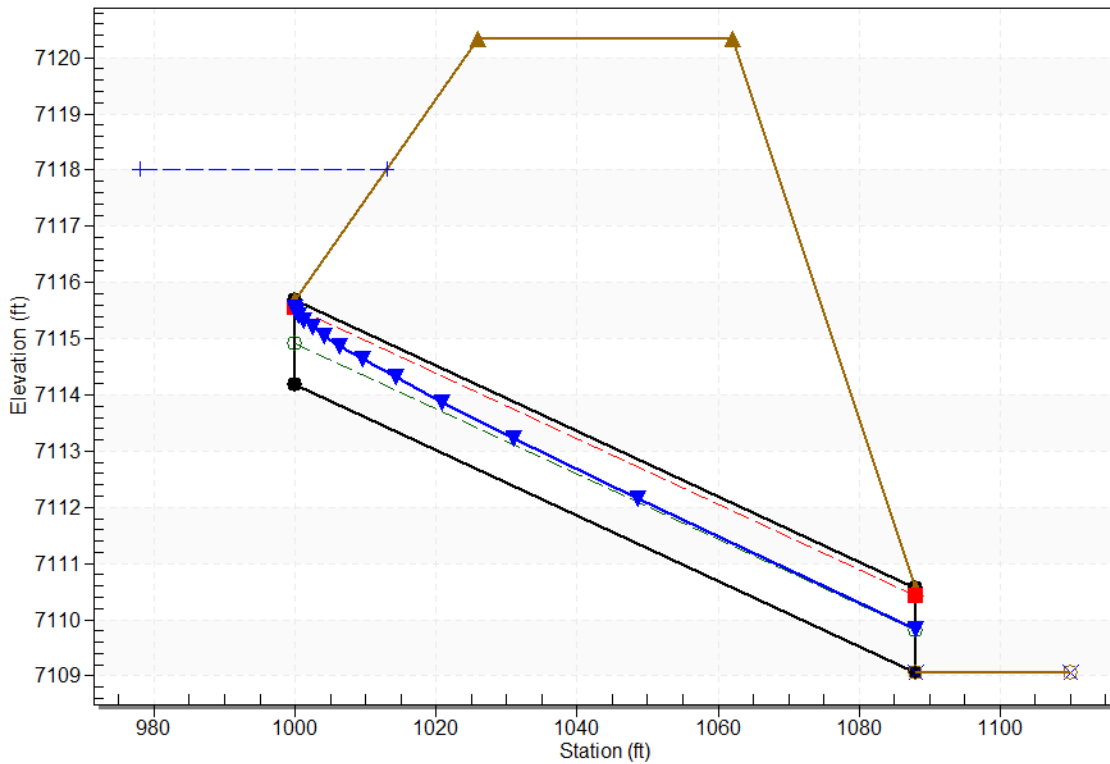
Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	HW / D (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
2.20	2.20	7115.05	0.87	0.0*	0.58	1-S2n	0.29	0.56	0.29	0.00	9.34	0.00
3.31	3.31	7115.26	1.08	0.0*	0.72	1-S2n	0.35	0.69	0.35	0.00	10.51	0.00
4.42	4.42	7115.45	1.27	0.0*	0.84	1-S2n	0.41	0.81	0.41	0.00	11.43	0.00
5.53	5.53	7115.64	1.46	0.0*	0.97	1-S2n	0.46	0.91	0.46	0.00	12.17	0.00
6.64	6.64	7115.86	1.68	0.0*	1.12	5-S2n	0.50	1.00	0.52	0.00	12.33	0.00
7.75	7.75	7116.12	1.94	0.0*	1.29	5-S2n	0.54	1.08	0.57	0.00	12.52	0.00
8.86	8.86	7116.42	2.24	0.0*	1.50	5-S2n	0.59	1.15	0.59	0.00	13.87	0.00
9.97	9.97	7116.77	2.59	0.0*	1.73	5-S2n	0.62	1.22	0.65	0.00	13.48	0.00
11.08	11.08	7117.15	2.97	0.0*	1.98	5-S2n	0.66	1.27	0.69	0.00	14.05	0.00
12.19	12.19	7117.56	3.38	0.0*	2.26	5-S2n	0.70	1.32	0.72	0.00	14.49	0.00
13.30	13.30	7118.01	3.83	0.0*	2.55	5-S2n	0.74	1.36	0.76	0.00	14.74	0.00
19.35	17.96	7120.37	6.19	1.247	4.13	5-S2n	0.88	1.41	0.93	0.00	15.58	0.00

* Full Flow Headwater elevation is below inlet invert.

Water Surface Profile Plot for Culvert: DP D11

Crossing - DP D11, Design Discharge - 13.3 cfs

Culvert - DP D11, Culvert Discharge - 13.3 cfs



Crossing Input: DP D12

Parameter	Value	Units
DISCHARGE DATA		
Discharge Method	Minimum, Design, and Maximum	
Minimum Flow	2.100	cfs
Design Flow	6.500	cfs
Maximum Flow	6.500	cfs
TAILWATER DATA		
Channel Type	Enter Constant Tailwater Elevation	
Channel Invert Elevation	7140.430	ft
Constant Tailwater Elevation	7140.430	ft
Rating Curve	View...	
ROADWAY DATA		
Roadway Profile Shape	Constant Roadway Elevation	
First Roadway Station	1036.580	ft
Crest Length	50.000	ft
Crest Elevation	7146.630	ft
Roadway Surface	Paved	
Top Width	36.000	ft

Culvert Input: DP D12

Parameter	Value	Units
CULVERT DATA		
Name	DP D12	
Shape	Circular	
Material	Concrete	
Diameter	1.500	ft
Embedment Depth	0.000	in
Manning's n	0.012	
Culvert Type	Straight	
Inlet Configuration	Mitered to Conform to Slope (Ke=0.7)	
Inlet Depression?	No	
SITE DATA		
Site Data Input Option	Culvert Invert Data	
Inlet Station	1025.990	ft
Inlet Elevation	7143.660	ft
Outlet Station	1096.400	ft
Outlet Elevation	7140.430	ft
Number of Barrels	1	
Computed Culvert Slope	0.045874	ft/ft

Table 4 - Culvert Summary Table: DP D12

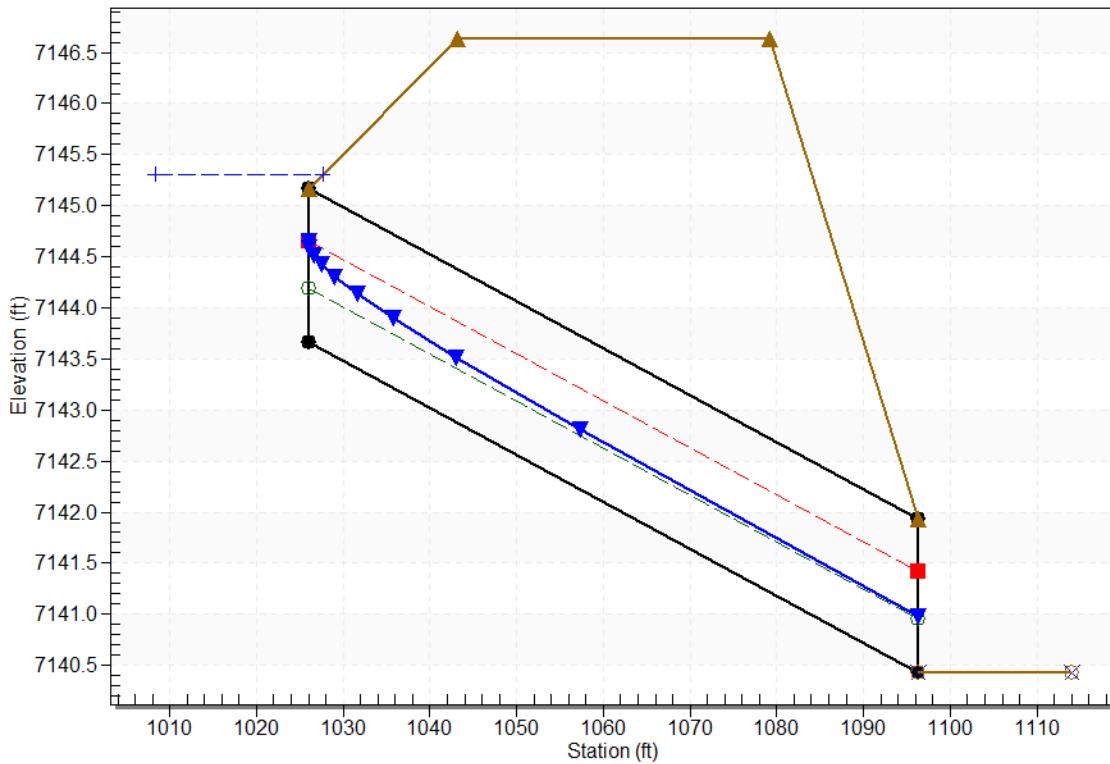
Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	HW / D (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
2.10	2.10	7144.49	0.83	0.0*	0.56	1-S2n	0.30	0.55	0.30	0.00	8.47	0.00
2.54	2.54	7144.58	0.92	0.0*	0.62	1-S2n	0.33	0.60	0.33	0.00	8.95	0.00
2.98	2.98	7144.67	1.01	0.0*	0.67	1-S2n	0.35	0.66	0.35	0.00	9.37	0.00
3.42	3.42	7144.74	1.08	0.0*	0.72	1-S2n	0.38	0.71	0.38	0.00	9.75	0.00
3.86	3.86	7144.82	1.16	0.0*	0.77	1-S2n	0.40	0.75	0.41	0.00	9.77	0.00
4.30	4.30	7144.89	1.23	0.0*	0.82	1-S2n	0.43	0.79	0.43	0.00	10.41	0.00
4.74	4.74	7144.97	1.31	0.0*	0.87	1-S2n	0.45	0.84	0.46	0.00	10.34	0.00
5.18	5.18	7145.05	1.39	0.0*	0.92	1-S2n	0.47	0.88	0.47	0.00	10.97	0.00
5.62	5.62	7145.13	1.47	0.0*	0.98	1-S2n	0.49	0.91	0.49	0.00	11.23	0.00
6.06	6.06	7145.21	1.55	0.0*	1.03	5-S2n	0.51	0.95	0.53	0.00	10.72	0.00
6.50	6.50	7145.30	1.64	0.0*	1.09	5-S2n	0.53	0.99	0.54	0.00	11.24	0.00
11.82	11.19	7146.66	3.00	0.0*	2.00	5-S2n	0.71	1.28	0.75	0.00	12.56	0.00

* Full Flow Headwater elevation is below inlet invert.

Water Surface Profile Plot for Culvert: DP D12

Crossing - DP D12, Design Discharge - 6.5 cfs

Culvert - DP D12, Culvert Discharge - 6.5 cfs



Crossing Input: DP D15

Parameter	Value	Units
DISCHARGE DATA		
Discharge Method	Minimum, Design, and Maximum	
Minimum Flow	0.500	cfs
Design Flow	2.000	cfs
Maximum Flow	2.000	cfs
TAILWATER DATA		
Channel Type	Enter Constant Tailwater Elevation	
Channel Invert Elevation	7122.540	ft
Constant Tailwater Elevation	7122.540	ft
Rating Curve	View...	
ROADWAY DATA		
Roadway Profile Shape	Constant Roadway Elevation	
First Roadway Station	1000.000	ft
Crest Length	50.000	ft
Crest Elevation	7126.630	ft
Roadway Surface	Paved	
Top Width	36.000	ft

Culvert Input: DP D15

Parameter	Value	Units
CULVERT DATA		
Name	DP D15	
Shape	Circular	
Material	Concrete	
Diameter	1.250	ft
Embedment Depth	0.000	in
Manning's n	0.012	
Culvert Type	Straight	
Inlet Configuration	Mitered to Conform to Slope (Ke=0.7)	
Inlet Depression?	No	
SITE DATA		
Site Data Input Option	Culvert Invert Data	
Inlet Station	1000.000	ft
Inlet Elevation	7122.850	ft
Outlet Station	1063.040	ft
Outlet Elevation	7122.540	ft
Number of Barrels	1	
Computed Culvert Slope	0.004918	ft/ft

Table 5 - Culvert Summary Table: DP D15

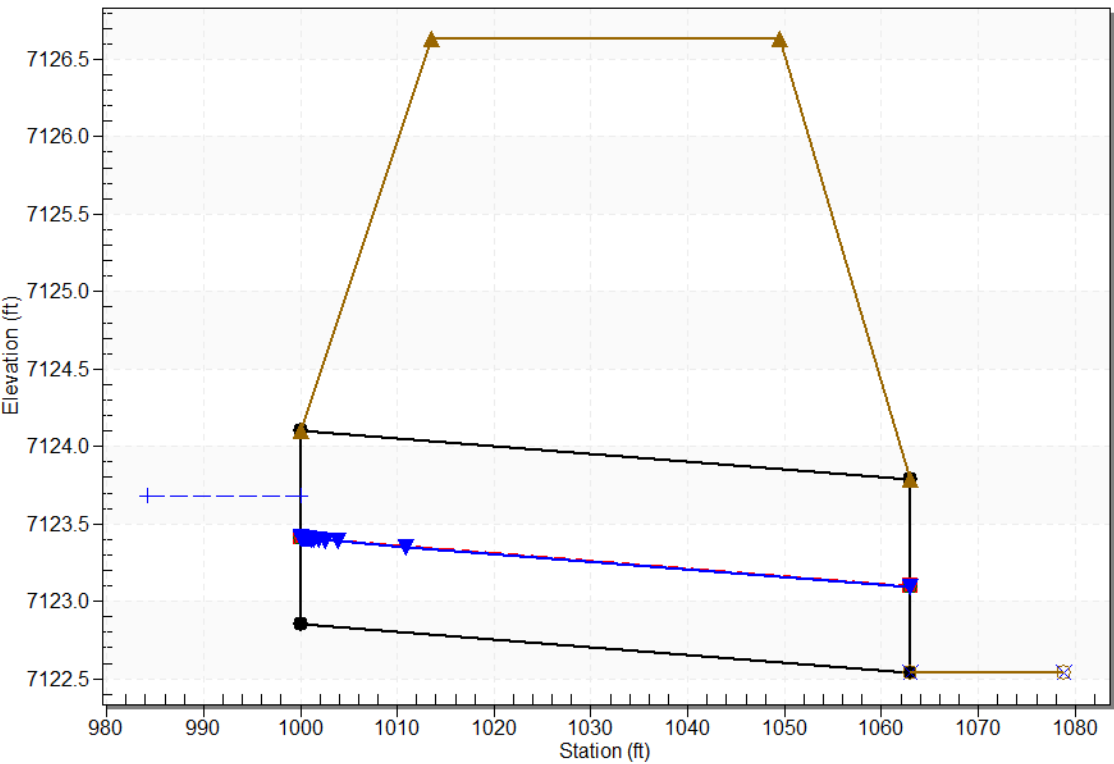
Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	HW / D (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
0.50	0.50	7123.24	0.39	0.0*	0.31	1-S2n	0.27	0.28	0.27	0.00	2.58	0.00
0.65	0.65	7123.30	0.45	0.018	0.36	1-S2n	0.31	0.31	0.31	0.00	2.78	0.00
0.80	0.80	7123.35	0.50	0.060	0.40	1-S2n	0.34	0.35	0.34	0.00	2.95	0.00
0.95	0.95	7123.40	0.55	0.100	0.44	1-S2n	0.37	0.38	0.37	0.00	3.10	0.00
1.10	1.10	7123.44	0.59	0.139	0.47	1-S2n	0.40	0.41	0.40	0.00	3.23	0.00
1.25	1.25	7123.49	0.64	0.178	0.51	1-S2n	0.43	0.44	0.43	0.00	3.35	0.00
1.40	1.40	7123.53	0.68	0.217	0.54	1-S2n	0.46	0.47	0.46	0.00	3.45	0.00
1.55	1.55	7123.57	0.72	0.256	0.58	1-S2n	0.48	0.49	0.48	0.00	3.55	0.00
1.70	1.70	7123.61	0.76	0.295	0.61	1-S2n	0.51	0.52	0.51	0.00	3.64	0.00
1.85	1.85	7123.64	0.79	0.335	0.64	1-S2n	0.53	0.54	0.53	0.00	3.72	0.00
2.00	2.00	7123.68	0.83	0.375	0.66	1-S2n	0.55	0.56	0.55	0.00	3.80	0.00
9.82	9.60	7126.64	3.79	3.703	3.03	7-M2c	1.25	1.17	1.17	0.00	8.02	0.00

* Full Flow Headwater elevation is below inlet invert.

Water Surface Profile Plot for Culvert: DP D15

Crossing - DP D15, Design Discharge - 2.0 cfs

Culvert - DP D15, Culvert Discharge - 2.0 cfs



HY-8 Culvert Analysis Report

Crossing Discharge Data

Discharge Selection Method: User Defined

Table 1 - Summary of Culvert Flows at crossing: DP D1 (Ex. Old Ranch Rd.)

Headwater Elevation (ft)	Discharge Names	Total Discharge (cfs)	DP D1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
7076.74	Ex. Minor	26.10	13.51	12.59	10
7076.83	Pr. Minor	30.60	13.61	16.98	6
7078.00	Ex. Major	110.10	14.79	95.31	5
7078.06	Pr. Major	115.10	14.85	100.24	3
7076.29	Overtopping	13.04	13.04	0.00	Overtopping

Table 2 - Culvert Summary Table: DP D1

Discharge Names	Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	HW / D (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
Ex. Minor	26.10	13.51	7076.74	4.09	4.765	3.18	7-M2c	1.50	1.37	1.37	0.21	7.99	2.75
Pr. Minor	30.60	13.61	7076.83	4.14	4.863	3.24	7-M2c	1.50	1.37	1.37	0.23	8.04	2.92
Ex. Major	110.10	14.79	7078.00	4.75	6.034	4.02	7-M2c	1.50	1.40	1.40	0.49	8.62	4.68
Pr. Major	115.10	14.85	7078.06	4.78	6.093	4.06	7-M2c	1.50	1.40	1.40	0.51	8.64	4.76
Overtopping	125.00	14.96	7078.18	4.84	6.207	4.14	7-M2c	1.50	1.40	1.40	0.53	8.70	4.90

Culvert Barrel Data

Culvert Barrel Type: Straight Culvert
Inlet Elevation(invert): 7071.97 ft
Outlet Elevation (invert): 7069.06 ft
Culvert Length: 80.64 ft
Culvert Slope: 0.04 ft/ft

Site Data - DP D1

Site Data Option: Culvert Invert Data
Inlet Station: 100.00 ft
Inlet Elevation: 7071.97 ft
Outlet Station: 180.59 ft
Outlet Elevation: 7069.06 ft
Number of Barrels: 1

Culvert Data Summary - DP D1

Barrel Shape: Circular
Barrel Diameter: 1.50 ft
Barrel Material: Corrugated Steel
Embedment: 0.00 in
Barrel Manning's n: 0.0240
Culvert Type: Straight
Inlet Configuration: Thin Edge Projecting
Inlet Depression: None

Roadway Data for crossing: DP D1 (Ex. Old Ranch Rd.)

Roadway Profile Shape: Constant Roadway Elevation
Crest Length: 101.00 ft
Crest Elevation: 7076.29 ft
Roadway Surface: Paved
Roadway Top Width: 30.00 ft

Crossing Discharge Data

Discharge Selection Method: User Defined

Table 3 - Summary of Culvert Flows at crossing: DP D11 / D17 (Studebaker)

Headwater Elevation (ft)	Discharge Names	Total Discharge (cfs)	DP D11 / D17 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
7093.34	Ex. Minor	18.20	8.82	9.33	11
7093.38	Pr. Minor	22.80	8.96	13.81	6
7093.73	Ex. Major	81.20	10.09	71.09	4
7093.76	Pr. Major	86.60	10.17	76.42	3
7093.20	Overtopping	8.32	8.32	0.00	Overtopping

Table 4 - Culvert Summary Table: DP D11 / D17

Discharge Names	Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	HW / D (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
Ex. Minor	18.20	8.82	7093.34	2.23	0.092	1.49	5-S2n	0.88	1.15	0.88	0.00	8.15	0.00
Pr. Minor	22.80	8.96	7093.38	2.27	0.163	1.51	5-S2n	0.89	1.16	0.89	0.00	8.18	0.00
Ex. Major	81.20	10.09	7093.73	2.62	0.764	1.75	5-S2n	0.97	1.22	0.97	0.00	8.39	0.00
Pr. Major	86.60	10.17	7093.76	2.65	0.809	1.77	5-S2n	0.97	1.23	0.97	0.00	8.40	0.00
Overtopping	100.00	10.35	7093.82	2.71	0.915	1.81	5-S2n	0.98	1.24	0.98	0.00	8.43	0.00

Culvert Barrel Data

Culvert Barrel Type: Straight Culvert
Inlet Elevation(invert): 7091.11 ft
Outlet Elevation (invert): 7087.82 ft
Culvert Length: 58.59 ft
Culvert Slope: 0.06 ft/ft

Site Data - DP D11 / D17

Site Data Option: Culvert Invert Data
Inlet Station: 0.00 ft
Inlet Elevation: 7091.11 ft
Outlet Station: 58.50 ft
Outlet Elevation: 7087.82 ft
Number of Barrels: 1

Culvert Data Summary - DP D11 / D17

Barrel Shape: Circular
Barrel Diameter: 1.50 ft
Barrel Material: Corrugated Steel
Embedment: 0.00 in
Barrel Manning's n: 0.0240
Culvert Type: Straight
Inlet Configuration: Mitered to Conform to Slope
Inlet Depression: None

Roadway Data for crossing: DP D11 / D17 (Studebaker)

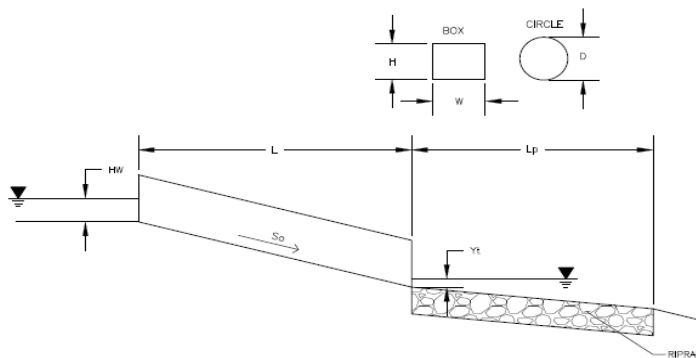
Roadway Profile Shape: Constant Roadway Elevation
Crest Length: 75.00 ft
Crest Elevation: 7093.20 ft
Roadway Surface: Paved
Roadway Top Width: 30.00 ft

DETERMINATION OF CULVERT HEADWATER AND OUTLET PROTECTION

MHFD-Culvert, Version 4.01 (April 2025)

Project: **Elk View Estates**

ID: **DP D10 Culvert**



Soil Type:

Choose One:

☒ Sandy

☐ Non-Sandy

Supercritical Flow! Using Adjusted Diameter to calculate protection type.

Design Information:

Design Discharge

Q = 55.1 cfs

Circular Culvert:

Barrel Diameter in Inches

D = 30 inches

Inlet Edge Type (Choose from pull-down list)

Square Edge Projecting

OR:

Box Culvert:

Barrel Height (Rise) in Feet

H (Rise) =

Barrel Width (Span) in Feet

W (Span) =

Inlet Edge Type (Choose from pull-down list)

Number of Barrels

Barrels = 2

Inlet Elevation

Elev IN = 7114.76 ft

Outlet Elevation **OR** Slope

Elev OUT = 7111.41 ft

Culvert Length

L = 81.5 ft

Manning's Roughness

n = 0.012

Bend Loss Coefficient

k_b = 0

Exit Loss Coefficient

k_x = 1

Tailwater Surface Elevation

Y_t Elevation =

Max Allowable Channel Velocity

V = 5 ft/s

Calculated Results:

Culvert Cross Sectional Area Available

A = 4.91 ft²

Culvert Normal Depth

Y_n = 0.95 ft

Culvert Critical Depth

Y_c = 1.79 ft

Froude Number

Fr = 3.36 **Supercritical!**

Entrance Loss Coefficient

k_e = 0.20

Friction Loss Coefficient

k_f = 0.64

Sum of All Loss Coefficients

k_s = 1.84 ft

Headwater:

Inlet Control Headwater

HW_I = 2.93 ft

Outlet Control Headwater

HW_O = N/A ft

Design Headwater Elevation

HW = 7117.69 ft

Headwater/Diameter OR Headwater/Rise Ratio

HW/D = 1.17

Outlet Control Headwater Approximation Method Inaccurate for Low Flow - Backwater Calculations Required

Outlet Protection:

Flow/(Diameter^{2.5})

Q/D^{2.5} = 2.79 ft^{0.5}/s

Tailwater Surface Height

Y_t = 1.00 ft

Tailwater/Diameter

Y_t/D = 0.40

Expansion Factor

1/(2*tan(θ)) = 4.67

Flow Area at Max Channel Velocity

A_t = 11.02 ft²

Width of Equivalent Conduit for Multiple Barrels

W_{eq} = 5.00 ft

Length of Riprap Protection

L_p = 25 ft

Width of Riprap Protection at Downstream End

T = 11 ft

Adjusted Diameter for Supercritical Flow

Da = 1.73 ft

Minimum Theoretical Riprap Size

d₅₀ min = 6 in

Nominal Riprap Size

d₅₀ nominal = 9 in

MHFD Riprap Type

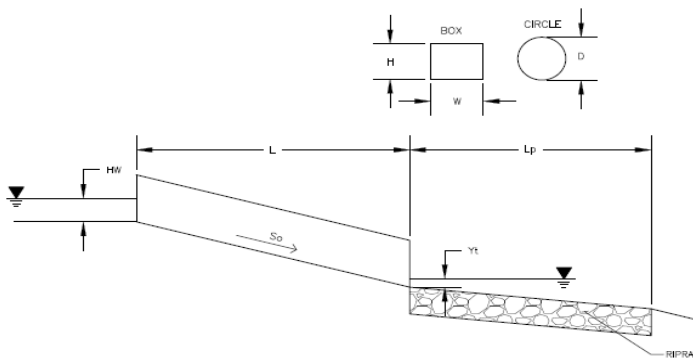
Type = L

DETERMINATION OF CULVERT HEADWATER AND OUTLET PROTECTION

MHFD-Culvert, Version 4.01 (April 2025)

Project: **Elk View Estates**

ID: **DP D11 Culvert**



Soil Type:

Choose One:

☒ Sandy

☐ Non-Sandy

Supercritical Flow! Using Adjusted Diameter to calculate protection type.

Design Information:

Design Discharge

Q = 13.3 cfs

Circular Culvert:

Barrel Diameter in Inches

D = 18 inches

Inlet Edge Type (Choose from pull-down list)

Square Edge Projecting

OR:

Box Culvert:

Barrel Height (Rise) in Feet

H (Rise) =

Barrel Width (Span) in Feet

W (Span) =

Inlet Edge Type (Choose from pull-down list)

Number of Barrels

Barrels = 1

Inlet Elevation

Elev IN = 7114.18 ft

Outlet Elevation **OR** Slope

Elev OUT = 7109.06 ft

Culvert Length

L = 88.1 ft

Manning's Roughness

n = 0.012

Bend Loss Coefficient

k_b = 0

Exit Loss Coefficient

k_x = 1

Tailwater Surface Elevation

Y_t Elevation =

Max Allowable Channel Velocity

V = 5 ft/s

Calculated Results:

Culvert Cross Sectional Area Available

A = 1.77 ft²

Culvert Normal Depth

Y_n = 0.80 ft

Culvert Critical Depth

Y_c = 1.36 ft

Froude Number

Fr = 3.07 **Supercritical!**

Entrance Loss Coefficient

k_e = 0.20

Friction Loss Coefficient

k_f = 1.36

Sum of All Loss Coefficients

k_s = 2.56 ft

Headwater:

Inlet Control Headwater

HW_I = 3.25 ft

Outlet Control Headwater

HW_O = N/A ft

Design Headwater Elevation

HW = 7117.43 ft

Headwater/Diameter OR Headwater/Rise Ratio

HW/D = 2.17

Outlet Control Headwater Approximation Method Inaccurate for Low Flow - Backwater Calculations Required

Outlet Protection:

Flow/(Diameter^{2.5})

Q/D^{2.5} = 4.83 ft^{0.5}/s

Tailwater Surface Height

Y_t = 0.60 ft

Tailwater/Diameter

Y_t/D = 0.40

Expansion Factor

1/(2*tan(θ)) = 2.60

Flow Area at Max Channel Velocity

A_t = 2.66 ft²

Width of Equivalent Conduit for Multiple Barrels

W_{eq} = - ft

Length of Riprap Protection

L_p = 8 ft

Width of Riprap Protection at Downstream End

T = 5 ft

Adjusted Diameter for Supercritical Flow

Da = 1.15 ft

Minimum Theoretical Riprap Size

d₅₀ min = 6 in

Nominal Riprap Size

d₅₀ nominal = 9 in

MHFD Riprap Type

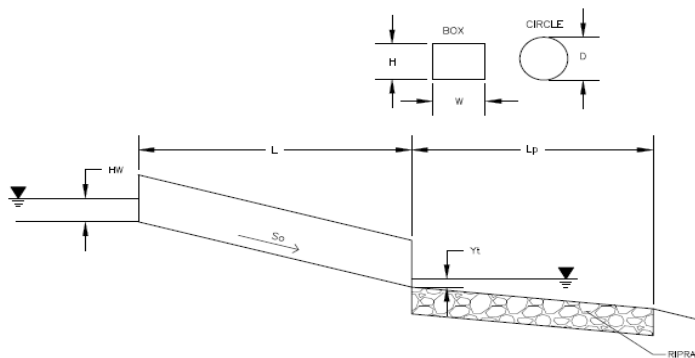
Type = L

DETERMINATION OF CULVERT HEADWATER AND OUTLET PROTECTION

MHFD-Culvert, Version 4.01 (April 2025)

Project: **Elk View Estates**

ID: **DP D12 Culvert**



Soil Type:

Choose One:

☒ Sandy

☐ Non-Sandy

Supercritical Flow! Using Adjusted Diameter to calculate protection type.

Design Information:

Design Discharge

Q = 6.5 cfs

Circular Culvert:

Barrel Diameter in Inches

D = 18 inches

Inlet Edge Type (Choose from pull-down list)

Square Edge Projecting

OR:

Box Culvert:

Barrel Height (Rise) in Feet

H (Rise) =

Barrel Width (Span) in Feet

W (Span) =

Inlet Edge Type (Choose from pull-down list)

Number of Barrels

Barrels = 1

Inlet Elevation

Elev IN = 7143.66 ft

Outlet Elevation **OR** Slope

So = 0.0459 ft/ft

Culvert Length

L = 70.4 ft

Manning's Roughness

n = 0.012

Bend Loss Coefficient

k_b = 0

Exit Loss Coefficient

k_x = 1

Tailwater Surface Elevation

Y_t Elevation =

Max Allowable Channel Velocity

V = 5 ft/s

Calculated Results:

Culvert Cross Sectional Area Available

A = 1.77 ft²

Culvert Normal Depth

Y_n = 0.53 ft

Culvert Critical Depth

Y_c = 0.99 ft

Froude Number

Fr = 3.31 **Supercritical!**

Entrance Loss Coefficient

k_e = 0.20

Friction Loss Coefficient

k_f = 1.09

Sum of All Loss Coefficients

k_s = 2.29

Headwater:

Inlet Control Headwater

HW_I = 1.53 ft

Outlet Control Headwater

HW_O = N/A

Design Headwater Elevation

HW = 7145.19 ft

Headwater/Diameter **OR Headwater/Rise Ratio**

HW/D = 1.02

Outlet Control Headwater Approximation Method Inaccurate for Low Flow - Backwater Calculations Required

Outlet Protection:

Flow/(Diameter^{2.5})

Q/D^{2.5} = 2.36 ft^{0.5}/s

Tailwater Surface Height

Y_t = 0.60 ft

Tailwater/Diameter

Y_t/D = 0.40

Expansion Factor

1/(2*tan(θ)) = 5.25

Flow Area at Max Channel Velocity

A_t = 1.30 ft²

Width of Equivalent Conduit for Multiple Barrels

W_{eq} = - ft

Length of Riprap Protection

L_p = 5 ft

Width of Riprap Protection at Downstream End

T = 3 ft

Adjusted Diameter for Supercritical Flow

Da = 1.01 ft

Minimum Theoretical Riprap Size

d₅₀ min = 3 in

Nominal Riprap Size

d₅₀ nominal = 6 in

MHFD Riprap Type

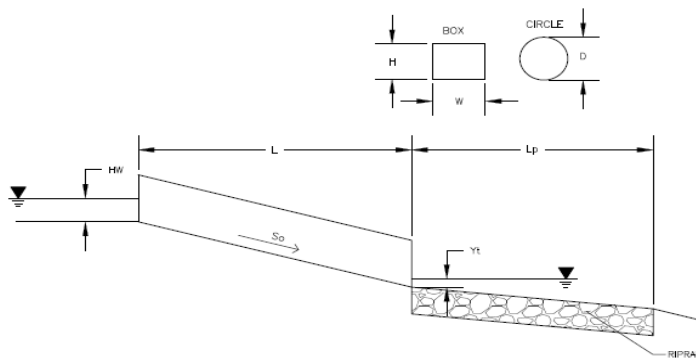
Type = VL

DETERMINATION OF CULVERT HEADWATER AND OUTLET PROTECTION

MHFD-Culvert, Version 4.01 (April 2025)

Project: **Elk View Estates**

ID: **DP D15 Culvert**



Soil Type:

Choose One:

☒ Sandy

☐ Non-Sandy

Supercritical Flow! Using Adjusted Diameter to calculate protection type.

Design Information:

Design Discharge

Q = 2 cfs

Circular Culvert:

Barrel Diameter in Inches

D = 15 inches

Inlet Edge Type (Choose from pull-down list)

Square Edge Projecting

OR:

Box Culvert:

Barrel Height (Rise) in Feet

H (Rise) =

Barrel Width (Span) in Feet

W (Span) =

Inlet Edge Type (Choose from pull-down list)

Number of Barrels

Barrels = 1

Inlet Elevation

Elev IN = 7122.85 ft

Outlet Elevation OR Slope

Elev OUT = 7122.54 ft

Culvert Length

L = 63 ft

Manning's Roughness

n = 0.012

Bend Loss Coefficient

k_b = 0

Exit Loss Coefficient

k_x = 1

Tailwater Surface Elevation

Y_t Elevation =

Max Allowable Channel Velocity

V = 5 ft/s

Calculated Results:

Culvert Cross Sectional Area Available

A = 1.23 ft²

Culvert Normal Depth

Y_n = 0.30 ft

Culvert Critical Depth

Y_c = 0.49 ft

Froude Number

Fr = 2.68 **Supercritical!**

Entrance Loss Coefficient

k_e = 0.20

Friction Loss Coefficient

k_f = 1.24

Sum of All Loss Coefficients

k_s = 2.44 ft

Headwater:

Inlet Control Headwater

HW_I = 0.67 ft

Outlet Control Headwater

HW_O = N/A ft

Design Headwater Elevation

HW = N/A ft

Headwater/Diameter OR Headwater/Rise Ratio

HW/D = N/A

Outlet Control Headwater Approximation Method Inaccurate for Low Flow - Backwater Calculations Required

Outlet Protection:

Flow/(Diameter^{2.5})

Q/D^{2.5} = 1.14 ft^{0.5}/s

Tailwater Surface Height

Y_t = 0.50 ft

Tailwater/Diameter

Y_t/D = 0.40

Expansion Factor

1/(2*tan(θ)) = 6.56

Flow Area at Max Channel Velocity

A_t = 0.40 ft²

Width of Equivalent Conduit for Multiple Barrels

W_{eq} = - ft

Length of Riprap Protection

L_p = 5 ft

Width of Riprap Protection at Downstream End

T = 3 ft

Adjusted Diameter for Supercritical Flow

Da = 0.77 ft

Minimum Theoretical Riprap Size

d₅₀ min = 1 in

Nominal Riprap Size

d₅₀ nominal = 6 in

MHFD Riprap Type

Type = VL

Hydraulic Analysis Report

Project Data

Project Title: Elk View

Designer:

Project Date: Tuesday, July 28, 2026

Project Units: U.S. Customary Units

Notes:

Channel Analysis: Easement Channel - Proposed Condition

Notes:

Input Parameters

Channel Type: Custom Cross Section

Cross Section Data

Station (ft)	Elevation (ft)	Manning's n
100.00	7101.52	0.0300
168.30	7094.54	0.0300
231.97	7099.55	-----

Longitudinal Slope: 0.0299 ft/ft

Flow 86.6000 cfs

Result Parameters

Depth 1.1438 ft

Area of Flow 14.7140 ft²

Wetted Perimeter 25.8315 ft

Hydraulic Radius 0.5696 ft

Average Velocity 5.8856 ft/s

Top Width 25.7283 ft

Froude Number: 1.3715

Critical Depth 1.2978 ft

Critical Velocity 4.5714 ft/s

Critical Slope: 0.0152 ft/ft

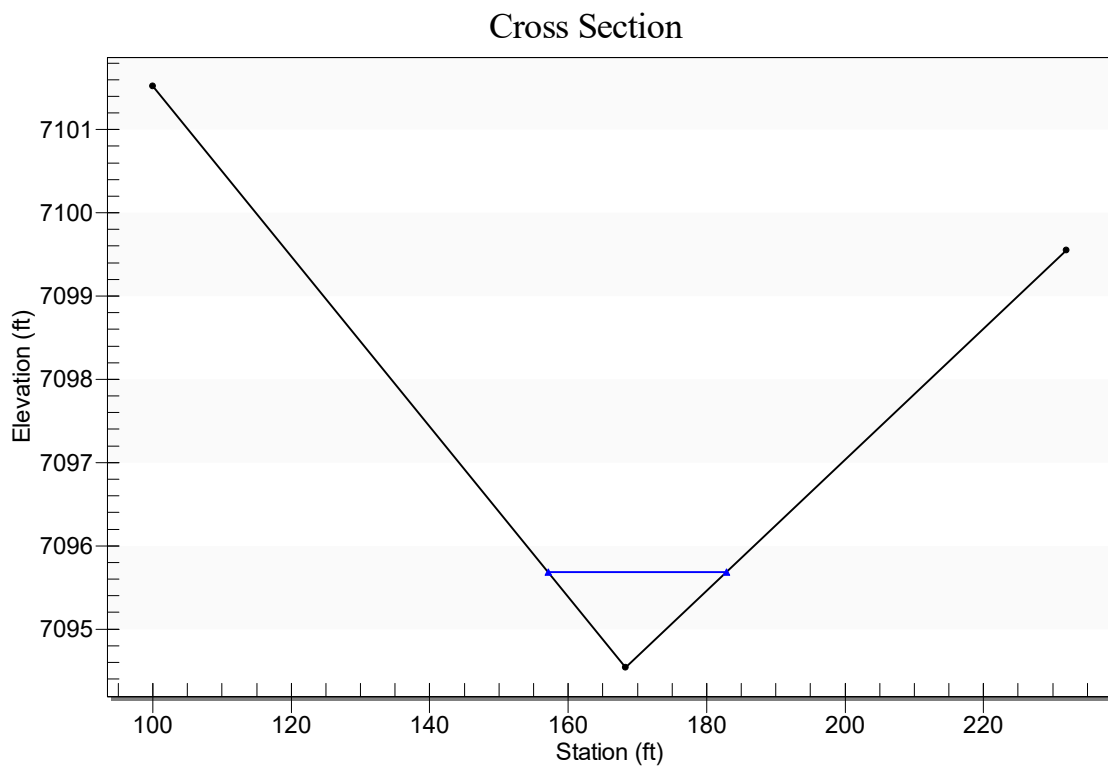
Critical Top Width 29.19 ft

Calculated Max Shear Stress 2.1341 lb/ft²

Calculated Avg Shear Stress 1.0628 lb/ft²

Composite Manning's n Equation: Lotter method

Manning's n: 0.0300



Channel Analysis: Easement Channel - Existing Condition

Notes:

Input Parameters

Channel Type: Custom Cross Section

Cross Section Data

Station (ft)	Elevation (ft)	Manning's n
100.00	7101.52	0.0300
168.30	7094.54	0.0300
231.97	7099.55	-----

Longitudinal Slope: 0.0299 ft/ft

Flow 81.2000 cfs

Result Parameters

Depth 1.1165 ft

Area of Flow 14.0204 ft²

Wetted Perimeter 25.2153 ft

Hydraulic Radius 0.5560 ft

Average Velocity 5.7916 ft/s

Top Width 25.1145 ft

Froude Number: 1.3660

Critical Depth 1.2649 ft

Critical Velocity 4.5128 ft/s

Critical Slope: 0.0154 ft/ft

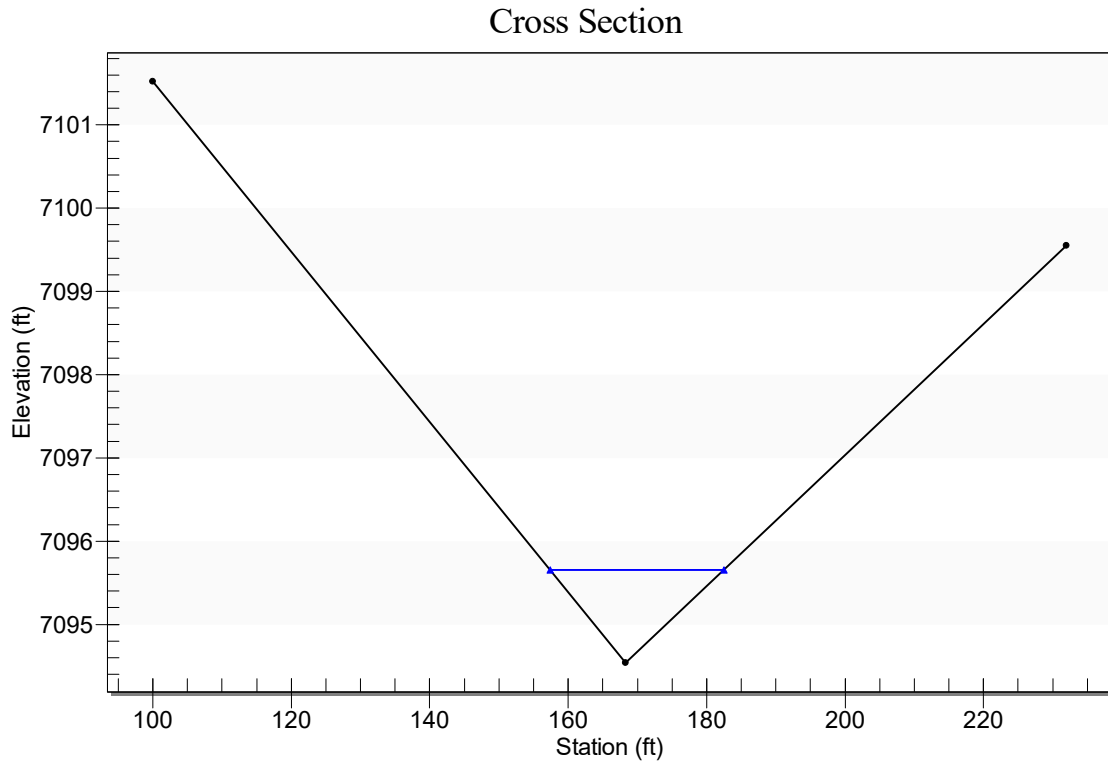
Critical Top Width 28.45 ft

Calculated Max Shear Stress 2.0831 lb/ft²

Calculated Avg Shear Stress 1.0374 lb/ft²

Composite Manning's n Equation: Lotter method

Manning's n: 0.0300



Channel Analysis: Proposed Roadside Ditch - Typical Section

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0138 ft/ft

Manning's n: 0.0500

Flow 2.8000 cfs

Result Parameters

Depth 0.6910 ft

Area of Flow 1.6713 ft²

Wetted Perimeter 5.0344 ft

Hydraulic Radius 0.3320 ft

Average Velocity 1.6753 ft/s

Top Width 4.8372 ft

Froude Number: 0.5023

Critical Depth 0.5268 ft

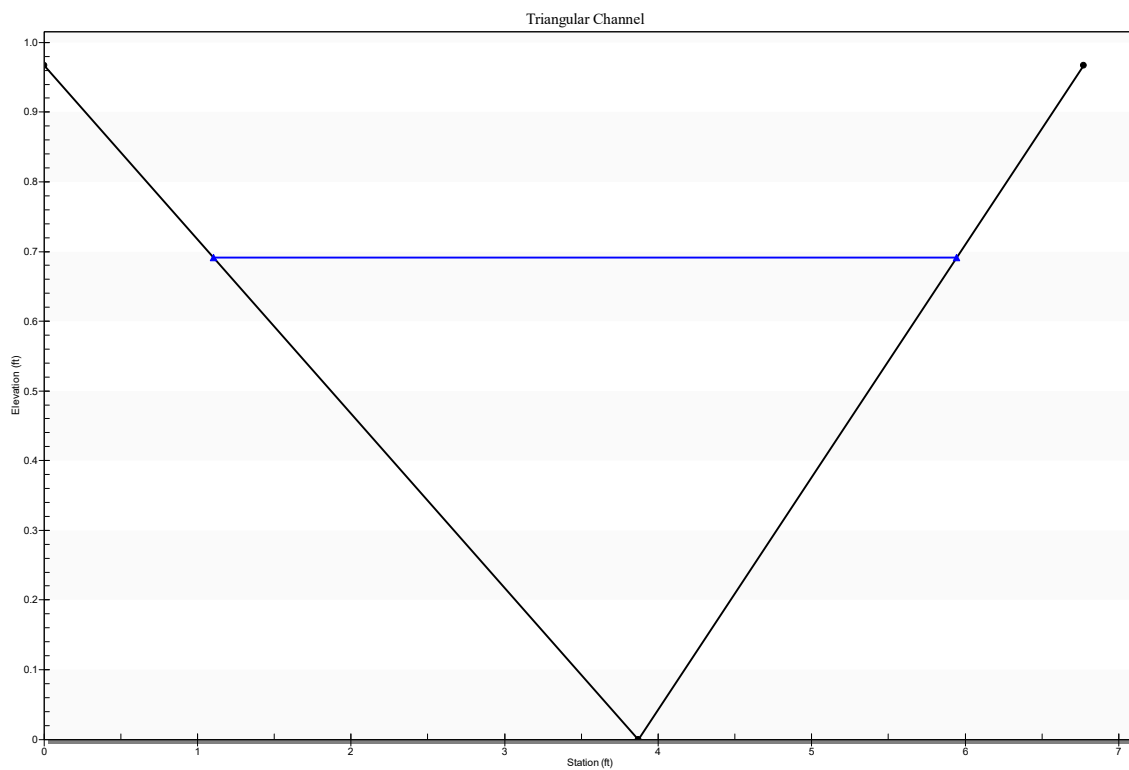
Critical Velocity 2.8825 ft/s

Critical Slope: 0.0587 ft/ft

Critical Top Width 3.76 ft

Calculated Max Shear Stress 0.5951 lb/ft²

Calculated Avg Shear Stress 0.2859 lb/ft²



Channel Lining Analysis: Roadside Ditch Stability

Notes:

Lining Input Parameters

Channel Lining Type: Vegetation

Specific Weight of Water: 62.4 lb/ft³

Height of Vegetation: 0.333 ft

Vegetation Condition is fair

Growth Form of Vegetation is mixed

Cf: 0.7

See HEC-15, Table 4.5 (default: 0.75 for Good cover factor and Mixed growth form)

soil is noncohesive

D75: 2.54 mm

Safety Factor: 1

Lining Results

Cn: 0.142135

Permissible Soil Shear Stress: 0.04 lb/ft²

Mean Boundary Shear Stress: 0.285877 lb/ft²

Maximum Shear Stress on the Channel Bottom: 0.595063 lb/ft²

Manning's n: 0.0499586

Soil Grain Roughness: 0.0177136

Effective Shear Stress: 0.0183107 lb/ft²

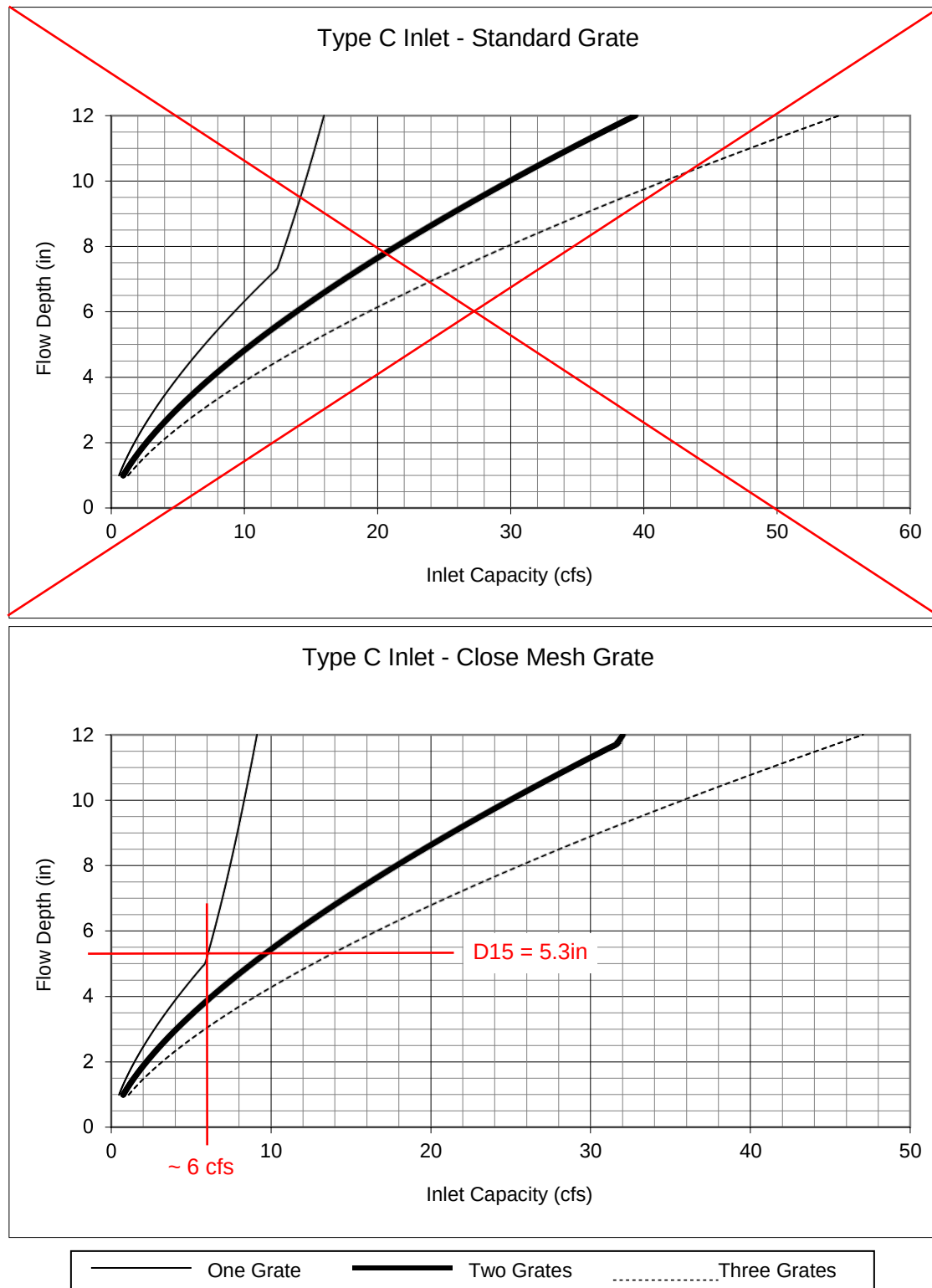
Permissible Shear Stress on Vegetation: 1.06059 lb/ft²

This value is compared with the maximum shear stress times the safety factor to determine lining stability

Channel bottom is stable

Channel Lining Stability Results 2

The channel is stable

Figure 8-10. Inlet Capacity Chart Sump Conditions, Area (Type C) Inlet**Notes:**

1. The standard inlet parameters must apply to use these charts.

APPENDIX D

WATER QUALITY COMPUTATIONS



2880 International Circle, Suite 110
Colorado Springs, CO 80910
Phone: 719-520-6300
Email: Stormwater@elpasoco.com
publicworks.elpasoco.com/stormwater/

EL PASO COUNTY PCM APPLICABILITY FORM

EPC Project Number: _____

This form is to be used by the Engineer of Record to determine if the proposed construction activities are eligible for an exclusion to stormwater quality permanent control measure (PCM) requirements. All “applicable construction activity” within El Paso County (EPC) must comply with the post-construction stormwater management criteria. Reference ECM Appendix I for information about PCMs.

Note that this form only addresses stormwater quality for the site. Even if the site is fully excluded from needing a stormwater quality PCM, the site may still need to address stormwater detention (per DCMv1 Chap 1.5 and ECM Chap 3.2.8.B). However, if the site requires stormwater detention, then it must also address stormwater quality (per DCMv2 Chap 4.1 and ECM Appendix I.7.3). Refer to the Reference Information pages below for more guidance.

Part I. Project Summary			
Project Name:	Elk View Estates Subdivision		
Is Stormwater Detention Required?	☐ Yes ☒ No	Is Water Quality Treatment Required? (i.e.: non-excluded disturbance >1ac)	☒ Yes ☐ No
Is an ESQCP Required? If “No,” Check Applicable Reason	☒ Yes ☐ No	☐ Not an Applicable Construction Activity ☐ Oil & Gas ☐ R-Factor	
Engineer of Record Email Address:	TrevenEdwards@GallowayUS.com		

Part II. PCM Exclusions				
Note: Questions A through K directly correlate to Part I.E.4.a.i (A) to (K) on page 27 of the 2016 CDPS Statewide Standard MS4 General Permit COR090000 (i.e.: the MS4 Permit), as amended. Document exclusions that apply to the whole project or parts of it.				
Questions	Excluded Acreage	Yes	No	Notes
A. Is this project a “Pavement Management Site?”			☒	This exclusion applies to the maintenance, rehabilitation, and reconstruction of pavement on existing roads, bridges, bike lanes, and parking along roads. Areas used primarily for parking (i.e.: separate lots not along roadway) or access to parking are not included. No increase in impervious area is allowable.
B. Review two options below to see if project is an “Excluded Roadway Development.”				Does <u>not</u> include sidewalks. Does include curb & gutter.
• Does the project include improvements to an existing roadway that adds < 1 acre of paved or gravel area per mile of roadway?			☒	If selected, list the proposed additional acreage per mile in Part IV Notes below.
• Does the project include improvements to an existing roadway that adds ≤ 8.25 ft of paved width at any location?			☒	If selected, list the proposed additional width in Part IV Notes below.



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EL PASO COUNTY PCM APPLICABILITY FORM

EPC Project Number: _____

Part II. PCM Exclusions (continued)				
Questions	Excluded Acreage	Yes	No	Notes
C. Does the project include "Excluded Existing Roadway Areas?"			X	For redevelopment of <u>existing</u> roadways. This exclusion only excludes the original roadway area, it does NOT apply to the entire project. This exclusion applies only when the proposed project will expand the existing roadway width by <2x on average. If selected, list the proposed expanded width in Part IV Notes below.
D. Is the project considered an Aboveground or Underground Utilities activity?			X	Activity can <u>not</u> permanently alter the terrain, ground cover, or drainage patterns from existing conditions.
E. Is the project considered a "Large Lot Single-Family Site"? <i>This exclusion only pertains to the lots and does not include roadways.</i>		X		Must be a single-family residential lot or agricultural zoned land with ≥ 2.5 acres per dwelling and total lot impervious area < 10%. If "Yes," notate the percent impervious below in Part IV: Notes.
F. Do Non-Residential or Non-Commercial Infiltration Conditions exist? <i>Post-development surface conditions do not result in concentrated stormwater flow or surface water discharge during an 80th percentile stormwater runoff event, and the 80th percentile event must be infiltrated.</i>			X	Exclusion does not apply to residential or commercial sites for buildings. A site-specific study is required and must show rainfall and soil conditions, allowable slopes, surface conditions, and ratios of imperviousness area to pervious area.
G. Is the project land disturbance to Undeveloped Land where undeveloped land remains undeveloped following the activity?			X	Project must be on land with no human made structures such as buildings or pavement. The proposed development must return the disturbed area to its historical condition. See CDPHE's "Standard MS4 Permit FAQ" for more detail on how this exclusion applies.
H. Is the project a Stream Stabilization Site?			X	
I. Is the project a Bike or Pedestrian Trail?			X	Bike lanes for roadways are not included in this exclusion but may qualify if attached to a larger roadway activity that is excluded in A, B or C above. Pedestrian trails (e.g. sidewalks) that are attached to a roadway do not apply.
J. Is the project Oil and Gas Exploration?			X	Activities and facilities associated with oil and gas exploration are excluded.
K. Is the project in a County Growth Area?				El Paso County does not apply this exclusion.
If any exclusions above apply (via a "Yes" for any row), runoff from those areas is excluded from stormwater quality treatment requirements. All runoff from remaining non-excluded disturbed areas will need to be treated by a stormwater quality PCM, unless remaining area is <1ac. If remaining area is >1ac, select at least one Design Standard on the next page.				



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EL PASO COUNTY PCM APPLICABILITY FORM

EPC Project Number: _____

Part III: PCM Information		
Questions	Yes	No
1. Which of the following Design Standard(s) will the project utilize? <i>(If a PCM is required, you must select at least one. See Control Measure Requirements identified in MS4 Permit Part I.E.4.a.iv on page 29.)</i>		
A. Water Quality Capture Volume (WQCV) Standard		X
B. Pollutant Removal Standard - 80% Total Suspended Solids Removal (TSS) <i>(must treat runoff to <30mg/L of TSS)</i>		X
C. Runoff Reduction Standard	X	
D. Applicable Development Site Draining to a Regional WQCV Control Measure <i>(no conveyance via "Waters of the State")</i>		X
E. Applicable Development Site Draining to a Regional WQCV Facility <i>(conveyance allowable via "Waters of the State," if the 8 conditions in the MS4 permit are met and documented in the drainage report)</i>		X
F. Constrained Redevelopment Sites Standard <i>(must be pre-approved by ECM Administrator)</i>		X
G. Previous Permit Term Standard		X
2. Will any of the PCMs be located within any other jurisdiction besides EPC?		X

Part IV: Notes
Provide info regarding all applicable PCM(s) and PCM Exclusion(s) including location, PCM name(s)/number(s), and additional relevant filings or reports or maintenance agreements, etc. Attach an additional sheet if you need more space. Attaching a detailed summary table would replace the need for any notes here. The proposed development plan is to subdivide the existing undeveloped parcel of land into (6) 2.5-acre single-family residential lots. (2) proposed, private, roadways are proposed within this subdivision. In conformance with the County MS4, the proposed residential lots will be excluded from the stormwater quality requirements per the ECM, Appendix I.7.1.B.5. The proposed roadway is not included in this exclusion. The proposed road has been analyzed to meet the County runoff reduction requirement to provide water quality treatment. The impervious areas are routed across pervious landscape buffers, adjacent to the roadway, still within the private roadway easement.



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EL PASO COUNTY PCM APPLICABILITY FORM

EPC Project Number: _____

Part V: Signatures

Applicant: This PCM Applicability Form was prepared under my direction and supervision and is correct to the best of my knowledge and belief. It was prepared along with the project design, construction plans, drainage report, specifications, and maintenance and access agreements as required. And it has been reviewed for compliance with the Post Construction Stormwater Management criteria and MS4 Permit requirements.

Signature and Stamp of Engineer of Record
(If the project is not an Applicable Construction Activity, this line can be signed by
the Applicant or their rep, they do not have to be an engineer)

Date

El Paso County: This PCM Applicability Form has been reviewed and the project design, construction plans, drainage report, specifications, and maintenance and access agreements as required, have been reviewed for compliance with the Post Construction Stormwater Management process and MS4 Permit requirements.

Signature of El Paso County Project Engineer

Date

Reference Information:

If a PCM is required, then these additional documents will also need to be submitted:

- PCM Maintenance Agreement
- PCM O&M Manual
- MHFD Detention Basin Design Workbook*
- Proof of Submittal of: Notice of Intent to Construct a Non-Jurisdictional Water Impoundment Structure*

*Not required for all PCMs, check ECM Appendix I for requirements

The following are screenshots of example Water Quality Treatment Summary Tables. The Excel versions can be found at the EPC DPW Stormwater website linked below. These are optional tables that can be used to summarize water quality treatment and applicable exclusions. Select the table that best suits the project based on the number of basins, PCMs, and/or exclusions. A PDF of the selected table(s) can be attached to this form and/or to the Drainage Report. It is helpful to also include a basic overview map with color shading or hatch patterns that shows areas tributary to each type of PCM (pond, runoff reduction, etc.) and those areas that are not captured by a PCM, with the applicable exclusion(s) labeled.

<https://publicworks.elpasoco.com/stormwater/>

Water Quality Treatment Summary Table		
Basin ID(s)	PCM Tributary Area (ac)	PCM ID
A1 - A5	4	Pond 1
B1 - B3	3.25	Pond 2
C, D	5.5	Runoff Reduction
E	10	Excluded*
* Excluded based on ECM App I.7.1.B.5		

Water Quality Treatment Summary Table							
Basin ID	Total Area (ac)	Total Proposed Disturbed Area (ac)	Area Trib to Pond A (ac)	Disturbed Area Treated via Runoff Reduction (ac)	Disturbed Area Excluded from WQ per ECM App I.7.1.C.1 (ac)	Disturbed Area Excluded from WQ per ECM App I.7.1.B.# (ac)	Applicable WQ Exclusions (App I.7.1.B.#)
A	4.50	4.50	4.50				
B	1.25	1.25		1.25			
C	6.00	4.00				4.00	ECM App I.7.1.B.5
D	2.50	2.50	1.00		0.50	1.00	ECM App I.7.1.B.7
E	3.00		3.00				
F	8.25						
Total	25.50	12.25	8.50	1.25	0.50	5.00	

Min Required Area to Receive WQ Treatment	Total Proposed Disturbed Area (ac)	Total Proposed Treated Area (ac)	Total Proposed Disturbed Area Excluded from WQ (ac)	Net Treatment (ac)
6.75	12.25	9.75	5.50	3.00

Design Standard D, definition of “Waters of the State of Colorado” per MS4 Permit:

“Any and all surface waters and subsurface waters which are contained in or flow in or through this state, but does not include waters in sewage systems, waters in treatment works of disposal systems, waters in potable water distribution systems, and all water withdrawn for use until use and treatment have been completed. This definition can include water courses that are usually dry.”

The following website shows Waters of the State of Colorado:

<https://cdphe.maps.arcgis.com/apps/Viewer/index.html?appid=f1541d2f21834642ba1551c674fd4a79>

Design Standard E, additional info from the MS4 Permit:

Before discharging to a water of the state, at least 20 percent of the upstream imperviousness of the applicable development site must be disconnected from the storm drainage system and drain through a receiving pervious area control measure comprising a footprint of at least 10 percent of the upstream disconnected impervious area of the applicable development site. The control measure must be designed in accordance with a design manual identified by the permittee. In addition, the stream channel between the discharge point of the applicable development site and the regional WQCV facility must be stabilized.

Below are the 8 conditions that must be met:

- 1) The regional WQCV facility must be implemented, functional, and maintained following good engineering, hydrologic and pollution control practices.*
- 2) The regional WQCV facility must be designed and maintained for 100% WQCV for its entire drainage area.*
- 3) The regional WQCV facility must have capacity to accommodate the drainage from the applicable development site.*
- 4) The regional WQCV facility be designed and built to comply with all assumptions for the development activities planned by the permittee within its drainage area, including the imperviousness of its drainage area and the applicable development site.*
- 5) Evaluation of the minimum drain time shall be based on the pollutant removal mechanism and functionality of the facility. Consideration of drain time shall include maintaining vegetation necessary for operation of the facility (e.g., wetland vegetation).*
- 6) The permittee shall meet the requirements in Parts I.E.4.a.v. and vii. and Part I.E.4.b. for the regional WQCV facility consistent with requirements and actions for control measures.*
- 7) The regional WQCV facility must be subject to the permittee’s authority consistent with requirements and actions for a Control Measure in accordance with Part I.E.4.a.iv.*
- 8) Regional Facilities must be designed and implemented with flood control or water quality as the primary use. Recreational ponds and reservoirs may not be considered Regional Facilities. Water bodies listed by name in surface water quality classifications and standards regulations (5 CCR 1002-32 through 5 CCR 1002-38) may not be considered regional facilities.*

Use current
spreadsheet

Design Procedure Form: Runoff Reduction

UD-BMP (Version 3.07, March 2018)

Sheet 1 of 1

Designer: TJE
Company: Galloway & Company
Date: June 2, 2026
Project: Elk View Estates - KO SUB-BASINS
Location: El Paso County, CO

SITE INFORMATION (User Input in Blue Cells)

WQCV Rainfall Depth = 0.60 inches
Depth of Average Runoff Producing Storm, d_p = 0.43 inches (for Watersheds Outside of the Denver Region, Figure 3-1 in USDCM Vol. 3)

Area Type	UIA:RPA	SPA	UIA:RPA	SPA	UIA:RPA	UIA:RPA	DCIA	UIA:RPA	SPA	UIA:RPA	SPA	
Area ID	D201	D201_SPA	D203	D203_SPA	D204	D205	D207	D221	D221_SPA	D222	D222_SPA	
Downstream Design Point ID	D10/D11	D10/D11	D12	D12	D13	D13	D17	D15	D15	D16	D16	
Downstream BMP Type	None	None	None	None	None	None	None	None	None	None	None	
DCIA (ft ²)	--	--	--	--	--	--	391	--	--	--	--	
UIA (ft ²)	15,291	--	6,216	--	3,673	14,677	--	525	--	4,231	--	
RPA (ft ²)	9,657	--	2,008	--	4,556	11,430	--	1,008	--	1,395	--	
SPA (ft ²)	--	1,017	--	1,571	--	--	--	--	1,130	--	857	
HSG A (%)	0%	0%	0%	0%	0%	0%	--	0%	0%	0%	0%	
HSG B (%)	100%	100%	100%	100%	100%	100%	--	100%	100%	100%	100%	
HSG C/D (%)	0%	0%	0%	0%	0%	0%	--	0%	0%	0%	0%	
Average Slope of RPA (ft/ft)	0.250	--	0.250	--	0.250	0.250	--	0.250	--	0.250	--	
UIA:RPA Interface Width (ft)	945.00	--	230.00	--	400.00	890.00	--	130.00	--	104.00	--	

CALCULATED RUNOFF RESULTS

LW should be between 0.0625 and 16

Area ID	D201	D201_SPA	D203	D203_SPA	D204	D205	D207	D221	D221_SPA	D222	D222_SPA	
UIA:RPA Area (ft ²)	24,948	--	8,224	--	8,229	26,407	--	1,533	--	5,626	--	
L / W Ratio	0.06	--	0.16	--	0.06	0.06	--	0.09	--	0.52	--	
UIA / Area	0.6129	--	0.7558	--	0.4463	0.5622	--	0.3425	--	0.7520	--	
Runoff (in)	0.00	0.00	0.13	0.00	0.00	0.00	0.50	0.00	0.00	0.12	0.00	
Runoff (ft ³)	0	0	87	0	0	0	16	0	0	57	0	
Runoff Reduction (ft ³)	637	51	172	79	153	612	0	22	57	119	43	

CALCULATED WQCV RESULTS

Area ID	D201	D201_SPA	D203	D203_SPA	D204	D205	D207	D221	D221_SPA	D222	D222_SPA	
WQCV (ft ³)	637	0	259	0	153	612	16	22	0	176	0	
WQCV Reduction (ft ³)	637	0	172	0	153	612	0	22	0	119	0	
WQCV Reduction (%)	100%	0%	67%	0%	100%	100%	0%	100%	0%	68%	0%	
Untreated WQCV (ft ³)	0	0	87	0	0	0	16	0	0	57	0	

CALCULATED DESIGN POINT RESULTS (sums results from all columns with the same Downstream Design Point ID)

Downstream Design Point ID	D10/D11	D12	D13	D17	D15	D16						
DCIA (ft ²)	0	0	0	391	0	0						
UIA (ft ²)	15,291	6,216	18,350	0	525	4,231						
RPA (ft ²)	9,657	2,008	15,986	0	1,008	1,395						
SPA (ft ²)	1,017	1,571	0	0	1,130	857						
Total Area (ft ²)	25,965	9,795	34,336	391	2,663	6,483						
Total Impervious Area (ft ²)	15,291	6,216	18,350	391	525	4,231						
WQCV (ft ³)	637	259	765	16	22	176						
WQCV Reduction (ft ³)	637	172	765	0	22	119						
WQCV Reduction (%)	100%	67%	100%	0%	100%	68%						
Untreated WQCV (ft ³)	0	87	0	16	0	57						

CALCULATED SITE RESULTS (sums results from all columns in worksheet)

Total Area (ft ²)	79,633
Total Impervious Area (ft ²)	45,004
WQCV (ft ³)	1,875
WQCV Reduction (ft ³)	1,715
WQCV Reduction (%)	91%
Untreated WQCV (ft ³)	160

APPENDIX E

DRAINAGE MAPS

PROPERTY BOUNDARY LINE

ADJACENT PROPERTY BOUNDARY LINE

RIGHT OF WAY BOUNDARY LINE

ROAD CENTERLINE

EXISTING EASEMENT LINE

SD EXISTING STORM SEWER (LESS THAN 12")

EXISTING STORM SEWER (12" & >LARGER)

5460 EXISTING MAJOR CONTOUR

52 EXISTING MINOR CONTOUR

MAJOR BASIN BOUNDARY LINE

DESIGN POINT

EXISTING FLOW ARROW

BASIN DESIGNATION

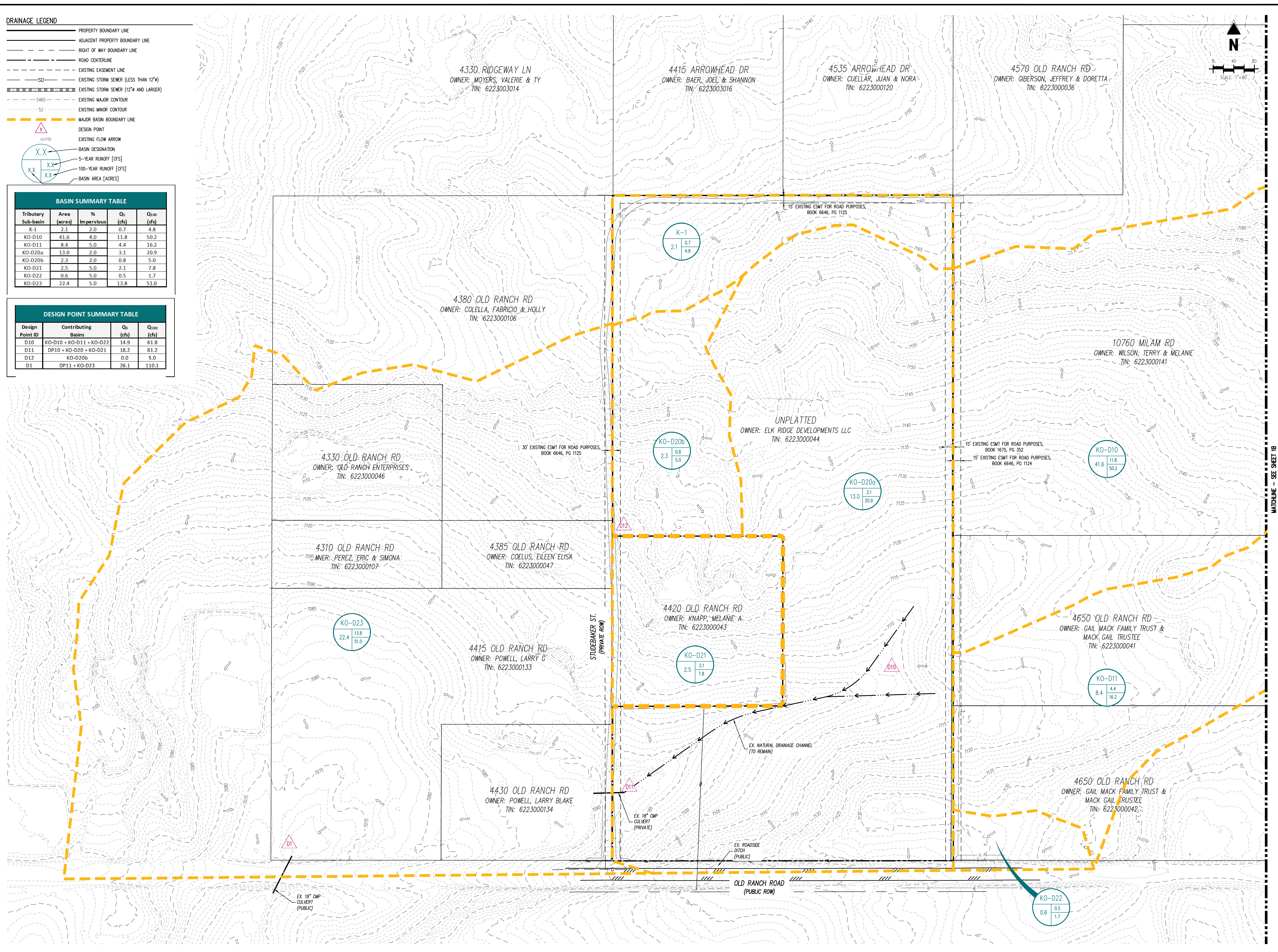
5-YEAR RUNOFF [CFS]

100-YEAR RUNOFF [CFS]

BASIN AREA [ACRES]

Tributary Sub-basin	Area (acres)	% Impervious	Q ₅ (cfs)	Q ₁₀₀ (cfs)
K-1	2.1	2.0	0.7	4.8
KO-D10	41.6	4.0	11.8	50.2
KO-D11	8.4	5.0	4.4	16.2
KO-D20a	13.0	2.0	3.1	20.9
KO-D20b	2.3	2.0	0.8	5.0
KO-D21	2.5	5.0	2.1	7.8
KO-D22	0.6	5.0	0.5	1.7
KO-D23	22.4	5.0	13.8	51.0

DESIGN POINT SUMMARY TABLE			
Design Point ID	Contributing Basins	Q ₅ (cfs)	Q ₁₀₀ (cfs)
D10	KO-D10 + KO-D11 + KO-D22	14.9	61.8
D11	DP10 + KO-D20 + KO-D21	18.2	81.2
D12	KO-D20b	0.0	5.0
D1	DP11 + KO-D23	26.1	110.1

[illegible]

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ELK VIEW ESTATES
CONSTRUCTION DRAWINGS

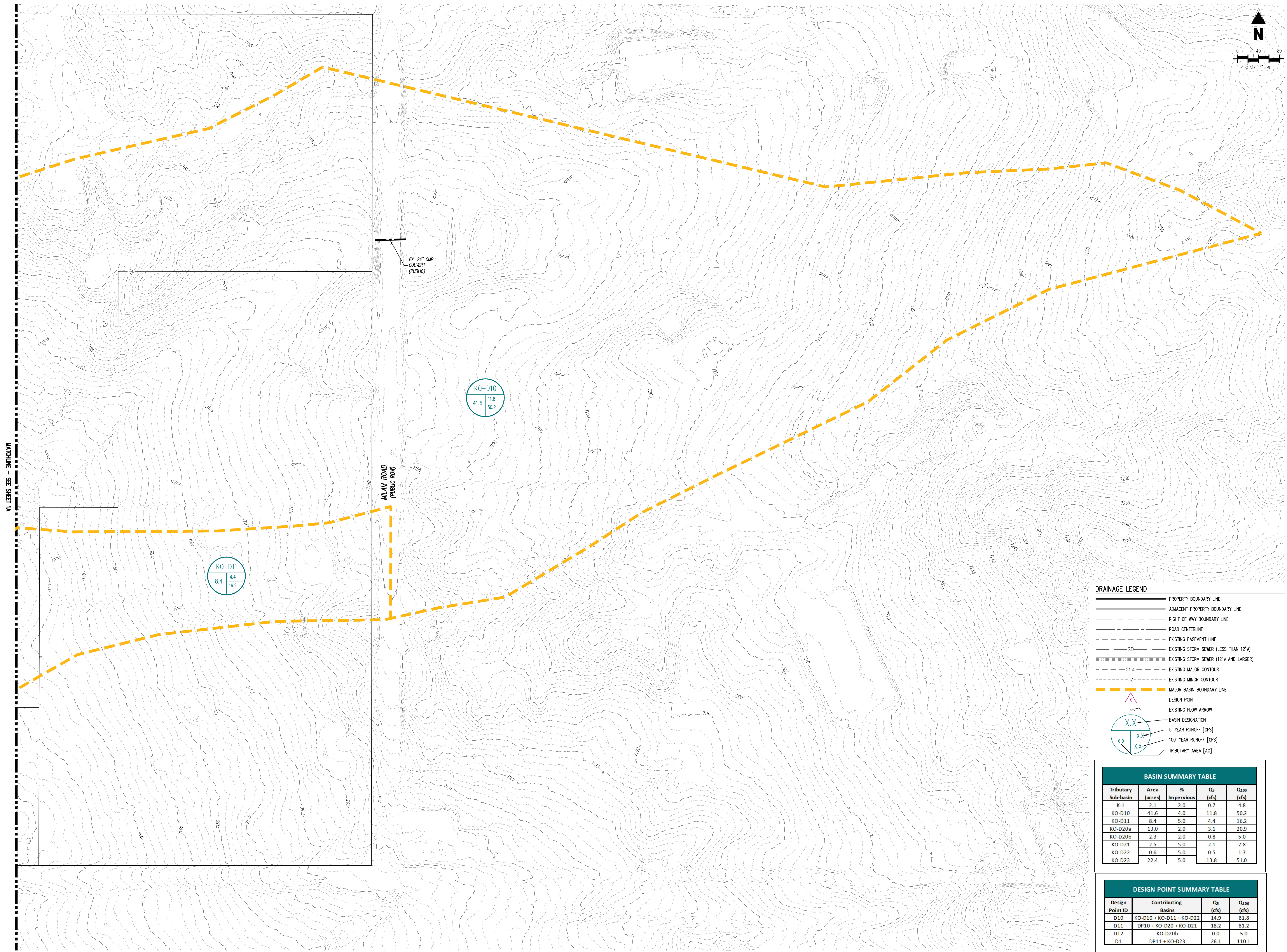
OLD RANCH ROAD
EL PASO COUNTY, CO

[illegible]

Project No:	ERD01
Drawn By:	ELB
Checked By:	TJE
Date:	06/19/2025

EXISTING CONDITIONS
DRAINAGE MAP

1B

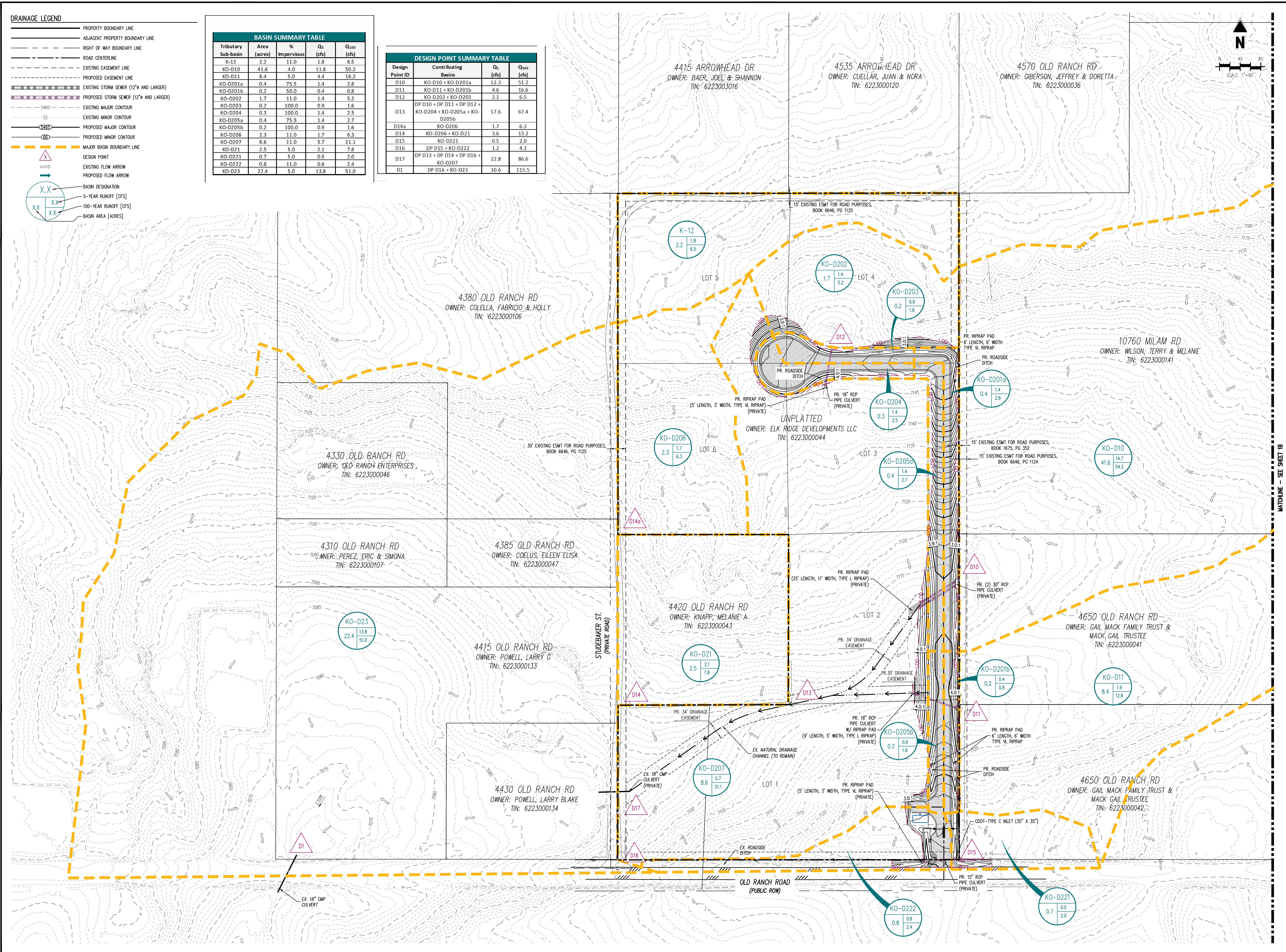


Drainage Legend

- PROPERTY BOUNDARY LINE
- ADJACENT PROPERTY BOUNDARY LINE
- RIGHT OF WAY BOUNDARY LINE
- ROAD CENTERLINE
- EXISTING EASEMENT LINE
- PROPOSED EASEMENT LINE
- EXISTING STORM SEWER (12" AND LARGER)
- PROPOSED STORM SEWER (12" AND LARGER)
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- MAJOR BASIN BOUNDARY LINE
- DESIGN POINT
- EXISTING FLOW ARROW
- PROPOSED FLOW ARROW
- BASIN DESIGNATION
- 5-YEAR RUNOFF [CFS]
- 100-YEAR RUNOFF [CFS]
- BASIN AREA [ACRES]

BASIN SUMMARY TABLE				
Tributary Sub-basin	Area (acres)	% Impervious	Q ₅ (cfs)	Q ₁₀₀ (cfs)
K-12	2.2	11.0	1.8	6.5
KO-D10	41.6	4.0	11.8	50.2
KO-D11	8.4	5.0	4.4	16.2
KO-D201a	0.4	75.5	1.4	2.8
KO-D201b	0.2	50.0	0.4	0.8
KO-D202	1.7	11.0	1.4	5.2
KO-D203	0.2	100.0	0.9	1.6
KO-D204	0.3	100.0	1.4	2.5
KO-D205a	0.4	75.5	1.4	2.7
KO-D205b	0.2	100.0	0.9	1.6
KO-D206	2.3	11.0	1.7	6.3
KO-D207	8.6	11.0	5.7	21.1
KO-D21	2.5	5.0	2.1	7.8
KO-D221	0.7	5.0	0.5	2.0
KO-D222	0.8	11.0	0.6	2.4
KO-D23	22.4	5.0	13.8	51.0

DESIGN POINT SUMMARY TABLE			
Design Point ID	Contributing Basins	Q ₅ (cfs)	Q ₁₀₀ (cfs)
D10	KO-D10 + KO-D201a	12.3	51.2
D11	KO-D11 + KO-D201b	4.6	16.6
D12	KO-D202 + KO-D203	2.1	6.5
D13	DP D10 + DP D11 + DP D12 + KO-D204 + KO-D205a + KO-D205b	17.6	67.4
D14a	KO-D206	1.7	6.3
D14	KO-D206 + KO-D21	3.6	13.2
D15	KO-D221	0.5	2.0
D16	DP D15 + KO-D222	1.2	4.3
D17	DP D13 + DP D14 + DP D16 + KO-D207	22.8	86.6
D1	DP D16 + KO-D23	30.6	115.5



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ELK VIEW ESTATES
CONSTRUCTION DRAWINGS

OLD RANCH ROAD
EL PASO COUNTY, CO

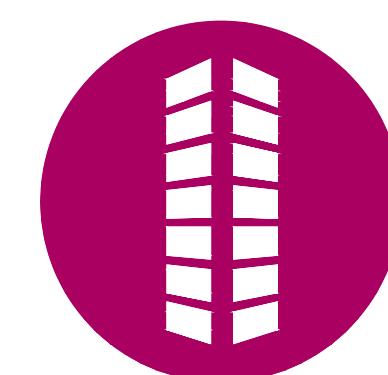
#	Date	Issue / Description	Init.
1	11/26/2025	1ST COUNTY SUBMITTAL	ELB
2	06/02/2026	2ND COUNTY SUBMITTAL	ELB

Project No:	ERD01
Drawn By:	ELB
Checked By:	TJE
Date:	06/19/2025

PROPOSED CONDITIONS
DRAINAGE MAP

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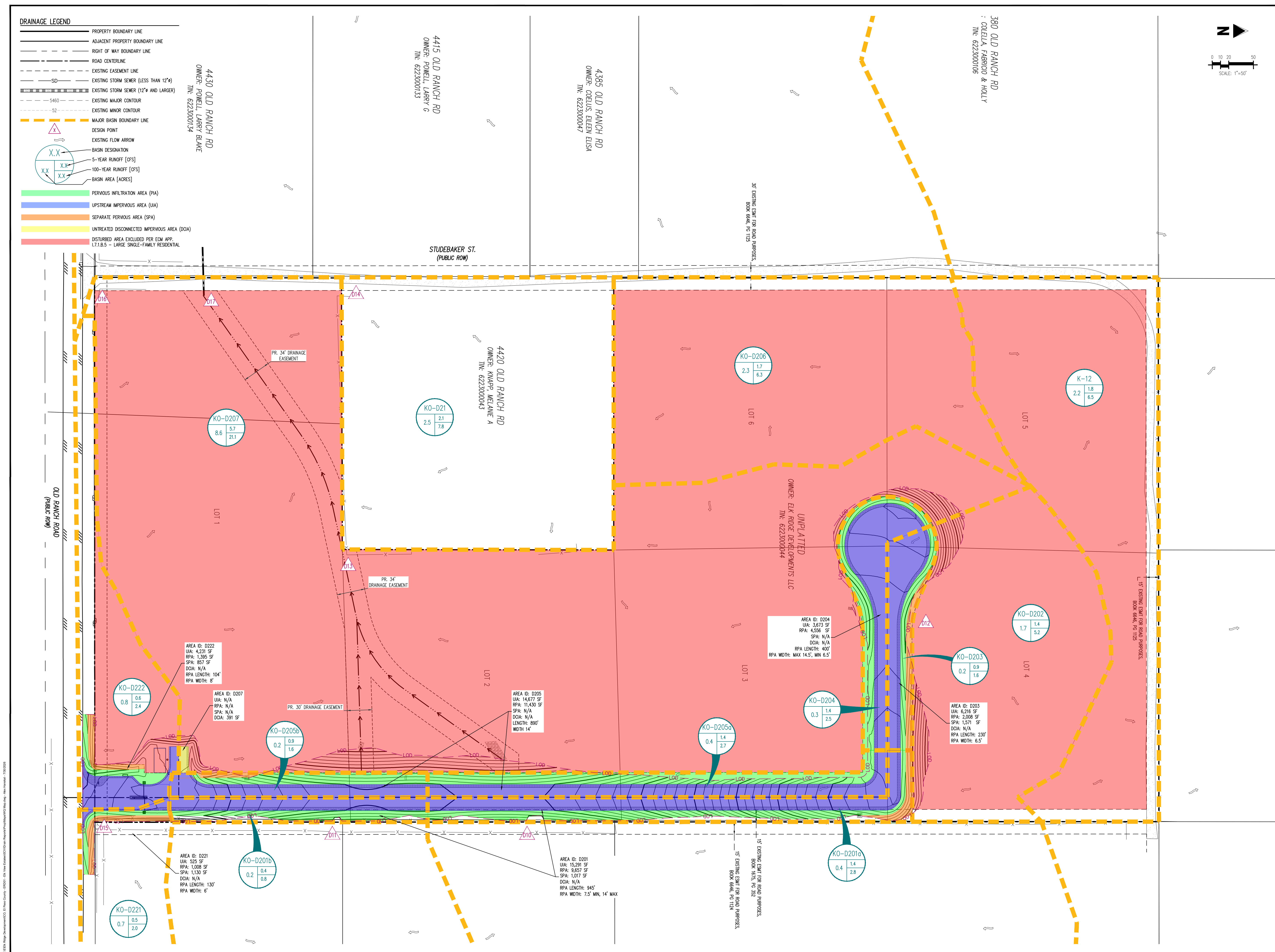
**ELK VIEW ESTATES
CONSTRUCTION DRAWINGS**

OLD RANCH ROAD
EL PASO COUNTY, CO

[illegible]

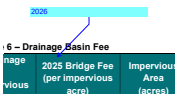
Project No:	ERD01
Drawn By:	ELB
Checked By:	TJE
Date:	06/19/2025

WATER QUALITY & RUNOFF REDUCTION DRAINAGE MAP



V3_Drainage Report-Final_comments.pdf Markup Summary

Callout (1)



Subject: Callout
Page Label: 18
Author: CDurham
Date: 9/10/2026 8:20:33 AM
Status:
Color: ■
Layer:
Space:

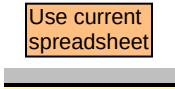
2026

Stamp - Stormwater Comment Legend (1)



Subject: Stamp - Stormwater Comment Legend
Page Label: 1
Author: Mikayla Hartford
Date: 9/9/2026 1:46:05 PM
Status:
Color: ■
Layer:
Space:

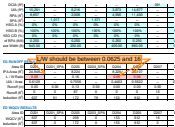
SW - Textbox (1)



Subject: SW - Textbox
Page Label: 75
Author: Mikayla Hartford
Date: 9/9/2026 1:43:52 PM
Status:
Color: ■
Layer:
Space:

Use current spreadsheet

SW - Textbox with Arrow (1)



Subject: SW - Textbox with Arrow
Page Label: 75
Author: Mikayla Hartford
Date: 9/9/2026 1:43:52 PM
Status:
Color: ■
Layer:
Space:

L/W should be between 0.0625 and 16