

SOILS AND GEOLOGY STUDY FLYING HORSE NORTH, FILING NO. 4 EL PASO COUNTY, COLORADO

Prepared for:

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Respectfully Submitted,

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1 SUMMARY

Project Location

The site consists of portions of the S½ of Section 30, and the NE¼ of Section 31, Township 11 South, Range 65 West of the 6th Principal Meridian, and a portion of the NE¼ of Section 36, Township 11 South, Range 66 West of the 6th Principal Meridian in El Paso County, Colorado. The site is located approximately 3½ miles northeast of Colorado Springs, Colorado.

Project Description

Flying Horse North Filing No. 4 Subdivision is approximately 175 acres with forty-eight (48) lots proposed for the filing along with three full spectrum detention ponds and other associated site improvements. The proposed development is to consist of approximately 2.5 to 5-acre single-family residential lots. The development will be serviced by individual water wells and on-site wastewater systems (OWTS).

Scope of Report

This report presents the results of our geologic evaluation and treatment of engineering geologic hazard study.

Land Use and Engineering Geology

This site was found to be suitable for the proposed development. Areas were encountered where the geologic conditions will impose some minor constraints on development and land use. These include areas of seasonal and potentially seasonal shallow groundwater areas, drainage areas, areas of seasonally ponded water, erosion, artificial fill, expansive soils, and potential for elevated radon levels. Based on the proposed development plan, it appears that these areas will have some impact on the development. These conditions will be discussed in greater detail in the report.

In general, it is our opinion that the development can be achieved if the observed geologic conditions on site are either avoided or properly mitigated. All recommendations are subject to the limitations discussed in the report.

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2 GENERAL SITE CONDITIONS AND PROJECT DESCRIPTION

The site consists of portions of the S½ of Section 30, and the NE¼ of Section 31, Township 11 South, Range 65 West of the 6th Principal Meridian, and a portion of the NE¼ of Section 36, Township 11 South, Range 66 West of the 6th Principal Meridian in El Paso County, Colorado. The site is located approximately 3½ miles northeast of Colorado Springs, Colorado, at the east end of Stagecoach Road between Highway 83 and Black Forest Road. The location of the site is as shown on the Vicinity Map, Figure 1.

The topography of the site varies from gently to moderately sloping generally to the east and north with some steeper slopes along the drainages in portions of the site. Palmer Divide is located to the west of Filing No. 4. The drainages on site generally flow in a northerly direction through the site. Water was not observed in any of the drainages or ponds within Filing No. 4 at the time of our site investigation. The site contains primarily field grasses and weeds in with areas of scattered ponderosa pine trees in the northern portion of the site along Old Stagecoach Road. Site photographs are included in Appendix A. The locations and directions of the photographs are indicated in Figure 3.

Flying Horse North Filing No. 4 Subdivision is approximately 175 acres with forty-eight (48) lots proposed for the filing along with three full spectrum detention ponds and other associated site improvements. The proposed development is to consist of approximately 2.5 to 5-acre single-family residential lots. Grading is expected to be primarily associated with the construction of roads and the three detention ponds. The Site and Exploration Plan is presented in Figure 3.

3 SCOPE OF THE REPORT

The scope of the report will include a general geologic analysis utilizing published geologic data. Detailed site-specific mapping will be conducted to obtain general information in respect to major geographic and geologic features, geologic descriptions and their effects on the development of the property.

4 FIELD INVESTIGATION

Our field investigation consisted of the preparation of a geologic map of any bedrock features and significant surficial deposits. The Natural Resource Conservation Service (NRCS), previously the Soil Conservation Service (SCS) survey was also reviewed to evaluate the site. The position of mappable units within the subject property are shown on the Geologic Map. Our mapping



procedures involved both field reconnaissance and measurements and air photo reconnaissance and interpretation. The same mapping procedures have also been utilized to produce the Engineering Geology Map which identified pertinent geologic conditions affecting development. The field mapping was initially performed by personnel of Entech Engineering, Inc. (Entech) on November 21 and December 2, 2014. Field mapping was updated by Entech on October 31 and November 3, 2017 (References 1 and 2). The site was revisited and additional mapping completed in July and August of 2024. Recent site photographs are included in Appendix A.

Fifteen (15) test borings were drilled and six (6) test pits excavated across the site as part of this study to determine the soils classification and engineering characteristics. The borings were drilled to depths of 20 feet using a truck-mounted, continuous flight auger drilling rig supplied and operated by Entech, and the test pits were excavated to depths ranging from 6 to 8 feet. Test Boring Nos. 13 – 15 were placed in proposed pond locations.

Five (5) test borings and six (6) test pits from previous Flying Horse North investigations were used in the in preparing this report (Reference 3). The location of the previous Test Borings and Test Pits indicated on the Site and Exploration Plan, Figure 3.

Laboratory testing was performed on some of the soils to classify and determine the soils engineering characteristics. Laboratory tests included moisture content testing, ASTM D-2216, tests included grain-size analysis ASTM D-422, Atterberg Limits ASTM D-4318, volume change testing using Swell/Consolidation test. Sulfate testing was performed on select samples to evaluate potential for below grade concrete degradation due to sulfate attack. Results of the laboratory testing are included in Appendix C. A Summary of Laboratory Test Results is presented in Table C-1. Previous Laboratory Testing Summary and Test Boring and Test Pit Logs are included in Appendix D.

5 SOIL, GEOLOGY, AND ENGINEERING GEOLOGY

5.1 General Geology

Physiographically, the site lies in the western portion of the Great Plains Physiographic Province. Approximately 10 miles to the west is a major structural feature known as the Rampart Range Fault. This fault marks the boundary between the Great Plains Physiographic Province and the Southern Rocky Mountain Province. The site exists within the southeastern edge of a large structural feature known as the Denver Basin. Bedrock in the area tends to be very gently dipping in a northerly direction (Reference 4). The rocks in the area of the site are sedimentary in nature,



and typically Tertiary to Cretaceous in age. The bedrock underlying the site consists of the Dawson Arkose Formation. Overlying this formation are unconsolidated deposits of residual, colluvial, man-made, and alluvial soils of the Quaternary Age. The residual soils are produced by the in-situ action of weathering of the bedrock on site. Some colluvial soils exist which are deposited by gravity and sheetwash. The alluvial soils were deposited by water in the drainages on site. Man-made soils exist as earthen dams and erosion berms. The site's stratigraphy will be discussed in more detail in Section 5.3.

5.2 Soil Conservation Survey

The Natural Resource Conservation Service (Reference 5), previously the Soil Conservation Service (Reference 6) has mapped two soil types on the site (Figure 4). In general, the soils classify as coarse sandy loam, and sandy loam. The soils are described as follows:

Туре	Description
8	Blakeland Loamy Sand, 1 to 9% slopes
19	Columbine Gravelly Sandy Loam, 0 to 3% slopes

Complete descriptions of each soil type are presented in Appendix E. The soils have generally been described to have moderate to rapid permeabilities. Limitations on development include, limited ability to support a load, shrink swell potential, slopes and frost action potential. Possible hazards with soil erosion are present on the site. The erosion potential can be controlled with vegetation. The majority of the soils have been described to have moderate erosion hazards

5.3 Site Stratigraphy

The Black Forest Quadrangle Geology Map showing the site is presented in Figure 6 (Reference 7). The Geology Map prepared for the site is presented in Figure 7. Three mappable units were identified on this site which are described as follows:

Qaf Artificial Fill of Holocene Age: These are man placed fill deposits associated with erosion berms, earthen dams on-site, and stockpiles of fill. Additionally, temporary stockpiles were observed on the site. Other areas of fill may exist on the site other than those mapped due to on-going construction.



Qal Recent Alluvium of Quaternary Age: These are recent stream deposits associated with the drainages on-site. These materials generally consist of silty to clayey sands and may contain clay lenses. Highly organic soils may be encountered in some of these areas.

Tkd Dawson Formation of Tertiary to Cretaceous Age: The Dawson formation typically consists of arkosic sandstone with interbedded fine-grained sandstone, siltstone and claystone. Overlying this formation is a variable layer of residual soil. The residual soils were derived from the in-situ weathering of the bedrock materials on-site. These soils consisted of silty to clayey sands and sandy clays. Areas of colluvial soils may exist on some of the slopes on site. These materials are derived from the bedrock materials and have been re-deposited by the action of sheetwash and gravity.

The bedrock underlying the site consists of the Dawson Formation of Tertiary to Cretaceous Age. The Dawson Formation typically consists of arkosic sandstone with interbedded fine-grained sandstone, siltstone and claystone. Overlying this formation are variable layers of man placed fill deposits, alluvial deposits, and residual soil. The residual soils were derived from the in-situ weathering of the bedrock materials on-site. These soils consisted of silty to clayey sands and sandy clays.

The soils listed above were mapped from site-specific mapping, the *Geologic Map of the Black Forest Quadrangle* distributed by the Colorado Geological Survey in 2003 (References 7), the *Geologic Map of the Colorado Springs-Castle Rock Area*, distributed by the US Geological Survey in 1979 (Reference 8), and the *Geologic Map of the Denver 1^ox 2^o Quadrangle*, distributed by the US Geological Survey in 1981 (Reference 9). The Test Borings and Test Pit Logs used in evaluating the site and are included in Appendix B. The Geology Map prepared for the site is presented in Figure 7.

5.4 Soil Conditions

The soils encountered in the Test Borings can be grouped into four general soil and rock types. The soils were classified using the Unified Soil Classification System (USCS).

<u>Soil Type 1</u> classified as sand with varying amounts of silt, and clayey sand (SW-SM, SM, SC, SM). The sand was encountered in 13 of the 15 test borings at the ground surface to 17 feet bgs and extending to depths ranging from 9 to 19 feet bgs and to the termination of TB-8, TB-9, and TB-12 (20 feet). The sand was encountered at medium dense to dense states. The majority of the samples indicated medium dense states.



<u>Soil Type 2</u> classified as sandy clay and clay with sand (CL). The clay was encountered in 9 of the 15 test borings at the ground surface to 19 feet bgs and extending to depths ranging from 4 to 16 feet bgs. The clay was encountered at medium stiff to very stiff consistencies. One-dimensional swell or collapse testing on samples of the clay resulted in expansions of 0.4% to 1.4 indicating a low potential for collapse or expansion.

<u>Soil Type 3</u> classified as silty sandstone and clayey sandstone (SM, SC), or as a silty sand and clayey sand when classified as a soil. The sandstone was encountered in 10 of the 15 boings at depths ranging from the 7 to 18 feet bgs, and extended to the termination of the borings (20 feet). The sandstone was encountered at very dense states.

<u>Soil Type 4</u> classified as sandy claystone (CL), or as a sandy clay when classified as a soil. The claystone was encountered in TB-4 at 18 feet bgs, and extended to the termination of the test boring (20 feet). The claystone was encountered at hard consistencies. The claystone is typically expansive.

The Test Boring Logs are presented in Appendix B, and the depth to bedrock and groundwater are presented on Table B-1. Laboratory Test Results are presented in Appendix C, and a Summary of Laboratory Test Results is presented in Table C-1.

5.5 Groundwater

Groundwater was encountered in TB-2 and TB-13 at depths of 18 and 20 feet bgs. Groundwater was not encountered in the remaining test borings which were drilled to 20 feet. Areas of seasonal, potentially seasonal shallow groundwater, and seasonally ponded water have been mapped in the drainages and low-lying areas on the site. These areas are discussed in the following section. Fluctuation in groundwater conditions may occur due to variations in rainfall and other factors not readily apparent at this time. It should be noted that in the sandy materials on-site, some groundwater conditions might be encountered due to the variability in the soil profile. Isolated sand and gravel layers within the soils, sometimes only a few feet in thickness and width, can carry water in the subsurface. Groundwater may also flow on top of the underlying bedrock. Builders and planners should be cognizant of the potential for the occurrence of such subsurface water features during construction on-site and deal with each individual problem as necessary at the time of construction.



6 ENGINEERING GEOLOGY – IDENTIFICATION AND MITIGATION OF GEOLOGIC HAZARDS

Detailed mapping has been performed on this site to produce an Engineering Geology Map Figure 6. This map shows the location of various geologic conditions of which the developers should be cognizant during the planning, design and construction stages of the project. These hazards and the recommended mitigation techniques are as follows:

<u>Artificial Fill – Constraint</u>

These are areas of man-placed fill associated with minor fill piles in the future road areas, and erosion berms on the site.

<u>Mitigation</u>: The fill piles in the areas of future roadways and erosion berms will be mitigated with the proposed roadway site grading. The erosion berms can either be avoided or removed from building areas on each. The fill on this site is considered uncontrolled for construction purposes. Any uncontrolled fill encountered beneath foundations or drainage structures will require removal and recompaction at a minimum of 95% of its maximum Modified Procter Dry Density, ASTM D-1557.

Areas of Erosion – Constraint

These are areas that are undergoing erosion by water and sheetwash producing gullies and rill erosion and primarily located along portions of the drainages in the southeastern and northern portions of the site.

<u>Mitigation</u>: Due to the nature of the soils on this site, virtually all the soils are subject to erosion by wind and water. Other minor areas of erosion were observed on site other than those mapped, particularly where some rill erosion has occurred. Areas of erosion can occur across the entire site, particularly if the soils are disturbed during construction. Vegetation reduces the potential for erosion. The areas identified where erosion is actually taking place may require check dams, regrading and revegetation using channel lining mats to anchor vegetation. Further recommendations for erosion control are discussed under Section 9.0 "Erosion Control" of this report. Recommendations pertaining to revegetation may require input from a qualified landscape architect and/or the Natural Resource Conservation Service (previously Soil Conservation Service)

Expansive Soils – Constraint

The site is classified in an area of low to moderate swell potential according to the Map of Potentially Swelling Soil and Rock in the Front Range Urban Corridor, Colorado by Hart, 1974



(Reference 10). Potentially expansive soils were encountered in some of the test borings drilled on the site as a part of the entire Flying Horse North Subdivision (References 1-3). These occurrences are typically sporadic; therefore, none have been indicated on the maps. These clays or claystone, if encountered beneath foundations, can cause differential movement in the structure foundation. These occurrences should be identified and dealt with on an individual basis.

<u>Mitigation</u> Should expansive soils be encountered beneath the foundation; mitigation will be necessary. Mitigation of expansive soils will require special foundation design. Overexcavation and replacement with non-expansive soils at a minimum of 95% of its maximum Modified Proctor Dry Density, ASTM D-1557 is a suitable mitigation, which is common in the area. Another alternative in areas of highly expansive soils is the use of drilled pier foundation systems. Typical minimum pier depths are on the order of 25 feet or more and require penetration into the bedrock material a minimum of 4 to 6 feet, depending upon building loads. Floor slabs on expansive soils should be expected to experience movement. Overexcavation and replacement has been successful in minimizing slab movements. The use of structural floors should be considered for basement construction on highly expansive clays. Final recommendations should be determined after additional investigation of each building site.

Groundwater and Floodplain Areas - Constraint

Drainages are located in the southeast and northern portions of Filing No. 4, and several minor drainages are located across the site that generally flow in northerly directions. None of the drainages on the site have been mapped within floodplain zones according to the FEMA Map No. 08041CO315G, (Figure 7, Reference 11). Areas where potentially seasonal shallow, seasonal shallow, and seasonally ponded water have been indicated on the site geology/engineering geology map, Figure 6. Lots adjacent to the drainages may experience higher groundwater levels during peak flows. Finished floor levels must be a minimum of one floor above any floodplain level. **Exact floodplain locations and drainage studies are beyond the scope of this report.**

The seasonally shallow groundwater and potentially seasonal shallow groundwater areas located on the site are shown on the Geology/Engineering Geology Map, Figure 7. Portions of these areas mapped with these hazards have been identified in the National Wetland Inventory as Freshwater Emergent Wetland habitats classified as PEM1C (Palustrine – P, Emergent – EM, Persistent – 1, Seasonally Flooded – C), and Freshwater Pond habitat classified as PUSCh (Palustrine – P, Emergent – US, Seasonally Flooded – C, Diked/Impounded – H), (Figure 8, Reference 12).



Groundwater was encountered in two of the test borings drilled within Filing No. 4 at depths of 18 and 20 feet. Areas of seasonally shallow and potential seasonally shallow groundwater, and seasonally ponded water were observed on the site and are further discussed below. Buildings should maintain a minimum separation of 3 feet between the lowest foundation grade and the maximum anticipated groundwater level. Subsurface perimeter drains are recommended for structures with useable below grade space, and additional drains may be required in building areas close drainages to help prevent the intrusion of water into areas below grade. Typical drain details are presented in Figures 9 - 12. Shallow groundwater areas can be mitigated with the installation of drains. Typical drain options/details are presented in Figures 8 through 11. These areas are discussed as follows:

<u>Seasonal Shallow and Potential Seasonally Shallow Groundwater – Constraint</u>

In these areas, we would anticipate periodic high subsurface moisture conditions and frost heave potential on a seasonal basis. Additional, highly organic soils could be encountered in these areas. These areas lie within defined drainages and it is anticipated they will be avoided by development. Minor drainage swales in building areas should be properly diverted away from the structures. Any structures in or adjacent to these areas should follow the mitigation discussed below.

<u>Mitigation:</u> In these locations, foundations subject to severe frost heave potential should penetrate sufficient depth so as to discourage the formation of ice lenses beneath foundations. At this location and elevation, foundation depth for frost protection is 30 inches. In areas where high subsurface moisture conditions are anticipated periodically, a subsurface perimeter drain will be necessary to help prevent the intrusion of water into areas located below grade. Subsurface perimeter drains may be necessary to prevent the intrusion of water into areas below grade. Typical drain details are presented in Figure 8. Where shallow groundwater is encountered, underslab drains or interceptor drains may be necessary Figures 9 and 10. Specific recommendations should be made after additional investigation has been completed and building locations have been identified on a lot by lot basis. Swales should be created to intercept surface runoff and carry it safely around and away from structures.

Areas of Seasonally Ponded Water – Constraint

These are areas of seasonally ponded water behind existing embankments in the northern and southeastern portions of the site. Groundwater is not anticipated to affect the construction of the proposed detention ponds, however if groundwater is encountered during construction temporary dewatering during construction may be required. Should complete regrading of the site be



considered, all organic matter and soft, wet soils should be completely removed before filling. Any drainage into these areas should be rerouted in a non-erosive manner off of the site where it does not create areas of ponded water around proposed structures.

Landslide Hazard and Slope Stability – Hazard

The topography of the site varies from gently to moderately sloping generally to the east and north with some steeper slopes along the drainages in portions of the site. No signs of unstable or potentially unstable slopes were observed within Filing No. 4.

Shallow Bedrock - Constraint

Bedrock was encountered in ten of the test borings and one test pit located within Filing No. 4 at depths ranging from the 7 to 19 feet in the borings and at 2.5 feet in Test Pit No. 1. A Summary of the Depth to Bedrock is included in Table B-1. Shallow bedrock will be encountered across the majority of this site. Where shallow bedrock is encountered, excavation/grading may be difficult requiring track-mounted excavators with ripper attachments. Bedrock will likely be encountered in cuts for utility excavations.

Radon – Hazard

Radon levels for the area have been reported by the Colorado Geologic Survey in the open file, Report No. 91-4 (Reference 13). Average Radon levels for the 80908-zip code is 3.40 pCi/l. The following is a table of radon levels in this area:

Average Radon Levels for the 80908 Zip Code			
0 < 4 pCi/L	50.00%		
4 < 10 pCi/L	50.00%		
10 < 20 pCi/L	0.00%		
> 20 pCi/L	0.00%		

Mitigation:

The potential for high radon levels is present for the site. Build-up of radon gas can usually be mitigated by providing increased ventilation of basement and crawlspace and sealing joints. Specific requirements for mitigation should be based on site specific testing.

6.1 Relevance of Geologic Conditions to Land Use Planning

The development will be single-family rural residential utilizing individual water wells and OWTS. It is our opinion that the existing geologic and engineering geologic conditions will impose some constraints on the proposed development and construction. The most significant problems



affecting development will be those associated with the drainages on site that can be avoided or mitigated. Other hazards on site may be satisfactorily mitigated through proper engineering design and construction practices.

The upper residual soils are typically at medium to very dense states. The granular soils encountered in the upper soil profiles of the test borings should provide good support for foundations. Expansive soils were encountered on portions of the site that will require mitigation. Foundations anticipated for the site are standard spread footings possibly in conjunction with overexcavation in areas of expansive soils. Areas containing arkosic sandstone will have high allowable bearing conditions. Difficult excavation should be anticipated in areas of shallow bedrock. Expansive layers may also be encountered in the soil and bedrock on this site. Areas of expansive soils encountered on site are sporadic; therefore, none have been indicated on the maps. Expansive soils, if encountered, will require special foundation design and/or overexcavation. These soils will not prohibit development.

Areas of seasonal and potentially seasonal high groundwater areas and seasonally ponded water were encountered on the site. Due to the size of the lots and the proposed development, these areas can be avoided by construction or properly mitigated. Absorption fields are not recommended in these areas. Structures should not block drainages. Drains may be necessary for structures adjacent to these areas to help prevent the intrusion of water into areas below grade. Buildings should maintain a minimum separation of 3 feet between the lowest foundation grade and the maximum anticipated groundwater level. Shallow groundwater areas can be mitigated with the installation of drains. Typical drain options/details are presented in Figures 9 through 12. The site is not mapped within any floodplain zones according to FEMA Map No. 08041CO315G, dated December 7, 2018 (Figure 7, Reference 11). Exact locations of floodplain and specific drainage studies are beyond the scope of this report.

Areas of fill were observed on site associated with erosion berms, embankments, and areas of man-placed fill piles. It is anticipated the fill piles/erosion berms will be mitigated during site grading. Any uncontrolled fill encountered beneath foundations should be removed and recompacted at a minimum of 95% of its maximum Modified Proctor Dry Density, ASTM D-1557.

Areas of erosion and gullying may require the construction of check dams and revegetation if construction encroaches on these areas. General recommendations for erosion control are discussed under Section 8 "Erosion Control".



In summary, development of the site can be achieved if the items mentioned above are mitigated. These items can be mitigated through proper design and construction or through avoidance. Investigation on each lot is also recommended prior to construction.

7 ECONOMIC MINERAL RESOURCES

Some of the sandy materials on-site could be considered a low grade sand resource. According to the *El Paso County Aggregate Resource Evaluation Map* (Reference 14), portions of the area are mapped as stream terrace and floodplain deposits. According to the *Atlas of Sand, Gravel and Quarry Aggregate Resources, Colorado Front Range Counties* distributed by the Colorado Geological Survey (Reference 15), areas of the site are not mapped with any resources. According to the *Evaluation of Mineral and Mineral Fuel Potential* (Reference 16), the area of the site has been mapped as "Little or No Potential" for industrial minerals. It is possible sand materials on site could be an aggregate resource. However, considering the silty to clayey nature of much of these materials and abundance of similar materials through the region and the close proximity to developed land, they would be considered to have little significance as an economic resource.

According to the Evaluation of Mineral and Mineral Fuel Potential of El Paso County State Mineral Lands (Reference 16), the site is mapped within the Denver Basin Coal Region. However, the area of the site has been mapped as "Poor" for coal resources. No active or inactive mines have been mapped in the area of the site. No metallic mineral resources have been mapped on the site (Reference 16).

The site has been mapped as "Fair" for oil and gas resources (Reference 16). No oil or gas fields have been discovered in the area of the site. The sedimentary rocks in the area may lack the geologic structure for trapping oil or gas; therefore, it may not be considered a significant resource. Hydraulic fracturing is a new method that is being used to extract oil and gas from rocks. It utilizes pressurized fluid to extract oil and gas from rocks that would not normally be productive. The area of the site has not been explored to determine if the rocks underlying the site would be commercially viable utilizing hydraulic fracturing. The practice of hydraulic fracturing has come under review due to concerns about environmental impacts, health and safety.



8 EROSION CONTROL

The soil types observed on the site are mildly to highly susceptible to wind erosion, and moderately to highly susceptible to water erosion. A minor wind erosion and dust problem may be created for a short time during and immediately after construction. Should the problem be considered severe enough during this time, watering of the cut areas or the use of chemical palliative may be required to control dust. However, once construction has been completed and vegetation re-established, the potential for wind erosion should be considerably reduced.

With regard to water erosion, loosely compacted soils will be the most susceptible to water erosion, residually weathered soils and weathered bedrock materials become increasingly less susceptible to water erosion. For the typical soils observed on site, allowable velocities or unvegetated and unlined earth channels would be on the order of 3 to 4 feet/second, depending upon the sediment load carried by the water. Permissible velocities may be increased through the use of vegetation to something on the order of 4 to 7 feet/second, depending upon the type of vegetation established. Should the anticipated velocities exceed these values, some form of channel lining material may be required to reduce erosion potential. These might consist of some of the synthetic channel lining materials on the market or conventional riprap. In cases where ditch-lining materials are still insufficient to control erosion, small check dams or sediment traps may be required. The check dams will serve to reduce flow velocities, as well as provide small traps for containing sediment. The determination of the amount, location and placement of ditch linings, check dams and of the special erosion control features should be performed by or in conjunction with the drainage engineer who is more familiar with the flow quantities and velocities.

Cut and fill slope areas will be subjected primarily to sheetwash and rill erosion. Unchecked rill erosion can eventually lead to concentrated flows of water and gully erosion. The best means to combat this type of erosion is, where possible, the adequate re-vegetation of cut and fill slopes. Cut and fill slopes having gradients more than three (3) horizontal to one (1) vertical become increasingly more difficult to revegetate successfully. Therefore, recommendations pertaining to the vegetation of the cut and fill slopes may require input from a qualified landscape architect and/or the Soil Conservation Service.



9 ROADWAY, EMBANKMENT, and STORM WATER FACILITY CONSTRUCTION RECOMMENDATIONS

In general, the site soils are suitable for the proposed roadways and embankments. Groundwater is not anticipated to affect roadway or pond construction. If road or embankment excavations encroach on the groundwater level unstable soil conditions may be encountered. Unstable soils are not anticipated in areas of shallow bedrock. Excavation of saturated soils will be difficult with rubber-tired equipment. Stabilization using shot rock or geogrids may be necessary.

Any areas to receive fill should have all topsoil, organic material or debris removed. Prior to fill placement Entech should observe the subgrade. Fill must be properly benched and compacted to minimize potentially unstable conditions in slope areas. Fill slopes should be 3:1. The subgrade should be scarified and moisture conditioned to within 2% of optimum moisture content and compacted to a minimum of 95% of its maximum Modified Proctor Dry Density, ASTM D-1557, prior to placing new fill. Areas receiving fill may require stabilization with rock or fabric if shallow groundwater conditions are encountered.

New fill should be placed in thin lifts not to exceed 6 inches after compaction while maintaining at least 95% of its maximum Modified Proctor Dry Density, ASTM D-1557. These materials should be placed at a moisture content conducive to compaction, usually 0 to ±2% of Proctor optimum moisture content. The placement and compaction of fill should be observed and tested by Entech during construction. Entech should approve any import materials prior to placing or hauling them to the site. Additional investigation will be required for pavement designs once roadway grading is completed and utilities are installed.



10 CLOSURE

It is our opinion that the existing geologic engineering and geologic conditions will impose some constraints on development and construction of the site. The majority of these conditions can be mitigated through proper engineering design and construction practices. The proposed development and use are consistent with anticipated geologic and engineering geologic conditions.

It should be pointed out that because of the nature of data obtained by random sampling of such variable and non-homogeneous materials as soil and rock, it is important that we be informed of any differences observed between surface and subsurface conditions encountered in construction and those assumed in the body of this report. Individual investigations for building sites will be required prior to construction. Construction and design personnel should be made familiar with the contents of this report. Reporting such discrepancies to Entech Engineering, Inc. soon after they are discovered would be greatly appreciated and could possibly help avoid construction and development problems.

This report has been prepared for Flying Horse North, LLC for application to the proposed project in accordance with generally accepted geologic soil and engineering practices. No other warranty expressed or implied is made.

We trust that this report has provided you with all the information that you required. Should you require additional information, please do not hesitate to contact Entech Engineering, Inc.

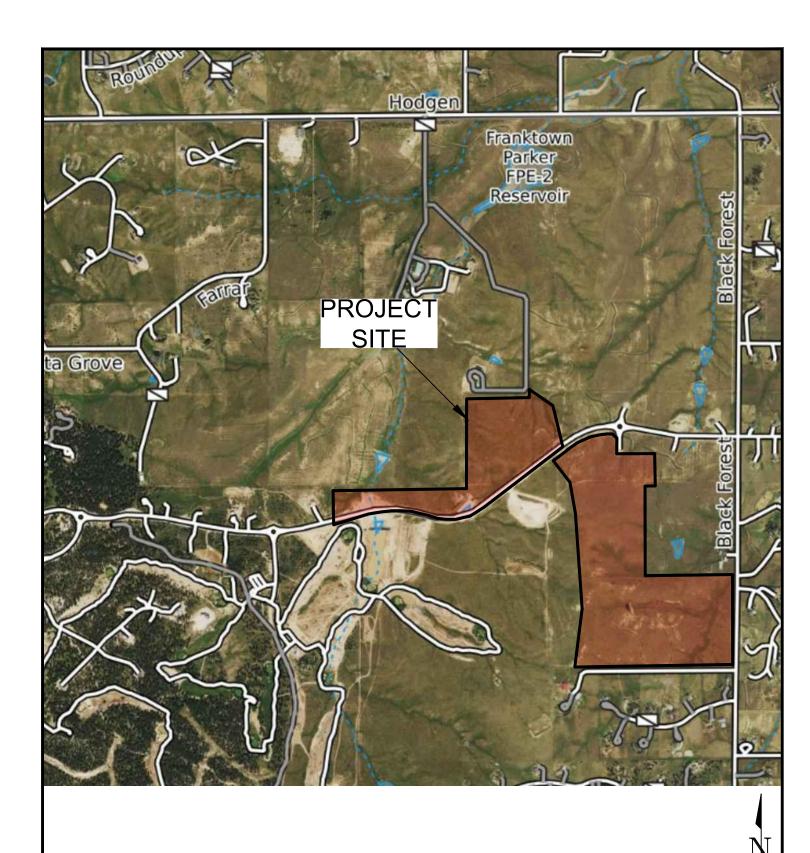


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- 5. Natural Resource Conservation *Service*, June 20, 2007. *Web Soil Survey*. United States Department Agriculture, hhtp://web soil survey.nrcs.usda.gov.
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- 11. Federal Emergency Management Agency. December 7, 2018. Flood Insurance Rate Maps for the City of Colorado Springs, Colorado. Map Number 08041CO315G.
- 12. U.S. Fish & Wildlife Service. *National Wetlands Inventory*. Department of the Interior, fws.gov/wetlands/data/Mapper.html.
- 13. Colorado Geological Survey. 1991. Results of the 1987-88 EPA Supported Radon Study in Colorado. Open-file Report 91-4.
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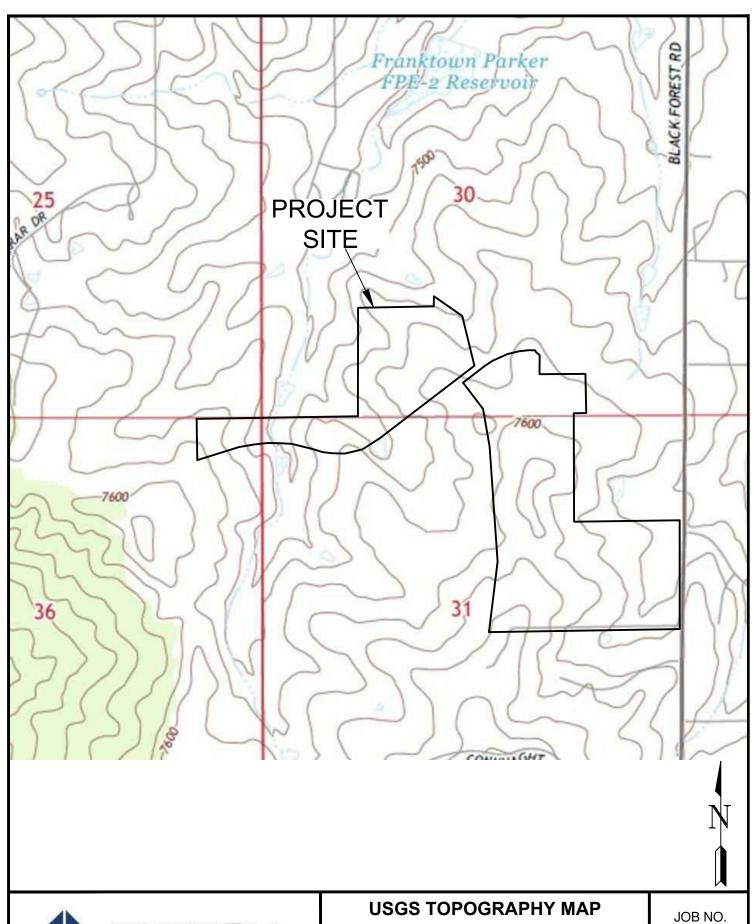
FIGURES





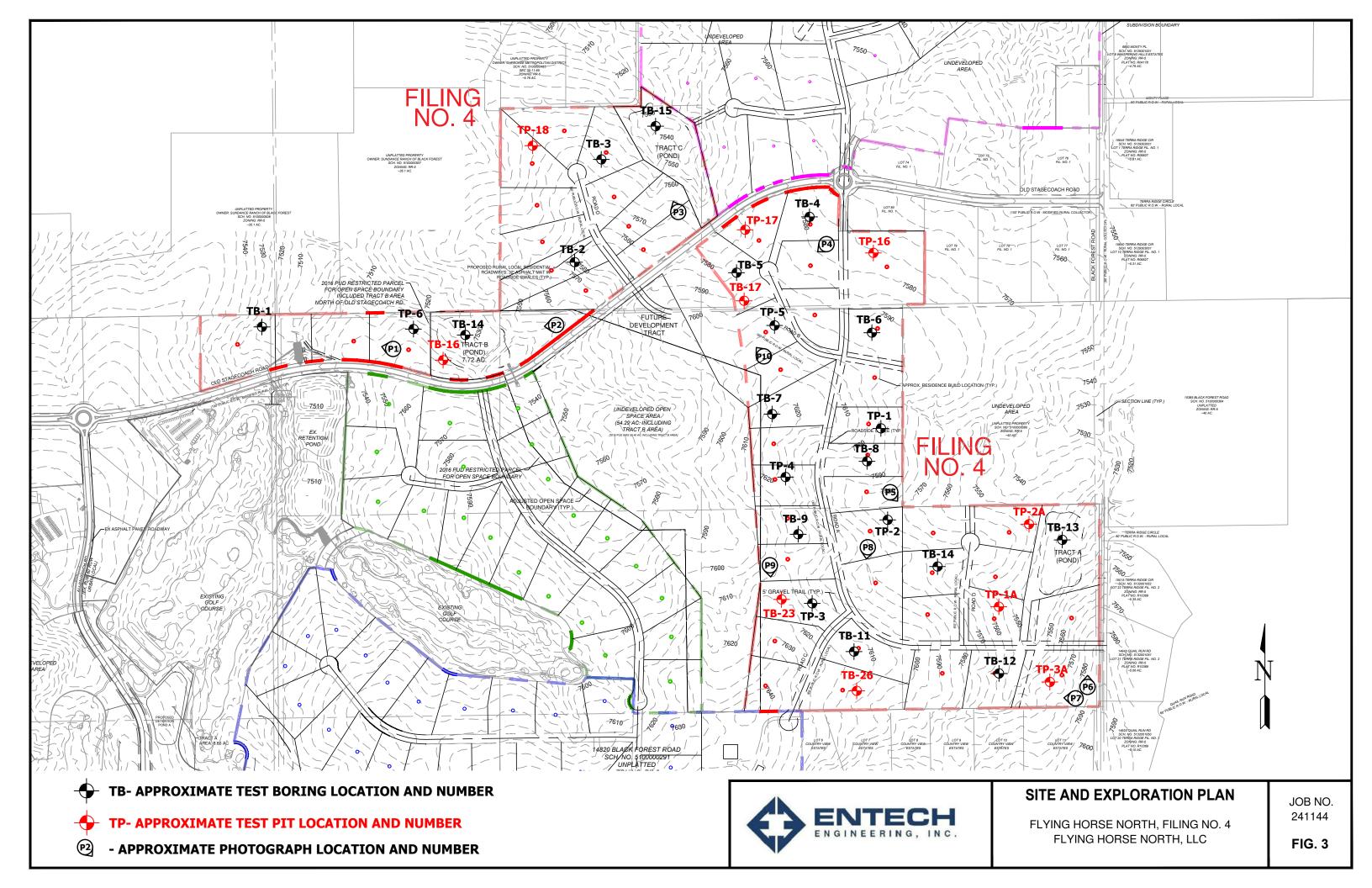
VICINITY MAP

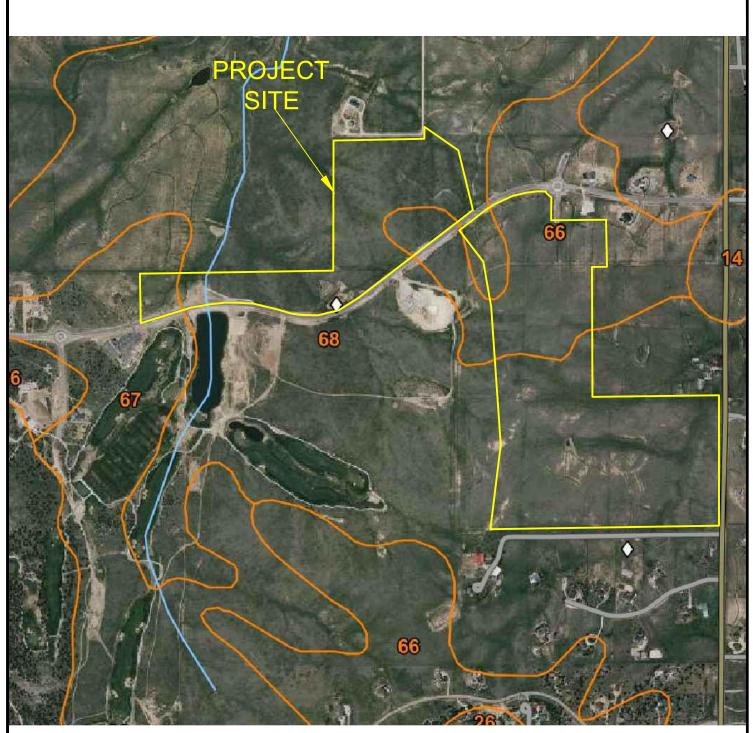
FLYING HORSE NORTH FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





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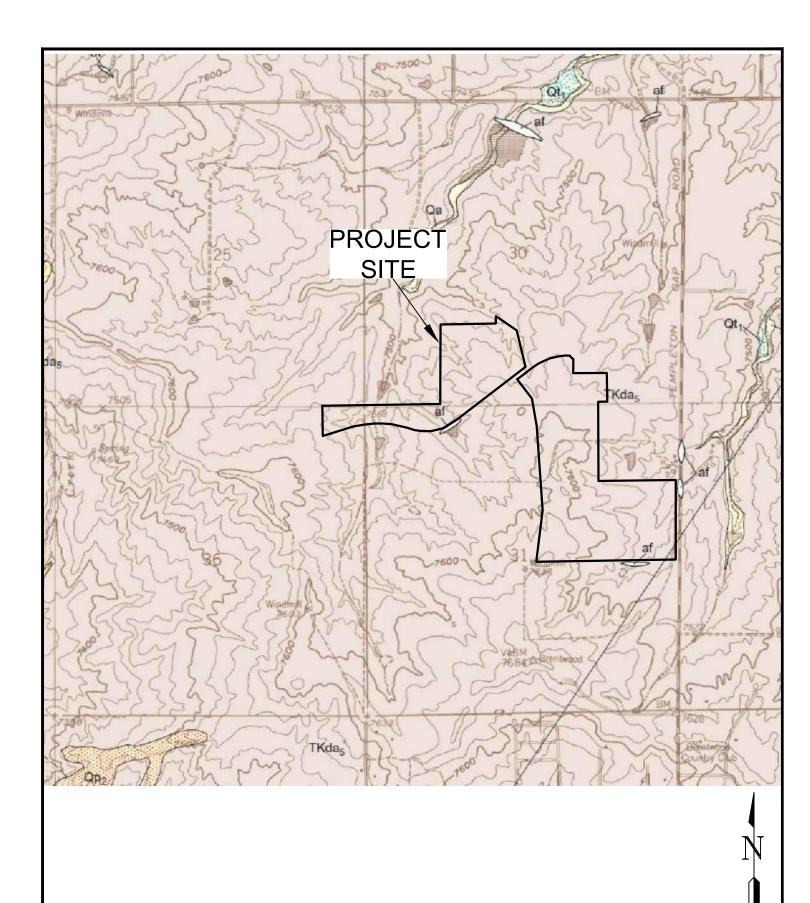






SOIL SURVEY MAP

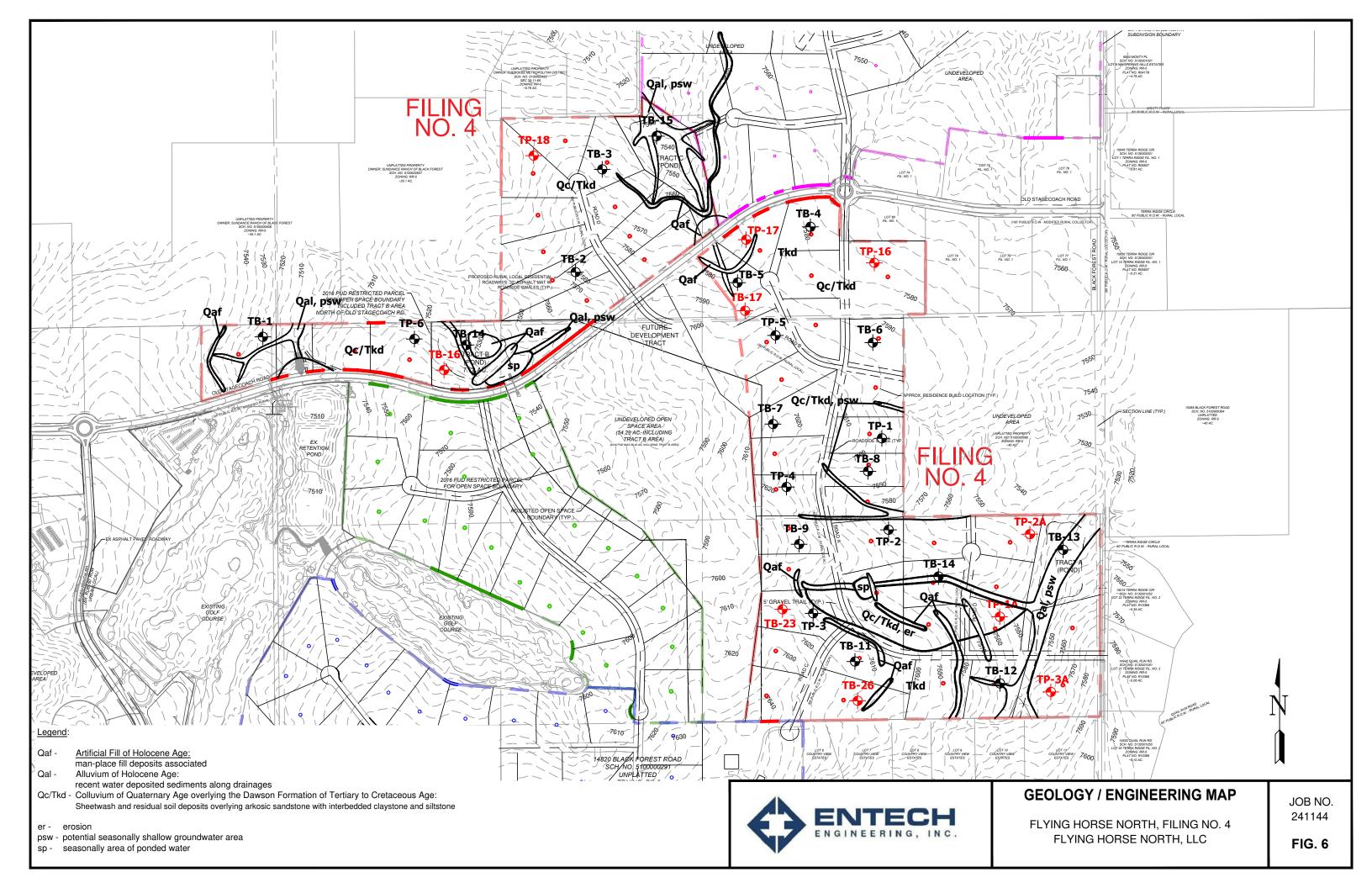
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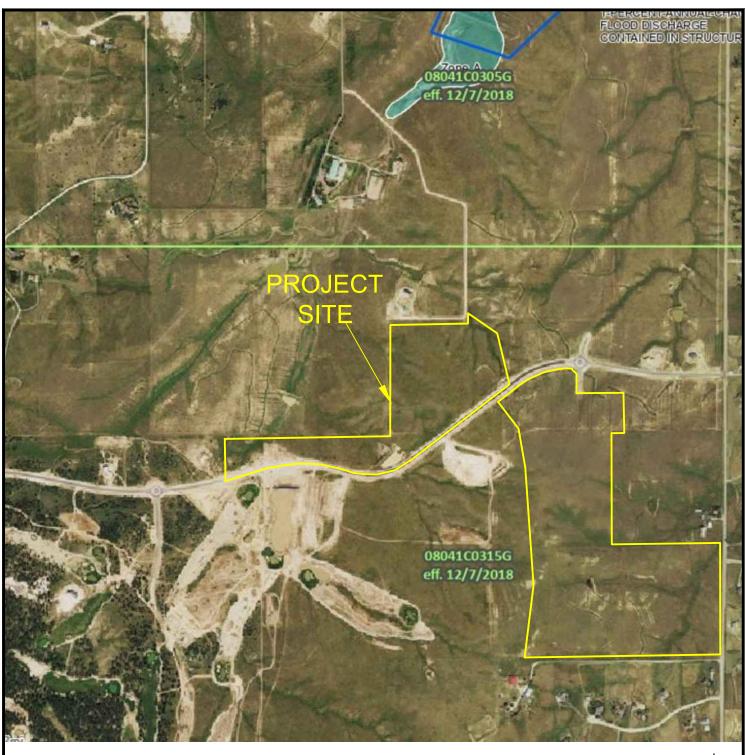




GEOLOGIC MAP OF THE BLACK FOREST QUADRANGLE

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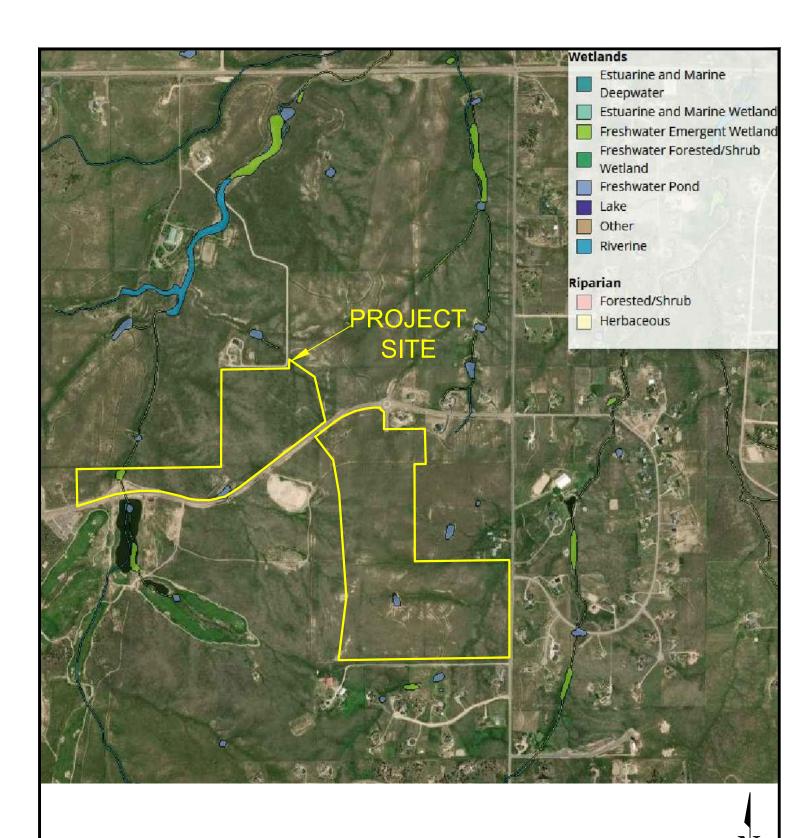






FEMA FLOODPLAIN MAP

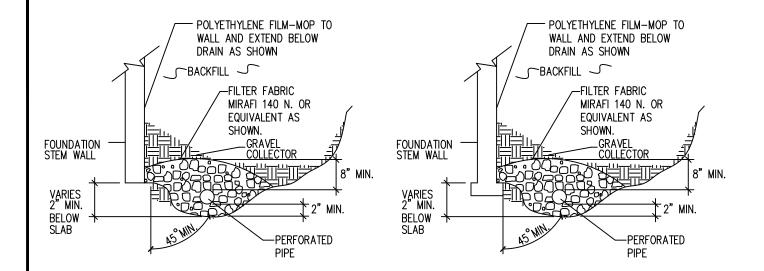
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U.S. FISH AND WILDLIFE SERVICES NATIONAL WETLANDS INVENTORY

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144



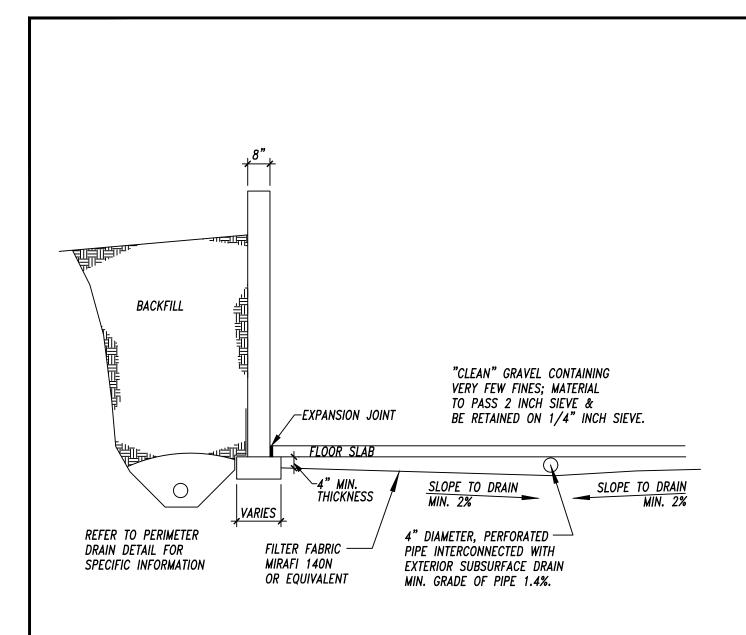
NOTES:

- -GRAVEL SIZE IS RELATED TO DIAMETER OF PIPE PERFORATIONS-85% GRAVEL GREATER THAN 2x PERFORATION DIAMETER.
- -PIPE DIAMETER DEPENDS UPON EXPECTED SEEPAGE. 4-INCH DIAMETER IS MOST OFTEN USED.
- -ALL PIPE SHALL BE PERFORATED PLASTIC. THE DISCHARGE PORTION OF THE PIPE SHOULD BE NON-PERFORATED PIPE.
- -FLEXIBLE PIPE MAY BE USED UP TO 8 FEET IN DEPTH, IF SUCH PIPE IS DESIGNED TO WITHSTAND THE PRESSURES. RIGID PLASTIC PIPE WOULD OTHERWISE BE REQUIRED.
- -MINIMUM GRADE FOR DRAIN PIPE TO BE 1% OR 3 INCHES OF FALL IN 25 FEET.
- -DRAIN TO BE PROVIDED WITH A FREE GRAVITY OUTFALL, IF POSSIBLE. A SUMP AND PUMP MAY BE USED IF GRAVITY OUT FALL IS NOT AVAILABLE.



PERIMETER DRAIN DETAIL

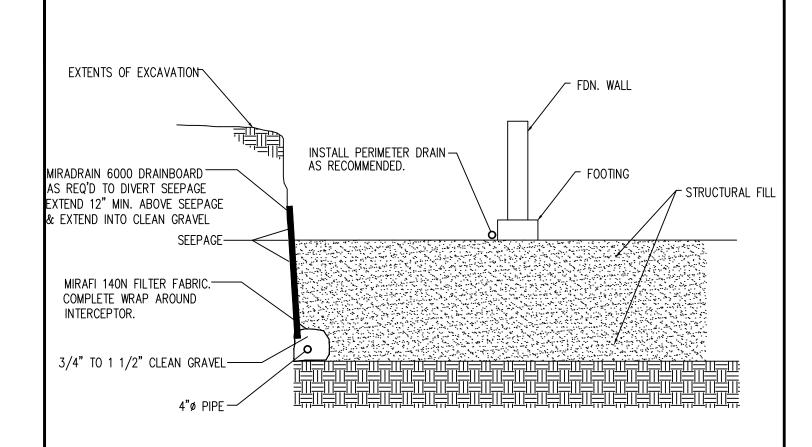
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TYP. UNDERSLAB DRAINAGE LAYER (CAPILLARY BREAK)

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144



NOTE:

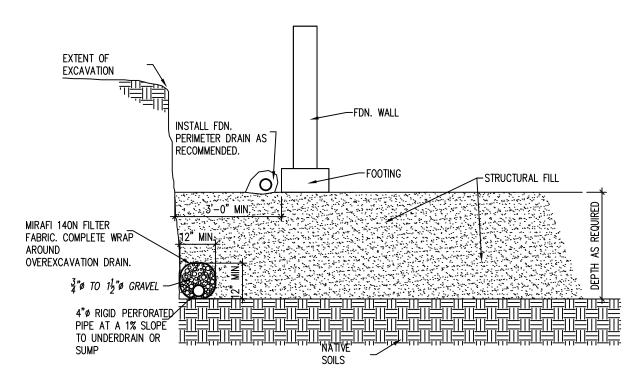
EXTEND INTERCEPTOR DRAIN TO UNDERDRAIN OR TO SUMP. BENCH DRAIN INTO NATIVE SOILS 12 INCHES MINIMUM.

INTERCEPTOR DRAIN DETAIL N.T.S.



INTERCEPTOR DRAIN DETAIL

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144



OVEREXCAVATION DRAIN DETAIL

N.T.S.

NOTE:

EXTEND DRAIN TO SUMP AS REQ'D.



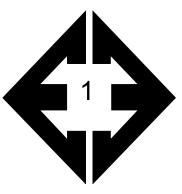
OVEREXCAVATION DRAIN

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144



APPENDIX A: Site Photographs

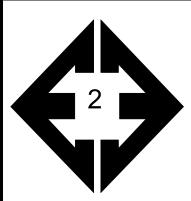




Looking west from the northwestern portion of the site.

July 11, 2024





Looking west from the northern portion of the site towards Tract B.

July 11, 2024

Job No. 241144

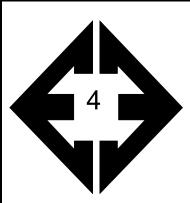




Looking northwest from the northeastern portion of the site.

July 11, 2024





Looking southwest from the northeastern portion of the site.

July 11, 2024

Job No. 241144

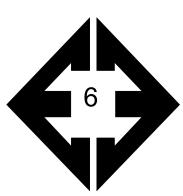




Looking southeast from the southeastern portion of the site.

July 11, 2024



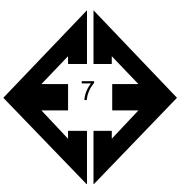


Looking north from the southeast corner of the site.

July 11, 2024

Job No. 241144

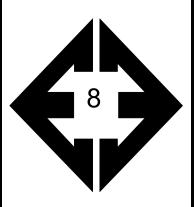




Looking west from the southeast corner of the site.

July 11, 2024



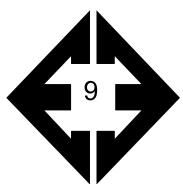


Looking southwest along existing earthen dam and pond in the southern portion of the site.

August 22, 2024

Job No. 241144

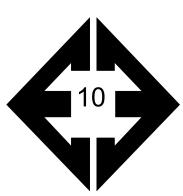




Looking south from the southwestern portion of the site.

July 11, 2024





Looking northwest from the west-central portion of the site.

August 22, 2024

Job No. 241144



APPENDIX B: Test Boring and Test Pit Logs

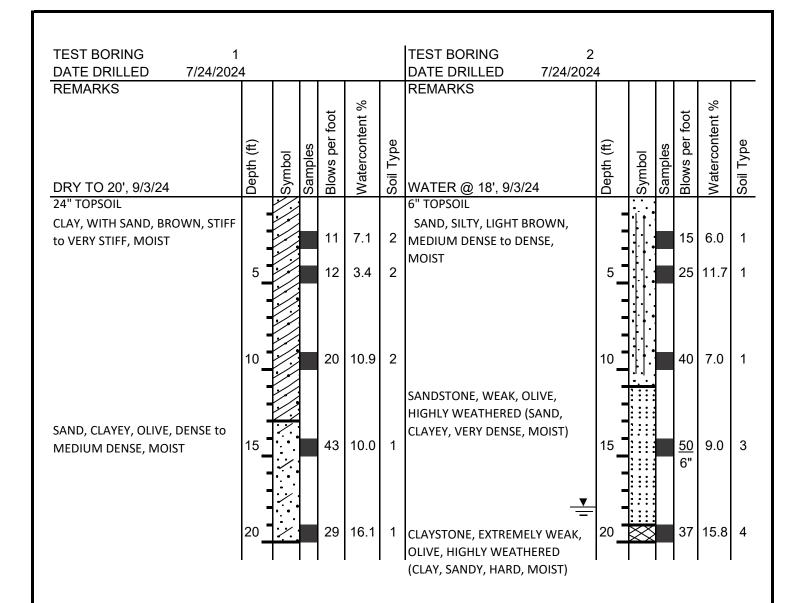


TABLE B-1 DEPTH TO BEDROCK

TEST BORING	DEPTH TO BEDROCK (ft.)	DEPTH TO GROUNDWATER (ft.)
1	>20	>20
2	11	18
3	17	>20
4	9	>20
5	12	>20
6	7	>20
7	16	>20
8	>20	>20
9	>20	>20
10	13	>20
11	>20	>20
12	>20	>20
13	11	>20
14	18	>20
15	12	>20

Project: Flying Horse North, Filing No. 4 Client: Flying Horse North

Job No: 241144





FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

TEST BORING 3 DATE DRILLED 7/24/202							TEST BORING 4 DATE DRILLED 7/24/202	•				
REMARKS	4		1				REMARKS	:4				Т
DRY TO 20', 9/3/24	Depth (ft)		Samples	Blows per foot	Watercontent %	Soil Type	DRY TO 20', 9/3/24	Depth (ft)	Symbol	Samples Blows per foot	Watercontent %	Soil Type
6" TOPSOIL	_	°.0					12" TOPSOIL	-				
SAND, GRAVELLY, WITH SILT, LIGHT BROWN to OLIVE,	-	0		22	4.6	1	CLAY, SANDY, BROWN, STIFF to SOFT, MOIST	-		1	6.4	2
MEDIUM DENSE to DENSE, MOIST	5_	, 0		29	5.8	1		5_		4	8.3	2
	10_	0		32	6.4	1	SANDSTONE, VERY WEAK, OLIVE, HIGHLY WEATHERED (SAND, SILTY, VERY DENSE, MOIST)	10		<u>5</u>		3
	15_	00		44	8.0	1		15_		5 1		3
SANDSTONE, WEAK, OLIVE, HIGHLY WEATHERED (SAND, SILTY, VERY DENSE, MOIST)	20_			<u>50</u> 8"	7.5	3		20_		<u>5</u>		3



FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

TEST BORING 5						TEST BORING 6	6					
DATE DRILLED 7/24/2024	4					DATE DRILLED 7/29/202	24					
REMARKS						REMARKS					_	
DRY TO 20', 9/3/24	Depth (ft)	Symbol	Samples Blows per foot	Watercontent %	Soil Type	DRY TO 20', 9/3/24	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
12" TOPSOIL		<u> </u>	מן מ) <u>></u>	(y)	12" TOPSOIL		S	ഗ	Ш	>	<u>o</u>
CLAY, SANDY, BROWN, STIFF, MOIST			1:	3 9.1	2	CLAY, SANDY, BROWN, STIFF, MOIST	-			13	10.7	2
	5		1	1 8.7	2		5_			10	11.5	2
SAND, CLAYEY, OLIVE, MEDIUM DENSE, MOIST	10		2:	5 3.2	1	SANDSTONE, EXTREMELY WEAK, BROWN to OLIVE, COMPLETELY WEATHERED (SAND, CLAYEY, VERY DENSE, MOIST)	10_			50 10"	3.3	3
SANDSTONE, EXTREMELY WEAK, OLIVE, COMPLETELY WEATHERED (SAND, CLAYEY, VERY DENSE, MOIST)	15_		<u>5</u>		3		15			50	4.5	3
	20		3	9 6.9	3		20_			<u>50</u> 8"	7.6	3



FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

TEST BORING 7						TEST BORING 8						
DATE DRILLED 7/29/202						DATE DRILLED 7/24/202						
REMARKS						REMARKS						
DRY TO 20', 9/3/24	Depth (ft)	Symbol	Blows per foot	Watercontent %	Soil Type	DRY TO 20', 9/3/24	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
12" TOPSOIL	4					12" TOPSOIL	_					
CLAY, SANDY, BROWN to OLIVE, STIFF, MOIST	-{		9	8.8	2	CLAY, SANDY, BROWN, STIFF, MOIST	-			13	7.6	2
	5_		10	13.6	2		5_		•	15	8.3	2
SAND, CLAYEY, OLIVE, MEDIUM DENSE, MOIST	10		27	9.1	1	SAND, CLAYEY, LIGHT BROWN to BROWN, MEDIUM DENSE to DENSE, MOIST	10	// / / /	,	15	4.3	1
CLAY, SANDY, OLIVE, VERY STIFF, MOIST	15		26	13.5	2		15	/ ./.		26	8.0	1
SANDSTONE, MEDIUM STRONG, TAN, MODERATELY WEATHERED (SAND, CLAYEY, VERY DENSE, MOIST)	20		<u>50</u> 4"	7.4	3		20	<i>/</i> .		45	7.5	1



FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

TEST BORING 9 DATE DRILLED 8/5/2024							TEST BORING 10 DATE DRILLED 8/5/2024					
REMARKS DRY TO 20', 9/3/24	Depth (ft)	Symbol	Samples	Blows per toot	Watercontent %	Soil Type	REMARKS DRY TO 20', 9/3/24	Depth (ft)	Symbol	Samples Blows per foot	Watercontent %	Soil Type
12" TOPSOIL SAND, SILTY, BROWN, MEDIUM DENSE to DENSE, MOIST to DRY	5		_	13	5.9 2.8		12" TOPSOIL SAND, CLAYEY, BROWN to OLIVE, DENSE to MEDIUM DENSE, MOIST	5_	/. /.	3 2	1 5.2	1
CLAY, SANDY, BROWN, VERY STIFF, MOIST SAND, SILTY, LIGHT BROWN to	10			17	7.3	2		10		3	6 8.8	1
OLIVE, MEDIUM DENSE to DENSE, MOIST	15_			21	5.0	1	SANDSTONE, VERY WEAK, OLIVE, HIGHLY WEATHERED (SAND, CLAYEY, VERY DENSE, MOIST)	15		5 <u>5</u>		3
	20_	<i>-</i> /-	4	46	5.7	1		20_		<u>5</u>	_	3

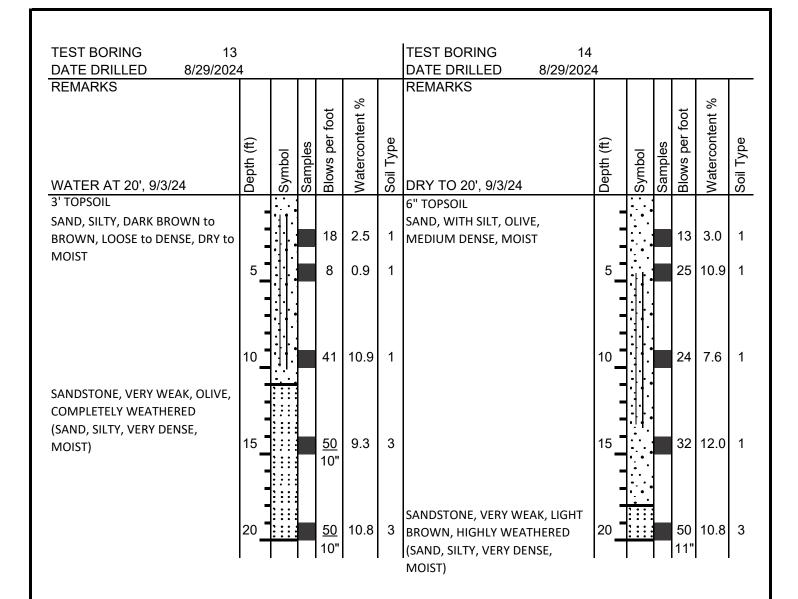


FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

TEST BORING 11 DATE DRILLED 8/5/2024							TEST BORING 12 DATE DRILLED 8/5/2024						
REMARKS							REMARKS						
DRY TO 20', 9/3/24	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	DRY TO 20', 9/3/24	Jepth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
12" TOPSOIL		0,	(O	ш	_>_	0)	12" TOPSIL		900	0)	<u>ш</u>	>	0)
CLAY, SANDY, BROWN, STIFF, MOIST	-			11	10.6	2	CLAY, WITH SAND, BROWN, STIFF, MOIST	- -			9	8.1	2
	5			12	6.7	2	SAND, SILTY, BROWN, MEDIUM DENSE, DRY	5			25	2.6	1
SAND, SILTY, OLIVE, MEDIUM DENSE to DENSE, MOIST	10			29	4.4	1	CLAY, SANDY, OLIVE to BROWN, VERY STIFF to HARD, MOIST	10			16	14.1	2
	15			32	5.6	1		15			47	15.5	2
CLAY, SANDY, OLIVE, HARD, MOIST	20			34	10.9	2	SAND, SILTY, OLIVE, MEDIUM DENSE, MOIST	20			14	5.7	1



FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144



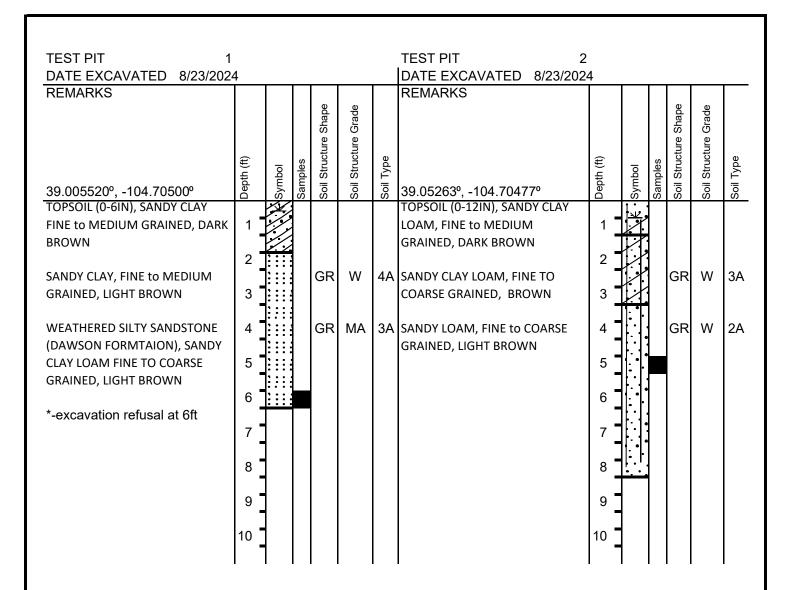


FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

TEST BORING 15 8/29/2024 DATE DRILLED REMARKS Watercontent % Blows per foot Soil Type Symbol DRY TO 20', 9/3/24 12" TOPSOIL SAND, SILTY, LIGHT BROWN to 21 3.6 1 OLIVE, MEDIUM DENSE, MOIST 23 6.4 10 27 11.3 1 SAND, CLAYEY, OLIVE, MEDIUM DENSE, MOIST SANDSTONE, VERY WEAK, OLIVE, **COMPLETELY WEATHERED** 15 <u>50</u> 8.1 3 (SAND, CLAYEY, VERY DENSE, 10" MOIST) <u>50</u> 16.8 3



TEST BORING LOGS



Soil Structure Shape

granular - gr platy - pl blocky - bl prismatic - pr single grain - sg Soil Structure Grade

weak - w moderate - m strong - s loose - l massive - ma



TEST PIT LOGS

TEST PIT 3 DATE EXCAVATED 8/23/2024	4						TEST PIT 4 DATE EXCAVATED 8/23/2024	4					
REMARKS							REMARKS						
39.05030°, -104.70699°	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	Soil Type	39.05382°, -104.70712°	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	Soil Type
TOPSOIL (0-12IN), SANDY CLAY,							TOPSOIL (0-12IN), SANDY LOAM,						
FINE TO COARSE GRAINED, DARK BROWN	1						FINE TO COARSE GRAINED, DARK BROWN						
	2 -							2					
SANDY CLAY, FINE to MEDIUM		·/,		GR	W	4A	SANDY CLAY, FINE TO COARSE				GR	W	4A
GRAINED, LIGHT BROWN	3						GRAINED, LIGHT BROWN	3					
	4							4					
	5						SANDY CLAY, FINE to MEDIUM	5			GR	MA	4A
SANDY CLAY, FINE to MEDIUM				GR	MA	4A	GRAINED, LIGHT BROWN	_					
GRAINED, LIGHT BROWN	6							6					
	7							7 -					
	8							8					
	9							9					
	10							10					

Soil Structure Shape

granular - gr platy - pl blocky - bl prismatic - pr single grain - sg Soil Structure Grade

weak - w moderate - m strong - s loose - l massive - ma



TEST PIT LOGS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

TEOT BODING							TEGE BODING						
TEST BORING 5 DATE DRILLED 8/23/202							TEST BORING 6 DATE DRILLED 8/23/202						
REMARKS	4 	I	1				DATE DRILLED 8/23/202 REMARKS	:4 					
39.05618°, -104.70669°	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	Soil Type	39.05608°, -104.71579°	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	Soil Type
TOPSOIL (0-12IN), SANDY CLAY,							TOPSOIL (0-12IN) SANDY CLAY		ربلا				
FINE TO COARSE GRAINED, DARK BROWN	1.						LOAM, FINE TO MEDIUM GRAINED, DARK BROWN	1 -	<i>/</i>				
CANDY CLAY FINE +- NAFRILINA	2						CDAVELLY CANDY CLAY LOADA	2 _	///		O D	١٨/	2.4
SANDY CLAY, FINE to MEDIUM GRAINED, LIGHT BROWN	3 -			GR	М	4	GRAVELLY, SANDY CLAY LOAM, FINE TO COARSE GRAINED,	3 -			GR	W	3A
GRAINED, LIGHT BROWN				GIV	IVI	-	BROWN	-					
	4							4 =					
SANDY CLAY, FINE to MEDIUM	5			GR	MA	4A		5					
GRAINED, LIGHT BROWN	6			O. C	1417	., \	SANDY CLAY, FINE to MEDIUM GRAINED, LIGHT BROWN	6			GR	W	4A
	7						oww.	7 -					
	-						GRAVELLY, SANDY CLAY LOAM,		<i>;</i>		GR	W	3A
	8						FINE TO COARSE GRAINED,	8	<u>;/</u>				
	9						BROWN	9					
	10							10					
	I	l	l			I		I					I

Soil Structure Shape

granular - gr platy - pl blocky - bl prismatic - pr single grain - sg Soil Structure Grade

weak - w moderate - m strong - s loose - I massive - ma



TEST PIT LOGS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

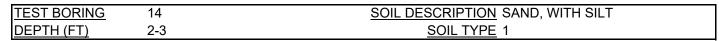


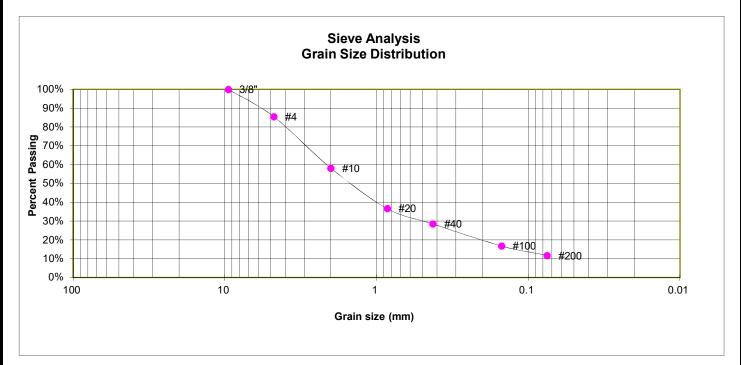
APPENDIX C: Laboratory Testing Results



TABLE C-1 SUMMARY OF LABORATORY TEST RESULTS

SOIL TYPE	TEST BORING NO.	DEPTH (FT)	WATER (%)	DRY DENSITY (PCF)	PASSING NO. 200 SIEVE (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTIC INDEX	SULFATE (WT %)	SWELL/ CONSOL (%)	USCS	SOIL DESCRIPTION
1	14	2-3			11.8						SW-SM	SAND, WITH SILT
1	15	10			47.3						SC	SAND, CLAYEY
1	3	5			11.1	NV	NP	NP	<0.01		SW-SM	SAND, WITH SILT
1	8	15			12.4						SM	SAND, SILTY
1	11	10			12.0	26	24	2			SM	SAND, SILTY
2	1	2-3			72.5	29	22	7	0.00		CL	CLAY, WITH SAND
2	5	5			50.7						CL	CLAY, SANDY
2	7	5	13.2	104.2	77.0					0.4	CL	CLAY, WITH SAND
2	9	10	7.4	103.8	50.8	27	16	11		1.2	CL	CLAY, SANDY
2	12	2-3			70.7						CL	CLAY, WITH SAND
3	13	15			20.1						SM	SANDSTONE (SAND, SILTY)
3	4	10			41.8	32	18	14	0.00		CL	SANDSTONE (SAND, CLAYEY)
3	6	10			12.1						SM	SANDSTONE (SAND, SILTY)
3	10	15			35.1	32	21	11			CL	SANDSTONE (SAND, CLAYEY)
4	2	20			55.8	32	21	11	<0.01		CL	CLAYSTONE (CLAY, SANDY)





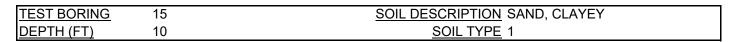
U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	85.5%
10	58.2%
20	36.7%
40	28.5%
100	16.8%
200	11.8%

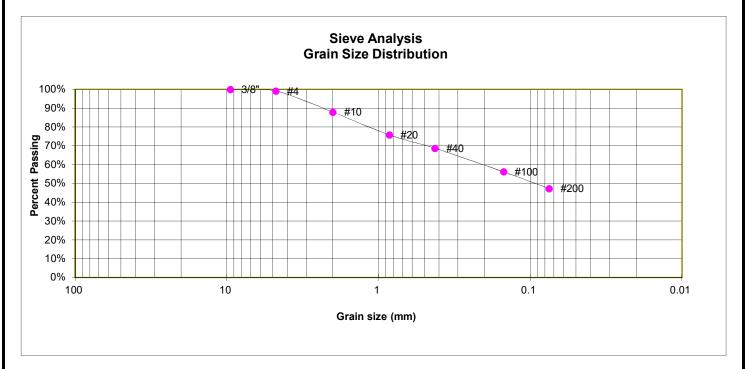
SOIL CLASSIFICATION

USCS CLASSIFICATION: SW-SM



LABORATORY TEST RESULTS





U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.1%
10	88.0%
20	75.9%
40	68.8%
100	56.3%
200	47.3%

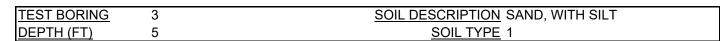
SOIL CLASSIFICATION

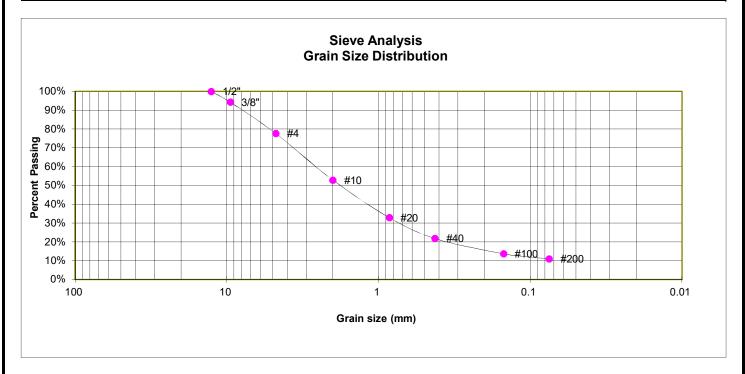
USCS CLASSIFICATION: SC



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	94.3%
4	77.6%
10	52.9%
20	33.0%
40	21.8%
100	13.7%
200	11.1%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SW-SM

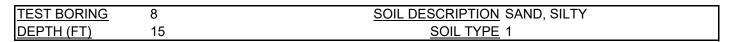
ATTERBERG LIMITS

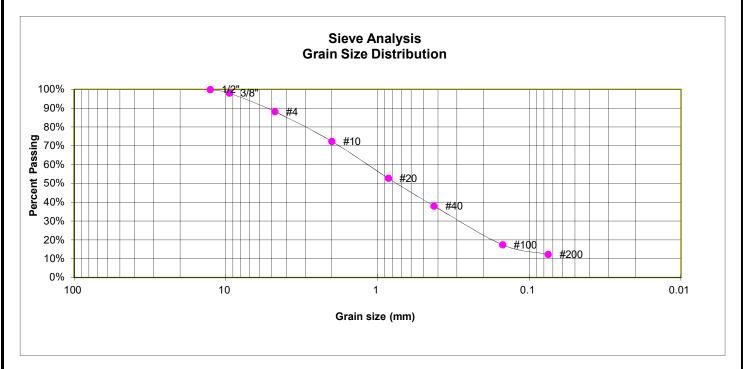
Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





Percent
<u>Finer</u>
100.0%
98.1%
88.2%
72.4%
52.8%
38.1%
17.4%
12.4%

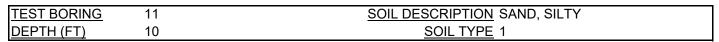
SOIL CLASSIFICATION

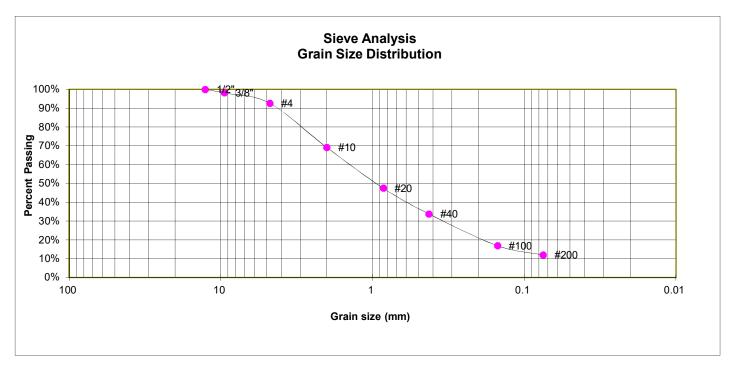
USCS CLASSIFICATION: SM



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	98.2%
4	92.6%
10	69.1%
20	47.5%
40	33.8%
100	16.9%
200	12.0%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SM

ATTERBERG LIMITS

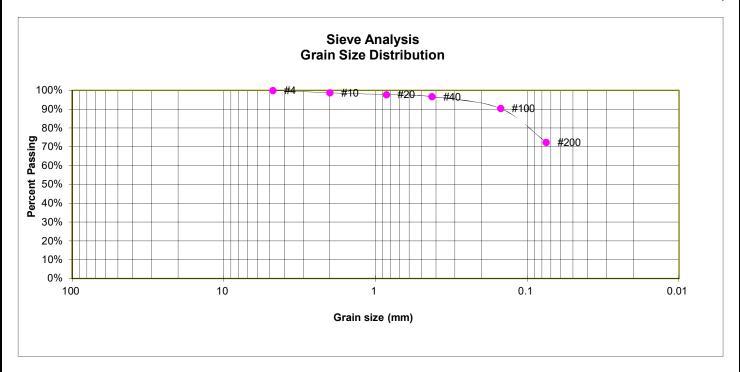
Plastic Limit	24
Liquid Limit	26
Plastic Index	2



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

TEST BORING1SOIL DESCRIPTION CLAY, WITH SANDDEPTH (FT)2-3SOIL TYPE 2



GRAIN SIZE ANALYSIS

U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	98.7%
20	97.8%
40	96.7%
100	90.4%
200	72.5%

SOIL CLASSIFICATION

USCS CLASSIFICATION: CL

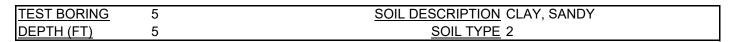
ATTERBERG LIMITS

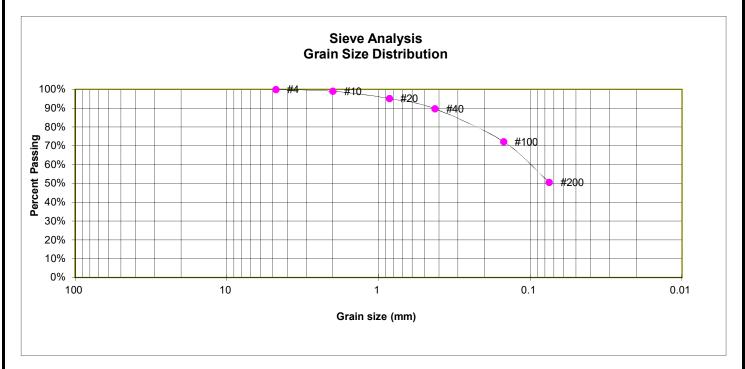
Plastic Limit	22
Liquid Limit	29
Plastic Index	7



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





Percent
<u>Finer</u>
100.0%
99.0%
95.2%
89.7%
72.3%
50.7%

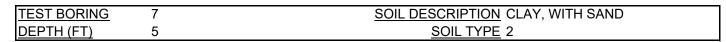
SOIL CLASSIFICATION

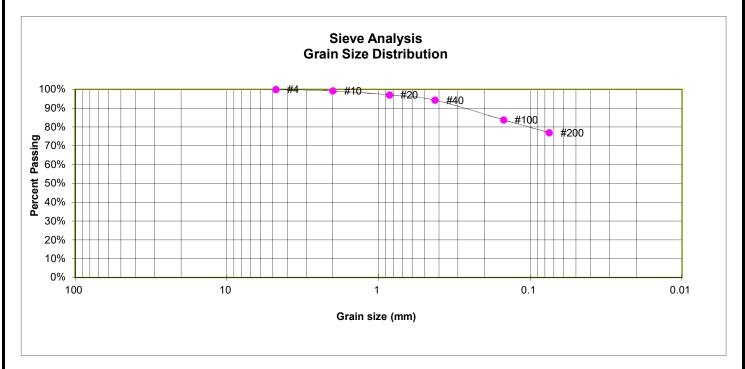
USCS CLASSIFICATION: CL



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	99.2%
20	97.1%
40	94.4%
100	83.9%
200	77.0%

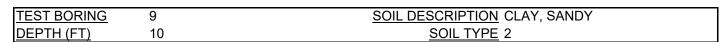
SOIL CLASSIFICATION

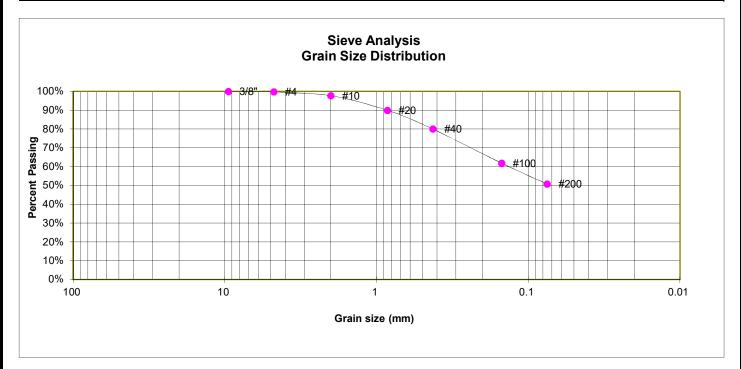
USCS CLASSIFICATION: CL



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.7%
10	97.8%
20	90.0%
40	80.0%
100	61.9%
200	50.8%

SOIL CLASSIFICATION

USCS CLASSIFICATION: CL

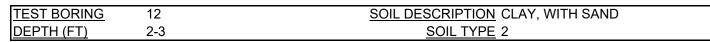
ATTERBERG LIMITS

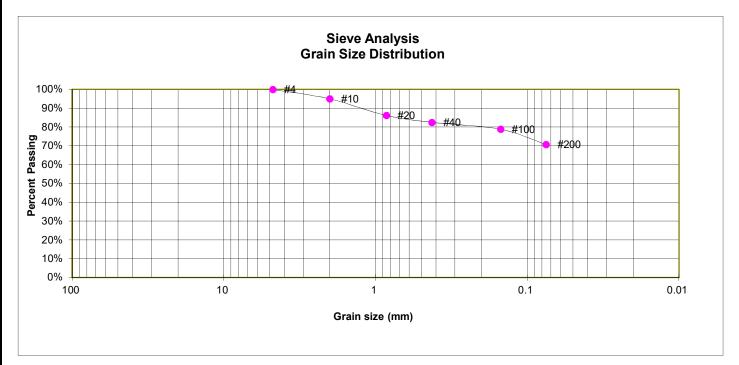
Plastic Limit	16
Liquid Limit	27
Plastic Index	11



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	95.0%
20	86.2%
40	82.5%
100	78.8%
200	70.7%

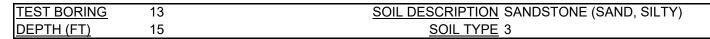
SOIL CLASSIFICATION

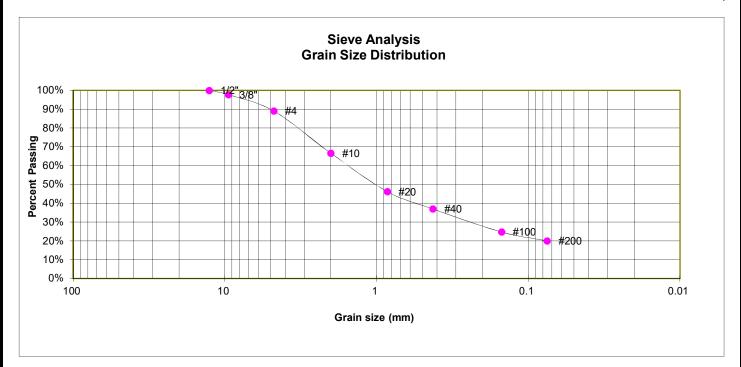
USCS CLASSIFICATION: CL



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





Percent
<u>Finer</u>
100.0%
97.8%
89.1%
66.6%
46.3%
37.0%
24.8%
20.1%

SOIL CLASSIFICATION

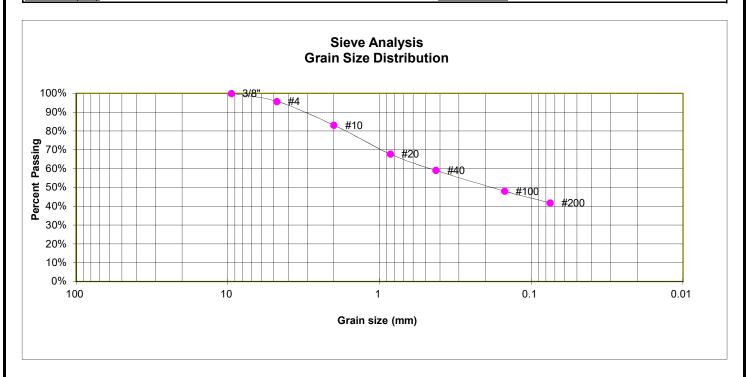
USCS CLASSIFICATION: SM



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

TEST BORING4SOIL DESCRIPTION SANDSTONE (SAND, CLAYEY)DEPTH (FT)10SOIL TYPE 3



GRAIN SIZE ANALYSIS

U.S.	Percent
Sieve#	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	95.8%
10	83.1%
20	67.8%
40	59.1%
100	48.1%
200	41.8%

SOIL CLASSIFICATION

USCS CLASSIFICATION: CL

ATTERBERG LIMITS

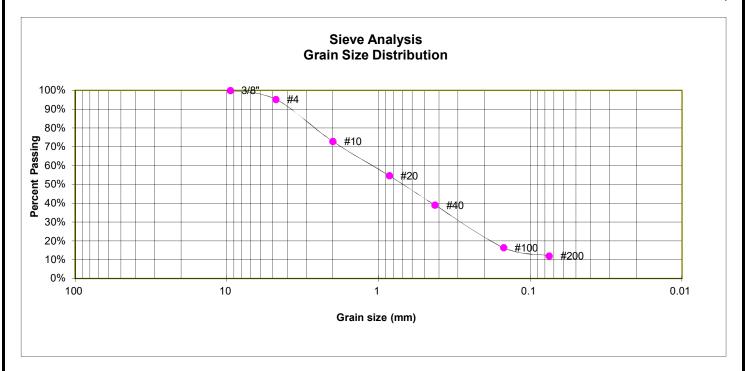
Plastic Limit	18
Liquid Limit	32
Plastic Index	14



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	95.3%
10	72.9%
20	54.7%
40	39.1%
100	16.5%
200	12.1%

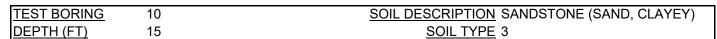
SOIL CLASSIFICATION

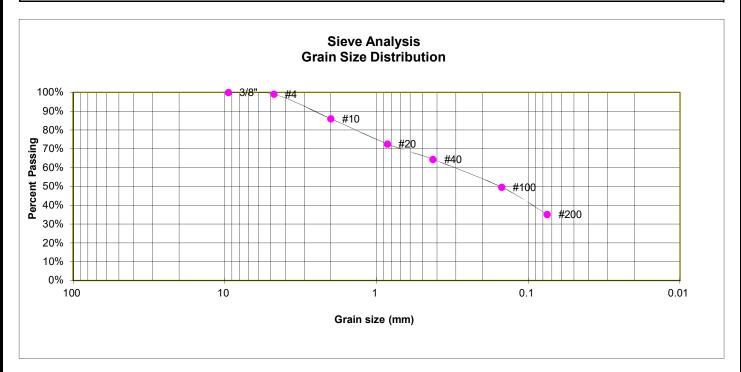
USCS CLASSIFICATION: SM



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.1%
10	86.1%
20	72.6%
40	64.4%
100	49.7%
200	35.1%

SOIL CLASSIFICATION

USCS CLASSIFICATION: CL

ATTERBERG LIMITS

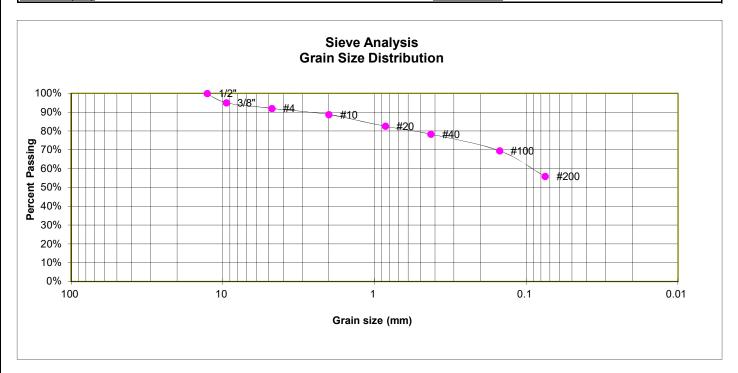
Plastic Limit	21
Liquid Limit	32
Plastic Index	11



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

TEST BORING2SOIL DESCRIPTION CLAYSTONE (CLAY, SANDY)DEPTH (FT)20SOIL TYPE 4



GRAIN SIZE ANALYSIS

U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	95.1%
4	92.1%
10	88.8%
20	82.5%
40	78.3%
100	69.5%
200	55.8%

SOIL CLASSIFICATION

USCS CLASSIFICATION: CL

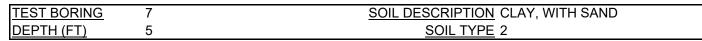
ATTERBERG LIMITS

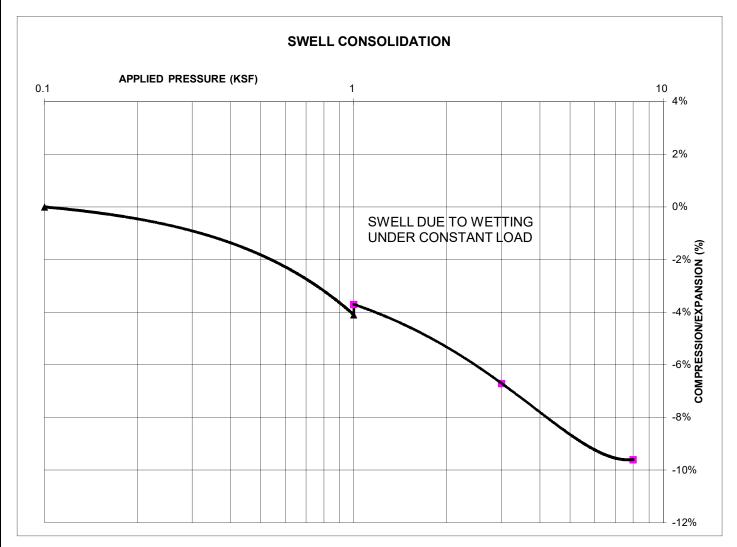
Plastic Limit	21
Liquid Limit	32
Plastic Index	11



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





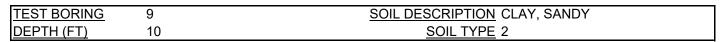
SWELL/COLLAPSE TEST RESULTS

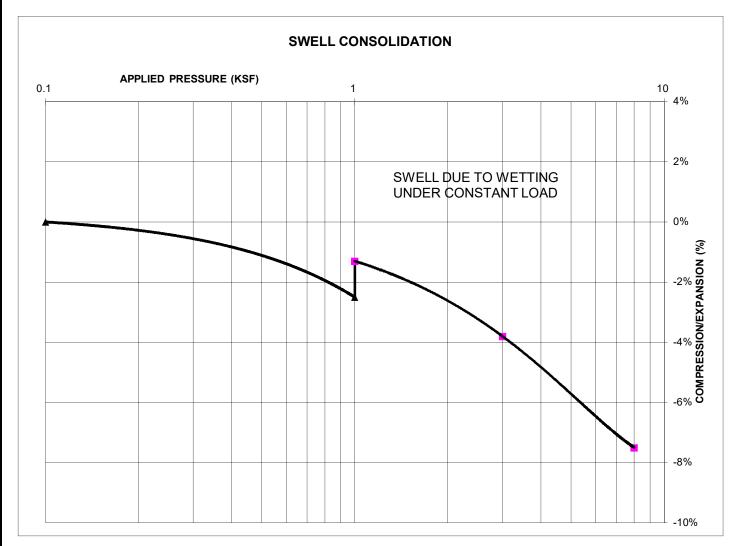
NATURAL UNIT DRY WEIGHT (PCF): 104 NATURAL MOISTURE CONTENT: 13.2% SWELL/COLLAPSE (%): 0.4%



SWELL TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





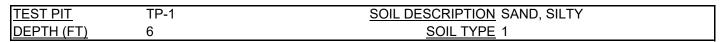
SWELL/COLLAPSE TEST RESULTS

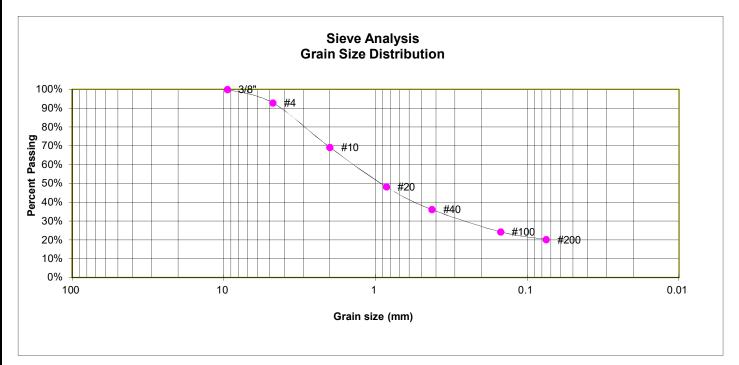
NATURAL UNIT DRY WEIGHT (PCF): 104 NATURAL MOISTURE CONTENT: 7.4% SWELL/COLLAPSE (%): 1.2%



SWELL TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	92.9%
10	69.2%
20	48.3%
40	36.2%
100	24.3%
200	20.2%

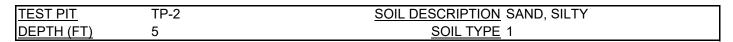
SOIL CLASSIFICATION

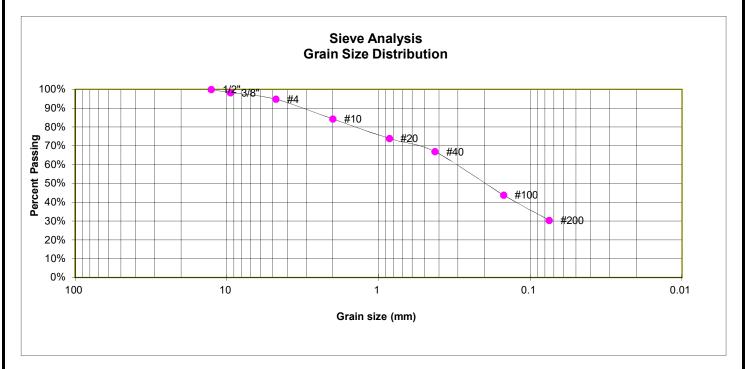
USCS CLASSIFICATION: SM



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





Percent
<u>Finer</u>
100.0%
98.3%
94.8%
84.3%
74.0%
66.9%
43.9%
30.5%

SOIL CLASSIFICATION

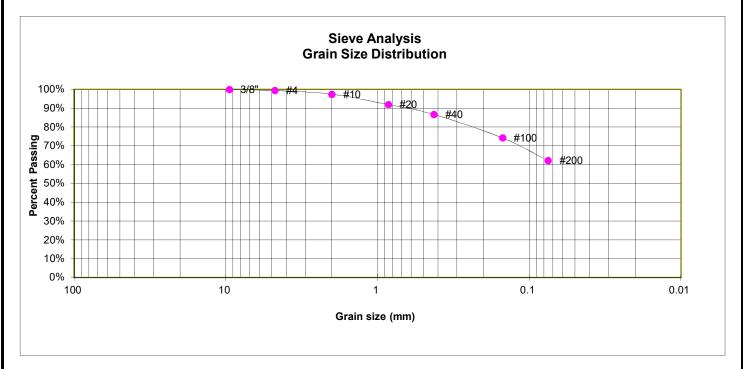
USCS CLASSIFICATION: SM



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.4%
10	97.5%
20	92.0%
40	86.7%
100	74.3%
200	62.2%

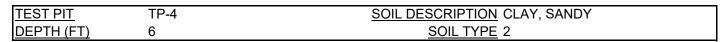
SOIL CLASSIFICATION

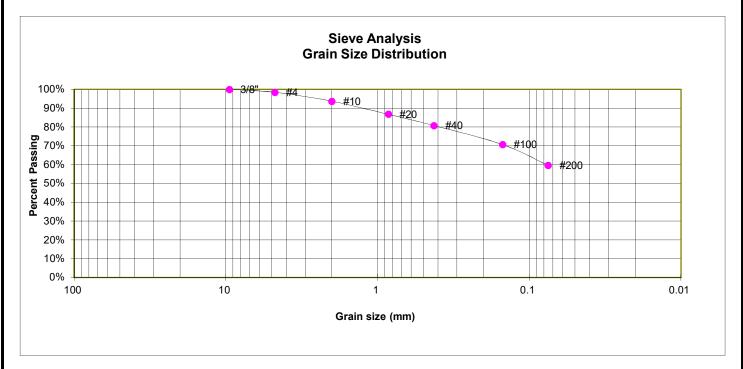
USCS CLASSIFICATION: CL



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144





GRAIN SIZE ANALYSIS

Percent
<u>Finer</u>
100.0%
98.5%
93.7%
86.8%
80.8%
70.8%
59.7%

SOIL CLASSIFICATION

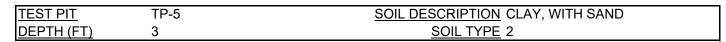
USCS CLASSIFICATION: CL

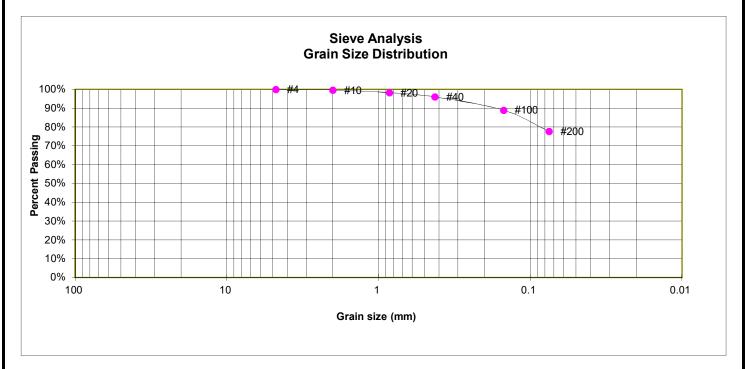


LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

FIG. C-21





GRAIN SIZE ANALYSIS

U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	99.7%
20	98.3%
40	96.1%
100	89.0%
200	77.7%

SOIL CLASSIFICATION

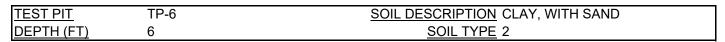
USCS CLASSIFICATION: CL

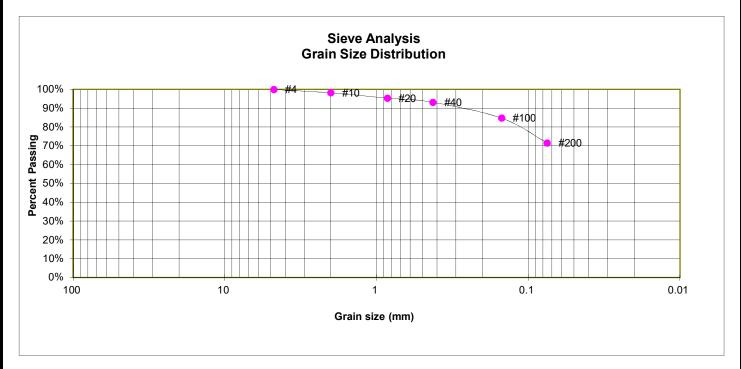


LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

FIG. C-22





GRAIN SIZE ANALYSIS

U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	98.2%
20	95.4%
40	93.2%
100	84.8%
200	71.7%

SOIL CLASSIFICATION

USCS CLASSIFICATION: CL



LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 4 FLYING HORSE NORTH, LLC JOB NO. 241144

FIG. C-23



APPENDIX D: EEI Laboratory Testing Summary and Test Boring Logs Job No. 220404

TEST BORING 15 DATE DRILLED 12/22/20							TEST BORING 16 DATE DRILLED 1/3/2024					
REMARKS					%		REMARKS				9	
DRY TO 20', 12/22/23	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent 9	Soil Type	DRY TO 20', 1/3/24	Depth (ft)	Symbol	Blows per foot	Watercontent %	Soil Type
6" TOPSOIL		5	כט	Ш	>	0)	SAND, SILTY, TAN, MEDIUM		0) 0	ЛШ	>	0)
SAND, CLAYEY, OLIVE, MEDIUM DENSE, MOIST	-	\		17	5.5	1	DENSE, MOIST	-		25	6.5	1
	5	·/·		14	6.3	1		5_		23	13.8	1
	10			25	4.4	1		10		10	12.5	1
SANDSTONE, VERY WEAK, LIGHT BROWN, HIGHLY WEATHERED (SAND, CLAYEY, VERY DENSE, MOIST)	15_	9 0 0 0		50	6.6	3	SAND, SILTY, TAN, DENSE to VERY DENSE, MOIST (SANDSTONE, WEAK, RESIDUAL SOIL)	15_		47	8.9	1
	20	9 0 0 0		<u>50</u> 10"	8.0	3		20_		50	11.1	1



FLYING HORSE NORTH SKETCH PLAN FLYING HORSE DEVELOPMENT

JOB NO. 220404

TEST BORING 17							TEST BORING 18					
DATE DRILLED 12/28/20. REMARKS	23 T		П			Γ	DATE DRILLED 1/3/2024 REMARKS			1		
DRY TO 20', 12/28/23	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	DRY TO 20', 1/3/24	Depth (ft)	Symbol	Blows per foot	Watercontent %	Soil Type
6" TOPSOIL	_		П				SAND, SILTY, TAN, MEDIUM	Ι.				
CLAY, SANDY, BROWN, VERY STIFF, MOIST	-			19	8.0	2	DENSE, MOIST	:		23	6.5	1
SILT, SANDY, BROWN, MEDIUM STIFF, MOIST	5_			5	8.6	2		5_		17	13.8	1
CLAY, SANDY, BROWN, VERY STIFF, MOIST	10			22	3.8	2		10_		27	12.5	1
SAND, SILTY, TAN, DENSE, MOIST								-				
SANDSTONE, VERY WEAK, OLIVE, HIGHLY WEATHERED (SAND,	15			44	3.9	1	SAND, SILTY, TAN, DENSE, MOIST (SANDSTONE, WEAK, RESIDUAL SOIL)	15_		47	8.9	1
SILTY, VERY DENSE, MOIST)	20	1		<u>50</u> 10"	4.4	4		20		49	11.1	1



FLYING HORSE NORTH SKETCH PLAN FLYING HORSE DEVELOPMENT

JOB NO. 220404

TEST BORING 23						TEST BORING 24					
DATE DRILLED 1/9/2024						DATE DRILLED 1/9/2024					
REMARKS						REMARKS					
DRY TO 20', 1/9/24	Depth (ft)	Samples	Blows per foot	Watercontent %	Soil Type	DRY TO 20', 1/9/24	Depth (ft)	Symbol Samples	Blows per foot	Watercontent %	Soil Type
SAND, CLAYEY, LIGHT BROWN,	.	۲.				SAND, SILTY, TAN, MEDIUM	l .	111			
LOOSE to MEDIUM DENSE, MOIST			10	12.1	1	DENSE to DENSE, MOIST	-		15	5.5	1
	5		7	13.2	1		5_		16	9.2	1
	10		7	11.2	1		10		19	6.2	1
SAND, SILTY, LIGHT BROWN, MEDIUM DENSE, MOIST	15		19	7.9	1		15_		31	9.4	1
	20		27	5.1	1	SANDSTONE, VERY WEAK, OLIVE, HIGHLY WEATHERED (SAND, SILTY, VERY DENSE, MOIST)	20		<u>50</u> 11"	10.5	3



FLYING HORSE NORTH SKETCH PLAN FLYING HORSE DEVELOPMENT

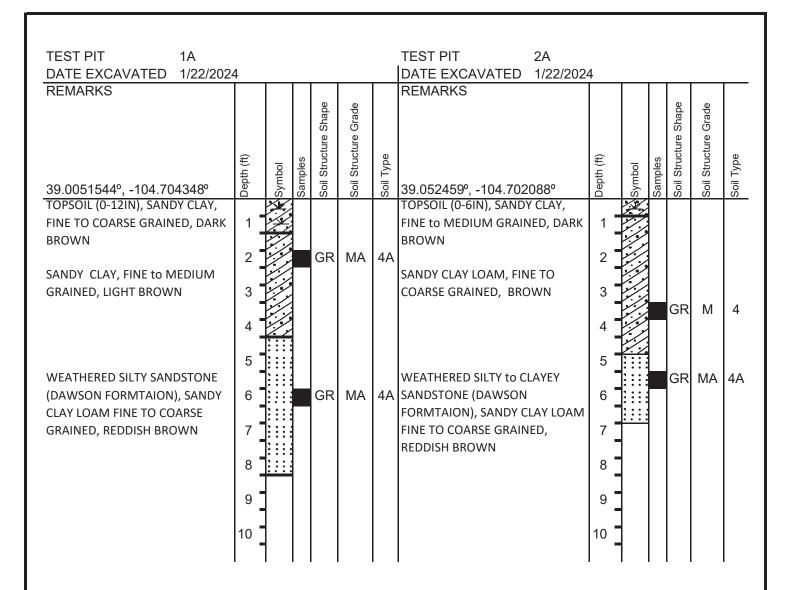
JOB NO. 220404

TEST BORING 25 DATE DRILLED 1/9/2024							TEST BORING 26 DATE DRILLED 1/9/2024						
REMARKS							REMARKS						
DRY TO 20', 1/9/24	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	DRY TO 20', 1/9/24	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
6" TOPSOIL	_						SAND, CLAYEY, BROWN, MEDIUM		<i>-</i> /.				
CLAY, WITH SAND, BROWN to OLIVE, VERY STIFF, MOIST	:			21	6.2	2	DENSE, MOIST	=	/		12	13.0	1
	5			19	16.4	2	SAND, SILTY, BROWN to TAN, MEDIUM DENSE, MOIST	5			12	6.2	1
SANDSTONE, VERY WEAK, TAN to OLIVE, HIGHLY WEATHERED (SAND, CLAYEY, VERY DENSE, MOIST)	10			<u>50</u> 8"	8.1	3		10			23	7.7	1
	15			<u>50</u> 9"	10.0	3	SAND, SILTY, TAN, DENSE to VERY DENSE, MOIST (SANDSTONE, WEAK, RESIDUAL SOIL)	15_			<u>50</u> 11"	6.8	1
	20			<u>50</u> 10"	8.9	3		20		-	41	12.6	1



FLYING HORSE NORTH SKETCH PLAN FLYING HORSE DEVELOPMENT

JOB NO. 220404



Soil Structure Shape

granular - gr platy - pl blocky - bl prismatic - pr single grain - sg Soil Structure Grade

weak - w moderate - m strong - s loose - l massive - ma



TEST PIT LOGS

FLYING HORSE NORTH SKETCH PLAN FLYING HORSE NORTH, LLC JOB NO. 220404

REMARKS				ture Shape	ture Grade					ture Shape	Soil Structure Grade	
39.050334°, -104.702484°	Depth (ft)	Symbol	Samples	Soil Structure	Soil Structure	Soil Type	Depth (ft)	Symbol	Samples	Soil Structure	Soil Struct	Soil Type
FOPSOIL (0-12IN), SANDY CLAY, FINE TO COARSE GRAINED, DARK BROWN	1						1 =					
SANDY CLAY, FINE to MEDIUM GRAINED, OLIVE BROWN	2 3			GR	W	4A	2					
	4 - 5 -						4 - 5					
CORMATIONAL SITLY TO CLAYEY SANDSTONE (DAWSON CORMATION), SANDY CLAY LOAM	_			GR	MA	4A	6					
o SANDY CLAY, FINE TO COARSE GRAINED, LIGHT BROWN TO	7 8						7 8					
SIGNS OF SEASONAL GW AT 411	9 -						9 -					

Soil Structure Shape

granular - gr platy - pl blocky - bl prismatic - pr single grain - sg Soil Structure Grade

weak - w moderate - m strong - s loose - l massive - ma



TEST PIT LOGS

FLYING HORSE NORTH SKETCH PLAN FLYING HORSE NORTH, LLC JOB NO. 220404



APPENDIX E: Soil Survey Descriptions

El Paso County Area, Colorado

66—Peyton sandy loam, 1 to 5 percent slopes

Map Unit Setting

National map unit symbol: 369c Elevation: 6,800 to 7,600 feet

Farmland classification: Prime farmland if irrigated and the product of

I (soil erodibility) x C (climate factor) does not exceed 60

Map Unit Composition

Peyton and similar soils: 85 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Peyton

Setting

Landform: Hills, flats

Landform position (three-dimensional): Side slope, talf

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Arkosic alluvium derived from sedimentary rock and/or arkosic residuum weathered from sedimentary rock

Typical profile

A - 0 to 12 inches: sandy loam
Bt - 12 to 25 inches: sandy clay loam
BC - 25 to 35 inches: sandy loam
C - 35 to 60 inches: sandy loam

Properties and qualities

Slope: 1 to 5 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 7.3

inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4c

Hydrologic Soil Group: B

Ecological site: R049XY216CO - Sandy Divide

Hydric soil rating: No

Minor Components

Other soils

Percent of map unit: Hydric soil rating: No

Pleasant

Percent of map unit: Landform: Depressions Hydric soil rating: Yes

Data Source Information

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 21, Aug 24, 2023

El Paso County Area, Colorado

67—Peyton sandy loam, 5 to 9 percent slopes

Map Unit Setting

National map unit symbol: 369d Elevation: 6,800 to 7,600 feet

Mean annual air temperature: 43 to 45 degrees F

Frost-free period: 115 to 125 days

Farmland classification: Not prime farmland

Map Unit Composition

Peyton and similar soils: 85 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Peyton

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Arkosic alluvium derived from sedimentary rock and/or arkosic residuum weathered from sedimentary rock

Typical profile

A - 0 to 12 inches: sandy loam
Bt - 12 to 25 inches: sandy clay loam
BC - 25 to 35 inches: sandy loam
C - 35 to 60 inches: sandy loam

Properties and qualities

Slope: 5 to 9 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 7.3

inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4e

Hydrologic Soil Group: B

Ecological site: R049XY216CO - Sandy Divide

Hydric soil rating: No

Minor Components

Other soils

Percent of map unit: Hydric soil rating: No

Pleasant

Percent of map unit: Landform: Depressions Hydric soil rating: Yes

Data Source Information

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 21, Aug 24, 2023

El Paso County Area, Colorado

68—Peyton-Pring complex, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 369f Elevation: 6,800 to 7,600 feet

Farmland classification: Not prime farmland

Map Unit Composition

Peyton and similar soils: 40 percent Pring and similar soils: 30 percent

Estimates are based on observations, descriptions, and transects of

the mapunit.

Description of Peyton

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Arkosic alluvium derived from sedimentary rock and/or arkosic residuum weathered from sedimentary rock

Typical profile

A - 0 to 12 inches: sandy loam
Bt - 12 to 25 inches: sandy clay loam
BC - 25 to 35 inches: sandy loam
C - 35 to 60 inches: sandy loam

Properties and qualities

Slope: 3 to 5 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Moderate (about 7.3

inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4c

Hydrologic Soil Group: B

Ecological site: R049XY216CO - Sandy Divide

Hydric soil rating: No

Description of Pring

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Arkosic alluvium derived from sedimentary rock

Typical profile

A - 0 to 14 inches: coarse sandy loam
C - 14 to 60 inches: gravelly sandy loam

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High

(2.00 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 6.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: B

Ecological site: R048AY222CO - Loamy Park

Hydric soil rating: No

Minor Components

Other soils

Percent of map unit: Hydric soil rating: No

Pleasant

Percent of map unit: Landform: Depressions Hydric soil rating: Yes

Data Source Information

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 21, Aug 24, 2023