October 9th, 2024

Gilbert LaForce, PE El Paso County 2880 International Circle, Suite 110 Colorado Springs, CO 8010

Re: La Foret Certification Letter El Paso County, Colorado

Dear Mr. LaForce:

This section was intended to describe the ECM requirements. Statement of Engineer of record is provided below. This section updated to provide additional statement as engineer of record.

Please don't just quote the ECM but provide that statement as the engineer of record.

This letter serves as the certification for La Foret, El Paso County, CO consistent with El Paso County Engineering Manual ("ECM") Section 5.10.B, which states, "Engineering Record Drawings shall be accompanied by a certification letter from the Engineer of Record which shall state that the site and adjacent properties (as affected by work performed under the County permit) are stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover, and that the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements. For sites including detention and/or water quality facilities, the certification letter shall include a statement that the facilities provide the required storage volume and will meet the required release rates, as documented by an attached UDFCD design form submitted with the original application, the stage areas, elevations and outlet dimensions."

Based upon this information and information gathered during periodic site visits to the site during significant/key phases of the infrastructure installation, *Kimley-Horn & Associates, Inc.* is of the opinion that the work performed under the County Permit, per Section 5.10.B of the ECM, have been constructed in general compliance with the approved Construction Plans as filed with the County dated November 15, 2023.

Statement Of Engineer in Responsible Charge:

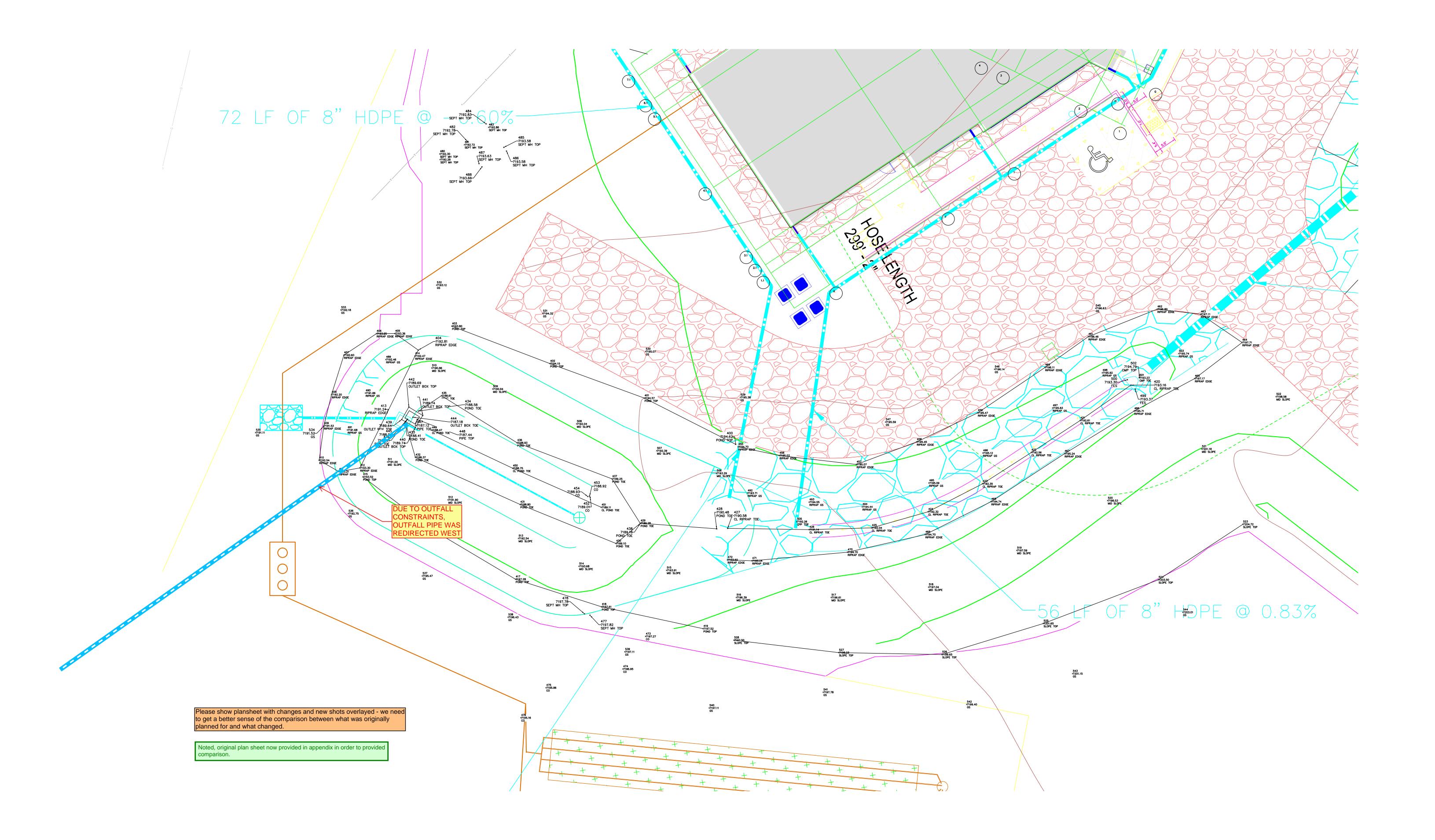
I, Kevin R. Kofford, a registered Professional Engineer in the State of Colorado, in accordance with Sections 5.2 and 5.3 of the Bylaws and Rules of the State Board of Registration for Professional Engineers and Professional Land Surveyors, do hereby certify that I or a person under my responsible charge periodically observed the construction of the above-mentioned project. Based on the on-site field observations and review of pertinent documentation, it is my professional opinion that the required infrastructure improvements have been installed and are in general compliance with the approved Construction Plans as filed with El Paso County. For BMPs with a Water Quality Capture Volume (WQCV), I have attached the post-construction As-Built drawings. The As-Built drawings accurately depict the final installation of the stormwater BMPs and verify the WQCV.

Kevin R. Kofford, P.E. Colorado No. 57234

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DETENTION BASIN STAGE-STORAGE TABLE BUILDER

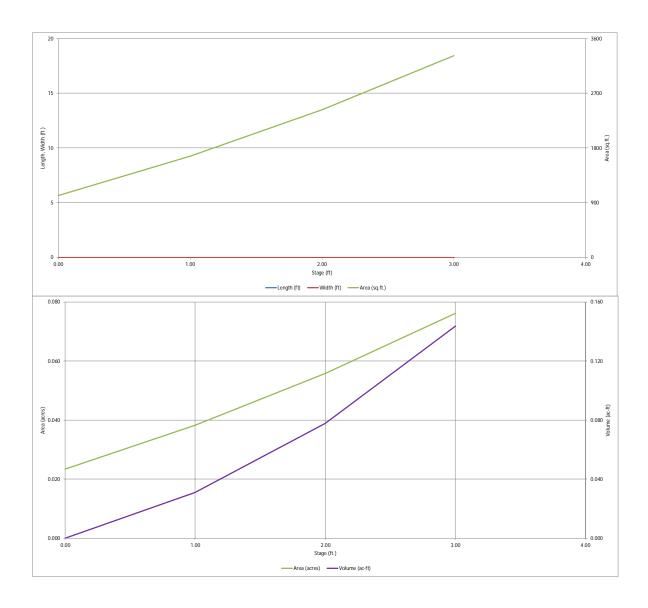
		DET	ENTION BASI				BLE BU	IILDER					
			MHFD	-Detention, Version	on 4.06 (Ju	ıly 2022)							
Project:													
Basin ID:													
ZONE 3 ZONE Z	2 ONE 1		_										
100-YR VOLUME EURV WQCV													
T was t		100-YE				1							
ZONE	1 AND 2	ORIFIC	CE CE	Depth Increment =		ft Optional	1	1	1	Optional			
POOL Example Zone		ation (Reter	ntion Pond)	Stage - Storage	Stage	Override	Length	Width	Area	Override	Area	Volume	Volume
Makanbad Information				Description Media Surface	(ft)	Stage (ft) 0.00	(ft) 	(ft)	(ft ²)	Area (ft 2) 1,019	(acre) 0.023	(ft ³)	(ac-ft)
Watershed Information	RG			iviedia Surrace		1.00				1,665	0.023	1.342	0.031
Selected BMP Type =												.,	
Watershed Area =	2.89	acres				2.00				2,430	0.056	3,389	0.078
Watershed Length = Watershed Length to Centroid =	500 250	ft e				3.00				3,319	0.076	6,264	0.144
Watershed Length to Centrold = Watershed Slope =	0.050	ft/ft										-	
Watershed Imperviousness =	26.00%	percent											
Percentage Hydrologic Soil Group A =	0.0%	percent											
Percentage Hydrologic Soil Group B =	100.0%	percent											
Percentage Hydrologic Soil Groups C/D =	0.0%	percent			-								
Target WQCV Drain Time =	12.0	hours			-								
Location for 1-hr Rainfall Depths =	User Input												
After providing required inputs above inc	cluding 1-hour	rainfall											
depths, click 'Run CUHP' to generate run the embedded Colorado Urban Hydro	off hydrograph ograph Proced	ns using											
	0.027	_	Optional User Overrides									_	
Water Quality Capture Volume (WQCV) = Excess Urban Runoff Volume (EURV) =	0.027	acre-feet acre-feet	acre-feet acre-feet									 	
2-yr Runoff Volume (P1 = 1.19 in.) =	0.076	acre-feet	1.19 inches										
5-yr Runoff Volume (P1 = 1.5 in.) =	0.130	acre-feet	1.50 inches										
10-yr Runoff Volume (P1 = 1.75 in.) =	0.179	acre-feet	1.75 inches										
25-yr Runoff Volume (P1 = 2 in.) =	0.255	acre-feet	2.00 inches										
50-yr Runoff Volume (P1 = 2.25 in.) =	0.312	acre-feet	2.25 inches										
100-yr Runoff Volume (P1 = 2.52 in.) =	0.389	acre-feet	2.52 inches		-								
500-yr Runoff Volume (P1 = 3.14 in.) =	0.535	acre-feet	inches										
Approximate 2-yr Detention Volume =	0.054	acre-feet										ļ	
Approximate 5-yr Detention Volume =	0.078	acre-feet											
Approximate 10-yr Detention Volume =	0.117	acre-feet										-	
Approximate 25-yr Detention Volume =	0.139 0.147	acre-feet acre-feet										₩	
Approximate 50-yr Detention Volume = Approximate 100-yr Detention Volume =	0.176	acre-feet										-	
Approximate 100-yr Determion volume =	0.170	acre-leet											
Define Zones and Basin Geometry													
Zone 1 Volume (WQCV) =	0.027	acre-feet											
Select Zone 2 Storage Volume (Optional) =		acre-feet	Total detention										
Select Zone 3 Storage Volume (Optional) =		acre-feet	volume is less than		-								
Total Detention Basin Volume =	0.027	acre-feet	100-year volume.										
Initial Surcharge Volume (ISV) =	N/A	ft ³											
Initial Surcharge Depth (ISD) =	N/A	ft										 	
Total Available Detention Depth (H _{total}) =	user	ft										_	——
Depth of Trickle Channel (H_{TC}) = Slope of Trickle Channel (S_{TC}) =	N/A N/A	ft ft/ft										 	-
		H:V										 	1
Slopes of Main Basin Sides (S_{main}) = Basin Length-to-Width Ratio ($R_{L/W}$) =	user	n.v										+	-
casiii congurto-widii Nado (K[/W]) =	usei	4										 	1
Initial Surcharge Area (A _{ISV}) =	user	ft ²										 	
Surcharge Volume Length (L _{ISV}) =	user	ft										†	
Surcharge Volume Width (W _{ISV}) =	user	ft											
Depth of Basin Floor (H _{FLOOR}) =	user	ft											
Length of Basin Floor (L _{FLOOR}) =	user	ft			-								
Width of Basin Floor (W_{FLOOR}) =	user	ft			-								
Area of Basin Floor (A_{FLOOR}) =	user	ft ²											
Volume of Basin Floor (V _{FLOOR}) =	user	ft ³										 	<u> </u>
Depth of Main Basin (H _{MAIN}) =	user	ft											
Length of Main Basin (L_{MAIN}) =	user	ft										↓	

This spreadsheet is the revised UD Detention spreadhseet with updated as-built information. The revised spreadsheet confirms the drain times and that the rg is functioning as intended. The Original spreadsheet, as found in the approved FDR, has been provided in the appendix as well for comparison.

Please include the full MHFD-Detention Spreadsheet so drain times can be confirmed. Please update the MHFD-Detention Spreadsheet with the updated as-built information (pond elevations, spillway length, orifice hole elevations, etc.) to confirm that the pond is functioning as intended. The updated Spreadsheet should be attached to the Pond Certification Letter. If some items haven't change keep that as-is from the design calcs.

Width of Main Basin (W_{MAIN}) = Area of Main Basin (A_{MAIN}) = Volume of Main Basin (V_{MAIN}) =

ompanson.								
				-	-			
				-	-			
				-	-			
				-	-			
				-	-			



M#FD-Detention_v4-06_Rain Garden_Cert.xtsm, Basin

10/9/2024, 12:10 PM

TON BASIN OUTLET STRUCTURE DESIGN Project Basin ID: Estimated Estimated Stage (ft) Volume (ac-ft) Outlet Type Filtration Media Zone 1 (WQCV 0.89 0.027 7one Zone 3 Example Zone Configuration (Retention Pond) Total (all zones) 0.027 User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP) Calculated Parameters for Underdrain Underdrain Orifice Invert Depth = 1 88 ft (distance below the filtration media surface) Underdrain Orifice Area 0.0 Underdrain Orifice Diameter 0.82 Underdrain Orifice Centroid : 0.03 User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP) Calculated Parameters for Plate Centroid of Lowest Orifice = ft (relative to basin bottom at Stage = 0 ft) WQ Orifice Area per Row N/A Depth at top of Zone using Orifice Plate = ft (relative to basin bottom at Stage = 0 ft) Elliptical Half-Width N/A Orifice Plate: Orifice Vertical Spacing inches Elliptical Slot Centroid N/A feet Orifice Plate: Orifice Area per Row = Elliptical Slot Area N/A sa, inches User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest) Row 1 (optional) Row 2 (optional) Row 3 (optional) Row 4 (optional) Row 5 (optional) Row 6 (optional) Row 7 (optional) Row 8 (optional) Stage of Orifice Centroid (ft Orifice Area (sq. inches) Row 9 (optional) Row 10 (optional) Row 11 (optional) Row 12 (optional) Row 13 (optional) Row 14 (optional) Row 15 (optional) Row 16 (optional) Stage of Orifice Centroid (ft) Orifice Area (sq. inches) User Input: Vertical Orifice (Circular or Rectangular) Calculated Parameters for Vertical Orifice Not Selected Not Selected Not Selected Not Selected Invert of Vertical Orifice Vertical Orifice Area ft (relative to basin bottom at Stage = 0 ft) Depth at top of Zone using Vertical Orifice ft (relative to basin bottom at Stage = 0 ft) Vertical Orifice Centroid = Vertical Orifice Diameter User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe) Calculated Parameters for Overflow Weir Not Selected Not Selected Not Selected Not Selected Overflow Weir Front Edge Height, Ho 1.69 Height of Grate Upper Edge, Ht t (relative to basin bottom at Stage = 0 ft) 1.69 feet Overflow Weir Front Edge Length 2.60 feet Overflow Weir Slope Length 2.60 feet Overflow Weir Grate Slope 0.00 H:V Grate Open Area / 100-yr Orifice Area Horiz, Length of Weir Sides 2.60 eet Overflow Grate Open Area w/o Debris 4.70 Overflow Grate Type Type C Grate Overflow Grate Open Area w/ Debris = 2.35 Debris Clogging % = 50% User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice) Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate Not Selected Not Selected Not Selected Not Selected Depth to Invert of Outlet Pipe (distance below basin bottom at Stage = 0 ft) Outlet Orifice Area Circular Orifice Diameter Outlet Orifice Centroid Half-Central Angle of Restrictor Plate on Pipe = N/A N/A radians User Input: Emergency Spillway (Rectangular or Trapezoidal) Calculated Parameters for Spillway Spillway Invert Stage= ft (relative to basin bottom at Stage = 0 ft) Spillway Design Flow Depth= feet Spillway Crest Length : Stage at Top of Freeboard : feet feet Spillway End Slopes H·V Basin Area at Top of Freeboard acres Freeboard above Max Water Surface = Basin Volume at Top of Freeboard = cre-ft Routed Hydrograph Results ns W through AF Design Storm Return Period WOCV FURV 10 Yea 100 Yea 500 Year One-Hour Rainfall Depth (in) N/A N/A 1.19 1.50 1.75 2.00 2.25 2.52 3.14 CUHP Runoff Volume (acre-ft) 0.027 0.130 0.179 0.25 0.312 0.389 Inflow Hydrograph Volume (acre-ft) N/A N/A 0.077 0.130 0.179 0.312 0.389 0.535 0.255 CUHP Predevelopment Peak Q (cfs) N/A N/A 0.4 1.1 1.6 2.9 3.6 4.5 6.2 OPTIONAL Override Predevelopment Peak Q (cfs) N/A N/A Predevelopment Unit Peak Flow, q (cfs/acre) N/A N/A 0.14 0.38 0.56 0.99 1.24 1.55 2.16 Peak Inflow Q (cfs) N/A N/A 1.9 2.6 3.8 4.5 7.4 Peak Outflow Q (cfs) 0.03 1.4 0.2 1.1 1.7 4.1 7.2 Ratio Peak Outflow to Predevelopment Q N/A N/A N/A 1.0 1.1 1.2

Structure Controlling Flow

Maximum Ponding Depth (ft)

Maximum Volume Stored (acre-ft)

Max Velocity through Grate 1 (fps)

Max Velocity through Grate 2 (fps)

Time to Drain 97% of Inflow Volume (hours)

Time to Drain 99% of Inflow Volume (hours)

Area at Maximum Ponding Depth (acres)

Filtration Media

N/A

N/A

12

0.90

0.04

Overflow Weir 1

-0.01

N/A

25

26

1 97

0.06

Overflow Weir 1

-0.01

N/A

27

28

1 74

0.05

Overflow Weir 1

0.0

N/A

26

28

1.85

0.05

Overflow Weir 1

0.0

N/A

26

28

1 91

0.05

Overflow Weir 1

0.0

N/A

25

27

2.03

0.06

Overflow Weir 1

0.0

N/A

24

27

2.08

0.06

Overflow Weir 1

0.0

N/A

23

2 15

0.06

Overflow Weir

0.0

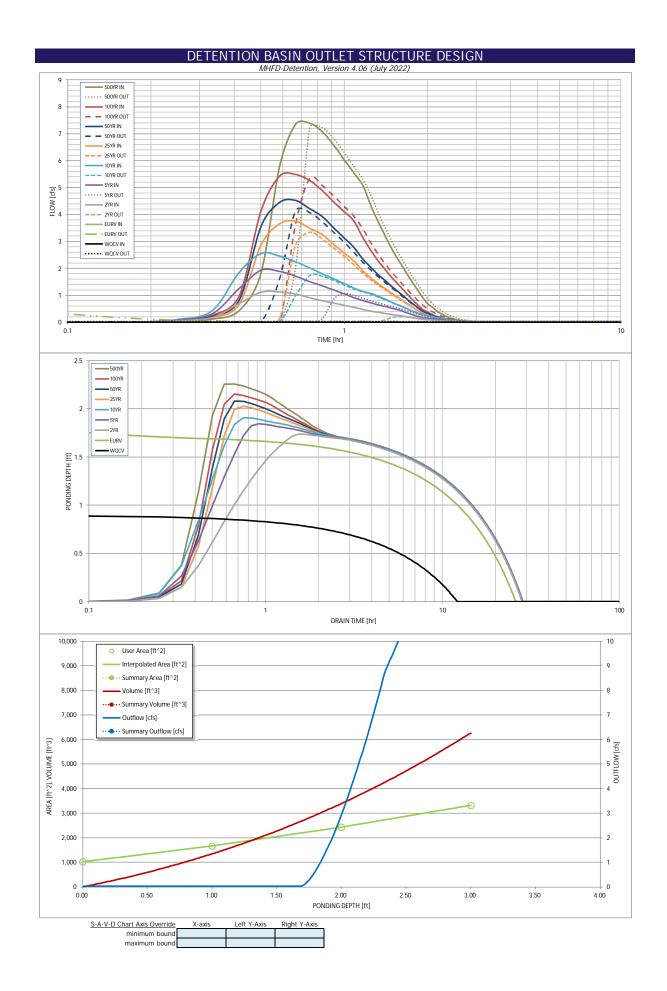
N/A

21

26

2.26

0.06



DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.02
	0:15:00	0.00	0.00	0.05	0.08	0.10	0.07	0.09	0.09	0.12
	0:20:00	0.00	0.00	0.18	0.33	0.44	0.18	0.22	0.26	0.44
	0:25:00	0.00	0.00	0.70	1.27	1.86	0.68	0.83	0.99	1.85
	0:30:00	0.00	0.00	1.12	1.94	2.55	2.88	3.57	4.15	5.80
	0:35:00	0.00	0.00	1.13	1.89	2.46	3.66	4.45	5.43	7.35
	0:40:00	0.00	0.00	1.04	1.72	2.25	3.75	4.53	5.49	7.39
	0:45:00	0.00	0.00	0.91	1.51	2.03	3.50	4.22	5.27	7.08
	0:50:00 0:55:00	0.00	0.00	0.80	1.35	1.80	3.26	3.92	4.89	6.58
	1:00:00	0.00	0.00	0.72	1.20	1.61	2.90 2.57	3.51	4.47	6.03 5.55
	1:05:00	0.00	0.00	0.64	1.05 0.92	1.44	2.28	3.13 2.78	4.11 3.76	5.09
	1:10:00	0.00	0.00	0.48	0.81	1.15	1.92	2.35	3.15	4.31
	1:15:00	0.00	0.00	0.42	0.72	1.07	1.64	2.03	2.66	3.69
	1:20:00	0.00	0.00	0.37	0.65	0.96	1.42	1.75	2.25	3.13
	1:25:00	0.00	0.00	0.33	0.58	0.84	1.23	1.52	1.91	2.66
	1:30:00	0.00	0.00	0.29	0.52	0.73	1.05	1.30	1.62	2.25
	1:35:00	0.00	0.00	0.25	0.45	0.63	0.89	1.10	1.36	1.88
	1:40:00	0.00	0.00	0.22	0.37	0.53	0.74	0.91	1.11	1.54
	1:45:00	0.00	0.00	0.18	0.30	0.44	0.59	0.73	0.88	1.22
	1:50:00	0.00	0.00	0.15	0.23	0.35	0.46	0.56	0.67	0.94
	1:55:00	0.00	0.00	0.12	0.19	0.30	0.33	0.42	0.49	0.70
	2:00:00	0.00	0.00	0.10	0.16 0.13	0.25	0.27	0.34	0.38	0.56
	2:10:00	0.00	0.00	0.06	0.10	0.20	0.20	0.23	0.28	0.42
	2:15:00	0.00	0.00	0.05	0.08	0.13	0.13	0.15	0.21	0.23
	2:20:00	0.00	0.00	0.04	0.06	0.10	0.09	0.11	0.11	0.16
	2:25:00	0.00	0.00	0.03	0.05	0.08	0.07	0.08	0.08	0.12
	2:30:00	0.00	0.00	0.02	0.04	0.06	0.05	0.06	0.05	0.08
	2:35:00	0.00	0.00	0.02	0.03	0.04	0.04	0.05	0.04	0.06
	2:40:00	0.00	0.00	0.02	0.02	0.03	0.03	0.04	0.03	0.05
	2:45:00	0.00	0.00	0.01	0.02	0.03	0.02	0.03	0.02	0.04
	2:50:00	0.00	0.00	0.01	0.01	0.02	0.02	0.02	0.02	0.03
	2:55:00	0.00	0.00	0.01	0.01	0.01	0.01	0.02	0.01	0.02
	3:00:00 3:05:00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.02
	3:10:00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00 4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00 4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00 4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00 5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00 5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00 6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
ļ	0.00.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00