

Because this drainage report is associated with both the PUD/Preliminary Plan and final plat application, rename the report to:

Preliminary Drainage Report for Winding Walk Filings 1 & 2 and Final Drainage Report for Windingwalk Filing 1 at Meridian Ranch.

Final Drainage Report for
for

Windingwalk Filing 1

at
Meridian Ranch



EL PASO COUNTY, COLORADO

December 2017

Prepared For:

GTL DEVELOPMENT, INC.
P.O. Box 80036
San Diego, CA 92138

Prepared By:
Tech Contractors
11886 Stapleton Drive
Falcon, CO 80831
719.495.7444

PCD File No.'s PUDSP-18-002 and SF-18-002

PCD Project No. ESQ-17-XXX

FYI: A Final drainage report will be required with the final plat application for Filing 2.

CERTIFICATIONS

Design Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Thomas A. Kerby, P.E. #31429

Date

Owner/Developer's Statement:

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

Raul Guzman, Vice President
GTL Development, Inc.
P.O. Box 80036
San Diego, CA 92138

Date

El Paso County:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 & 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

Jennifer Irvine, P.E.
County Engineer / ECM Administrator

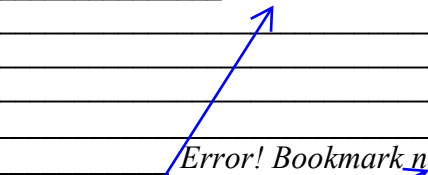
Date

Windingwalk Filing 1 at Meridian Ranch Preliminary Drainage Plan

Table of Contents

EXECUTIVE SUMMARY	<i>i</i>
INTRODUCTION	<i>1</i>
Purpose	1
Scope	1
Background	1
EXISTING CONDITIONS	2
General Location	2
Land Use	2
Climate	2
Topography and Floodplains	2
Geology	2
Natural Hazards Analysis	6
DRAINAGE BASINS AND SUB-BASINS	6
DRAINAGE DESIGN CRITERIA	7
SCS Hydrograph Procedure	7
Full Spectrum Design	7
DRAINAGE CALCULATIONS	8
General Concept	8
SCS Calculations	10
Historic Drainage - SCS Calculation Method	10
Interim Drainage - SCS Calculation Method	11
Future Drainage - SCS Calculation Method	13
Pond H Detention Storage Criteria	15
Rational Calculations	16
EROSION CONTROL DESIGN	19
General Concept	19
Detention Pond	Error! Bookmark not defined.
Silt Fence	20
Erosion Bales	20
Miscellaneous	20
DRAINAGE FEES	21
REFERENCES	Error! Bookmark not defined.

Fix reference



Figures

Figure 1: Vicinity Map.....	3
Figure 2: FEMA Floodplain Map	4
Figure 3: Soils Map.....	5
Figure 4 - Meridian Ranch SCS Method – Historic Basin Map	BACK POCKET
Figure 5- Meridian Ranch SCS Method – Interim Basin Map	BACK POCKET
Figure 6- Meridian Ranch SCS Method – Future Basin Map	BACK POCKET

Tables

Table 1: SCS Runoff Curve Numbers	7
Table 2: Detention Pond Summary:.....	8
Table 3: Historic Drainage Basins – SCS	10
Table 4: Interim Drainage Basins-SCS.....	11
Table 5: Future Drainage Basins-SCS	13
Table 6: Key Design Point Comparison - SCS.....	15
Table 7: Pond H Summary Data	16

Appendices:

- Appendix A – Rational Calculations
- Appendix B – Street Flow Tables
- Appendix C - HEC-HMS Data
- Appendix D - Detention Pond Information
- Appendix E – Temporary Sedimentation
- Appendix F – Soil Resource Report

EXECUTIVE SUMMARY

The purpose of the following Final Drainage Report (FDR) is to present proposed changes for Windingwalk Filing 1 at Meridian Ranch due to development. Runoff quantities and proposed facilities have been calculated using the current City of Colorado Springs/El Paso County Drainage Criteria Manual (DCM) (1994 version) and portions of the City of Colorado Springs Drainage Criteria Manual, Volume 1 (DCM-1) ((2014 version).

This report anticipates the revisions to the most recent sketch plan amendment in process with the Planning and Community Development Department. The submitted Sketch Plan includes a change of use from business to residential resulting in lower developed runoff. Another significant change from previous drainage reports submitted to El Paso County concerning development associated within Meridian Ranch is the adopted changes to the drainage criteria. El Paso County by Resolution 15-042 adopted Chapter 6 of the 2014 version of the City of Colorado Springs Drainage Criteria Manual (COSDCM). Chapter 6 addresses the hydrologic calculations and includes an updated hydrograph to be used with storm drainage runoff. The new hydrograph results in lower historic values for runoff rates and higher developed values given the same input values. The county adopted Section 3.2.1 of Chapter 13 of the COSDCM referencing Full Spectrum Detention; the concept “provides better control of the full range of runoff rates that pass through detention facilities than the convention multi-stage concept. By providing an Excess Urban Runoff Volume (EURV) in the lower portion of the facility storage with an outlet similar to the Water Quality Capture Volume (WQCV), *frequent and infrequent inflows are released at rates approximating undeveloped conditions.*” This report includes hydrologic models from HEC-HMS for the historic, interim and future conditions for the 2-yr, 5-yr, 10-yr, 25-yr, 50-yr, and 100-yr design storm frequencies. The interim and the future conditions include detention facilities sized and modeled such that “*frequent and infrequent inflows are released at rates approximating undeveloped conditions*”

On November 16, 2000 the El Paso County Board of County Commissioners approved the rezoning of the Meridian Ranch project (PUD-00-010) from A-35 to PUD with several conditions. Condition number seven stated in part that “drainage plans shall release and/or retain at approximately eight percent (80%) of historic rates.” At the time of the initial approvals there were no drainage improvements downstream of the Meridian Ranch project and the existing natural channels were shallow and undefined. Since the time of the original approvals, development has occurred downstream of Meridian Ranch with drainage facilities designed and constructed of sufficient size to safely convey the historic flow rates discharged from Meridian Ranch to downstream properties.

Windingwalk Filing 1 at Meridian Ranch grading encompasses 114± acres and is located in Sections 29, 30 and 32, Township 12 South, Range 64 West of the 6th Principal Meridian. It is approximately 12 miles northeast of the city of Colorado Springs, 2.5 miles north of the unincorporated town of Falcon, and immediately north of the Woodmen Hills development.

Windingwalk Filing 1 at Meridian Ranch is located within two separate drainage basins; the Bennett Ranch Basin and the Haegler Ranch Basin. Each have been studied as part of the respective Drainage Basin and have final approval from the El Paso County.

Based on the aforementioned design parameters the development of the project will not adversely affect downstream properties.

INTRODUCTION

Purpose

The purpose of the following Final Drainage Report (FDR) is to present proposed changes for Windingwalk Filing 1 at Meridian Ranch due to grading operations with drainage mitigation based on calculated developed flows in excess of allowable exiting runoff discharge.

Scope

The scope of this report includes:

- Location and description of the proposed development stating the proposed land use, density, acreage and adjacent features to the site.
- Calculations for design peak flows from all off-site tributary drainage areas.
- Calculations for design peak flows within the proposed project area for all drainage areas.
- Discussion of major drainage facilities required as a result of the development.
- Discussion and analysis of existing and proposed facilities.

Runoff quantities and proposed facilities have been calculated using the current City of Colorado Springs/El Paso County Drainage Criteria Manual (DCM) (1994 version) and those portions of the City of Colorado Springs Drainage Criteria Manual, Volume 1 (DCM-1) ((2014 version) adopted by Resolution 15-042 of the El Paso County Board of County Commissioners.

Background

On November 16, 2000 the El Paso County Board of County Commissioners approved the rezoning of the Meridian Ranch project (PUD-00-010) from A-35 to PUD with several conditions. Condition number seven stated in part that “drainage plans shall release and/or retain at approximately eight percent (80%) of historic rates.” At the time of the initial approvals there were no drainage improvements downstream of the Meridian Ranch project and the existing natural channels were shallow and undefined.

Development has occurred downstream of Meridian Ranch since the time of the original approvals with drainage facilities designed and constructed of sufficient size to safely convey the historic flow rates off of Meridian Ranch further downstream. The 4-Way Ranch development located adjacent and downstream of Meridian Ranch has processed a Letter of Map Revision (LOMR) and constructed storm drainage improvements downstream of the existing Pond E outlets. The LOMR was processed and the improvements constructed assuming historic flow rates from Meridian Ranch using the original El Paso County DCM. Storm drain improvements near the intersection of Stapleton Drive and Eastonville have also been designed and constructed to convey the historic flow rates from Meridian Ranch. The design of these improvements and the downstream system anticipated 87 CFS to be collected near outlet of the future Pond H from Meridian Ranch. The design of Pond H has yielded a 100-year flow rate of 60 CFS, well below the anticipated 87 CFS figure.

Current estimates show the design discharge Pond E to 4-Way are near or below 90% of historic flow rates for the 100-year discharge at full buildout and the 5-year discharge at or slightly above historic.

EXISTING CONDITIONS

General Location

Windingwalk Filing 1 at Meridian Ranch grading encompasses 114+ acres and is located in Sections 29, 30 and 32, Township 12 South, Range 64 West of the 6th Principal Meridian. It is approximately 12 miles northeast of the city of Colorado Springs, 2.5 miles north of the unincorporated town of Falcon, and immediately north of the Woodmen Hills development.

Land Use

Historically, ranching dominated the area surrounding Meridian Ranch; however, currently urbanization has occurred in the general vicinity. Most notably, urbanization is occurring to the north with Latigo Trails, to the south in the Woodmen Hills Subdivision, to the east in Four Way Ranch, to the west in the Falcon Hills subdivision, and to the northwest in the Paint Brush Hills subdivision.

Climate

Mild summers and winter, light precipitation; high evaporation and moderately high wind velocities characterize the climate of the study area. The average annual monthly temperature is 48.4 F with an average monthly low of 30.3 F in the winter and an average monthly high of 68.1 F in the summer. Two years in ten will have maximum temperature higher than 98 F and a minimum temperature lower than -16 F. Precipitation averages 15.73" annually, with 80% of this occurring during the months of April through September. The average annual Class A pan evaporation is 45 inches. (Soil Survey of El Paso County Area, Colorado).

Topography and Floodplains

The topography of the site is typical of a high desert, short prairie grass with relatively flat slopes generally ranging from 2% to 4%. The project site drains generally from the northwest to southeast and is tributary to the Black Squirrel Creek.

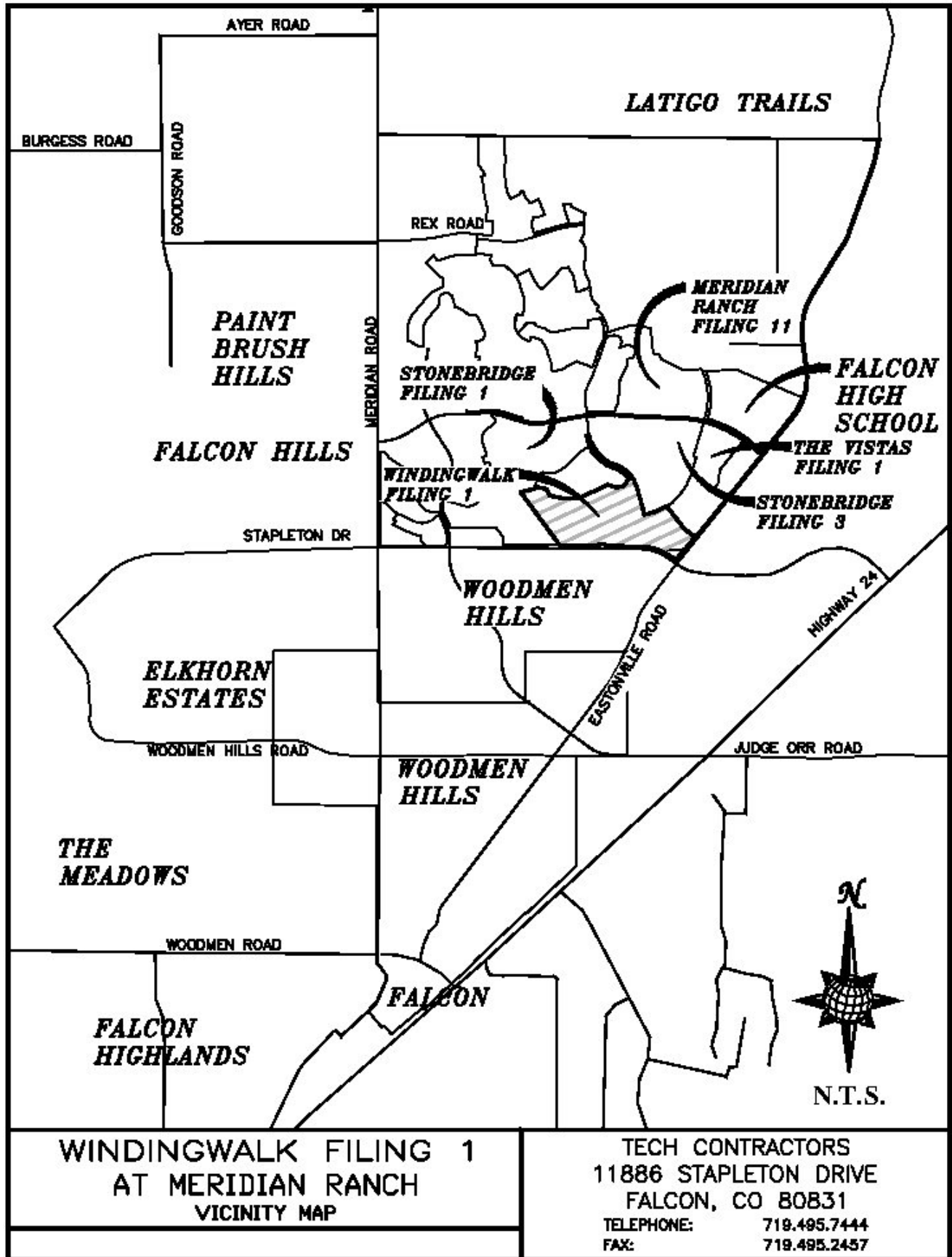
The Flood Insurance Rate Maps (FIRM No. 08041C0575-F dated 3/17/1997) indicates that the Windingwalk Filing 1 at Meridian Ranch development is outside of any designated flood plain. Letter of Map Revision (LOMR), Case No. 14-08-1121P was approved by FEMA on November 6, 2014 with an effective date of March 24, 2015. Please see Figure 2: Windingwalk Filing 1 at Meridian Ranch Federal Emergency Management Agency (FEMA) Floodplain Map.

Geology

The National Resources Conservation Service (NRCS) soil survey records indicate that the service area is predominately covered by soils classified in the Stapleton series. This series is categorized in the Hydrological Group B.

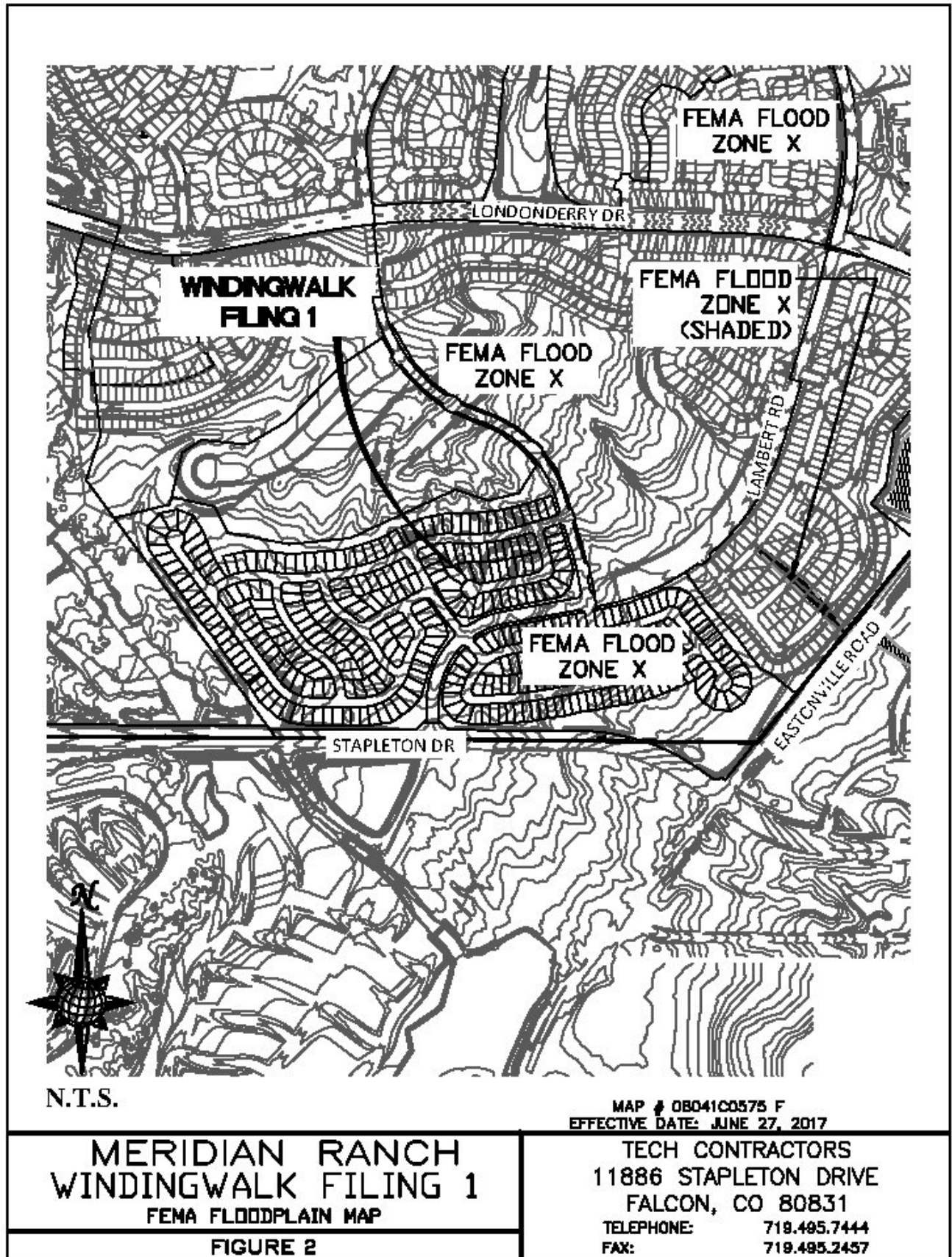
WINDINGWALK FILING 1 AT MERIDIAN RANCH

Figure 1: Vicinity Map



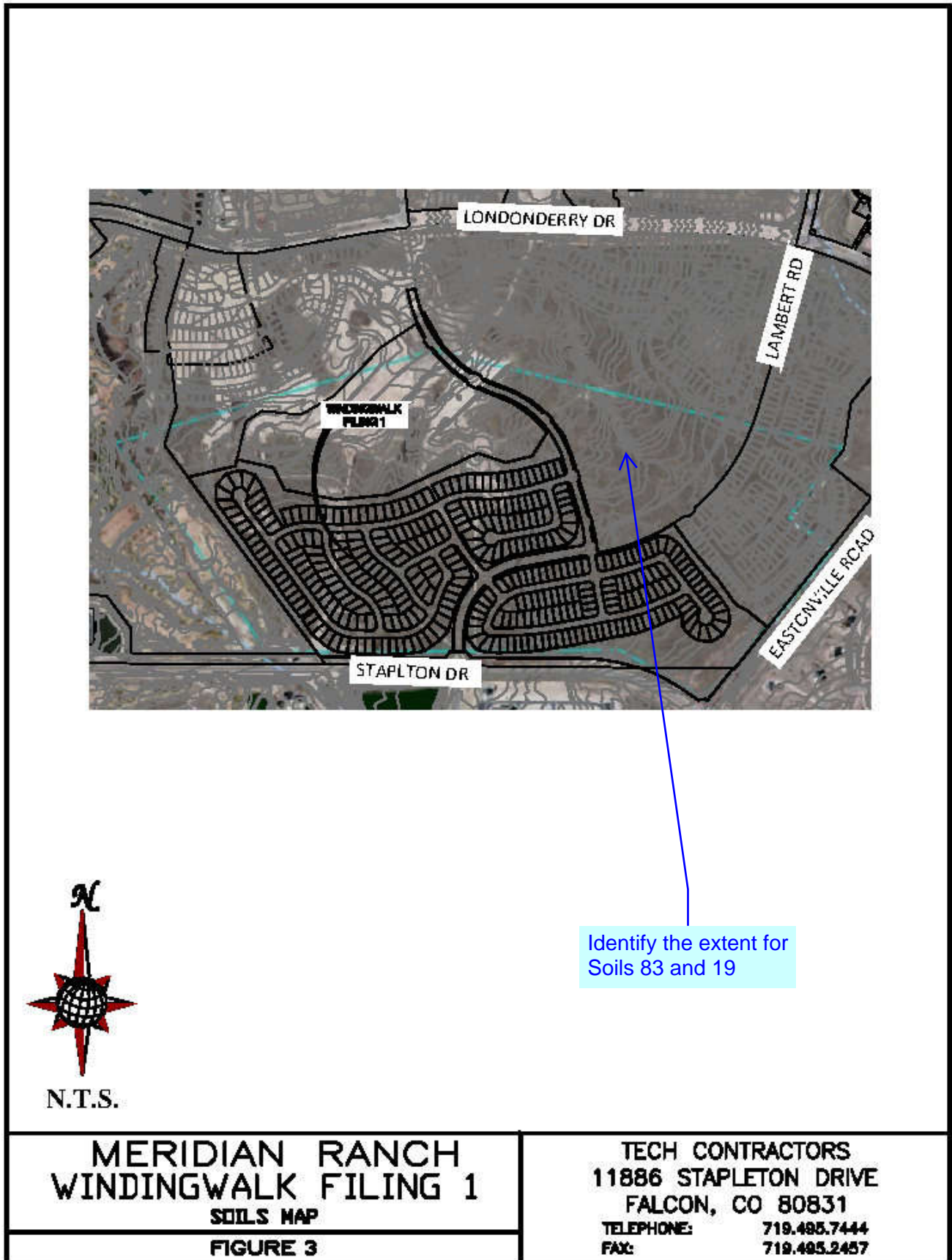
WINDINGWALK FILING 1 AT MERIDIAN RANCH

Figure 2: FEMA Floodplain Map



WINDINGWALK FILING 1 AT MERIDIAN RANCH

Figure 3: Soils Map



The Columbine (19) gravelly sandy loam is a deep, well-drained to excessively drained soil formed in coarse textured material on alluvial terraces, fans and flood plains. Permeability of this soil is very rapid. Available water capacity is low to moderate, surface runoff is slow, and the hazard of erosion is slight to moderate. This soil is used mainly for grazing livestock, for wildlife habitat and for home sites. The main limitation of this soil for urban development is a hazard of flooding in some areas.

The Stapleton (83) sandy loam is a deep, non-calcareous, well-drained soil formed in alluvium derived from arkosic bedrock on uplands. Permeability of this soil is rapid. Available water capacity is moderate, surface runoff is slow, and the hazard of erosion and soil blowing is moderate. This soil is suited to habitat for open land and rangeland wildlife. The main limitation of this soil for urban development is frost-action potential.

Typically, these soils are well-drained, gravelly sandy loams that form on alluvial terraces and fans and exhibit high permeability and low available water capacity with depth to bedrock greater than 6 feet.

Note: (#) indicates Soil Conservation Survey soil classification number. See Figure 3 Windingwalk Filing 1 at Meridian Ranch – Soils Map.

Natural Hazards Analysis

Natural hazards analysis indicates that no unusual surface or subsurface hazards are located near the vicinity. However, because the soils are cohesionless, sloughing of steep banks during drilling and/or excavation could occur. By citing improvements in a manner that provides an opportunity to lay the banks of excavations back at a 1:1 slope during construction, the problems associated with sloughing soils can be minimized.

DRAINAGE BASINS AND SUB-BASINS

The site is within the Bennett Ranch and the Haegler Ranch Basins and accepts flow from areas north of the project site within portions of Meridian Ranch.

Three different scenarios were analyzed for the drainage condition for the Windingwalk Filing 1 at Meridian Ranch grading.

The first scenario analyzes the historic conditions for Windingwalk Filing 1 at Meridian Ranch. This condition has all of the Meridian Ranch development in the pre-development state. Where the entirety of Meridian Ranch is modeled in its predeveloped, undisturbed condition.

The second scenario, the interim conditions scenario is the existing conditions with the addition of Windingwalk Filing 1 in the developed condition. This condition was analyzed to ensure that historic conditions at given design points along Eastonville Road and Stapleton Drive were maintained after Windingwalk Filing 1 is completed.

The final scenario analyzes the future build out conditions to ensure the storm drain facilities located at the discharge points of the project are able to properly convey the historic peak flow rates as the storm drainage exits the project.

DRAINAGE DESIGN CRITERIA

SCS Hydrograph Procedure

The US Army Corp of Engineers HEC-HMS computer program was used to model the Soil Conservation Service (SCS) Hydrograph procedure was used to determine final design parameters for the major drainage facilities within the project. Onsite basin areas were calculated using aerial topography of the site and approved final design data. Times of concentration were estimated using the SCS procedures described in the DCM. Based upon the hydrologic soil type, the natural conditions found in the basins and the runoff curve numbers (CN) chart from Table 6-10 of the City of Colorado Springs DCM for Antecedent Runoff Condition II (ARC II), the following CN values were used for the given conditions.

Table 1: SCS Runoff Curve Numbers

Condition	CN	School	80
Residential Lots (5 acre)	63	Parks/Open Space	62
Residential Lots (2.5 acre)	66	Commercial	85
Residential Lots (1 acre)	68	Roadways	98
Residential Lots (1/2 acre)	70	Graded	67
Residential Lots (1/3 acre)	72	Golf Course	62
Residential Lots (1/4 acre)	75	Latigo Undeveloped	65
Residential Lots (1/5 acre)	78	Undeveloped	61
Residential Lots (1/6 acre)	80		

*Curve Numbers were interpolated and based on amount of impervious area per lot. The 24 hour storm precipitation values were selected from the NOAA Atlas 14, Volume 8, Version 2 for the Meridian Ranch location (Latitude 38.9783°, Longitude -104.5842°, Elevation 7054 ft). These numbers along with SCS information were used as input to the U.S. Army Corp of Engineers HEC-HMS computer model to determine design runoffs. See the table for all the design storm events in Appendix A. These numbers along with SCS information were used as input to the U.S. Army Corp of Engineers HEC-HMS computer model to determine design runoffs.

Full Spectrum Design

The City of Colorado Springs adopted a new Drainage Criteria Manual (DCM) in 2014 which incorporated the use of *Full Spectrum Design* for storm drainage analysis for projects located within the city limits. Full Spectrum analyzes the storm water runoff for the 2-year, 5-year, 10-year, 25-year, 50-year and the 100-year design storms in order ensure the analysis more accurately project the conditions of post development. El Paso County adopted portions of the City’s 2014 DCM by resolution in January 2015; the County resolution adopted Chapter 6 (Hydrology) and Section 3.2.1 of Chapter 13 (Full Spectrum Detention) for projects outside of the City of Colorado Springs establishing a 1 year review period to analyze the impacts of the Full Spectrum Design on the storm drainage analysis of projects. This report has incorporated the use of full spectrum in the analysis of the interim and future conditions.

The idea behind full spectrum detention is to release the developed runoff flows to at or below those of the pre-developed condition. The design of Pond H and control structure meets or exceeds the intent and spirit of the concept.

Table 2: Detention Pond Summary:

POND H						
	PEAK INFLOW	PEAK OUTFLOW	TOTAL INFLOW	TOTAL OUTFLOW	PEAK STORAGE	PEAK ELEVATION
	CFS	CFS	AC-FT	AC-FT	AC-FT	FT
INTERIM CONDITIONS						
5-YEAR STORM	20	3	3.4	2.8	1.6	6971.2
10-YEAR STORM	35	5	5.3	4.0	2.6	6971.7
25-YEAR STORM	61	13	8.7	6.7	4.1	6972.2
50-YEAR STORM	86	22	11.9	9.7	5.3	6972.7
100-YEAR STORM	116	38	15.7	13.3	6.7	6973.1
FUTURE CONDITIONS						
5-YEAR STORM	52	4	4.5	3.5	2.3	6971.5
10-YEAR STORM	81	8	6.7	5.2	3.3	6971.9
25-YEAR STORM	129	19	10.5	10.5	4.9	6972.5
50-YEAR STORM	174	32	14.0	11.8	6.4	6973.0
100-YEAR STORM	226	60				

revise the verbiage. This report is associated with the subdivision development, not the early grading operation associated with EGP-17-001. Review the rest of the narrative and update accordingly.

DRAINAGE CALCULATIONS

General Concept

The grading operations associated with Windingwalk Filing 1 are located within portions of the Bennett Ranch and the Haegler Ranch Basins. Storm water runoff will be conveyed across the site overland to proposed temporary sedimentation ponds. Those portions of the site tributary to the Bennett Ranch Basin will be directed to an existing sedimentation pond prior to being released into the adjacent channel then conveyed downstream to the existing Bennett Ranch Regional Detention facility. Those portions of the Haegler Ranch Basin will be directed overland to a series of sedimentation ponds. The proposed detention facilities will be released the proposed Pond H detention. The proposed detention facilities will be a combination sedimentation/detention pond until such time as the site has sufficient ground cover or development in the area is complete.

Identify when these temporary ponds, which were installed as part of the early grading, be removed.

The facilities have been adequately sized such that the developed flows will be detained and released at or below the historic flow rates for the various design storm events as outlined in the El Paso County DCM and those sections of the City of Colorado Springs DCM adopted by the El Paso County. Existing facilities located downstream of the project are at historic flow rates from Meridian Ranch.

For clarity, reference the specific SCS Model ID

The portion of the site located within the Bennett Ranch Basin is tributary to an existing 48” RCP storm drain pipe located within Lambert Road. The storm drain conveys the flow southerly toward the North Channel where it discharges into the Bennett Regional Detention Pond located within the Bennett Ranch Basin. The storm drain was designed using the old criteria hydrologic methods and was expected to accept 143 CFS for the 100-year storm

Reference the basin ID

Is this a different basin from the previous paragraph? For clarity, reference the specific SCS Model ID.

event, the results of this analysis estimates 148 CFS will be discharged into the 48" RCP. The storm drain was hydraulically analyzed against the new CFS value and found to be sized adequately to convey the flow.

A portion of the site is tributary to a channel that is tributary to the existing Bennett Regional Detention Pond located within the Bennett Ranch Basin, the pond was designed using the old criteria hydrologic methods and with a release rate approximating the flow rates for the 5-year and the 100-year storm event releasing the developed peak flows below the historic flow rates for the design storms. **Categorically state whether or not the existing Bennett Regional Pond has the capacity to provide detention for the proposed Winding Walk and Enclaves.**

A portion of the site is tributary to the proposed Pond H located within the Haegler Ranch Basin, the pond was designed with this report using the new criteria hydrologic methods and with a release rate approximating the historic peak flow rates for the full spectrum of storm events. The analysis shows the pond releasing the developed peak flows below the historic flow rates for the full spectrum of design storms and below 90 percent of the 100-year historic flow rate for that location. Additionally, the release rate of the 2-year storm event has been calculated to be 2 CFS.

Figure 4: Meridian Ranch SCS Calculations – Historic Conditions Map, Figure 5: Meridian Ranch SCS Calculations – Interim Conditions and Figure 6: Meridian Ranch SCS Calculations – Future Conditions Map depict the historic, graded and future development patterns for Windingwalk Filing 1 portion of Meridian Ranch. **development of Windingwalk Filing 1 & 2.**

The purpose of this report is to show that the grading of Windingwalk Filing 1 at Meridian Ranch will not adversely impact the existing drainage facilities adjacent and downstream of the graded area and the proposed Pond H is properly sized for the anticipated future development of the area tributary to the pond. Further evaluation will be necessary at each stage of future development within the Meridian Ranch and the anticipated build-out is reached.

Specify the specific basin where future development will occur.

SCS Calculations

Historic Drainage - SCS Calculation Method

Following is a tabulation of the surface drainage characteristics under Existing Conditions using the SCS calculation method. Please refer to Figure 4 - Meridian Ranch SCS Calculations - Historic Basin Map.

Table 3: Historic Drainage Basins – SCS

HISTORIC							
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q100 (CFS)	DISCHARGE PEAK Q50 (CFS)	DISCHARGE PEAK Q25 (CFS)	DISCHARGE PEAK Q10 (CFS)	DISCHARGE PEAK Q5 (CFS)	DISCHARGE PEAK Q2 (CFS)
OS02	0.2219	148	102	65	30	13	3.0
B01	0.2219	148	102	65	30	13	3.0
B01-B07	0.2219	148	102	65	30	13	3.0
OS03	0.1984	130	88	55	23	9	2.0
B02-B03	0.1984	129	88	55	23	9	2.0
HB01	0.0234	19	13	8	3	1	0.0
B03	0.2218	140	95	59	25	10	2.0
B03-B07	0.2218	140	94	59	25	10	2.0
OS04	0.1359	83	54	32	12	4	1.0
B04-B05	0.1359	82	54	32	12	4	1.0
HB03	0.1266	103	68	41	15	5	1.0
B05	0.2625	144	91	52	20	7	1.0
B05-B07	0.2625	144	91	52	20	7	1.0
HB02	0.1063	77	51	30	11	4	0.0
HB04	0.0609	47	31	19	7	2	0.0
B07	0.8734	519	344	207	86	33	6.0
B07-B12	0.8734	518	343	207	86	33	6.0
HB05	0.1375	102	67	40	15	5	1.0
HB06	0.1641	111	73	43	16	5	1.0
B12	1.175	679	440	259	103	40	7.0
B12-PB	1.175	677	440	259	103	39	7.0
HB07	0.0313	29	19	12	4	1	0.0
POND B	1.2063	688	446	262	105	40	7.0
PB-19	1.2063	687	444	261	104	40	7.0
OS01	1.5594	757	510	316	136	55	11
OS01-B19	1.5594	756	509	315	136	55	11
HB08	0.1344	81	53	32	12	4	1.0
HB09	0.3047	138	90	54	21	7	1.0
B19	3.2048	1563	1041	635	266	105	20
B19-B26	3.2048	1563	1039	634	266	105	20
HB10	0.3047	172	113	67	26	9	1.0
HB12	0.0797	54	36	21	8	3	0.0
HB12-B26	0.0797	54	35	21	8	3	0.0
B26	3.5892	1737	1147	693	288	113	21
26-32	3.5892	1734	1146	693	287	113	21
HB11	0.1125	60	40	23	9	3	0.0
32	3.7017	1782	1177	709	293	115	22
32-37	3.7017	1782	1175	708	293	115	22
B-14	0.4039	178	117	70	27	10	2.0
B-13	0.2813	127	83	50	19	7	1.0
36	0.6852	306	200	119	47	17	3.0
36-37	0.6852	305	200	119	47	17	3.0
B-15	0.075	39	26	15	6	2	0.0
37	4.4619	2117	1391	834	338	131	25
HH01	0.0984	70	46	27	10	3	0.0
H12	0.0984	70	46	27	10	3	0.0

Since the drainage map only shows a portion of the SCS Hydrologic Element shown on this table, add a footnote referencing the MDDP (include the PCD Project No. if know) that provides the complete analysis.

Clearly identify the pertinent hydrologic element.

Similar comment applies to the subsequent Tables.

Interim Drainage - SCS Calculation Method

Following is a tabulation of the surface drainage characteristics for the interim conditions using the SCS calculation method. Please refer to Figure 5 - Meridian Ranch SCS Calculations – Interim Basins Map

Table 4: Interim Drainage Basins-SCS

INTERIM CONDITIONS							
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q100 (CFS)	DISCHARGE PEAK Q50 (CFS)	DISCHARGE PEAK Q25 (CFS)	DISCHARGE PEAK Q10 (CFS)	DISCHARGE PEAK Q5 (CFS)	DISCHARGE PEAK Q2 (CFS)
OS01	1.5594	757	510	316	136	55	11
DB16	0.0578	92	72	54	35	23	13
B10	1.6172	794	537	335	147	62	13
B10-B11	1.6172	793	537	335	147	62	13
DB17	0.0048	16	13	11	9	7	6
B11	1.622	795	538	336	148	63	15
B11-POND C	1.622	795	538	336	148	63	15
DB21	0.0519	54	38	25	12	5	1
DB18	0.0346	64	50	39	26	18	10
DB19	0.0281	36	27	20	11	7	3
DB20	0.0147	25	19	15	9	6	3
POND C	1.7513	749	507	310	129	50	11
POND C-B16	1.7513	749	507	310	128	50	11
DB25	0.0211	45	35	27	18	12	7
B16	1.7724	754	511	313	130	51	11
B16-B17	1.7724	754	510	312	130	51	11
DB26	0.0682	136	110	88	62	46	29
B17	1.8406	778	529	326	138	56	34
B17-B18	1.8406	778	529	326	138	56	34
OS03	0.1984	130	88	55	24	9	2
DB01	0.0719	90	66	46	25	14	5
B01	0.2703	199	139	89	42	19	5
B01-B02	0.2703	199	138	89	42	19	5
OS02	0.2219	148	102	65	30	13	3
DB02	0.0516	71	52	36	20	10	3
B02	0.5438	380	263	169	79	36	9
B02-POND A	0.5438	379	263	169	79	36	9
OS04	0.1359	83	54	32	12	4	1
DB03	0.0703	70	49	32	16	7	2
B03	0.2062	145	98	61	26	10	2
B03-B04	0.2062	145	98	60	26	10	2
DB04	0.0422	44	31	21	10	5	1
DB05	0.0384	37	27	18	9	5	1
B04	0.2868	218	150	94	42	18	4
B04-B05	0.2868	218	149	94	42	18	4
DB06	0.0219	44	35	28	19	14	9
B05	0.3087	253	176	115	55	26	10
B05-POND A	0.3087	252	176	114	55	26	10
DB07	0.0254	35	26	18	10	6	2
DB08	0.0297	32	22	15	7	3	1
POND A	0.9076	557	401	244	98	34	6
POND A-B06	0.9076	557	400	244	98	34	6
DB09	0.0189	34	26	19	12	8	4
B06	0.9265	565	407	248	100	35	6
B06-B07	0.9265	564	406	248	99	35	6
DB11	0.0969	114	85	60	35	20	8
DB10	0.0364	56	43	32	19	12	6
B07	1.0598	652	469	286	116	42	15
B07-B09	1.0598	651	468	285	116	42	14
DB12	0.0453	81	63	48	31	21	11
B09	1.1051	677	486	296	121	45	19
B09-POND B	1.1051	676	485	296	121	45	19
DB15	0.1234	105	75	50	25	12	3
DB13	0.0703	89	67	49	29	18	8
DB14	0.0556	93	72	54	35	23	12
POND B	1.3544	688	539	337	140	69	30
POND B-B12	1.3544	688	539	336	140	69	30
DB22	0.0516	91	72	55	36	25	14
DB23	0.0172	45	38	31	23	18	13
B12	1.4232	714	562	353	148	83	38

INTERIM CONDITIONS							
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q100 (CFS)	DISCHARGE PEAK Q50 (CFS)	DISCHARGE PEAK Q25 (CFS)	DISCHARGE PEAK Q10 (CFS)	DISCHARGE PEAK Q5 (CFS)	DISCHARGE PEAK Q2 (CFS)
B12-B14	1.4232	714	562	352	148	83	38
DB24	0.0531	94	73	56	36	24	13
B14	1.4763	743	577	363	162	92	46
B14-B15	1.4763	742	576	362	162	92	46
DB28	0.0741	67	49	34	18	10	4
B15	1.5504	800	605	380	179	102	49
B15-B18	1.5504	798	605	379	178	102	49
DB29	0.1697	146	105	71	37	19	6
DB27	0.0508	68	53	40	25	17	9
B26	3.6115	1623	1178	735	315	174	87
B26-27	3.6115	1622	1178	735	315	173	87
FB-02	0.05	86	68	51	33	23	13
FB-01	0.0373	41	29	20	10	5	1
FB01-27a	0.0373	41	29	20	10	5	1
B19	0.0873	127	97	71	43	27	13
B19-27	0.0873	126	96	70	43	27	13
FB-03	0.0078	23	19	15	11	9	6
27	3.7066	1648	1197	749	322	189	94
27-32	3.7066	1648	1196	748	322	188	93
WH-24	0.1325	218	171	129	84	57	31
WH-26	0.0839	49	33	20	8	3	1
WH-27	0.0217	23	16	10	4	1	0
30	0.2381	271	205	150	91	59	31
30-31	0.2381	270	205	149	91	59	31
WH-28	0.0398	60	47	36	23	15	8
31	0.2779	330	252	185	114	74	39
31-32	0.2779	329	251	185	113	74	39
WH-29	0.0495	77	60	45	29	19	10
WH-31	0.0406	75	60	46	30	21	12
WH-30	0.0159	26	19	13	7	4	1
32	4.0905	1790	1290	809	419	257	123
WH32	0.0458	55	38	24	10	4	0
BEN POND	4.1363	1393	984	595	252	99	44
WH-33	0.0064	12	9	7	5	3	2
33	4.1427	1394	985	596	252	100	44
33-37	4.1427	1394	984	595	252	100	44
WH35	0.155	171	124	84	44	22	6
WH34	0.045	68	52	38	23	15	7
B34-36	0.045	68	52	38	23	15	7
36	0.2	239	176	122	67	37	13
36-37	0.2	238	174	121	66	37	13
WH36	0.075	63	43	27	11	4	1
37	4.4177	1432	1014	615	262	104	47
FH01	0.1344	196	147	106	62	37	15
POND H	0.1344	43	23	13	5	3	2
FH02	0.0138	19	14	10	6	4	1
H12	0.1482	48	26	15	8	5	2

A comparison of the peak flow rates at Eastonville Road for the design storms may be found in Table 6 – Key Design Point Comparison (below). As a result of the grading of the Windingwalk Filing 1, the calculations show that the project does not adversely affect the existing drainage facilities.

Future Drainage - SCS Calculation Method

Following is a tabulation of the surface drainage characteristics for the future conditions using the SCS calculation method. Please refer to Figure 6 - Meridian Ranch SCS Calculations – Future Basins Map

Table 5: Future Drainage Basins-SCS

FUTURE CONDITIONS							
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q100 (CFS)	DISCHARGE PEAK Q50 (CFS)	DISCHARGE PEAK Q25 (CFS)	DISCHARGE PEAK Q10 (CFS)	DISCHARGE PEAK Q5 (CFS)	DISCHARGE PEAK Q2 (CFS)
OS01	1.5594	757	510	316	136	55	11
DB16	0.0578	92	72	54	35	23	13
B10	1.6172	794	537	335	147	62	13
B10-B11	1.6172	793	537	335	147	62	13
DB17	0.0048	16	13	11	9	7	6
B11	1.6220	795	538	336	148	63	15
B11-POND C	1.6220	795	538	336	148	63	15
DB21	0.0519	54	38	25	12	5	1
DB18	0.0346	64	50	39	26	18	10
DB19	0.0281	36	27	20	11	7	3
DB20	0.0147	25	19	15	9	6	3
POND C	1.7513	749	507	310	129	50	11
POND C-B16	1.7513	749	507	310	128	50	11
DB25	0.0211	45	35	27	18	12	7
B16	1.7724	754	511	313	130	51	11
B16-B17	1.7724	754	510	312	130	51	11
DB26	0.0682	136	110	88	62	46	29
B17	1.8406	778	529	326	138	56	34
B17-B18	1.8406	778	529	326	138	56	34
OS03	0.1984	130	88	55	24	9	2
DB01	0.0719	90	66	46	25	14	5
B01	0.2703	199	139	89	42	19	5
B01-B02	0.2703	199	138	89	42	19	5
OS02	0.2219	148	102	65	30	13	3
DB02	0.0516	71	52	36	20	10	3
B02	0.5438	380	263	169	79	36	9
B02-POND A	0.5438	379	263	169	79	36	9
OS04	0.1359	83	53	32	12	4	1
DB03	0.0703	70	49	32	16	7	2
B03	0.2062	145	98	61	26	10	2
B03-B04	0.2062	145	98	60	26	10	2
DB04	0.0422	44	31	21	10	5	1
DB05	0.0384	37	27	18	9	5	1
B04	0.2868	218	150	94	42	18	4
B04-B05	0.2868	218	149	94	42	18	4
DB06	0.0219	44	35	28	19	14	9
B05	0.3087	253	176	115	55	26	10
B05-POND A	0.3087	252	176	114	55	26	10
DB07	0.0254	35	26	18	10	6	2
DB08	0.0297	32	22	15	7	3	1
POND A	0.9076	557	401	244	98	34	6
POND A-B06	0.9076	557	400	244	98	34	6
DB09	0.0189	34	26	19	12	8	4
B06	0.9265	565	407	248	100	35	6
B06-B07	0.9265	564	406	248	99	35	6
DB11	0.0969	114	85	60	35	20	8
DB10	0.0364	56	43	32	19	12	6
B07	1.0598	652	469	286	116	42	15
B07-B09	1.0598	651	468	285	116	42	14
DB12	0.0453	81	63	48	31	21	11
B09	1.1051	677	486	296	121	45	19
B09-POND B	1.1051	676	485	296	121	45	19

FUTURE CONDITIONS							
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q100 (CFS)	DISCHARGE PEAK Q50 (CFS)	DISCHARGE PEAK Q25 (CFS)	DISCHARGE PEAK Q10 (CFS)	DISCHARGE PEAK Q5 (CFS)	DISCHARGE PEAK Q2 (CFS)
DB15	0.1234	105	75	50	25	12	3
DB13	0.0703	89	67	49	29	18	8
DB14	0.0556	93	72	54	35	23	12
POND B	1.3544	688	539	337	140	69	30
POND B-B12	1.3544	688	539	336	140	69	30
DB22	0.0516	91	72	55	36	25	14
DB23	0.0172	45	38	31	23	18	13
B12	1.4232	714	562	353	148	83	38
B12-B14	1.4232	714	562	352	148	83	38
DB24	0.0531	94	73	56	36	24	13
B14	1.4763	743	577	363	162	92	46
B14-B15	1.4763	742	576	362	162	92	46
DB28	0.0741	82	60	42	23	13	4
B15	1.5504	787	597	375	176	102	50
B15-B18	1.5504	785	596	375	176	102	50
DB29	0.1697	146	105	71	37	19	6
DB27	0.0508	68	53	40	25	17	9
B26	3.6115	1612	1171	731	314	179	89
B26-27	3.6115	1612	1170	731	314	179	89
FB-02	0.0500	67	53	40	26	17	10
FB-01	0.0373	40	28	19	9	5	1
FB01-27a	0.0373	40	28	19	9	5	1
B19	0.0873	103	78	57	34	22	11
B19-27	0.0873	103	78	57	34	22	11
FB-03	0.0078	22	18	15	11	8	6
27	3.7066	1647	1197	748	322	199	98
27-32	3.7066	1647	1196	748	322	198	98
WH-24	0.1325	218	171	129	84	57	31
WH-26	0.0839	49	33	20	8	3	1
WH-27	0.0217	23	16	10	4	1	0
30	0.2381	271	205	150	91	59	31
30-31	0.2381	270	205	149	91	59	31
WH-28	0.0398	60	47	36	23	15	8
31	0.2779	330	252	185	114	74	39
31-32	0.2779	329	251	185	113	74	39
WH-29	0.0495	77	60	45	29	19	10
WH-31	0.0406	75	60	46	30	21	12
WH-30	0.0159	26	19	13	7	4	1
32	4.0905	1792	1290	809	435	269	128
WH32	0.0458	55	38	24	10	4	0
BEN POND	4.1363	1394	985	595	252	100	44
WH-33	0.0064	12	9	7	5	3	2
33	4.1427	1395	986	596	253	100	44
33-37	4.1427	1395	985	596	253	100	44
WH35	0.1550	171	124	84	44	22	6
WH34	0.0450	68	52	38	23	15	7
B34-36	0.0450	68	52	38	23	15	7
36	0.2000	239	176	122	67	37	13
36-37	0.2000	238	174	121	66	37	13
WH36	0.0750	63	43	27	11	4	1
37	4.4177	1433	1015	616	262	104	47
4W4-1-4W4	1.2304	279	194	130	67	35	14
4W-D	0.0560	56	39	25	11	5	1
4W4	1.2864	321	228	152	76	38	15
FG34	0.0922	64	43	27	12	5	1

A comparison of the peak flow rates at Eastonville Road for the design storms may be found in Table 6 – Key Design Point Comparison (below). As a result of the future development of the Windingwalk Filing 1, the calculations show that the project does not adversely affect the existing drainage facilities.

Table 6: Key Design Point Comparison - SCS

KEY DESIGN POINT FLOW RATES					
EVENT	HISTORIC	INTERIM	PERCENT OF HISTORIC	FUTURE	PERCENT OF HISTORIC
	PEAK FLOW (CFS)	PEAK FLOW (CFS)		PEAK FLOW (CFS)	
POND H (H12)					
5-YEAR	3.0	2.9	97%	3.7	123%
10-YEAR	10	5.2	52%	8.1	81%
25-YEAR	27	13	48%	18	67%
50-YEAR	46	23	50%	31	67%
100-YEAR	70	43	61%	55	79%
BENNETT POND OUTLET (B32)					
5-YEAR	115	99	86%	100	87%
10-YEAR	293	252	86%	252	86%
25-YEAR	709	595	84%	595	84%
50-YEAR	1177	984	84%	985	84%
100-YEAR	1782	1393	78%	1394	78%
JUDGE ORR ROAD (B37)					
5-YEAR	131	104	79%	104	79%
10-YEAR	338	262	78%	262	78%
25-YEAR	834	615	74%	616	74%
50-YEAR	1391	1014	73%	1015	73%
100-YEAR	2117	1432	68%	1432	68%

Windingwalk Filing 1 final plat application (PCD File No. SF-18-002)

Pond H Detention Storage Criteria

Detention Pond H is to be constructed as a part of the Windingwalk Filing 1 in anticipation of future development in accordance with the approved Sketch Plan within the tributary area. The proposed pond is located within the Haegler Ranch Drainage Basin near the southeastern corner of Meridian Ranch near the intersection of Eastonville Road and Stapleton Drive. The pond will be owned and maintained by the Meridian Service Metropolitan District (MSMD). A maintenance agreement between the Meridian Service Metropolitan District and El Paso County is included with the grading package. Pond H is included in the 2009 Drainage Basin Planning Study as a reimbursable Sulphur Creek. The DBPS estimated the construction and engineering fees associated with this development as a result of the grading and the future development will not adversely impact drainage patterns downstream of the project.

Provide a detailed comparison analysis between the proposed design and the DBPS analysis. Are the analysis between this report and the DBPS comparable or significantly different? What is the difference in the detention volume and release rates? Provide a statement identifying whether or not this FDR is in conformance to the DBPS.

The SCS calculation method was used with the aid of the HydroCAD program to determine inflow and outflow from the detention pond to ensure the excess runoff as a result of the grading and the future development will not adversely impact drainage patterns downstream of the project.

The pond is designed to accommodate the final inflow from Windingwalk Filing 1 at Meridian Ranch near the intersection of Eastonville Road and Stapleton Drive. Permanent

Include some pertinent sheets from the DBPS within the Appendix for reference. Staff recommends Table 7-4 (highlight the relevant pond), Sheet SR01, Sheet M-25 (Figure 5-4). You may want to overlay Windingwalk and Stapleton to show that it is at the approximate location as Pond H since the DBPS does not have any readily identifiable reference points.

concrete control structures has been designed to handle full build out of the tributary area and reduce the developed flows to at or below the historic full spectrum peak flow rates.

A WQCV analysis for Pond H was also performed based on proposed future development of the proposed tributary area to the pond; this analysis shows that Pond H will require 0.3 acre-ft of storage for first flush water quality for all the areas tributary to the pond. The control structure at DP H12 is proposed to consist of a 10” water quality control riser with a trash grate having a top elevation of 6970.0 to achieve the required 0.3 ac-ft of storage.

The WQCV was calculated by using the equations found in Volume 2, of the Drainage Criteria Manual (DCM). The release rate from the WQCV is generally very small, which helps minimize downstream impacts. Detaining the WQCV also serves to cleanse the “first flush” of runoff from the higher initial concentration of sediment and pollutants by allowing for settlement to occur. This greatly improves the quality of runoff, leaving the facility and reduces the potential for erosion. The positive impact on water quality is expected to be significant, particularly during the construction phase of the development.

A concrete control structure is proposed for the outlet of Pond H. The structure will attenuate the peak developed flow rates to historic peak rates or less for the full spectrum of design storms as per the requirements set forth in Resolution 15-042 adopted by the Board of County Commissioners, County of El Paso. The control structure will consist of a water quality control standpipe, a rectangular slotted orifice located on the front and a grated top to reduce the developed peak flow rates. Table 7 provides summary data for the various design storms in both the interim graded condition and future developed condition.

Table 7: Pond H Summary Data

POND H						
	PEAK INFLOW	PEAK OUTFLOW	TOTAL INFLOW	TOTAL OUTFLOW	PEAK STORAGE	PEAK ELEVATION
	CFS	CFS	AC-FT	AC-FT	AC-FT	FT
INTERIM CONDITIONS						
5-YEAR STORM	20	3	3.4	2.8	1.6	6971.2
10-YEAR STORM	35	5	5.3	4.0	2.6	6971.7
25-YEAR STORM	61	13	8.7	6.7	4.1	6972.2
50-YEAR STORM	86	22	11.9	9.7	5.3	6972.7
100-YEAR STORM	116	38	15.7	13.3	6.7	6973.1
FUTURE CONDITIONS						
5-YEAR STORM	52	4	4.5	3.5	2.3	6971.5
10-YEAR STORM	81	8	6.7	5.2	3.3	6971.9
25-YEAR STORM	129	19	10.5	10.5	4.9	6972.5
50-YEAR STORM	174	32	14.0	11.8	6.4	6973.0
100-YEAR STORM	226	60	18.1	15.7	7.8	6973.5

Rational Calculations

The Rational Hydrologic Calculation Method was used to estimate the total runoff from the design storm and thus establish the storm drainage system design. Using the rational

calculation methodology outlined in the Hydrology Section (Ch 6) of the COSDCM coupled with the El Paso County EPCDCM an effective storm drainage design for the Windingwalk Filing 1 has been designed. The storm drainage facility has been designed such that the minor storm will be captured by the inlets and conveyed by the storm drain pipes such that the street flow does not overtop the curbs. The storm drainage facility has been designed such that the major storm will be captured by the inlets and conveyed by the storm drain pipes such that the street flow does not exceed the right-of-way widths for residential streets and the hydraulic grade line will be more than one foot below the surface.

The eastern portion of the site is located within the Haegler Ranch Drainage Basin and the western portion of the site is within the Bennett Ranch Drainage Basin. The project will discharge the collected surface flow from the project into existing downstream facilities properly sized to safely convey the storm water flows away from the project without damaging adjacent property.

The Haegler Ranch portion will consist of a single backbone storm drain system along Rainbow Bridge Dr ranging in size from 24" to 48" that collects runoff from laterals and inlets, conveying the collected flow southerly and discharging the storm water into Pond H located near northwest of the intersection of Stapleton Drive and Eastonville Road. The storm water (64 CFS) will be released into an existing storm drainage system below historic flow rates and significantly below the anticipated design flow (87 CFS) when the system was designed and constructed. Flanked on either side will be lateral storm drain pipes with inlets of various sized to collect the developed runoff from the proposed and future single family development within the Haegler Basin.

Include the existing pipe capacity calculation.

The Bennett Ranch portion will discharge the collected surface flow into the main Bennett Ranch channel or into an existing 48" RCP located at the intersection of Stapleton Drive and Lambert Road. A storm drain pipe ranging in size from 18" to 30" with several inlets flanked on either side is located along Fairway Glen Circle on the far western portion of the subdivision will collect the surface runoff before discharging into an existing sedimentation basin. The flow will combine with other upstream flow from Meridian Ranch where it will cross under Stapleton Drive then be conveyed to the Bennett Ranch Regional Detention Pond. The middle portion of the subdivision will have its surface runoff collected by a storm drain system located along Winding Park Lane and Lambert Road with pipe sizes ranging from 30" to 48" before discharging the flow into an existing 48" RCP located at Stapleton Drive and Lambert Road. Several inlets and laterals are located along this storm drain that will collect the surface runoff from the proposed subdivision and future development to the north. The storm water (148 CFS) will be released into an existing storm drainage system located along Lambert Road at rates near the anticipated design flow (143 CFS) when the system was designed and constructed. The flow cross Stapleton Drive and continue along Lambert Road where it will be discharged into the main Bennett Ranch Channel then be conveyed to the Bennett Ranch Regional Detention Pond. The Bennett Ranch Regional Detention Pond was designed and constructed to provide water quality and detain developed runoff from the Bennett Ranch portions of both the Meridian Ranch and Woodmen Hills subdivisions such that the storm flow rates at Judge Orr Road and the Bennett Channel are at or near 80% of historic flow rates.

Hydraulic analyses were completed on the three storm drain sections, the two detention ponds, and the existing downstream facilities; the analyses show the existing facilities are sized adequately to accept, convey and discharge the design flow without adversely affecting downstream properties.

Under the Rational Calculation section of the report provide a narrative for all the sub-basins. Attached are examples. Review of the subbasins will be done with the resubmittal once the sub-basin descriptions are provided.



EROSION CONTROL DESIGN

replace with "temporary sediment basins"

General Concept

Historically, erosion on this property has been held to a minimum by a variety of natural features and agricultural practices including:

- Substantial prairie grass growth
- Construction of drainage arresting berms
- Construction of multiple stock ponds along drainage courses

Existing detention ponds will also help to minimize erosion by reducing both the volume and velocity of the peak runoff.

During construction, best management practices (BMP) for erosion control will be employed based on El Paso county Criteria. BMP's will be utilized as deemed necessary by the contractor and/or engineer and are not limited to the measures shown on the construction drawing set. The contractor shall minimize the amount of area disturbed during all construction activities.

In general the following shall be applied in developing the sequence of major activities:

- Install down-slope and side-slope perimeter BMP's before the land disturbing activity occurs.
- Do not disturb an area until it is necessary for the construction activity to proceed
- Cover or stabilize as soon as possible.
- Time the construction activities to reduce the impacts from seasonal climatic changes or weather events.
- The construction of filtration BMP's should wait until the end of the construction project when upstream drainage areas have been stabilized.
- Do not remove the temporary perimeter controls until after all upstream areas are stabilized.

Four Step Process

The following four step process is recommended for selecting structural BMP's in developing urban areas:

Step 1: Employ Runoff Reduction Practices

This development incorporates wider rights-of-way than other developments, thus decreasing the amount area devoted to pavement. The rights-of-way within Meridian Ranch are 20% wider, 60 ft. instead of 50 ft., creating more landscaped area within the development.

The project has over ten acres of open space, accounting for over 20% of the entire project, creating a lower density development.

Home owners and builders are encouraged to direct roof drains to the sideyards where the runoff will travel overland to the streets and creating an opportunity to allow the runoff to infiltrate into the ground.

Step 2: Stabilize Drainageways

The drainage swale located adjacent and south of the project was designed to have a wide flat bottom and slope reducing the velocity of the concentrated flow traveling along the drainageway. The construction of the swale also included erosion control mat along the entire length of the swale. At steeper sections of the swale straw logs or rip-rap has been installed to reduce velocities and erosion.

Step 3: Provide Water Quality Capture Volume (WQCV)

An existing extended detention pond with water quality capture volume is located to the east of the project that was designed to accommodate the runoff from this development.

Step 4: Consider Need for Industrial and Commercial BMP's

This project is neither industrial nor commercial and therefore this section does not apply.

Detention Pond

The existing detention ponds will act as the primary sedimentation control facility for the areas upstream. Runoff will be diverted into the detention pond where practical. The pond will serve a dual purpose: first, by facilitating the settling of sediment in runoff during and after construction (by means of the WQCV) and, second, by maintaining runoff at or below existing levels.

Silt Fence

Silt fence will be placed along downstream limits of disturbed areas. This will prevent suspended sediment from leaving the site during infrastructure construction. Silt fencing is to remain in place until vegetation is reestablished.

Erosion Bales

Erosion bales will be placed ten (10) feet from the inlet of all culverts during construction to prevent culverts from filling with sediment. Erosion bales will remain in place until vegetation is reestablished. Erosion bale checks will be used on slopes greater than 1 percent to reduce flow velocities until vegetation is reestablished.

Miscellaneous

Best erosion control practices will be utilized as deemed necessary by the Contractor or Engineer and are not limited to the measures described above.

DRAINAGE FEES

The proposed Windingwalk Filing 1 development is located within two major drainage basins; the Bennett Ranch and the Haegler Ranch Drainage Basins. Of the 114.06 acres of Windingwalk Filing 1, 52.97 acres fall within the Bennett Ranch Basin and 61.09 acres is located within the Haegler Ranch Basin. The Bennett Ranch portion includes 11.4 acres are residential development and 3.5 acres are designated as right-of-way, and 0.8 acres landscape tract. The portion within the Haegler Ranch includes 11.4 acres of residential development and 3.5 acres designated as right-of-way, and 0.8 acres landscape tract. See the calculation below.

The following is the imperviousness calculation:

BENNETT RANCH

	<u>Acres</u>	<u>Assumed Imperviousness</u>	<u>Impervious Acres</u>
Right-of-way	11.12	85%	9.5
Residential Lots	32.28	52% (73 Lots)	16.8
Landscape Tract	9.57	5%	0.5

Discrepancy with PCD's drainage tracker which shows available credit of \$543,531.93

Update to the 2018 drainage fee = \$10,832/imp ac.

26.7 = 50.4% imp.

Bennett Ranch

Drainage Basin Fees: $52.97 \text{ ac} * \$ 9,901/\text{Ac} * 0.504 \text{ Imp Area} = \$ 264,516.00$

Meridian Ranch holds Bridge Fee credits for the construction of the Stapleton Drive bridge constructed in 200X, these credits are to be applied against the bridge fee requirements associated with this project.

Bridge Fees: $52.97 \text{ ac} * \$ 3,798/\text{Ac} * 0.504 \text{ Imp Area} = \$ 101,467.00$

Update year

Existing Credits:

\$ -521,531.63

Remaining Credits

\$ -442,064.63

Update to the 2018 bridge fee = \$4,155/imp ac.

HAEGLER RANCH

	<u>Acres</u>	<u>Assumed Imperviousness</u>	<u>Impervious Acres</u>
Right-of-way	13.96	85%	11.9
Residential Lots	29.56	52% (73 Lots)	15.4
Landscape Tract	17.57	5%	0.9
Total	61.09		28.1 = 46.0% imp.

The proposed Pond H is identified in the approved Haegler Basin Planning Study as a regional detention pond and is reimbursable to developer after construction. The estimated cost of construction of the pond found in the approved May 2009 planning study is \$430,217.

By applying the Denver Area Consumer Price Index to the original 2009 estimate, the estimated cost of the pond in 2017 will be \$521,423 (see data below)

Denver CPI		
Year	CPI-U all items 1982-84=100	Rate of Increase
2009	208.548	
2017	252.760	1.212

Add a section for Engineer's Cost Estimate for the sub-regional pond.

FYI: The lower cost between the Engineer's Cost Estimate and the DBPS cost estimate brought forward will be used as the cap that is creditable towards drainage basin fees in the event that additional filings are submitted prior to construction/completion of the pond.

Final credit will be based on actual construction cost construction will be submitted to the County for review and approval upon commencement of construction. Upon completion of construction, the contractor will submit a certification from a Colorado Professional Engineer stating the facilities have been constructed in accordance with the approved plans along with the records of the actual construction cost.

Update to 2018 drainage fee = \$9,676

Haegler Ranch

Drainage Basin Fees: $61.09 \text{ ac} * \$ 8,844/\text{Ac} * 0.460 \text{ Imp Area} = \$ 248,530.00$

Estimated Credits: \$ -521,423.00

Remaining Credits \$ -272,893.00

Bridge Fees: $61.09 \text{ ac} * \$ 1,305/\text{Ac} * 0.460 \text{ Imp Area} = \$ 36,672.00$

Update to 2018 bridge fee = \$1,428

Revise to "Sub-regional Pond SR-01 construction cost estimate"

Revise to "Estimated drainage reimbursement"

Add a 4th row identifying drainage basin fee at plat recording = 0

REFERENCES

1. “City of Colorado Springs/El Paso County Drainage Criteria Manual” September 1987, Revised November 1991, Revised October 1994.
2. Chapter 6, Hydrology and Chapter 11, Storage, Section 3.2.1 of the “City of Colorado Springs Drainage Criteria Manual” May 2014.
3. “Volume 2, El Paso County/City of Colorado Springs Drainage Criteria Manual-Stormwater Quality Policies, Procedures and Best Management Practices” November 1, 2002.
4. Flood Insurance Rate Study for El Paso County, Colorado and Incorporated Areas. Federal Emergency Management Agency, Revised March 17, 1997.
5. Soils Survey of El Paso County area, Natural Resources Conservation Services of Colorado.
6. Master Development Drainage Plan Meridian Ranch. August 2000. Prepared by URS Corp.
7. Revision to Master Development Drainage Plan Meridian Ranch. May 2015. Prepared by Tech Contractors.
8. Master Development Drainage Plan Latigo Trails. October 2001. Prepared by URS Corp.
9. Final Drainage Report for Meridian Ranch Filing 1. November 2001. Prepared by URS Corp.
10. Preliminary Drainage Plan for Meridian Ranch Phase II. September 2003. Prepared by URS.
11. Final Drainage Plan for The Trails Filing No.7. March 2005. Prepared by URS.
12. Final Drainage Report for Meridian Ranch Filing 3. August 2011. Prepared by Tech Contractors.
13. Preliminary and Final Drainage Report for Meridian Ranch Filing 7. June 2012. Prepared by Tech Contractors.
14. Final Drainage Report for Meridian Ranch Estates Filing 2. July 2013. Prepared by Tech Contractors.
15. Final Drainage Report for Meridian Ranch Filing 11A. March 2014. Prepared by Tech Contractors.

16. Preliminary and Final Drainage Report for Meridian Ranch Filing 8. December 2014. Prepared by Tech Contractors.
17. Preliminary and Final Drainage Report for Meridian Ranch Filing 4B. April 2014. Prepared by Tech Contractors.
18. Final Drainage Report for Stonebridge Filing 1 at Meridian Ranch. June 2014. Prepared by Tech Contractors.
19. Final Drainage Report for Meridian Ranch Filing 9. May 2015. Prepared by Tech Contractors.
20. Revision to Master Development Drainage Plan Meridian Ranch. July 2015. Prepared by Tech Contractors.
21. Final Drainage Report for Meridian Ranch Estates Filing 3. October 2015. Prepared by Tech Contractors.
22. Final Drainage Report for the Vistas Filing 1 at Meridian Ranch. July 2016. Prepared by Tech Contractors.
23. Final Drainage Report for Stonebridge Filing 2 at Meridian Ranch. September 2016. Prepared by Tech Contractors.
24. Final Drainage Report for Stonebridge Filing 3 at Meridian Ranch. April 2017. Prepared by Tech Contractors.
25. Revision to Master Development Drainage Plan Meridian Ranch. November 2017. Prepared by Tech Contractors.

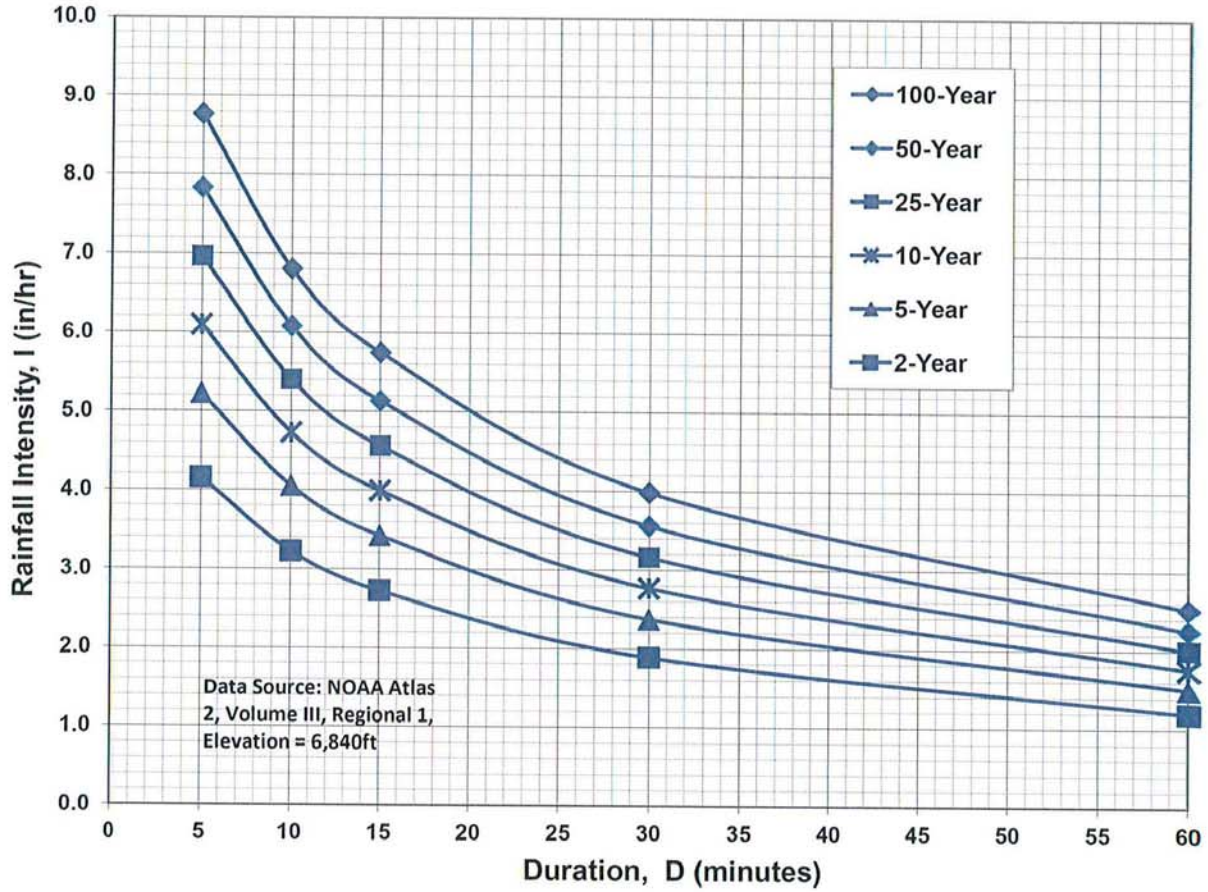
Appendices

Appendix A – Rational Calculations

Table 6-6. Runoff Coefficients for Rational Method
(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients											
		2-year		5-year		10-year		25-year		50-year		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries													
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis-- Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks													
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency



IDF Equations

$$I_{100} = -2.52 \ln(D) + 12.735$$

$$I_{50} = -2.25 \ln(D) + 11.375$$

$$I_{25} = -2.00 \ln(D) + 10.111$$

$$I_{10} = -1.75 \ln(D) + 8.847$$

$$I_5 = -1.50 \ln(D) + 7.583$$

$$I_2 = -1.19 \ln(D) + 6.035$$

Note: Values calculated by equations may not precisely duplicate values read from figure.

COMPOSITE 'C' FACTORS

PROJECT: **Windingwalk Filing 1**

12/11/2017

BASIN DESIGNATION	AREA (AC.)					COMPOSITE FACTOR		Percent Impervious	
	UNDEV	6 DU/AC	STREETS	REC CENTER	OPEN SPACE PARKS/GC	TOTAL	5-year		100-year
B01		1.9				1.9	0.40	0.55	52.0%
B02		1.9				1.9	0.40	0.55	52.0%
B03		4.4				4.4	0.40	0.55	52.0%
B04		7.4				7.4	0.40	0.55	52.0%
B05		2.5				2.5	0.40	0.55	52.0%
B06		2.8			3.0	5.8	0.32	0.48	26.2%
B07		3.3				3.3	0.40	0.55	52.0%
B08		3.2				3.2	0.40	0.55	52.0%
B09		2.4				2.4	0.40	0.55	52.0%
B10		4.1				4.1	0.40	0.55	52.0%
B11		3.3				3.3	0.40	0.55	52.0%
B12		7.1				7.1	0.40	0.55	52.0%
B13		2.3				2.3	0.40	0.55	52.0%
B14		2.5	1.4		1.0	4.9	0.51	0.64	55.8%
B15		0.5	0.6		0.3	1.4	0.58	0.69	61.3%
B16		0.8	1.9		0.5	3.2	0.66	0.76	71.5%
B17			1.7			1.7	0.90	0.96	100.0%
B18		1.6			4.6	6.1	0.28	0.44	14.8%
B19		4.1				4.1	0.40	0.55	52.0%
B20		3.3				3.3	0.40	0.55	52.0%
B21		2.0				2.0	0.40	0.55	52.0%
B22		3.9		1.3	6.4	11.6	0.34	0.49	24.4%
B23		4.0			5.8	9.8	0.30	0.46	22.3%
B24		3.1			5.9	9.1	0.30	0.46	19.3%
B25		0.8			0.7	1.5	0.32	0.48	28.5%
H01		1.0				1.0	0.40	0.55	52.0%
H02		1.9	0.7		0.9	3.5	0.45	0.59	47.9%
H03		1.2	0.7	0.5	0.5	3.0	0.54	0.66	55.7%
H04		1.8	0.4		0.2	2.4	0.46	0.60	55.3%
H05		2.0				2.0	0.40	0.55	52.0%
H06		2.5				2.5	0.40	0.55	52.0%
H07		2.4	0.5		0.3	3.1	0.46	0.60	54.9%
H08		1.5			2.9	4.4	0.29	0.46	19.1%
H09		1.1			1.7	2.8	0.30	0.46	21.4%
H10		3.6	0.8		0.6	5.0	0.46	0.59	53.8%
H11		1.4	0.4		0.2	2.0	0.48	0.61	56.0%
H12		4.9				4.9	0.40	0.55	52.0%
H13		1.3				1.3	0.40	0.55	52.0%
H14		1.5				1.5	0.40	0.55	52.0%
H15		1.9				1.9	0.40	0.55	52.0%
H16		2.3	1.1		0.7	4.1	0.50	0.63	55.4%
H17		2.4	0.7		0.4	3.4	0.48	0.61	56.0%
H18		6.0				6.0	0.40	0.55	52.0%
H19		3.8				3.8	0.40	0.55	52.0%
H20		4.6				4.6	0.40	0.55	52.0%
H21		4.0				4.0	0.40	0.55	52.0%
H22		1.8	0.8		0.4	3.0	0.51	0.64	58.5%
H23		0.0	0.7		0.3	1.0	0.67	0.77	66.6%
H24		2.0			1.2	3.3	0.34	0.49	33.2%
H25		3.7			7.6	11.3	0.29	0.45	18.3%
H26		2.7			0.9	3.6	0.36	0.51	39.0%
H27		0.3			1.9	2.2	0.26	0.43	9.0%
H28			1.5		0.2	1.7	0.83	0.90	89.4%
							Composite:		42.9%

TIME OF CONCENTRATION

SCS Calculations

PROJECT: **Windingwalk Filing 1**

DATE: 12/11/

TIME OF CONCENTRATION																
SUBBASIN DATA			INIT/OVERLAND TIME (T _i)				TRAVEL TIME (T _t)						TOTAL	T _c Check (Urbanized Basins)		
BASIN DESIGNATION	C _s	AREA (AC)	LENGTH (FT)	ΔH	SLOPE %	T _i (Min.)*	LENGTH (FT)	ΔH	SLOPE %	CONVEYANCE		VEL. (FPS)	T _t (Min.)**	T _i +T _t (Min.)	L (FT)	T _c = (L/180) + 10
										TYPE	COEF.					
B01	0.40	1.9	40	0.8	2.0%	6.4	1110	12	1.1%	P	20	2.1	8.9	15.3	1150.00	16.4
B02	0.40	1.9	40	0.8	2.0%	6.4	977	11	1.1%	P	20	2.1	7.7	14.1	1017.00	15.7
B03	0.40	4.4	40	0.8	2.0%	6.4	795	23	2.9%	P	20	3.4	3.9	10.3	835.00	14.6
B04	0.40	7.4	40	0.8	2.0%	6.4	1022	13	1.3%	P	20	2.3	7.6	14.0	1062.00	15.9
B05	0.40	2.5	40	0.8	2.0%	6.4	1433	18	1.3%	P	20	2.2	10.7	17.1	1473.00	18.2
B06	0.32	5.8	210	0.8	4.3%	12.8	580	6	1.0%	B	10	1.0	9.5	22.3	790.00	14.4
B07	0.40	3.3	145	0.8	2.1%	12.1	745	8	1.1%	P	20	2.1	6.0	18.1	890.00	14.9
B08	0.40	3.2	30	0.6	2.0%	5.6	772	13	1.7%	P	20	2.6	5.0	10.5	802.00	14.5
B09	0.40	2.4	40	0.8	2.0%	6.4	755	17	2.3%	P	20	3.0	4.2	10.6	795.00	14.4
B10	0.40	4.1	170	0.8	2.0%	13.3	775	13	1.7%	P	20	2.6	5.0	18.3	945.00	15.3
B11	0.40	3.3	150	0.8	2.0%	12.5	687	19	2.8%	P	20	3.3	3.4	15.9	837.00	14.7
B12	0.40	7.1	15	0.3	2.0%	5.0	1382	26	1.9%	P	20	2.7	8.4	13.4	1397.00	17.8
B13	0.40	2.3	40	0.8	2.0%	6.4	817	17	2.1%	P	20	2.9	4.7	11.2	857.00	14.8
B14	0.51	4.9	15	0.3	2.0%	5.0	1233	21	1.7%	P	20	2.6	7.9	12.9	1248.00	16.9
B15	0.58	1.4	35	0.7	2.0%	5.0	505	13	2.6%	P	20	3.2	2.6	7.6	540.00	13.0
B16	0.66	3.2	13	0.3	2.0%	5.0	1226	22	1.8%	P	20	2.7	7.6	12.6	1238.50	16.9
B17	0.90	1.7	13	0.3	2.0%	5.0	1230	22	1.8%	P	20	2.7	7.7	12.7	1242.50	16.9
B18	0.28	6.1	170	1.0	8.2%	9.7	594	15	2.5%	B	10	1.6	6.2	15.9	764.00	14.2
B19	0.40	4.1	145	0.8	2.0%	12.3	556	3	0.5%	P	20	1.5	6.3	18.6	701.00	13.9
B20	0.40	3.3	30	0.6	2.0%	5.6	999	8	0.8%	P	20	1.8	9.3	14.9	1029.00	15.7
B21	0.40	2.0	150	0.8	2.0%	12.5	1110	13	1.2%	P	20	2.2	8.5	21.0	1260.00	17.0
B22	0.34	11.6	195	0.8	4.6%	11.7	1043	18	1.7%	G	15	2.0	8.8	20.5	1238.00	16.9
B23	0.30	9.8	300	1.0	5.0%	14.8	884	10	1.1%	G	15	1.6	9.2	24.0	1184.00	16.6
B24	0.30	9.1	300	1.0	5.3%	14.6	1700	26	1.5%	G	15	1.9	15.3	29.9	2000.00	21.1
B25	0.32	1.5	100	0.8	2.0%	11.3	125	1	1.0%	B	10	1.0	2.1	13.4	225.00	11.3
H01	0.40	1.0	150	0.8	2.0%	12.5	320	13	4.1%	P	20	4.0	1.3	13.8	470.00	12.6
H02	0.45	3.5	140	0.8	2.0%	11.2	476	15	3.2%	P	20	3.6	2.2	13.4	616.00	13.4
H03	0.54	3.0	140	0.8	2.0%	9.6	810	12	1.5%	P	20	2.4	5.5	15.2	950.00	15.3
H04	0.46	2.4	15	0.3	2.0%	5.0	707	18	2.5%	P	20	3.2	3.7	8.7	722.00	14.0
H05	0.40	2.0	15	0.3	2.0%	5.0	606	9	1.5%	P	20	2.4	4.1	9.1	621.00	13.5
H06	0.40	2.5	15	0.3	2.0%	5.0	800	23	2.9%	P	20	3.4	3.9	8.9	815.00	14.5
H07	0.46	3.1	25	0.5	2.0%	5.0	764	22	2.9%	P	20	3.4	3.8	8.8	789.00	14.4
H08	0.29	4.4	300	1.0	4.7%	15.3	600	15	2.5%	B	10	1.6	6.3	21.6	900.00	15.0
H09	0.30	2.8	100	0.8	2.0%	11.6	455	8	1.8%	B	10	1.3	5.7	17.3	555.00	13.1
H10	0.46	5.0	140	0.8	2.0%	11.0	752	16	2.1%	P	20	2.9	4.3	15.3	892.00	15.0
H11	0.48	2.0	40	0.8	2.0%	5.7	810	14	1.7%	P	20	2.6	5.1	10.8	850.00	14.7
H12	0.40	4.9	300	0.8	2.0%	17.6	561	18	3.2%	P	20	3.6	2.6	20.3	861.00	14.8
H13	0.40	1.3	40	0.8	2.0%	6.4	703	13	1.8%	P	20	2.7	4.3	10.8	743.00	14.1
H14	0.40	1.5	30	0.6	2.0%	5.6	503	13	2.6%	P	20	3.2	2.6	8.2	533.00	13.0
H15	0.40	1.9	15	0.3	2.0%	5.0	467	11	2.4%	P	20	3.1	2.5	7.5	482.00	12.7
H16	0.50	4.1	150	0.8	2.0%	10.7	671	12	1.8%	P	20	2.7	4.2	14.9	821.00	14.6
H17	0.48	3.4	135	0.7	2.0%	10.5	562	13	2.3%	P	20	3.0	3.1	13.6	697.00	13.9
H18	0.40	6.0	241	0.8	2.0%	15.8	605	11	1.8%	P	20	2.7	3.7	19.6	846.00	14.7

Revise to 100 ft maximum length for urban land uses. (COSDCM Ch. 6 Section 3.2.1)

Revise the minor storm to be based on 5yr.

STORM DRAINAGE SYSTEM DESIGN (RATIONAL METHOD PROCEDURE) SURFACE ROUTING

PROJECT: **Windingwalk Filing 1**

Date: 12/11/2017

DESIGN POINT	DIRECT RUNOFF											TOTAL RUNOFF						OVERLAND TRAVEL					
	BASIN	AREA (AC)	Tc (Min.)	I (in./hr.)		COEFF. ©		CA		Q		Sum Tc (min.)	I (in./hr.)		CA		Q		DESTINATION DP	CONVEYANCE TYPE	COEFFICIENT Cv	SLOPE %	
				(10 YR)	(100 YR)	(10 YR)	(100 YR)	(10 YR)	(100 YR)	(10 YR)	(100 YR)		(10 YR)	(100 YR)	(10 YR)	(100 YR)							
I01	B01	1.9	15.3	3.49	5.85	0.40	0.55	0.75	1.03	2.6	6.0						2.6	6.0					
I02	B02	1.9	14.1	3.61	6.06	0.40	0.55	0.76	1.04	2.7	6.3						2.7	6.3	I04	P	20.0	1.30%	
I03	B03	4.4	10.3	4.08	6.85	0.40	0.55	1.76	2.40	7.2	16						7.2	16	I04	P	20.0	1.80%	
I04	B04	7.4	22.0	2.95	4.95	0.40	0.55	2.96	4.03	8.7	20	22.0	2.95	4.95	3.13	4.59	9.2	23					
I05	B05	2.5	17.1	3.32	5.58	0.40	0.55	1.00	1.36	3.3	7.6						3.3	7.6					
CB01	B06	5.8	14.4	3.58	6.02	0.32	0.48	1.85	2.76	6.6	17						6.6	17					
I06	B07	3.3	14.9	3.53	5.92	0.40	0.55	1.34	1.82	4.7	11						4.7	11	I08	P	20.0	2.24%	
I07	B08	3.2	10.5	4.05	6.80	0.40	0.55	1.27	1.73	5.2	12						5.2	12					
I08	B09	2.4	10.6	4.04	6.78	0.40	0.55	0.97	1.32	3.9	9.0						3.9	9.0					
I09	B10	4.1	15.3	3.50	5.87	0.40	0.55	1.63	2.22	5.7	13						5.7	13					
I10	B11	3.3	14.7	3.56	5.97	0.40	0.55	1.30	1.77	4.6	11						4.6	11					
I11	B12	7.1	13.4	3.69	6.20	0.40	0.55	2.83	3.85	10	24						10	24	I12	P	20.0	2.00%	
I12	B13	2.3	11.2	3.96	6.66	0.40	0.55	0.91	1.24	3.6	8.3	13.5	3.68	6.18	0.91	1.97	3.6	12					
I13	B14	4.9	12.9	3.75	6.30	0.51	0.64	2.52	3.14	9.5	20	13.2	3.71	6.22	3.14	4.03	12	25	I14	P	20.0	2.00%	
I14	B15	1.4	7.6	4.54	7.62	0.58	0.69	0.83	0.99	3.8	7.6	13.4	3.69	6.20	0.83	1.98	3.8	12	EI37a	P	20.0	2.10%	
EI15	B16	3.2	12.6	3.78	6.34	0.66	0.76	2.15	2.47	8.1	16						8.1	16	I13	P	20.0	2.20%	
EI16	B17	1.7	12.7	3.77	6.34	0.90	0.96	1.55	1.65	5.8	10						5.8	10	EI17	P	20.0	2.00%	
EI17	E15	1.7	13.0	3.74	6.27	0.65	0.75	1.11	1.29	4.2	8.1	15.8	3.44	5.78	1.49	1.81	5.1	10					
EI18	E16	1.5	12.5	3.80	6.37	0.71	0.81	1.08	1.21	4.1	7.7						4.1	7.7					
DP05	H01	1.0	12.6	3.78	6.35	0.40	0.55	0.40	0.54	1.5	3.4						1.5	3.4	I20	P	20.0	1.20%	
I20	H02	3.5	13.4	3.69	6.20	0.45	0.59	1.57	2.04	5.8	13	13.7	3.66	6.14	1.97	2.58	7.2	16	I23	P	20.0	3.10%	
DP06	H03	3.0	15.2	3.50	5.88	0.54	0.66	1.60	1.96	5.6	12						5.6	12	I21	P	20.0	2.75%	
I21	H04	2.4	8.7	4.34	7.29	0.46	0.60	1.10	1.42	4.8	10	18.3	3.22	5.41	2.70	3.38	8.7	18					
I22	H05	2.0	9.1	4.26	7.16	0.40	0.55	0.81	1.11	3.5	7.9						3.5	7.9					
DP07	H06	2.5	8.9	4.30	7.22	0.40	0.55	1.01	1.37	4.3	9.9						4.3	9.9	I23	P	20.0	1.00%	
I23	H07	3.1	8.8	4.33	7.27	0.46	0.60	1.45	1.87	6.3	14	16.8	3.36	5.63	2.46	3.40	8.2	19	I25	P	20.0	3.30%	
CB03	H08	4.4	15.0	3.52	5.91	0.29	0.46	1.30	2.01	4.6	12						4.6	12					
CB04	H09	2.8	13.1	3.73	6.26	0.30	0.46	0.84	1.28	3.1	8.0						3.1	8.0					
I24	H10	5.0	15.0	3.53	5.92	0.46	0.59	2.29	2.96	8.1	18						8.1	18	I29	P	20.0	2.50%	
I25	H11	2.0	10.8	4.01	6.73	0.48	0.61	0.95	1.21	3.8	8.1	19.1	3.16	5.30	0.95	1.95	3.8	10					
I26	H12	4.9	14.8	3.54	5.95	0.40	0.55	1.95	2.66	6.9	16						6.9	16	I27	P	20.0	2.00%	
I27	H13	1.3	10.8	4.02	6.75	0.40	0.55	0.51	0.70	2.1	4.7	14.9	3.53	5.93	0.52	1.47	2.1	8.7					
DP08	H14	1.5	8.2	4.43	7.44	0.40	0.55	0.61	0.83	2.7	6.2						2.7	6.2	I28	P	20.0	1.20%	
I28	H15	1.9	7.5	4.55	7.65	0.40	0.55	0.75	1.03	3.4	7.9	9.5	4.21	7.07	1.37	1.86	5.8	13					
I29	H16	4.1	14.6	3.57	5.99	0.50	0.63	2.07	2.60	7.4	16	17.7	3.27	5.49	2.07	3.72	7.4	20	I35	P	20.0	1.20%	
I30	H17	3.4	13.6	3.67	6.16	0.48	0.61	1.65	2.10	6.0	13						6.0	13	I31	P	20.0	2.30%	
I31	H18	6.0	14.7	3.55	5.96	0.40	0.55	2.40	3.27	8.5	19	15.2	3.50	5.87	2.40	3.59	8.5	21	I33	p	20.0	1.00%	
I32	H19	3.8	15.2	3.51	5.88	0.40	0.55	1.53	2.09	5.4	12						5.4	12	I34	P	20.0	1.00%	
I33	H20	4.6	12.3	3.82	6.42	0.40	0.55	1.84	2.50	7.0	16	18.6	3.19	5.36	1.84	2.87	7.0	16	I34	P	20.0	2.00%	
I34	H21	4.0	20.1	3.09	5.18	0.40	0.55	1.60	2.19	4.9	11	20.1	3.09	5.18	1.64	3.32	5.1	17					

DESIGN POINT	DIRECT RUNOFF											TOTAL RUNOFF						OVERLAND TRAVEL					
	BASIN	AREA (AC)	Tc (Min.)	I (in./hr.)		COEFF. ©		CA		Q		Sum Tc (min.)	I (in./hr.)		CA		Q		DESTINATION DP	CONVEYANCE TYPE	COEFFICIENT Cv	SLOPE %	
				(10 YR)	(100 YR)	(10 YR)	(100 YR)	(10 YR)	(100 YR)	(10 YR)	(100 YR)		(10 YR)	(100 YR)	(10 YR)	(100 YR)							
CB02	B18	6.1	14.2	3.60	6.04	0.28	0.44	1.72	2.72	6.2	16							6.2	16				
I17	B19	4.1	13.9	3.64	6.10	0.40	0.55	1.63	2.22	5.9	14							5.9	14	I18	P	20.0	0.94%
I18	B20	3.3	14.9	3.53	5.93	0.40	0.55	1.30	1.78	4.6	11	19.4	3.14	5.27	1.30	2.20		4.6	12	I19	P	20.0	2.00%
I19	B21	2.0	17.0	3.33	5.60	0.40	0.55	0.80	1.09	2.7	6.1	19.5	3.13	5.25	0.80	1.16		2.7	6.1				
DP01	B22	11.6	16.9	3.34	5.61	0.34	0.49	3.94	5.71	13	32							13	32	DP02	G	15.0	1.10%
DP02	B23	9.8	16.6	3.37	5.66	0.30	0.46	2.99	4.56	10	26	26.6	2.66	4.46	6.93	10.28		18	46	DP03	G	15.0	1.40%
DP03	B24	9.1	21.1	3.01	5.05	0.30	0.46	2.68	4.14	8.1	21	41.2	2.01	3.37	9.61	14.42		19	49				
DP04	B25	1.5	11.3	3.95	6.64	0.32	0.48	0.50	0.74	2.0	4.9							2.0	4.9				
I35	H22	3.0	13.8	3.65	6.13	0.51	0.64	1.55	1.93	5.7	12	22.3	2.93	4.91	1.55	2.20		5.7	12				
I36	H23	1.0	8.7	4.34	7.28	0.67	0.77	0.69	0.79	3.0	5.7							3.0	5.7				
OS4	H24	3.3	13.6	3.02	6.16	0.34	0.49	1.11	1.62	3.4	10							3.4	10	POND	G	15.0	1.20%
POND	H25	11.3	13.9	2.98	6.11	0.29	0.45	3.30	5.13	10	31	17.9	3.26	5.46	4.42	6.76		14	37				

TYPE OF SURFACE		Cv
HEAVY MEADOW	H	3
TILLAGE/FIELD	T	5
RIPRAP (not buried)	R	7
SHORT PASTURE AND LAWNS	L	7
NEARLY BARE GROUND	B	10
GRASSED WATERWAY	G	15
PAVED AREAS	P	20

**STORM DRAINAGE SYSTEM DESIGN
INLET CALCULATIONS**

PROJECT: **Windingwalk Filing 1**

Date: 12/11/2017

DP	Inlet size L(i)	Proposed or Existing	INLET TYPE	CROSS SLOPE	STREET SLOPE	T _c	Q _{Total}		Q _{Capture}				Q _{Flow-by}				DEPTH (max)		SPREAD	
							Q ₁₀ (cfs)	Q ₁₀₀ (cfs)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)	CA _{eqv.} (10-yr)	CA _{eqv.} (100-yr)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)	CA _{eqv.} (10-yr)	CA _{eqv.} (100-yr)	Q ₁₀ (cfs)	Q ₁₀₀ (ft)	Q ₁₀ (cfs)	Q ₁₀₀ (ft)
I01	5	PROP	SUMP	2.0%		15.3	2.6	6.0	2.6	6.0	0.75	1.03	-	-	-	-	0.50	0.50		
I02	10	PROP	FLOW-BY	2.0%	0.5%	14.1	2.7	6.3	2.1	4.2	0.59	0.70	0.6	2.1	0.17	0.34	0.32	0.41	12.0	16.3
I03	15	PROP	SUMP ¹	2.0%		10.3	7.2	16	7.2	15	1.76	2.18	-	1.5	-	0.22	0.40	0.50		
I04	15	PROP	SUMP	2.0%		22.0	9.2	23	9.2	23	3.13	4.59	-	-	-	-	0.50	0.70		
I05	10	PROP	SUMP	2.0%		17.1	3.3	7.6	3.3	7.6	1.00	1.36	-	-	-	-	0.50	0.70		
CB01	Type C	PROP	SUMP	2.0%		14.4	6.6	17	6.6	17	1.85	2.76	-	-	-	-	0.46	0.68		
I06	10	PROP	SUMP	2.0%		14.9	4.7	11	4.7	11	1.34	1.82	-	-	-	-	0.50	0.50		
I07	15	PROP	SUMP ¹	2.0%		10.5	5.2	12	5.2	12	1.27	1.73	-	-	-	-	0.50	0.50		
I08	15	PROP	SUMP ¹	2.0%		10.6	3.9	9.0	3.9	9.0	0.97	1.32	-	-	-	-	0.50	0.50		
I09	15	PROP	SUMP ¹	2.0%		15.3	5.7	13	5.7	13	1.63	2.22	-	-	-	-	0.50	0.50		
I10	10	PROP	SUMP ¹	2.0%		14.7	4.6	11	4.6	11	1.30	1.77	-	-	-	-	0.50	0.50		
I11	15	PROP	SUMP	2.0%		13.4	10	24	10	19	2.83	3.12	-	4.5	-	0.73	0.50	0.60		
I12	10	PROP	SUMP	2.0%		13.5	3.6	12	3.6	12	0.98	1.97	-	-	-	-	0.50	0.60		
I13	20	PROP	SUMP	2.0%		13.2	12	25	12	19	3.14	3.04	-	6.2	-	0.99	0.50	0.50		
I14	20	PROP	FLOW-BY	2.0%	0.5%	13.4	3.8	12	3.6	9.7	0.99	1.57	0.1	2.6	0.03	0.41	0.35	0.51	13.5	21.0
EI15	20	PROP	FLOW-BY	2.0%	2.2%	12.6	8.1	16	5.8	10	1.54	1.58	2.3	5.6	0.61	0.89	0.36	0.43	13.6	17.4
EI16	20	PROP	FLOW-BY	2.0%	2.2%	12.7	5.8	10	4.4	7.2	1.17	1.13	1.4	3.3	0.38	0.52	0.33	0.38	12.0	15.0
EI17	5	PROP	SUMP	2.0%		15.8	5.1	10	5.1	10	1.49	1.81	-	-	-	-	0.50	1.00		
EI18	12	PROP	SUMP	2.0%		12.5	4.1	7.7	4.1	7.7	1.08	1.21	-	-	-	-	0.50	1.00		
CB02	Type C	PROP	SUMP	2.0%		14.2	6.2	16	6.2	16	1.72	2.72	-	-	-	-	0.45	0.68		
I17	10	PROP	SUMP	2.0%		13.9	5.9	14	5.9	11	1.63	1.79	-	2.6	-	0.43	0.50	0.50		
I18	5	PROP	SUMP	2.0%		19.4	4.6	12	4.6	11	1.47	2.13	-	0.4	-	0.07	0.50	0.70		
I19	10	PROP	SUMP	2.0%		19.5	2.7	6.1	2.7	6.1	0.85	1.16	-	-	-	-	0.50	1.00		

DP	Inlet size L(i)	Proposed or Existing	INLET TYPE	CROSS SLOPE	STREET SLOPE	T _c	Q _{Total}		Q _{Capture}				Q _{Flow-by}				DEPTH (max)		SPREAD	
							Q ₁₀ (cfs)	Q ₁₀₀ (cfs)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)	CA _{eqv.} (10-yr)	CA _{eqv.} (100-yr)	Q ₁₀ (cfs)	Q ₁₀₀ (cfs)	CA _{eqv.} (10-yr)	CA _{eqv.} (100-yr)	Q ₁₀ (cfs)	Q ₁₀₀ (ft)	Q ₁₀ (cfs)	Q ₁₀₀ (ft)
I20	15	PROP	SUMP ¹	2.0%		13.7	7.2	16	7.2	15	1.97	2.43	-	0.9	-	0.15	0.50	0.50		
I21	20	PROP	SUMP ¹	2.0%		18.3	8.7	18	8.7	18	2.70	3.38	-	-	-	-	0.50	0.50		
I22	10	PROP	SUMP	2.0%		9.1	3.5	7.9	3.5	7.9	0.81	1.11	-	-	-	-	0.50	1.00		
I23	15	PROP	SUMP ¹	2.0%		16.8	8.2	19	8.2	15	2.46	2.65	-	4.2	-	0.75	0.50	0.50		
CB03	Type C	PROP	SUMP	2.0%		15.0	4.6	12	4.6	12	1.30	2.01	-	-	-	-	0.40	0.59		
CB04	Type C	PROP	SUMP	2.0%		13.1	3.1	8.0	3.1	8.0	0.84	1.28	-	-	-	-	0.32	0.50		
I24	10	PROP	SUMP ¹	2.0%		15.0	8.1	18	8.1	11	2.29	1.85	-	6.6	-	1.12	0.50	0.50		
I25	10	PROP	SUMP ¹	2.0%		19.1	3.8	10	3.8	10	1.21	1.95	-	-	-	-	0.50	0.50		
I26	5	PROP	SUMP	2.0%		14.8	6.9	16	6.9	11	1.95	1.89	0.0	4.6	0.00	0.77	0.50	0.70		
I27	10	PROP	SUMP	2.0%		14.9	2.1	8.7	2.1	8.7	0.58	1.47	-	-	-	-	0.50	0.70		
I28	15	PROP	SUMP ¹	2.0%		9.5	5.8	13	5.8	13	1.37	1.86	-	-	-	-	0.50	0.50		
I29	20	PROP	SUMP ¹	2.0%		17.7	7.4	20	7.4	19	2.26	3.45	-	1.5	-	0.27	0.50	0.50		
I30	10	PROP	SUMP ¹	2.0%		13.6	6.0	13	6.0	11	1.65	1.77	-	2.0	-	0.33	0.50	0.50		
I31	20	PROP	SUMP ¹	2.0%		15.2	8.5	21	8.5	19	2.43	3.23	-	2.1	-	0.37	0.50	0.50		
I32	10	PROP	SUMP ¹	2.0%		15.2	5.4	12	5.4	11	1.53	1.86	-	1.3	-	0.23	0.50	0.50		
I33	5	PROP	SUMP	2.0%		18.6	7.0	16	6.9	11	2.16	2.09	0.1	4.8	0.04	0.90	0.50	0.70		
I34	10	PROP	SUMP	2.0%		20.1	5.1	17	5.1	17	1.64	3.32	-	-	-	-	0.50	0.70		

¹ Forced sump at intersection

**STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)
PIPE ROUTING**

PROJECT: **Windingwalk Filing 1**

Date: 12/11/2017

UPSTREAM DESIGN POINT	UPSTREAM BASIN	INLET FLOW						SYSTEM FLOW						TRAVEL TIME								
		Tc (Min.)	I (in./hr.)		CA		Q		Sum Tc (min.)	I (in./hr.)		CA		Q		PIPE DIA	ROUGHNESS (n)	DESTINATION DP	SLOPE %	LENGTH (FT)	VEL. (FPS) (Estimate)*	TRAVEL TIME Tt
			(10 YR)	(100 YR)	(10 YR)	(100 YR)	(10 YR)	(100 YR)		(10 YR)	(100 YR)	(10 YR)	(100 YR)	(10 YR)	(100 YR)							
I01	B01	15.3	3.49	5.85	0.75	1.03	2.6	6.0						2.6	6.0	18	0.013	J01	0.90%	33	6	0.1
I02	B02	14.1	3.61	6.06	0.59	0.70	2.1	4.2						2.1	4.2	18	0.013	J01	1.40%	24	7	0.1
J01									15.4	2.79	5.84	1.34	1.72	3.7	10	18	0.013	J02	3.34%	297	11	0.5
J02									15.9	2.74	5.77	1.34	1.72	3.7	10	18	0.013	J02	0.81%	285	5	0.9
J03									16.8	2.65	5.63	1.34	1.72	3.6	10	24	0.013	J05	0.55%	487	5	1.5
I03	B03	10.3	4.08	6.85	1.76	2.18	7.2	15						7.2	15	18	0.013	J04	1.04%	48	6	0.1
J04									10.5	3.47	6.82	1.76	2.18	6.1	15	24	0.013	J05	1.85%	127	10	0.2
I04	B04	22.0	2.95	4.95	3.13	4.59	9.2	23						9.2	23	18	0.013	J05	1.93%	5	8	0.0
J05									22.0	2.18	4.95	6.24	8.50	14	42	24	0.013	I05	7.55%	25	20	0.0
I05	B05	17.1	3.32	5.58	1.00	1.36	3.3	7.6						16	49	24	0.013	J06	5.71%	147	17	0.1
J06									22.1	2.16	4.93	7.24	9.86	16	49	30	0.013	CB01	1.08%	102	9	0.2
CB01	B06	14.4	3.58	6.02	1.85	2.76	6.6	17						20	62	36	0.013	I06	1.62%	160	12	0.2
I06	B07	14.9	3.53	5.92	1.34	1.82	4.7	11						22	71	36	0.013	J07	3.12%	296	17	0.3
I07	B08	10.5	4.05	6.80	1.27	1.73	5.2	12						5.2	12	18	0.013	J07	1.00%	45	6	0.1
J07									22.8	2.11	4.85	11.69	16.18	25	79	36	0.013	J08	1.19%	332	10	0.5
I08	B09	10.6	4.04	6.78	0.97	1.32	3.9	9.0						3.9	9.0	18	0.013	J08	1.00%	8	6	0.0
J08									23.4	2.07	4.79	12.66	17.50	26	84	42	0.013	J09	1.06%	57	11	0.1
J09									23.5	2.06	4.78	12.66	17.50	26	84	42	0.013	J10	2.81%	306	18	0.3
J10									23.7	2.04	4.75	12.66	17.50	26	83	42	0.013	J15	1.85%	205	14	0.2
I09	B10	15.3	3.50	5.87	1.63	2.22	5.7	13						5.7	13	18	0.013	J11	1.04%	48	6	0.1
J11									15.4	2.80	5.85	1.63	2.22	4.6	13	18	0.013	J13	1.62%	334	8	0.7
I10	B11	14.7	3.56	5.97	1.30	1.77	4.6	11						4.6	11	18	0.013	J12	1.04%	58	6	0.2
J12									14.8	2.87	5.94	1.30	1.77	3.7	11	18	0.013	J13	1.06%	76	6	0.2
I11	B12	13.4	3.69	6.20	2.83	3.12	10	19						10	19	18	0.013	J13	1.93%	5	8	0.0
J13									16.1	2.72	5.73	5.76	7.11	16	41	24	0.013	I12	2.78%	25	12	0.0
I12	B13	13.5	3.68	6.18	0.98	1.97	3.6	12						18	52	30	0.013	J14	4.17%	166	17	0.2
J14									16.3	2.70	5.70	6.74	9.08	18	52	36	0.013	J15	0.88%	113	9	0.2
J15									24.0	2.02	4.73	19.40	26.59	39	126	48	0.013	J16	0.99%	85	11	0.1
I13	B14	13.2	3.71	6.22	3.14	3.04	12	19						12	19	24	0.013	J16	4.86%	13	16	0.0
I14	B15	13.4	3.69	6.20	0.99	1.57	3.6	9.7						3.6	9.7	18	0.013	J16	1.93%	33	8	0.1
J16									24.0	2.02	4.73	23.52	31.20	48	148	48	0.013	EJ16	1.00%	153	11	0.2
E115	B16	12.6	3.78	6.34	1.54	1.58	5.8	10						5.8	10	24	0.013	E16	1.10%	108	8	0.2
E116	B17	12.7	3.77	6.34	1.17	1.13	4.4	7.2						8	17	30	0.013	EJ16	1.60%	100	11	0.2
EJ16									24.2	2.01	4.70	26.23	33.91	53	160	48	0.013	EJ17	1.30%	343	13	0.4
EJ17									24.6	1.97	4.66	26.23	33.91	52	158	48	0.013	EJ18	0.50%	120	8	0.2
E117	E15	15.8	3.44	5.78	1.49	1.81	5.1	10						5.1	10	24	0.013	EJ18	1.33%	13	8	0.0
E118	E16	12.5	3.80	6.37	1.08	1.21	4.1	7.7						4.1	7.7	24	0.013	EJ18	0.50%	33	5	0.1
EJ18									24.9	1.96	4.63	28.80	36.94	56	171	54	0.013	EJ19	0.50%	342	9	0.7
EJ19									25.5	1.91	4.57	28.80	36.94	55	169	54	0.013	EJ19	2.10%	96	18	0.1

UPSTREAM DESIGN POINT	UPSTREAM BASIN	INLET FLOW							SYSTEM FLOW							TRAVEL TIME						
		Tc (Min.)	I (in./hr.)		CA		Q		Sum Tc (min.)	I (in./hr.)		CA		Q		PIPE DIA	ROUGHNESS (n)	DESTINATION DP	SLOPE %	LENGTH (FT)	VEL. (FPS) (Estimate) ^y	TRAVEL TIME Tt
			(10 YR)	(100 YR)	(10 YR)	(100 YR)	(10 YR)	(100 YR)		(10 YR)	(100 YR)	(10 YR)	(100 YR)	(10 YR)	(100 YR)							
CB02	B18	14.2	3.60	6.04	1.72	2.72	6.2	16						6.2	16	24	0.013	J17	1.00%	180	7	0.4
J17									14.7	2.88	5.97	1.72	2.72	5.0	16	24	0.013	J18	0.99%	116	7	0.3
I17	B19	13.9	3.64	6.10	1.63	1.79	5.9	11						5.9	11	18	0.013	J18	1.93%	5	8	0.0
J18									14.9	2.85	5.92	3.35	4.51	10	27	30	0.013	J19	1.05%	38	9	0.1
J19									15.0	2.84	5.91	3.35	4.51	10	27	30	0.013	J20	1.03%	107	9	0.2
J20									15.2	2.82	5.88	3.35	4.51	9	27	30	0.013	J21	0.71%	489	7	1.2
I18	B20	19.4	3.14	5.27	1.47	2.13	4.6	11						4.6	11	18	0.013	J21	9.64%	5	19	0.0
J21									19.4	2.40	5.26	4.82	6.64	12	35	30	0.013	I19	1.22%	25	9	0.0
I19	B21	19.5	3.13	5.25	0.85	1.16	2.7	6.1	19.5	2.39	5.25	5.67	7.81	14	41	30	0.013	OS2	2.20%	159	12	0.2
I20	H02	13.7	3.66	6.14	1.97	2.43	7.2	15						7.2	15	18	0.013	J22	1.00%	65	6.0	0.2
J22									13.9	2.98	6.11	1.97	2.43	5.9	15	18	0.013	J23	3.69%	330	11.4	0.5
J23									14.3	2.92	6.02	1.97	2.43	5.8	15	18	0.013	J24	3.82%	234	11.6	0.3
I23	H07	16.8	3.36	5.63	2.46	2.65	8.2	15						8.2	15	24	0.013	J24	1.08%	32	7.5	0.1
J24									16.8	2.64	5.62	4.43	5.08	12	29	24	0.013	J26	2.91%	105	12.3	0.1
I21	H04	18.3	3.22	5.41	2.70	3.38	8.7	18						8.7	18	24	0.013	J25A	1.03%	183	7.3	0.4
J25A									18.7	2.46	5.36	2.70	3.38	6.6	18	24	0.013	I22	3.32%	344	13.2	0.4
I22	H05	9.1	4.26	7.16	0.81	1.11	3.5	7.9	19.1	2.42	5.30	3.52	4.49	9	24	24	0.013	J25B	4.97%	178	16	0.2
J25B									19.3	2.40	5.28	3.52	4.49	8	24	24	0.013	J26	1.00%	100	7	0.2
J26									19.5	2.38	5.25	7.94	9.57	19	50	30	0.013	J27	4.13%	216	17	0.2
CB03	H08	15.0	3.52	5.91	1.30	2.01	4.6	12						4.6	12	18	0.013	J27	0.99%	71	6	0.2
CB04	H09	13.1	3.73	6.26	0.84	1.28	3.1	8.0						3.1	8.0	18	0.013	J27	1.38%	51	7	0.1
J27									19.7	2.36	5.22	10.08	12.86	24	67	42	0.013	J28	3.10%	158	18	0.1
I24	H10	15.0	3.53	5.92	2.29	1.85	8.1	11						8.1	11	18	0.013	J28	1.04%	53	6	0.1
I25	H11	19.1	3.16	5.30	1.21	1.95	3.8	10						3.8	10	18	0.013	J28	1.68%	33	8	0.1
J28									19.9	2.35	5.20	13.58	16.66	32	87	42	0.013	J29	3.43%	264	19	0.2
I26	H12	14.8	3.54	5.95	1.95	1.89	6.9	11						6.9	11	18	0.013	I27	4.25%	35	12	0.0
I27	H13	14.9	3.53	5.93	0.58	1.47	2.1	8.7	14.9	2.86	5.93	2.53	3.36	7.2	20	24	0.013	J29	3.07%	192	13	0.3
J29									20.1	2.33	5.17	13.58	20.02	32	104	42	0.013	J30	1.79%	90	14	0.1
I28	H15	9.5	4.21	7.07	1.37	1.86	5.8	13						5.8	13	18	0.013	J30	1.38%	33	7	0.1
J30									20.2	2.32	5.16	14.94	21.88	35	113	42	0.013	J31	1.95%	169	15	0.2
I29	H16	17.7	3.27	5.49	2.26	3.45	7.4	19						7.4	19	24	0.013	J31	5.91%	28	18	0.0
J31									20.4	2.30	5.13	17.20	25.33	40	130	48	0.013	J32	1.43%	249	14	0.3
I30	H17	13.6	3.67	6.16	1.65	1.77	6.0	11						6.0	11	18	0.013	J32	2.45%	45	9	0.1
J32									20.7	2.28	5.10	18.85	27.10	43	138	48	0.013	J33	3.06%	303	20	0.3
I31	H18	15.2	3.50	5.87	2.43	3.23	8.5	19						8.5	19	24	0.013	J33	6.82%	25	19	0.0
J33									21.0	2.26	5.07	21.28	30.33	48	154	48	0.013	J34	1.04%	48	12	0.1
J34									21.0	2.25	5.06	21.28	30.33	48	153	48	0.013	J35	2.55%	387	18	0.4
I32	H19	15.2	3.51	5.88	1.53	1.86	5.4	11						5.4	11	24	0.013	J35	1.62%	312	9	0.6
I33	H20	18.6	3.19	5.36	2.16	2.09	6.9	11						6.9	11	18	0.013	J35	9.90%	24	19	0.0
J32									21.4	2.22	5.02	24.97	34.28	56	172	48	0.013	I34	3.05%	5	20	0.0
I34	H21	20.1	3.09	5.18	1.64	3.32	5.1	17	21.4	2.22	5.02	26.61	37.60	59	189	48	0.013	OS3	1.01%	248	12	0.4

* Velocity estimated for calculation of travel time. Refer to Hydraulics for calculated velocity.

**STORM DRAINAGE SYSTEM DESIGN
HYDRAULICS**

PROJECT: **Windingwalk Filing 1**

Date: 12/11/2017

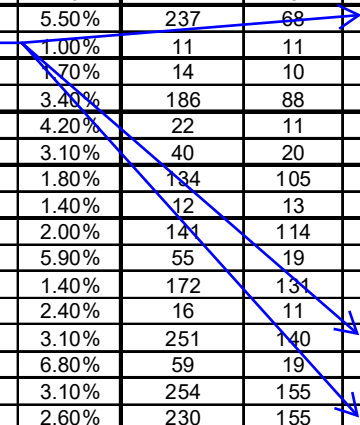
Label	Upstrm Node	Dnstrm Node	Inlet CA (acres)	Inlet Tc (min)	Inlet Flow (ft ³ /s)	System CA (acres)	System Flow Time (min)	System Intensity (in/hr)	Length (ft)	Section Size (in)	Slope (%)	Capacity (Full Flow) (ft ³ /s)	System Flow (ft ³ /s)	Velocity (Ave) (ft/s)	Elevation Ground (Upstrm) (ft)	Hydraulic Grade Line (Upstrm) (ft)	Invert (Upstrm) (ft)	Elevation Ground (Dnstrm) (ft)	Hydraulic Grade Line (Dnstrm) (ft)	Invert (Dnstrm) (ft)
P01	I01	J01	1.03	15.3	6	1.03	15.3	5.86	34	18	0.90%	10	6	5.9	7061.73	7058.3	7057.20	7061.16	7058.3	7056.90
P02	I02	J01	0.70	14.1	4	0.70	14.1	6.07	24	18	1.40%	13	4	6	7061.76	7058.3	7057.25	7061.16	7058.3	7056.90
P03	J01	J02				1.73	15.4	5.85	297	18	3.40%	19	10	11.1	7061.16	7058.1	7056.90	7051.06	7048.3	7046.90
P04	J02	J03				1.73	15.8	5.77	285	18	0.80%	9	10	6	7051.06	7048.3	7046.90	7048.74	7045.8	7044.60
P05	J03	J05				1.73	16.7	5.64	487	24	0.60%	17	10	6	7048.74	7045.2	7044.10	7046.29	7043.8	7041.40
P06	I03	J04	2.18	10.3	15	2.18	10.3	6.86	48.1	18	1.00%	11	15	8.5	7049.27	7046.7	7044.75	7048.77	7045.7	7044.25
P07	J04	J05				2.18	10.4	6.84	127	24	1.90%	31	15	10	7048.77	7045.2	7043.75	7046.29	7044.0	7041.40
P08	I04	J05	4.59	22.0	23	4.59	22.0	4.95	5	18	1.90%	15	23	13	7046.52	7044.3	7042.00	7046.29	7044.0	7041.90
P08A	J05	I05				8.50	22.0	4.95	25	24	7.50%	62	42	21.3	7046.29	7043.4	7041.40	7046.52	7042.5	7039.50
P09	I05	J06	1.36	17.1	8	9.86	22.0	4.94	147	24	5.70%	54	49	16	7046.52	7041.7	7039.50	7038.50	7034.8	7031.10
P10	J06	CB01				9.86	22.2	4.93	102	30	1.10%	43	49	10	7038.50	7033.8	7030.60	7033.00	7032.3	7029.50
P11	CB01	I06	2.76	14.4	17	12.62	22.4	4.91	160	36	1.60%	85	62	13	7033.00	7031.5	7029.00	7032.48	7029.3	7026.40
P12	I06	J07	1.82	14.9	11	14.44	22.6	4.88	296	36	3.10%	118	71	17	7032.48	7029.1	7026.40	7024.03	7020.6	7017.15
P13	I07	J07	1.73	10.5	12	1.73	10.5	6.81	45	18	1.00%	10	12	7	7023.64	7021.3	7019.10	7024.03	7020.7	7018.65
P14	J07	J08				16.17	22.8	4.85	332	36	1.20%	73	79	11.2	7024.03	7020.5	7017.15	7019.13	7016.0	7013.20
P15	I08	J08	1.32	10.6	9	1.32	10.6	6.81	45	18	1.00%	15	9	5.1	7019.34	7016.6	7014.80	7019.13	7016.6	7014.70
P16	J08	J09				17.49	23.3	4.85	332	36	1.20%	103	85	12.0	7019.13	7015.6	7012.70	7018.80	7015.3	7012.10
P17	J09	J10				17.49	23.4	4.85	332	36	1.20%	169	84	17.6	7018.80	7015.0	7012.10	7019.71	7006.5	7003.50
P18	J10	J15				17.49	23.7	4.85	332	36	1.20%	137	84	15	7009.71	7006.4	7003.50	7006.80	7004.7	6999.70
P19	I09	J11	2.22	15.3	13	2.22	15.3	6.81	45	18	1.00%	11	13	7	7020.73	7019.7	7016.20	7020.34	7018.9	7015.70
P20	J11	J13				2.22	15.4	6.81	45	18	1.00%	13	13	7	7020.34	7018.1	7015.70	7015.22	7012.9	7010.30
P21	I10	J12	1.77	14.7	11	1.77	14.7	5.90	30	18	1.00%	11	11	6	7016.22	7014.5	7011.70	7015.90	7013.9	7011.10
P22	J12	J13				1.77	14.9	5.93	76	18	1.10%	11	11	6.0	7015.90	7013.5	7011.10	7015.22	7012.7	7010.30
P23	I11	J13	3.12	13.4	19	3.12	13.4	6.20	5	18	1.90%	15	19	11.0	7015.46	7013.1	7010.40	7015.22	7012.9	7010.30
P24	J13	I12				7.11	16.2	5.72	25	24	2.80%	38	41	13.1	7015.22	7012.2	7009.80	7015.46	7011.4	7009.10
P25	I12	J14	1.97	13.5	12	9.08	16.2	5.72	166	30	4.20%	84	52	18	7015.46	7010.9	7008.60	7009.00	7005.7	7001.70
P26	J14	J15				9.08	16.3	5.69	113	36	0.90%	63	52	7	7009.00	7005.6	7001.20	7006.80	7004.9	7000.20
P26	J14	J15				9.08	16.3	5.69	113	36	0.90%	63	52	7	7009.00	7005.6	7001.20	7006.80	7004.9	7000.20
P27	J15	J16				26.57	23.9	4.73	85	48	1.00%	143	127	10.1	7006.80	7004.6	6999.20	7005.99	7004.0	6998.36
P28	I13	J16	3.04	13.2	19	3.04	13.2	6.23	13	24	4.90%	50	19	6.1	7006.01	7004.4	7001.00	7005.99	7004.3	7000.36
P29	I14	J16	1.57	13.4	10	1.57	13.4	6.20	33	18	1.90%	15	10	5.6	7006.01	7004.6	7001.50	7005.99	7004.3	7000.86
P30	J16	EJ16				31.18	24.1	4.72	153	48	1.00%	145	148	11.8	7005.99	7003.9	6998.36	7006.30	7002.3	6996.80
P31	EI15	EI16	1.58	12.6	10	1.58	12.6	6.35	108	24	1.10%	24	10	7.2	7009.20	7005.5	7004.37	7009.12	7004.6	7003.20
P32	EI16	EJ16	1.13	12.7	7	2.71	12.9	6.30	100	30	1.60%	52	17	9.5	7009.12	7004.4	7002.70	7006.30	7004.4	7001.10
P33	EJ16	EJ17				33.89	24.3	4.70	343	48	1.30%	161	160	12.8	7006.30	7002.0	6996.50	6999.14	6997.8	6992.20
P34	EJ17	EJ18				33.89	24.7	4.65	120	48	0.50%	102	159	12.6	6999.14	6997.4	6992.00	6997.94	6996.0	6991.40
P35	EI17	EJ18	1.81	15.8	11	1.81	15.8	5.78	12	24	1.30%	26	11	3.4	6998.25	6996.2	6992.91	6997.94	6996.1	6992.75
P36	EI18	EJ18	1.21	12.5	8	1.21	12.5	6.37	32	24	0.50%	16	8	2.5	6998.25	6996.2	6992.91	6997.94	6996.1	6992.75
P37	EJ18	EJ19				36.91	24.9	4.63	342	54	0.50%	139	172	10.8	6997.94	6995.9	6990.40	6999.33	6993.3	6988.69
P38	EJ19	OS1				36.91	25.4	4.58	96	54	2.10%	287	170	18.8	6999.33	6992.2	6988.40	6999.00	6989.2	6986.36
P39	CB02	J17	2.72	14.2	17	2.72	14.2	6.05	180	24	1.00%	23	17	7.9	7035.50	7033.7	7032.25	7036.59	7032.4	7030.45
P40	J17	J18				2.72	14.6	5.98	116	24	1.00%	23	16	7.8	7036.59	7031.9	7030.45	7036.00	7030.9	7029.30
P41	I17	J18	1.79	13.9	11	1.79	13.9	6.10	5	18	1.90%	15	11	9.1	7036.22	7031.2	7029.90	7036.00	7030.9	7029.80
P42	J18	J19				4.51	14.8	5.94	38	30	1.00%	42	27	9.1	7036.00	7030.6	7028.80	7035.92	7030.5	7028.40
P43	J19	J20				4.51	14.9	5.93	107	30	1.00%	42	27	9.0	7035.92	7030.2	7028.40	7034.26	7029.1	7027.30
P44	J20	J21				4.51	15.1	5.90	489	30	0.70%	35	27	7.8	7034.26	7029.1	7027.30	7029.57	7026.6	7023.80
P45	I18	J21	2.13	19.4	11	2.13	19.4	5.26	5	18	9.60%	33	11	16.8	7029.79	7026.5	7025.25	7029.57	7025.7	7024.80
P46	J21	I19				6.64	19.4	5.26	25	18	9.60%	33	11	16.8	7029.79	7026.5	7025.25	7023.80	7025.8	7023.50
P47	I19	OS2	1.16	19.5	6	7.80	19.5	5.25	159	30	1.00%	33	11	16.8	7029.79	7026.5	7025.25	7025.00	7021.5	7020.00

Maximum storm sewer velocity is 18 fps. Update the system design accordingly.

Identify in the sub-basin/DP narrative. Provide findings/solution/recommendation since this is an existing box culvert. What is the condition downstream? Is there potential issues?

Label	Upstrm Node	Dnstrm Node	Inlet CA (acres)	Inlet Tc (min)	Inlet Flow (ft³/s)	System CA (acres)	System Flow Time (min)	System Intensity (in/hr)	Length (ft)	Section Size (in)	Slope (%)	Capacity (Full Flow) (ft³/s)	System Flow (ft³/s)	Velocity (Ave) (ft/s)	Elevation Ground (Upstrm) (ft)	Hydraulic Grade Line (Upstrm) (ft)	Invert (Upstrm) (ft)
P48	I20	J22	2.43	13.7	15	2.43	13.7	6.14	65	18	1.00%	11	15	8.5	7057.79	7055.9	7053.25
P49	J22	J23				2.43	13.8	6.12	330	18	3.70%	20	15	12.5	7057.29	7054.0	7052.60
P50	J23	J24				2.43	14.3	6.04	234	18	3.80%	21	15	12.7	7044.71	7041.9	7040.45
P51	I23	J24	2.65	16.8	15	2.65	16.8	5.63	32	24	1.10%	24	15	4.8	7036.38	7033.4	7031.35
P52	J24	J26				5.08	16.9	5.61	105	24	2.90%	39	29	13.5	7036.79	7032.8	7031.00
P53	I21	J25A	3.38	18.3	18	3.38	18.3	5.41	183	24	1.00%	23	18	8.1	7056.10	7052.7	7051.10
P54	J25A	I22				3.38	18.7	5.36	344	24	3.30%	41	18	12.7	7054.38	7050.7	7049.20
P55	I22	J25B	1.11	9.1	8	4.49	19.1	5.30	178	24	5.00%	50	24	15.9	7045.74	7039.5	7037.80
P56	J25B	J26				4.49	19.3	5.27	100	24	1.00%	23	24	7.6	7035.00	7031.4	7028.95
P57	J26	J27				9.57	19.5	5.25	216	30	4.10%	83	51	17.8	7034.44	7029.8	7027.45
P58	CB03	J27	2.01	15.0	12	2.01	15.0	5.01	71	18	1.00%	10	12	6.8	7023.00	7021.8	7020.25
P59	CB04	J27	1.28	13.1	8	1.28	13.1	5.01	71	18	1.40%	12	8	7.5	7023.00	7021.4	7020.25
P60	J27	J28				12.86	19.5	5.01	71	18	5.50%	237	68	21.2	7024.54	7020.1	7017.55
P61	I24	J28	1.85	15.0	11	1.85	15.0	5.01	71	18	1.00%	11	11	6.2	7019.71	7017.3	7015.20
P62	I25	J28	1.95	19.1	10	1.95	19.1	5.01	71	18	1.70%	14	10	5.9	7019.71	7017.0	7015.20
P63	J28	J29				16.66	19.5	5.01	71	18	3.40%	186	88	19.1	7020.13	7015.6	7012.65
P64	I26	I27	1.89	14.8	11	1.89	14.8	5.95	35	18	4.20%	22	11	12.4	7017.49	7014.3	7013.00
P65	I27	J29	1.47	14.9	9	3.36	14.9	5.93	192	24	3.10%	40	20	12.7	7017.49	7012.6	7011.00
P66	J29	J30				20.02	20.0	5.18	90	42	1.80%	134	105	15.4	7011.65	7006.7	7003.60
P67	I28	J30	1.86	9.5	13	1.86	9.5	7.06	33	18	1.40%	12	13	7.5	7008.96	7007.4	7004.45
P68	J30	J31				21.88	20.1	5.17	169	42	2.00%	141	114	16.3	7009.38	7005.2	7002.00
P69	I29	J31	3.45	17.7	19	3.45	17.7	5.49	28	24	5.90%	55	19	15.9	7006.86	7003.4	7001.85
P70	J31	J32				25.33	20.3	5.15	249	48	1.40%	172	131	15.1	7006.72	7001.6	6998.20
P71	I30	J32	1.77	13.6	11	1.77	13.6	6.16	45	18	2.40%	16	11	6.2	7002.77	7000.2	6998.25
P72	J32	J33				27.10	20.6	5.11	303	48	3.10%	251	140	20.5	7003.17	6998.2	6994.65
P73	I31	J33	3.23	15.2	19	3.23	15.2	5.88	25	24	6.80%	59	19	6.1	6994.14	6992.0	6989.10
P74	J33	J34				30.33	20.8	5.08	48	48	3.10%	254	155	12.4	6993.93	6989.9	6985.40
P75	J34	J35				30.33	20.9	5.08	387	48	2.60%	230	155	19.6	6994.04	6987.5	6983.90
P76	I32	J35	1.86	15.2	11	1.86	15.2	5.88	312	24	1.60%	29	11	3.5	6986.06	6983.7	6981.05
P77	I33	J35	2.09	18.6	11	2.09	18.6	5.37	24	18	9.90%	33	11	6.4	6984.42	6980.8	6978.90
P78	J35	I34				34.28	21.2	5.04	5	48	3.00%	251	174	13.9	6984.19	6980.1	6974.00
P79	I34	OS3	3.32	20.1	17	37.60	21.2	5.04	209	48	0.90%	135	191	15.2	6984.42	6979.6	6973.85

Maximum storm sewer velocity is 18 fps. Update the system design accordingly.



Appendix B – Street Flow Tables

Worksheet for Ramp Full Street Section

Project Description

Friction Method Manning Formula
 Solve For Discharge

Input Data

Channel Slope 0.00500 ft/ft
 Normal Depth 0.75 ft
 Section Definitions

Station (ft)	Elevation (ft)
0+00	0.00
0+13	-0.25
0+14	-0.75
0+15	-0.59
0+30	-0.29
0+45	-0.59
0+46	-0.75
0+48	-0.25
0+60	0.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 0.00)	(0+13, -0.25)	0.030
(0+13, -0.25)	(0+15, -0.59)	0.013
(0+15, -0.59)	(0+45, -0.59)	0.015
(0+45, -0.59)	(0+48, -0.25)	0.013
(0+48, -0.25)	(0+60, 0.00)	0.030
<None>	(0+60, 0.00)	0.030

Options

Current Roughness Weighted Method Pavlovskii's Method
 Open Channel Weighting Method Pavlovskii's Method
 Closed Channel Weighting Method Pavlovskii's Method

Worksheet for Ramp Full Street Section

Results

Discharge		42.54	ft ³ /s
Elevation Range	-0.75 to 0.00 ft		
Flow Area		19.32	ft ²
Wetted Perimeter		60.21	ft
Hydraulic Radius		0.32	ft
Top Width		60.00	ft
Normal Depth		0.75	ft
Critical Depth		0.66	ft
Critical Slope		0.01121	ft/ft
Velocity		2.20	ft/s
Velocity Head		0.08	ft
Specific Energy		0.83	ft
Froude Number		0.68	
Flow Type	Subcritical		

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.75	ft
Critical Depth	0.66	ft
Channel Slope	0.00500	ft/ft
Critical Slope	0.01121	ft/ft

Cross Section for Ramp Full Street Section

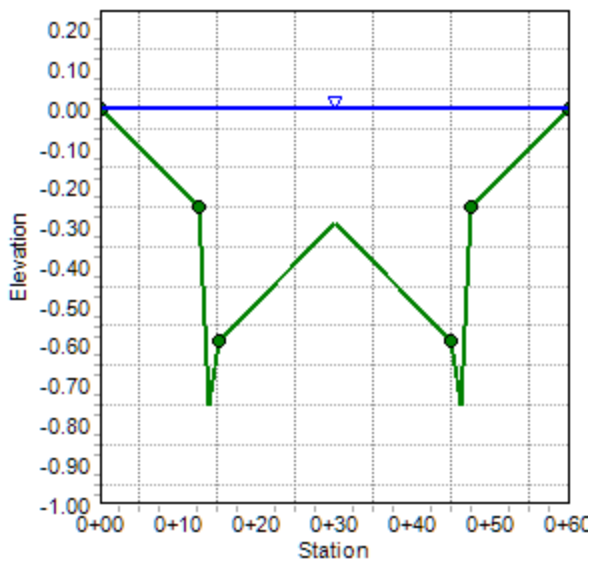
Project Description

Friction Method	Manning Formula
Solve For	Discharge

Input Data

Channel Slope	0.00500	ft/ft
Normal Depth	0.75	ft
Discharge	42.54	ft ³ /s

Cross Section Image



RESIDENTIAL STREET SECTION
RAMP CURB

5-Year Storm Event Maximum Allowable Street Flows (Maximum Flow to Top of Curb)									
Channel Slope (ft/ft)	Full Street Width					Half Street Width			
	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Wetted Perimeter (ft)	Top Width (ft)	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Top Width (ft)
0.0050	19	2.5	7.45	35.2	35.0	9.4	2.5	3.7	17.5
0.0063	21	2.8	7.45	35.2	35.0	11	2.8	3.7	17.5
0.0075	23	3.1	7.45	35.2	35.0	12	3.1	3.7	17.5
0.0088	25	3.4	7.45	35.2	35.0	12	3.3	3.7	17.5
0.0100	27	3.6	7.45	35.2	35.0	13	3.6	3.7	17.5
0.0113	28	3.8	7.45	35.2	35.0	14	3.8	3.7	17.5
0.0125	30	4.0	7.45	35.2	35.0	15	4.0	3.7	17.5
0.0138	31	4.2	7.45	35.2	35.0	16	4.2	3.7	17.5
0.0150	33	4.4	7.45	35.2	35.0	16	4.4	3.7	17.5
0.0163	34	4.6	7.45	35.2	35.0	17	4.5	3.7	17.5
0.0175	35	4.7	7.45	35.2	35.0	18	4.7	3.7	17.5
0.0188	37	4.9	7.45	35.2	35.0	18	4.9	3.7	17.5
0.0200	38	5.1	7.45	35.2	35.0	19	5.0	3.7	17.5
0.0213	39	5.2	7.45	35.2	35.0	19	5.2	3.7	17.5
0.0225	40	5.4	7.45	35.2	35.0	20	5.4	3.7	17.5
0.0238	41	5.5	7.45	35.2	35.0	20	5.5	3.7	17.5
0.0250	42	5.7	7.45	35.2	35.0	21	5.6	3.7	17.5
0.0263	43	5.8	7.45	35.2	35.0	22	5.8	3.7	17.5
0.0275	44	5.9	7.45	35.2	35.0	22	5.9	3.7	17.5
0.0288	45	6.1	7.45	35.2	35.0	23	6.0	3.7	17.5
0.0300	46	6.2	7.45	35.2	35.0	23	6.2	3.7	17.5
0.0313	47	6.3	7.45	35.2	35.0	23	6.3	3.7	17.5
0.0325	48	6.5	7.45	35.2	35.0	24	6.4	3.7	17.5
0.0338	49	6.6	7.45	35.2	35.0	24	6.6	3.7	17.5
0.0350	50	6.7	7.45	35.2	35.0	25	6.7	3.7	17.5
0.0363	51	6.8	7.45	35.2	35.0	25	6.8	3.7	17.5
0.0375	52	6.9	7.45	35.2	35.0	26	6.9	3.7	17.5
0.0388	53	7.1	7.45	35.2	35.0	26	7.0	3.7	17.5
0.0400	53	7.2	7.45	35.2	35.0	27	7.1	3.7	17.5
100-Year Storm Event Maximum Allowable Street Flows (Maximum Flow to Right-of-Way)									
Channel Slope (ft/ft)	Full Street Width					Half Street Width			
	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Wetted Perimeter (ft)	Top Width (ft)	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Top Width (ft)
0.0050	43	2.2	19.32	60.2	60.0	21	2.2	9.7	30
0.0063	48	2.5	19.32	60.2	60.0	24	2.4	9.7	30
0.0075	52	2.7	19.32	60.2	60.0	26	2.7	9.7	30
0.0088	56	2.9	19.32	60.2	60.0	28	2.9	9.7	30
0.0100	60	3.1	19.32	60.2	60.0	30	3.1	9.7	30
0.0113	64	3.3	19.32	60.2	60.0	32	3.3	9.7	30
0.0125	67	3.5	19.32	60.2	60.0	33	3.5	9.7	30
0.0138	71	3.7	19.32	60.2	60.0	35	3.6	9.7	30
0.0150	74	3.8	19.32	60.2	60.0	36	3.8	9.7	30
0.0163	77	4.0	19.32	60.2	60.0	38	3.9	9.7	30
0.0175	80	4.1	19.32	60.2	60.0	39	4.1	9.7	30
0.0188	82	4.3	19.32	60.2	60.0	41	4.2	9.7	30
0.0200	85	4.4	19.32	60.2	60.0	42	4.4	9.7	30
0.0213	88	4.5	19.32	60.2	60.0	43	4.5	9.7	30
0.0225	90	4.7	19.32	60.2	60.0	45	4.6	9.7	30
0.0238	93	4.8	19.32	60.2	60.0	46	4.8	9.7	30
0.0250	95	4.9	19.32	60.2	60.0	47	4.9	9.7	30
0.0263	97	5.0	19.32	60.2	60.0	48	5.0	9.7	30
0.0275	100	5.2	19.32	60.2	60.0	49	5.1	9.7	30
0.0288	102	5.3	19.32	60.2	60.0	50	5.2	9.7	30
0.0300	104	5.4	19.32	60.2	60.0	52	5.3	9.7	30
0.0313	106	5.5	19.32	60.2	60.0	53	5.5	9.7	30
0.0325	108	5.6	19.32	60.2	60.0	54	5.6	9.7	30
0.0338	111	5.7	19.32	60.2	60.0	55	5.7	9.7	30
0.0350	113	5.8	19.32	60.2	60.0	56	5.8	9.7	30
0.0363	115	5.9	19.32	60.2	60.0	57	5.9	9.7	30
0.0375	117	6.0	19.32	60.2	60.0	58	6.0	9.7	30
0.0388	118	6.1	19.32	60.2	60.0	59	6.1	9.7	30
0.0400	120	6.2	19.32	60.2	60.0	60	6.2	9.7	30

Street Flows Ramp Curb
(Maximum Flow to Crown of Roadway)

Channel Slope (ft/ft)	Full Street Width					Half Street Width			
	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Wetted Perimeter (ft)	Top Width (ft)	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Top Width (ft)
0.0050	13	2.2	6.05	35.0	34.8	6.7	2.2	3.0	17.4
0.0063	15	2.5	6.05	35.0	34.8	7.5	2.5	3.0	17.4
0.0075	16	2.7	6.05	35.0	34.8	8.2	2.7	3.0	17.4
0.0088	18	2.9	6.05	35.0	34.8	8.9	2.9	3.0	17.4
0.0100	19	3.1	6.05	35.0	34.8	9.5	3.1	3.0	17.4
0.0113	20	3.3	6.05	35.0	34.8	10	3.3	3.0	17.4
0.0125	21	3.5	6.05	35.0	34.8	11	3.5	3.0	17.4
0.0138	22	3.7	6.05	35.0	34.8	11	3.7	3.0	17.4
0.0150	23	3.8	6.05	35.0	34.8	12	3.8	3.0	17.4
0.0163	24	4.0	6.05	35.0	34.8	12	4.0	3.0	17.4
0.0175	25	4.1	6.05	35.0	34.8	13	4.1	3.0	17.4
0.0188	26	4.3	6.05	35.0	34.8	13	4.3	3.0	17.4
0.0200	27	4.4	6.05	35.0	34.8	13	4.4	3.0	17.4
0.0213	28	4.6	6.05	35.0	34.8	14	4.6	3.0	17.4
0.0225	28	4.7	6.05	35.0	34.8	14	4.7	3.0	17.4
0.0238	29	4.8	6.05	35.0	34.8	15	4.8	3.0	17.4
0.0250	30	5.0	6.05	35.0	34.8	15	5.0	3.0	17.4
0.0263	31	5.1	6.05	35.0	34.8	15	5.1	3.0	17.4
0.0275	31	5.2	6.05	35.0	34.8	16	5.2	3.0	17.4
0.0288	32	5.3	6.05	35.0	34.8	16	5.3	3.0	17.4
0.0300	33	5.4	6.05	35.0	34.8	16	5.4	3.0	17.4
0.0313	34	5.5	6.05	35.0	34.8	17	5.5	3.0	17.4
0.0325	34	5.7	6.05	35.0	34.8	17	5.6	3.0	17.4
0.0338	35	5.8	6.05	35.0	34.8	17	5.8	3.0	17.4
0.0350	35	5.9	6.05	35.0	34.8	18	5.9	3.0	17.4
0.0363	36	6.0	6.05	35.0	34.8	18	6.0	3.0	17.4
0.0375	37	6.1	6.05	35.0	34.8	18	6.1	3.0	17.4
0.0388	37	6.2	6.05	35.0	34.8	19	6.2	3.0	17.4
0.0400	38	6.3	6.05	35.0	34.8	19	6.3	3.0	17.4

Worksheet for Vertical Full Street Section

Project Description

Friction Method Manning Formula
 Solve For Discharge

Input Data

Channel Slope 0.00500 ft/ft
 Normal Depth 0.75 ft
 Section Definitions

Station (ft)	Elevation (ft)
0+00	0.00
0+13	-0.25
0+13	-0.25
0+13	-0.75
0+15	-0.58
0+30	-0.28
0+45	-0.58
0+47	-0.75
0+47	-0.25
0+48	-0.25
0+60	0.00

Roughness Segment Definitions

Start Station	Ending Station	Roughness Coefficient
(0+00, 0.00)	(0+13, -0.25)	0.030
(0+13, -0.25)	(0+15, -0.58)	0.013
(0+15, -0.58)	(0+45, -0.58)	0.015
(0+45, -0.58)	(0+48, -0.25)	0.013
(0+48, -0.25)	(0+60, 0.00)	0.030
<None>	(0+60, 0.00)	0.030

Options

Current Roughness Weighted Method Pavlovskii's Method
 Open Channel Weighting Method Pavlovskii's Method

Worksheet for Vertical Full Street Section

Options

Closed Channel Weighting Method Pavlovskii's Method

Results

Discharge		41.33	ft ³ /s
Elevation Range	-0.75 to 0.00 ft		
Flow Area		19.04	ft ²
Wetted Perimeter		61.02	ft
Hydraulic Radius		0.31	ft
Top Width		60.00	ft
Normal Depth		0.75	ft
Critical Depth		0.66	ft
Critical Slope		0.01143	ft/ft
Velocity		2.17	ft/s
Velocity Head		0.07	ft
Specific Energy		0.82	ft
Froude Number		0.68	
Flow Type	Subcritical		

GVF Input Data

Downstream Depth	0.00	ft
Length	0.00	ft
Number Of Steps	0	

GVF Output Data

Upstream Depth	0.00	ft
Profile Description		
Profile Headloss	0.00	ft
Downstream Velocity	Infinity	ft/s
Upstream Velocity	Infinity	ft/s
Normal Depth	0.75	ft
Critical Depth	0.66	ft
Channel Slope	0.00500	ft/ft
Critical Slope	0.01143	ft/ft

Cross Section for Vertical Full Street Section

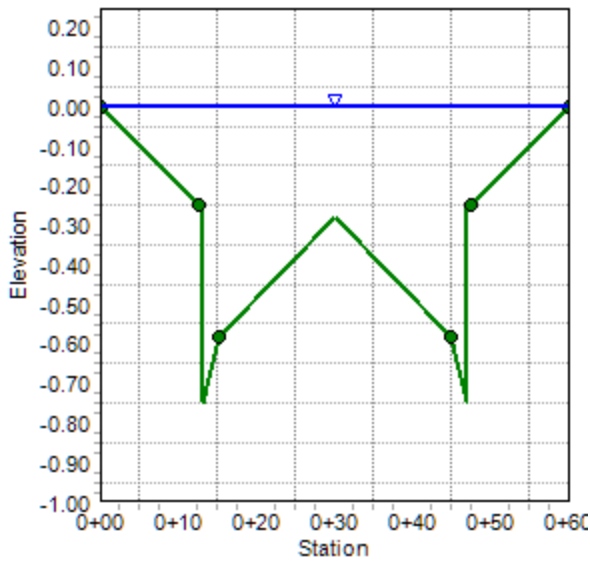
Project Description

Friction Method Manning Formula
Solve For Discharge

Input Data

Channel Slope	0.00500	ft/ft
Normal Depth	0.75	ft
Discharge	41.33	ft ³ /s

Cross Section Image



RESIDENTIAL STREET SECTION
VERTICAL CURB

5-Year Storm Event Maximum Allowable Street Flows (Maximum Flow to Top of Curb)									
Channel Slope (ft/ft)	Full Street Width					Half Street Width			
	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Wetted Perimeter (ft)	Top Width (ft)	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Top Width (ft)
0.0050	18	2.5	7.17	35.0	34.0	8.9	2.5	3.6	17
0.0063	20	2.8	7.17	35.0	34.0	9.9	2.8	3.6	17
0.0075	22	3.0	7.17	35.0	34.0	11	3.0	3.6	17
0.0088	23	3.3	7.17	35.0	34.0	12	3.3	3.6	17
0.0100	25	3.5	7.17	35.0	34.0	13	3.5	3.6	17
0.0113	27	3.7	7.17	35.0	34.0	13	3.7	3.6	17
0.0125	28	3.9	7.17	35.0	34.0	14	3.9	3.6	17
0.0138	29	4.1	7.17	35.0	34.0	15	4.1	3.6	17
0.0150	31	4.3	7.17	35.0	34.0	15	4.3	3.6	17
0.0163	32	4.5	7.17	35.0	34.0	16	4.5	3.6	17
0.0175	33	4.6	7.17	35.0	34.0	17	4.6	3.6	17
0.0188	34	4.8	7.17	35.0	34.0	17	4.8	3.6	17
0.0200	36	5.0	7.17	35.0	34.0	18	5.0	3.6	17
0.0213	37	5.1	7.17	35.0	34.0	18	5.1	3.6	17
0.0225	38	5.3	7.17	35.0	34.0	19	5.3	3.6	17
0.0238	39	5.4	7.17	35.0	34.0	19	5.4	3.6	17
0.0250	40	5.5	7.17	35.0	34.0	20	5.5	3.6	17
0.0263	41	5.7	7.17	35.0	34.0	20	5.7	3.6	17
0.0275	42	5.8	7.17	35.0	34.0	21	5.8	3.6	17
0.0288	43	5.9	7.17	35.0	34.0	21	5.9	3.6	17
0.0300	43	6.1	7.17	35.0	34.0	22	6.1	3.6	17
0.0313	44	6.2	7.17	35.0	34.0	22	6.2	3.6	17
0.0325	45	6.3	7.17	35.0	34.0	23	6.3	3.6	17
0.0338	46	6.4	7.17	35.0	34.0	23	6.4	3.6	17
0.0350	47	6.6	7.17	35.0	34.0	23	6.6	3.6	17
0.0363	48	6.7	7.17	35.0	34.0	24	6.7	3.6	17
0.0375	49	6.8	7.17	35.0	34.0	24	6.8	3.6	17
0.0388	49	6.9	7.17	35.0	34.0	25	6.9	3.6	17
0.0400	50	7.0	7.17	35.0	34.0	25	7.0	3.6	17
100-Year Storm Event Maximum Allowable Street Flows (Maximum Flow to Right-of-Way)									
Channel Slope (ft/ft)	Full Street Width					Half Street Width			
	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Wetted Perimeter (ft)	Top Width (ft)	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Top Width (ft)
0.0050	41	2.2	19.04	61.0	60.0	21	2.2	9.5	30
0.0063	46	2.4	19.04	61.0	60.0	23	2.4	9.5	30
0.0075	51	2.7	19.04	61.0	60.0	25	2.7	9.5	30
0.0088	55	2.9	19.04	61.0	60.0	27	2.9	9.5	30
0.0100	58	3.1	19.04	61.0	60.0	29	3.1	9.5	30
0.0113	62	3.3	19.04	61.0	60.0	31	3.2	9.5	30
0.0125	65	3.4	19.04	61.0	60.0	33	3.4	9.5	30
0.0138	69	3.6	19.04	61.0	60.0	34	3.6	9.5	30
0.0150	72	3.8	19.04	61.0	60.0	36	3.8	9.5	30
0.0163	75	3.9	19.04	61.0	60.0	37	3.9	9.5	30
0.0175	77	4.1	19.04	61.0	60.0	39	4.1	9.5	30
0.0188	80	4.2	19.04	61.0	60.0	40	4.2	9.5	30
0.0200	83	4.3	19.04	61.0	60.0	41	4.3	9.5	30
0.0213	85	4.5	19.04	61.0	60.0	42	4.5	9.5	30
0.0225	88	4.6	19.04	61.0	60.0	44	4.6	9.5	30
0.0238	90	4.7	19.04	61.0	60.0	45	4.7	9.5	30
0.0250	92	4.9	19.04	61.0	60.0	46	4.8	9.5	30
0.0263	95	5.0	19.04	61.0	60.0	47	5.0	9.5	30
0.0275	97	5.1	19.04	61.0	60.0	48	5.1	9.5	30
0.0288	99	5.2	19.04	61.0	60.0	49	5.2	9.5	30
0.0300	101	5.3	19.04	61.0	60.0	50	5.3	9.5	30
0.0313	103	5.4	19.04	61.0	60.0	51	5.4	9.5	30
0.0325	105	5.5	19.04	61.0	60.0	52	5.5	9.5	30
0.0338	107	5.6	19.04	61.0	60.0	53	5.6	9.5	30
0.0350	109	5.7	19.04	61.0	60.0	54	5.7	9.5	30
0.0363	111	5.8	19.04	61.0	60.0	55	5.8	9.5	30
0.0375	113	5.9	19.04	61.0	60.0	56	5.9	9.5	30
0.0388	115	6.0	19.04	61.0	60.0	57	6.0	9.5	30
0.0400	117	6.1	19.04	61.0	60.0	58	6.1	9.5	30

Street Flows Vertical Curb
(Maximum Flow to Crown of Roadway)

Channel Slope (ft/ft)	Full Street Width					Half Street Width			
	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Wetted Perimeter (ft)	Top Width (ft)	Discharge (ft ³ /s)	Velocity (ft/s)	Flow Area (ft ²)	Top Width (ft)
0.0050	14	2.2	6.15	35.0	34.0	6.7	2.2	3.0	17
0.0063	15	2.5	6.15	35.0	34.0	7.5	2.5	3.0	17
0.0075	17	2.7	6.15	35.0	34.0	8.2	2.7	3.0	17
0.0088	18	3.0	6.15	35.0	34.0	8.8	2.9	3.0	17
0.0100	19	3.2	6.15	35.0	34.0	9.4	3.1	3.0	17
0.0113	21	3.4	6.15	35.0	34.0	10	3.3	3.0	17
0.0125	22	3.5	6.15	35.0	34.0	11	3.5	3.0	17
0.0138	23	3.7	6.15	35.0	34.0	11	3.7	3.0	17
0.0150	24	3.9	6.15	35.0	34.0	12	3.8	3.0	17
0.0163	25	4.0	6.15	35.0	34.0	12	4.0	3.0	17
0.0175	26	4.2	6.15	35.0	34.0	12	4.1	3.0	17
0.0188	27	4.3	6.15	35.0	34.0	13	4.3	3.0	17
0.0200	28	4.5	6.15	35.0	34.0	13	4.4	3.0	17
0.0213	28	4.6	6.15	35.0	34.0	14	4.6	3.0	17
0.0225	29	4.8	6.15	35.0	34.0	14	4.7	3.0	17
0.0238	30	4.9	6.15	35.0	34.0	15	4.8	3.0	17
0.0250	31	5.0	6.15	35.0	34.0	15	4.9	3.0	17
0.0263	32	5.1	6.15	35.0	34.0	15	5.1	3.0	17
0.0275	32	5.3	6.15	35.0	34.0	16	5.2	3.0	17
0.0288	33	5.4	6.15	35.0	34.0	16	5.3	3.0	17
0.0300	34	5.5	6.15	35.0	34.0	16	5.4	3.0	17
0.0313	34	5.6	6.15	35.0	34.0	17	5.5	3.0	17
0.0325	35	5.7	6.15	35.0	34.0	17	5.6	3.0	17
0.0338	36	5.8	6.15	35.0	34.0	17	5.7	3.0	17
0.0350	36	5.9	6.15	35.0	34.0	18	5.9	3.0	17
0.0363	37	6.0	6.15	35.0	34.0	18	6.0	3.0	17
0.0375	38	6.1	6.15	35.0	34.0	18	6.1	3.0	17
0.0388	38	6.2	6.15	35.0	34.0	19	6.2	3.0	17
0.0400	39	6.3	6.15	35.0	34.0	19	6.3	3.0	17

Appendix C - HEC-HMS Data

Input Data

Windingwalk Filing 1

BASIN	AREA		CURVE NO.	PERCENT IMPERV.	LAG TIME (min)	
	(acre)	(mi ²)				
HISTORIC						
OS01	998	1.5594	62.9	6.5%	35.5	♦♦
OS02	142	0.2219	64.5	7.7%	25.5	♦♦
OS03	127	0.1984	63.2	4.8%	23.6	♦♦
OS04	87	0.1359	61.0	0.0%	21.4	♦♦
HB01	15	0.0234	61.0	0.0%	12.6	♦♦
HB02	68	0.1063	61.0	0.0%	16.2	♦♦
HB03	81	0.1266	61.0	0.0%	13.2	♦♦
HB04	39	0.0609	61.0	0.0%	14.4	♦♦
HB05	88	0.1375	61.0	0.0%	15.6	♦♦
HB06	105	0.1641	61.0	0.0%	18.0	♦♦
HB07	20	0.0313	61.0	0.0%	10.2	♦♦
HB08	86	0.1344	61.0	0.0%	21.6	♦♦
HB09	195	0.3047	61.0	0.0%	33.0	♦♦
HB10	195	0.3047	61.0	0.0%	24.0	♦♦
HB12	51	0.0797	61.0	0.0%	18.0	♦♦
B-11	72	0.1125	61.0	0.0%	25.8	♦♦
B-13	180	0.2813	61.0	0.0%	33.0	♦♦
B-14	259	0.4039	61.0	0.0%	34.2	♦♦
B-15	48	0.0750	61.0	0.0%	27.0	♦♦
HG07	63	0.0984	61.0	0.0%	28.3	♦♦
HG08	85	0.1328	61.0	0.0%	22.9	♦♦
HG09	114	0.1781	61.0	0.0%	35.6	♦♦
HG10	88	0.1375	61.0	0.0%	61.4	♦♦
HG11	131	0.2047	61.0	0.0%	40.4	♦♦
HG12	83	0.1297	61.0	0.0%	32.0	♦♦
HH01	63	0.0984	61.0	0.0%	16.6	♦♦
INTERIM						
OS01	998	1.5594	62.9	6.5%	35.5	♦♦
OS02	142	0.2219	64.5	7.7%	25.5	♦♦
OS03	127	0.1984	63.2	4.8%	23.6	♦♦
OS04	87	0.1359	61.0	0.0%	21.4	♦♦
DB01	46	0.0719	69.7	23.8%	13.7	♦♦
DB02	33	0.0516	69.0	21.8%	10.5	♦♦
DB03	45	0.0703	65.8	12.9%	15.0	♦♦
DB04	27	0.0422	66.8	16.5%	15.3	♦♦
DB05	25	0.0384	68.0	20.0%	19.1	♦♦
DB06	14	0.0219	84.0	62.6%	14.6	♦♦
DB07	16	0.0254	70.0	25.0%	11.7	♦♦
DB08	19	0.0297	64.9	10.5%	11.9	♦♦
DB09	12	0.0189	75.0	40.0%	9.6	♦♦
DB10	23	0.0364	75.0	40.0%	13.7	♦♦
DB11	62	0.0969	72.0	31.3%	18.4	♦♦
DB12	29	0.0453	78.2	42.5%	12.7	♦♦

BASIN	AREA		CURVE NO.	PERCENT IMPERV.	LAG TIME (min)	
	(acre)	(mi ²)				
INTERIM						
DB13	45	0.0703	73.9	33.0%	18.6	♦♦
DB14	36	0.0556	78.0	43.0%	14.6	♦♦
DB15	79	0.1234	67.1	16.8%	21.8	♦♦
DB16	37	0.0578	78.5	47.3%	16.4	♦♦
DB17	3	0.0048	98.0	100.0%	7.4	♦♦
DB18	22	0.0346	80.0	47.0%	13.4	♦♦
DB19	18	0.0281	72.6	28.5%	16.2	♦♦
DB20	9	0.0147	78.7	46.1%	15.2	♦♦
DB21	33	0.0519	65.6	10.9%	13.6	♦♦
DB22	33	0.0516	80.0	48.2%	14.8	♦♦
DB23	11	0.0172	91.6	81.2%	11.3	♦♦
DB24	34	0.0531	78.5	43.3%	13.3	♦♦
DB25	14	0.0211	80.0	47.0%	9.7	♦♦
DB26	44	0.0692	85.8	71.5%	16.1	◇◇
DB27	33	0.0508	78.1	42.2%	21.9	◇◇
DB28	47	0.0741	70.0	19.6%	17.6	◇◇
DB29	109	0.1697	68.5	21.6%	23.9	◇◇
FB01	24	0.0373	66.6	0.2%	14.2	
FB02	32	0.0500	79.1	44.8%	22.8	
FB03	5	0.0078	90.1	77.6%	9.0	
WH-24	85	0.1325	79.0	46.0%	16.0	♦
WH-26	54	0.0839	62.0	2.0%	25.1	♦
WH-27	14	0.0217	62.0	2.0%	8.6	♦
WH-28	26	0.0398	78.3	43.9%	17.7	♦
WH-29	32	0.0495	78.0	43.0%	16.6	♦
WH-30	10	0.0159	68.6	18.9%	6.0	♦
WH-31	26	0.0406	80.0	47.0%	13.2	♦
WH-32	29	0.0458	62.0	2.0%	6.0	♦
WH-33	4	0.0064	80.0	47.0%	13.0	♦
WH-34	29	0.0453	75.0	N/A	14.4	♦
WH-35	99	0.1547	68.0	N/A	15.0	♦
WH-36	48	0.0750	63.0	N/A	15.6	♦
FG08A	48	0.0750	76.8	42.6%	13.3	✓
FG08B	40	0.0630	76.7	39.8%	16.6	✓
FG09	31	0.0484	71.7	26.9%	20.8	●●
HG10	30	0.0467	63.2	6.1%	23.1	
FG11	40	0.0625	78.2	44.1%	23.2	●
FG12	21	0.0328	80.0	47.0%	16.1	❖❖
FG13	42	0.0661	66.9	14.2%	29.6	
HG15	19	0.0297	62.1	2.6%	35.0	
FG15a	10	0.0156	78.7	43.9%	11.2	❖
FG16	50	0.0773	78.8	44.8%	13.0	❖
FG17a	44	0.0694	76.5	39.3%	14.4	✓✓
FG17b	14	0.0214	79.9	47.1%	11.4	✓✓
FG17c	20	0.0313	65.2	9.9%	11.8	✓✓
FG18	41	0.0644	64.9	0.8%	29.9	◇
FG19	34	0.0527	76.8	38.4%	15.3	✓✓
FG19a	5	0.0077	75.2	36.3%	16.4	
FG20	7	0.0109	92.9	86.0%	10.1	

BASIN	AREA		CURVE NO.	PERCENT IMPERV.	LAG TIME (min)	
	(acre)	(mi ²)				
INTERIM						
FG21	42	0.0656	66.9	16.8%	22.0	
FG22	41	0.0641	66.9	16.2%	27.4	
FG23	52	0.0813	66.5	15.7%	26.5	
FG24	67	0.1041	64.9	11.2%	22.7	
FG25	14	0.0219	70.8	26.4%	26.6	
FG26	52	0.0813	72.5	28.8%	24.8	
FG27a	17	0.0259	65.5	12.2%	31.4	
FG27b	33	0.0508	77.2	40.9%	24.3	
FG28	13	0.0203	65.6	11.1%	17.5	
FG29	66	0.1031	61.3	0.8%	23.3	
HG30	118	0.1844	61.0	0.0%	65.1	
FG31	59	0.0922	80.0	52.0%	24.0	♦♦
FH01	86	0.1344	72.4	23.8%	30.9	
FH02	6	0.0091	71.3	25.3%	14.6	
FH03	5	0.0081	80.7	52.4%	14.4	

FUTURE						
OS01	998	1.5594	62.9	6.5%	35.5	♦♦
OS02	142	0.2219	64.5	7.7%	25.5	♦♦
OS03	127	0.1984	63.2	4.8%	23.6	♦♦
OS04	87	0.1359	61.0	0.0%	21.4	♦♦
DB01	46	0.0719	69.7	23.8%	13.7	♦♦
DB02	33	0.0516	69.0	21.8%	10.5	♦♦
DB03	45	0.0703	65.8	12.9%	15.0	♦♦
DB04	27	0.0422	66.8	16.5%	15.3	♦♦
DB05	25	0.0384	68.0	20.0%	19.1	♦♦
DB06	14	0.0219	84.0	62.6%	14.6	♦♦
DB07	16	0.0254	70.0	25.0%	11.7	♦♦
DB08	19	0.0297	64.9	10.5%	11.9	♦♦
DB09	12	0.0189	75.0	40.0%	9.6	♦♦
DB10	23	0.0364	75.0	40.0%	13.7	♦♦
DB11	62	0.0969	72.0	31.3%	18.4	♦♦
DB12	29	0.0453	78.2	42.5%	12.7	♦♦
DB13	45	0.0703	73.9	33.0%	18.6	♦♦
DB14	36	0.0556	78.0	43.0%	14.6	♦♦
DB15	79	0.1234	67.1	16.8%	21.8	♦♦
DB16	37	0.0578	78.5	47.3%	16.4	♦♦
DB17	3	0.0048	98.0	100.0%	7.4	♦♦
DB18	22	0.0346	80.0	47.0%	13.4	♦♦
DB19	18	0.0281	72.6	28.5%	16.2	♦♦
DB20	9	0.0147	78.7	46.1%	15.2	♦♦
DB21	33	0.0519	65.6	10.9%	13.6	♦♦
DB22	33	0.0516	80.0	48.2%	14.8	♦♦
DB23	11	0.0172	91.6	81.2%	11.3	♦♦
DB24	34	0.0531	78.5	43.3%	13.3	♦♦
DB25	14	0.0211	80.0	47.0%	9.7	♦♦
DB26	44	0.0692	85.8	71.5%	16.1	◇◇
DB27	33	0.0508	78.1	42.2%	21.9	◇◇
DB28	47	0.0741	70.7	23.8%	17.6	◇◇
DB29	109	0.1697	68.5	21.6%	23.9	◇◇

BASIN	AREA		CURVE NO.	PERCENT IMPERV.	LAG TIME (min)	
	(acre)	(mi ²)				
FUTURE						
FB01	24	0.0373	77.7	41.4%	14.2	◆◆◆
FB02	32	0.0500	79.1	44.8%	22.8	◆◆◆
FB03	5	0.0078	90.1	77.6%	9.0	◆◆◆
WH-24	85	0.1325	79.0	46.0%	16.0	◆
WH-26	54	0.0839	62.0	2.0%	25.1	◆
WH-27	14	0.0217	62.0	2.0%	8.6	◆
WH-28	26	0.0398	78.3	43.9%	17.7	◆
WH-29	32	0.0495	78.0	43.0%	16.6	◆
WH-30	10	0.0159	68.6	18.9%	6.0	◆
WH-31	26	0.0406	80.0	47.0%	13.2	◆
WH-32	29	0.0458	62.0	2.0%	6.0	◆
WH-33	4	0.0064	80.0	47.0%	13.0	◆
WH-34	29	0.0453	75.0	N/A	14.4	◆
FG04	11	0.0172	68.0	20.0%	7.6	■ ■
FG05	59	0.0922	66.9	16.9%	28.7	■
FG06	12	0.0188	68.0	20.0%	15.3	
FG08A	48	0.0750	76.8	42.6%	13.3	✓
FG08B	40	0.0630	76.7	39.8%	16.6	✓
FG09	31	0.0484	71.7	26.9%	20.8	● ●
FG10	43	0.0669	72.7	29.5%	41.8	
FG11	40	0.0625	78.2	44.1%	23.2	●
FG12	21	0.0328	80.0	47.0%	16.1	❖ ❖
FG13	42	0.0661	66.9	14.3%	29.6	
FG14	21	0.0331	77.5	41.8%	20.9	
FG15	65	0.1017	72.9	29.9%	25.9	
FG15a	10	0.0156	78.7	43.9%	11.2	❖
FG16	50	0.0773	78.8	44.8%	13.0	❖
FG17a	44	0.0694	76.5	39.3%	14.4	✓ ✓
FG17b	14	0.0214	79.9	47.1%	11.4	✓ ✓
FG17c	20	0.0313	65.2	9.9%	11.8	✓ ✓
FG18	41	0.0644	73.5	30.8%	29.9	❖
FG35	36	0.0566	62.7	4.8%	20.7	
FG36	18	0.0281	61.0	0.0%	24.9	
FG37	51	0.0797	61.0	0.0%	21.8	
FH01	86	0.1344	76.2	37.6%	23.4	◆◆◆
FH02	6	0.0091	71.3	25.3%	14.6	◆◆◆

◇	From Meridian Ranch Drainage Reports (Windingwalk Rational Calcs., September 2017)
◆	From Retrofit Drainage Analysis For Bennett Regional Detention Pond, Jun 2014)
◆◆	From Approved Meridian Ranch Filing MDDP, Aug 2015
◆◆◆	From Approved Meridian Ranch Filing MDDP, Dec 2017
◇◇	From Approved Meridian Ranch Final Drainage Reports (Stonebridge Filing 2, Oct 2016)
■	From Estates Filing 2 Final Drainage Report, July 2013
■ ■	From Estates Filing 3 Final Drainage Report, Nov 2015
◇	From Meridian Ranch Filing 11b Approved Final Drainage Report, Nov 2014
◇◇	From Meridian Ranch Filing 3 Approved Final Drainage Report, Aug 2012
●	From Meridian Ranch Filing 7 Approved Final Drainage Report, Aug 2012
● ●	From Meridian Ranch Filing 8 Approved Final Drainage Report, Feb 2015
✓	From Meridian Ranch Filing 9 Approved Final Drainage Report, July 2015
✓✓	From Stonebridge Filing 3 Approved Final Drainage Report, April 2017



NOAA Atlas 14, Volume 6, Version 2
 Location name: Peyton, Colorado, USA*
 Latitude: 38.9783°, Longitude: -104.8842°
 Elevation: 7054.14 ft*



* source: ERI Maps
 ** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Benja Petras, Deborah Meath, Sandra Perovich, Ishani Roy, Michael Di Laurent, Carl Tysalluk,
 Dale Urruth, Michael Yelton, Geoffrey Bannin

NOAA, National Weather Service, Silver Spring, Maryland

[PF tabular](#) | [PF graphical](#) | [Maps & aerials](#)

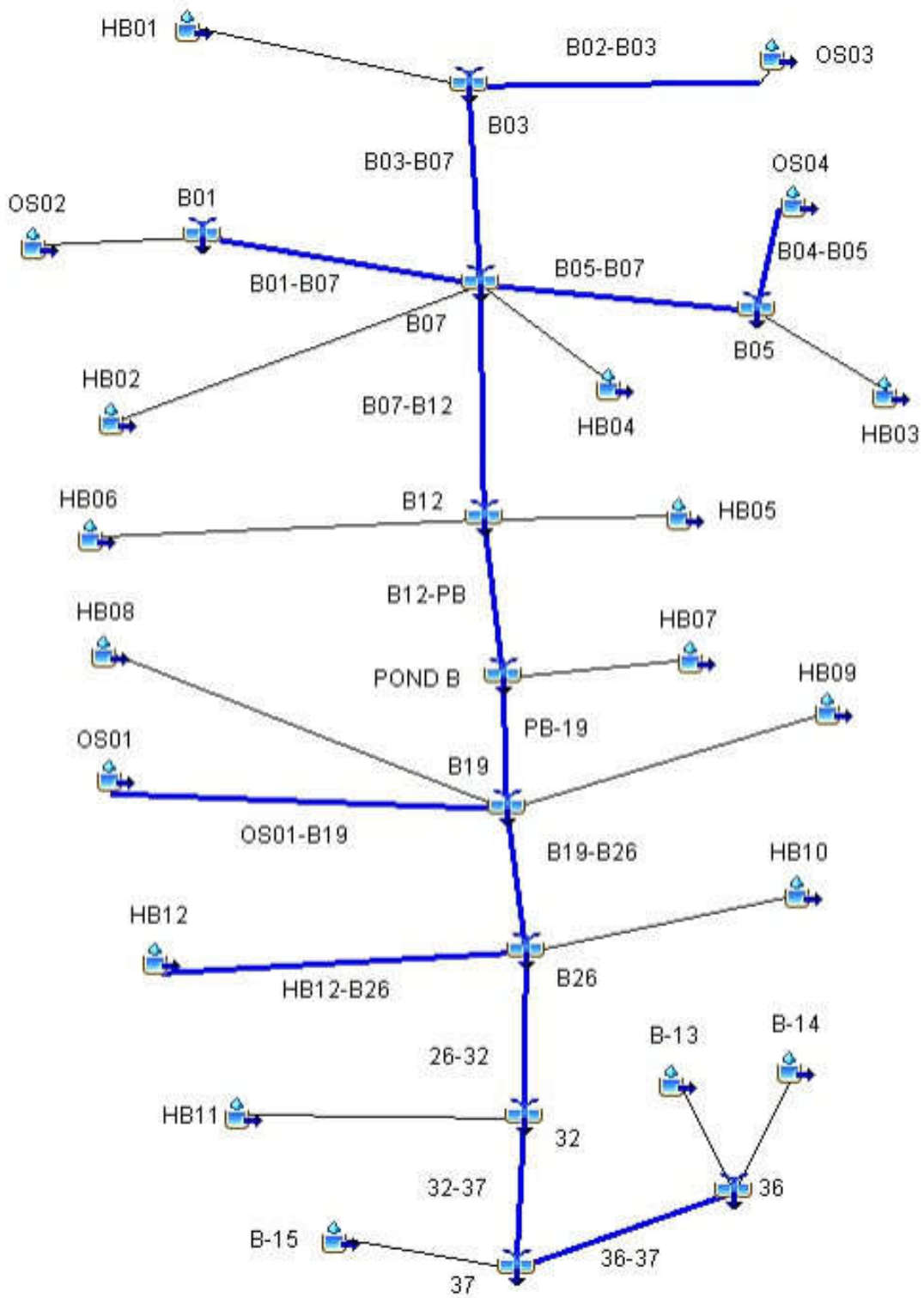
PF tabular

PDF-based point precipitation frequency estimates with 90% confidence intervals (in inches)¹

Duration	Average recurrence Interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.239 (0.190-0.301)	0.281 (0.232-0.327)	0.341 (0.302-0.482)	0.400 (0.353-0.595)	0.576 (0.442-0.764)	0.870 (0.501-0.997)	0.770 (0.598-1.05)	0.870 (0.500-1.23)	1.02 (0.590-1.45)	1.14 (0.737-1.65)
10-min	0.348 (0.278-0.441)	0.426 (0.338-0.538)	0.558 (0.443-0.705)	0.874 (0.632-0.957)	0.843 (0.647-1.12)	0.882 (0.734-1.32)	1.13 (0.814-1.55)	1.28 (0.888-1.80)	1.50 (0.908-2.16)	1.67 (1.08-2.44)
15-min	0.426 (0.340-0.538)	0.518 (0.413-0.660)	0.680 (0.540-0.801)	0.822 (0.648-1.04)	1.03 (0.788-1.38)	1.20 (0.885-1.81)	1.37 (0.989-1.89)	1.58 (1.08-2.20)	1.82 (1.22-2.54)	2.03 (1.31-2.87)
30-min	0.568 (0.486-0.768)	0.741 (0.630-0.938)	0.969 (0.788-1.23)	1.17 (0.923-1.48)	1.46 (1.12-1.94)	1.70 (1.27-2.25)	1.93 (1.41-2.68)	2.31 (1.53-3.12)	2.58 (1.72-3.73)	2.87 (1.88-4.20)
60-min	0.778 (0.620-0.982)	0.934 (0.744-1.18)	1.21 (0.982-1.54)	1.47 (1.16-1.88)	1.84 (1.42-2.48)	2.18 (1.62-2.81)	2.50 (1.81-3.44)	2.87 (1.88-4.05)	3.38 (2.25-4.81)	3.80 (2.48-5.58)
2-hr	0.948 (0.782-1.18)	1.13 (0.905-1.41)	1.46 (1.16-1.83)	1.78 (1.40-2.22)	2.23 (1.73-2.88)	2.82 (1.98-3.91)	3.08 (2.23-4.18)	3.52 (2.47-4.88)	4.18 (2.82-5.84)	4.73 (3.08-6.87)
3-hr	1.04 (0.839-1.28)	1.22 (0.985-1.52)	1.57 (1.26-1.88)	1.90 (1.51-2.38)	2.41 (1.90-3.21)	2.96 (2.18-3.83)	3.35 (2.47-4.58)	3.90 (2.75-5.47)	4.68 (3.18-6.75)	5.33 (3.50-7.71)
6-hr	1.21 (0.980-1.48)	1.48 (1.14-1.73)	1.78 (1.44-2.21)	2.18 (1.74-2.88)	2.78 (2.19-3.85)	3.29 (2.53-4.38)	3.88 (2.88-5.28)	4.53 (3.23-6.34)	5.48 (3.76-7.88)	6.28 (4.17-8.84)
12-hr	1.38 (1.14-1.70)	1.62 (1.23-1.98)	2.08 (1.68-2.68)	2.48 (2.02-3.08)	3.18 (2.63-4.14)	3.78 (2.92-4.98)	4.42 (3.31-5.87)	5.18 (3.70-7.14)	6.22 (4.30-8.88)	7.10 (4.76-10.1)
24-hr	1.81 (1.33-1.95)	1.88 (1.65-2.28)	2.38 (1.97-2.92)	2.88 (2.35-3.52)	3.53 (2.91-4.88)	4.27 (3.34-5.68)	4.98 (3.76-6.88)	5.78 (4.17-7.88)	6.87 (4.78-9.70)	7.78 (5.25-11.1)

HISTORIC 100-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀₀ (AC. FT.)
OS02	0.2219	148	01Jul2015, 12:20	19
B01	0.2219	148	01Jul2015, 12:20	19
B01-B07	0.2219	148	01Jul2015, 12:24	19
OS03	0.1984	130	01Jul2015, 12:18	16
B02-B03	0.1984	129	01Jul2015, 12:20	16
HB01	0.0234	19	01Jul2015, 12:08	2
B03	0.2218	140	01Jul2015, 12:20	17
B03-B07	0.2218	140	01Jul2015, 12:22	17
OS04	0.1359	83	01Jul2015, 12:16	10
B04-B05	0.1359	82	01Jul2015, 12:24	10
HB03	0.1266	103	01Jul2015, 12:08	9
B05	0.2625	144	01Jul2015, 12:16	19
B05-B07	0.2625	144	01Jul2015, 12:16	19
HB02	0.1063	77	01Jul2015, 12:12	8
HB04	0.0609	47	01Jul2015, 12:10	4
B07	0.8734	519	01Jul2015, 12:18	67
B07-B12	0.8734	518	01Jul2015, 12:24	66
HB05	0.1375	102	01Jul2015, 12:10	10
HB06	0.1641	111	01Jul2015, 12:14	12
B12	1.1750	679	01Jul2015, 12:20	88
B12-PB	1.1750	677	01Jul2015, 12:22	88
HB07	0.0313	29	01Jul2015, 12:06	2
POND B	1.2063	688	01Jul2015, 12:22	90
PB-19	1.2063	687	01Jul2015, 12:26	89
OS01	1.5594	757	01Jul2015, 12:32	122
OS01-B19	1.5594	756	01Jul2015, 12:38	121
HB08	0.1344	81	01Jul2015, 12:16	10
HB09	0.3047	138	01Jul2015, 12:30	22
B19	3.2048	1563	01Jul2015, 12:30	241
B19-B26	3.2048	1563	01Jul2015, 12:32	241
HB10	0.3047	172	01Jul2015, 12:20	22
HB12	0.0797	54	01Jul2015, 12:14	6
HB12-B26	0.0797	54	01Jul2015, 12:16	6
B26	3.5892	1737	01Jul2015, 12:30	268
26-32	3.5892	1734	01Jul2015, 12:34	267
HB11	0.1125	60	01Jul2015, 12:22	8
32	3.7017	1782	01Jul2015, 12:34	275
32-37	3.7017	1782	01Jul2015, 12:36	273
B-14	0.4039	178	01Jul2015, 12:32	29
B-13	0.2813	127	01Jul2015, 12:30	20
36	0.6852	306	01Jul2015, 12:30	49
36-37	0.6852	305	01Jul2015, 12:34	49
B-15	0.0750	39	01Jul2015, 12:22	5
37	4.4619	2117	01Jul2015, 12:36	327
HH01	0.0984	70	01Jul2015, 12:12	7
H12	0.0984	70	01Jul2015, 12:12	7

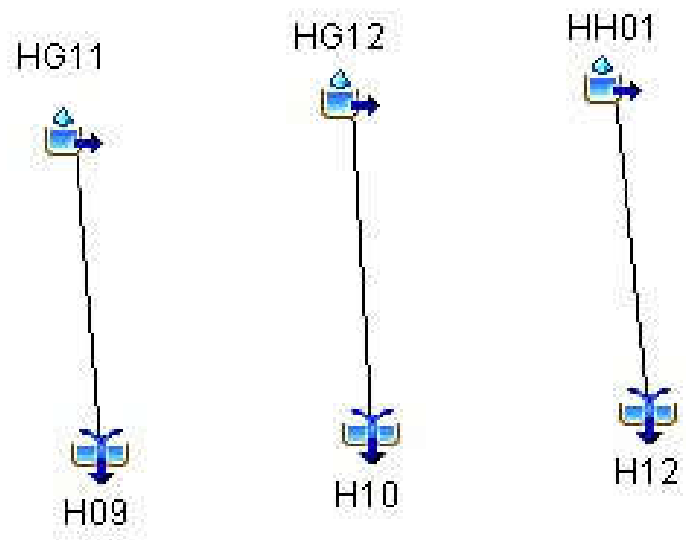
BENNETT HISTORIC



HISTORIC 50-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅₀ (AC. FT.)
OS02	0.2219	102	01Jul2015, 12:22	14
B01	0.2219	102	01Jul2015, 12:22	14
B01-B07	0.2219	102	01Jul2015, 12:24	14
OS03	0.1984	88	01Jul2015, 12:20	11
B02-B03	0.1984	88	01Jul2015, 12:22	11
HB01	0.0234	13	01Jul2015, 12:08	1
B03	0.2218	95	01Jul2015, 12:20	12
B03-B07	0.2218	94	01Jul2015, 12:24	12
OS04	0.1359	54	01Jul2015, 12:18	7
B04-B05	0.1359	54	01Jul2015, 12:26	7
HB03	0.1266	68	01Jul2015, 12:08	6
B05	0.2625	91	01Jul2015, 12:18	13
B05-B07	0.2625	91	01Jul2015, 12:20	13
HB02	0.1063	51	01Jul2015, 12:12	5
HB04	0.0609	31	01Jul2015, 12:10	3
B07	0.8734	344	01Jul2015, 12:20	47
B07-B12	0.8734	343	01Jul2015, 12:26	47
HB05	0.1375	67	01Jul2015, 12:12	7
HB06	0.1641	73	01Jul2015, 12:14	8
B12	1.1750	440	01Jul2015, 12:22	62
B12-PB	1.1750	440	01Jul2015, 12:24	62
HB07	0.0313	19	01Jul2015, 12:06	2
POND B	1.2063	446	01Jul2015, 12:24	64
PB-19	1.2063	444	01Jul2015, 12:28	63
OS01	1.5594	510	01Jul2015, 12:34	87
OS01-B19	1.5594	509	01Jul2015, 12:40	86
HB08	0.1344	53	01Jul2015, 12:18	7
HB09	0.3047	90	01Jul2015, 12:32	15
B19	3.2048	1041	01Jul2015, 12:34	171
B19-B26	3.2048	1039	01Jul2015, 12:34	171
HB10	0.3047	113	01Jul2015, 12:20	15
HB12	0.0797	36	01Jul2015, 12:14	4
HB12-B26	0.0797	35	01Jul2015, 12:18	4
B26	3.5892	1147	01Jul2015, 12:34	190
26-32	3.5892	1146	01Jul2015, 12:36	189
HB11	0.1125	40	01Jul2015, 12:22	6
32	3.7017	1177	01Jul2015, 12:36	194
32-37	3.7017	1175	01Jul2015, 12:40	193
B-14	0.4039	117	01Jul2015, 12:32	20
B-13	0.2813	83	01Jul2015, 12:32	14
36	0.6852	200	01Jul2015, 12:32	34
36-37	0.6852	200	01Jul2015, 12:36	34
B-15	0.0750	26	01Jul2015, 12:24	4
37	4.4619	1391	01Jul2015, 12:38	231
HH01	0.0984	46	01Jul2015, 12:12	5
H12	0.0984	46	01Jul2015, 12:12	5

HISTORIC 25-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂₅ (AC. FT.)
OS02	0.2219	65	01Jul2015, 12:22	9.3
B01	0.2219	65	01Jul2015, 12:22	9.3
B01-B07	0.2219	65	01Jul2015, 12:26	9.2
OS03	0.1984	55	01Jul2015, 12:20	7.7
B02-B03	0.1984	55	01Jul2015, 12:24	7.6
HB01	0.0234	8	01Jul2015, 12:08	0.8
B03	0.2218	59	01Jul2015, 12:22	8.4
B03-B07	0.2218	59	01Jul2015, 12:26	8.4
OS04	0.1359	32	01Jul2015, 12:18	4.5
B04-B05	0.1359	32	01Jul2015, 12:28	4.4
HB03	0.1266	41	01Jul2015, 12:10	4.2
B05	0.2625	52	01Jul2015, 12:24	8.7
B05-B07	0.2625	52	01Jul2015, 12:26	8.7
HB02	0.1063	30	01Jul2015, 12:12	3.6
HB04	0.0609	19	01Jul2015, 12:10	2.0
B07	0.8734	207	01Jul2015, 12:24	31.9
B07-B12	0.8734	207	01Jul2015, 12:30	31.5
HB05	0.1375	40	01Jul2015, 12:12	4.6
HB06	0.1641	43	01Jul2015, 12:14	5.5
B12	1.1750	259	01Jul2015, 12:26	41.6
B12-PB	1.1750	259	01Jul2015, 12:28	41.5
HB07	0.0313	12	01Jul2015, 12:06	1.0
POND B	1.2063	262	01Jul2015, 12:28	42.6
PB-19	1.2063	261	01Jul2015, 12:34	42.1
OS01	1.5594	316	01Jul2015, 12:36	58.6
OS01-B19	1.5594	315	01Jul2015, 12:44	57.8
HB08	0.1344	32	01Jul2015, 12:20	4.5
HB09	0.3047	54	01Jul2015, 12:34	10.1
B19	3.2048	635	01Jul2015, 12:38	114.5
B19-B26	3.2048	634	01Jul2015, 12:38	114.3
HB10	0.3047	67	01Jul2015, 12:22	10.1
HB12	0.0797	21	01Jul2015, 12:14	2.7
HB12-B26	0.0797	21	01Jul2015, 12:20	2.6
B26	3.5892	693	01Jul2015, 12:38	127.0
26-32	3.5892	693	01Jul2015, 12:42	126.0
HB11	0.1125	23	01Jul2015, 12:24	3.7
32	3.7017	709	01Jul2015, 12:42	129.8
32-37	3.7017	708	01Jul2015, 12:44	128.7
B-14	0.4039	70	01Jul2015, 12:34	13.3
B-13	0.2813	50	01Jul2015, 12:34	9.3
36	0.6852	119	01Jul2015, 12:34	22.6
36-37	0.6852	119	01Jul2015, 12:38	22.5
B-15	0.0750	15	01Jul2015, 12:26	2.5
37	4.4619	834	01Jul2015, 12:44	153.7
HH01	0.0984	27	01Jul2015, 12:14	3.3
H12	0.0984	27	01Jul2015, 12:14	3.3

MISC. HISTORIC



HISTORIC 10-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀ (AC. FT.)
OS02	0.2219	30	01 Jul2015, 12:26	5.1
B01	0.2219	30	01 Jul2015, 12:26	5.1
B01-B07	0.2219	30	01 Jul2015, 12:30	5.0
OS03	0.1984	23	01 Jul2015, 12:24	4.1
B02-B03	0.1984	23	01 Jul2015, 12:26	4.0
HB01	0.0234	3	01 Jul2015, 12:10	0.4
B03	0.2218	25	01 Jul2015, 12:26	4.4
B03-B07	0.2218	25	01 Jul2015, 12:30	4.4
OS04	0.1359	12	01 Jul2015, 12:22	2.3
B04-B05	0.1359	12	01 Jul2015, 12:34	2.2
HB03	0.1266	15	01 Jul2015, 12:12	2.1
B05	0.2625	20	01 Jul2015, 12:30	4.4
B05-B07	0.2625	20	01 Jul2015, 12:32	4.4
HB02	0.1063	11	01 Jul2015, 12:16	1.8
HB04	0.0609	7	01 Jul2015, 12:12	1.0
B07	0.8734	86	01 Jul2015, 12:30	16.6
B07-B12	0.8734	86	01 Jul2015, 12:38	16.4
HB05	0.1375	15	01 Jul2015, 12:14	2.3
HB06	0.1641	16	01 Jul2015, 12:18	2.8
B12	1.1750	103	01 Jul2015, 12:36	21.5
B12-PB	1.1750	103	01 Jul2015, 12:38	21.4
HB07	0.0313	4	01 Jul2015, 12:08	0.5
POND B	1.2063	105	01 Jul2015, 12:38	22.0
PB-19	1.2063	104	01 Jul2015, 12:46	21.7
OS01	1.5594	136	01 Jul2015, 12:38	30.9
OS01-B19	1.5594	136	01 Jul2015, 12:48	30.4
HB08	0.1344	12	01 Jul2015, 12:22	2.3
HB09	0.3047	21	01 Jul2015, 12:38	5.1
B19	3.2048	266	01 Jul2015, 12:46	59.4
B19-B26	3.2048	266	01 Jul2015, 12:48	59.2
HB10	0.3047	26	01 Jul2015, 12:26	5.1
HB12	0.0797	8	01 Jul2015, 12:18	1.3
HB12-B26	0.0797	8	01 Jul2015, 12:24	1.3
B26	3.5892	288	01 Jul2015, 12:48	65.7
26-32	3.5892	287	01 Jul2015, 12:52	65.0
HB11	0.1125	9	01 Jul2015, 12:28	1.9
32	3.7017	293	01 Jul2015, 12:52	66.9
32-37	3.7017	293	01 Jul2015, 12:58	66.1
B-14	0.4039	27	01 Jul2015, 12:38	6.7
B-13	0.2813	19	01 Jul2015, 12:38	4.7
36	0.6852	47	01 Jul2015, 12:38	11.4
36-37	0.6852	47	01 Jul2015, 12:42	11.3
B-15	0.0750	6	01 Jul2015, 12:30	1.3
37	4.4619	338	01 Jul2015, 12:56	78.7
HH01	0.0984	10	01 Jul2015, 12:16	1.7
H12	0.0984	10	01 Jul2015, 12:16	1.7

HISTORIC 5-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅ (AC. FT.)
OS02	0.2219	13	01Jul2015, 12:28	2.8
B01	0.2219	13	01Jul2015, 12:28	2.8
B01-B07	0.2219	13	01Jul2015, 12:34	2.8
OS03	0.1984	9	01Jul2015, 12:28	2.2
B02-B03	0.1984	9	01Jul2015, 12:32	2.2
HB01	0.0234	1	01Jul2015, 12:14	0.2
B03	0.2218	10	01Jul2015, 12:32	2.4
B03-B07	0.2218	10	01Jul2015, 12:36	2.4
OS04	0.1359	4	01Jul2015, 12:28	1.2
B04-B05	0.1359	4	01Jul2015, 12:44	1.1
HB03	0.1266	5	01Jul2015, 12:14	1.1
B05	0.2625	7	01Jul2015, 12:42	2.2
B05-B07	0.2625	7	01Jul2015, 12:44	2.2
HB02	0.1063	4	01Jul2015, 12:20	0.9
HB04	0.0609	2	01Jul2015, 12:16	0.5
B07	0.8734	33	01Jul2015, 12:38	8.9
B07-B12	0.8734	33	01Jul2015, 12:48	8.7
HB05	0.1375	5	01Jul2015, 12:18	1.2
HB06	0.1641	5	01Jul2015, 12:22	1.4
B12	1.175	40	01Jul2015, 12:48	11.3
B12-PB	1.175	39	01Jul2015, 12:52	11.3
HB07	0.0313	1	01Jul2015, 12:10	0.3
POND B	1.2063	40	01Jul2015, 12:52	11.5
PB-19	1.2063	40	01Jul2015, 13:00	11.3
OS01	1.5594	55	01Jul2015, 12:46	16.6
OS01-B19	1.5594	55	01Jul2015, 12:58	16.3
HB08	0.1344	4	01Jul2015, 12:28	1.2
HB09	0.3047	7	01Jul2015, 12:46	2.6
B19	3.2048	105	01Jul2015, 13:00	31.4
B19-B26	3.2048	105	01Jul2015, 13:02	31.3
HB10	0.3047	9	01Jul2015, 12:32	2.6
HB12	0.0797	3	01Jul2015, 12:22	0.7
HB12-B26	0.0797	3	01Jul2015, 12:30	0.7
B26	3.5892	113	01Jul2015, 13:02	34.6
26-32	3.5892	113	01Jul2015, 13:08	34
HB11	0.1125	3	01Jul2015, 12:34	1
32	3.7017	115	01Jul2015, 13:08	35
32-37	3.7017	115	01Jul2015, 13:14	34.5
B-14	0.4039	10	01Jul2015, 12:48	3.4
B-13	0.2813	7	01Jul2015, 12:46	2.4
36	0.6852	17	01Jul2015, 12:46	5.8
36-37	0.6852	17	01Jul2015, 12:54	5.8
B-15	0.075	2	01Jul2015, 12:36	0.6
37	4.4619	131	01Jul2015, 13:14	40.9
HH01	0.0984	3	01Jul2015, 12:20	0.9
H12	0.0984	3	01Jul2015, 12:20	0.9

HISTORIC 2-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂ (AC. FT.)
OS02	0.2219	3.00	01 Jul 2015, 12:46	1.1
B01	0.2219	3.00	01 Jul 2015, 12:46	1.1
B01-B07	0.2219	3.00	01 Jul 2015, 12:56	1.1
OS03	0.1984	2.00	01 Jul 2015, 13:02	0.8
B02-B03	0.1984	2.00	01 Jul 2015, 13:08	0.8
HB01	0.0234	0.00	01 Jul 2015, 13:08	0.1
B03	0.2218	2.00	01 Jul 2015, 13:08	0.9
B03-B07	0.2218	2.00	01 Jul 2015, 13:16	0.8
OS04	0.1359	1.00	01 Jul 2015, 13:30	0.4
B04-B05	0.1359	1.00	01 Jul 2015, 13:58	0.3
HB03	0.1266	1.00	01 Jul 2015, 13:10	0.3
B05	0.2625	1.00	01 Jul 2015, 13:42	0.7
B05-B07	0.2625	1.00	01 Jul 2015, 13:46	0.7
HB02	0.1063	0.00	01 Jul 2015, 13:22	0.3
HB04	0.0609	0.00	01 Jul 2015, 13:16	0.2
B07	0.8734	6.00	01 Jul 2015, 13:26	3.1
B07-B12	0.8734	6.00	01 Jul 2015, 13:44	3.0
HB05	0.1375	1.00	01 Jul 2015, 13:20	0.4
HB06	0.1641	1.00	01 Jul 2015, 13:24	0.4
B12	1.1750	7.00	01 Jul 2015, 13:42	3.8
B12-PB	1.1750	7.00	01 Jul 2015, 13:46	3.8
HB07	0.0313	0.00	01 Jul 2015, 13:06	0.1
POND B	1.2063	7.00	01 Jul 2015, 13:46	3.9
PB-19	1.2063	7.00	01 Jul 2015, 14:02	3.7
OS01	1.5594	11.00	01 Jul 2015, 13:24	5.9
OS01-B19	1.5594	11.00	01 Jul 2015, 13:44	5.7
HB08	0.1344	1.00	01 Jul 2015, 13:30	0.4
HB09	0.3047	1.00	01 Jul 2015, 13:50	0.8
B19	3.2048	20.00	01 Jul 2015, 13:44	10.6
B19-B26	3.2048	20.00	01 Jul 2015, 13:48	10.6
HB10	0.3047	1.00	01 Jul 2015, 13:34	0.8
HB12	0.0797	0.00	01 Jul 2015, 13:24	0.2
HB12-B26	0.0797	0.00	01 Jul 2015, 13:38	0.2
B26	3.5892	21.00	01 Jul 2015, 13:46	11.6
26-32	3.5892	21.00	01 Jul 2015, 13:58	11.3
HB11	0.1125	0.00	01 Jul 2015, 13:38	0.3
32	3.7017	22.00	01 Jul 2015, 13:58	11.6
32-37	3.7017	22.00	01 Jul 2015, 14:10	11.3
B-14	0.4039	2.00	01 Jul 2015, 13:52	1.1
B-13	0.2813	1.00	01 Jul 2015, 13:50	0.7
36	0.6852	3.00	01 Jul 2015, 13:50	1.8
36-37	0.6852	3.00	01 Jul 2015, 14:02	1.8
B-15	0.0750	0.00	01 Jul 2015, 13:40	0.2
37	4.4619	25.00	01 Jul 2015, 14:10	13.2
HH01	0.0984	0.00	01 Jul 2015, 13:22	0.3
H12	0.0984	0.00	01 Jul 2015, 13:22	0.3

FILING 1 100-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀₀ (AC. FT.)
OS01	1.5594	757	01 Jul 2015, 12:32	122
DB16	0.0578	92	01 Jul 2015, 12:10	8
B10	1.6172	794	01 Jul 2015, 12:30	130
B10-B11	1.6172	793	01 Jul 2015, 12:32	130
DB17	0.0048	16	01 Jul 2015, 12:02	1
B11	1.6220	795	01 Jul 2015, 12:32	131
B11-POND C	1.6220	795	01 Jul 2015, 12:34	131
DB21	0.0519	54	01 Jul 2015, 12:08	5
DB18	0.0346	64	01 Jul 2015, 12:08	5
DB19	0.0281	36	01 Jul 2015, 12:10	3
DB20	0.0147	25	01 Jul 2015, 12:08	2
POND C	1.7513	749	01 Jul 2015, 12:46	141
POND C-B16	1.7513	749	01 Jul 2015, 12:48	141
DB25	0.0211	45	01 Jul 2015, 12:04	3
B16	1.7724	754	01 Jul 2015, 12:48	144
B16-B17	1.7724	754	01 Jul 2015, 12:50	144
DB26	0.0682	136	01 Jul 2015, 12:10	12
B17	1.8406	778	01 Jul 2015, 12:50	156
B17-B18	1.8406	778	01 Jul 2015, 12:52	156
OS03	0.1984	130	01 Jul 2015, 12:18	16
DB01	0.0719	90	01 Jul 2015, 12:08	8
B01	0.2703	199	01 Jul 2015, 12:14	23
B01-B02	0.2703	199	01 Jul 2015, 12:14	23
OS02	0.2219	148	01 Jul 2015, 12:20	19
DB02	0.0516	71	01 Jul 2015, 12:06	5
B02	0.5438	380	01 Jul 2015, 12:14	48
B02-POND A	0.5438	379	01 Jul 2015, 12:16	47
OS04	0.1359	83	01 Jul 2015, 12:16	10
DB03	0.0703	70	01 Jul 2015, 12:10	6
B03	0.2062	145	01 Jul 2015, 12:12	16
B03-B04	0.2062	145	01 Jul 2015, 12:18	16
DB04	0.0422	44	01 Jul 2015, 12:10	4
DB05	0.0384	37	01 Jul 2015, 12:14	4
B04	0.2868	218	01 Jul 2015, 12:16	24
B04-B05	0.2868	218	01 Jul 2015, 12:16	24
DB06	0.0219	44	01 Jul 2015, 12:08	4
B05	0.3087	253	01 Jul 2015, 12:14	28
B05-POND A	0.3087	252	01 Jul 2015, 12:16	27
DB07	0.0254	35	01 Jul 2015, 12:06	3
DB08	0.0297	32	01 Jul 2015, 12:06	3
POND A	0.9076	557	01 Jul 2015, 12:26	77
POND A-B06	0.9076	557	01 Jul 2015, 12:26	77
DB09	0.0189	34	01 Jul 2015, 12:04	2
B06	0.9265	565	01 Jul 2015, 12:26	80

FILING 1 100-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀₀ (AC. FT.)
B06-B07	0.9265	564	01 Jul2015, 12:30	79
DB11	0.0969	114	01 Jul2015, 12:12	11
DB10	0.0364	56	01 Jul2015, 12:08	5
B07	1.0598	652	01 Jul2015, 12:26	95
B07-B09	1.0598	651	01 Jul2015, 12:28	95
DB12	0.0453	81	01 Jul2015, 12:06	7
B09	1.1051	677	01 Jul2015, 12:26	101
B09-POND B	1.1051	676	01 Jul2015, 12:28	101
DB15	0.1234	105	01 Jul2015, 12:16	12
DB13	0.0703	89	01 Jul2015, 12:12	9
DB14	0.0556	93	01 Jul2015, 12:08	8
POND B	1.3544	688	01 Jul2015, 12:42	129
POND B-B12	1.3544	688	01 Jul2015, 12:44	128
DB22	0.0516	91	01 Jul2015, 12:08	8
DB23	0.0172	45	01 Jul2015, 12:04	4
B12	1.4232	714	01 Jul2015, 12:36	140
B12-B14	1.4232	714	01 Jul2015, 12:38	140
DB24	0.0531	94	01 Jul2015, 12:08	8
B14	1.4763	743	01 Jul2015, 12:28	147
B14-B15	1.4763	742	01 Jul2015, 12:28	147
DB28	0.0741	67	01 Jul2015, 12:18	8
B15	1.5504	800	01 Jul2015, 12:28	155
B15-B18	1.5504	798	01 Jul2015, 12:34	154
DB29	0.1697	146	01 Jul2015, 12:18	17
DB27	0.0508	68	01 Jul2015, 12:16	7
B26	3.6115	1623	01 Jul2015, 12:46	334
B26-27	3.6115	1622	01 Jul2015, 12:48	333
FB-02	0.0500	86	01 Jul2015, 12:08	7
FB-01	0.0373	41	01 Jul2015, 12:08	4
FB01-27a	0.0373	41	01 Jul2015, 12:08	4
B19	0.0873	127	01 Jul2015, 12:08	11
B19-27	0.0873	126	01 Jul2015, 12:10	11
FB-03	0.0078	23	01 Jul2015, 12:02	2
27	3.7066	1648	01 Jul2015, 12:48	345
27-32	3.7066	1648	01 Jul2015, 12:48	345
WH-24	0.1325	218	01 Jul2015, 12:10	20
WH-26	0.0839	49	01 Jul2015, 12:20	6
WH-27	0.0217	23	01 Jul2015, 12:04	2
30	0.2381	271	01 Jul2015, 12:10	28
30-31	0.2381	270	01 Jul2015, 12:12	28
WH-28	0.0398	60	01 Jul2015, 12:12	6
31	0.2779	330	01 Jul2015, 12:12	33
31-32	0.2779	329	01 Jul2015, 12:14	33
WH-29	0.0495	77	01 Jul2015, 12:10	7

FILING 1 100-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀₀ (AC. FT.)
WH-31	0.0406	75	01 Jul 2015, 12:06	6
WH-30	0.0159	26	01 Jul 2015, 12:02	2
32	4.0905	1790	01 Jul 2015, 12:40	393
WH32	0.0458	55	01 Jul 2015, 12:02	4
BEN POND	4.1363	1393	01 Jul 2015, 13:18	377
WH-33	0.0064	12	01 Jul 2015, 12:06	1
33	4.1427	1394	01 Jul 2015, 13:18	377
33-37	4.1427	1394	01 Jul 2015, 13:20	376
WH35	0.1550	171	01 Jul 2015, 12:10	15
WH34	0.0450	68	01 Jul 2015, 12:08	6
B34-36	0.0450	68	01 Jul 2015, 12:10	6
36	0.2000	239	01 Jul 2015, 12:10	21
36-37	0.2000	238	01 Jul 2015, 12:12	21
WH36	0.0750	63	01 Jul 2015, 12:10	6
37	4.4177	1432	01 Jul 2015, 13:20	403
FH01	0.1344	196	01 Jul 2015, 12:06	16
POND H	0.1344	43	01 Jul 2015, 12:36	13
FH02	0.0138	19	01 Jul 2015, 12:08	2
H12	0.1482	48	01 Jul 2015, 12:32	15

FILING 1 50-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅₀ (AC. FT.)
OS01	1.5594	510	01Jul2015, 12:34	87
DB16	0.0578	72	01Jul2015, 12:10	7
B10	1.6172	537	01Jul2015, 12:32	93
B10-B11	1.6172	537	01Jul2015, 12:32	93
DB17	0.0048	13	01Jul2015, 12:02	1
B11	1.6220	538	01Jul2015, 12:32	94
B11-POND C	1.6220	538	01Jul2015, 12:36	94
DB21	0.0519	38	01Jul2015, 12:08	3
DB18	0.0346	50	01Jul2015, 12:08	4
DB19	0.0281	27	01Jul2015, 12:10	3
DB20	0.0147	19	01Jul2015, 12:08	2
POND C	1.7513	507	01Jul2015, 12:48	101
POND C-B16	1.7513	507	01Jul2015, 12:50	101
DB25	0.0211	35	01Jul2015, 12:04	3
B16	1.7724	511	01Jul2015, 12:50	103
B16-B17	1.7724	510	01Jul2015, 12:54	103
DB26	0.0682	110	01Jul2015, 12:10	10
B17	1.8406	529	01Jul2015, 12:52	113
B17-B18	1.8406	529	01Jul2015, 12:54	113
OS03	0.1984	88	01Jul2015, 12:20	11
DB01	0.0719	66	01Jul2015, 12:08	6
B01	0.2703	139	01Jul2015, 12:14	17
B01-B02	0.2703	138	01Jul2015, 12:16	17
OS02	0.2219	102	01Jul2015, 12:22	14
DB02	0.0516	52	01Jul2015, 12:06	4
B02	0.5438	263	01Jul2015, 12:16	34
B02-POND A	0.5438	263	01Jul2015, 12:16	34
OS04	0.1359	54	01Jul2015, 12:18	7
DB03	0.0703	49	01Jul2015, 12:10	5
B03	0.2062	98	01Jul2015, 12:14	11
B03-B04	0.2062	98	01Jul2015, 12:18	11
DB04	0.0422	31	01Jul2015, 12:10	3
DB05	0.0384	27	01Jul2015, 12:14	3
B04	0.2868	150	01Jul2015, 12:16	17
B04-B05	0.2868	149	01Jul2015, 12:16	17
DB06	0.0219	35	01Jul2015, 12:08	3
B05	0.3087	176	01Jul2015, 12:16	20
B05-POND A	0.3087	176	01Jul2015, 12:16	20
DB07	0.0254	26	01Jul2015, 12:06	2
DB08	0.0297	22	01Jul2015, 12:06	2
POND A	0.9076	401	01Jul2015, 12:26	56
POND A-B06	0.9076	400	01Jul2015, 12:26	55
DB09	0.0189	26	01Jul2015, 12:04	2
B06	0.9265	407	01Jul2015, 12:26	57

FILING 1 50-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅₀ (AC. FT.)
B06-B07	0.9265	406	01Jul2015, 12:30	57
DB11	0.0969	85	01Jul2015, 12:12	9
DB10	0.0364	43	01Jul2015, 12:08	4
B07	1.0598	469	01Jul2015, 12:28	69
B07-B09	1.0598	468	01Jul2015, 12:30	69
DB12	0.0453	63	01Jul2015, 12:06	5
B09	1.1051	486	01Jul2015, 12:30	74
B09-POND B	1.1051	485	01Jul2015, 12:30	74
DB15	0.1234	75	01Jul2015, 12:16	9
DB13	0.0703	67	01Jul2015, 12:12	7
DB14	0.0556	72	01Jul2015, 12:08	6
POND B	1.3544	539	01Jul2015, 12:38	95
POND B-B12	1.3544	539	01Jul2015, 12:40	94
DB22	0.0516	72	01Jul2015, 12:08	6
DB23	0.0172	38	01Jul2015, 12:04	3
B12	1.4232	562	01Jul2015, 12:38	104
B12-B14	1.4232	562	01Jul2015, 12:40	103
DB24	0.0531	73	01Jul2015, 12:08	6
B14	1.4763	577	01Jul2015, 12:40	109
B14-B15	1.4763	576	01Jul2015, 12:40	109
DB28	0.0741	49	01Jul2015, 12:20	6
B15	1.5504	605	01Jul2015, 12:38	115
B15-B18	1.5504	605	01Jul2015, 12:44	114
DB29	0.1697	105	01Jul2015, 12:18	13
DB27	0.0508	53	01Jul2015, 12:16	6
B26	3.6115	1178	01Jul2015, 12:48	245
B26-27	3.6115	1178	01Jul2015, 12:50	244
FB-02	0.0500	68	01Jul2015, 12:08	6
FB-01	0.0373	29	01Jul2015, 12:08	3
FB01-27a	0.0373	29	01Jul2015, 12:10	3
B19	0.0873	97	01Jul2015, 12:08	8
B19-27	0.0873	96	01Jul2015, 12:10	8
FB-03	0.0078	19	01Jul2015, 12:02	1
27	3.7066	1197	01Jul2015, 12:50	254
27-32	3.7066	1196	01Jul2015, 12:52	253
WH-24	0.1325	171	01Jul2015, 12:10	15
WH-26	0.0839	33	01Jul2015, 12:22	5
WH-27	0.0217	16	01Jul2015, 12:04	1
30	0.2381	205	01Jul2015, 12:10	21
30-31	0.2381	205	01Jul2015, 12:12	21
WH-28	0.0398	47	01Jul2015, 12:12	5
31	0.2779	252	01Jul2015, 12:12	25
31-32	0.2779	251	01Jul2015, 12:14	25
WH-29	0.0495	60	01Jul2015, 12:10	6

FILING 1 50-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅₀ (AC. FT.)
WH-31	0.0406	60	01Jul2015, 12:08	5
WH-30	0.0159	19	01Jul2015, 12:02	1
32	4.0905	1290	01Jul2015, 12:50	290
WH32	0.0458	38	01Jul2015, 12:02	3
BEN POND	4.1363	984	01Jul2015, 13:16	276
WH-33	0.0064	9	01Jul2015, 12:06	1
33	4.1427	985	01Jul2015, 13:16	276
33-37	4.1427	984	01Jul2015, 13:20	275
WH35	0.1550	124	01Jul2015, 12:10	11
WH34	0.0450	52	01Jul2015, 12:08	5
B34-36	0.0450	52	01Jul2015, 12:10	5
36	0.2000	176	01Jul2015, 12:10	16
36-37	0.2000	174	01Jul2015, 12:14	16
WH36	0.0750	43	01Jul2015, 12:10	4
37	4.4177	1014	01Jul2015, 13:20	295
FH01	0.1344	147	01Jul2015, 12:06	12
POND H	0.1344	23	01Jul2015, 12:52	10
FH02	0.0138	14	01Jul2015, 12:08	1
H12	0.1482	26	01Jul2015, 12:34	11

FILING 1 25-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂₅ (AC. FT.)
OS01	1.5594	316	01Jul2015, 12:36	58.6
DB16	0.0578	54	01Jul2015, 12:10	5.0
B10	1.6172	335	01Jul2015, 12:34	63.6
B10-B11	1.6172	335	01Jul2015, 12:34	63.5
DB17	0.0048	11	01Jul2015, 12:02	0.9
B11	1.6220	336	01Jul2015, 12:34	64.4
B11-POND C	1.6220	336	01Jul2015, 12:38	64.0
DB21	0.0519	25	01Jul2015, 12:08	2.3
DB18	0.0346	39	01Jul2015, 12:08	3.2
DB19	0.0281	20	01Jul2015, 12:10	1.9
DB20	0.0147	15	01Jul2015, 12:10	1.3
POND C	1.7513	310	01Jul2015, 12:52	68.6
POND C-B16	1.7513	310	01Jul2015, 12:54	68.4
DB25	0.0211	27	01Jul2015, 12:04	2.0
B16	1.7724	313	01Jul2015, 12:54	70.3
B16-B17	1.7724	312	01Jul2015, 12:58	69.9
DB26	0.0682	88	01Jul2015, 12:10	8.0
B17	1.8406	326	01Jul2015, 12:58	77.9
B17-B18	1.8406	326	01Jul2015, 13:00	77.5
OS03	0.1984	55	01Jul2015, 12:20	7.7
DB01	0.0719	46	01Jul2015, 12:08	4.1
B01	0.2703	89	01Jul2015, 12:14	11.7
B01-B02	0.2703	89	01Jul2015, 12:16	11.7
OS02	0.2219	65	01Jul2015, 12:22	9.3
DB02	0.0516	36	01Jul2015, 12:06	2.8
B02	0.5438	169	01Jul2015, 12:16	23.8
B02-POND A	0.5438	169	01Jul2015, 12:18	23.8
OS04	0.1359	32	01Jul2015, 12:18	4.5
DB03	0.0703	32	01Jul2015, 12:10	3.2
B03	0.2062	61	01Jul2015, 12:14	7.7
B03-B04	0.2062	60	01Jul2015, 12:20	7.6
DB04	0.0422	21	01Jul2015, 12:10	2.0
DB05	0.0384	18	01Jul2015, 12:14	2.0
B04	0.2868	94	01Jul2015, 12:18	11.7
B04-B05	0.2868	94	01Jul2015, 12:18	11.7
DB06	0.0219	28	01Jul2015, 12:08	2.4
B05	0.3087	115	01Jul2015, 12:16	14.0
B05-POND A	0.3087	114	01Jul2015, 12:18	14.0
DB07	0.0254	18	01Jul2015, 12:06	1.5
DB08	0.0297	15	01Jul2015, 12:08	1.3
POND A	0.9076	244	01Jul2015, 12:28	37.8
POND A-B06	0.9076	244	01Jul2015, 12:30	37.8
DB09	0.0189	19	01Jul2015, 12:04	1.4
B06	0.9265	248	01Jul2015, 12:30	39.2

FILING 1 25-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂₅ (AC. FT.)
B06-B07	0.9265	248	01Jul2015, 12:34	38.8
DB11	0.0969	60	01Jul2015, 12:12	6.2
DB10	0.0364	32	01Jul2015, 12:08	2.7
B07	1.0598	286	01Jul2015, 12:32	47.7
B07-B09	1.0598	285	01Jul2015, 12:36	47.4
DB12	0.0453	48	01Jul2015, 12:06	3.9
B09	1.1051	296	01Jul2015, 12:36	51.3
B09-POND B	1.1051	296	01Jul2015, 12:36	51.2
DB15	0.1234	50	01Jul2015, 12:18	6.1
DB13	0.0703	49	01Jul2015, 12:12	4.9
DB14	0.0556	54	01Jul2015, 12:08	4.7
POND B	1.3544	337	01Jul2015, 12:42	66.5
POND B-B12	1.3544	336	01Jul2015, 12:44	66.3
DB22	0.0516	55	01Jul2015, 12:08	4.8
DB23	0.0172	31	01Jul2015, 12:04	2.5
B12	1.4232	353	01Jul2015, 12:42	73.6
B12-B14	1.4232	352	01Jul2015, 12:44	73.4
DB24	0.0531	56	01Jul2015, 12:08	4.6
B14	1.4763	363	01Jul2015, 12:44	78.0
B14-B15	1.4763	362	01Jul2015, 12:46	77.9
DB28	0.0741	34	01Jul2015, 12:20	4.3
B15	1.5504	380	01Jul2015, 12:44	82.1
B15-B18	1.5504	379	01Jul2015, 12:50	81.2
DB29	0.1697	71	01Jul2015, 12:20	9.0
DB27	0.0508	40	01Jul2015, 12:16	4.3
B26	3.6115	735	01Jul2015, 12:54	172.0
B26-27	3.6115	735	01Jul2015, 12:56	171.3
FB-02	0.0500	51	01Jul2015, 12:08	4.4
FB-01	0.0373	20	01Jul2015, 12:08	1.8
FB01-27a	0.0373	20	01Jul2015, 12:10	1.8
B19	0.0873	71	01Jul2015, 12:08	6.2
B19-27	0.0873	70	01Jul2015, 12:10	6.2
FB-03	0.0078	15	01Jul2015, 12:02	1.1
27	3.7066	749	01Jul2015, 12:56	178.6
27-32	3.7066	748	01Jul2015, 12:58	178.2
WH-24	0.1325	129	01Jul2015, 12:10	11.7
WH-26	0.0839	20	01Jul2015, 12:22	3.0
WH-27	0.0217	10	01Jul2015, 12:04	0.8
30	0.2381	150	01Jul2015, 12:10	15.5
30-31	0.2381	149	01Jul2015, 12:12	15.5
WH-28	0.0398	36	01Jul2015, 12:12	3.4
31	0.2779	185	01Jul2015, 12:12	18.9
31-32	0.2779	185	01Jul2015, 12:14	18.8
WH-29	0.0495	45	01Jul2015, 12:10	4.2

FILING 1 25-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂₅ (AC. FT.)
WH-31	0.0406	46	01Jul2015, 12:08	3.8
WH-30	0.0159	13	01Jul2015, 12:02	0.9
32	4.0905	809	01Jul2015, 12:56	205.8
WH32	0.0458	24	01Jul2015, 12:02	1.6
BEN POND	4.1363	595	01Jul2015, 13:26	194.5
WH-33	0.0064	7	01Jul2015, 12:08	0.6
33	4.1427	596	01Jul2015, 13:26	195.1
33-37	4.1427	595	01Jul2015, 13:32	193.6
WH35	0.1550	84	01Jul2015, 12:10	8.0
WH34	0.0450	38	01Jul2015, 12:08	3.3
B34-36	0.0450	38	01Jul2015, 12:10	3.3
36	0.2000	122	01Jul2015, 12:10	11.4
36-37	0.2000	121	01Jul2015, 12:14	11.3
WH36	0.0750	27	01Jul2015, 12:12	2.9
37	4.4177	615	01Jul2015, 13:30	207.8
FH01	0.1344	106	01Jul2015, 12:08	8.8
POND H	0.1344	13	01Jul2015, 13:12	7.0
FH02	0.0138	10	01Jul2015, 12:10	0.9
H12	0.1482	15	01Jul2015, 12:16	7.9

FILING 1 10-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀ (AC. FT.)
OS01	1.5594	136	01 Jul 2015, 12:38	30.9
DB16	0.0578	35	01 Jul 2015, 12:10	3.3
B10	1.6172	147	01 Jul 2015, 12:36	34.2
B10-B11	1.6172	147	01 Jul 2015, 12:38	34.1
DB17	0.0048	9	01 Jul 2015, 12:02	0.7
B11	1.6220	148	01 Jul 2015, 12:38	34.8
B11-POND C	1.6220	148	01 Jul 2015, 12:42	34.5
DB21	0.0519	12	01 Jul 2015, 12:10	1.3
DB18	0.0346	26	01 Jul 2015, 12:08	2.1
DB19	0.0281	11	01 Jul 2015, 12:12	1.1
DB20	0.0147	9	01 Jul 2015, 12:10	0.8
POND C	1.7513	129	01 Jul 2015, 13:02	36.3
POND C-B16	1.7513	128	01 Jul 2015, 13:06	36.1
DB25	0.0211	18	01 Jul 2015, 12:04	1.3
B16	1.7724	130	01 Jul 2015, 13:06	37.4
B16-B17	1.7724	130	01 Jul 2015, 13:10	37.1
DB26	0.0682	62	01 Jul 2015, 12:10	5.6
B17	1.8406	138	01 Jul 2015, 13:08	42.7
B17-B18	1.8406	138	01 Jul 2015, 13:12	42.4
OS03	0.1984	24	01 Jul 2015, 12:24	4.1
DB01	0.0719	25	01 Jul 2015, 12:10	2.4
B01	0.2703	42	01 Jul 2015, 12:14	6.5
B01-B02	0.2703	42	01 Jul 2015, 12:16	6.5
OS02	0.2219	30	01 Jul 2015, 12:26	5.1
DB02	0.0516	20	01 Jul 2015, 12:06	1.7
B02	0.5438	79	01 Jul 2015, 12:18	13.2
B02-POND A	0.5438	79	01 Jul 2015, 12:20	13.1
OS04	0.1359	12	01 Jul 2015, 12:22	2.3
DB03	0.0703	16	01 Jul 2015, 12:12	1.8
B03	0.2062	26	01 Jul 2015, 12:16	4.1
B03-B04	0.2062	26	01 Jul 2015, 12:22	4.0
DB04	0.0422	10	01 Jul 2015, 12:12	1.2
DB05	0.0384	9	01 Jul 2015, 12:16	1.1
B04	0.2868	42	01 Jul 2015, 12:20	6.3
B04-B05	0.2868	42	01 Jul 2015, 12:20	6.3
DB06	0.0219	19	01 Jul 2015, 12:08	1.6
B05	0.3087	55	01 Jul 2015, 12:18	8.0
B05-POND A	0.3087	55	01 Jul 2015, 12:20	8.0
DB07	0.0254	10	01 Jul 2015, 12:08	0.9
DB08	0.0297	7	01 Jul 2015, 12:08	0.7
POND A	0.9076	98	01 Jul 2015, 12:38	20.1
POND A-B06	0.9076	98	01 Jul 2015, 12:38	20.1
DB09	0.0189	12	01 Jul 2015, 12:04	0.9
B06	0.9265	100	01 Jul 2015, 12:38	21.0

FILING 1 10-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀ (AC. FT.)
B06-B07	0.9265	99	01 Jul 2015, 12:46	20.7
DB11	0.0969	35	01 Jul 2015, 12:14	3.8
DB10	0.0364	19	01 Jul 2015, 12:08	1.7
B07	1.0598	116	01 Jul 2015, 12:44	26.2
B07-B09	1.0598	116	01 Jul 2015, 12:48	26.0
DB12	0.0453	31	01 Jul 2015, 12:08	2.5
B09	1.1051	121	01 Jul 2015, 12:48	28.5
B09-POND B	1.1051	121	01 Jul 2015, 12:48	28.5
DB15	0.1234	25	01 Jul 2015, 12:20	3.4
DB13	0.0703	29	01 Jul 2015, 12:14	3.1
DB14	0.0556	35	01 Jul 2015, 12:08	3.1
POND B	1.3544	140	01 Jul 2015, 12:56	37.8
POND B-B12	1.3544	140	01 Jul 2015, 12:58	37.7
DB22	0.0516	36	01 Jul 2015, 12:08	3.2
DB23	0.0172	23	01 Jul 2015, 12:06	1.8
B12	1.4232	148	01 Jul 2015, 12:26	42.7
B12-B14	1.4232	148	01 Jul 2015, 12:28	42.5
DB24	0.0531	36	01 Jul 2015, 12:08	3.0
B14	1.4763	162	01 Jul 2015, 12:26	45.6
B14-B15	1.4763	162	01 Jul 2015, 12:28	45.5
DB28	0.0741	18	01 Jul 2015, 12:22	2.5
B15	1.5504	179	01 Jul 2015, 12:28	48.0
B15-B18	1.5504	178	01 Jul 2015, 12:36	47.3
DB29	0.1697	37	01 Jul 2015, 12:20	5.2
DB27	0.0508	25	01 Jul 2015, 12:16	2.8
B26	3.6115	315	01 Jul 2015, 13:08	97.8
B26-27	3.6115	315	01 Jul 2015, 13:12	97.2
FB-02	0.0500	33	01 Jul 2015, 12:08	2.9
FB-01	0.0373	10	01 Jul 2015, 12:10	1.0
FB01-27a	0.0373	10	01 Jul 2015, 12:10	1.0
B19	0.0873	43	01 Jul 2015, 12:10	3.9
B19-27	0.0873	43	01 Jul 2015, 12:10	3.9
FB-03	0.0078	11	01 Jul 2015, 12:02	0.8
27	3.7066	322	01 Jul 2015, 13:12	101.9
27-32	3.7066	322	01 Jul 2015, 13:14	101.6
WH-24	0.1325	84	01 Jul 2015, 12:10	7.7
WH-26	0.0839	8	01 Jul 2015, 12:26	1.5
WH-27	0.0217	4	01 Jul 2015, 12:06	0.4
30	0.2381	91	01 Jul 2015, 12:10	9.7
30-31	0.2381	91	01 Jul 2015, 12:12	9.7
WH-28	0.0398	23	01 Jul 2015, 12:12	2.2
31	0.2779	114	01 Jul 2015, 12:12	11.9
31-32	0.2779	113	01 Jul 2015, 12:14	11.9
WH-29	0.0495	29	01 Jul 2015, 12:10	2.7

FILING 1 10-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀ (AC. FT.)
WH-31	0.0406	30	01 Jul 2015, 12:08	2.5
WH-30	0.0159	7	01 Jul 2015, 12:02	0.5
32	4.0905	419	01 Jul 2015, 12:24	119.2
WH32	0.0458	10	01 Jul 2015, 12:04	0.9
BEN POND	4.1363	252	01 Jul 2015, 13:52	111.9
WH-33	0.0064	5	01 Jul 2015, 12:08	0.4
33	4.1427	252	01 Jul 2015, 13:52	112.3
33-37	4.1427	252	01 Jul 2015, 13:58	111.2
WH35	0.1550	44	01 Jul 2015, 12:10	4.6
WH34	0.0450	23	01 Jul 2015, 12:10	2.1
B34-36	0.0450	23	01 Jul 2015, 12:12	2.1
36	0.2000	67	01 Jul 2015, 12:12	6.7
36-37	0.2000	66	01 Jul 2015, 12:16	6.7
WH36	0.0750	11	01 Jul 2015, 12:14	1.5
37	4.4177	262	01 Jul 2015, 13:58	119.4
FH01	0.1344	62	01 Jul 2015, 12:08	5.4
POND H	0.1344	5	01 Jul 2015, 14:02	4.1
FH02	0.0138	6	01 Jul 2015, 12:10	0.6
H12	0.1482	8	01 Jul 2015, 12:12	4.6

FILING 1 5-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅ (AC. FT.)
OS01	1.5594	55	01 Jul 2015, 12:46	16.6
DB16	0.0578	23	01 Jul 2015, 12:12	2.3
B10	1.6172	62	01 Jul 2015, 12:42	18.9
B10-B11	1.6172	62	01 Jul 2015, 12:42	18.9
DB17	0.0048	7	01 Jul 2015, 12:02	0.6
B11	1.6220	63	01 Jul 2015, 12:42	19.4
B11-POND C	1.6220	63	01 Jul 2015, 12:48	19.2
DB21	0.0519	5	01 Jul 2015, 12:12	0.7
DB18	0.0346	18	01 Jul 2015, 12:08	1.5
DB19	0.0281	7	01 Jul 2015, 12:12	0.7
DB20	0.0147	6	01 Jul 2015, 12:10	0.6
POND C	1.7513	50	01 Jul 2015, 13:28	19.4
POND C-B16	1.7513	50	01 Jul 2015, 13:32	19.2
DB25	0.0211	12	01 Jul 2015, 12:04	0.9
B16	1.7724	51	01 Jul 2015, 13:32	20.1
B16-B17	1.7724	51	01 Jul 2015, 13:36	19.9
DB26	0.0682	46	01 Jul 2015, 12:10	4.1
B17	1.8406	56	01 Jul 2015, 12:12	24.0
B17-B18	1.8406	56	01 Jul 2015, 12:16	23.8
OS03	0.1984	9	01 Jul 2015, 12:28	2.2
DB01	0.0719	14	01 Jul 2015, 12:10	1.5
B01	0.2703	19	01 Jul 2015, 12:14	3.7
B01-B02	0.2703	19	01 Jul 2015, 12:18	3.7
OS02	0.2219	13	01 Jul 2015, 12:28	2.8
DB02	0.0516	10	01 Jul 2015, 12:06	1.0
B02	0.5438	36	01 Jul 2015, 12:18	7.6
B02-POND A	0.5438	36	01 Jul 2015, 12:22	7.5
OS04	0.1359	4	01 Jul 2015, 12:28	1.2
DB03	0.0703	7	01 Jul 2015, 12:14	1.0
B03	0.2062	10	01 Jul 2015, 12:16	2.2
B03-B04	0.2062	10	01 Jul 2015, 12:26	2.2
DB04	0.0422	5	01 Jul 2015, 12:14	0.7
DB05	0.0384	5	01 Jul 2015, 12:18	0.7
B04	0.2868	18	01 Jul 2015, 12:22	3.5
B04-B05	0.2868	18	01 Jul 2015, 12:24	3.5
DB06	0.0219	14	01 Jul 2015, 12:08	1.2
B05	0.3087	26	01 Jul 2015, 12:22	4.7
B05-POND A	0.3087	26	01 Jul 2015, 12:22	4.7
DB07	0.0254	6	01 Jul 2015, 12:08	0.5
DB08	0.0297	3	01 Jul 2015, 12:10	0.4
POND A	0.9076	34	01 Jul 2015, 12:58	10.7
POND A-B06	0.9076	34	01 Jul 2015, 13:00	10.7
DB09	0.0189	8	01 Jul 2015, 12:06	0.6
B06	0.9265	35	01 Jul 2015, 13:00	11.3

FILING 1 5-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅ (AC. FT.)
B06-B07	0.9265	35	01 Jul 2015, 13:08	11.1
DB11	0.0969	20	01 Jul 2015, 12:14	2.4
DB10	0.0364	12	01 Jul 2015, 12:08	1.1
B07	1.0598	42	01 Jul 2015, 13:06	14.7
B07-B09	1.0598	42	01 Jul 2015, 13:12	14.5
DB12	0.0453	21	01 Jul 2015, 12:08	1.7
B09	1.1051	45	01 Jul 2015, 12:16	16.2
B09-POND B	1.1051	45	01 Jul 2015, 12:18	16.2
DB15	0.1234	12	01 Jul 2015, 12:22	2.0
DB13	0.0703	18	01 Jul 2015, 12:14	2.0
DB14	0.0556	23	01 Jul 2015, 12:10	2.1
POND B	1.3544	69	01 Jul 2015, 12:30	22.2
POND B-B12	1.3544	69	01 Jul 2015, 12:32	22.1
DB22	0.0516	25	01 Jul 2015, 12:10	2.2
DB23	0.0172	18	01 Jul 2015, 12:06	1.4
B12	1.4232	83	01 Jul 2015, 12:28	25.7
B12-B14	1.4232	83	01 Jul 2015, 12:30	25.6
DB24	0.0531	24	01 Jul 2015, 12:08	2.1
B14	1.4763	92	01 Jul 2015, 12:26	27.7
B14-B15	1.4763	92	01 Jul 2015, 12:28	27.6
DB28	0.0741	10	01 Jul 2015, 12:24	1.6
B15	1.5504	102	01 Jul 2015, 12:26	29.2
B15-B18	1.5504	102	01 Jul 2015, 12:36	28.7
DB29	0.1697	19	01 Jul 2015, 12:24	3.2
DB27	0.0508	17	01 Jul 2015, 12:16	1.9
B26	3.6115	174	01 Jul 2015, 12:24	57.6
B26-27	3.6115	173	01 Jul 2015, 12:28	57.2
FB-02	0.0500	23	01 Jul 2015, 12:10	2.0
FB-01	0.0373	5	01 Jul 2015, 12:10	0.6
FB01-27a	0.0373	5	01 Jul 2015, 12:12	0.6
B19	0.0873	27	01 Jul 2015, 12:10	2.6
B19-27	0.0873	27	01 Jul 2015, 12:12	2.6
FB-03	0.0078	9	01 Jul 2015, 12:02	0.6
27	3.7066	189	01 Jul 2015, 12:26	60.4
27-32	3.7066	188	01 Jul 2015, 12:28	60.1
WH-24	0.1325	57	01 Jul 2015, 12:10	5.4
WH-26	0.0839	3	01 Jul 2015, 12:32	0.8
WH-27	0.0217	1	01 Jul 2015, 12:08	0.2
30	0.2381	59	01 Jul 2015, 12:10	6.4
30-31	0.2381	59	01 Jul 2015, 12:12	6.4
WH-28	0.0398	15	01 Jul 2015, 12:12	1.5
31	0.2779	74	01 Jul 2015, 12:12	7.9
31-32	0.2779	74	01 Jul 2015, 12:16	7.9
WH-29	0.0495	19	01 Jul 2015, 12:12	1.9

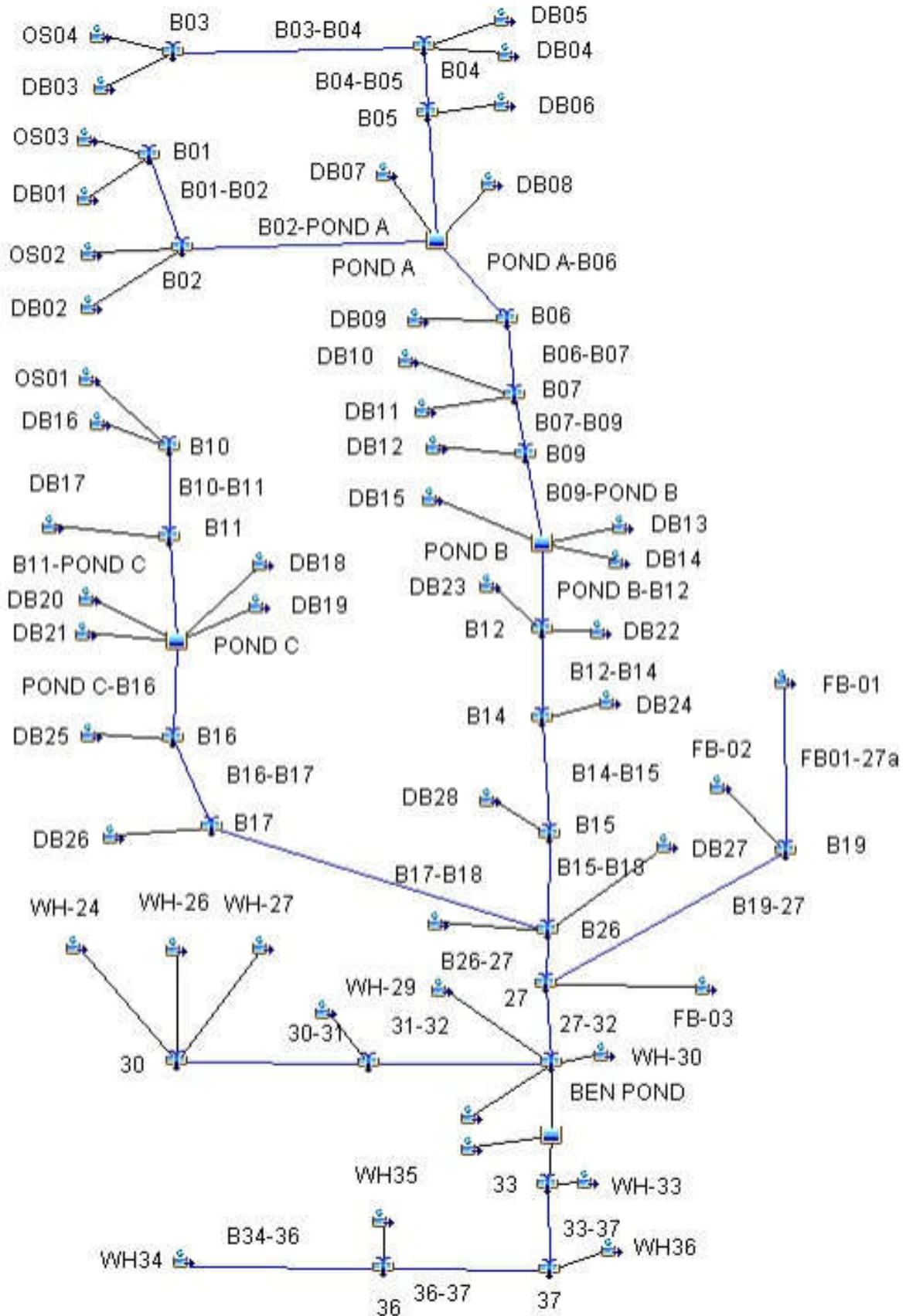
FILING 1 5-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅ (AC. FT.)
WH-31	0.0406	21	01 Jul 2015, 12:08	1.8
WH-30	0.0159	4	01 Jul 2015, 12:02	0.3
32	4.0905	257	01 Jul 2015, 12:28	72.0
WH32	0.0458	4	01 Jul 2015, 12:04	0.5
BEN POND	4.1363	99	01 Jul 2015, 14:52	66.1
WH-33	0.0064	3	01 Jul 2015, 12:08	0.3
33	4.1427	100	01 Jul 2015, 14:52	66.4
33-37	4.1427	100	01 Jul 2015, 15:00	65.6
WH35	0.1550	22	01 Jul 2015, 12:12	2.8
WH34	0.0450	15	01 Jul 2015, 12:10	1.4
B34-36	0.0450	15	01 Jul 2015, 12:12	1.4
36	0.2000	37	01 Jul 2015, 12:12	4.2
36-37	0.2000	37	01 Jul 2015, 12:18	4.1
WH36	0.0750	4	01 Jul 2015, 12:16	0.8
37	4.4177	104	01 Jul 2015, 14:50	70.6
FH01	0.1344	37	01 Jul 2015, 12:08	3.5
POND H	0.1344	3	01 Jul 2015, 14:36	2.9
FH02	0.0138	4	01 Jul 2015, 12:10	0.4
H12	0.1482	5	01 Jul 2015, 12:14	3.3

FILING 1 2-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂ (AC. FT.)
OS01	1.5594	11.0	01Jul2015, 13:24	5.9
DB16	0.0578	12.7	01Jul2015, 12:12	1.3
B10	1.6172	13.2	01Jul2015, 12:12	7.3
B10-B11	1.6172	13.2	01Jul2015, 12:14	7.2
DB17	0.0048	5.7	01Jul2015, 12:02	0.4
B11	1.6220	15.3	01Jul2015, 12:12	7.7
B11-POND C	1.6220	15.2	01Jul2015, 12:20	7.5
DB21	0.0519	1.1	01Jul2015, 12:18	0.3
DB18	0.0346	10.0	01Jul2015, 12:08	0.9
DB19	0.0281	2.8	01Jul2015, 12:14	0.4
DB20	0.0147	3.4	01Jul2015, 12:10	0.3
POND C	1.7513	10.9	01Jul2015, 15:00	6.3
POND C-B16	1.7513	10.9	01Jul2015, 15:06	6.2
DB25	0.0211	7.1	01Jul2015, 12:06	0.6
B16	1.7724	11.3	01Jul2015, 15:06	6.7
B16-B17	1.7724	11.3	01Jul2015, 15:16	6.6
DB26	0.0682	29.4	01Jul2015, 12:10	2.7
B17	1.8406	34.4	01Jul2015, 12:14	9.3
B17-B18	1.8406	34.0	01Jul2015, 12:20	9.1
OS03	0.1984	1.6	01Jul2015, 13:02	0.8
DB01	0.0719	4.7	01Jul2015, 12:12	0.7
B01	0.2703	5.0	01Jul2015, 12:14	1.5
B01-B02	0.2703	5.0	01Jul2015, 12:18	1.5
OS02	0.2219	2.6	01Jul2015, 12:46	1.1
DB02	0.0516	3.4	01Jul2015, 12:08	0.5
B02	0.5438	8.6	01Jul2015, 12:18	3.1
B02-POND A	0.5438	8.6	01Jul2015, 12:22	3.1
OS04	0.1359	0.6	01Jul2015, 13:30	0.4
DB03	0.0703	1.5	01Jul2015, 12:20	0.4
B03	0.2062	1.5	01Jul2015, 12:20	0.8
B03-B04	0.2062	1.5	01Jul2015, 12:36	0.8
DB04	0.0422	1.2	01Jul2015, 12:18	0.3
DB05	0.0384	1.4	01Jul2015, 12:22	0.3
B04	0.2868	3.6	01Jul2015, 12:32	1.4
B04-B05	0.2868	3.6	01Jul2015, 12:34	1.4
DB06	0.0219	8.6	01Jul2015, 12:10	0.8
B05	0.3087	10.3	01Jul2015, 12:12	2.2
B05-POND A	0.3087	10.2	01Jul2015, 12:14	2.1
DB07	0.0254	1.9	01Jul2015, 12:10	0.3
DB08	0.0297	0.5	01Jul2015, 12:16	0.2
POND A	0.9076	5.5	01Jul2015, 15:32	3.3
POND A-B06	0.9076	5.5	01Jul2015, 15:34	3.3
DB09	0.0189	3.7	01Jul2015, 12:06	0.3
B06	0.9265	5.7	01Jul2015, 15:32	3.6

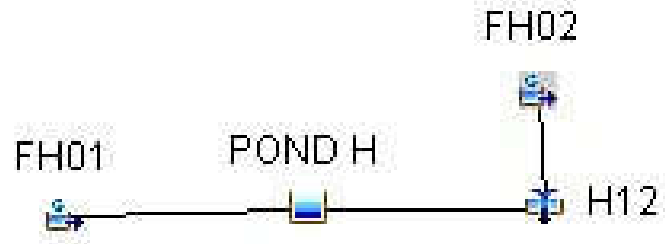
FILING 1 2-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂ (AC. FT.)
B06-B07	0.9265	5.7	01Jul2015, 15:48	3.5
DB11	0.0969	8.1	01Jul2015, 12:16	1.2
DB10	0.0364	5.8	01Jul2015, 12:10	0.6
B07	1.0598	14.7	01Jul2015, 12:22	5.4
B07-B09	1.0598	14.4	01Jul2015, 12:30	5.3
DB12	0.0453	11.2	01Jul2015, 12:08	1.0
B09	1.1051	19.4	01Jul2015, 12:20	6.3
B09-POND B	1.1051	19.3	01Jul2015, 12:22	6.3
DB15	0.1234	3.3	01Jul2015, 12:28	0.9
DB13	0.0703	7.9	01Jul2015, 12:16	1.1
DB14	0.0556	12.4	01Jul2015, 12:10	1.2
POND B	1.3544	29.7	01Jul2015, 12:34	9.4
POND B-B12	1.3544	29.7	01Jul2015, 12:38	9.3
DB22	0.0516	14.1	01Jul2015, 12:10	1.3
DB23	0.0172	13.1	01Jul2015, 12:06	1.0
B12	1.4232	37.5	01Jul2015, 12:28	11.7
B12-B14	1.4232	37.5	01Jul2015, 12:32	11.6
DB24	0.0531	13.2	01Jul2015, 12:08	1.2
B14	1.4763	45.9	01Jul2015, 12:16	12.8
B14-B15	1.4763	45.9	01Jul2015, 12:18	12.8
DB28	0.0741	3.6	01Jul2015, 12:26	0.8
B15	1.5504	49.0	01Jul2015, 12:18	13.5
B15-B18	1.5504	48.8	01Jul2015, 12:30	13.2
DB29	0.1697	6.1	01Jul2015, 12:28	1.5
DB27	0.0508	8.9	01Jul2015, 12:18	1.1
B26	3.6115	86.9	01Jul2015, 12:28	24.9
B26-27	3.6115	86.6	01Jul2015, 12:32	24.6
FB-02	0.0500	12.5	01Jul2015, 12:10	1.2
FB-01	0.0373	1.1	01Jul2015, 12:14	0.3
FB01-27a	0.0373	1.1	01Jul2015, 12:16	0.3
B19	0.0873	13.3	01Jul2015, 12:10	1.5
B19-27	0.0873	13.2	01Jul2015, 12:12	1.5
FB-03	0.0078	6.2	01Jul2015, 12:02	0.4
27	3.7066	93.5	01Jul2015, 12:32	26.5
27-32	3.7066	92.7	01Jul2015, 12:36	26.3
WH-24	0.1325	31.1	01Jul2015, 12:12	3.2
WH-26	0.0839	0.5	01Jul2015, 13:18	0.3
WH-27	0.0217	0.1	01Jul2015, 12:50	0.1
30	0.2381	31.1	01Jul2015, 12:12	3.5
30-31	0.2381	31.1	01Jul2015, 12:14	3.5
WH-28	0.0398	8.1	01Jul2015, 12:14	0.9
31	0.2779	39.2	01Jul2015, 12:14	4.4
31-32	0.2779	38.9	01Jul2015, 12:16	4.4
WH-29	0.0495	10.2	01Jul2015, 12:12	1.1

FILING 1 2-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂ (AC. FT.)
WH-31	0.0406	11.8	01Jul2015, 12:08	1.1
WH-30	0.0159	1.3	01Jul2015, 12:04	0.1
32	4.0905	122.8	01Jul2015, 12:34	33.0
WH32	0.0458	0.3	01Jul2015, 12:48	0.2
BEN POND	4.1363	43.8	01Jul2015, 13:48	28.4
WH-33	0.0064	1.9	01Jul2015, 12:08	0.2
33	4.1427	43.9	01Jul2015, 13:48	28.6
33-37	4.1427	43.9	01Jul2015, 14:00	28.1
WH35	0.1550	6.4	01Jul2015, 12:16	1.3
WH34	0.0450	7.0	01Jul2015, 12:10	0.8
B34-36	0.0450	6.9	01Jul2015, 12:14	0.8
36	0.2000	13.3	01Jul2015, 12:14	2.0
36-37	0.2000	13.2	01Jul2015, 12:22	2.0
WH36	0.0750	0.6	01Jul2015, 12:52	0.3
37	4.4177	47.3	01Jul2015, 13:56	30.4
FH01	0.1344	15.4	01Jul2015, 12:10	1.8
POND H	0.1344	1.8	01Jul2015, 14:30	1.7
FH02	0.0138	1.4	01Jul2015, 12:12	0.2
H12	0.1482	2.3	01Jul2015, 12:16	1.9

BENNETT INTERIM CONDITIONS



HAEGLER INTERIM CONDITIONS



FUTURE 100-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀₀ (AC. FT.)
OS01	1.5594	757	01Jul2015, 12:32	122
DB16	0.0578	92	01Jul2015, 12:10	8
B10	1.6172	794	01Jul2015, 12:30	130
B10-B11	1.6172	793	01Jul2015, 12:32	130
DB17	0.0048	16	01Jul2015, 12:02	1
B11	1.6220	795	01Jul2015, 12:32	131
B11-POND C	1.6220	795	01Jul2015, 12:34	131
DB21	0.0519	54	01Jul2015, 12:08	5
DB18	0.0346	64	01Jul2015, 12:08	5
DB19	0.0281	36	01Jul2015, 12:10	3
DB20	0.0147	25	01Jul2015, 12:08	2
POND C	1.7513	749	01Jul2015, 12:46	141
POND C-B16	1.7513	749	01Jul2015, 12:48	141
DB25	0.0211	45	01Jul2015, 12:04	3
B16	1.7724	754	01Jul2015, 12:48	144
B16-B17	1.7724	754	01Jul2015, 12:50	144
DB26	0.0682	136	01Jul2015, 12:10	12
B17	1.8406	778	01Jul2015, 12:50	156
B17-B18	1.8406	778	01Jul2015, 12:52	156
OS03	0.1984	130	01Jul2015, 12:18	16
DB01	0.0719	90	01Jul2015, 12:08	8
B01	0.2703	199	01Jul2015, 12:14	23
B01-B02	0.2703	199	01Jul2015, 12:14	23
OS02	0.2219	148	01Jul2015, 12:20	19
DB02	0.0516	71	01Jul2015, 12:06	5
B02	0.5438	380	01Jul2015, 12:14	48
B02-POND A	0.5438	379	01Jul2015, 12:16	47
OS04	0.1359	83	01Jul2015, 12:16	10
DB03	0.0703	70	01Jul2015, 12:10	6
B03	0.2062	145	01Jul2015, 12:12	16
B03-B04	0.2062	145	01Jul2015, 12:18	16
DB04	0.0422	44	01Jul2015, 12:10	4
DB05	0.0384	37	01Jul2015, 12:14	4
B04	0.2868	218	01Jul2015, 12:16	24
B04-B05	0.2868	218	01Jul2015, 12:16	24
DB06	0.0219	44	01Jul2015, 12:08	4
B05	0.3087	253	01Jul2015, 12:14	28
B05-POND A	0.3087	252	01Jul2015, 12:16	27
DB07	0.0254	35	01Jul2015, 12:06	3
DB08	0.0297	32	01Jul2015, 12:06	3
POND A	0.9076	557	01Jul2015, 12:26	77
POND A-B06	0.9076	557	01Jul2015, 12:26	77
DB09	0.0189	34	01Jul2015, 12:04	2
B06	0.9265	565	01Jul2015, 12:26	80

FUTURE 100-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀₀ (AC. FT.)
B06-B07	0.9265	564	01Jul2015, 12:30	79
DB11	0.0969	114	01Jul2015, 12:12	11
DB10	0.0364	56	01Jul2015, 12:08	5
B07	1.0598	652	01Jul2015, 12:26	95
B07-B09	1.0598	651	01Jul2015, 12:28	95
DB12	0.0453	81	01Jul2015, 12:06	7
B09	1.1051	677	01Jul2015, 12:26	101
B09-POND B	1.1051	676	01Jul2015, 12:28	101
DB15	0.1234	105	01Jul2015, 12:16	12
DB13	0.0703	89	01Jul2015, 12:12	9
DB14	0.0556	93	01Jul2015, 12:08	8
POND B	1.3544	688	01Jul2015, 12:42	129
POND B-B12	1.3544	688	01Jul2015, 12:44	128
DB22	0.0516	91	01Jul2015, 12:08	8
DB23	0.0172	45	01Jul2015, 12:04	4
B12	1.4232	714	01Jul2015, 12:36	140
B12-B14	1.4232	714	01Jul2015, 12:38	140
DB24	0.0531	94	01Jul2015, 12:08	8
B14	1.4763	743	01Jul2015, 12:28	147
B14-B15	1.4763	742	01Jul2015, 12:28	147
DB28	0.0741	82	01Jul2015, 12:12	8
B15	1.5504	787	01Jul2015, 12:28	155
B15-B18	1.5504	785	01Jul2015, 12:34	154
DB29	0.1697	146	01Jul2015, 12:18	17
DB27	0.0508	68	01Jul2015, 12:16	7
B26	3.6115	1612	01Jul2015, 12:46	334
B26-27	3.6115	1612	01Jul2015, 12:48	333
FB-02	0.0500	67	01Jul2015, 12:16	7
FB-01	0.0373	40	01Jul2015, 12:08	4
FB01-27a	0.0373	40	01Jul2015, 12:10	4
B19	0.0873	103	01Jul2015, 12:12	11
B19-27	0.0873	103	01Jul2015, 12:14	11
FB-03	0.0078	22	01Jul2015, 12:04	2
27	3.7066	1647	01Jul2015, 12:48	345
27-32	3.7066	1647	01Jul2015, 12:48	345
WH-24	0.1325	218	01Jul2015, 12:10	20
WH-26	0.0839	49	01Jul2015, 12:20	6
WH-27	0.0217	23	01Jul2015, 12:04	2
30	0.2381	271	01Jul2015, 12:10	28
30-31	0.2381	270	01Jul2015, 12:12	28
WH-28	0.0398	60	01Jul2015, 12:12	6
31	0.2779	330	01Jul2015, 12:12	33
31-32	0.2779	329	01Jul2015, 12:14	33
WH-29	0.0495	77	01Jul2015, 12:10	7

FUTURE 100-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀₀ (AC. FT.)
WH-31	0.0406	75	01Jul2015, 12:06	6
WH-30	0.0159	26	01Jul2015, 12:02	2
32	4.0905	1792	01Jul2015, 12:38	393
WH32	0.0458	55	01Jul2015, 12:02	4
BEN POND	4.1363	1394	01Jul2015, 13:18	377
WH-33	0.0064	12	01Jul2015, 12:06	1
33	4.1427	1395	01Jul2015, 13:18	378
33-37	4.1427	1395	01Jul2015, 13:20	376
WH35	0.1550	171	01Jul2015, 12:10	15
WH34	0.0450	68	01Jul2015, 12:08	6
B34-36	0.0450	68	01Jul2015, 12:10	6
36	0.2000	239	01Jul2015, 12:10	21
36-37	0.2000	238	01Jul2015, 12:12	21
WH36	0.0750	63	01Jul2015, 12:10	6
37	4.4177	1433	01Jul2015, 13:20	403
FH01	0.1344	161	01Jul2015, 12:16	18
POND H	0.1344	55	01Jul2015, 12:52	16
FH02	0.0091	12	01Jul2015, 12:08	1
FH03	0.0081	15	01Jul2015, 12:08	1
H12	0.1516	60	01Jul2015, 12:50	18

FUTURE 50-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅₀ (AC. FT.)
OS01	1.5594	510	01 Jul2015, 12:34	87
DB16	0.0578	72	01 Jul2015, 12:10	7
B10	1.6172	537	01 Jul2015, 12:32	93
B10-B11	1.6172	537	01 Jul2015, 12:32	93
DB17	0.0048	13	01 Jul2015, 12:02	1
B11	1.6220	538	01 Jul2015, 12:32	94
B11-POND C	1.6220	538	01 Jul2015, 12:36	94
DB21	0.0519	38	01 Jul2015, 12:08	3
DB18	0.0346	50	01 Jul2015, 12:08	4
DB19	0.0281	27	01 Jul2015, 12:10	3
DB20	0.0147	19	01 Jul2015, 12:08	2
POND C	1.7513	507	01 Jul2015, 12:48	101
POND C-B16	1.7513	507	01 Jul2015, 12:50	101
DB25	0.0211	35	01 Jul2015, 12:04	3
B16	1.7724	511	01 Jul2015, 12:50	103
B16-B17	1.7724	510	01 Jul2015, 12:54	103
DB26	0.0682	110	01 Jul2015, 12:10	10
B17	1.8406	529	01 Jul2015, 12:52	113
B17-B18	1.8406	529	01 Jul2015, 12:54	113
OS03	0.1984	88	01 Jul2015, 12:20	11
DB01	0.0719	66	01 Jul2015, 12:08	6
B01	0.2703	139	01 Jul2015, 12:14	17
B01-B02	0.2703	138	01 Jul2015, 12:16	17
OS02	0.2219	102	01 Jul2015, 12:22	14
DB02	0.0516	52	01 Jul2015, 12:06	4
B02	0.5438	263	01 Jul2015, 12:16	34
B02-POND A	0.5438	263	01 Jul2015, 12:16	34
OS04	0.1359	54	01 Jul2015, 12:18	7
DB03	0.0703	49	01 Jul2015, 12:10	5
B03	0.2062	98	01 Jul2015, 12:14	11
B03-B04	0.2062	98	01 Jul2015, 12:18	11
DB04	0.0422	31	01 Jul2015, 12:10	3
DB05	0.0384	27	01 Jul2015, 12:14	3
B04	0.2868	150	01 Jul2015, 12:16	17
B04-B05	0.2868	149	01 Jul2015, 12:16	17
DB06	0.0219	35	01 Jul2015, 12:08	3
B05	0.3087	176	01 Jul2015, 12:16	20
B05-POND A	0.3087	176	01 Jul2015, 12:16	20
DB07	0.0254	26	01 Jul2015, 12:06	2
DB08	0.0297	22	01 Jul2015, 12:06	2
POND A	0.9076	401	01 Jul2015, 12:26	56
POND A-B06	0.9076	400	01 Jul2015, 12:26	55
DB09	0.0189	26	01 Jul2015, 12:04	2
B06	0.9265	407	01 Jul2015, 12:26	57

FUTURE 50-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅₀ (AC. FT.)
B06-B07	0.9265	406	01 Jul2015, 12:30	57
DB11	0.0969	85	01 Jul2015, 12:12	9
DB10	0.0364	43	01 Jul2015, 12:08	4
B07	1.0598	469	01 Jul2015, 12:28	69
B07-B09	1.0598	468	01 Jul2015, 12:30	69
DB12	0.0453	63	01 Jul2015, 12:06	5
B09	1.1051	486	01 Jul2015, 12:30	74
B09-POND B	1.1051	485	01 Jul2015, 12:30	74
DB15	0.1234	75	01 Jul2015, 12:16	9
DB13	0.0703	67	01 Jul2015, 12:12	7
DB14	0.0556	72	01 Jul2015, 12:08	6
POND B	1.3544	539	01 Jul2015, 12:38	95
POND B-B12	1.3544	539	01 Jul2015, 12:40	94
DB22	0.0516	72	01 Jul2015, 12:08	6
DB23	0.0172	38	01 Jul2015, 12:04	3
B12	1.4232	562	01 Jul2015, 12:38	104
B12-B14	1.4232	562	01 Jul2015, 12:40	103
DB24	0.0531	73	01 Jul2015, 12:08	6
B14	1.4763	577	01 Jul2015, 12:40	109
B14-B15	1.4763	576	01 Jul2015, 12:40	109
DB28	0.0741	60	01 Jul2015, 12:12	6
B15	1.5504	597	01 Jul2015, 12:40	115
B15-B18	1.5504	596	01 Jul2015, 12:46	114
DB29	0.1697	105	01 Jul2015, 12:18	13
DB27	0.0508	53	01 Jul2015, 12:16	6
B26	3.6115	1171	01 Jul2015, 12:48	245
B26-27	3.6115	1170	01 Jul2015, 12:50	244
FB-02	0.0500	53	01 Jul2015, 12:16	6
FB-01	0.0373	28	01 Jul2015, 12:08	3
FB01-27a	0.0373	28	01 Jul2015, 12:10	3
B19	0.0873	78	01 Jul2015, 12:14	8
B19-27	0.0873	78	01 Jul2015, 12:14	8
FB-03	0.0078	18	01 Jul2015, 12:04	1
27	3.7066	1197	01 Jul2015, 12:50	254
27-32	3.7066	1196	01 Jul2015, 12:52	253
WH-24	0.1325	171	01 Jul2015, 12:10	15
WH-26	0.0839	33	01 Jul2015, 12:22	5
WH-27	0.0217	16	01 Jul2015, 12:04	1
30	0.2381	205	01 Jul2015, 12:10	21
30-31	0.2381	205	01 Jul2015, 12:12	21
WH-28	0.0398	47	01 Jul2015, 12:12	5
31	0.2779	252	01 Jul2015, 12:12	25
31-32	0.2779	251	01 Jul2015, 12:14	25
WH-29	0.0495	60	01 Jul2015, 12:10	6

FUTURE 50-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅₀ (AC. FT.)
WH-31	0.0406	60	01Jul2015, 12:08	5
WH-30	0.0159	19	01Jul2015, 12:02	1
32	4.0905	1290	01Jul2015, 12:50	290
WH32	0.0458	38	01Jul2015, 12:02	3
BEN POND	4.1363	985	01Jul2015, 13:16	276
WH-33	0.0064	9	01Jul2015, 12:06	1
33	4.1427	986	01Jul2015, 13:16	276
33-37	4.1427	985	01Jul2015, 13:20	275
WH35	0.1550	124	01Jul2015, 12:10	11
WH34	0.0450	52	01Jul2015, 12:08	5
B34-36	0.0450	52	01Jul2015, 12:10	5
36	0.2000	176	01Jul2015, 12:10	16
36-37	0.2000	174	01Jul2015, 12:14	16
WH36	0.0750	43	01Jul2015, 12:10	4
37	4.4177	1015	01Jul2015, 13:20	295
FH01	0.1344	123	01Jul2015, 12:18	14
POND H	0.1344	31	01Jul2015, 13:04	12
FH02	0.0091	9	01Jul2015, 12:08	1
FH03	0.0081	12	01Jul2015, 12:08	1
H12	0.1516	34	01Jul2015, 13:00	14

FUTURE 25-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂₅ (AC. FT.)
OS01	1.5594	316	01Jul2015, 12:36	58.6
DB16	0.0578	54	01Jul2015, 12:10	5.0
B10	1.6172	335	01Jul2015, 12:34	63.6
B10-B11	1.6172	335	01Jul2015, 12:34	63.5
DB17	0.0048	11	01Jul2015, 12:02	0.9
B11	1.6220	336	01Jul2015, 12:34	64.4
B11-POND C	1.6220	336	01Jul2015, 12:38	64.0
DB21	0.0519	25	01Jul2015, 12:08	2.3
DB18	0.0346	39	01Jul2015, 12:08	3.2
DB19	0.0281	20	01Jul2015, 12:10	1.9
DB20	0.0147	15	01Jul2015, 12:10	1.3
POND C	1.7513	310	01Jul2015, 12:52	68.6
POND C-B16	1.7513	310	01Jul2015, 12:54	68.4
DB25	0.0211	27	01Jul2015, 12:04	2.0
B16	1.7724	313	01Jul2015, 12:54	70.3
B16-B17	1.7724	312	01Jul2015, 12:58	69.9
DB26	0.0682	88	01Jul2015, 12:10	8.0
B17	1.8406	326	01Jul2015, 12:58	77.9
B17-B18	1.8406	326	01Jul2015, 13:00	77.5
OS03	0.1984	55	01Jul2015, 12:20	7.7
DB01	0.0719	46	01Jul2015, 12:08	4.1
B01	0.2703	89	01Jul2015, 12:14	11.7
B01-B02	0.2703	89	01Jul2015, 12:16	11.7
OS02	0.2219	65	01Jul2015, 12:22	9.3
DB02	0.0516	36	01Jul2015, 12:06	2.8
B02	0.5438	169	01Jul2015, 12:16	23.8
B02-POND A	0.5438	169	01Jul2015, 12:18	23.8
OS04	0.1359	32	01Jul2015, 12:18	4.5
DB03	0.0703	32	01Jul2015, 12:10	3.2
B03	0.2062	61	01Jul2015, 12:14	7.7
B03-B04	0.2062	60	01Jul2015, 12:20	7.6
DB04	0.0422	21	01Jul2015, 12:10	2.0
DB05	0.0384	18	01Jul2015, 12:14	2.0
B04	0.2868	94	01Jul2015, 12:18	11.7
B04-B05	0.2868	94	01Jul2015, 12:18	11.7
DB06	0.0219	28	01Jul2015, 12:08	2.4
B05	0.3087	115	01Jul2015, 12:16	14.0
B05-POND A	0.3087	114	01Jul2015, 12:18	14.0
DB07	0.0254	18	01Jul2015, 12:06	1.5
DB08	0.0297	15	01Jul2015, 12:08	1.3
POND A	0.9076	244	01Jul2015, 12:28	37.8
POND A-B06	0.9076	244	01Jul2015, 12:30	37.8
DB09	0.0189	19	01Jul2015, 12:04	1.4
B06	0.9265	248	01Jul2015, 12:30	39.2

FUTURE 25-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂₅ (AC. FT.)
B06-B07	0.9265	248	01Jul2015, 12:34	38.8
DB11	0.0969	60	01Jul2015, 12:12	6.2
DB10	0.0364	32	01Jul2015, 12:08	2.7
B07	1.0598	286	01Jul2015, 12:32	47.7
B07-B09	1.0598	285	01Jul2015, 12:36	47.4
DB12	0.0453	48	01Jul2015, 12:06	3.9
B09	1.1051	296	01Jul2015, 12:36	51.3
B09-POND B	1.1051	296	01Jul2015, 12:36	51.2
DB15	0.1234	50	01Jul2015, 12:18	6.1
DB13	0.0703	49	01Jul2015, 12:12	4.9
DB14	0.0556	54	01Jul2015, 12:08	4.7
POND B	1.3544	337	01Jul2015, 12:42	66.5
POND B-B12	1.3544	336	01Jul2015, 12:44	66.3
DB22	0.0516	55	01Jul2015, 12:08	4.8
DB23	0.0172	31	01Jul2015, 12:04	2.5
B12	1.4232	353	01Jul2015, 12:42	73.6
B12-B14	1.4232	352	01Jul2015, 12:44	73.4
DB24	0.0531	56	01Jul2015, 12:08	4.6
B14	1.4763	363	01Jul2015, 12:44	78.0
B14-B15	1.4763	362	01Jul2015, 12:46	77.9
DB28	0.0741	42	01Jul2015, 12:12	4.3
B15	1.5504	375	01Jul2015, 12:44	82.1
B15-B18	1.5504	375	01Jul2015, 12:52	81.2
DB29	0.1697	71	01Jul2015, 12:20	9.0
DB27	0.0508	40	01Jul2015, 12:16	4.3
B26	3.6115	731	01Jul2015, 12:54	172.0
B26-27	3.6115	731	01Jul2015, 12:56	171.3
FB-02	0.0500	40	01Jul2015, 12:16	4.4
FB-01	0.0373	19	01Jul2015, 12:10	1.8
FB01-27a	0.0373	19	01Jul2015, 12:10	1.8
B19	0.0873	57	01Jul2015, 12:14	6.2
B19-27	0.0873	57	01Jul2015, 12:16	6.2
FB-03	0.0078	15	01Jul2015, 12:04	1.1
27	3.7066	748	01Jul2015, 12:56	178.6
27-32	3.7066	748	01Jul2015, 12:58	178.2
WH-24	0.1325	129	01Jul2015, 12:10	11.7
WH-26	0.0839	20	01Jul2015, 12:22	3.0
WH-27	0.0217	10	01Jul2015, 12:04	0.8
30	0.2381	150	01Jul2015, 12:10	15.5
30-31	0.2381	149	01Jul2015, 12:12	15.5
WH-28	0.0398	36	01Jul2015, 12:12	3.4
31	0.2779	185	01Jul2015, 12:12	18.9
31-32	0.2779	185	01Jul2015, 12:14	18.8
WH-29	0.0495	45	01Jul2015, 12:10	4.2

FUTURE 25-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂₅ (AC. FT.)
WH-31	0.0406	46	01Jul2015, 12:08	3.8
WH-30	0.0159	13	01Jul2015, 12:02	0.9
32	4.0905	809	01Jul2015, 12:56	205.8
WH32	0.0458	24	01Jul2015, 12:02	1.6
BEN POND	4.1363	595	01Jul2015, 13:26	194.6
WH-33	0.0064	7	01Jul2015, 12:08	0.6
33	4.1427	596	01Jul2015, 13:26	195.2
33-37	4.1427	596	01Jul2015, 13:32	193.6
WH35	0.1550	84	01Jul2015, 12:10	8.0
WH34	0.0450	38	01Jul2015, 12:08	3.3
B34-36	0.0450	38	01Jul2015, 12:10	3.3
36	0.2000	122	01Jul2015, 12:10	11.4
36-37	0.2000	121	01Jul2015, 12:14	11.3
WH36	0.0750	27	01Jul2015, 12:12	2.9
37	4.4177	616	01Jul2015, 13:30	207.8
FH01	0.1344	91	01Jul2015, 12:18	10.5
POND H	0.1344	18	01Jul2015, 13:18	8.4
FH02	0.0091	6	01Jul2015, 12:10	0.6
FH03	0.0081	9	01Jul2015, 12:08	0.8
H12	0.1516	20	01Jul2015, 13:12	9.7

FUTURE 10-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀ (AC. FT.)
OS01	1.5594	136	01 Jul 2015, 12:38	30.9
DB16	0.0578	35	01 Jul 2015, 12:10	3.3
B10	1.6172	147	01 Jul 2015, 12:36	34.2
B10-B11	1.6172	147	01 Jul 2015, 12:38	34.1
DB17	0.0048	9	01 Jul 2015, 12:02	0.7
B11	1.6220	148	01 Jul 2015, 12:38	34.8
B11-POND C	1.6220	148	01 Jul 2015, 12:42	34.5
DB21	0.0519	12	01 Jul 2015, 12:10	1.3
DB18	0.0346	26	01 Jul 2015, 12:08	2.1
DB19	0.0281	11	01 Jul 2015, 12:12	1.1
DB20	0.0147	9	01 Jul 2015, 12:10	0.8
POND C	1.7513	129	01 Jul 2015, 13:02	36.3
POND C-B16	1.7513	128	01 Jul 2015, 13:06	36.1
DB25	0.0211	18	01 Jul 2015, 12:04	1.3
B16	1.7724	130	01 Jul 2015, 13:06	37.4
B16-B17	1.7724	130	01 Jul 2015, 13:10	37.1
DB26	0.0682	62	01 Jul 2015, 12:10	5.6
B17	1.8406	138	01 Jul 2015, 13:08	42.7
B17-B18	1.8406	138	01 Jul 2015, 13:12	42.4
OS03	0.1984	24	01 Jul 2015, 12:24	4.1
DB01	0.0719	25	01 Jul 2015, 12:10	2.4
B01	0.2703	42	01 Jul 2015, 12:14	6.5
B01-B02	0.2703	42	01 Jul 2015, 12:16	6.5
OS02	0.2219	30	01 Jul 2015, 12:26	5.1
DB02	0.0516	20	01 Jul 2015, 12:06	1.7
B02	0.5438	79	01 Jul 2015, 12:18	13.2
B02-POND A	0.5438	79	01 Jul 2015, 12:20	13.1
OS04	0.1359	12	01 Jul 2015, 12:22	2.3
DB03	0.0703	16	01 Jul 2015, 12:12	1.8
B03	0.2062	26	01 Jul 2015, 12:16	4.1
B03-B04	0.2062	26	01 Jul 2015, 12:22	4.0
DB04	0.0422	10	01 Jul 2015, 12:12	1.2
DB05	0.0384	9	01 Jul 2015, 12:16	1.1
B04	0.2868	42	01 Jul 2015, 12:20	6.3
B04-B05	0.2868	42	01 Jul 2015, 12:20	6.3
DB06	0.0219	19	01 Jul 2015, 12:08	1.6
B05	0.3087	55	01 Jul 2015, 12:18	8.0
B05-POND A	0.3087	55	01 Jul 2015, 12:20	8.0
DB07	0.0254	10	01 Jul 2015, 12:08	0.9
DB08	0.0297	7	01 Jul 2015, 12:08	0.7
POND A	0.9076	98	01 Jul 2015, 12:38	20.1
POND A-B06	0.9076	98	01 Jul 2015, 12:38	20.1
DB09	0.0189	12	01 Jul 2015, 12:04	0.9
B06	0.9265	100	01 Jul 2015, 12:38	21.0

FUTURE 10-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀ (AC. FT.)
B06-B07	0.9265	99	01 Jul 2015, 12:46	20.7
DB11	0.0969	35	01 Jul 2015, 12:14	3.8
DB10	0.0364	19	01 Jul 2015, 12:08	1.7
B07	1.0598	116	01 Jul 2015, 12:44	26.2
B07-B09	1.0598	116	01 Jul 2015, 12:48	26.0
DB12	0.0453	31	01 Jul 2015, 12:08	2.5
B09	1.1051	121	01 Jul 2015, 12:48	28.5
B09-POND B	1.1051	121	01 Jul 2015, 12:48	28.5
DB15	0.1234	25	01 Jul 2015, 12:20	3.4
DB13	0.0703	29	01 Jul 2015, 12:14	3.1
DB14	0.0556	35	01 Jul 2015, 12:08	3.1
POND B	1.3544	140	01 Jul 2015, 12:56	37.8
POND B-B12	1.3544	140	01 Jul 2015, 12:58	37.7
DB22	0.0516	36	01 Jul 2015, 12:08	3.2
DB23	0.0172	23	01 Jul 2015, 12:06	1.8
B12	1.4232	148	01 Jul 2015, 12:26	42.7
B12-B14	1.4232	148	01 Jul 2015, 12:28	42.5
DB24	0.0531	36	01 Jul 2015, 12:08	3.0
B14	1.4763	162	01 Jul 2015, 12:26	45.6
B14-B15	1.4763	162	01 Jul 2015, 12:28	45.5
DB28	0.0741	23	01 Jul 2015, 12:14	2.5
B15	1.5504	176	01 Jul 2015, 12:26	48.0
B15-B18	1.5504	176	01 Jul 2015, 12:34	47.3
DB29	0.1697	37	01 Jul 2015, 12:20	5.2
DB27	0.0508	25	01 Jul 2015, 12:16	2.8
B26	3.6115	314	01 Jul 2015, 13:08	97.8
B26-27	3.6115	314	01 Jul 2015, 13:12	97.2
FB-02	0.0500	26	01 Jul 2015, 12:18	2.9
FB-01	0.0373	9	01 Jul 2015, 12:10	1.0
FB01-27a	0.0373	9	01 Jul 2015, 12:12	1.0
B19	0.0873	34	01 Jul 2015, 12:16	3.9
B19-27	0.0873	34	01 Jul 2015, 12:16	3.9
FB-03	0.0078	11	01 Jul 2015, 12:04	0.8
27	3.7066	322	01 Jul 2015, 13:12	101.9
27-32	3.7066	322	01 Jul 2015, 13:14	101.6
WH-24	0.1325	84	01 Jul 2015, 12:10	7.7
WH-26	0.0839	8	01 Jul 2015, 12:26	1.5
WH-27	0.0217	4	01 Jul 2015, 12:06	0.4
30	0.2381	91	01 Jul 2015, 12:10	9.7
30-31	0.2381	91	01 Jul 2015, 12:12	9.7
WH-28	0.0398	23	01 Jul 2015, 12:12	2.2
31	0.2779	114	01 Jul 2015, 12:12	11.9
31-32	0.2779	113	01 Jul 2015, 12:14	11.9
WH-29	0.0495	29	01 Jul 2015, 12:10	2.7

FUTURE 10-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₁₀ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₁₀ (AC. FT.)
WH-31	0.0406	30	01Jul2015, 12:08	2.5
WH-30	0.0159	7	01Jul2015, 12:02	0.5
32	4.0905	435	01Jul2015, 12:24	119.2
WH32	0.0458	10	01Jul2015, 12:04	0.9
BEN POND	4.1363	252	01Jul2015, 13:52	111.9
WH-33	0.0064	5	01Jul2015, 12:08	0.4
33	4.1427	253	01Jul2015, 13:52	112.3
33-37	4.1427	253	01Jul2015, 13:58	111.2
WH35	0.1550	44	01Jul2015, 12:10	4.6
WH34	0.0450	23	01Jul2015, 12:10	2.1
B34-36	0.0450	23	01Jul2015, 12:12	2.1
36	0.2000	67	01Jul2015, 12:12	6.7
36-37	0.2000	66	01Jul2015, 12:16	6.7
WH36	0.0750	11	01Jul2015, 12:14	1.5
37	4.4177	262	01Jul2015, 13:58	119.4
FH01	0.1344	56	01Jul2015, 12:18	6.7
POND H	0.1344	8	01Jul2015, 13:48	5.1
FH02	0.0091	4	01Jul2015, 12:10	0.3
FH03	0.0081	6	01Jul2015, 12:08	0.5
H12	0.1516	11	01Jul2015, 12:10	6.0

FUTURE 5-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅ (AC. FT.)
OS01	1.5594	55	01Jul2015, 12:46	16.6
DB16	0.0578	23	01Jul2015, 12:12	2.3
B10	1.6172	62	01Jul2015, 12:42	18.9
B10-B11	1.6172	62	01Jul2015, 12:42	18.9
DB17	0.0048	7	01Jul2015, 12:02	0.6
B11	1.6220	63	01Jul2015, 12:42	19.4
B11-POND C	1.6220	63	01Jul2015, 12:48	19.2
DB21	0.0519	5	01Jul2015, 12:12	0.7
DB18	0.0346	18	01Jul2015, 12:08	1.5
DB19	0.0281	7	01Jul2015, 12:12	0.7
DB20	0.0147	6	01Jul2015, 12:10	0.6
POND C	1.7513	50	01Jul2015, 13:28	19.4
POND C-B16	1.7513	50	01Jul2015, 13:32	19.2
DB25	0.0211	12	01Jul2015, 12:04	0.9
B16	1.7724	51	01Jul2015, 13:32	20.1
B16-B17	1.7724	51	01Jul2015, 13:36	19.9
DB26	0.0682	46	01Jul2015, 12:10	4.1
B17	1.8406	56	01Jul2015, 12:12	24.0
B17-B18	1.8406	56	01Jul2015, 12:16	23.8
OS03	0.1984	9	01Jul2015, 12:28	2.2
DB01	0.0719	14	01Jul2015, 12:10	1.5
B01	0.2703	19	01Jul2015, 12:14	3.7
B01-B02	0.2703	19	01Jul2015, 12:18	3.7
OS02	0.2219	13	01Jul2015, 12:28	2.8
DB02	0.0516	10	01Jul2015, 12:06	1.0
B02	0.5438	36	01Jul2015, 12:18	7.6
B02-POND A	0.5438	36	01Jul2015, 12:22	7.5
OS04	0.1359	4	01Jul2015, 12:28	1.2
DB03	0.0703	7	01Jul2015, 12:14	1.0
B03	0.2062	10	01Jul2015, 12:16	2.2
B03-B04	0.2062	10	01Jul2015, 12:26	2.2
DB04	0.0422	5	01Jul2015, 12:14	0.7
DB05	0.0384	5	01Jul2015, 12:18	0.7
B04	0.2868	18	01Jul2015, 12:22	3.5
B04-B05	0.2868	18	01Jul2015, 12:24	3.5
DB06	0.0219	14	01Jul2015, 12:08	1.2
B05	0.3087	26	01Jul2015, 12:22	4.7
B05-POND A	0.3087	26	01Jul2015, 12:22	4.7
DB07	0.0254	6	01Jul2015, 12:08	0.5
DB08	0.0297	3	01Jul2015, 12:10	0.4
POND A	0.9076	34	01Jul2015, 12:58	10.7
POND A-B06	0.9076	34	01Jul2015, 13:00	10.7
DB09	0.0189	8	01Jul2015, 12:06	0.6
B06	0.9265	35	01Jul2015, 13:00	11.3

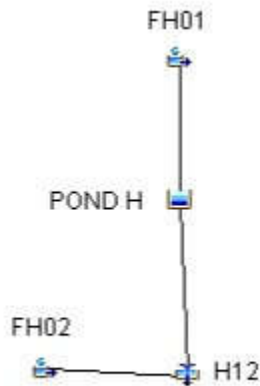
FUTURE 5-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅ (AC. FT.)
B06-B07	0.9265	35	01Jul2015, 13:08	11.1
DB11	0.0969	20	01Jul2015, 12:14	2.4
DB10	0.0364	12	01Jul2015, 12:08	1.1
B07	1.0598	42	01Jul2015, 13:06	14.7
B07-B09	1.0598	42	01Jul2015, 13:12	14.5
DB12	0.0453	21	01Jul2015, 12:08	1.7
B09	1.1051	45	01Jul2015, 12:16	16.2
B09-POND B	1.1051	45	01Jul2015, 12:18	16.2
DB15	0.1234	12	01Jul2015, 12:22	2.0
DB13	0.0703	18	01Jul2015, 12:14	2.0
DB14	0.0556	23	01Jul2015, 12:10	2.1
POND B	1.3544	69	01Jul2015, 12:30	22.2
POND B-B12	1.3544	69	01Jul2015, 12:32	22.1
DB22	0.0516	25	01Jul2015, 12:10	2.2
DB23	0.0172	18	01Jul2015, 12:06	1.4
B12	1.4232	83	01Jul2015, 12:28	25.7
B12-B14	1.4232	83	01Jul2015, 12:30	25.6
DB24	0.0531	24	01Jul2015, 12:08	2.1
B14	1.4763	92	01Jul2015, 12:26	27.7
B14-B15	1.4763	92	01Jul2015, 12:28	27.6
DB28	0.0741	13	01Jul2015, 12:14	1.6
B15	1.5504	102	01Jul2015, 12:20	29.2
B15-B18	1.5504	102	01Jul2015, 12:30	28.7
DB29	0.1697	19	01Jul2015, 12:24	3.2
DB27	0.0508	17	01Jul2015, 12:16	1.9
B26	3.6115	179	01Jul2015, 12:22	57.6
B26-27	3.6115	179	01Jul2015, 12:26	57.2
FB-02	0.0500	17	01Jul2015, 12:18	2.0
FB-01	0.0373	5	01Jul2015, 12:12	0.6
FB01-27a	0.0373	5	01Jul2015, 12:14	0.6
B19	0.0873	22	01Jul2015, 12:16	2.6
B19-27	0.0873	22	01Jul2015, 12:18	2.6
FB-03	0.0078	8	01Jul2015, 12:04	0.6
27	3.7066	199	01Jul2015, 12:26	60.4
27-32	3.7066	198	01Jul2015, 12:28	60.2
WH-24	0.1325	57	01Jul2015, 12:10	5.4
WH-26	0.0839	3	01Jul2015, 12:32	0.8
WH-27	0.0217	1	01Jul2015, 12:08	0.2
30	0.2381	59	01Jul2015, 12:10	6.4
30-31	0.2381	59	01Jul2015, 12:12	6.4
WH-28	0.0398	15	01Jul2015, 12:12	1.5
31	0.2779	74	01Jul2015, 12:12	7.9
31-32	0.2779	74	01Jul2015, 12:16	7.9
WH-29	0.0495	19	01Jul2015, 12:12	1.9

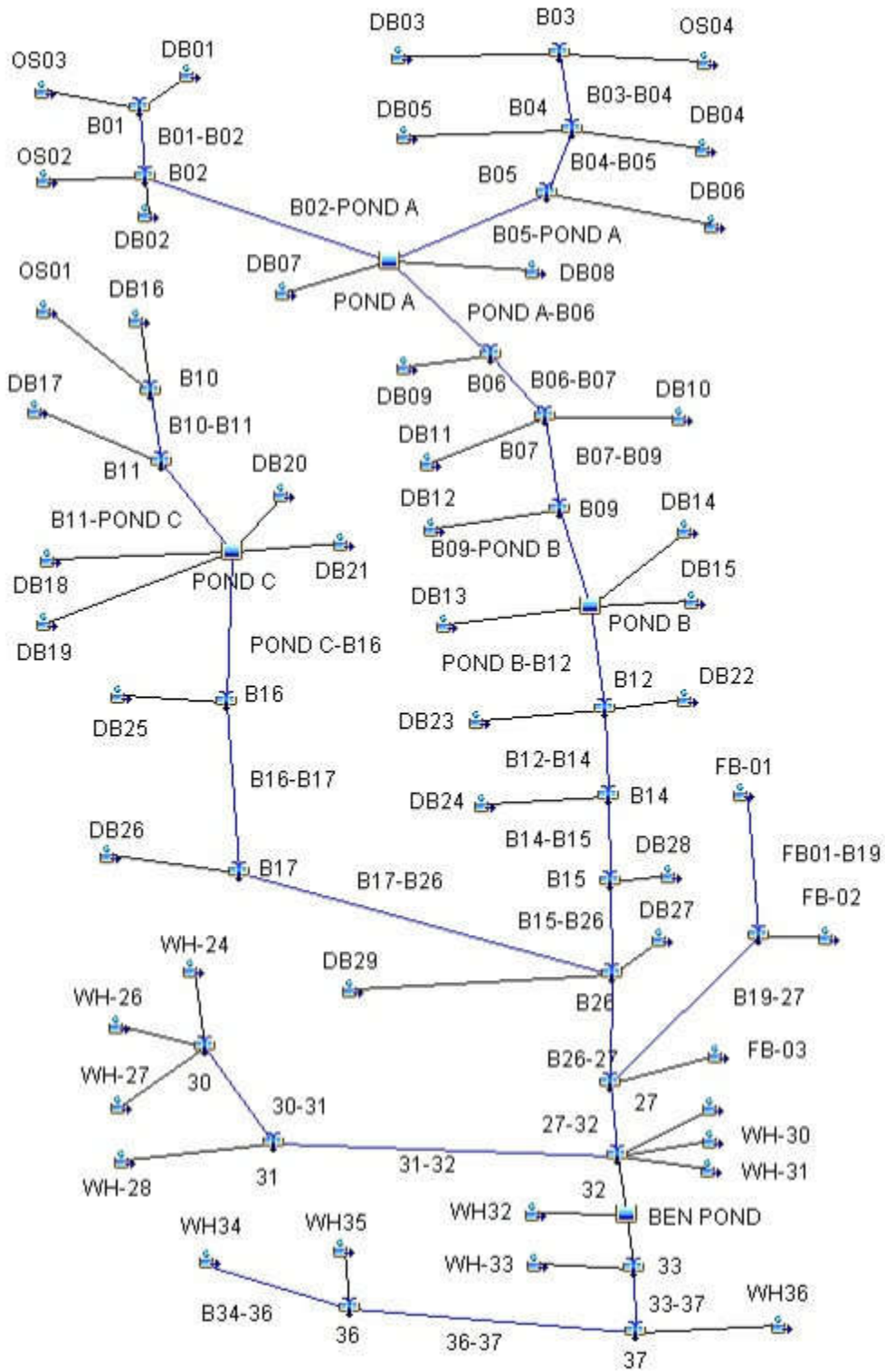
FUTURE 5-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₅ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₅ (AC. FT.)
WH-31	0.0406	21	01Jul2015, 12:08	1.8
WH-30	0.0159	4	01Jul2015, 12:02	0.3
32	4.0905	269	01Jul2015, 12:26	72.0
WH32	0.0458	4	01Jul2015, 12:04	0.5
BEN POND	4.1363	100	01Jul2015, 14:52	66.2
WH-33	0.0064	3	01Jul2015, 12:08	0.3
33	4.1427	100	01Jul2015, 14:52	66.4
33-37	4.1427	100	01Jul2015, 15:00	65.6
WH35	0.1550	22	01Jul2015, 12:12	2.8
WH34	0.0450	15	01Jul2015, 12:10	1.4
B34-36	0.0450	15	01Jul2015, 12:12	1.4
36	0.2000	37	01Jul2015, 12:12	4.2
36-37	0.2000	37	01Jul2015, 12:18	4.1
WH36	0.0750	4	01Jul2015, 12:16	0.8
37	4.4177	104	01Jul2015, 14:50	70.6
FH01	0.1344	36	01Jul2015, 12:20	4.5
POND H	0.1344	4	01Jul2015, 14:42	3.4
FH02	0.0091	2	01Jul2015, 12:10	0.2
FH03	0.0081	4	01Jul2015, 12:08	0.4
H12	0.1516	7	01Jul2015, 12:10	4.0

FUTURE 2-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂ (AC. FT.)
OS01	1.5594	11.0	01Jul2015, 13:24	5.9
DB16	0.0578	12.7	01Jul2015, 12:12	1.3
B10	1.6172	13.2	01Jul2015, 12:12	7.3
B10-B11	1.6172	13.2	01Jul2015, 12:14	7.2
DB17	0.0048	5.7	01Jul2015, 12:02	0.4
B11	1.6220	15.3	01Jul2015, 12:12	7.7
B11-POND C	1.6220	15.2	01Jul2015, 12:20	7.5
DB21	0.0519	1.1	01Jul2015, 12:18	0.3
DB18	0.0346	10.0	01Jul2015, 12:08	0.9
DB19	0.0281	2.8	01Jul2015, 12:14	0.4
DB20	0.0147	3.4	01Jul2015, 12:10	0.3
POND C	1.7513	10.9	01Jul2015, 15:00	6.3
POND C-B16	1.7513	10.9	01Jul2015, 15:06	6.2
DB25	0.0211	7.1	01Jul2015, 12:06	0.6
B16	1.7724	11.3	01Jul2015, 15:06	6.7
B16-B17	1.7724	11.3	01Jul2015, 15:16	6.6
DB26	0.0682	29.4	01Jul2015, 12:10	2.7
B17	1.8406	34.4	01Jul2015, 12:14	9.3
B17-B18	1.8406	34.0	01Jul2015, 12:20	9.1
OS03	0.1984	1.6	01Jul2015, 13:02	0.8
DB01	0.0719	4.7	01Jul2015, 12:12	0.7
B01	0.2703	5.0	01Jul2015, 12:14	1.5
B01-B02	0.2703	5.0	01Jul2015, 12:18	1.5
OS02	0.2219	2.6	01Jul2015, 12:46	1.1
DB02	0.0516	3.4	01Jul2015, 12:08	0.5
B02	0.5438	8.6	01Jul2015, 12:18	3.1
B02-POND A	0.5438	8.6	01Jul2015, 12:22	3.1
OS04	0.1359	0.6	01Jul2015, 13:30	0.4
DB03	0.0703	1.5	01Jul2015, 12:20	0.4
B03	0.2062	1.5	01Jul2015, 12:20	0.8
B03-B04	0.2062	1.5	01Jul2015, 12:36	0.8
DB04	0.0422	1.2	01Jul2015, 12:18	0.3
DB05	0.0384	1.4	01Jul2015, 12:22	0.3
B04	0.2868	3.6	01Jul2015, 12:32	1.4
B04-B05	0.2868	3.6	01Jul2015, 12:34	1.4
DB06	0.0219	8.6	01Jul2015, 12:10	0.8
B05	0.3087	10.3	01Jul2015, 12:12	2.2
B05-POND A	0.3087	10.2	01Jul2015, 12:14	2.1
DB07	0.0254	1.9	01Jul2015, 12:10	0.3
DB08	0.0297	0.5	01Jul2015, 12:16	0.2
POND A	0.9076	5.5	01Jul2015, 15:32	3.3
POND A-B06	0.9076	5.5	01Jul2015, 15:34	3.3
DB09	0.0189	3.7	01Jul2015, 12:06	0.3
B06	0.9265	5.7	01Jul2015, 15:32	3.6

FUTURE 2-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂ (AC. FT.)
B06-B07	0.9265	5.7	01Jul2015, 15:48	3.5
DB11	0.0969	8.1	01Jul2015, 12:16	1.2
DB10	0.0364	5.8	01Jul2015, 12:10	0.6
B07	1.0598	14.7	01Jul2015, 12:22	5.4
B07-B09	1.0598	14.4	01Jul2015, 12:30	5.3
DB12	0.0453	11.2	01Jul2015, 12:08	1.0
B09	1.1051	19.4	01Jul2015, 12:20	6.3
B09-POND B	1.1051	19.3	01Jul2015, 12:22	6.3
DB15	0.1234	3.3	01Jul2015, 12:28	0.9
DB13	0.0703	7.9	01Jul2015, 12:16	1.1
DB14	0.0556	12.4	01Jul2015, 12:10	1.2
POND B	1.3544	29.7	01Jul2015, 12:34	9.4
POND B-B12	1.3544	29.7	01Jul2015, 12:38	9.3
DB22	0.0516	14.1	01Jul2015, 12:10	1.3
DB23	0.0172	13.1	01Jul2015, 12:06	1.0
B12	1.4232	37.5	01Jul2015, 12:28	11.7
B12-B14	1.4232	37.5	01Jul2015, 12:32	11.6
DB24	0.0531	13.2	01Jul2015, 12:08	1.2
B14	1.4763	45.9	01Jul2015, 12:16	12.8
B14-B15	1.4763	45.9	01Jul2015, 12:18	12.8
DB28	0.0741	4.4	01Jul2015, 12:18	0.8
B15	1.5504	50.3	01Jul2015, 12:18	13.5
B15-B18	1.5504	50.2	01Jul2015, 12:30	13.2
DB29	0.1697	6.1	01Jul2015, 12:28	1.5
DB27	0.0508	8.9	01Jul2015, 12:18	1.1
B26	3.6115	89.0	01Jul2015, 12:26	24.9
B26-27	3.6115	88.8	01Jul2015, 12:32	24.6
FB-02	0.0500	9.5	01Jul2015, 12:18	1.2
FB-01	0.0373	1.1	01Jul2015, 12:16	0.3
FB01-27a	0.0373	1.1	01Jul2015, 12:18	0.3
B19	0.0873	10.5	01Jul2015, 12:18	1.5
B19-27	0.0873	10.5	01Jul2015, 12:20	1.5
FB-03	0.0078	5.9	01Jul2015, 12:04	0.4
27	3.7066	98.1	01Jul2015, 12:32	26.5
27-32	3.7066	97.6	01Jul2015, 12:34	26.3
WH-24	0.1325	31.1	01Jul2015, 12:12	3.2
WH-26	0.0839	0.5	01Jul2015, 13:18	0.3
WH-27	0.0217	0.1	01Jul2015, 12:50	0.1
30	0.2381	31.1	01Jul2015, 12:12	3.5
30-31	0.2381	31.1	01Jul2015, 12:14	3.5
WH-28	0.0398	8.1	01Jul2015, 12:14	0.9
31	0.2779	39.2	01Jul2015, 12:14	4.4
31-32	0.2779	38.9	01Jul2015, 12:16	4.4
WH-29	0.0495	10.2	01Jul2015, 12:12	1.1

FUTURE 2-YEAR				
HYDROLOGIC ELEMENT	DRAINAGE AREA (SQ. MI.)	DISCHARGE PEAK Q ₂ (CFS)	TIME OF PEAK	TOTAL VOLUME Q ₂ (AC. FT.)
WH-31	0.0406	11.8	01Jul2015, 12:08	1.1
WH-30	0.0159	1.3	01Jul2015, 12:04	0.1
32	4.0905	127.8	01Jul2015, 12:34	33.0
WH32	0.0458	0.3	01Jul2015, 12:48	0.2
BEN POND	4.1363	43.9	01Jul2015, 13:48	28.4
WH-33	0.0064	1.9	01Jul2015, 12:08	0.2
33	4.1427	44.0	01Jul2015, 13:48	28.6
33-37	4.1427	44.0	01Jul2015, 14:00	28.1
WH35	0.1550	6.4	01Jul2015, 12:16	1.3
WH34	0.0450	7.0	01Jul2015, 12:10	0.8
B34-36	0.0450	6.9	01Jul2015, 12:14	0.8
36	0.2000	13.3	01Jul2015, 12:14	2.0
36-37	0.2000	13.2	01Jul2015, 12:22	2.0
WH36	0.0750	0.6	01Jul2015, 12:52	0.3
37	4.4177	47.4	01Jul2015, 13:56	30.4
FH01	0.1344	17.8	01Jul2015, 12:20	2.5
POND H	0.1344	2.5	01Jul2015, 14:34	2.3
FH02	0.0091	0.8	01Jul2015, 12:12	0.1
FH03	0.0081	2.4	01Jul2015, 12:10	0.2
H12	0.1516	3.7	01Jul2015, 12:12	2.6





Appendix D - Detention Pond Information

Replace the pond design calculations with the UD-Detention Worksheet. The WQCV seems small. Also drain time should be at 40hrs.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

**Meridian Ranch Proposed Detention Pond H-INTERIM
Gieck Basin - El Paso County, Colorado**

Data for spillway and embankment:

embankment length =	500
embankment elev =	6976
spillway length =	50
spillway elevation =	6974.5
100 year storage elev.=	6973.19
100 year storage vol.=	6.9
100 year discharge=	43
5 year storage elev.=	6971.1
5 year storage vol.=	1.6
5 year discharge=	2.9
WQCV storage elev.=	6970.0
WQCV storage vol.=	0.3
1/2 WQCV storage elev.=	6969.6
1/2 WQCV storage vol.=	0.15

Data for outlet pipe and grate:

Type	Orifice	H or V	Dimensions		Dia.(in)	Area =	(sqft)	Elev to cl =	
			Width (ft.)	X Height (ft.)					
Rectangular	Orifice 1:	V	0.0167	1.50		0.025		6969.25	
Rectangular	Orifice 2:	V	4.5000	1.40		6.300		6972.20	
None Selected	Orifice 3:	H				0.000			
Circular	Orifice 4:	H			10	0.545		6970.00	

Stand Pipe Dimensions

Rec Grate	9	x	4.5	Elev =	6972.90
Circ. Grate		dia.		Elev =	6972.90

50 year storage elev.=	6972.70
50 year discharge=	23
25 year storage elev.=	6972.2
25 year discharge=	13
10 year storage elev.=	6971.7
10 year discharge=	5.2
2 year storage elev.=	6970.4
2 year discharge=	1.8

Outlet Culvert Dimensions

	Width (ft.)	Height (ft.)	Dia. (ft.)	Type
Outlet Culvert		x	3.5	Circular
Area	9.6	TOP		
Outlet I. E.	6968.5	6972.38		
Wall Thick.	4.5	in.		

STAGE		STORAGE				DISCHARGE										REALIZED CULVERT OUTFLOW	TOTAL FLOW	
ELEV	HEIGHT	AREA		VOLUME		TOP OF BANK	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)		PIPE				
		sqft	acre	acft	cum acft			1	2	3	4	Rectangular	1	2				
6968.5	0	0	0.00	0.0	0.0			-	-	-	-	-	-	-	-	-	-	-
6969	0.5	477	0.01	0.00	0.003	-	-	0.02	-	-	-	-	-	-	1		0.02	0.02
6970	1.5	22422	0.51	0.26	0.27	-	-	0.02	-	-	-	0.8	-	9		0.9	0.87	
6970.5	2	44606	1.02	0.78	0.78	-	-	0.09	-	-	-	1.9	-	24		1.9	1.9	
6971	2.5	67898	1.56	1.04	1.30	-	-	0.09	-	-	-	2.6	-	24		2.7	2.7	
6971.5	3	92319	2.12	0.92	2.22	-	-	0.16	-	-	-	3.2	-	32		3.4	3.4	
6972	3.5	116739	2.68	1.20	3.42	-	-	0.18	4.8	-	-	3.7	-	41		8.7	8.7	
6972.5	4	125636	2.88	1.39	4.81	-	-	0.20	13.5	4.81	-	4.2	-	50		18	18	
6973	4.5	134533	3.09	1.49	6.31	-	-	0.22	24.8	-	-	4.5	2	56		31	31	
6973.5	5	141972	3.26	1.59	7.89	-	-	0.23	34.6	-	-	4.9	25	61		61	61	
6974	5.5	149410	3.43	1.67	9.57	-	-	0.25	40.7	-	-	5.3	62	66		66	66	
6975	6.5	165140	3.79	3.61	13.18	53.0	53.0	0.26	50.8	-	-	5.9	164	75		75	128	
6976	7.5	192114	4.41	4.10	17.28	275.6	275.6	0.29	59.1	-	-	6.4	295	83		83	359	

- Notes:
- 1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. $Q=CLH^{1.5}$ (C=3.0)
 - 2) Orifice flows are also from section 11.3.1. $Q=CA(2gH)^{.5}$ (C=.6)
 - 3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow $Q=(3PH^{1.5})/F$, Orifice Flow $Q=4.815*AH^{0.5}$
 - 4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

STAGE/STORAGE/DISCHARGE CURVES FOR DETENTION POND ANALYSIS

Meridian Ranch Proposed Detention Pond H-FUTURE

Gieck Basin - El Paso County, Colorado

Data for spillway and embankment:

embankment length =	500
embankment elev =	6976
spillway length =	50
spillway elevation =	6974.5
100 year storage elev.=	6973.4
100 year storage vol.=	7.6
100 year discharge=	55
5 year storage elev.=	6971.5
5 year storage vol.=	2.3
5 year discharge=	4
WQCV storage elev.=	6970.0
WQCV storage vol.=	0.3
1/2 WQCV storage elev.=	6969.6
1/2 WQCV storage vol.=	0.15

Data for outlet pipe and grate:

Type	Orifice	H or V	Dimensions		Dia.(in)	Area =	(sqft)	
			Width (ft.)	Height (ft.)			Elev to cl =	
Rectangular	Orifice 1:	V	0.0167	1.50		0.025	Elev to cl =	6969.25
Rectangular	Orifice 2:	V	4.5000	1.40		6.300	Elev to cl =	6972.20
None Selected	Orifice 3:	H				0.000	Elev to cl =	
Circular	Orifice 4:	H			10	0.545	Elev to cl =	6970.00

Stand Pipe Dimensions	
Rec Grate	9 x 4.5 Elev = 6972.90
Circ. Grate	dia. Elev = 6972.90

50 year storage elev.=	6973.0
50 year discharge=	31
25 year storage elev.=	6972.5
25 year discharge=	18
10 year storage elev.=	6971.9
10 year discharge=	8
2 year storage elev.=	6970.9
2 year discharge=	2.5

Outlet Culvert Dimensions

Outlet Culvert	Width (ft.)	H or V	Height (ft.)	Dia. (ft.)	Type
Outlet Culvert	9.6	x	4.5	3.5	Circular
Area	9.6		TOP		
Outlet I. E.	6968.5		6972.38		
Wall Thick.	4.5	in.			

STAGE		STORAGE				DISCHARGE										REALIZED CULVERT OUTFLOW	TOTAL FLOW	
ELEV	HEIGHT	AREA		VOLUME		TOP OF BANK	SPILLWAY	ORIFICE (max outflow)				GRATE (max outflow)	PIPE					
		sqft	acre	acft	cum acft			1	2	3	4		Rectangular	1	2			
6968.5	0	0	0.00	0.0	0.0	-	-	-	-	-	-	-	-	-	-	-	-	-
6969	0.5	477	0.01	0.00	0.003	-	-	0.02	-	-	-	-	-	-	1	-	0.02	0.018
6970	1.5	22422	0.51	0.26	0.27	-	-	0.02	-	-	-	0.8	-	-	9	-	0.9	0.865
6970.5	2	44606	1.02	0.78	0.78	-	-	0.09	-	-	-	1.9	-	-	24	-	1.9	1.949
6971	2.5	67898	1.56	1.04	1.30	-	-	0.09	-	-	-	2.6	-	-	24	-	2.7	2.718
6971.5	3	92319	2.12	0.92	2.22	-	-	0.16	-	-	-	3.2	-	-	32	-	3.4	3.376
6972	3.5	116739	2.68	1.20	3.42	-	-	0.18	4.8	-	-	3.7	-	-	41	-	8.7	8.668
6972.5	4	125636	2.88	1.39	4.81	-	-	0.20	13.5	-	-	4.2	-	-	50	-	17.9	17.852
6973	4.5	134533	3.09	1.49	6.31	-	-	0.22	24.8	-	-	4.5	2	-	56	-	31.3	31.274
6973.5	5	141972	3.26	1.59	7.89	-	-	0.23	34.6	-	-	4.9	25	-	61	-	61	61.340
6974	5.5	149410	3.43	1.67	9.57	-	-	0.25	40.7	-	-	5.3	62	-	66	-	66	66.260
6975	6.5	165140	3.79	3.61	13.18	53.0	53.0	0.26	50.8	-	-	5.9	164	-	75	-	75	128.173
6976	7.5	192114	4.41	4.10	17.28	275.6	275.6	0.29	59.1	-	-	6.4	295	-	83	-	83	358.648

- Notes:
- 1) Top-of-bank and spillway flows are weir equations from section 11.3.1 in the DCM. $Q=CLH^{1.5}$ (C=3.0)
 - 2) Orifice flows are also from section 11.3.1. $Q=CA(2gH)^{0.5}$ (C=.6)
 - 3) Grate flows are determined from equations 7-2 and 7-3. Weir Flow $Q=(3PH^{1.5})/F$, Orifice Flow $Q=4.815*AH^{0.5}$

4) Pipe flows use the lesser of: 1) Inlet control equations 27 & 28, page 146 of HDS No. 5 - or - 2) Allowable Pipe Flow equation on page 11-9 of the DCM. Use Table 9, page 147-148, HDS No. 5 for formulas 26 & 27.

FUTURE POND H

WQCV Control Riser Calculations

TRIBUTARY AREA		86	acres	
DRAIN TIME		6	hr	
	<i>a</i>	0.7		
IMPERVIOUSNESS RATIO		0.38		
	<i>i</i>			
DEPTH OF OUTLET		1.6		
WQCV			0.12 inches	
WQCV DESIGN VOL			0.3 ac-ft	
	<i>K₄₀</i>		0.29	
AREA PER ROW ¹	<i>a</i>		0.57 in ²	
No. of columns		2	per riser	
Hole size			5/8 in	
Steel Plate Thickness			5/16 in	
			6 rows of holes per riser	
¹ AREA PER ROW PER RISER				
Actual area per row per riser:			0.61 in ²	
Actual area per riser:			3.7 in ²	
Actual area per riser:			0.025 ft ²	

TABLE SB-2							
Hole Dia (in)		Area per Row (in ²)					
Holes per Row		1	2	3	4	5	6
Min steel thickness		1/4	5/16	3/8	3/8	3/8	1/2
1/4	0.2500	0.05	0.10	0.15	0.20	0.25	0.29
5/16	0.3125	0.08	0.15	0.23	0.31	0.38	0.46
3/8	0.3750	0.11	0.22	0.33	0.44	0.55	0.66
7/16	0.4375	0.15	0.30	0.45	0.60	0.75	0.90
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
7/8	0.8750	0.60	1.20	1.80	2.41	3.01	3.61
1	1.0000	0.79	1.57	2.36	3.14	3.93	4.71
1 1/8	1.1250	0.99	1.99	2.98	3.98	4.97	5.96
1 1/4	1.2500	1.23	2.45	3.68	4.91	6.14	7.36
1 3/8	1.3750	1.48	2.97	4.45	5.94	7.42	8.91
1 1/2	1.5000	1.77	3.53	5.30	7.07	8.84	10.60
1 5/8	1.6250	2.07	4.15	6.22	8.30	10.37	12.44
1 3/4	1.7500	2.41	4.81	7.22	9.62	12.03	14.43
1 7/8	1.8750	2.76	5.52	8.28	11.04	13.81	16.57
2	2.0000	3.14	6.28	9.42	12.57	15.71	18.85

n = Number of columns of perforations

WINDINGWALK FILING 1 INTERIM CONDITION

Simulation Run: WW1-100 YR Reservoir: POND H

Start of Run: 01Jul2015, 00:00 Basin Model: WW Grading
End of Run: 02Jul2015, 00:00 Meteorologic Model: SCS TYPE IIA 100YR
Compute Time: 05Sep2017 13:11:34 Control Specifications: 24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow: 116(CFS) Date/Time of Peak Inflow: 01Jul2015, 06:12
Peak Outflow: 38 (CFS) Date/Time of Peak Outflow: 01Jul2015, 07:24
Total Inflow : 15.7 (AC-FT) Peak Storage: 6.7 (AC-FT)
Total Outflow: 13.3 (AC-FT) Peak Elevation: 6973.1 (FT)

Simulation Run: WW1-005 YR Reservoir: POND H

Start of Run: 01Jul2015, 00:00 Basin Model: WW Grading
End of Run: 02Jul2015, 00:00 Meteorologic Model: SCS TYPE IIA 005YR
Compute Time: 11Sep2017 13:11:34 Control Specifications: 24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow: 21 (CFS) Date/Time of Peak Inflow: 01Jul2015, 06:14
Peak Outflow: 3 (CFS) Date/Time of Peak Outflow: 01Jul2015, 08:24
Total Inflow : 3.4 (AC-FT) Peak Storage: 1.6 (AC-FT)
Total Outflow: 2.8 (AC-FT) Peak Elevation: 6971.2 (FT)

WINDINGWALK FILING 1 FUTURE CONDITION

Simulation Run: F-100 YR Reservoir: POND H

Start of Run: 01Jul2015, 00:00 Basin Model: Future SCS
End of Run: 02Jul2015, 00:00 Meteorologic Model: SCS TYPE IIA 100YR
Compute Time: 11Sep2017 13:11:34 Control Specifications: 24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow: 161(CFS) Date/Time of Peak Inflow: 01Jul2015, 12:06
Peak Outflow: 55 (CFS) Date/Time of Peak Outflow: 01Jul2015, 12:32
Total Inflow : 18.0 (AC-FT) Peak Storage: 7.6 (AC-FT)
Total Outflow: 15.7 (AC-FT) Peak Elevation: 6973.4 (FT)

Simulation Run: F-005 YR Reservoir: POND H

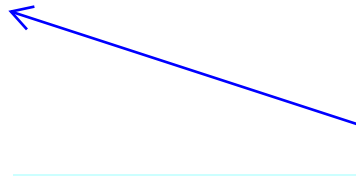
Start of Run: 01Jul2015, 00:00 Basin Model: Future SCS
End of Run: 02Jul2015, 00:00 Meteorologic Model: SCS TYPE IIA 005YR
Compute Time: 11Sep2017 13:26:34 Control Specifications: 24 HR-2 MIN.

Volume Units: AC-FT

Computed Results:

Peak Inflow: 36 (CFS)	Date/Time of Peak Inflow: 01Jul2015, 06:14
Peak Outflow: 4 (CFS)	Date/Time of Peak Outflow: 01Jul2015, 08:24
Total Inflow : 4.5 (AC-FT)	Peak Storage: 2.3 (AC-FT)
Total Outflow: 3.4 (AC-FT)	Peak Elevation: 6971.5 (FT)

Appendix E – Temporary Sedimentation



Are these the same temporary sediment ponds approved with the early grading drainage report? If so, remove from this report. Adding a section in the narrative that notes that temporary sediment have been designed per the approved [name of the early grading FDR] and installed with the early grading permit should be sufficient.

If these are new sediment ponds, then show the locations on the drainage map.

**WINDING WALK GRADING
TEMPORARY SEDIMENTATION SIZING**

TEMP POND 1

Tributary Area: 12.6 ac. Required Volume: 0.5 ac-ft Depth at Outlet: 1.9 ft.

Area required
per Row
1.9 in²

WS Elev: 7020.9

No. of columns: 4
Hole size: 3/4 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	7019	0	629	0.014	0.000	0.00
2	7020	1	14341	0.33	0.17	0.17
3	7021	2	19014	0.44	0.38	0.55
4	7022	3	25889	0.59	0.52	1.07
5	7023	4	32619	0.75	0.67	1.74
6	7024	5	40586	0.93	0.84	2.58

Minimum steel thickness		1	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11
7/8	0.8750	0.60	1.20	1.80	2.41	3.01	3.61
15/16	0.9375	0.69	1.38	2.07	2.76	3.45	4.14
1	1.0000	0.79	1.57	2.36	3.14	3.93	4.71
1 1/16	1.0625	0.89	1.77	2.66	3.55	4.43	5.32

* 4 Columns of 13/16 holes existing.

**WINDING WALK GRADING
TEMPORARY SEDIMENTATION SIZING**

TEMP POND 2

Tributary Area: Required Volume Depth at Outlet
16.4 ac. 0.7 ac-ft 6.0 ft.

Area required
per Row
0.6 in²

WS Elev: 7045.8

No. of
columns
3

Hole size
1/2 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	7039.8	0	0	0.000	0.000	0.00
2	7040	0.2	664	0.015	0.002	0.00
3	7041	1.2	3342	0.08	0.05	0.05
4	7042	2.2	4357	0.10	0.09	0.14
5	7043	3.2	5416	0.12	0.11	0.25
5	7044	4.2	6551	0.15	0.14	0.39
6	7045	5.2	7750	0.18	0.16	0.55
7	7046	6.2	9003	0.21	0.19	0.74

Minimum steel thickness		1	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
1/4	0.2500	0.05	0.10	0.15	0.20	0.25	0.29
5/16	0.3125	0.08	0.15	0.23	0.31	0.38	0.46
3/8	0.3750	0.11	0.22	0.33	0.44	0.55	0.66
7/16	0.4375	0.15	0.30	0.45	0.60	0.75	0.90
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11

**WINDING WALK GRADING
TEMPORARY SEDIMENTATION SIZING**

TEMP POND 3

Tributary Area: Required Volume Depth at Outlet
6.5 ac. 0.3 ac-ft 2.6 ft.

Area required
per Row
0.4 in²

WS Elev: 7036.8

No. of columns
2
Hole size
1/2 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	7034.2	0	0	0.000	0.000	0.00
2	7034.5	0.3	1336	0.031	0.005	0.00
3	7035	0.8	3750	0.09	0.03	0.03
4	7036	1.8	6389	0.15	0.12	0.15
5	7037	2.8	10332	0.24	0.19	0.34

Minimum steel thickness		1	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
1/4	0.2500	0.05	0.10	0.15	0.20	0.25	0.29
5/16	0.3125	0.08	0.15	0.23	0.31	0.38	0.46
3/8	0.3750	0.11	0.22	0.33	0.44	0.55	0.66
7/16	0.4375	0.15	0.30	0.45	0.60	0.75	0.90
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11

**WINDING WALK GRADING
TEMPORARY SEDIMENTATION SIZING**

TEMP POND 4

Tributary Area: Required Volume Depth at Outlet
32.0 ac. 1.3 ac-ft 6.3 ft.

Area required
 per Row
 1.1 in²

WS Elev: 7004.8

No. of
 columns
3

Hole size
 5/8 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	6998.5	0	0	0.000	0.000	0.00
2	6999	0.5	1701	0.04	0.01	0.01
3	7000	1.5	5758	0.13	0.09	0.10
4	7001	2.5	8748	0.20	0.17	0.26
5	7002	3.5	10282	0.24	0.22	0.48
6	7003	4.5	11887	0.27	0.25	0.73
7	7004	5.5	13562	0.31	0.29	1.03
8	7005	6.5	15307	0.35	0.33	1.36

Minimum steel thickness		1	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
1/4	0.2500	0.05	0.10	0.15	0.20	0.25	0.29
5/16	0.3125	0.08	0.15	0.23	0.31	0.38	0.46
3/8	0.3750	0.11	0.22	0.33	0.44	0.55	0.66
7/16	0.4375	0.15	0.30	0.45	0.60	0.75	0.90
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11

**WINDING WALK GRADING
TEMPORARY SEDIMENTATION SIZING**

TEMP POND 5

Tributary Area: Required Volume Depth at Outlet
4.4 ac. 0.2 ac-ft 3.7 ft.

Area required
per Row
0.2 in²

WS Elev: 7044.7

No. of
columns

Hole size

1

1/2 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	7041	0	0	0.000	0.000	0.00
2	7042	1	2030	0.05	0.02	0.02
3	7043	2	2607	0.06	0.05	0.08
4	7044	3	3247	0.07	0.07	0.14
5	7045	4	3950	0.09	0.08	0.23

TABLE SB-2

Minimum steel thickness		1	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
1/4	0.2500	0.05	0.10	0.15	0.20	0.25	0.29
5/16	0.3125	0.08	0.15	0.23	0.31	0.38	0.46
3/8	0.3750	0.11	0.22	0.33	0.44	0.55	0.66
7/16	0.4375	0.15	0.30	0.45	0.60	0.75	0.90
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11

**WINDING WALK GRADING
TEMPORARY SEDIMENTATION SIZING**

TEMP POND 6

Tributary Area: Required Volume Depth at Outlet
8.2 ac. 0.3 ac-ft 3.4 ft.

Area required
 per Row
 0.3 in²

WS Elev: 7033.4

No. of
 columns

Hole size

2

7/16 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	7030	0	0	0.000	0.000	0.00
2	7031	1	3890	0.09	0.04	0.04
3	7032	2	4705	0.11	0.10	0.14
4	7033	3	5582	0.13	0.12	0.26
5	7033.7	3.7	6043	0.14	0.09	0.35

TABLE SB-2

Minimum steel thickness	1	2	3	4	5	6	
	1/4	5/16	3/8	3/8	3/8	1/2	
1/4	0.25	0.05	0.10	0.15	0.20	0.25	0.29
5/16	0.31	0.08	0.15	0.23	0.31	0.38	0.46
3/8	0.38	0.11	0.22	0.33	0.44	0.55	0.66
7/16	0.44	0.15	0.30	0.45	0.60	0.75	0.90
1/2	0.50	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.56	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.63	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.69	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.75	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.81	0.52	1.04	1.56	2.07	2.59	3.11

**WINDING WALK GRADING
TEMPORARY SEDIMENTATION SIZING**

TEMP POND 7

Tributary Area: Required Volume Depth at Outlet
8.4 ac. 0.3 ac-ft 6.6 ft.

Area required
per Row
0.2 in²

WS Elev: 7006.6

No. of
columns
1

Hole size
1/2 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	7000	0	0	0.000	0.000	0.00
2	7001	1	910	0.02	0.01	0.01
3	7002	2	1301	0.03	0.03	0.04
4	7003	3	1754	0.04	0.04	0.07
5	7004	4	2265	0.05	0.05	0.12
6	7005	5	2835	0.07	0.06	0.18
7	7006	6	3462	0.08	0.07	0.25
8	7006.95	6.95	4104	0.09	0.08	0.33

TABLE SB-2

Minimum steel thickness		1	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
1/4	0.2500	0.05	0.10	0.15	0.20	0.25	0.29
5/16	0.3125	0.08	0.15	0.23	0.31	0.38	0.46
3/8	0.3750	0.11	0.22	0.33	0.44	0.55	0.66
7/16	0.4375	0.15	0.30	0.45	0.60	0.75	0.90
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11

**WINDING WALK GRADING
 TEMPORARY SEDIMENTATION SIZING
 LAMBERT POND**

Tributary Area: Required Volume Depth at Outlet
8.1 ac. 0.3 ac-ft 4.1 ft.

Area required
 per Row
 0.2 in²

WS Elev: 6988.6

No. of
 columns
2 Hole size
 5/16 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	6984.5	0	32	0.001	0.000	0.00
2	6985	0.5	884	0.02	0.01	0.01
3	6986	1.5	2594	0.06	0.04	0.05
4	6987	2.5	3881	0.09	0.07	0.12
5	6988	3.5	5238	0.12	0.10	0.22
6	6989	4.5	6669	0.15	0.14	0.36
7	6990	5.5	8172	0.19	0.17	0.53
8	6991	6.5	9748	0.22	0.21	0.74

Minimum steel thickness		1	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
1/4	0.2500	0.05	0.10	0.15	0.20	0.25	0.29
5/16	0.3125	0.08	0.15	0.23	0.31	0.38	0.46
3/8	0.3750	0.11	0.22	0.33	0.44	0.55	0.66
7/16	0.4375	0.15	0.30	0.45	0.60	0.75	0.90
1/2	0.5000	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.5625	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.6250	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.6875	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.7500	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.8125	0.52	1.04	1.56	2.07	2.59	3.11

**WINDING WALK GRADING
TEMPORARY SEDIMENTATION SIZING**

POND H TEMP POND

Tributary Area: Required Volume Depth at Outlet
76.0 ac. 3.1 ac-ft 3.1 ft.

Area required
per Row
3.0 in²

WS Elev: 6971.6

No. of
columns
6

Hole size
3/4 in

STAGE			STORAGE			
STAGE	ELEV	HEIGHT	AREA		VOLUME	
			sqft	acre	acft	cum acft
1	6968.5	0	0	0.000	0.000	0.00
2	6969	0.5	4127	0.095	0.024	0.02
3	6970	1.5	37142	0.85	0.47	0.50
4	6971	2.5	82122	1.89	1.37	1.87
5	6972	3.5	110378	2.53	2.21	4.08
6	6973	4.5	125282	2.88	2.71	6.78
7	6974	5.5	138866	3.19	3.03	9.81
8	6975	6.5	153432	3.52	3.36	13.17

Minimum steel thickness		1	2	3	4	5	6
		1/4	5/16	3/8	3/8	3/8	1/2
3/8	0.38	0.11	0.22	0.33	0.44	0.55	0.66
7/16	0.44	0.15	0.30	0.45	0.60	0.75	0.90
1/2	0.50	0.20	0.39	0.59	0.79	0.98	1.18
9/16	0.56	0.25	0.50	0.75	0.99	1.24	1.49
5/8	0.63	0.31	0.61	0.92	1.23	1.53	1.84
11/16	0.69	0.37	0.74	1.11	1.48	1.86	2.23
3/4	0.75	0.44	0.88	1.33	1.77	2.21	2.65
13/16	0.81	0.52	1.04	1.56	2.07	2.59	3.11
7/8	0.88	0.60	1.20	1.80	2.41	3.01	3.61
15/16	0.94	0.69	1.38	2.07	2.76	3.45	4.14

Appendix F – Soil Resource Report



United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for El Paso County Area, Colorado

Windingwalk and the Enclave



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

Contents

Preface	2
How Soil Surveys Are Made	5
Soil Map	8
Soil Map.....	9
Legend.....	10
Map Unit Legend.....	11
Map Unit Descriptions.....	11
El Paso County Area, Colorado.....	13
19—Columbine gravelly sandy loam, 0 to 3 percent slopes.....	13
83—Stapleton sandy loam, 3 to 8 percent slopes.....	14
References	16

How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

Custom Soil Resource Report

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

Custom Soil Resource Report

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

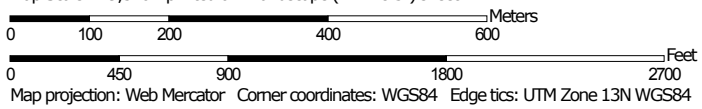
Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map




Map Scale: 1:9,510 if printed on A landscape (11" x 8.5") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)




















Soils







 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

Special Point Features






-  Blowout
-  Borrow Pit
-  Clay Spot
-  Closed Depression
-  Gravel Pit
-  Gravelly Spot
-  Landfill
-  Lava Flow
-  Marsh or swamp
-  Mine or Quarry
-  Miscellaneous Water
-  Perennial Water
-  Rock Outcrop
-  Saline Spot
-  Sandy Spot
-  Severely Eroded Spot
-  Sinkhole
-  Slide or Slip
-  Sodic Spot

-  Spoil Area
-  Stony Spot
-  Very Stony Spot
-  Wet Spot
-  Other
-  Special Line Features


Water Features

 Streams and Canals

Transportation

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
 Survey Area Data: Version 15, Oct 10, 2017

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: May 22, 2016—Mar 9, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	15.8	5.5%
83	Stapleton sandy loam, 3 to 8 percent slopes	272.3	94.5%
Totals for Area of Interest		288.1	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

Custom Soil Resource Report

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

El Paso County Area, Colorado

19—Columbine gravelly sandy loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 367p
Elevation: 6,500 to 7,300 feet
Mean annual precipitation: 14 to 16 inches
Mean annual air temperature: 46 to 50 degrees F
Frost-free period: 125 to 145 days
Farmland classification: Not prime farmland

Map Unit Composition

Columbine and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Columbine

Setting

Landform: Fan terraces, fans, flood plains
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium

Typical profile

A - 0 to 14 inches: gravelly sandy loam
C - 14 to 60 inches: very gravelly loamy sand

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Well drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Very low (about 2.5 inches)

Interpretive groups

Land capability classification (irrigated): 4e
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: A
Ecological site: Gravelly Foothill (R049BY214CO)
Hydric soil rating: No

Minor Components

Fluvaquentic haplaquolls

Percent of map unit:
Landform: Swales
Hydric soil rating: Yes

Other soils

Percent of map unit:

Hydric soil rating: No

Pleasant

Percent of map unit:

Landform: Depressions

Hydric soil rating: Yes

83—Stapleton sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 369z

Elevation: 6,500 to 7,300 feet

Mean annual precipitation: 14 to 16 inches

Mean annual air temperature: 46 to 48 degrees F

Frost-free period: 125 to 145 days

Farmland classification: Not prime farmland

Map Unit Composition

Stapleton and similar soils: 80 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Stapleton

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Sandy alluvium derived from arkose

Typical profile

A - 0 to 11 inches: sandy loam

Bw - 11 to 17 inches: gravelly sandy loam

C - 17 to 60 inches: gravelly loamy sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water storage in profile: Low (about 4.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: B

Custom Soil Resource Report

Ecological site: Gravelly Foothill (R049BY214CO)
Hydric soil rating: No

Minor Components

Fluvaquentic haplaquolls

Percent of map unit:
Landform: Swales
Hydric soil rating: Yes

Other soils

Percent of map unit:
Hydric soil rating: No

Pleasant

Percent of map unit:
Landform: Depressions
Hydric soil rating: Yes

References

- American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.
- American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.
- Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.
- Federal Register. July 13, 1994. Changes in hydric soils of the United States.
- Federal Register. September 18, 2002. Hydric soils of the United States.
- Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.
- National Research Council. 1995. Wetlands: Characteristics and boundaries.
- Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_054262
- Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053577
- Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053580
- Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.
- United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.
- United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/home/?cid=nrcs142p2_053374
- United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. <http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/landuse/rangepasture/?cid=stelprdb1043084>

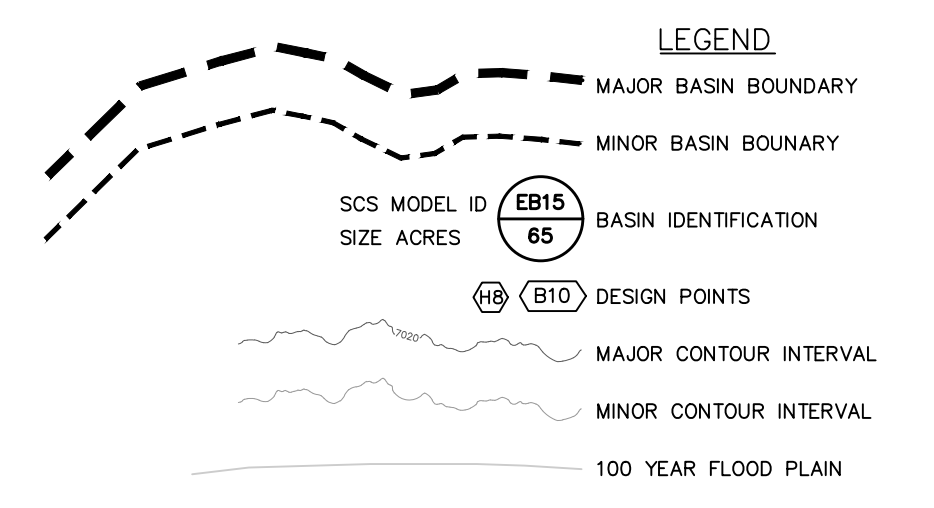
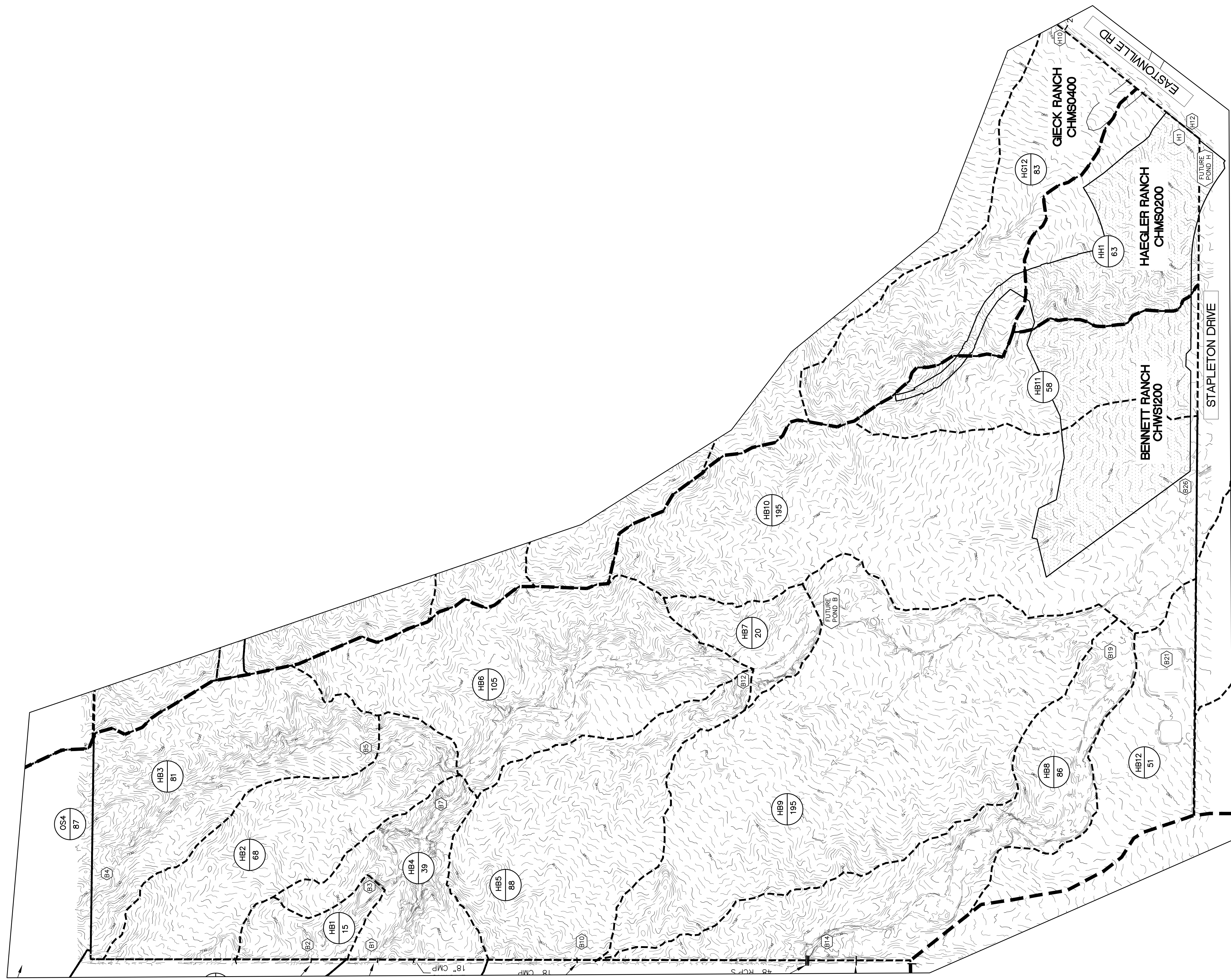
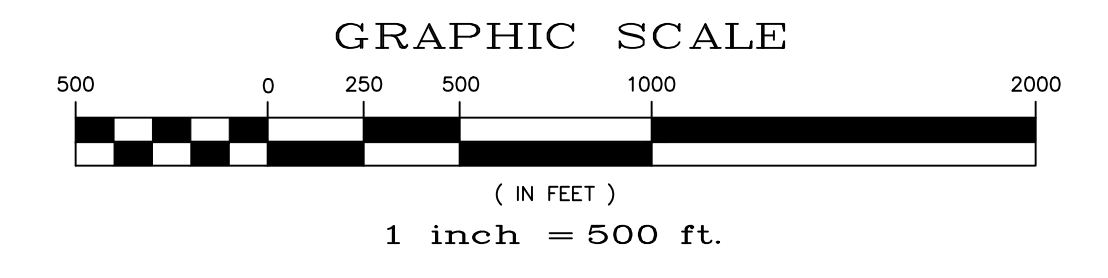
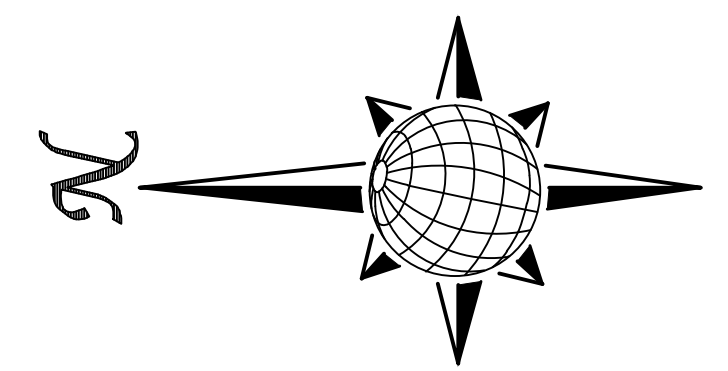
Custom Soil Resource Report

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf

SCS DRAINAGE MAP MERIDIAN RANCH WINDINGWALK FILING 1



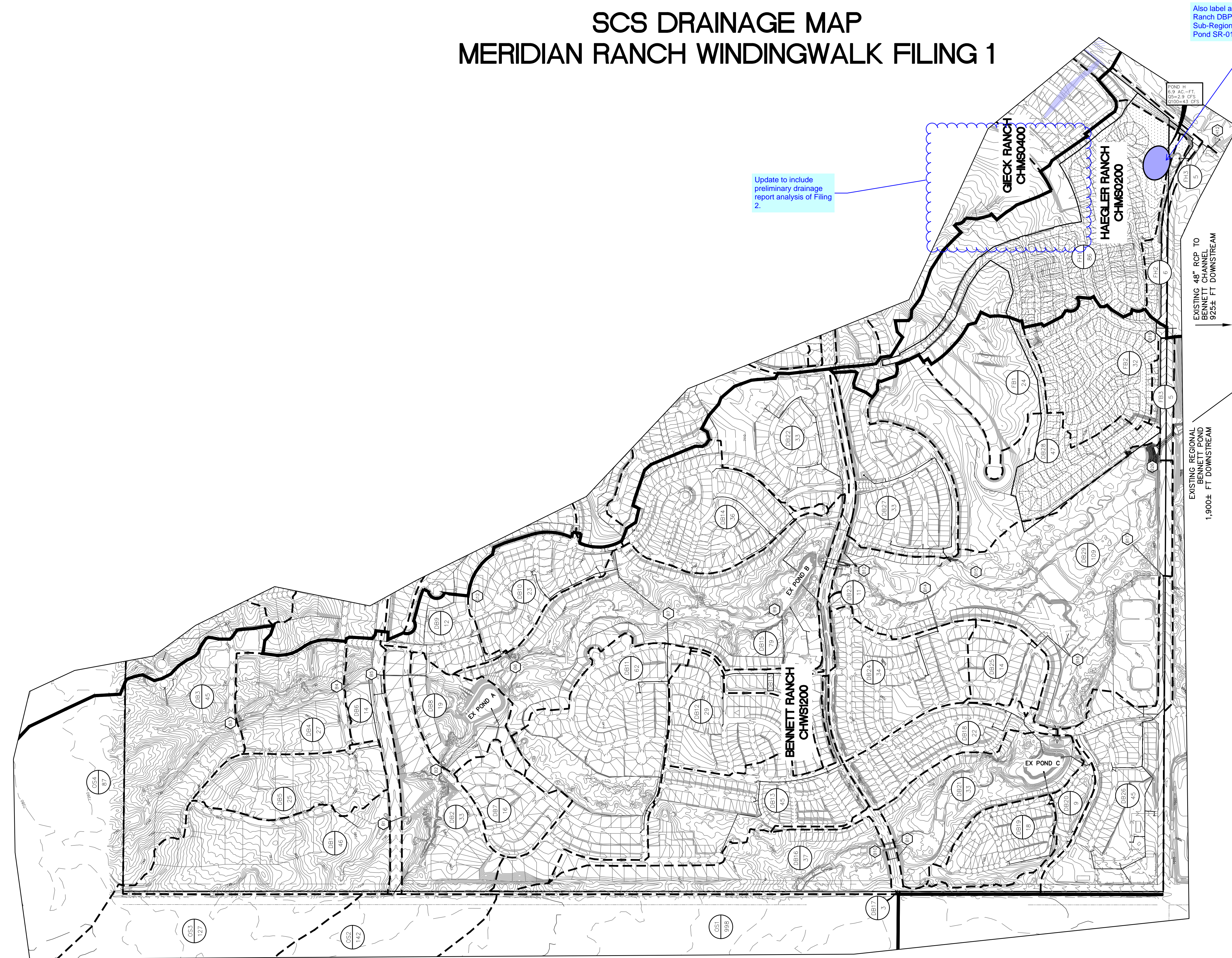
TECH CONTRACTORS
11886 STAPLETON DR
FALCON, CO 80831
TELEPHONE: 719.495.7444

HISTORIC CONDITIONS

FIGURE 4 DEC 2017

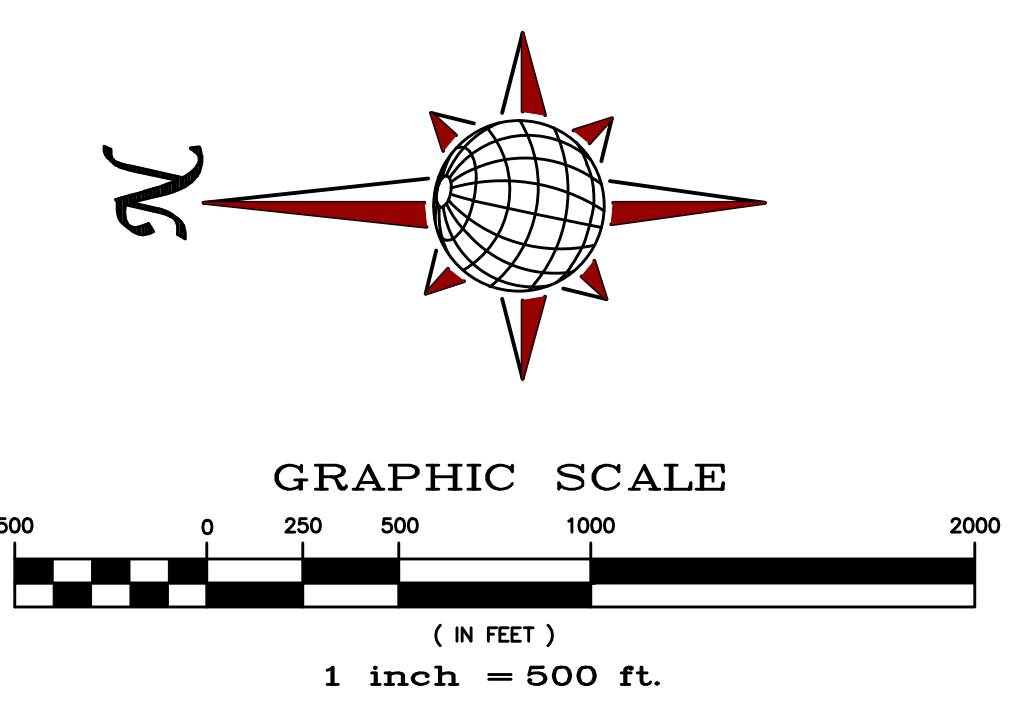
S:\C:\proj\Winding Walk Filing 1\DWG\PLAN SHEETS\BASIN\MAWS\FDR\FDR-WW1-SCS-HISTORIC.dwg, Fig. 4, 12/12/2017 9:43:36 AM

SCS DRAINAGE MAP MERIDIAN RANCH WINDINGWALK FILING 1



LEGEND

- MAJOR BASIN BOUNDARY
- MINOR BASIN BOUNDARY
- SCS MODEL ID (EB15) BASIN IDENTIFICATION
- SIZE ACRES (65)
- DESIGN POINTS (DB10)
- MAJOR CONTOUR INTERVAL
- MINOR CONTOUR INTERVAL
- 100 YEAR FLOOD PLAN



EXISTING 48" RCP TO BENNETT CHANNEL 925± FT DOWNSTREAM

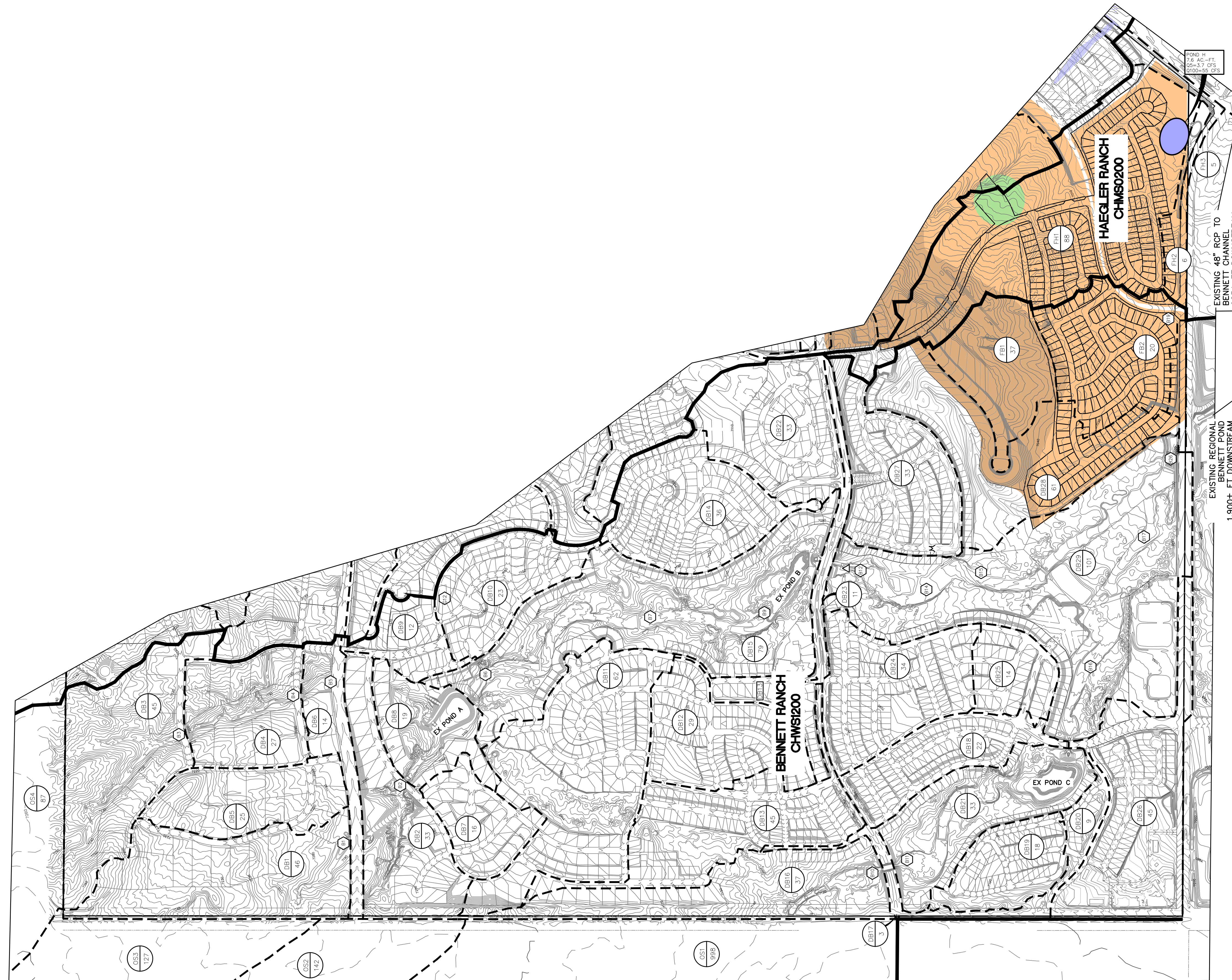
EXISTING REGIONAL BENNETT POND 1,900± FT DOWNSTREAM

INTERIM CONDITIONS

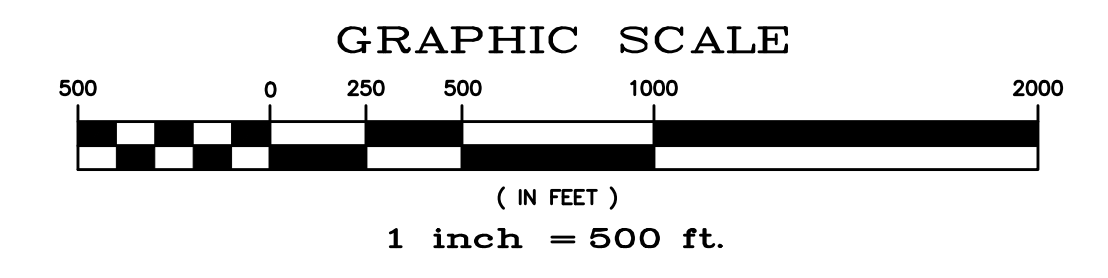
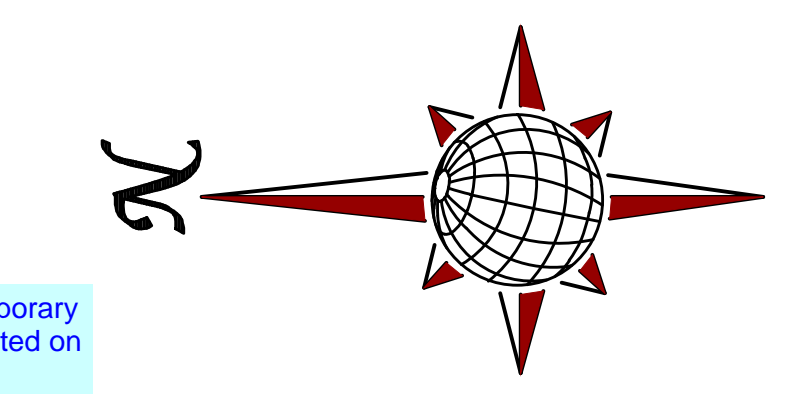
TECH CONTRACTORS
11886 STAPLETON DRIVE
FALCON, CO 80831
TELEPHONE: 719.495.7444

S:\C:\p\proj\Winding Walk Filing 1\DWG\PLAN_SHEETS\BASIN_MAPS\FDR\FDR-WV1-SCS-FILING 1.dwg Fig 5: 12/12/2017 9:53:51 AM

SCS DRAINAGE MAP MERIDIAN RANCH WINDINGWALK FILING 1



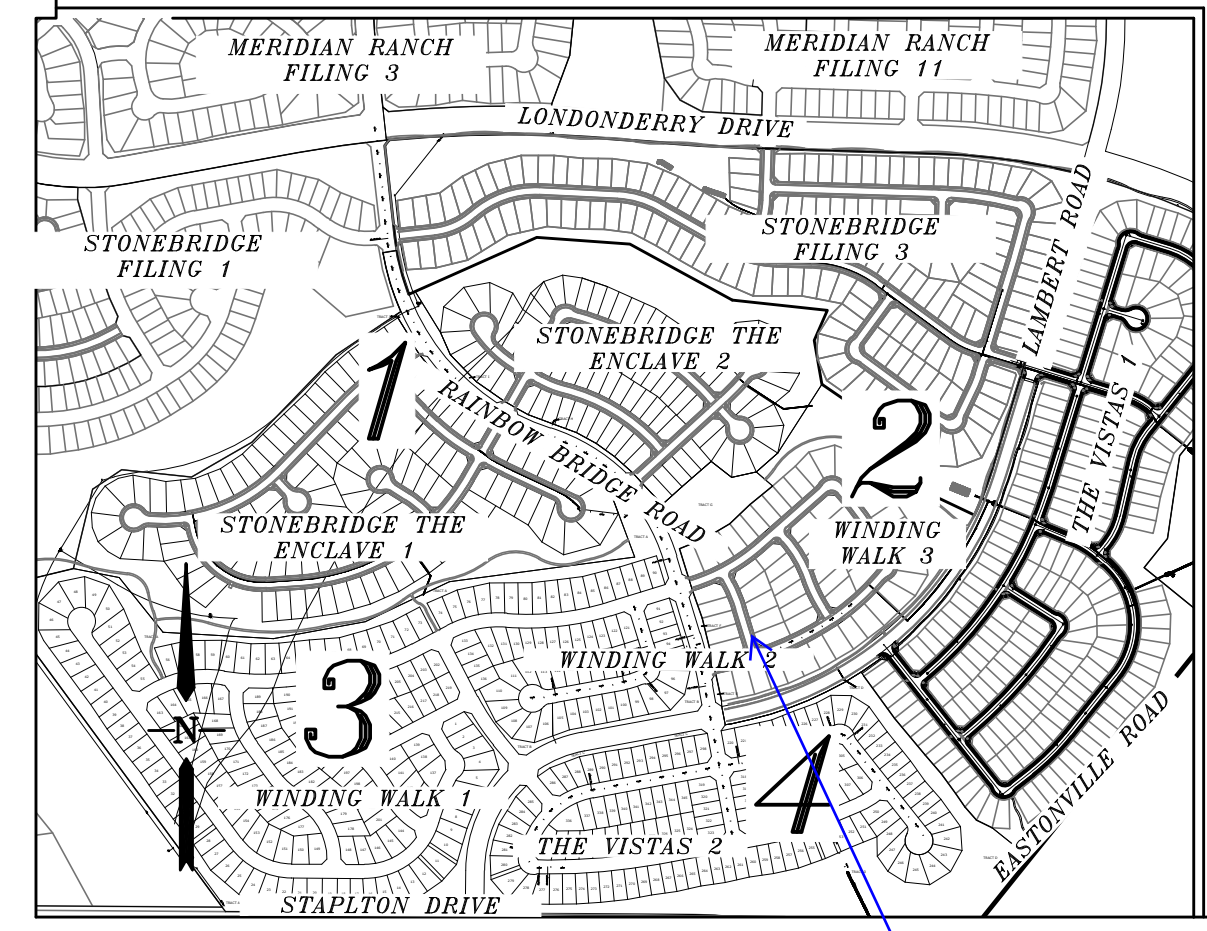
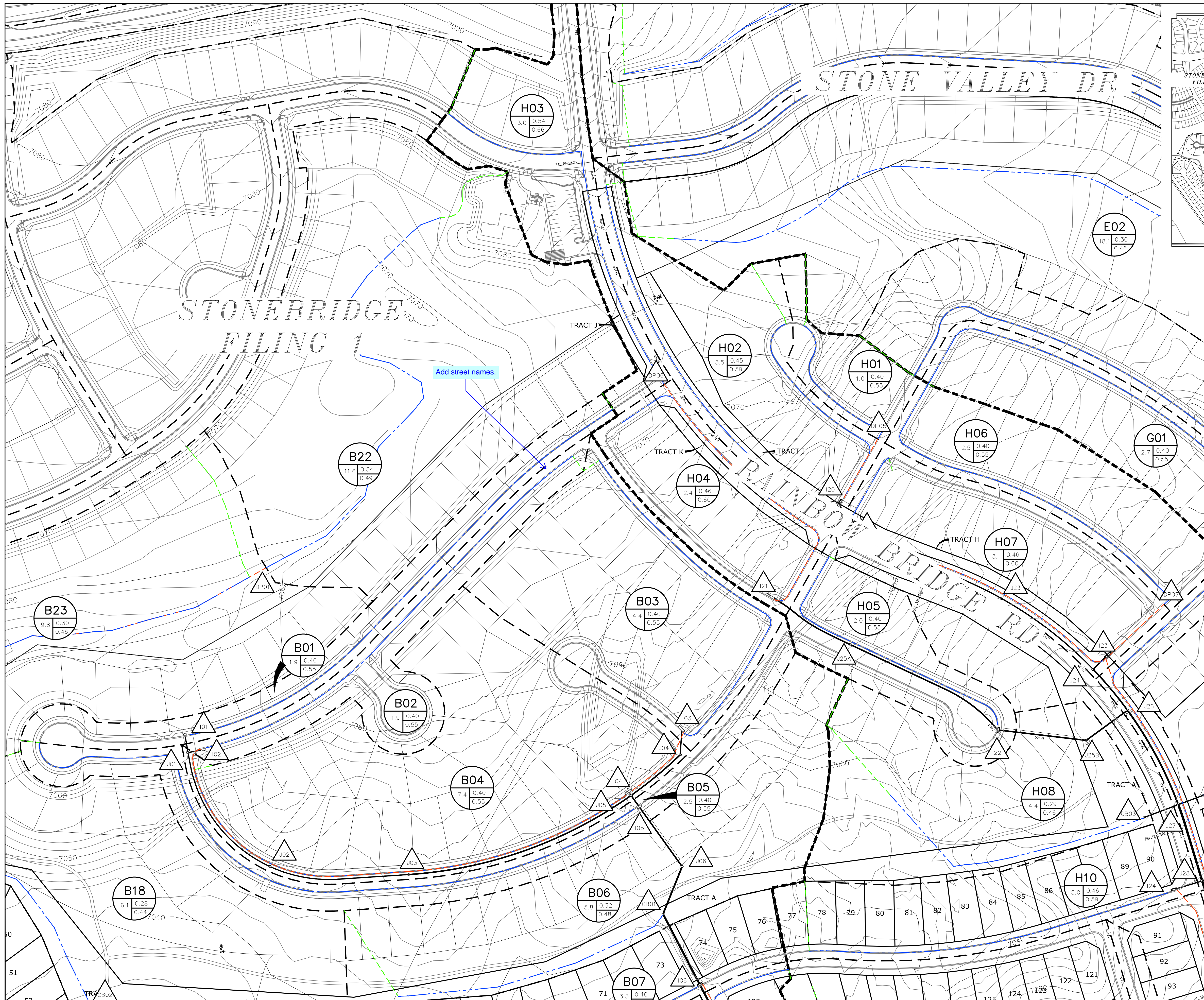
LEGEND	
	MAJOR BASIN BOUNDARY
	MINOR BASIN BOUNDARY
SCS MODEL ID SIZE ACRES	BASIN IDENTIFICATION
	DESIGN POINTS
	MAJOR CONTOUR INTERVAL
	MINOR CONTOUR INTERVAL
	100 YEAR FLOOD PLAN



FUTURE CONDITIONS

TECH CONTRACTORS
11886 STAPLETON DRIVE
FALCON, CO 80831
TELEPHONE: 719.495.7444

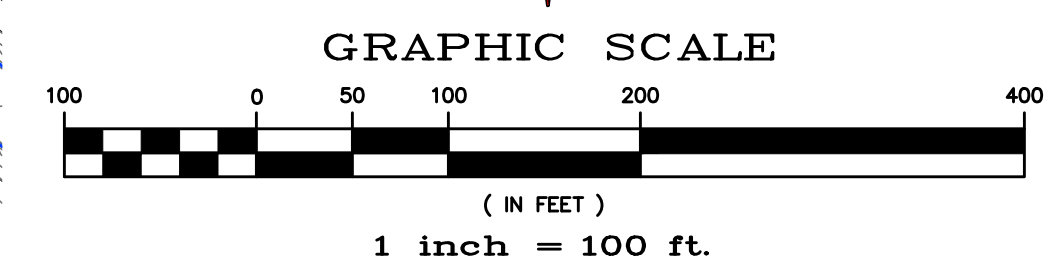
S:\Cadd\proj\Winding Walk Filing 1\DWG\PLAN SHEETS\BASIN\MAWS\FDR\FDR-MW1-SCS-FUTURE.dwg, Fig 6, 12/12/2017, 10:03:46 AM



INDEX MAP
N.T.S.



Update keymap to match the PUD which only shows filing 1 & 2



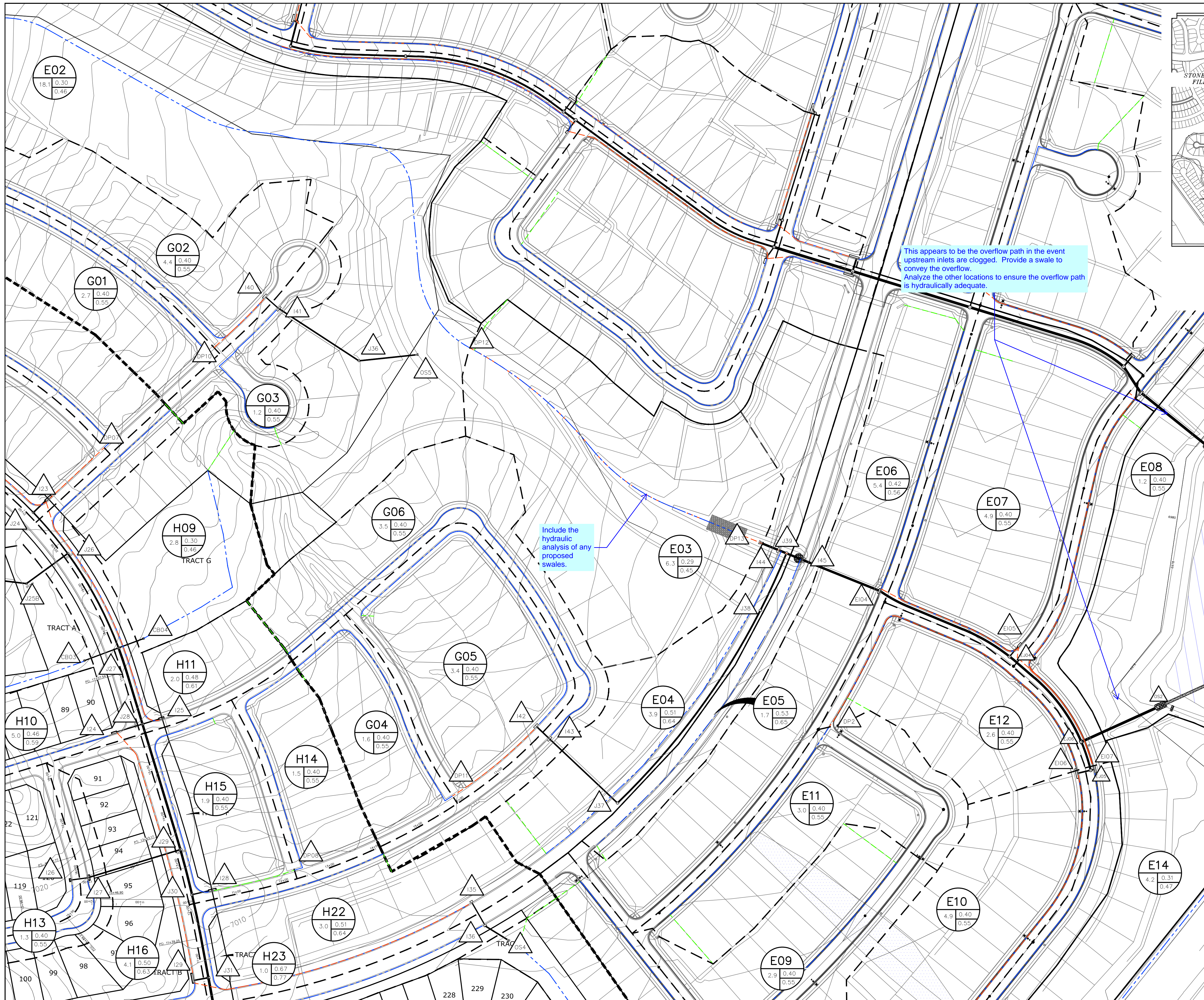
- BASIN DESIGNATION
- SUB-WATERSHED DESIGNATION
- MINOR/MAJOR STORM COEFFICIENT
- BASIN AREA IN ACRES
- DESIGN POINT DESIGNATION
- MAJOR BASIN BOUNDARY
- SUB-BASIN BOUNDARY
- 6130 EXISTING CONTOUR
- 6130 PROPOSED CONTOUR
- PROPOSED STORM SEWER
- INITIAL OVERLAND TIME (Ti)
- TRAVEL TIME (Tt)
- OVERLAND TIME (To)

INTERSECTION OF WOODMEN RD AND MERIDIAN ROAD AT SW CORNER (BRASS CAP W/ NO. GF-9)
ELEVATION = 6874.00

NOTE:
COUNTY PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN CRITERIA. THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR THE COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

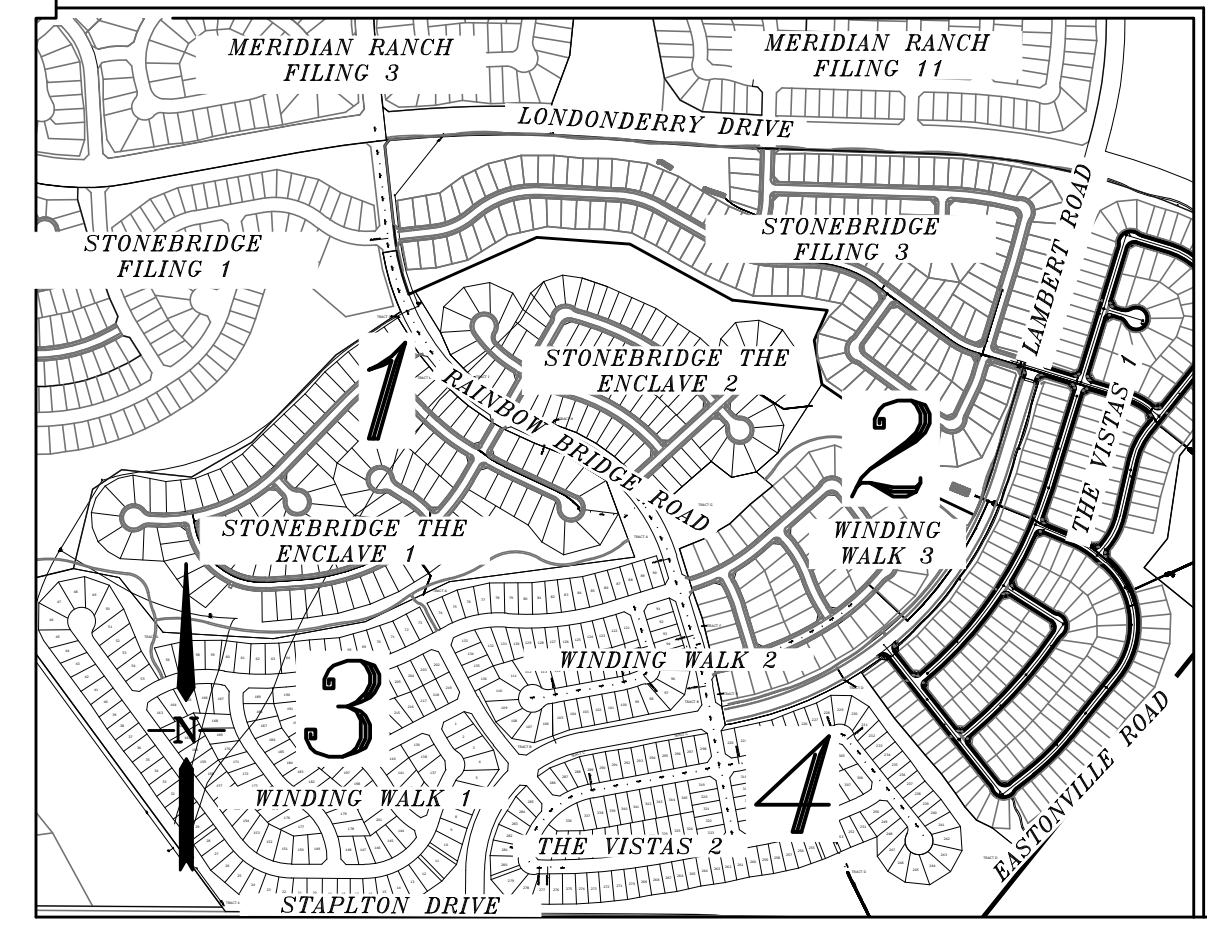
Add a summary of the minor/major flowrates at design points.

Revisions		No.	Date	Appr.	Date
TECH CONTRACTORS 12311 REX ROAD FALCON, CO 80831 TELEPHONE: 719.495.7444 FAX: 719.495.2457					
MERIDIAN RANCH					
RATIONAL DRAINAGE MAP FINAL DRAINAGE REPORT WINDING WALK FILING 3					
Scale	AS SHOWN	1 of 4	Date	DEC. 2007	
Drawn by	LOA	Checked by	TAK		

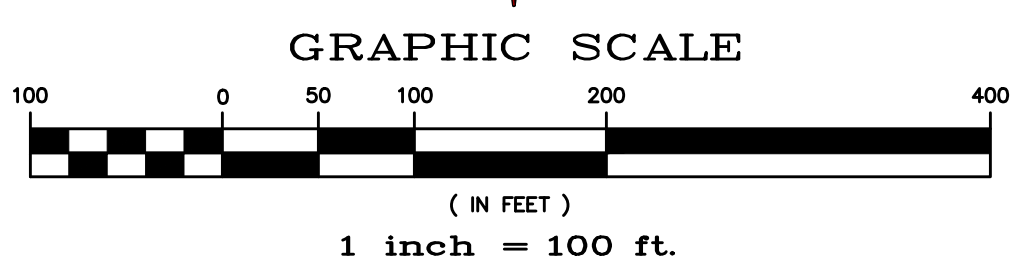


This appears to be the overflow path in the event upstream inlets are clogged. Provide a swale to convey the overflow. Analyze the other locations to ensure the overflow path is hydraulically adequate.

Include the hydraulic analysis of any proposed swales.



INDEX MAP
N.T.S.

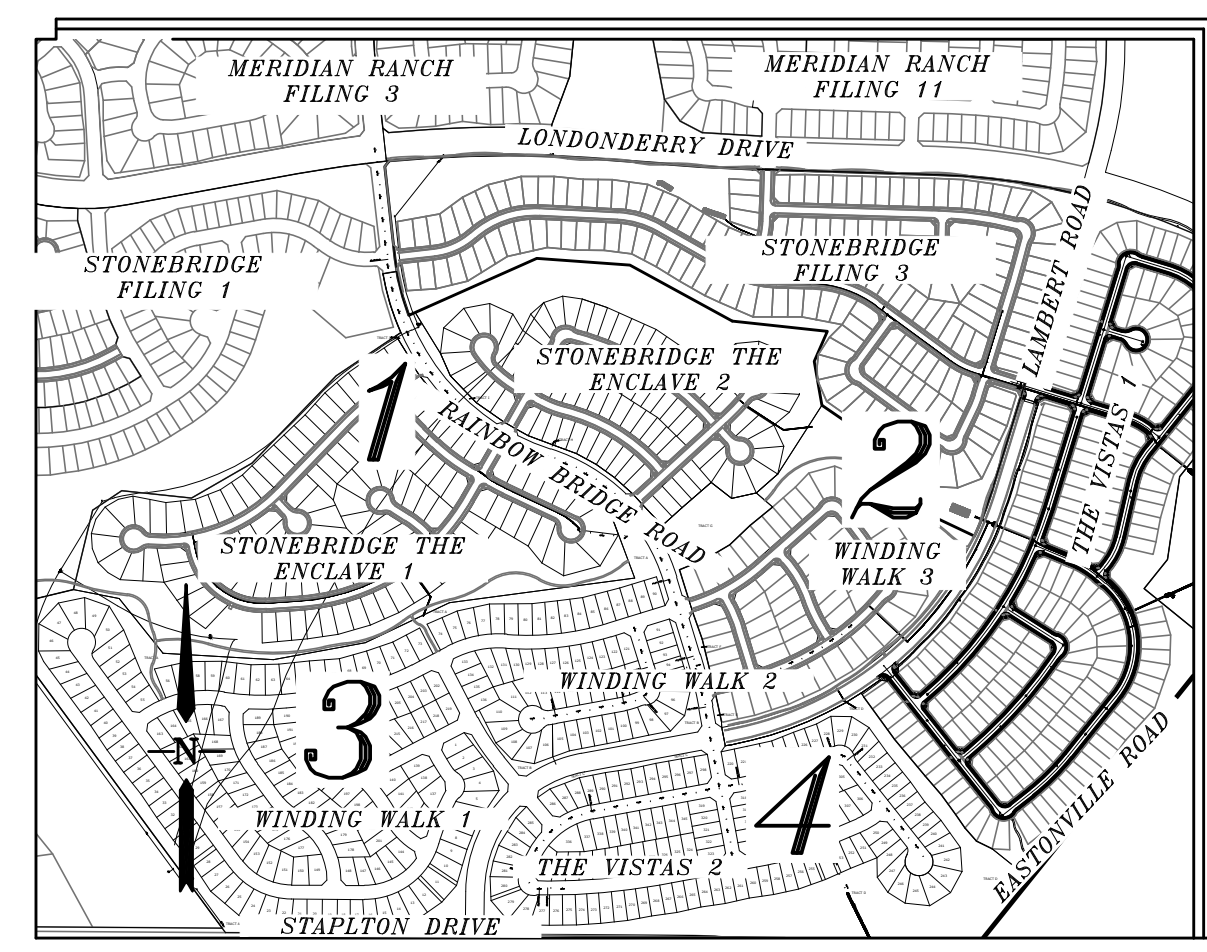
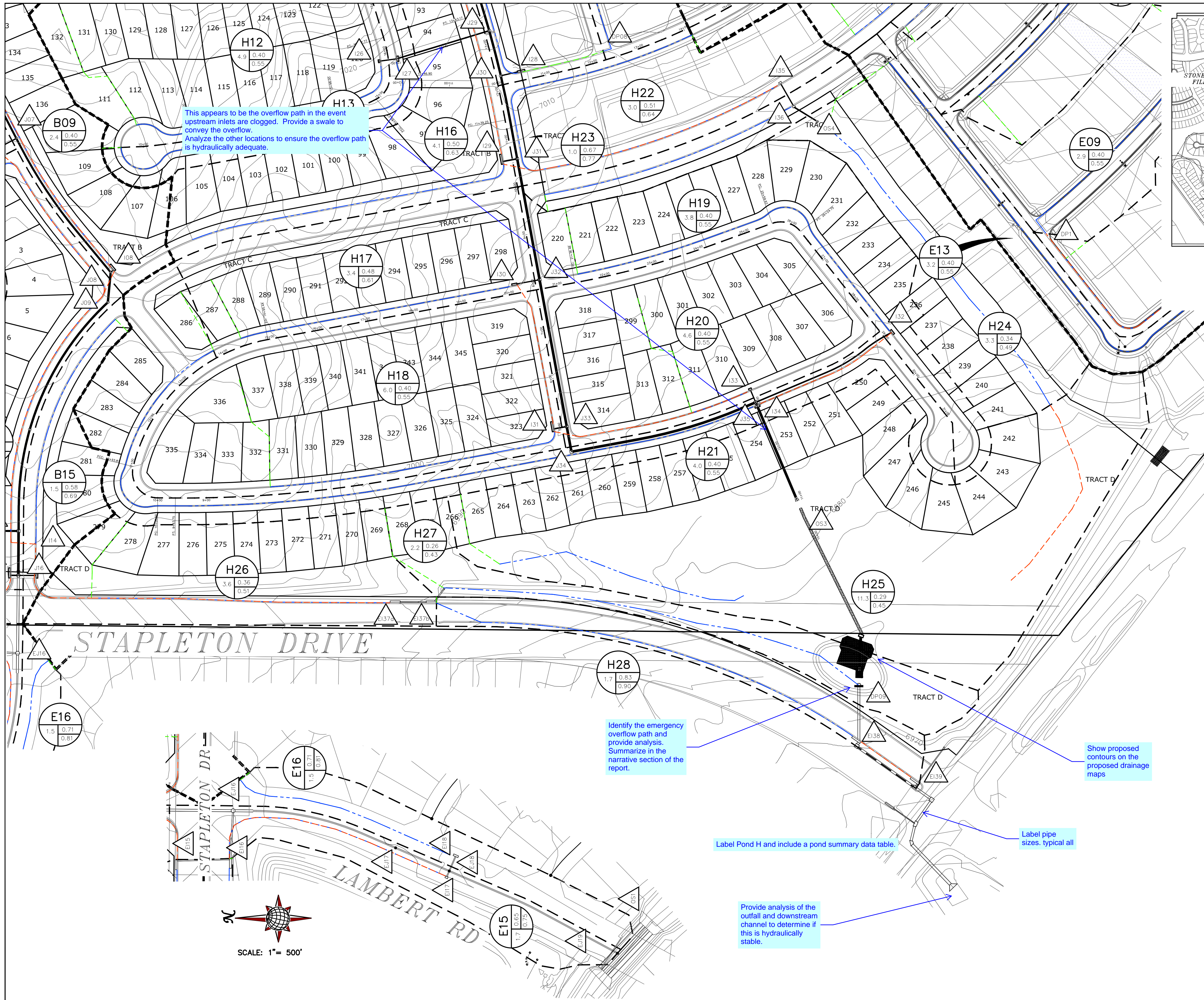


- BASIN DESIGNATION
- SUB-WATERSHED DESIGNATION
- MINOR/MAJOR STORM COEFFICIENT
- BASIN AREA IN ACRES
- DESIGN POINT DESIGNATION
- MAJOR BASIN BOUNDARY
- SUB-BASIN BOUNDARY
- 6130 EXISTING CONTOUR
- 6130 PROPOSED CONTOUR
- PROPOSED STORM SEWER
- INITIAL OVERLAND TIME (Ti)
- TRAVEL TIME (Tt)
- OVERLAND TIME (To)

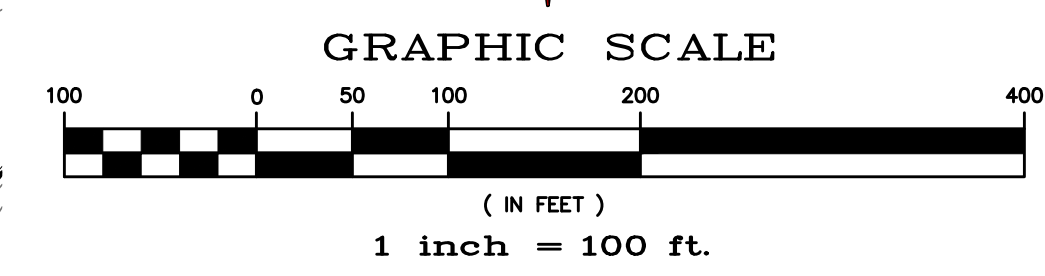
INTERSECTION OF WOODMEN RD AND MERIDIAN ROAD AT SW CORNER (BRASS CAP W/ NO. GF-9)
ELEVATION = 6874.00

NOTE:
COUNTY PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN CRITERIA. THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR THE COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

Scale	AS SHOWN	2 of 4	Date	DEC 2007
	Drawn by	LOA		Checked by
RATIONAL DRAINAGE MAP FINAL DRAINAGE REPORT WINDINGWALK FILING 1		TECH CONTRACTORS 12311 REX ROAD FALCON, CO 80831 TELEPHONE: 719.495.7444 FAX: 719.495.2457		
MERIDIAN RANCH		Revisions		
		No.	Date	Appr.
		-	-	-
		-	-	-



INDEX MAP
N.T.S.



- BASIN DESIGNATION
- SUB-WATERSHED DESIGNATION
- MINOR/MAJOR STORM COEFFICIENT
- BASIN AREA IN ACRES
- DESIGN POINT DESIGNATION
- MAJOR BASIN BOUNDARY
- SUB-BASIN BOUNDARY
- EXISTING CONTOUR
- PROPOSED CONTOUR
- PROPOSED STORM SEWER
- INITIAL OVERLAND TIME (Ti)
- TRAVEL TIME (Tt)
- OVERLAND TIME (To)

INTERSECTION OF WOODMEN RD AND MERIDIAN ROAD AT SW CORNER (BRASS CAP W/ NO. GF-9)
ELEVATION = 6874.00

NOTE:
COUNTY PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN CRITERIA. THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, DIMENSIONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY THROUGH THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR THE COMPLETENESS AND/OR ACCURACY OF THIS DOCUMENT.

This appears to be the overflow path in the event upstream inlets are clogged. Provide a swale to convey the overflow. Analyze the other locations to ensure the overflow path is hydraulically adequate.

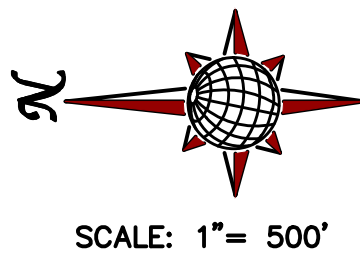
Identify the emergency overflow path and provide analysis. Summarize in the narrative section of the report.

Label Pond H and include a pond summary data table.

Show proposed contours on the proposed drainage maps

Label pipe sizes, typical all

Provide analysis of the outfall and downstream channel to determine if this is hydraulically stable.



SCALE: 1" = 500'

Revisions		No.	
Date	Init.	Date	Appr.

TECH CONTRACTORS 12311 REX ROAD FALCON, CO 80831 TELEPHONE: 719.495.7444 FAX: 719.495.2457		
RATIONAL DRAINAGE MAP FINAL DRAINAGE REPORT WINDING WALK FILING 1		
Drawn by AS SHOWN	Checked by TAK	Date DEC 2007
4	of 4	4