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SUBSURFACE SOIL INVESTIGATION **WINDINGWALK FILINGS 1 AND 2 AND** STONEBRIDGE THE ENCLAVE FILINGS 4 AND 5 STAPLETON DRIVE AND EASTONVILLE ROAD **EL PASO COUNTY, COLORADO**

Prepared for:

Tech Contractors

Add a section addressing EPC **Drainage Criteria Manual section** 11.3.3 regarding "recommendations for the foundation preparation and 3575 Kenyon Street, Suite 20(embankment construction for all San Diego, California 92110 permanent detention facilities."

Attn: Mr. Raul Guzman

addressed with rough grading plan. letter from that project added to Appendix C

November 8, 2017 Revised December 21, 2017 Revised January 2, 2018

Respectfully Submitted,

ENTECH ENGINEERING, INC.

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Encl.

Entech Job No. 171198 AAprojects/2017/171198 SSI_rev2 added

Add "PCD File No. PUDSP-18-002 & SF-18-002"

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SUBSURFACE SOIL INVESTIGATION WINDINGWALK FILINGS 1 AND 2 AND STONEBRIDGE THE ENCLAVE FILINGS 4 AND 5 STAPLETON DRIVE AND EASTONVILLE ROAD EL PASO COUNTY, COLORADO

1.0 INTRODUCTION

The project consists of the development of the site for the construction of single-family residences. Development is expected to include site grading, installation of subsurface utilities, roadways, and drainage structures. The site is Windingwalk Filings 1 and 2 and Stonebridge The Enclave Filings 4 and 5, north of Stapleton Drive and west of Eastonville Road in El Paso County, Colorado. The approximate location of the project site is shown on the Vicinity Location Map, Figure 1. The test boring locations are shown on Figure 2, the Test Boring Location Plan.

This report describes the subsurface investigation conducted for the site and provides recommendations for development design and construction. The Subsurface Soil Investigation included the drilling of fifty-five test borings across the site, collecting samples of soil, and conducting a geotechnical evaluation of the investigation findings. All drilling and subsurface investigation activities were performed by Entech Engineering, Inc. (Entech). The contents of this report, including the geotechnical evaluation and recommendations, are subject to the limitations and assumptions presented in Section 17.0.

2.0 PROJECT AND SITE DESCRIPTION

The project will consist of developing the site for single family residential structures. The planned lots are located in Windingwalk Filings 1 and 2 and Stonebridge The Enclave Filings 4 and 5, north of Stapleton Drive and west of Eastonville Road, just north of Falcon, Colorado in El Paso County. Investigation was performed at predetermined locations designated based on the roadway alignment and proposed grading on the site plan provided to Entech. At the time of drilling, the site was vacant and not developed. Enclave Scenic Way and Fairway Glen Circle have been graded prior to this investigation. The site has general gradual slope towards the south. Vegetation consisted of field grasses and weeds. Existing residences were located to the north and northeast of the site. A golf course exists west of the site. Stapleton Drive borders the southern boundary of the site.

3.0 SUBSURFACE EXPLORATIONS AND LABORATORY TESTING

Subsurface conditions on the site were explored by drilling fifty-five test borings at the approximate locations shown on Figure 2. The borings were drilled to depths of 20 to 30 feet below the existing ground surface (bgs). The drilling was performed using a truck-mounted, continuous flight auger-drilling rig supplied and operated by Entech. Boring logs descriptive of the subsurface conditions encountered during drilling are presented in Appendix A. At the conclusion and subsequent to drilling, observations for groundwater levels were made in each of the open boreholes.

Soil and bedrock samples were obtained from the borings utilizing the Standard Penetration Test (ASTM D-1586) using 2-inch O.D. split-barrel and California samplers. Results of the Standard Penetration Test (SPT) are included on the boring logs in terms of N-values expressed in blows per foot (bpf). Soil and bedrock samples recovered from the borings were visually classified and recorded on the boring logs. The soil and bedrock classifications were later verified utilizing laboratory testing and grouped by soil type. The soil and bedrock type numbers are included on the boring logs. It should be understood that the soil and bedrock descriptions shown on the boring logs may vary between boring location and sample depth. It should also be noted that the lines of stratigraphic separation shown on the boring logs

represent approximate boundaries between soil and bedrock types and the actual stratigraphic transitions may be more gradual or variable with location.

Water content testing (ASTM D-2216) was performed on the samples recovered from the borings, and the results are shown on the boring logs. Grain-Size Analysis (ASTM D-422) and Atterberg Limits testing (ASTM D-4318) were performed on selected samples to assist in classifying the materials encountered in the borings. Volume change testing was performed on selected samples using the Swell/Consolidation Test (ASTM D-4546) and the FHA Swell Test in order to evaluate potential expansion/compression characteristics of the soil and bedrock. Soluble sulfate testing was performed on select soil samples to evaluate the potential for below grade degradation of concrete due to sulfate attack. The Laboratory Testing Results are summarized on Table 1 and are presented in Appendix B.

4.0 SUBSURFACE CONDITIONS

Three soil types and three bedrock types were encountered in the test borings drilled for the subsurface investigation: Type 1A: silty to clayey sand fill (SM, SC), Type 1: slightly silty to silty sand and clayey sand (SM-SW, SM, SC), Type 2: sandy to very sandy clay (CL), Type 3: slightly silty to very silty and clayey to very clayey sandstone (SM-SW, SM, SC), Type 4: sandy to very sandy claystone (CL), and Type 5: clayey, sandy siltstone (ML). Soils were classified in accordance with the Unified Soil Classification System (USCS) and American Association of State Highway and Transportation Officials (AASHTO) System using the laboratory testing results and the observations made during drilling.

4.1 Soil and Bedrock

<u>Soil Type 1A</u> classified as a silty to clayey sand fill (SM, SC, A-1-b). the sand fill was encountered in seven of the test borings, Test Boring Nos. 1, 2, 3, 36, 37, 38, and 39 at the existing ground surface and extending to depths ranging from 3 to 14 feet below the ground surface (bgs). The fill was located along Enclave Scenic Way and Fairway Glen Circle as noted in Figure 2. Reportedly the fill was observed and tested by others. Records of testing will be provided as required. Standard Penetration Testing resulted in SPT N-values ranging from 5 to 34 bpf, indicating loose to dense states. Water content and grain size testing resulted in water

contents ranging from 5 to 15 percent with approximately 15 to 32 percent of the soil size particles passing the No. 200 sieve. Atterberg Limits Testing resulted in the silty sand fill being non-plastic. FHA Swell Testing resulted in a swell pressure of 240 psf, indicating low expansion potential.

Stockpiles of fill are located in the vicinity of six of the test borings, Test Boring Nos. 11, 12, 13, 14, 43, and 50. These test borings were drilled adjacent to and on the stockpiled fill. These fill piles will require complete removals prior to any additional fill planned for this portion of the site. Based on the soil type encountered in the single boring drilled in this stockpiled fill, it is likely that this stockpiled fill can be used within this development. The fill removals should be observed by Entech to determine if additional soil sampling or laboratory testing is required.

Soil Type 1 classified as a slightly silty to silty sand and clayey sand (SM-SW, SM, SC, A-1-b, A-2-4, A-5, A-6). The sand was encountered in fifty-two of the test borings at depths ranging from the existing ground surface to 14 feet and extending to depths ranging from one foot to the termination of the borings (20 feet). Standard Penetration Testing conducted on the sand resulted in SPT N-values ranging from 8 to greater than 50 blows per foot (bpf), indicating the soil to be at loose to very dense states. Water content and grain size testing of soil samples resulted in a water content range of 2 to 23 percent with approximately 6 to 38 percent of the soil size particles passing the No. 200 sieve. Atterberg Limits testing resulted in liquid limits ranging from 29 to no value and plastic indexes of 15 to non-plastic. Sulfate testing on samples of Soil Type 1 resulted 0.00 to 0.01 percent soluble sulfate by weight, which indicates a negligible potential for below grade concrete degradation due to sulfate attack.

Soil Type 2 classified as sandy to very sandy clay (CL, A-4, A-6). The clay was encountered in four of the test borings at depths ranging from the ground surface to 14 feet below the ground surface (bgs) and extending to depths ranging from 3 to 16 feet bgs. Standard Penetration Testing on the clay resulted in SPT N-values of 10 to 32 bpf, indicating firm to very stiff consistencies. Water content and grain size testing resulted in approximately 11 to 20 percent water content with approximately 52 to 53 percent of the soil size particles passing the No. 200 sieve. Atterberg Limits testing resulted in liquid limits of 20 to 30 and plastic indexes of 10 to 14. A FHA Swell pressure of 1720 psf was measured on the clay, indicating moderate to high expansion potential. No swell (0.0 percent) was measured in the Swell/Consolidation Test

conducted on the clay. Sulfate testing resulted in less than 0.01 percent sulfate by weight indicating negligible potential for below grade concrete degradation due to sulfate attack.

Soil Type 3 classified as a slightly silty to very silty and clayey to very clayey sandstone (SM-SW, SM, SC, A-1-b, A-2-4, A-2-6, A-2-7, A-4, A-6). The sandstone was encountered in fifty-one of the test borings at depths ranging from 1 to 18 feet bgs and extending to depths ranging from 6 feet to 19 feet bgs and to the termination of the borings (20 to 30 feet). Standard Penetration Testing conducted on the sandstone resulted in SPT N-values from 37 to greater than 50 bpf, which indicates dense to very dense states. Water content and grain size testing resulted in approximately 4 to 21 percent water content with approximately 8 to 45 percent of the soil size particles passing the No. 200 sieve. Atterberg Limits testing resulted in liquid limits of 41 to no value and plastic indexes of 18 to non-plastic. FHA Swell testing on the sandstone resulted in a swell pressure of 30 psf, indicating low expansion potential. Swell/Consolidation testing on the very clayey sandstone resulted in volume change of 2.1 percent indicating moderate to high expansion potential. Sulfate testing on the sandstone resulted in 0.00 to less than 0.01 percent sulfate by weight indicating the sandstone exhibits negligible potential for below grade concrete degradation due to sulfate attack.

Soil Type 4 classified as a sandy to very sandy claystone (CL, A-4, A-5, A-6). The claystone was encountered in thirteen of the test borings at depths ranging from 1 to 24 feet bgs and extending to depths ranging from 8 to 28 feet bgs or to the termination of the borings (20 to 25 feet). Standard Penetration Testing conducted on the claystone resulted in SPT N-values of 40 to greater than 50 bpf, which indicates very stiff to hard consistencies. Water content and grain size testing resulted in approximately 10 to 17 percent water content with approximately 57 to 66 percent of the soil size particles passing the No. 200 sieve. Atterberg limits testing on the claystone resulted in liquid limits of 21 to 29 and plastic indexes of 8 to 13. FHA Swell Testing resulted in swell pressures of 1970 to 2060 psf, indicting the claystone exhibits high expansion potential. Swell/Consolidation testing on the claystone resulted in a volume change of 1.7 percent, indicating moderate expansion potential. Sulfate testing on the claystone resulted in 0.00 to less than 0.01 percent sulfate by weight indicating the claystone exhibits negligible degradation to concrete due to sulfate attack.

<u>Soil Type 5</u> classified as a clayey, sandy siltstone (ML, A-6). The siltstone was encountered in two of the test borings at depths of 1 to 11 feet and extending to depths of 6 feet bgs to the

termination of the boring (20 feet). Standard Penetration Testing on the siltstone resulted in SPR N-values greater than 50 bpf, indicating hard consistencies. Water content and grain size testing resulted in water content of 9 to 19 percent with approximately 67 to 85 percent of the soil size particles passing the No. 200 sieve. Atterberg Limits Testing resulted in a liquid limit of 40 and a plastic index of 14. Swell/Consolidation conducted on the siltstone resulted in a volume change of 0.9 percent, indicating low to moderate expansion potential.

The test boring logs are included in Appendix A. Laboratory Test Results are included in Appendix B and Summarized in Table 1. Bedrock depths are summarized in Table 2.

4.2 Groundwater

Depth to groundwater was measured in each of the borings at the conclusion of drilling and subsequent to drilling. Groundwater was encountered in thirty-nine of the fifty-five test borings at depths ranging from 2.5 to 20.5 feet. Groundwater may affect construction and utilities construction on this site. Much of these areas are to be filled. The groundwater depths and estimated cuts and fills are summarized in Table 2. Unstable conditions may be encountered where excavations approach the groundwater level. Stabilization using shot rock or geogrids may be necessary. Typically the sandstone will remain stable when water is encountered. It should be noted that groundwater levels could change due to seasonal variations, changes in land runoff characteristics and future development including nearby areas. Table 2 presents depth to groundwater, estimated cut/fill and depth to bedrock.

5.0 DEVELOPMENT CONSIDERATIONS

The following discussion is based on the subsurface conditions encountered in the borings drilled at the site. This investigation is for the site discussed in 2.0 Project and Site Description. If subsurface conditions different from those described herein are encountered during construction or if the project elements change from those described, Entech Engineering, Inc. should be notified so that the evaluation and recommendations presented can be reviewed and revised if necessary.

Subsurface soil conditions encountered in the test borings drilled on the site generally consisted of sand fill, silty to clayey sand, and sandy clay overlying sandstone, claystone and siltstone bedrock. Bedrock was encountered at depths ranging from 1 to 16 feet bgs and greater than 20 feet in one of the test borings. The fill located along Enclave Scenic Way and Fairway Glen Circle is considered controlled, however records of fill placement should be provided to Entech for review to determine the extent of the testing in this area. Fill piles were encountered in the central portion of the site in the vicinity of Test Boring Nos. 11, 12, 13, 14, 43, and 50. All fill piles need to be completely removed. Complete removals of these undocumented fill piles will be required prior to developing this area or placing additional fill. Recommendations for site grading, fill preparation, and compaction efforts are described in Section 6.0. Consideration should be given to several conditions on this site in planning and excavating the development besides the fill pile mitigation described above including groundwater, expansive soils and bedrock materials.

5.1 Groundwater

Groundwater should be expected to impact the development. Table 2 presents depth to bedrock and groundwater. Subsequent to completion of overlot grading cuts per the grading plan presented to us, the measured water levels will be at 6 feet or shallower at locations of Test Boring Nos. 1, 5, 6, 8, 16, 19, 27 and 39. Groundwater measured as shallow as nine feet in planned fill areas is not expected to influence construction of fills. Unstable conditions should be expected where groundwater is shallow or close to excavated depths. Procedures and equipment to mitigate groundwater impact during and after construction should be anticipated. Pumps, cofferdams, wide area and localized drain systems and other procedures and equipment may be necessary. Shot rock and geotextiles may be appropriate for stabilizing excavations. An underdrain system can be considered for long term groundwater mitigation. Frequently, groundwater levels rise following development as result of increased irrigation and decreased potential area of evaporation.

5.2 Expansive Soils

Expansive soils (clayey sand, clay, very clayey sandstone, claystone and clayey siltstone) are present on the site exhibiting expansion potential ranging from low to high. Mitigation of expansive soils will be required on portions of the lots.

5.3 Sandstone, Siltstone and Claystone

Sandstone, siltstone and claystone were encountered at depths as shallow as one to four feet. Bedrock was encountered at shallow depths across the majority of the site. Excavation of sandstone, siltstone and claystone should be expected to be moderate to difficult. Track type equipment likely will be needed to accomplish excavations particularly where harder materials or lenses are present. Upon completion of site grading per the plan provided to us, sandstone is expected to be exposed across the majority of the areas tested.

6.0 SITE GRADING

Shallow bedrock (less than 6 feet) was encountered in thirty-nine of the fifty-five test borings drilled on the site. Excavation of most bedrock materials on site is expected to be moderate to difficult with heavy duty earthmoving equipment. Claystone, siltstone and sandstone materials may require track equipment and ripping teeth. For conditions with no groundwater seepage, cut and fill slopes no steeper than 3 to 1 (horizontal to vertical) should be considered. If seepage occurs, then flatter slopes or a drain system should be considered. Recommendations may be subject to change depending upon particular field conditions.

6.1 Stripping

Debris, topsoil and organic materials should be stripped from the ground surface of areas to be filled. The fill in Enclave Scenic Way and Fairway Glen Circle are believed to be controlled and tested by others and appeared to be prepared to the approved Stonebridge Filing 1 grading plans. Any uncontrolled fill materials should be completely removed. Fill was encountered in central portion of the site that is considered uncontrolled. The materials may be used as fill pending approval if they are free of organic material and debris. Any soft or loose soils should be stabilized or removed to expose suitable material prior to placement of fill. Topsoil may be stored in stock piles and placed at the surface in landscape areas.

6.2 Fill Preparation

Surfaces which will receive fill should be scarified to depths of 6 inches, moisture conditioned to within 2 percent of optimum moisture, and compacted to minimum of 95 percent of Standard Proctor Dry Density (ASTM D 698). On-site natural soils and bedrock are anticipated to be used

as site grading fill. Bedrock must be processed and broken down where placed in the fill. Expansive materials used for fill should be placed at sufficient moisture content to mitigate potential swell. The fill quality will influence the performance of foundations, slabs-on-grade, and pavements. Fill settlement can be minimized by placing thin lifts at suitable moisture content and by verification of compaction with frequent density tests.

6.3 Compaction

Overlot grading fill consisting of granular soils should be placed in lifts to exceed 6 inches following compaction and compacted to at least 95 percent of the maximum dry density determined by Modified Proctor (ASTM D-1557). Clay materials should be placed in compacted lifts less than 6 inches thick compacted to at least 95 percent of maximum Standard Proctor (ASTM D 698) Dry Density. Fills below 10 feet in depth should be moisture conditioned as above and compacted to 98 percent of Standard Proctor Dry Density (ASTM D 698) for cohesive materials or 98 percent of maximum Modified Proctor Dry Density (ASTM D 1557) for granular materials. The soil materials should be placed at a moisture content conducive to adequate compaction, usually within ±2 percent of optimum moisture content. Fill placement and compaction should be observed and tested by Entech during construction to verify that adequate moisture and density has been achieved.

7.0 UNDERGROUND UTILITY CONSTRUCTION

Generally excavation is expected to be moderate to difficult utilizing heavy-duty trackhoes. Rock buckets and rock teeth will likely be required where excavations extend into very hard sandstone or cemented materials. Special procedures or equipment may be required to remove water and/or achieve stability in utility trenches where excavations approach or intercept groundwater.

Utilities including water and sewer lines are usually constructed beneath paved roads. Placement of fill and degree of compaction applied to trench backfill will influence performance of overlying structures including pavements. Fill placed into utility trenches should be compacted according to requirements of the local jurisdiction. Fill should be placed in horizontal lifts having compacted thickness of six inches or less and at a water content conducive adequate compaction, usually within ±2 percent of optimum water content. Typical compaction

specifications would be similar to specifications in the Site Grading section. Mechanical methods should be used for fill placement, however, heavy equipment should be kept at a distance away from structures to avoid damage. No water flooding techniques of any type should be used for compaction or placement of utility trench backfill.

Trench backfill should be performed in accordance with El Paso County specifications and requirements. Excavations and excavation shoring/bracing should be performed in accordance with OSHA guidelines.

8.0 UNDERDRAIN SYSTEM

Depending on final site grading anticipated depths of excavations and structure foundations relative to groundwater occurrence, an underdrain system may be considered to be included as part of sewer system design and installation. The underdrain system drain pipe shall consist of smooth wall non perforated rigid PVC pipe placed at a minimum slope of 0.5 percent. Concrete or clay material fill may be strategically placed at the manhole locations to slow the water flow down the trench. The underdrain below sewer should be constructed with adequate depth to allow connection of residence foundation drain systems. Drain elements should be of appropriate slopes and sizes for anticipated flows. Maintenance of the underdrain system should be anticipated. Gravity outlet should be planned such that other developments and properties are not adversely affected.

9.0 PAVEMENT CONSIDERATIONS

Materials exposed at pavement subgrade elevations will be dependent upon native materials exposed at final overlot grading and the specific materials placed as fill at and near finish grade elevations after utility installation. The predominate materials are generally expected to be silty sand, sandstone, clayey sand, clay, claystone and siltstone. Materials anticipated at subgrade elevation generally would be rated as typically having good pavement support characteristics with some areas likely rated as having poor pavement support characteristics. AASHTO classifications of A-1-b, A-2-4, A-2-6, A-2-7, and A-4 were determined for the majority of the sandstone and upper granular soils. Based on depth to claystone and estimated cut, the

claystone may be exposed within the pavement subgrade. The claystone classifies as A-4, A-5, and A-6 using the AASHTO classification system which typically provide fair to poor pavement support and may be encountered on the site depending on the final grading plans. Thickness of asphalt pavement and aggregate base course sections may be 4 inches of asphalt overlying 6 to 10 inches of base course depending on specific subgrade materials and Roadway Classification of each particular street. Chemically treated soil subgrades may be used on this site with anticipated sections of 4 inches of asphalt overlying 10 inches of treated subgrade. For specific thickness determinations, a subsurface soil investigation and pavement design should be completed after completion of overlot grading and utility installation.

10.0 ANTICIPATED RESIDENTIAL FOUNDATION SYSTEMS

Subsurface soil conditions consisted of granular materials with some areas of expansive clay claystone or clayey siltstone materials. We anticipate conventional spread footing foundation systems will be appropriate for residences constructed on the majority of the site. Where expansive materials are encountered at or near foundation grades, use of spread footings with overexcavation and replacement with non-expansive fill should be expected. Drilled pier foundations may be a suitable alternative where expansive soils are encountered. A Subsurface Soils Investigation report should be prepared after completion of overlot grading to address appropriate foundation systems for each lot. Perimeter below grade foundation drain systems should be anticipated. Overexcavation drains may also be required. Figures 3 and 4 present typical drain overexcavation and building perimeter details. Shallow groundwater was encountered at Test Boring Nos. 1, 5, 6, 8, 16, 19, 27 and 39. Temporary and permanent dewatering systems may be necessary at various foundation excavations. Shotrock and geotextiles may be appropriate for stabilizing excavations. An area wide subdrain may be considered for discharge of collected water.

11.0 RESIDENCE ON-GRADE FLOOR SLABS

On-grade floor slabs for the planned structures could be supported by on-site non-expansive soils or compacted, non-expansive, structural fill. Loose or expansive soils encountered at or near floor slab grade should be penetrated or overexcavated a distance below slab subgrade

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and replaced with a non-expansive structural fill to improve floor slab performance. If slab movement and cracks cannot be tolerated a structural floor system should be used. Evaluation of subgrade materials should be included within a Subsurface Soils Investigation for each specific lot.

12.0 CONCRETE DEGRADATION DUE TO SULFATE ATTACK

Sulfate solubility testing was conducted on twenty-two samples recovered from the test borings to evaluate the potential for sulfate attack on concrete placed below surface grade. The test results indicated 0.00 to 0.01 percent soluble sulfate (by weight). The test results indicate the sulfate component of the in-place soils presents a negligible exposure threat to concrete placed below the site grade. Type II cement is recommended for the on-site soils. Additional testing should be conducted following completion of overlot grading.

13.0 EXCAVATION STABILITY

Excavation walls must be properly sloped/benched or otherwise supported in order to maintain stable conditions. All excavation openings and work execution shall conform to OSHA standards as in CFR 29, Part 1926.650-652 (Subpart D).

14.0 SURFACE AND SUBSURFACE DRAINAGE

Surface drainage will influence performance of structures at the site including streets and residences. Drainage is recommended around each building perimeter at a minimum slope of 5 percent in the first 10 feet adjacent to exterior foundation walls and for unpaved areas, where possible. For paved areas and other impervious surfaces, a minimum slope of 2 percent is recommended. Drainage should be planned to avoid ponding of water. Collected water and irrigation should discharge well beyond foundation backfill zones. Surface runoff should be designed to avoid sheet flow and erosion. Slopes should be protected from erosion by materials such as mulch or appropriate plants or other methods. All fills and backfills should be properly compacted. Unprotected surfaces may be subject to undesirable, heavy erosion.

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15.0 WINTER CONSTRUCTION

In the event construction occurs during winter, concrete and soil materials should be protected from freezing conditions. Concrete should not be placed on frozen soil and once concrete has been placed, it should not be allowed to freeze. Similarly, once exposed, the soil subgrades should not be allowed to freeze. During grading operations and subgrade preparation, care should be taken to avoid burial of snow, ice or frozen material within the planned construction area.

16.0 CONSTRUCTION OBSERVATIONS

It is recommended that Entech observe and document the following activities during construction of the building foundations.

- Excavated subgrades and subgrade preparation.
- Placement/compaction of fill materials.
- Placement/compaction of utility bedding and trench backfill.

17.0 CLOSURE

The subsurface investigation, geotechnical evaluation and recommendations presented in this report are intended for use by Tech Contractors with application to the planned development of the single-family residential project site located at Windingwalk Filings 1 and 2 and Stonebridge The Enclave Filings 4 and 5, in El Paso County, Colorado. In conducting the preliminary subsurface investigation, laboratory testing, engineering evaluation and reporting, Entech Engineering, Inc. endeavored to work in accordance with generally accepted professional geotechnical and geologic practices and principles consistent with the level of care and skill ordinarily exercised by members of the geotechnical profession currently practicing in same locality and under similar conditions. No other warranty, expressed or implied is made. During final design and/or construction, if conditions are encountered which appear different from those described in this report, Entech Engineering, Inc. requests that it be notified so that the evaluation and recommendations presented herein can be reviewed and modified as appropriate.

If there are any questions regarding the information provided herein or if Entech Engineering, Inc. can be of further assistance, please do not hesitate to contact us.

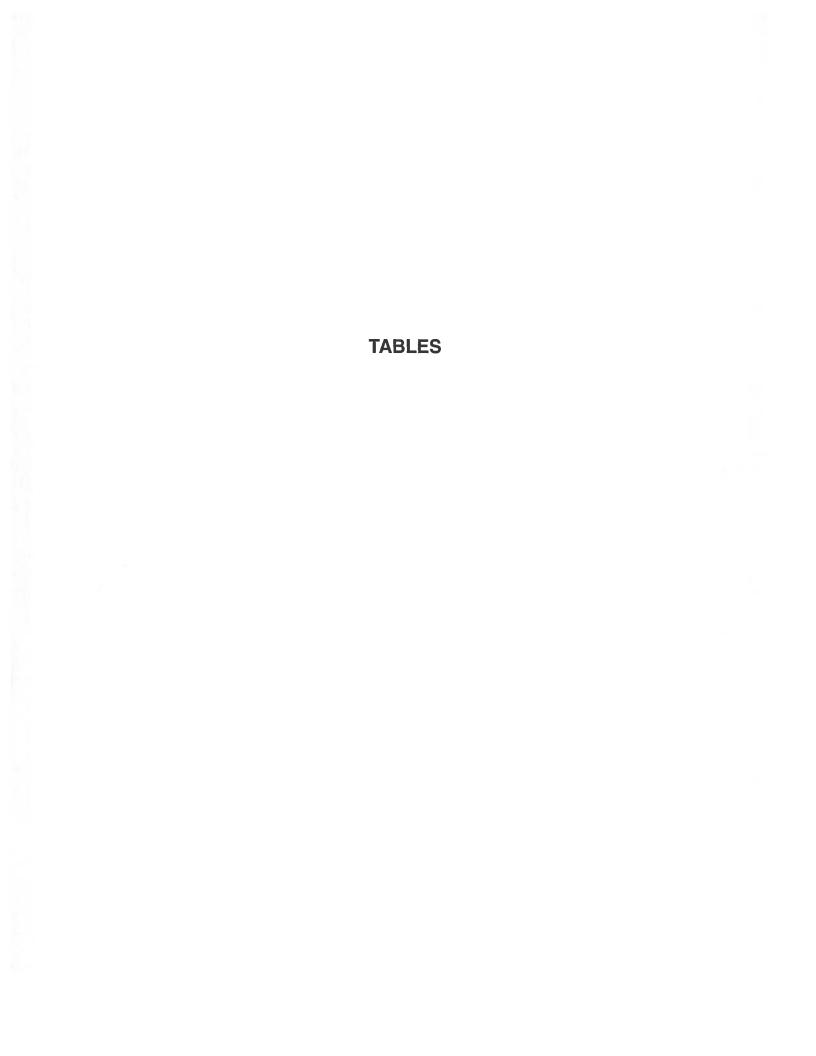


TABLE 1

SUMMARY OF LABORATORY TEST RESULTS

TECH CONTRACTORS
WINDINGWALK & STONEBRIDGE
171198 CLIENT PROJECT JOB NO.

															Ι	_																				
SOIL DESCRIPTION	FILL, SAND, SILTY	FILL, SAND, CLAYEY	SAND, SLIGHTLY SILTY	SAND, CLAYEY	SAND, SLIGHTLY SILTY	SAND, CLAYEY	SAND, SILTY	SAND, SLIGHTLY SILTY	SAND, SLIGHTLY SILTY	CLAY, VERY SANDY	CLAY, SANDY	CLAY, VERY SANDY	SANDSTONE, SILTY	SANDSTONE, CLAYEY	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, VERY CLAYEY	SANDSTONE, SILTY	SANDSTONE, VERY CLAYEY	SANDSTONE, SILTY	SANDSTONE, VERY CLAYEY	SANDSTONE, SILTY	SANDSTONE, SLIGHTLY SILTY	SANDSTONE, VERY SILTY	SANDSTONE, SILTY	SANDSTONE, SILTY								
UNIFIED CLASS.	SM	SC	WS-MS	SC	MS-MS	SC	WS	MS-MS	SM	WS	NS	SM	SM-SW	占	ರ	ر ا	SM	SC	SM	SM	SM	SM	SC	SM	SC	SM	SM	SM	SM	SM	sc	SM	SM-SW	SM	SM	SM
SWELL/ CONSOL (%)																0.0							2.1								2.1					
AASHTO CLASS.	A-1-b			A-5	A-1-b	A-6		A-1-b			A-2-4		A-1-b	A-4		A-6	P-4	A-2-6	A-2-7		A-2-4		A-6									A-1-b	A-1-b	A-4		
FHA SWELL (PSF)		240													1720																					
SULFATE (WT %)					<0.01	<0.01		0.00			<0.01	0.01	<0.01	<0.01		<0.01	<0.01				<0.01		0.00	0.00								<0.01	<0.01	<0.01		
PLASTIC INDEX (%)	ΔN			6	N P	15		NP			AN		AN	10		14	NP	12	14		٩N		17									ΔN	NP	NP		
LIQUID LIMIT (%)	≥			29	2	28		N			N		ž	20		30	N/	36	41	-	ž		32									N	N	N		
PASSING NO. 200 SIEVE (%)	15.3	31.8	11.7	35.5	6.1	38.4	12.6	11.0	26.9	13.7	28.2	17.1	8.7	51.6		53.2	38.7	29.2	28.7	16.9	27.1	25.2	47.2	15.1	38.2	24.4	28.7	24.3	13.2	18.5	43.5	15.6	11.8	40.4	19.3	23.4
DRY DENSITY (PCF)																101.2							111.8								108.3					
WATER (%)																22.2							14.3								20.1					
DEРТН (FT)	2-3	2	10	2-3	2-3	2-3	2	22	2-3	9	2-3	10	2-3	10	15	2-3	10	50	5	20	10	5	20	유	15	2-3	10	10	5	15	15	5	10	25	10	15
TEST BORING NO.	36	37	2	4	80	13	15	19	30	34	35	39	55	25	3	28	1	3	5	7	6	10	11	12	14	16	18	20	21	22	23	29	31	32	33	38
SOIL	1A	1A	-	-	-	-	-	-	-	-	+	-	-	2	2	2	3	3	9	3	3	3	3	က	3	3	3	3	3	3	ဗ	က	3	ဗ	3	3

								_												
SOIL DESCRIPTION	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, VERY SILTY	SANDSTONE, SLIGHTLY SILTY	SANDSTONE, VERY CLAYEY	SANDSTONE, SILTY	SANDSTONE, SLIGHTLY SILTY	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, SILTY	SANDSTONE, SLIGHTLY SILTY	SANDSTONE, SILTY	CLAYSTONE, SANDY	CLAYSTONE, SANDY	CLAYSTONE, SANDY	CLAYSTONE, VERY SANDY	CLAYSTONE, VERY SANDY	CLAYSTONE, SANDY	SILTSTONE, SANDY, CLAYEY	SILTSTONE, SANDY, CLAYEY
UNIFIED CLASS.	SM	SM	SM	SM-SW	SC	SM	SM-SW	SM	SM	SM	SM-SW	SM	CL	CL	CL	CL	겁	CF	ML	ML
SWELL/ CONSOL (%)														1.7						6.0
AASHTO CLASS.	A-1-b			A-1-b	A-6					A-1-b			A-5	A-4				A-6	A-6	
FHA SWELL (PSF)						30									2060	1970				
SULFATE (WT %)			<0.01	0.00	<0.01					00.0			<0.01	<0.01				00.00		
PLASTIC INDEX (%)	ΝP			NP	18					٩N			8	8				13	14	
LIQUID LIMIT (%)	N			N	33					NV			21	29				27	40	
PASSING NO. 200 SIEVE (%)	12.0	22.2	45.3	8.4	41.5	15.0	11.3	15.1	27.7	18.3	9.7	20.2	66.3	64.7	65.2	56.5	60.3	63.3	85.4	66.5
DRY DENSITY (PCF)														117.2						111.1
WATER (%)														10.9						15.6
DEPTH (FT)	2-3	5	20	5	2-3	5	10	15	2-3	2	10	15	10	20	10	10	5	15	15	2-3
TEST BORING NO.	40	41	42	45	46	47	48	49	20	51	52	53	9	24	56	43	44	54	2	17
SOIL	3	3	3	3	3	3	3	3	3	ဗ	3	3	4	4	4	4	4	4	5	2

TABLE 2: Summary of Estimated Cut/Fill, Depth to Bedrock, and Groundwater Depths

Client: Tech Contractors

<u>Project:</u> Windingwalk Filings 1 and 2, and Stonebridge The Enclave Filings 4 and 5

<u>Job No: 171198</u>

	Estimated Cut/Fill	Depths to Bedrock	Depth to Groundwate
Test Boring No.	(ft.)	(ft.) ¹	(ft.)¹
1	+0-2	8	2.5
2	+0-2	11	10.5
3	+ 0 - 2	16	12
4	+ 8 - 10	9*	4
5	+0-2	4	6
6	-0-2	7	4
7	-0-2	1	8.5
8	+ 0 - 2	11	6
9	+2-4	2	11.5
10	+0-2	2	9
11	-2-4	1	15
12	-0-2	1	13
13	- 6 - 8	6	>25
14	- 10 - 12	2	20.5
15	+0-2	6	>20
16	- 2 - 4	1	7.5
17	+0-2	1 -	9
18	- 0 - 2	5	16
19	- 0 - 2	8	6.5
20	+0-2	9	>20
21	+0-2	2	12.5
22	+2-4	2	14
23	+0-2	1	7.5
24	-2-4	1	11
25	- 4 - 6	13	9
26	+0-2	4	7.5
27	+2-4	7	4
28	+0-2	3	11

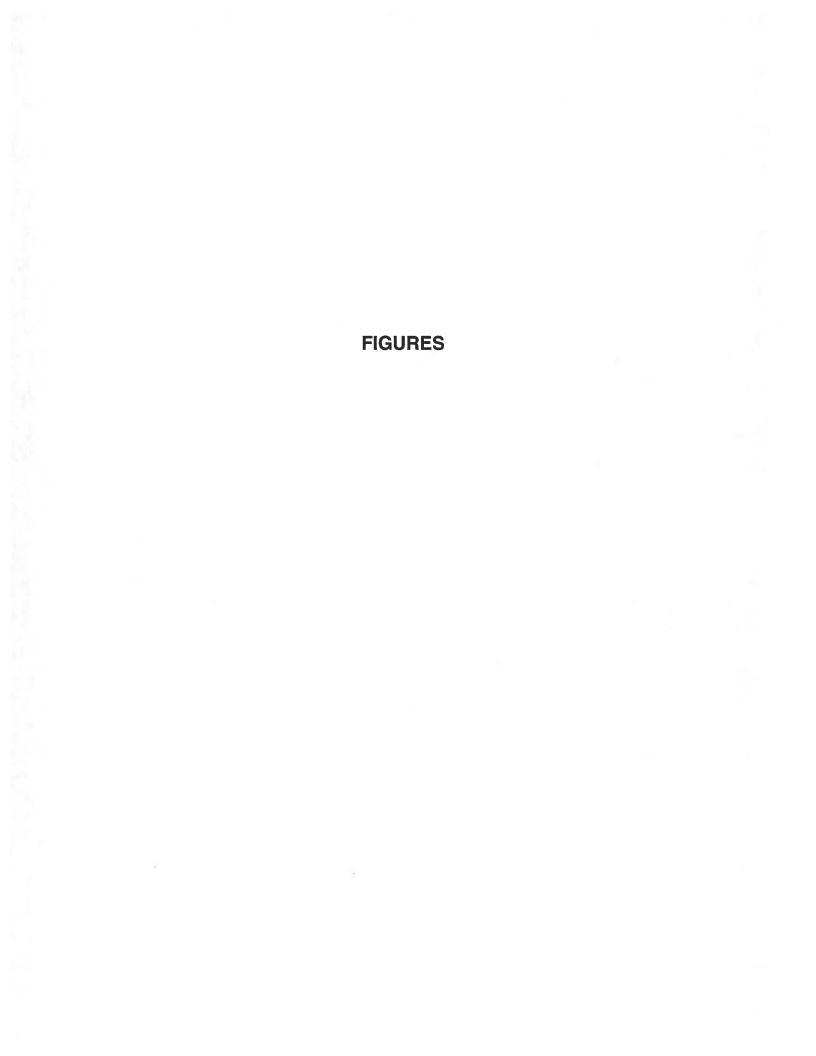
TABLE 2: Summary of Estimated Cut/Fill, Depth to Bedrock, and Groundwater Depths continued

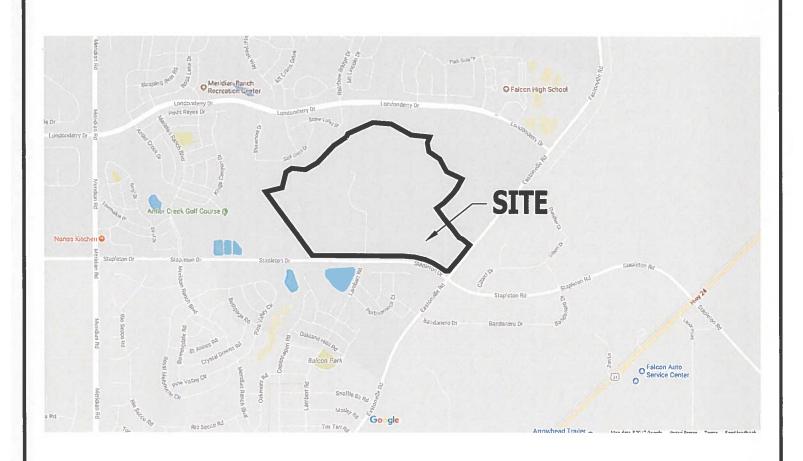
Client: Tech Contractors

Project: Winding Walk

<u>Job No: 171198</u>

	Estimated Cut/Fill	Depths to Bedrock	Depth to Groundwater
Test Boring No.	(ft.)	(ft.) ¹	(ft.) ¹
29	+ 0 - 2	1	>20
30	+ 10 - 12	3	19
31	-2-4	1	12.5
32	-6-8	7	15.5
33	-2-4	4	>20
34	+0-2	6	14.5
35	-2-4	6	15
36	- 0 - 2	>20	17.5
37	- 0 - 2	14	>20
38	- 0 - 2	6	>20
39	- 0 - 2	12	6
40	+ 2 - 4	1	15
41	+ 0 - 2	1	15.5
42	+ 2 - 4	4	18
43	- 4 - 6	2*	>20
44	+ 2 - 4	1	20
45	+ 2 - 4	1	18
46	- 4 - 6	1	>20
47	-2-4	4	>20
48	+ 0 - 2	1	18
49	- 2 - 4	1	>20
50	+ 2 - 4	1	>20
51	- 2 - 4	1	>20
52	+ 0 - 2	1	>20
53	- 0 - 2	1	>20
54	- 0 - 2	14	18.5
55	+ 0 - 2	9	>20
*Weathered Bedrock De	pth		



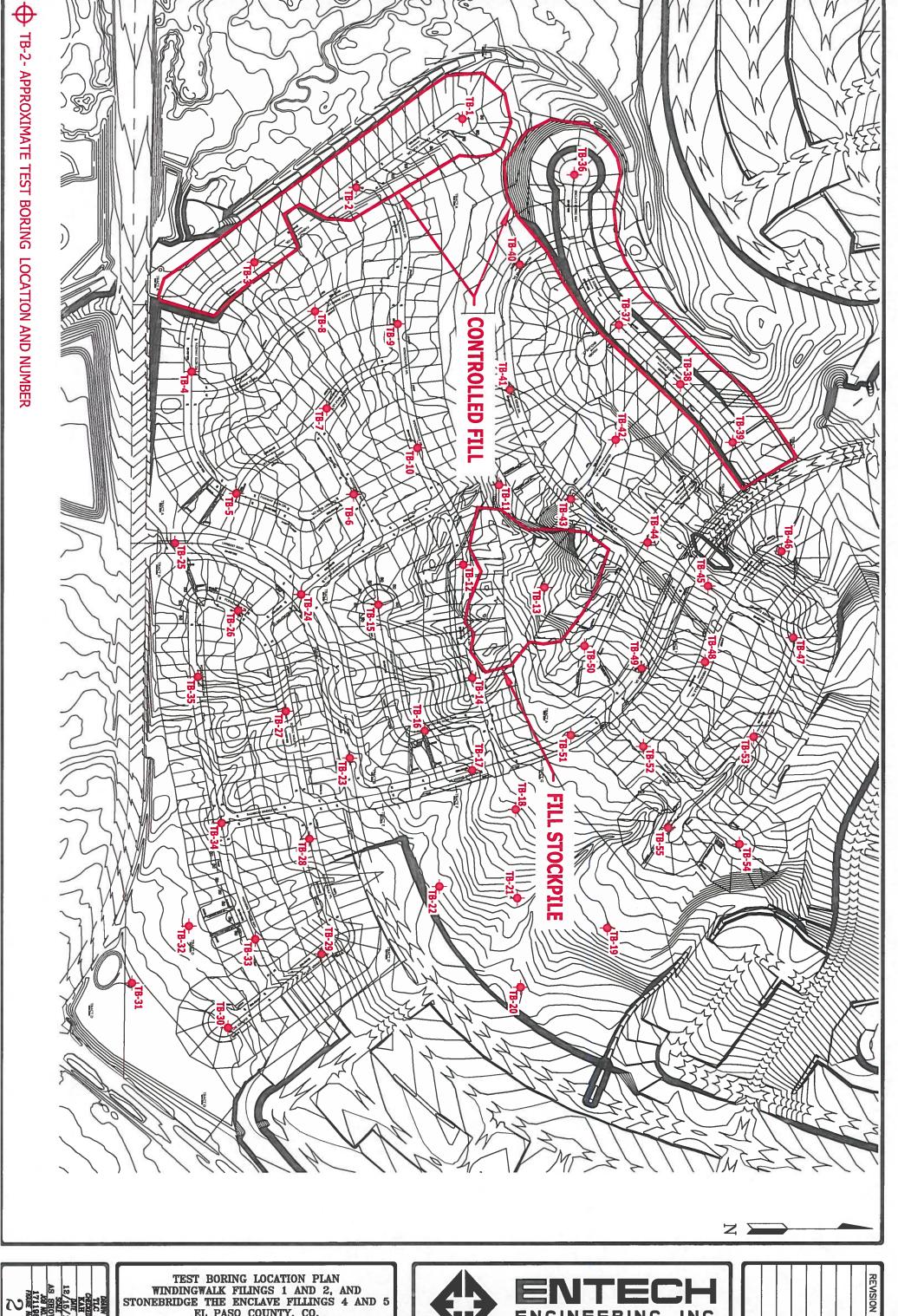




VICINITY LOCATION MAP
WINDINGWALK FILINGS 1 AND 2, AND
STONEBRIDGE THE ENCLAVE FILINGS 4 AND 5
EL PASO COUNTY, CO
FOR: TECH CONTRACTORS

DRAWN BY: TLC DATE DRAWN: 11/08/17 DESIGNED BY: KAH CHECKED: KAH JOB NO.: 171198 FIG. NO.:

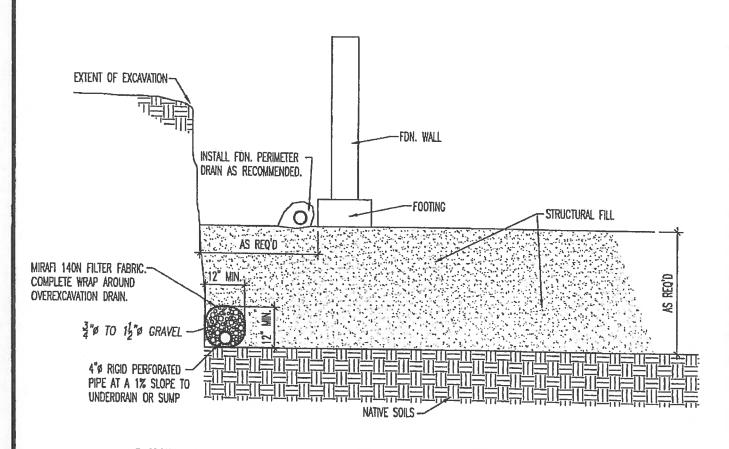
1



EL PASO COUNTY, CO. FOR: TECH CONTRACTORS







OVEREXCAVATION DRAIN DETAIL

N.T.S.

DRAWN:

NOTE:

EXTEND DRAIN TO SUMP AS REQ'D.



OVEREXCAVATION DRAIN DETAIL

DESIGNED BY:

D. STECMAN

W26/17

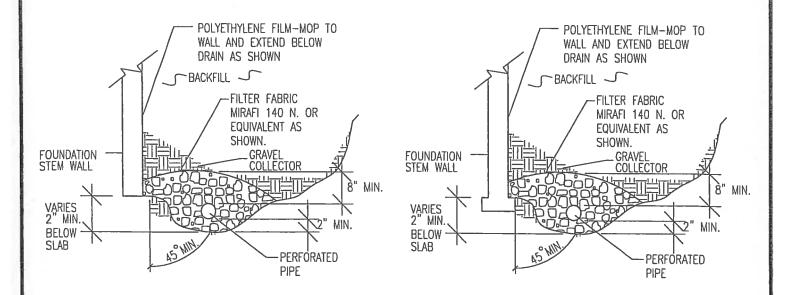
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NOTES:

- -GRAVEL SIZE IS RELATED TO DIAMETER OF PIPE PERFORATIONS-85% GRAVEL GREATER THAN 2x PERFORATION DIAMETER.
- -PIPE DIAMETER DEPENDS UPON EXPECTED SEEPAGE. 4-INCH DIAMETER IS MOST OFTEN USED.
- -ALL PIPE SHALL BE PERFORATED PLASTIC. THE DISCHARGE PORTION OF THE PIPE SHOULD BE NON-PERFORATED PIPE.
- -FLEXIBLE PIPE MAY BE USED UP TO 8 FEET IN DEPTH, IF SUCH PIPE IS DESIGNED TO WITHSTAND THE PRESSURES. RIGID PLASTIC PIPE WOULD OTHERWISE BE REQUIRED.
- -MINIMUM GRADE FOR DRAIN PIPE TO BE 1% OR 3 INCHES OF FALL IN 25 FEET.
- -DRAIN TO BE PROVIDED WITH A FREE GRAVITY OUTFALL, IF POSSIBLE. A SUMP AND PUMP MAY BE USED IF GRAVITY OUT FALL IS NOT AVAILABLE.

DRAWN:



•	PERIMETER L	RAIN DETAIL	,
	DATE: 9/26/17	designed: DS	CHECKED:

APPENDIX A: Test Boring Logs

TEST BORING NO. TEST BORING NO. 2 DATE DRILLED 8/16/2017 DATE DRILLED 8/21/2017 **CLIENT TECH CONTRACTORS** Job# 171198 LOCATION WINDINGWALK & STONEBRIDGE REMARKS **REMARKS** Blows per foot per foot Watercontent Watercontent Soil Type Depth (ft) Soil Type Samples Samples Symbol Symbol Blows WATER @ 2.5', 8/18/17 WATER @ 10.5', 9/2/17 FILL O-4', SAND, SILTY, FINE FILL 0-3', SAND, SILTY, FINE TO COARSE GRAINED, TAN, TO COARSE GRAINED, BROWN, 24 10.9 22 1A MEDIUM DENSE, MOIST 8.3 1A MEDIUM DENSE, MOIST SAND, CLAYEY, FINE TO COARSE GRAINED, GREEN 5 18 16.4 1 SAND, SLIGHTLY SILTY, FINE 11 8.4 1 BROWN, MEDIUM DENSE. TO COARSE GRAINED, TAN, MOIST MEDIUM DENSE, MOIST SANDSTONE, SILTY, FINE TO 10 3 10 COARSE GRAINED WITH FINE <u>50</u> 14.1 19 7.1 1 9" GRAINED LENSES, GREEN SILTSTONE, CLAYEY, SANDY, BROWN, VERY DENSE, MOIST GREEN BROWN, HARD, MOIST 15 15.7 3 15 18.8 5 <u>50</u> <u>50</u> 10" 10" <u>50</u> 13.7 <u>50</u> 14.1



	TE	ST BORING LOC	à
DRAWN:	DATE:	CHECKED: K	DATE: U/9/17

TEST BORING NO. TEST BORING NO. 3 4 DATE DRILLED DATE DRILLED 8/21/2017 8/21/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Soil Type Samples Samples Symbol Symbol Soil WATER @ 12', 92/17 WATER @ 4', 9/2/17 FILL O-5', SAND, CLAYEY, FINE SAND, CLAYEY, FINE TO TO COARSE GRAINED, TAN, MEDIUM GRAINED, GRAY MEDIUM DENSE, MOIST 10 11.6 1A BROWN, MEDIUM DENSE, MOIST 12 23.0 1 23 5 9.8 1A 23 16.7 1 SAND, SILTY TO SLIGHTLY SILTY, FINE TO COARSE GRAINED, DARK BROWN TO TAN, LOOSE, MOIST 10 3.5 10 42 WEATHERED TO FORMATIONAL 9.5 3 SANDSTONE, SILTY, FINE TO COARSE GRAINED, GRAY BROWN, DENSE TO VERY DENSE, MOIST 15 2 30 CLAY, SANDY, DARK BROWN, 18.4 15 9.3 3 <u>50</u> VERY STIFF, MOIST SANDSTONE, CLAYEY, FINE GRAINED, GREEN BROWN, VERY DENSE, MOIST <u>50</u> 21.3 <u>50</u> 13.7



	TE	ST BORING LO	G	
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JOB NO.: 171198

FIG NO.: A- 2

TEST BORING NO. TEST BORING NO. 5 6 DATE DRILLED DATE DRILLED 8/21/2017 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Soil Type Soil Type Samples Symbol Symbol WATER @ 6', 92/17 WATER @ 4', 92/17 SAND, SILTY, FINE TO COARSE SAND, SILTY, FINE TO COARSE GRAINED, GREEN BROWN, GRAINED, GREEN BROWN, 7.7 MEDIUM DENSE, MOIST 12 MEDIUM DENSE, MOIST 1 12 5.3 1 5 SANDSTONE, SILTY, FINE TO <u>50</u> 12.7 3 22 7.8 1 COARSE GRAINED, GRAY 11" BROWN, VERY DENSE, MOIST CLAYSTONE, SANDY TO CLAYSTONE, SANDY, BROWN VERY SANDY, GRAY BROWN, TO BLUE GRAY, HARD, MOIST HARD, MOIST 10 <u>50</u> 16.9 4 10 <u>50</u> 14.9 11" 5" 15 50 17.4 4 15 <u>50</u> 12.9 8" 5" <u>50</u> 13.9 50 15.8



	TEST	BORING LOG	à
DRAWN:	DATE:	CHECKED:	11/6/17

JOB NO.: 171198 FIG NO.:

FIG NO.: A- 3

TEST BORING NO. TEST BORING NO. 7 8 DATE DRILLED 8/21/2017 DATE DRILLED 8/25/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS **REMARKS** Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Soil Type Samples Samples Symbol Symbol Soil WATER @ 8.5', 92/17 WATER @ 6', 92/17 SAND, SILTY, TAN SAND, SLIGHTLY SILTY TO SANDSTONE, SILTY, FINE SILTY, FINE TO COARSE TO COARSE GRAINED, GREEN 50 4.2 3 2.1 1 GRAINED, TAN, MEDIUM 20 BROWN TO TAN, VERY DENSE, DENSE TO DENSE, MOIST MOIST TO WET <u>50</u> 8.7 3 25 10.1 1 5 7" 10 3 <u>50</u> 9.1 10 40 11.8 1 6" CLAYSTONE, SANDY, GRAY BROWN, HARD, MOIST 15 <u>50</u> 8.9 3 15 12.8 50 10" <u>50</u> 20.6 3 WEATHERED ZONE 40 16.4

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JOB NO.: 171198

FIG NO.:

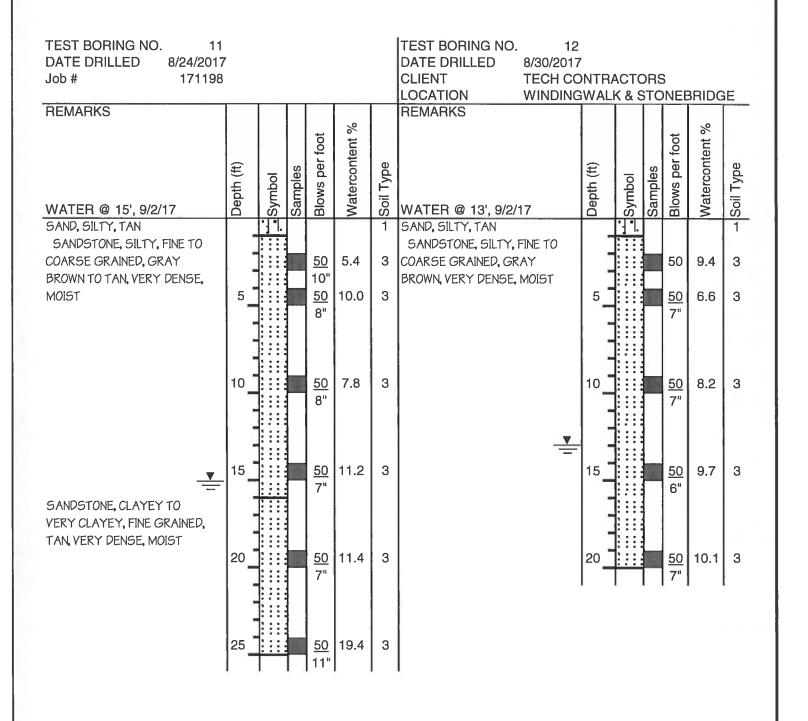
TEST BORING NO. TEST BORING NO. 10 DATE DRILLED 8/25/2017 DATE DRILLED 8/24/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot **Natercontent** Watercontent Depth (ft) Depth (ft) Samples Samples Symbol Symbol Soil WATER @ 11.5', 9/2/17 WATER @ 9', 9/2/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, SILTY, FINE TO SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, VERY COARSE GRAINED WITH FINE DENSE, MOIST <u>50</u> 7.1 GRAINED LENSES, TAN TO 5 <u>50</u> 7.7 6" GRAY BROWN, VERY DENSE. 10" MOIST 10 3 <u>50</u> 8.8 10 3 50 13.2 10" 8" 15 3 <u>50</u> 9.4 15 <u>50</u> 11.0 6" CLAYSTONE, SANDY, GRAY 20 <u>50</u> 15.9 <u>50</u> 9.1 3 8" BROWN, HARD, MOIST



	TES	T BORING LO	G
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JOB NO.: 171198 FIG NO.:

A- 5





	IESI	BORING LO	
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17

TEST BORING NO. TEST BORING NO. 13 14 DATE DRILLED DATE DRILLED 8/29/2017 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Samples Samples Symbol Symbol Soil DRY TO 25', 8/29/17 WATER @ 20.5', 9/2/17 FILL 0-6', SAND, CLAYEY TO SAND, SILTY, TAN VERY CLAYEY, FINE TO COARSE GRAINED, GREEN 5 11.6 1A SANDSTONE, CLAYEY TO BROWN, LOOSE TO MEDIUM VERY CLAYEY WITH SILTY 1A LENSES, FINE TO COARSE DENSE, MOIST 5 18 15.1 5 <u>50</u> 7.2 GRAINED, TAN, VERY DENSE, SANDSTONE, SILTY, FINE TO MOIST COARSE GRAINED, GREEN BROWN, VERY DENSE, MOIST 10 7.0 3 <u>50</u> 10 <u>50</u> 10.3 15 3 50 7.3 8.5 3 15 <u>50</u> 6" SANDSTONE, SILTY, FINE 20 GRAINED, TAN, VERY DENSE, 50 9.6 3 20 3 50 10.9 6" MOIST <u>50</u> 9.4 3 CLAYSTONE, SANDY, BLUE 25 50 13.3 6" GRAY, HARD, MOIST SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, VERY <u>50</u> 12.8 3 DENSE, MOIST

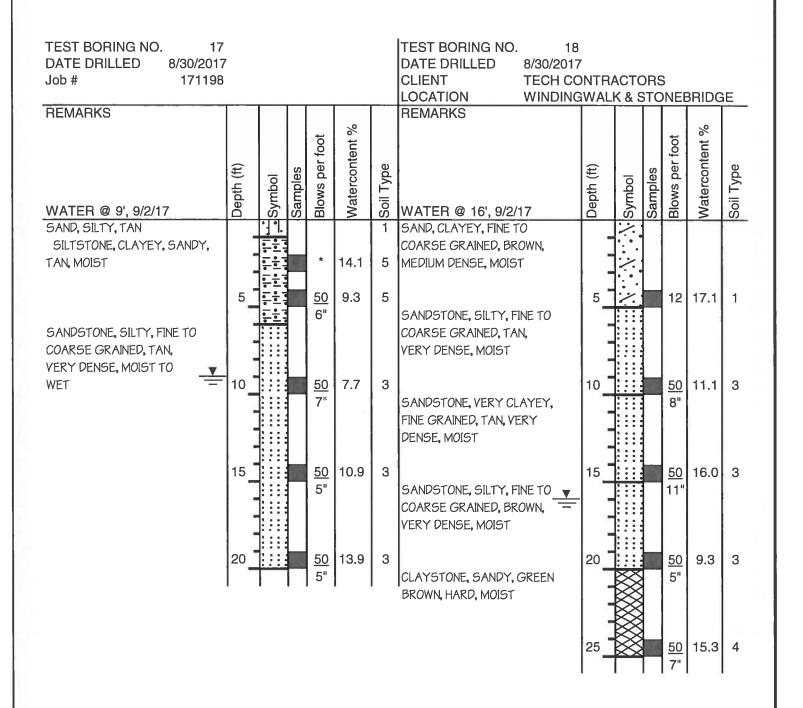


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TEST BORING NO. TEST BORING NO. 15 16 DATE DRILLED DATE DRILLED 8/21/2017 8/25/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Soil Type Samples Symbol Symbol DRY TO 20', 8/21/17 WATER @ 7.5', 9/2/17 SAND, SILTY, FINE TO COARSE SAND, SILTY, TAN GRAINED, TAN, MEDIUM SANDSTONE, SILTY, FINE DENSE, MOIST 27 5.9 50 TO COARSE GRAINED, TAN, 6.0 3 VERY DENSE, MOIST 5 24 8.6 1 5.4 3 50 SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, VERY DENSE, MOIST 10 7.8 3 10 <u>50</u> <u>50</u> 9.4 3 6" 5" 15 3 <u>50</u> 12.0 15 10.7 3 <u>50</u> | <u>50</u> 10.0 3 CLAYSTONE, VERY SANDY, 50 15.2 BLUE GRAY, HARD, MOIST



	TEST BORING LOG				
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17		





	TE	EST BORING LO	G
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17

TEST BORING NO. TEST BORING NO. 19 20 DATE DRILLED 8/30/2017 DATE DRILLED 8/30/2017 Job# CLIENT **TECH CONTRACTORS** 171198 LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Soil Type Samples Samples Symbol Symbol WATER @ 6.5', 9/2/17 DRY TO 20', 9/2/17 SAND, SLIGHTLY SILTY, FINE SAND, SILTY, FINE TO COARSE TO COASE GRAINED, TAN, GRAINED, TAN, MEDIUM DENSE TO DENSE, MOIST MEDIUM DENSE, MOIST 27 3.1 1 22 8.2 1 30 3.9 1 SANDSTONE, SILTY, FINE TO 10 COARSE GRAINED, TAN, VERY <u>50</u> 7.7 SANDSTONE, SILTY, FINE TO 10 <u>50</u> 8.6 3 DENSE, MOIST COARSE GRAINED, TAN, VERY DENSE, MOIST 15 7.3 <u>50</u> 5.7 3 3 15 <u>50</u> 3" SANDSTONE, SILTY, FINE <u>50</u> 9.4 <u>50</u> 6.3 3 GRAINED, TAN, VERY DENSE,



MOIST

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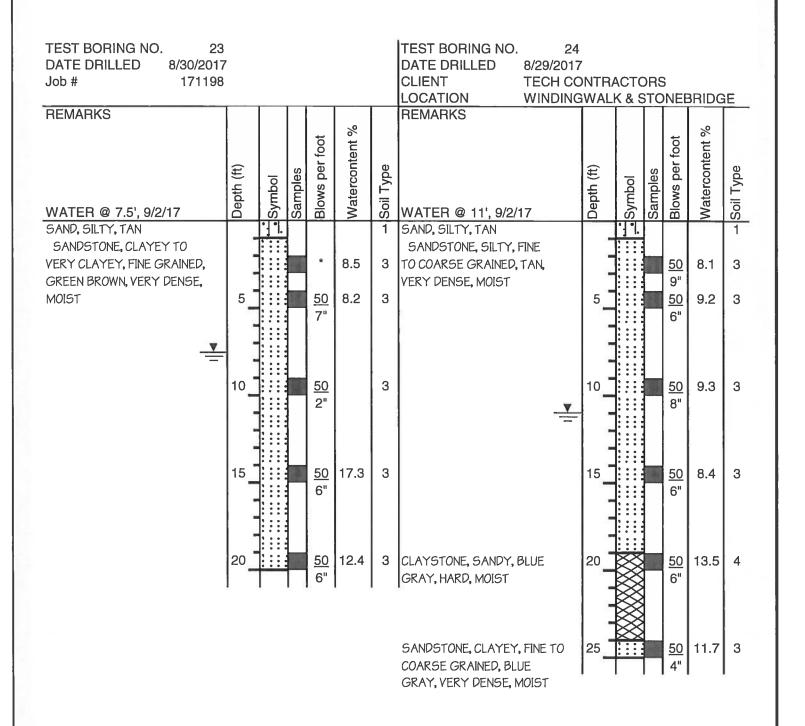
JOB NO.: 171198

TEST BORING NO. TEST BORING NO. 21 22 DATE DRILLED 8/30/2017 DATE DRILLED 8/30/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot **Natercontent** Watercontent Depth (ft) Soil Type - Soil Type Depth (ft) Samples Samples Symbol Symbol WATER @ 12.5', 9/2/17 WATER @ 14', 9/2/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, SILTY, FINE TO SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, VERY COARSE GRAINED, TAN, VERY DENSE, MOIST 5 50 9.6 3 DENSE, MOIST TO WET 5 3 <u>50</u> 9.1 9" 10 <u>50</u> 7.2 3 10 <u>50</u> 8.4 3 9" 6" 15 8.7 3 15 <u>50</u> <u>50</u> 8.5 3 6" 3 20 <u>50</u> 11.5 <u>50</u> 14.9 6" 9"



	TEST BORING LOG			
DRAWN:	DATE:	CHECKED:	n PATE:	

ЈОВ NO:: 171198





	TE	ST BORING LOG
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JOB NO.: 171198
FIG NO.:

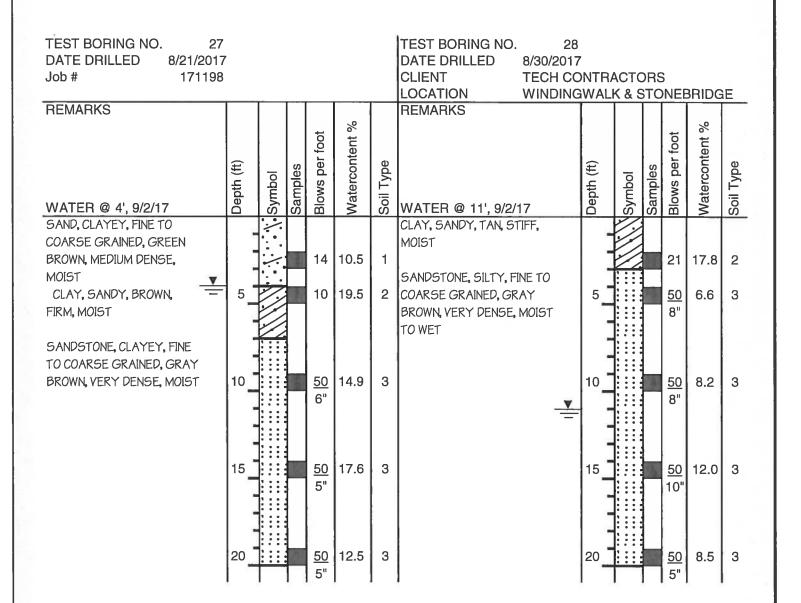
A- 12

TEST BORING NO. TEST BORING NO. 25 26 DATE DRILLED 8/29/2017 DATE DRILLED 8/21/2017 Job# **CLIENT TECH CONTRACTORS** 171198 LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot per foot Watercontent Watercontent Soil Type Depth (ft) Soil Type Samples Samples Symbol Symbol Blows r WATER @ 9', 9/2/17 WATER @ 7.5', 9/2/17 SAND, SILTY, CLAYEY, FINE SAND, SILTY, FINE TO COARSE TO MEDIUM GRAINED, TAN, GRAINED, TAN, MEDIUM DENSE, MOIST MEDIUM DENSE TO VERY 25 14.5 1 22 4.5 1 DENSE, MOIST <u>50</u> 14.3 1 SANDSTONE, SILTY, FINE 6.7 3 <u>50</u> 9" TO COARSE GRAINED, GREEN BROWN, VERY DENSE, MOIST 10 32 l 10.7 10 CLAY, VERY SANDY, TAN, 2 CLAYSTONE, SANDY, DARK 50 15.3 VERY STIFF, MOIST BROWN, HARD, MOIST SANDSTONE, SILTY, FINE TO COARSE GRAINED, GRAY 15 9.2 SANDSTONE, CLAYEY, FINE 15 <u>50</u> <u>50</u> 11.0 5" BROWN, VERY DENSE, MOIST GRAINED, GRAY BROWN, 5" VERY DENSE, MOIST TO WET 20 13.5 3 <u>50</u> <u>50</u> 18.5 25 3 <u>50</u> 15.4 <u>50</u> 5" 12.8 3



	TE	ST BORING LO	G
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JOB NO.: 171198 FIG NO.: A- 13





	TES	T BORING LO	G
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A- 14

TEST BORING NO. TEST BORING NO. 29 30 DATE DRILLED DATE DRILLED 8/30/2017 8/30/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Natercontent Watercontent Depth (ft) Samples Soil Type Soil Type Symbol Samples Symbol DRY TO 19.5', 9/2/17 WATER @ 19', 9/2/17 SAND, SILTY, TAN SAND, SILTY, FINE TO COARSE SANDSTONE, SILTY, FINE GRAINED, GREEN BROWN, TO COARSE GRAINED, GREEN 6.9 3 MOIST 8.7 1 BROWN, VERY DENSE, MOIST SANDSTONE, SILTY, FINE TO 5 <u>50</u> 10.6 COARSE GRAINED, GREEN 13.9 3 <u>50</u> BROWN, VERY DENSE, MOIST 10" 10 8.6 3 3 <u>50</u> CLAYEY LENSES 10 50 15.4 7" 15 3 <u>50</u> 8.8 15 5.9 3 <u>50</u> * - BULK SAMPLE TAKEN <u>50</u> 8.3 3 * - BULK SAMPLE TAKEN <u>50</u> 9.8



	TEST BORING LOG			
DRAWN:	DATE:	CHECKED	DATE: 11/8/17	

ЈОВ NO.: 171198

TEST BORING NO. TEST BORING NO. 31 32 DATE DRILLED 8/29/2017 DATE DRILLED 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Soil Type Symbol Samples Samples Symbol WATER @ 12.5', 9/2/17 WATER @ 15.5', 9/2/17 SAND, SILTY, TAN SAND, SILTY, FINE TO MEDIUM SANDSTONE, SLIGHTLY GRAINED, TAN, MEDIUM SILTY TO SILTY, FINE TO 50 4.8 3 DENSE TO DENSE, MOIST 15 5.7 COARSE GRAINED, TAN, VERY DENSE, MOIST TO WET <u>50</u> 4.8 3 5 33 9.3 1 SANDSTONE, SILTY TO VERY SILTY, FINE TO MEDIUM 10 10.6 <u>50</u> GRAINED, TAN, VERY DENSE, 10 <u>50</u> 11.7 3 11" MOIST TO WET 15 3 <u>50</u> 20.5 15 3 <u>50</u> 11.5 10" 20 3 <u>50</u> 14.6 20 17.3 <u>50</u> <u>50</u> 16.6 14.9 3 50 6" 19.2 3 <u>50</u>



	TEST BORING LOG		
DRAWN:	DATE:	CHECKED:	DATE: 118/17

JOB NO.: 171198 FIG NO.: A- 16

TEST BORING NO. TEST BORING NO. 33 34 DATE DRILLED DATE DRILLED 8/30/2017 8/30/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Watercontent Blows per foot Watercontent Depth (ft) Soil Type Soil Type Depth (ft) Samples Samples Symbol Symbol DRY TO 19.5', 9/2/17 WATER @ 14.5', 9/2/17 SAND, CLAYEY, FINE TO COARSE SAND, SILTY, FINE GRAINED GRAINED, TAN, MEDIUM DENSE, WITH COARSE GRAINED MOIST 19 9.9 1 LENSES, TAN, MEDIUM DENSE, 8.8 MOIST SANDSTONE, SILTY, FINE TO 5 50 8.9 3 5 17 11.2 COARSE GRAINED, TAN, VERY DENSE, MOIST SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN TO GREEN BROWN, VERY DENSE, 10 8.8 MOIST <u>50</u> 10 <u>50</u> 10.6 9" 8" 15 <u>50</u> 6.2 3 15 <u>50</u> 9.1 3 * - BULK SAMPLE TAKEN 20 <u>50</u> 7.3 SANDSTONE, CLAYEY, FINE <u>50</u> 11.9 6" GRAINED, GREEN BROWN, VERY DENSE, MOIST



	TE	TEST BORING LOG		
DRAWN:	DATE:	CHECKED:	DATE: 1/9/17	

JOB NO.: 171198

TEST BORING NO. TEST BORING NO. 35 36 DATE DRILLED DATE DRILLED 8/29/2017 8/16/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Soil Type Samples Samples Symbol Symbol Soil WATER @ 15', 9/2/17 WATER @ 17.5', 8/16/17 SAND, SILTY, FINE TO COARSE FILL 0-14', SAND, SILTY, FINE GRAINED, BROWN TO TAN, TO COARSE GRAINED, MEDIUM DENSE TO DENSE, 15 8.7 BROWN, MEDIUM DENSE, 18 11.9 1A MOIST MOIST 38 5 16.9 1 19 8.4 1A SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, VERY DENSE, MOIST TO WET 10 <u>50</u> 9.3 3 10 23 9.4 1A 8" 15 50 9.2 SAND, SILTY, FINE TO COARSE 15 22 8.5 1 8" GRAINED, DARK BROWN TO GREEN BROWN, MEDIUM DENSE, MOIST TO WET 20 <u>50</u> 9.1 3 24 9.9 <u>50</u> 14.3



	TES	ST BORING LO	g
DRAWN:	DATE:	CHECKED:	UIB/17

JOB NO.: 171198 FIG NO.:

A- 18

TEST BORING NO. TEST BORING NO. 37 38 DATE DRILLED DATE DRILLED 8/16/2017 8/16/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Soil Type Soil Type Samples Symbol Symbol DRY TO 19', 8/18/17 DRY TO 19.5', 8/18/17 FILL 0-8', SAND, SILTY TO FILL 0-6', SAND, SILTY, FINE CLAYEY, FINE TO COARSE TO COARSE GRAINED, BROWN, GRAINED, BROWN, MEDIUM 22 8.3 1A DENSE TO MEDIUM DENSE, 1A 34 6.3 DENSE, MOIST MOIST 28 10.7 1A 16 9.0 1A SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, SAND, SILTY, FINE TO COARSE VERY DENSE, MOIST TO GRAINED, BROWN, MEDIUM 10 16 11.6 1 WET 10 <u>50</u> 8.4 3 5" DENSE, MOIST 3 SANDSTONE, SILTY, FINE TO <u>50</u> 12.0 15 9.8 <u>50</u> COARSE GRAINED, BROWN, VERY DENSE, MOIST <u>50</u> 5.8 3 <u>50</u> 14.8



	TEST	BORING LOG	à
DRAWN:	DATE:	CHECKED:	11/8/17

JOB NO.: 171198

TEST BORING NO. TEST BORING NO. 39 40 DATE DRILLED 8/16/2017 DATE DRILLED 8/16/2017 Job# 171198 **CLIENT TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Blows per foot Blows per foot Watercontent Depth (ft) Soil Type Soil Type Samples Samples Symbol Symbol WATER @ 6', 8/18/17 WATER @ 15', 8/18/17 FILL O-7', SAND, CLAYEY TO SAND, SILTY, TAN SILTY, FINE TO COARSE SANDSTONE, SILTY, FINE TO GRAINED, BROWN, MEDIUM 21 10.8 1A COARSE GRAINED, BROWN TO 6.2 3 50 DENSE, MOIST GRAY BROWN, VERY DENSE, 10" 5 15 10.2 1A MOIST <u>50</u> 7.1 3 10" SAND, SILTY, FINE TO COARSE GRAINED, DARK BROWN. MEDIUM DENSE, MOIST 10 20 8.5 1 10 <u>50</u> 9.1 3 SANDSTONE, SILTY, FINE TO COARSE GRAINED, TAN, 15 VERY DENSE, MOIST 50 7.1 3 15 3 <u>50</u> 9.4 SANDSTONE, SILTY, FINE <u>50</u> 14.8 <u>50</u> 10.0 GRAINED, TAN, VERY DENSE, MOIST



	TEST	BORING LOG	
DRAWN:	DATE:	CHECKED:	DATE:

JOB NO.: 171198
FIG NO.:

A- 20

TEST BORING NO. 41 TEST BORING NO. 42 DATE DRILLED DATE DRILLED 8/16/2017 8/16/2017 CLIENT Job# 171198 **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Soil Type Soil Type Samples Samples Symbol Symbol WATER @ 15.5', 8/18/17 WATER @ 18', 8/18/17 SAND, SILTY, TAN SAND, SILTY, FINE TO COARSE SANDSTONE, SILTY, FINE GRAINED, TAN, MEDIUM DENSE, TO COARSE GRAINED, BROWN, <u>50</u> 6.0 MOIST 3 26 9.7 1 VERY DENSE, MOIST TO WET 9" 5 <u>50</u> 8.1 SANDSTONE, SILTY, FINE TO 50 8.9 3 COARSE GRAINED, BROWN, VERY DENSE, MOIST 10 10.5 3 10 <u>50</u> 50 9.1 3 15 <u>50</u> 8.5 3 15 12.7 <u>50</u> 6" SANDSTONE, VERY SILTY, <u>50</u> 3 FINE GRAINED, TAN, VERY 20 3 13.9 <u>50</u> 8.8 8" DENSE, MOIST



	TE	EST BORING LO	G
DRAWN:	DATE:	CHECKED:	h/8/17

JOB NO.: 171198 FIG NO.: A- 21

TEST BORING NO. 43 TEST BORING NO. 44 DATE DRILLED DATE DRILLED 8/16/2017 8/16/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Soil Type Samples Samples Symbol Symbol DRY TO 18.5', 8/18/17 WATER @ 20', 8/18/17 SAND, SILTY, TAN SAND, SILTY, TAN CLAYSTONE, VERY SANDY, WEATHERED TO FORMATIONAL 37 8.8 3 BLUE GRAY, HARD, MOIST <u>50</u> 16.1 SANDSTONE, SILTY, FINE 9" TO COARSE GRAINED, GREEN 5 50 7.8 3 <u>50</u> 10.0 4 BROWN, DENSE TO VERY 8" DENSE, MOIST CLAYSTONE, VERY SANDY, SANDSTONE, SILTY, FINE TO GREEN BROWN, HARD, MOIST 10 <u>50</u> 15.3 10 COARSE GRAINED, TAN TO 50 7.0 3 5" GREEN BROWN, VERY DENSE, SANDSTONE, SILTY, FINE TO MOIST COARSE GRAINED, TAN, VERY DENSE, MOIST 15 3 8.4 <u>50</u> 6.3 15 <u>50</u> 3 <u>50</u> 8.4 3 <u>50</u> 12.5



	TEST	BORING LO	G
DRAWN:	DATE:	CHECKED:	DATE:

JOB NO.: 171198

TEST BORING NO. TEST BORING NO. 45 46 DATE DRILLED 8/29/2017 DATE DRILLED 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Watercontent Blows per foot Watercontent Soil Type Depth (ft) Samples Samples Symbol Symbol WATER @ 18', 9/2/17 DRY TO 19', 9/2/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, SILTY TO SANDSTONE, VERY CLAYEY, SLIGHTLY SILTY, FINE TO <u>50</u> 6.6 3 FINE GRAINED, BROWN, VERY 9.9 3 COARSE GRAINED, TAN, 10" DENSE, MOIST VERY DENSE, MOIST 5 <u>50</u> 5.7 3 5 50 11.9 9" 6" 10 8.0 <u>50</u> SANDSTONE, SILTY, FINE TO 10 <u>50</u> 7.4 COARSE GRAINED, BROWN, CLAYSTONE, SANDY, GREEN VERY DENSE, MOIST BROWN, HARD, MOIST 15 <u>50</u> 12.2 4 15 <u>50</u> 7.9 3 6" SANDSTONE, SILTY, FINE TO ____ COARSE GRAINED, BROWN, VERY DENSE, MOIST <u>50</u> 7.5 <u>50</u> 9.7 9"



	TEST	BORING LOG	ì
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17

JOB NO.: 171198 FIG NO.:

A- 23

TEST BORING NO. TEST BORING NO. 47 48 DATE DRILLED DATE DRILLED 8/29/2017 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Soil Type Depth (ft) Samples Samples Symbol Symbol DRY TO 19', 9/2/17 WATER @ 18', 9/2/17 SAND, SILTY, FINE TO COARSE SAND, SILTY, TAN GRAINED, GREEN BROWN, SANDSTONE, SLIGHTLY MEDIUM DENSE, MOIST 28 5.3 SILTY TO SILTY, FINE TO 6.7 3 50 COARSE GRAINED, TAN TO 10" SANDSTONE, SILTY, FINE TO 5 50 10.6 3 GREEN BROWN, VERY DENSE, <u>50</u> 7.1 3 COARSE GRAINED, TAN TO 8" MOIST 6" GREEN BROWN, VERY DENSE, MOIST 10 5.1 3 <u>50</u> 10 50 6.2 3 7" 15 3 <u>50</u> 4.6 15 <u>50</u> 7.0 3 5" <u>50</u> 7.7 3 <u>50</u> 9.0 3



	TE	ST BORING LO	G
DRAWN:	DATE:	CHECKED:	11/8/17

JOB NO.: 171198

TEST BORING NO. TEST BORING NO. 49 50 DATE DRILLED 8/29/2017 DATE DRILLED 8/29/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Soil Type Depth (ft) - Soil Type Samples Samples Symbol Symbol DRY TO 20', 9/2/17 DRY TO 20', 8/29/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, SILTY, FINE TO SANDSTONE, SILTY, FINE TO COARSE GRAINED WITH FINE 7.8 3 COARSE GRAINED, TAN TO 3 50 <u>50</u> 4.8 11" GRAINED LENSES, TAN, VERY GREEN BROWN, VERY DENSE, 11" DENSE, MOIST <u>50</u> 8.2 3 MOIST <u>50</u> 5 8.4 10" 10" 10 5.2 <u>50</u> 7.1 3 10 <u>50</u> 3 5" 6" 15 7.6 3 <u>50</u> 15 <u>50</u> 8.6 3 8" <u>50</u> 12.2 <u>50</u> 8.8 3



	TE	ST BORING LO	G
DRAWN:	DATE:	CHECKED:	DATE:

JOB NO.: 171198 FIG NO.:

A- 25

TEST BORING NO. TEST BORING NO. 51 52 DATE DRILLED 8/29/2017 DATE DRILLED 8/29/2017 Job# 171198 **CLIENT TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS **REMARKS** Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Soil Type Soil Type Samples Samples Symbol Symbol DRY TO 18.5', 9/2/17 DRY TO 20', 8/29/17 SAND, SILTY, TAN SAND, SILTY, TAN SANDSTONE, SILTY, FINE TO SANDSTONE, SLIGHTLY]: : : : COARSE GRAINED, TAN TO 50 4.6 3 SILTY TO SILTY, FINE TO 10.1 3 GREEN BROWN, VERY DENSE, COARSE GRAINED, TAN TO MOIST <u>50</u> 6.2 3 GREEN BROWN, VERY DENSE, 5.2 3 50 9" MOIST 10 3 <u>50</u> 5.6 10 <u>50</u> 5.2 3 7" 5" 15 <u>50</u> 8.5 3 15 5.5 3 <u>50</u> <u>50</u> 7.6 3 * - BULK SAMPLE TAKEN 3 <u>50</u> 9.5



	, TE	ST BORING LO	G
DRAWN:	DATE:	CHECKED:	DATE: //7

JOB NO.: 171198

TEST BORING NO. TEST BORING NO. 53 54 DATE DRILLED 8/29/2017 DATE DRILLED 8/30/2017 Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Blows per foot Blows per foot Watercontent Watercontent Depth (ft) Depth (ft) Samples Samples Soil Type Symbol Symbol Soil DRY TO 19.5', 9/2/17 WATER @ 18.5', 9/2/17 SAND, SILTY, TAN SAND, SILTY, FINE TO COARSE SANDSTONE, SILTY, FINE GRAINED, TAN, MEDIUM DENSE 20.9 GRAINED, TAN, VERY DENSE, 3 TO DENSE, MOIST MOIST 5 10.0 3 50 26 4.3 6" 10 <u>50</u> 12.3 3 10 43 9.4 1 6" SANDSTONE, SILTY, FINE TO COARSE GRAINED, GREEN 15 7.3 BROWN, VERY DENSE, MOIST <u>50</u> CLAYSTONE, SANDY, TAN, 15 50 11.4 HARD, MOIST SANDSTONE, SILTY, FINE * - BULK SAMPLE TAKEN <u>50</u> 7.0 3 TO COARSE GRAINED, TAN, 20 8.3 3 6" VERY DENSE, MOIST



	TEST	BORING LOG	à
DRAWN:	DATE:	CHECKED	11/6/17

JOB NO.: 171198

TEST BORING NO. TEST BORING NO. 55 DATE DRILLED 8/30/2017 DATE DRILLED Job# 171198 CLIENT **TECH CONTRACTORS** LOCATION WINDINGWALK & STONEBRIDGE REMARKS REMARKS Watercontent % Watercontent % Blows per foot Blows per foot Soil Type Depth (ft) Soil Type Samples Samples Symbol Symbol DRY TO 20', 9/2/17 SAND, SLIGHTLY SILTY TO SILTY, FINE TO COARSE 12 | 2.7 1 GRAINED, TAN, MEDIUM DENSE TO DENSE, MOIST 5 40 6.9 1 SANDSTONE, SILTY, FINE TO 10 <u>50</u> 8.4 3 10 8' COARSE GRAINED, TAN, VERY DENSE, MOIST 15 8.8 3 <u>50</u> 15 9" <u>50</u> 7.0 3

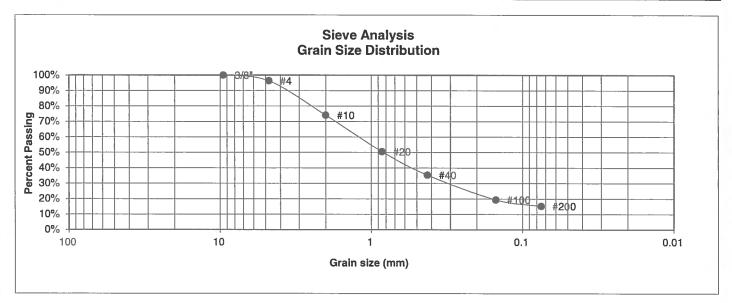


	TEST	BORING LOG	ì
DRAWN:	DATE:	CHECKED:	DATE: 01/8/17

JOB NO.: 171198

APPENDIX B: Laboratory Testing Results

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1A	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	36	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL

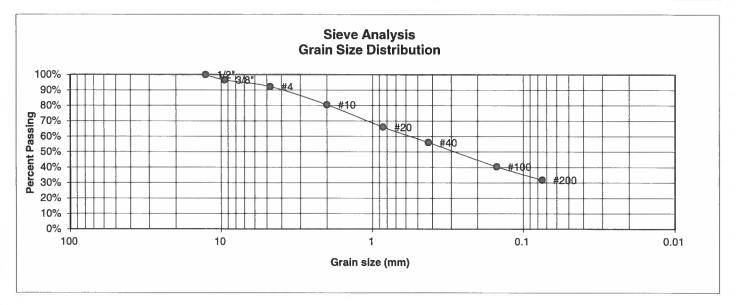


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	96.3%	<u>Swell</u>
10	74.1%	Moisture at start
20	50.6%	Moisture at finish
40	35.4%	Moisture increase
100	19.4%	Initial dry density (pcf)
200	15.3%	Swell (psf)



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DRAWN:	DATE:	CHECKED:	h	U/8/17

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1A	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	37	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



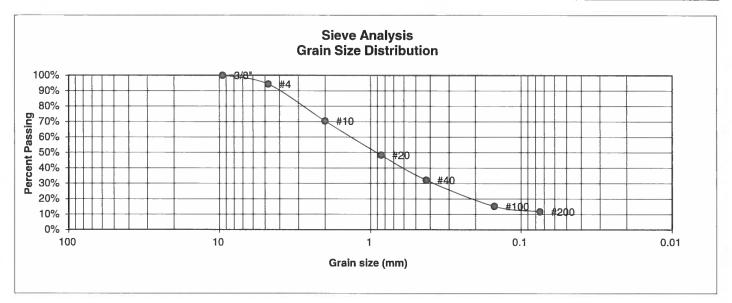
U.S. <u>Sieve #</u> 3"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit	
1 1/2"		Liquid Limit	
3/4"		Plastic Index	
1/2"	100.0%		
3/8"	96.6%		
4	92.2%	<u>Swell</u>	
10	80.5%	Moisture at start	7.6%
20	66.0%	Moisture at finish	16.3%
40	56.1%	Moisture increase	8.8%
100	40.4%	Initial dry density (pcf)	108
200	31.8%	Swell (psf)	240



	LABOI RESUI	RATORY TI	EST		
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FIG NO.: B-2

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	2	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

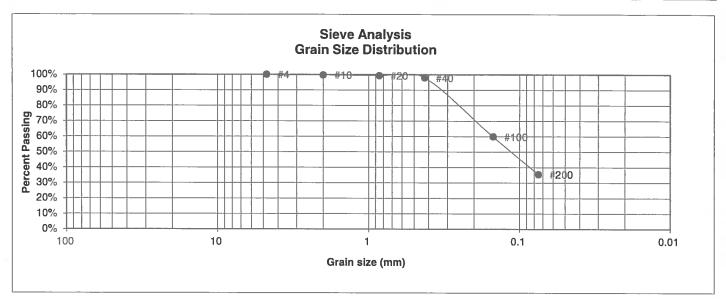


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	100.0%	
4	94.3%	<u>Swell</u>
10	70.4%	Moisture at start
20	48.3%	Moisture at finish
40	32.2%	Moisture increase
100 200	15.2% 11.7%	Initial dry density (pcf) Swell (psf)



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DRAWN:	DATE:	CHECKED:	n	11/8/17

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	4	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	100.0%	<u>Swell</u>
10	99.7%	Moisture at start
20	99.3%	Moisture at finish
40	97.9%	Moisture increase
100	59.8%	Initial dry density (pcf)
200	35.5%	Swell (psf)



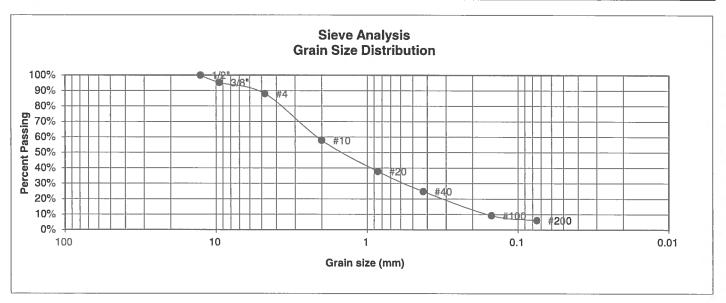
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DRAWN:	DATE:	CHECKED:	n	DATE:

20 29 9

> JOB NO.: 171198

FIGNO.: B-4

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	8	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL

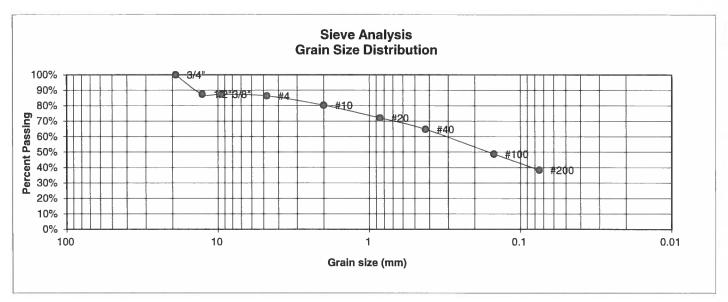


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0% 95.2%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	88.0%	<u>Swell</u>
10	57.8%	Moisture at start
20	37.8%	Moisture at finish
40	24.8%	Moisture increase
100	9.2%	Initial dry density (pcf)
200	6.1%	Swell (psf)



	LABO! RESUI	RATORY T LTS	EST	
DRAWN:	DATE:	CHECKED	n	DATE: 11/8/17

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	13	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



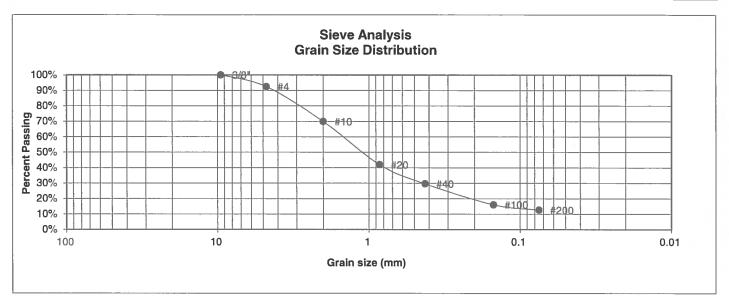
U.S. <u>Sieve #</u> 3" 1 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit 13 Liquid Limit 28
3/4"	100.0%	Plastic Index 15
1/2"	87.4%	
3/8"	87.4%	
4	86.4%	<u>Swell</u>
10	80.3%	Moisture at start
20	72.1%	Moisture at finish
40	64.7%	Moisture increase
100 200	48.8% 38.4%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	DATE: 11/8/17

FIG NO.: B-6

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	15	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL

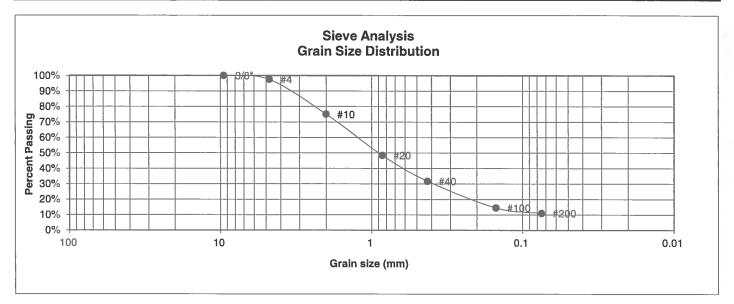


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	92.4%	<u>Swell</u>
10	69.9%	Moisture at start
20 40	42.0% 29.6%	Moisture at finish Moisture increase
100	16.1%	Initial dry density (pcf)
200	12.6%	Swell (psf)



	LABOI RESU	RATORY T LTS	EST	
DRAWN:	DATE:	CHECKED:	a	DATE: 11/8/17

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	19	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



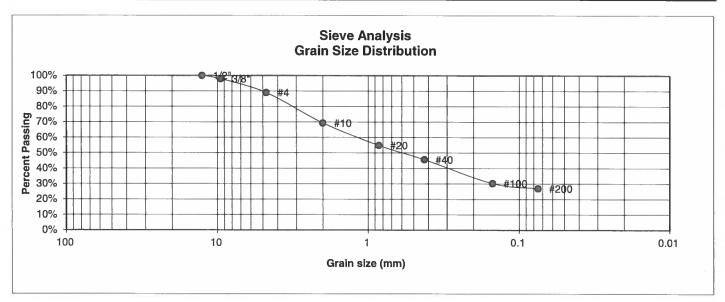
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	97.5%	<u>Swell</u>
10	75.1%	Moisture at start
20 40	48.5% 31.8%	Moisture at finish Moisture increase
100	14.5%	Initial dry density (pcf)
200	11.0%	Swell (psf)



LABORATORY TEST RESULTS			
DRAWN:	DATE	CHECKED:	DATE: 11/8/17

FIG NO.: B-8

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	30	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



U.S. Sieve # 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8" 4	97.9% 89.0%	Swell
10	69.3%	Moisture at start
20 40	54.8% 45.7%	Moisture at finish Moisture increase
100 200	30.3% 26.9%	Initial dry density (pcf) Swell (psf)

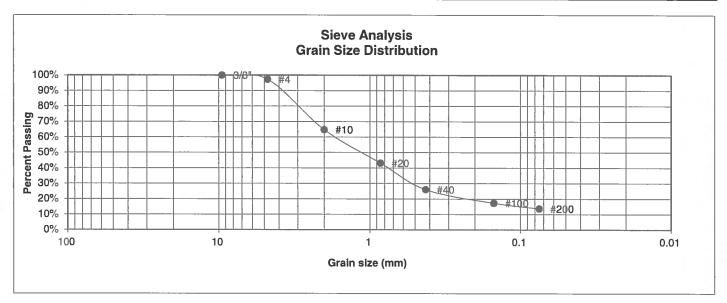


	LABO! RESUI	RATORY TEST LTS	
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17

JOB NO.: 171 198

B-9

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	34	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL

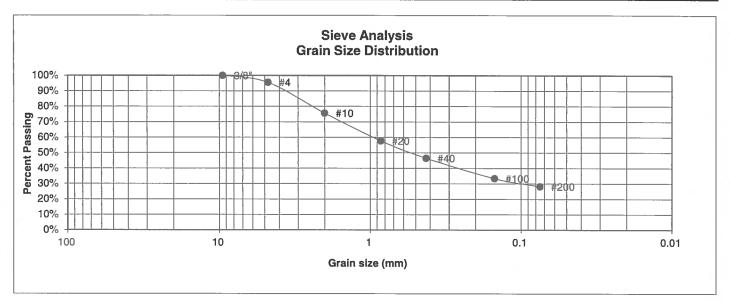


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8" 4	100.0% 97.3%	Swell
10	64.7%	Moisture at start
20 40	43.1% 26.0%	Moisture at finish Moisture increase
100 200	17.3% 13.7%	Initial dry density (pcf) Swell (psf)



	LABOI RESUI	RATORY T LTS	EST		
DRAWN:	DATE:	CHECKED;	a	1/8/17	

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	35	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL

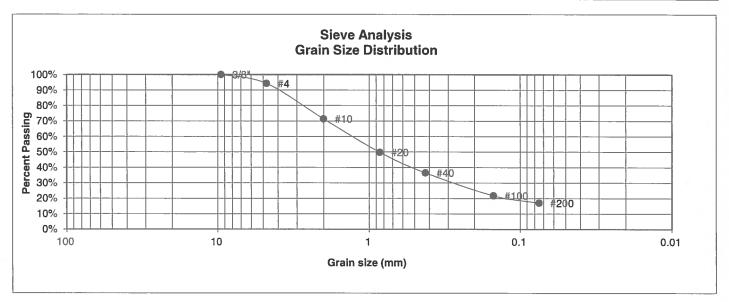


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	95.5%	<u>Swell</u>
10	75.6%	Moisture at start
20	57.7%	Moisture at finish
40	46.5%	Moisture increase
100	33.4%	Initial dry density (pcf)
200	28.2%	Swell (psf)



	LABOI RESUI	RATORY T LTS	EST	
DRAWN:	DATE	CHECKED:	n	1/8/17

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	39	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

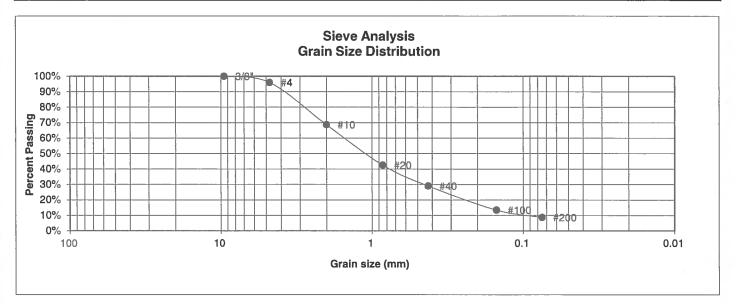


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	94.3%	<u>Swell</u>
10	71.3%	Moisture at start
20	49.8%	Moisture at finish
40	36.5%	Moisture increase
100	21.8%	Initial dry density (pcf)
200	17.1%	Swell (psf)



	LABOI RESU	RATORY TEST LTS	
DRAWN:	DATE:	CHECKED:	11/8/17

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	1	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	55	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL

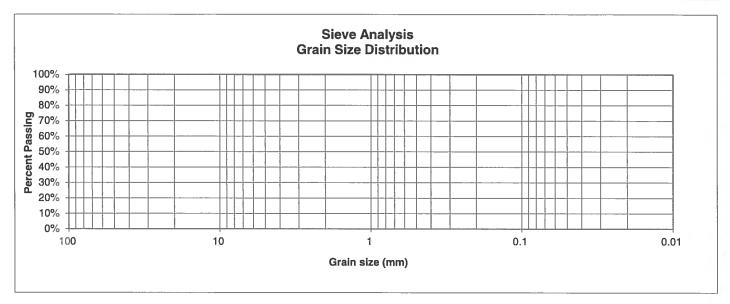


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
3/8" 4	100.0% 96.0%	Swell
10	68.6%	Moisture at start
20	42.5%	Moisture at finish
40	29.0%	Moisture increase
100 200	13.5% 8.7%	Initial dry density (pcf) Swell (psf)



	LABORATORY TEST RESULTS			
DRAWN:	DATE:	CHECKED:	a	DATE: 0/8/17

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	2	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	3	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL



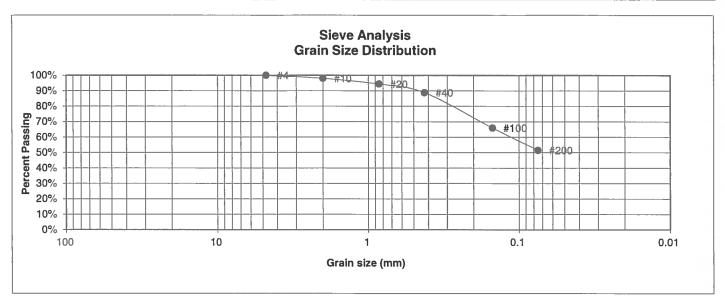
U.S.	Percent	Atterberg
Sieve # 3"	<u>Finer</u>	<u>Limits</u>
ى 1 1/2"		Plastic Limit Liquid Limit
3/4"		Plastic Index
1/2"		i lastic fildex
3/8"		
4		Swell
10		Moisture at start 13.6%
20		Moisture at finish 23.9%
40		Moisture increase 10.3%
100		Initial dry density (pcf) 101
200		Swell (psf) 1720



LABORATORY TEST RESULTS						
DRAWN:	DATE:	CHECKED:	h	U/8/17		

FIG NO.: B-14

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	2	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	25	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



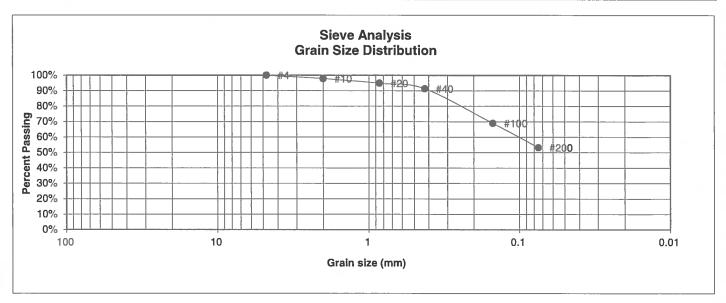
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index	10 20 10		
4	100.0%	<u>Swell</u>			
10	98.0%	Moisture at start			
20	94.4%	Moisture at finish			
40	88.8%	Moisture increase	Moisture increase		
100 200	66.0% 51.6%	Initial dry density (pcf) Swell (psf)			
		(1 - /			



LABORATORY TEST RESULTS						
DRAWN:	DATE:	CHECKED	a	DATE: 11/8/17		

JOB NO.: 171198

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	2	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	28	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



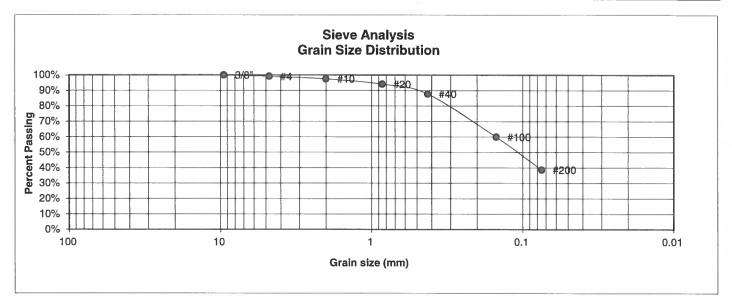
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4 10 20 40 100 200	100.0% 97.9% 94.9% 91.3% 69.0% 53.2%	Swell Moisture at start Moisture at finish Moisture increase Initial dry density (pcf) Swell (psf)



	RESULT		IESI	
DRAWN:	DATE:	CHECKED:	h	DATE: 11/8/17

> JOB NO.:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	1	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

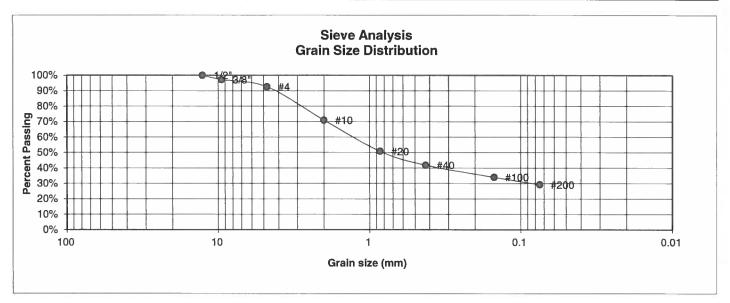


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4 10 20 40 100 200	99.1% 97.5% 94.1% 87.8% 59.9% 38.7%	Swell Moisture at start Moisture at finish Moisture increase Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS			
DRAWN:	DATE:	CHECKED:	DATE: 4/8/17

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	3	JOB NO.	171198
DEPTH (FT)	20	TEST BY	BL

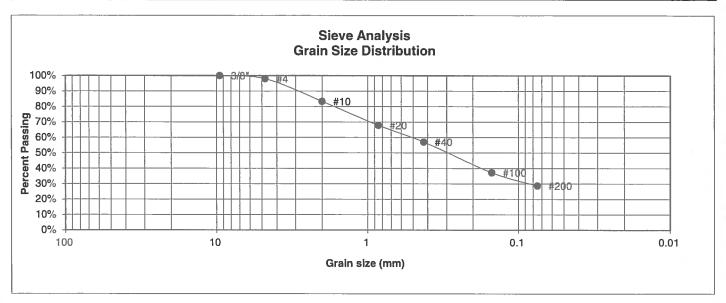


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit 24 Liquid Limit 36 Plastic Index 12
1/2"	100.0%	
3/8"	97.2%	
4	92.6%	<u>Swell</u>
10	71.0%	Moisture at start
20	50.8%	Moisture at finish
40	41.8%	Moisture increase
100	34.0%	Initial dry density (pcf)
200	29.2%	Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED: W	DATE: 11/8/17	

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	5	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL

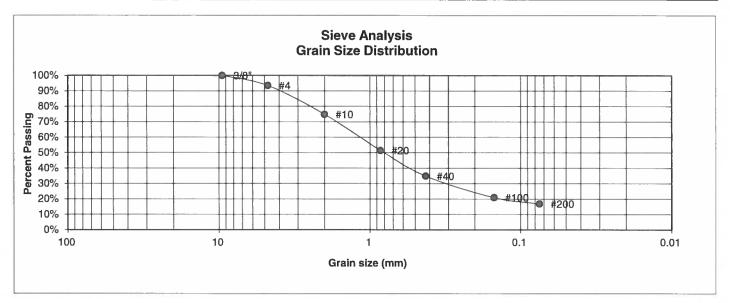


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit 27 Liquid Limit 41 Plastic Index 14
3/8"	100.0%	
4	98.0%	<u>Swell</u>
10	83.4%	Moisture at start
20	67.9%	Moisture at finish
40	57.1%	Moisture increase
100 200	37.3% 28.7%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN	DATE:	CHECKED:	h	DATE: 4/8/17

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	7	JOB NO.	171198
DEPTH (FT)	20	TEST BY	BL

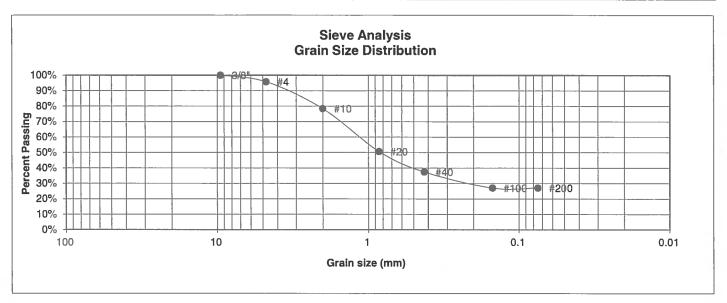


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	100.0%	
4	93.5%	<u>Swell</u>
10	74.8%	Moisture at start
20	51.5%	Moisture at finish
40	34.9%	Moisture increase
100 200	20.9% 16.9%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17	

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	9	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

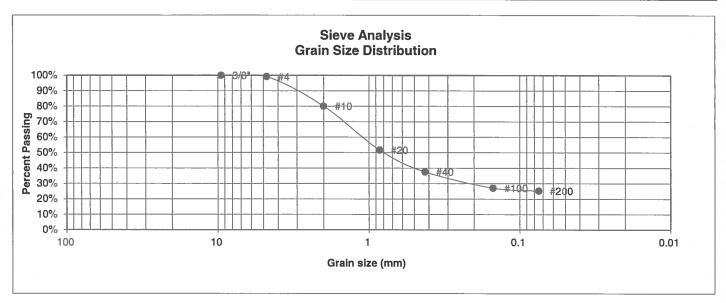


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg Limits Plastic Limit NP Liquid Limit NV Plastic Index NP
4	95.8% 78.4%	<u>Swell</u> Moisture at start
20	50.7%	Moisture at finish
40	37.4%	Moisture increase
100	27.1%	Initial dry density (pcf)
200	27.1%	Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	2	DATE: 11/8/17

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	10	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



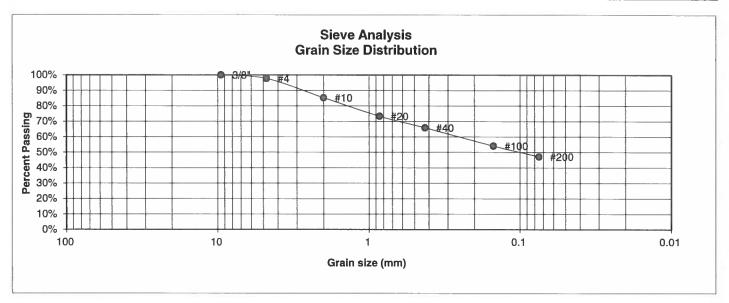
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	99.1%	<u>Swell</u>
10	80.1%	Moisture at start
20	51.7%	Moisture at finish
40	37.6%	Moisture increase
100	27.1%	Initial dry density (pcf)
200	25.2%	Swell (psf)

DRAWN:



LABOR/ RESULT	ATORY 1 [S_	ΓEST	
DATE:	CHECKED:	h	DATE:

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	11	JOB NO.	171198
DEPTH (FT)	20	TEST BY	BL



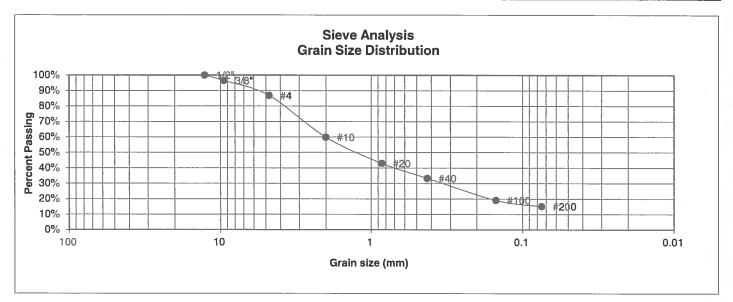
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	100.0%	CII
4	97.7%	<u>Swell</u>
10	85.3%	Moisture at start
20	73.3%	Moisture at finish
40	65.9%	Moisture increase
100	54.1%	Initial dry density (pcf)
200	47.2%	Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	DATE: 4/8/17	

> JOB NO.:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	12	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

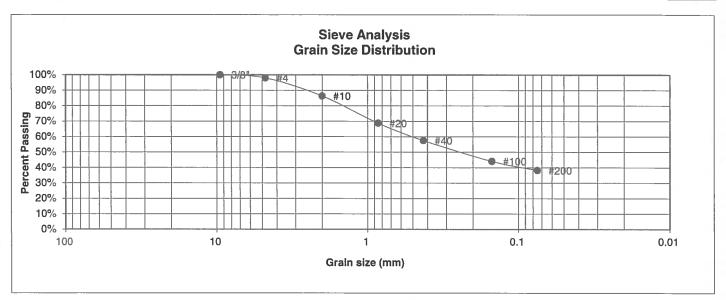


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
1/2" 3/8"	100.0% 96.5%	
4	86.9%	<u>Swell</u>
10	59.9%	Moisture at start
20	43.0%	Moisture at finish
40	33.3%	Moisture increase
100	19.1%	Initial dry density (pcf)
200	15.1%	Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	4/8/17

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	14	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL

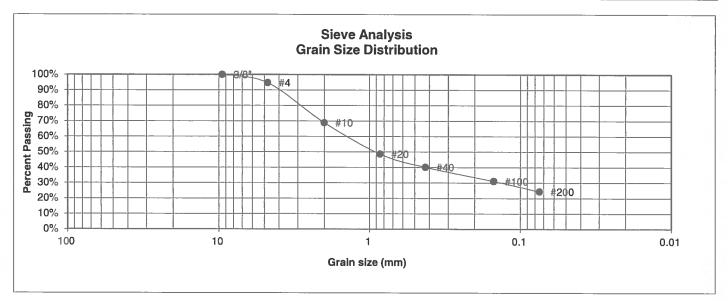


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8" 4	100.0% 98.0%	Swell
10	86.3%	Moisture at start
20 40	68.7% 57.5%	Moisture at finish Moisture increase
100 200	44.1% 38.2%	Initial dry density (pcf) Swell (psf)



	LABORA RESULT	TORY TE	ST	
DRAWN:	DATE:	CHECKED:	h	DATE: 11/8/17

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	16	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL

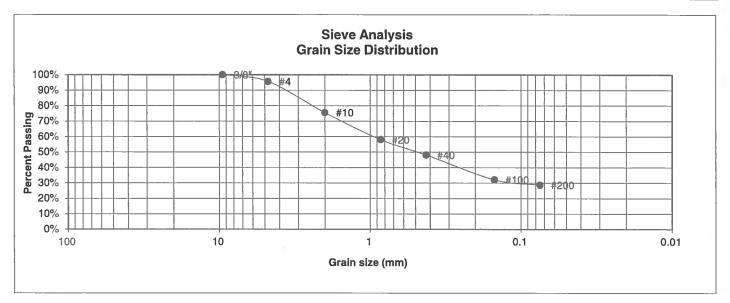


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	94.7%	<u>Swell</u>
10	68.9%	Moisture at start
20	48.6%	Moisture at finish
40	40.1%	Moisture increase
100	31.1%	Initial dry density (pcf)
200	24.4%	Swell (psf)



	LABOI RESU	RATORY TES LTS	Т
DRAWN:	DATE:	CHECKED:	DATE:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	18	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

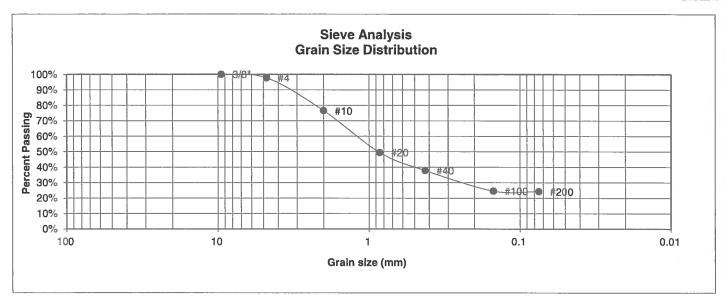


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	100.0%	
4	95.7%	<u>Swell</u>
10	75.5%	Moisture at start
20	58.0%	Moisture at finish
40	48.2%	Moisture increase
100 200	32.1% 28.7%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	n	DATE: 11/8/17

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	20	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



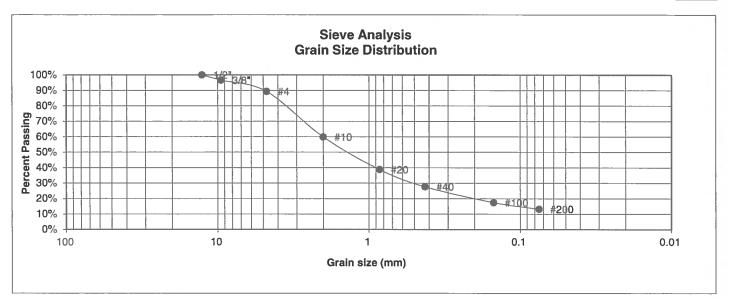
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	100.0%	
4	97.8%	<u>Swell</u>
10	76.6%	Moisture at start
20	49.5%	Moisture at finish
40	37.8%	Moisture increase
100 200	24.6% 24.3%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	U/8/17	

FIG NO.: 13-28

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	21	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



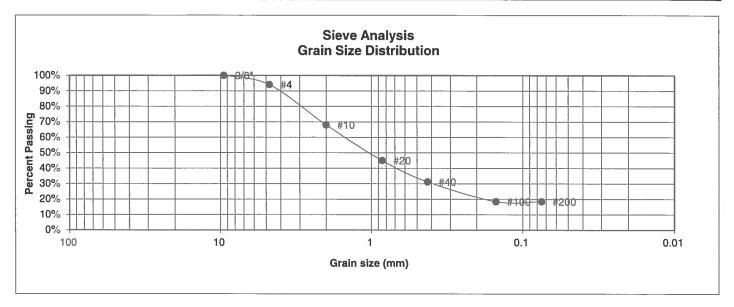
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
1/2" 3/8"	100.0% 96.6%	
4	89.3%	<u>Swell</u>
10	59.9%	Moisture at start
20	38.8%	Moisture at finish
40	27.7%	Moisture increase
100	17.5%	Initial dry density (pcf)
200	13.2%	Swell (psf)



LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED:	A	U/B/17	

FIG NO.: 13-29

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	22	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL

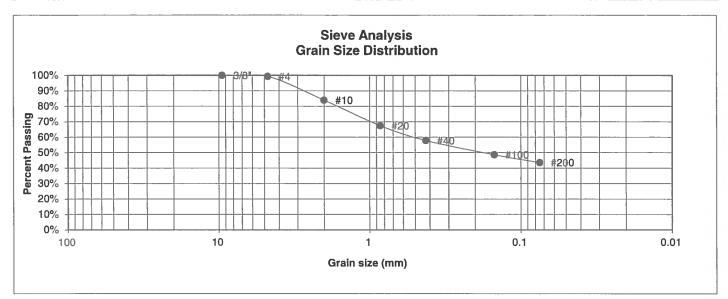


U.S. Sieve # 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
1/2"	100.00	
3/8"	100.0%	
4	94.1%	<u>Swell</u>
10	68.0%	Moisture at start
20	45.0%	Moisture at finish
40	31.2%	Moisture increase
100 200	18.5% 18.5%	Initial dry density (pcf) Swell (psf)



	LABOI RESUI	RATORY T LTS	EST	
DRAWN:	DATE:	CHECKED:	a	DATE: 4/8/17

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	23	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL



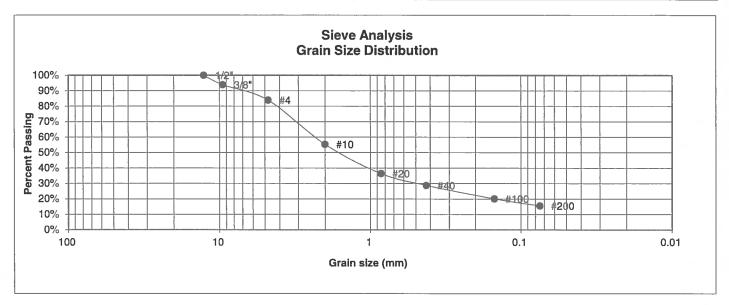
U.S. Sieve # 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8" 4	100.0% 99.3%	Swall
10	84.0%	<u>Swell</u> Moisture at start
20	67.5%	Moisture at finish
40	57.8%	Moisture increase
100 200	48.6% 43.5%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED	L	DATE: U/8/17

FIGNO .:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	29	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL

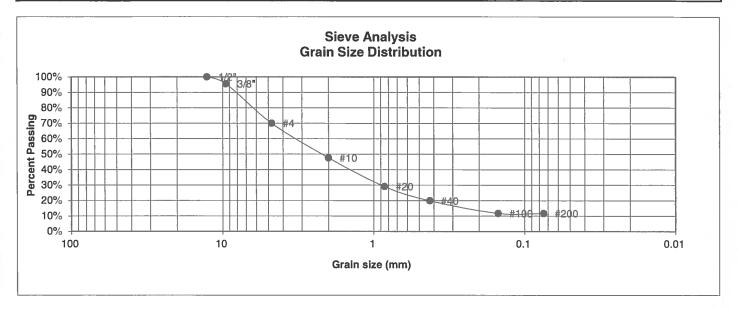


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
1/2"	100.0%	
3/8"	93.9%	
4	83.9%	<u>Swell</u>
10	55.3%	Moisture at start
20	36.4%	Moisture at finish
40	28.8%	Moisture increase
100	20.1%	Initial dry density (pcf)
200	15.6%	Swell (psf)



	LABOI RESU	RATORY T LTS	EST	
DRAWN:	DATE:	CHECKED:	h	DATE:

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	31	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL



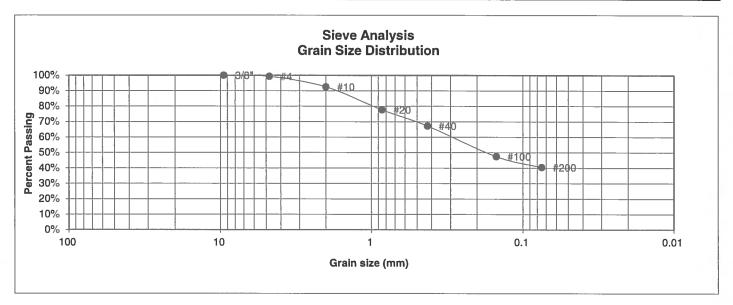
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent Finer	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
3/8" 4	95.6% 70.0%	Swell
10	47.6%	Moisture at start
20 40	29.1% 19.9%	Moisture at finish Moisture increase
100 200	11.8% 11.8%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	a	DATE/8/17

JOB NO.: 171198 FIG NO.:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	32	JOB NO.	171198
DEPTH (FT)	25	TEST BY	BL



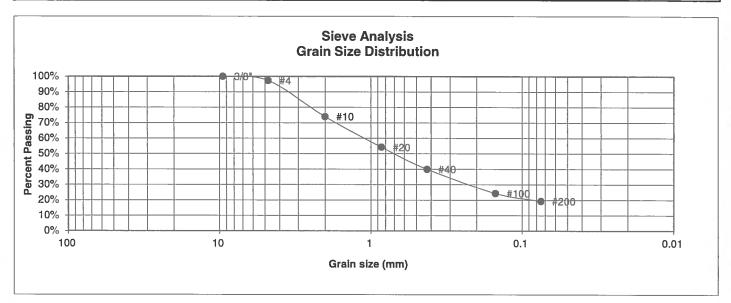
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	99.3%	Swell
10	92.5%	Moisture at start
20	77.7%	Moisture at finish
40	67.2%	Moisture increase
100 200	47.5% 40.4%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	a	U/8/17

B-34

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	33	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL]

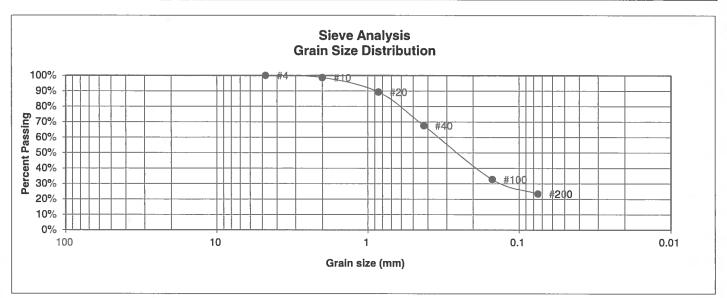


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
1/2"		Flastic index
3/8"	100.0%	
4	97.3%	<u>Swell</u>
10	73.9%	Moisture at start
20	54.2%	Moisture at finish
40	40.0%	Moisture increase
100 200	24.4% 19.3%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	a	DATE:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	38	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL

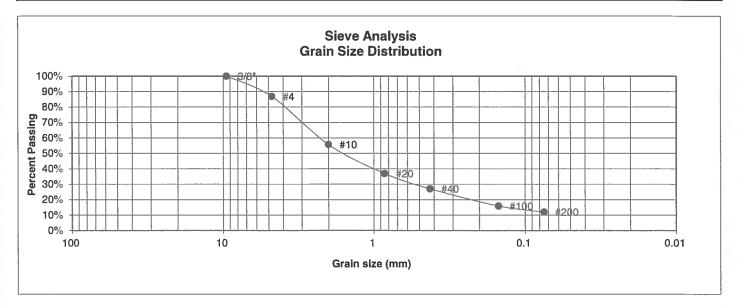


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	100.0%	Swell
10	98.7%	Moisture at start
20	89.3%	Moisture at finish
40	67.5%	Moisture increase
100 200	32.8% 23.4%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	n	UBITE

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	40	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL

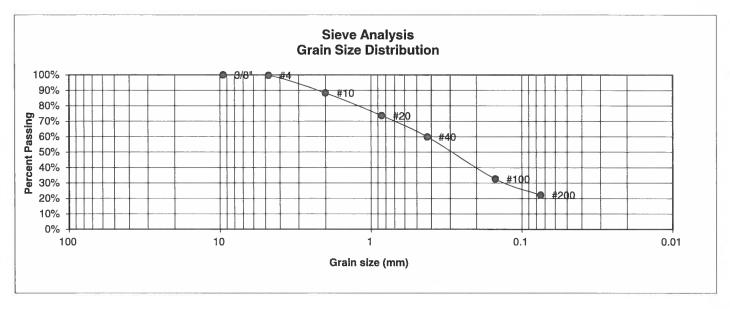


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
3/8"	100.0%	
4	86.9%	<u>Swell</u>
10	55.7%	Moisture at start
20	37.0%	Moisture at finish
40	27.0%	Moisture increase
100 200	15.9% 12.0%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	U/8/12	

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	41	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL

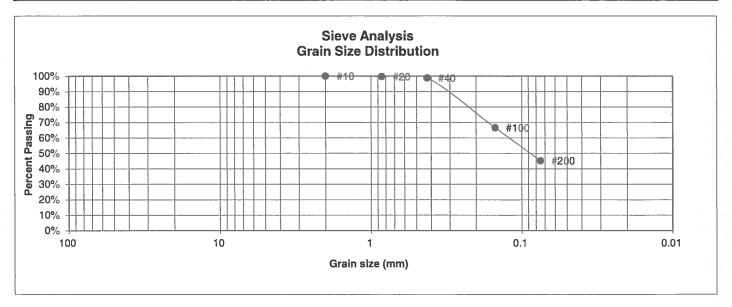


U.S. Sieve # 3" 1 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit
3/4"		Plastic Index
1/2" 3/8"	100.0%	
4	99.7%	Swell
10	88.3%	Moisture at start
20	73.6%	Moisture at finish
40	59.8%	Moisture increase
100 200	32.6% 22.2%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED:	a	DATE: 11/8/17	

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	42	JOB NO.	171198
DEPTH (FT)	20	TEST BY	BL

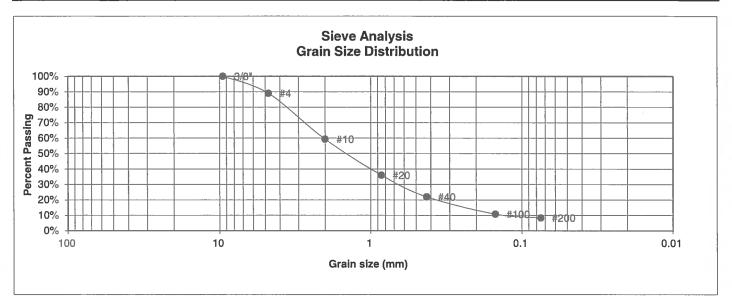


Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
100.0%	<u>Swell</u> Moisture at start
99.7% 98.8%	Moisture at finish Moisture increase
66.6% 45.3%	Initial dry density (pcf) Swell (psf)
	Finer 100.0% 99.7% 98.8% 66.6%



LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED: DATE: LI/B/17	_		

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	45	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL

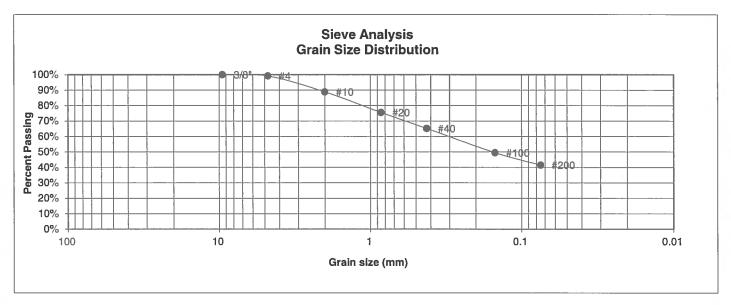


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4	89.0%	<u>Swell</u>
10	59.3%	Moisture at start
20	36.0%	Moisture at finish
40	21.9%	Moisture increase
100	10.8%	Initial dry density (pcf)
200	8.4%	Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	n	PATEOLIT

UNIFIED CLASSIFICATION	SC	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	46	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



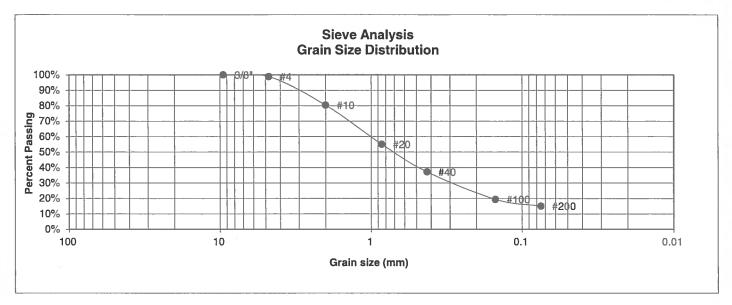
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit 15 Liquid Limit 33 Plastic Index 18
3/8"	100.0%	
4	99.2%	<u>Swell</u>
10	88.8%	Moisture at start
20	75.5%	Moisture at finish
40	65.2%	Moisture increase
100 200	49.5% 41.5%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	a	DATE: 11/8/17

FIGNO.: 3-41

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	47	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL

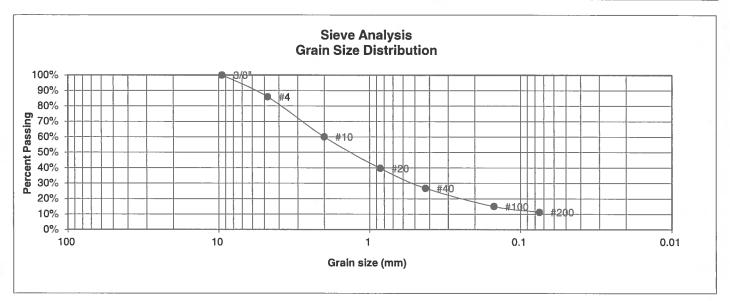


U.S.	Percent	Atterberg
Sieve #	<u>Finer</u>	<u>Limits</u>
3"		Plastic Limit
1 1/2"		Liquid Limit
3/4"		Plastic Index
1/2"		
3/8"	100.0%	
4	98.9%	<u>Swell</u>
10	80.4%	Moisture at start 9.0%
20	55.1%	Moisture at finish 20.0%
40	37.2%	Moisture increase 11.0%
100	19.4%	Initial dry density (pcf) 99
200	15.0%	Swell (psf) 30



DRAWN:	DATE:	CHECKED:	DATE: 4/8/17

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	48	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

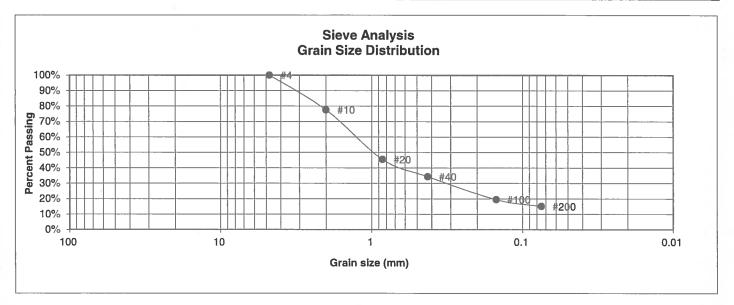


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8" 4	100.0% 85.9%	Swell
10	60.0%	Moisture at start
20 40	39.7% 26.8%	Moisture at finish Moisture increase
100 200	15.0% 11.3%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	n	DATE: 11/8/17

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	49	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL

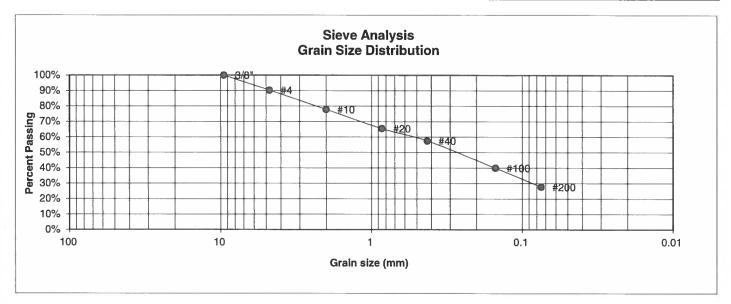


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	100.0%	<u>Swell</u>
10	77.6%	Moisture at start
20	45.5%	Moisture at finish
40	34.3%	Moisture increase
100	19.5%	Initial dry density (pcf)
200	15.1%	Swell (psf)



	LABO RESU	RATORY T LTS	EST	
DRAWN:	DATE:	CHECKED:	h	DATE:

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	50	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL

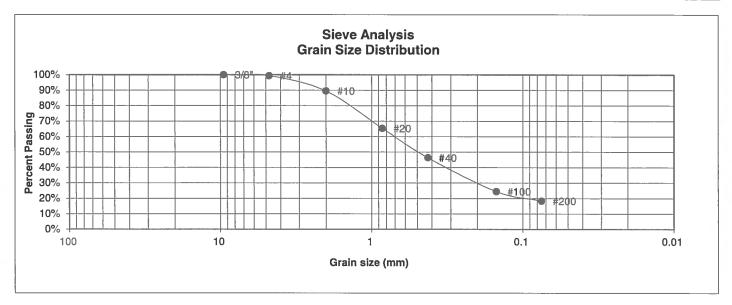


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8"	100.0%	
4	90.3%	<u>Swell</u>
10	77.8%	Moisture at start
20	65.4%	Moisture at finish
40	57.5%	Moisture increase
100 200	40.0% 27.7%	Initial dry density (pcf) Swell (psf)



	LABOI RESUI	RATORY 1 LTS	ΓEST		
DRAWN:	DATE:	CHECKED:	n	DATE: 1/8/17	

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	51	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL



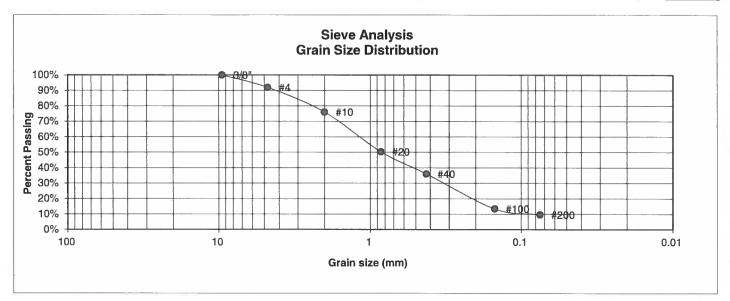
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
4 10 20 40 100 200	99.4% 89.5% 65.4% 46.3% 24.5%	Swell Moisture at start Moisture at finish Moisture increase Initial dry density (pcf) Swell (psf)



	LABO RESU	RATORY TEST ILTS
DRAWN:	DATE:	CHECKED: DATE:

FIG NO.: 13-46

UNIFIED CLASSIFICATION	SM-SW	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	52	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

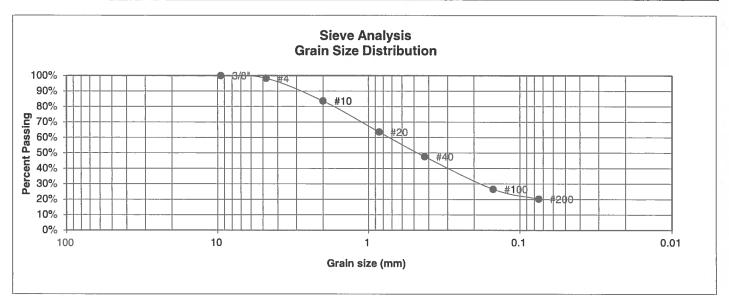


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	91.9%	<u>Swell</u>
10	76.1%	Moisture at start
20	50.3%	Moisture at finish
40	36.0%	Moisture increase
100	13.4%	Initial dry density (pcf)
200	9.7%	Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17	

UNIFIED CLASSIFICATION	SM	CLIENT	TECH CONTRACTORS
SOIL TYPE #	3	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	53	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL

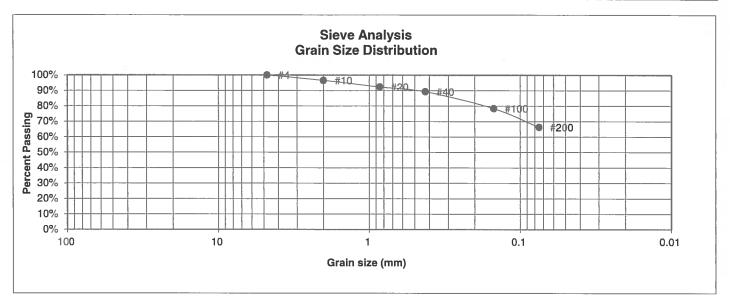


U.S <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u> 100.0%	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4 10	98.2% 83.6%	Swell Moisture at start
20 40 100 200	63.5% 47.6% 26.6% 20.2%	Moisture at finish Moisture increase Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS					
DRAWN:	DATE:	CHECKED:	n 11/8/17		

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	6	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

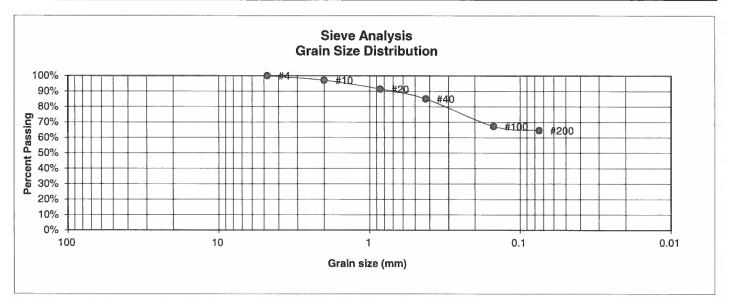


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit 13 Liquid Limit 21 Plastic Index 8
4	100.0%	Swell
10	96.6%	Moisture at start
20	92.3%	Moisture at finish
40	89.3%	Moisture increase
100	78.4%	Initial dry density (pcf)
200	66.3%	Swell (psf)



	LABOR/ RESUL	ATORY 1 IS	ΓEST		
DRAWN:	DATE:	CHECKED:	h	DATE:	

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	24	JOB NO.	171198
DEPTH (FT)	20	TEST BY	BL



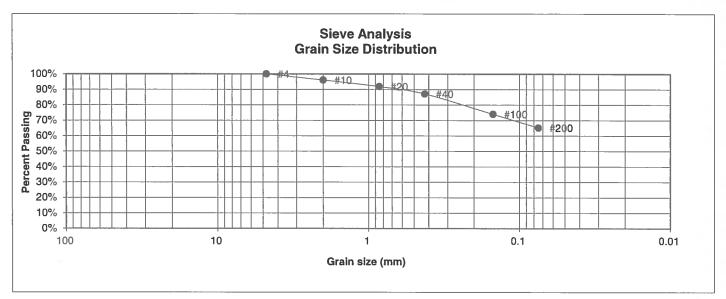
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	100.0%	<u>Swell</u>
10	97.1%	Moisture at start
20	91.4%	Moisture at finish
40	85.1%	Moisture increase
100	67.3%	Initial dry density (pcf)
200	64.7%	Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	U/9/17

> JOB NO.:

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	26	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

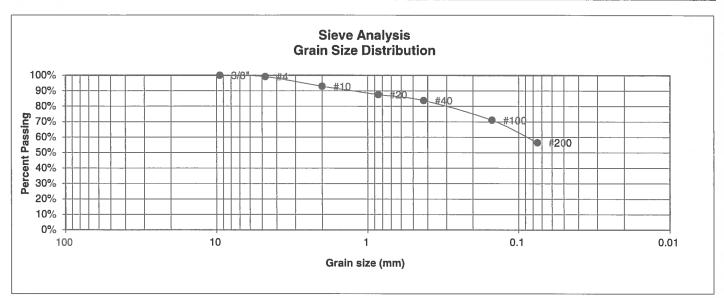


U.S. Sieve # 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
1/2" 3/8"		r lastic maex
4	100.0%	Swell
10	96.1%	Moisture at start 14.4%
20	91.9%	Moisture at finish 23.5%
40	87.2%	Moisture increase 9.1%
100	74.0%	Initial dry density (pcf) 100
200	65.2%	Swell (psf) 2060



	LABC RESU	DRATORY TEST JLTS
DRAWN:	DATE:	CHECKED: L 1/18/17

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	43	JOB NO.	171198
DEPTH (FT)	10	TEST BY	BL

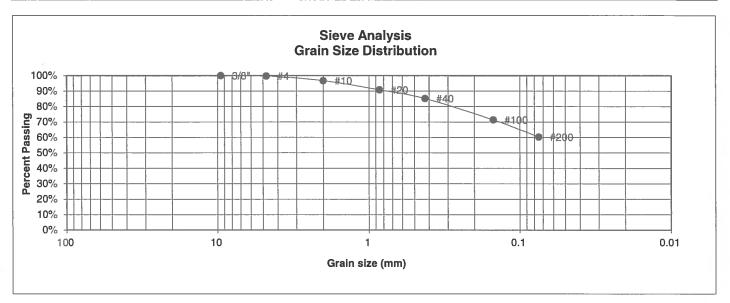


U.S.	Percent	Atterberg
Sieve #	<u>Finer</u>	<u>Limits</u>
3"		Plastic Limit
1 1/2"		Liquid Limit
3/4"		Plastic Index
1/2"		
3/8"	100.0%	
4	99.2%	<u>Swell</u>
10	92.9%	Moisture at start 12.1%
20	87.6%	Moisture at finish 21.6%
40	83.7%	Moisture increase 9.5%
100	71.0%	Initial dry density (pcf) 103
200	56.5%	Swell (psf) 1970



	LABOF RESUL	RATORY T	EST	
DRAWN:	DATE:	CHECKED:	h	DATE: 11/8/17

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	44	JOB NO.	171198
DEPTH (FT)	5	TEST BY	BL

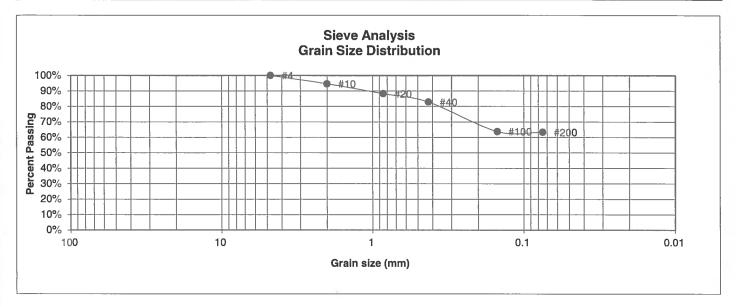


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
3/8" 4	100.0% 99.7%	Swell
10	96.7%	Moisture at start
20 40	90.8% 85.2%	Moisture at finish Moisture increase
100 200	71.3% 60.3%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED	h	DATE: 11/8/17

UNIFIED CLASSIFICATION	CL	CLIENT	TECH CONTRACTORS
SOIL TYPE #	4	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	54	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL

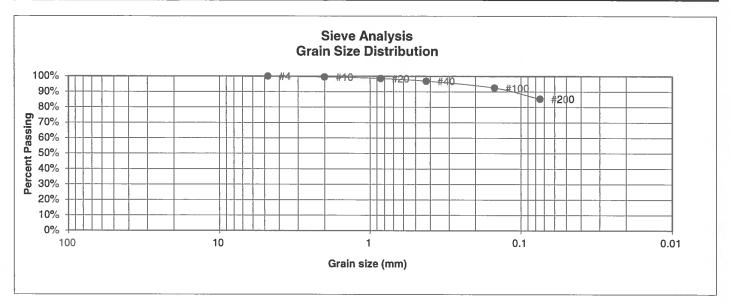


U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit 14 Liquid Limit 27 Plastic Index 13
4	100.0% 94.6%	Swell Moisture at start
20 40	88.2% 82.8%	Moisture at start Moisture at finish Moisture increase
100 200	63.8% 63.3%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	DATE: 117

UNIFIED CLASSIFICATION	ML	CLIENT	TECH CONTRACTORS
SOIL TYPE #	5	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	2	JOB NO.	171198
DEPTH (FT)	15	TEST BY	BL



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit Liquid Limit Plastic Index
4 10	100.0% 99.5%	Swell Moisture at start
20 40 100 200	98.6% 96.9% 92.5% 85.4%	Moisture at finish Moisture increase Initial dry density (pcf) Swell (psf)

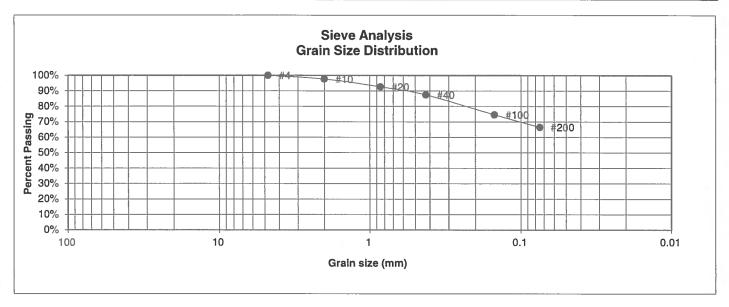


LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	DATE: 11/8/17	

26 40 14

> JOB NO.: 171198

UNIFIED CLASSIFICATION	ML	CLIENT	TECH CONTRACTORS
SOIL TYPE #	5	PROJECT	WINDINGWALK & STONEBRIDGE
TEST BORING #	17	JOB NO.	171198
DEPTH (FT)	2-3	TEST BY	BL



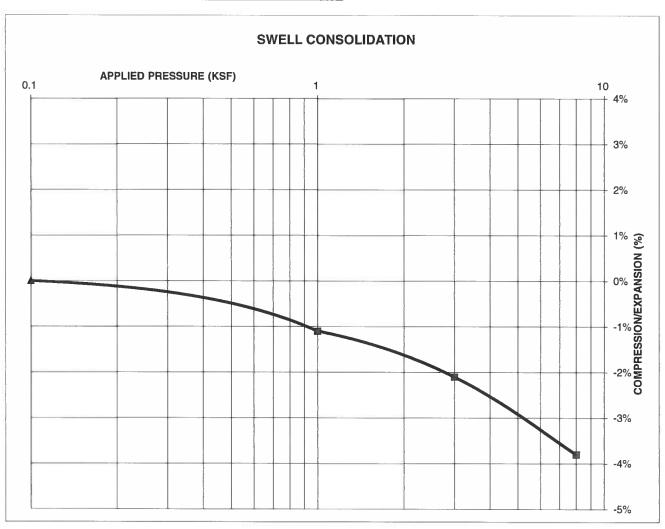
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4	100.0% 97.7%	<u>Swell</u> Moisture at start
20	92.7%	Moisture at finish
40	87.5%	Moisture increase
100	74.6%	Initial dry density (pcf)
200	66.5%	Swell (psf)



LABORATORY TEST RESULTS				
DRAWN:	DATE:	CHECKED:	h	DATE: 1/8/17

TEST BORING # 28 DEPTH(ft) 2-3
DESCRIPTION CL SOIL TYPE 2
NATURAL UNIT DRY WEIGHT (PCF) 101
NATURAL MOISTURE CONTENT 22.2%
SWELL/CONSOLIDATION (%) 0.0%

JOB NO. 171198
CLIENT TECH CONTRACTORS
PROJECT WINDINGWALK & STONEBRIDGE





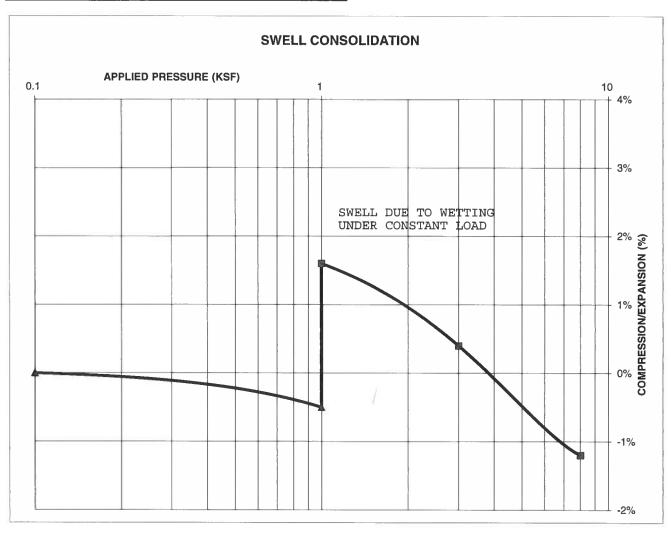
SWELL CONSOLIDATION TEST RESULTS

DRAWN: DATE: CHECKED: NATE: 1/8/17

JOB NO.: 171198

TEST BORING # 11 DEPTH(ft) 20
DESCRIPTION SC SOIL TYPE 3
NATURAL UNIT DRY WEIGHT (PCF) 112
NATURAL MOISTURE CONTENT 14.3%
SWELL/CONSOLIDATION (%) 2.1%

JOB NO. 171198
CLIENT TECH CONTRACTORS
PROJECT WINDINGWALK & STONEBRIDGE





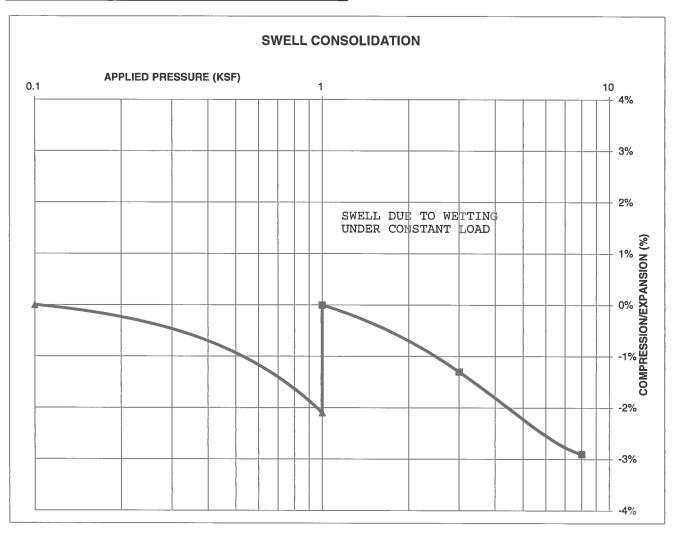
SWELL CONSOLIDATION
TEST RESULTS

DRAWN: DATE: CHECKED: (1/8/1

JOB NO.: 171198

TEST BORING # 23 DEPTH(ft) 15
DESCRIPTION SC SOIL TYPE 3
NATURAL UNIT DRY WEIGHT (PCF) 108
NATURAL MOISTURE CONTENT 20.1%
SWELL/CONSOLIDATION (%) 2.1%

JOB NO. 171198
CLIENT TECH CONTRACTORS
PROJECT WINDINGWALK & STONEBRIDGE





SWELL CONSOLIDATION
TEST RESULTS

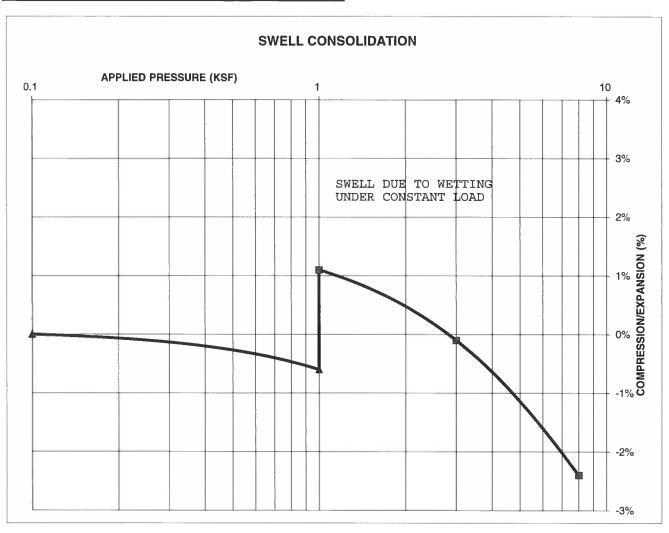
DRAWN: DATE: CHECKED: DATE:

JOB NO.: 171198

B-59

		A L PRINCE OF	
TEST BORING #	24	DEPTH(ft)	20
DESCRIPTION	CL	SOIL TYPE	4
NATURAL UNIT DRY	WEIGH	HT (PCF)	117
NATURAL MOISTUR	E CON	TENT	10.9%
SWELL/CONSOLIDA			1.7%

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SWELL CONSOLIDATION TEST RESULTS

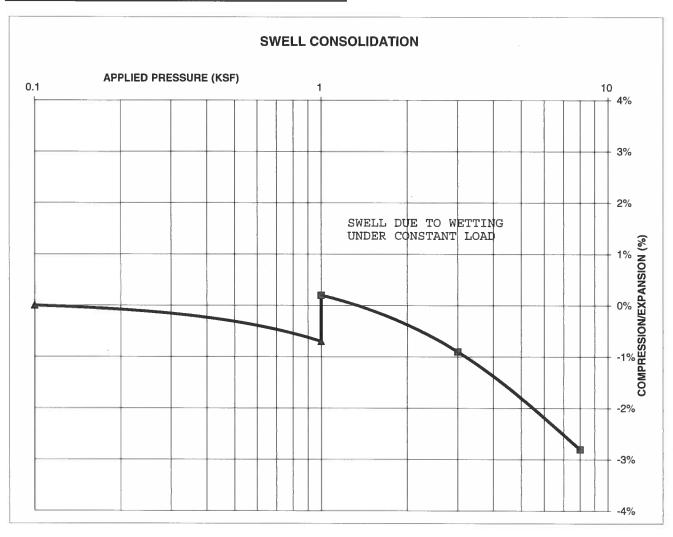
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JOB NO.: 171198

FIG NO.: 13-60

TEST BORING #	17	DEPTH(ft)	2-3	
DESCRIPTION	ML	SOIL TYPE	5	
NATURAL UNIT DRY NATURAL MOISTUR	WEIGH	HT (PCF)	111	
NATURAL MOISTUR	E CONT	ΓENT	15.6%	
SWELL/CONSOLIDA	TION (9	%)	0.9%	

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SWELL CONSOLIDATION
TEST RESULTS

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JOB NO.: 171198

C	LIENT	TECH CONTRACTORS	JOB NO.	171198
P	ROJECT	WINDINGWALK & STONEBRIDGE	DATE	9/6/2017
L	OCATION	WINDINGWALK & STONEBRIDGE	TEST BY	BL

BORING NUMBER	DEPTH, (ft)	SOIL TYPE NUMBER	UNIFIED CLASSIFICATION	WATER SOLUBLE SULFATE, (wt%)
TB-1	10	3	SM	<0.01
TB-39	10	1	SM	0.01
TB-42	20	3	SM	<0.01
TB-8	2-3	1	SM-SW	<0.01
TB-9	10	3	SM	<0.01
TB-11	20	3	SC	0.00
TB-6	10	4	CL	<0.01
TB-24	20	4	CL	<0.01
TB-25	10	2	CL	<0.01
TB-31	10	3	SM-SW	<0.01
TB-32	25	3	SM	<0.01
TB-35	2-3	1	SM	<0.01
TB-13	2-3	1	SC	<0.01
TB-45	5	3	SM-SW	0.00
TB-46	2-3	3	SC	<0.01
TB-51	5	3	SM	0.00
TB-12	10	3	SM	0.00
TB-29	5	3	SM	<0.01
TB-28	2-3	2	CL	<0.01
TB-19	5	1	SM-SW	0.00
TB-54	15	4	CL	0.00
TB-55	2-3	1	SM-SW	<0.01

QC BLANK PASS



LABORATORY TEST SULFATE RESULTS				
DRAWN:	DATE:	CHECKED:	DATE:	

JOB NO.: 171198

FIG NO.: 13-62

SUBSURFACE SOIL INVESTIGATION WINDINGWALK FILINGS 1 AND 2 AND STONEBRIDGE THE ENCLAVE FILINGS 4 AND 5 STAPLETON DRIVE AND EASTONVILLE ROAD EL PASO COUNTY, COLORADO

Prepared for:

Tech Contractors 3575 Kenyon Street, Suite 200 San Diego, California 92110

Attn: Mr. Raul Guzman

November 8, 2017 Revised December 21, 2017

Respectfully Submitted,

Ka am

ENTECH ENGINEERING, INC.

Kristen A. Andrew-Hoeser, P.G. Engineering Geologist

KAH/rm

Encl.

Entech Job No. 171198 AAprojects/2017/171198 SSI_rev