



ENTECH
ENGINEERING, INC.

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**WASTEWATER STUDY
FALCON ACRES
CURTIS ROAD AND DAVIS ROAD
PARCEL NO. 44040-00-014
EL PASO COUNTY, COLORADO**

Prepared for

Richard Elliott
812 East Monument Street
Colorado Springs, Colorado 80903

September 14, 2022

Respectfully Submitted,

ENTECH ENGINEERING, INC.

Logan L. Langford, P.G.
Geologist

Reviewed by:



Joseph C. Goode, Jr., P.E.
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LLL/jr

Encl.

Entech Job No. 221662
AAprojects/2022/221662 wws

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1.0 SUMMARY

Project Location

The project lies in the NE ¼ of Section 4, Township 14 South, Range 64 West of the 6th Principal Meridian in El Paso County, Colorado. The site is located at the southwest corner of Curtis Road and Davis Road, approximately 5 miles southeast of Falcon, Colorado.

Project Description

Total acreage involved in the project is approximately 49 acres. The proposed site development consists of 8 single-family rural residential lots. The development will utilize individual wells and on-site wastewater treatment systems.

Scope of Report

This report presents the results of our geologic investigation, treatment of engineering geologic hazard study, natural features, and wastewater study for individual sewage treatment systems.

Land Use and Engineering Geology

This site was found to be suitable for the proposed development. Areas were encountered where the geologic conditions will impose some constraints on development and land use. These include areas of artificial fill, potentially seasonal shallow groundwater areas, loose or collapsible soils, hydrocompaction, and possible expansive soils. Based on the proposed development plan, it appears that these areas will have some impact on the development. These conditions will be discussed in greater detail in the report.

In general, it is our opinion that the development can be achieved if the observed geologic conditions on site are either avoided or properly mitigated. All recommendations are subject to the limitations discussed in the report.

2.0 GENERAL SITE CONDITIONS AND PROJECT DESCRIPTION

The site is located in portions of the NE 1/4 Section 4, Township 14 South, Range 64 West of the 6th Principal Meridian in El Paso County, Colorado. The site is located at the southwest corner of Curtis Road and Davis Road, approximately 5 miles southeast of Falcon, Colorado. The approximate location of the site is as shown on the Vicinity Map, Figure 1.

The topography of the site is gently to moderately sloping. The site boundaries are indicated on the USGS Map, Figure 2. Previous land uses have included grazing and pasture land. Previously several trailers and out buildings exist along the northern portion of this site, a fire had occurred and the trailers burned. The out buildings still exist. The site contains primarily low grasses with some trees in the northern portion of the site around the previous trailer locations. Low areas with internal drainage exist on the site. No major drainages were observed on this site. Site Photographs are included in Appendix A. The locations and directions of the photographs are indicated on Figure 3.

Total acreage involved in the proposed development is approximately 49 acres. A total of 8 rural single-family lots are proposed. The area will be serviced by individual wells and sewage treatment systems. The Development Plan is shown on, Figure 3.

The site was previously investigated as part of a Soil, Geology and Wastewater Study was performed for a property east of the project site by Entech Engineering, Inc. revise date, May 13, 2002 (Reference 1), and a Soil, Geology, and Wastewater Study, Entech Job No. 42455, dated July 22, 2005 (Reference 2). Three percolation borings were previously performed on the site to determine general suitability of the site for construction and the use of on-site wastewater treatment systems. These reports were used in evaluating the site.

As part of this investigation, five test borings were drilled on July 26, 2022, and three tactile test pits were excavated on August 5, 2022. The Test Boring and Test Pit Logs are included in Appendix B, the Laboratory Testing Results are included in Appendix C, and a Summary of the Laboratory Testing Results is presented in Tables 1 and 2.

3.0 SCOPE OF THE REPORT

The scope of the report includes the evaluation of the site for individual on-site wastewater treatment systems in accordance with El Paso Land Development Code.

4.0 FIELD INVESTIGATION

Our field investigation on this site consisted of the preparation of a geologic map of any bedrock features and significant surficial deposits. The Soil Conservation Service (SCS) survey was also reviewed to evaluate the site. The position of mappable units within the subject property are shown on the Geologic Map. Our mapping procedures involved both field reconnaissance and measurements and air photo reconnaissance and interpretation. The same mapping procedures have also been utilized to produce the Engineering Geology Map which identified pertinent geologic conditions affecting development. The field reconnaissance was performed by personnel of Entech Engineering, Inc. in the previously studied report and on August 5, 2022.

In addition, three test pits and five test borings were performed on the site to determine the general suitability of the site for the use of individual wastewater treatment systems. The locations of the percolation tests are shown on the Development Plan, Test Boring and Test Pit Location Map, Figure 3. The profile hole logs are presented in Appendix B. Results of this testing will be discussed later in this report.

Laboratory testing was also performed on some of the soils to classify and determine the soils engineering characteristics. Laboratory tests include moisture content, ASTM D-2216 grain-size analysis, ASTM D-422, and Atterberg Limits, ASTM D-4318. Swell tests included both FHA Swell Tests and Swell/Consolidation Tests. Results of the laboratory testing are included in Appendix C. A Summary of Laboratory Test Results is presented in Tables 1 and 2.

5.0 SOIL, GEOLOGY AND ENGINEERING GEOLOGY

5.1 General Geology

Physiographically, the site lies in the western portion of the Great Plains Physiographic Province. Approximately 17 miles to the west is a major structural feature known as the Rampart Range

Fault. This fault marks the boundary between the Great Plains Physiographic Province and the Southern Rocky Mountain Province. The site exists within the southern edge of a large structural feature known as the Denver Basin. Bedrock in the area tends to be very gently dipping in a northerly direction. The rocks in the area of the site are sedimentary in nature, and typically Tertiary to Upper Cretaceous in age. The bedrock underlying the site consists of the Dawson Arkose Formation. Overlying this formation are unconsolidated deposits of man-made fill and eolian sand deposits of the Quaternary Age. The Eolian sands were deposited by wind in the form of low ridges or dunes. The site's stratigraphy will be discussed in more detail in Section 5.3.

5.2 Soil Conservation Survey

The Soil Conservation Service has mapped one soil type on the site (Figure 4) (Reference 3). In general, the soils consist of sandy loam and loamy sand. The soils are described as follows:

<u>Type</u>	<u>Description</u>
97	Truckton sandy loam, 3-9%

Complete descriptions of the soil types are presented in Appendix D. The soils have generally been described to have moderate to rapid permeabilities. The main limitation for these soils is frost action potential. Roads and streets may require special designs. Possible hazards with soil erosion are present on the site. The erosion potential can be controlled with vegetation. The majority of the soils have been described to have moderate erosion hazards.

5.3 Site Stratigraphy

The Corral Bluffs Quadrangle Geology Map showing the site is presented in Figure 5 (Reference 4). The Geology Map prepared for the site is presented in Figure 6. Two mappable units were identified on this site which are described as follows:

- **Qaf Artificial Fill of Quaternary Age:** These man-made fill deposits are associated with erosion berms observed on this site.
- **Qes Eolian Sand of Quaternary Age:** These are windblown fine grained sands that were deposited by the action of the prevailing winds from the west and northwest. They typically occur as large dune deposits or narrow ridges. The soils are

typically tan to brown and have a uniform gradation. The materials tend to have a high permeability and low density.

The bedrock underlying the site is the Dawson Formation of Tertiary to Cretaceous Age. The Dawson Formation typically consists of arkosic sandstone interbedded with fine sandstone, siltstone and claystone or shale. Typically, it is buff to light brown and light gray in color.

The soils listed above were mapped from the *Geologic Map of the Pueblo 1x2 Quadrangle, South-Central Colorado*, distributed by the USGS in 1978 (Reference 2), the *Geologic Map for the Corral Bluffs Quadrangle* by Paul E. Soister in 1968 (Reference 6, Figure 5) and site-specific mapping of the site. The profile holes drilled by Entech Engineering, Inc. were also used in evaluating the site and are included in Appendix B. The Geology Map prepared for the site is presented in Figure 6.

5.4 Soil Conditions

The soils encountered in the Test Borings and Test Pits can be grouped into three general soil and rock types. The soils were classified using the Unified Soil Classification System (USCS). The test pit soils were also classified using the USDA Textural Soil Classification.

Soil Type 1 is a silty sand and very clayey sand (SM, SC). The sand soils were encountered in all of the test borings, and in Test Pit Nos. 2 and 3. The sand was encountered at the existing surface and extending to depths of 3 feet to the termination of the test borings (20 feet bgs). These soils were encountered at loose to medium dense to loose states and at dry to moist conditions. Samples tested had 17 to 37 percent of the soil size particles passing the No. 200 Sieve. Atterberg Limits Testing resulted in a liquid limit of No-Value and a plastic index of Non-Plastic. Sulfate testing resulted in less than 0.01 percent soluble sulfate by weight, indicating negligible potential for below grade concrete degradation due to sulfate attack.

Soil Type 2 is a very sandy clay (CL). The clay soils were encountered in Test Boring Nos. 1 and 3, and in Test Pit Nos. 1 and 2. The clays were encountered at the existing ground surface to depths of 3 to 11 feet bgs in the test borings and at the existing ground surface in the test pits, and extended to depths of 6 to 14 feet bgs. The clay was encountered at firm to stiff consistencies and moist conditions. The samples tested had approximately 53 to 75 percent of the soil size particles passing the No. 200 sieve. Atterberg Limits Testing resulted in a liquid limit of 35 and a plastic index of 21. Swell/Consolidation Testing on a sample of the sandy clay resulted in a volume

change of 0.0%, which is in the low expansion range. Sulfate testing resulted in 0.01 percent soluble sulfate by weight, indicating negligible potential for below grade concrete degradation due to sulfate attack.

Soil Type 3 is sandy siltstone and very sandy claystone (ML, CL). The bedrock was encountered in Test Boring Nos. 1 and 3. The bedrock was encountered at depths ranging from 13 to 14 feet bgs and extended to the termination of the borings (20 feet bgs). The bedrock was encountered at hard consistencies and moist conditions. The bedrock had 57 to 96 percent of the soil sized particles passing the No. 200 sieve. Atterberg Limits Testing resulted in liquid limits of 60 to 30 and plastic indexes of 28 to 15. Swell/Consolidation Testing on a sample of the sandy siltstone resulted in a volume change of 1.4%, which is in the low expansion. Sulfate testing resulted in 0.00 to 0.01 percent soluble sulfate by weight, indicating negligible potential for below grade concrete degradation due to sulfate attack.

The Test Borings and Test Pit Logs are presented in Appendix B. Laboratory Test Results are presented in Appendix C. The Laboratory Test Results are summarized in Tables 1 and 2.

5.5 Groundwater

Groundwater or signs of seasonal groundwater were not encountered in the test borings or test pits (Appendix B). Areas of potentially seasonal shallow groundwater have been mapped in low-lying areas on the site and are indicated on the geology map, Figure 6. These areas are discussed in the following section. Fluctuation in groundwater conditions may occur due to variations in rainfall and other factors not readily apparent at this time. Isolated sand layers within the variable soil profile, sometimes only a few feet in thickness and width, can carry water in the subsurface. Additionally, perched water conditions can occur on this site where water can flow through permeable sands overlying less permeable bedrock. Builders and planners should be cognizant of the potential for the occurrence of such subsurface water features during construction on-site and deal with each individual problem as necessary at the time of construction.

Potentially Seasonal Shallow Groundwater Area – Constraint

In these areas, we would anticipate the potential for periodically high subsurface moisture conditions. The areas of internal drainage are mapped as having the potential for seasonal

shallow groundwater on a seasonal basis. These Areas appear to be associated with older blowout features and surrounded by older sand dune ridges. Should construction be necessary in portions of these areas the following precautions should be taken. No areas of the site have been mapped in any floodplain zones, according to Fema Map No. 08041CO785G.

Mitigation: In these locations, foundation in areas subject to severe frost heave potential should penetrate to a sufficient depth so as to discourage the formation of ice lenses beneath foundations. At this location and elevation, a foundation depth for frost protection of 30 inches is recommended. In areas where high subsurface moisture conditions are anticipated periodically, a subsurface perimeter drain will be necessary to help prevent the seepage of water into areas located below grade. Typical drain details are presented in Figure 8 through 10. Any grading in these areas should be done in a manner that directs surface flow around construction to avoid areas of ponded water. Areas of organic material will require removal before any filling is done. Specific recommendations should be made after additional investigation of each building site. The groundwater level may be at sufficient depth in some areas as to not to affect construction.

6.0 ON-SITE WASTEWATER TREATMENT

The site was evaluated for individual on-site wastewater treatment systems in accordance with El Paso Land Development Code. Three tactile test pits were performed on the property. The approximate locations of the test pits are indicated on Figure 3, on the Geology/Engineering Geology Map, Figure 6, and on the Septic Suitability Map, Figure 8. A table showing the results of the Tactile Test Pits is presented in Table 2. Test Pit Logs are included in Appendix B.

The Natural Resource Conservation Service (Reference 3), previously the Soil Conservation Service (Reference 4) has mapped the site with one soil descriptions. The Soil Survey Map (Reference 3) is presented in Figure 4, and the Soil Survey Descriptions are presented in Appendix D. The soils are described as having moderate to rapid percolation rates. The Natural Resource Conservation Service (NRCS) has rated the soil suitability with respect to septic tank absorption fields. The soils in the area have been described as not limited due to seepage, bottom layer, and filtering capacity. These areas are typically associated with loose sands. Soils encountered in the tactile test pits consisted of sandy clay, sandy clay loam and sandy loam. Refusal was encountered in Test Pit No. 2 at depths ranging from 6 feet bgs. The limiting layers

encountered in the test pits are the sandy clay loam (USDA Soil Type 3), and sandy clay (USDA Soil Type 4 to 4A) which corresponds to LTAR values ranging from 0.35 to 0.15 gallons per day per square foot. Designed systems will be required where USDA Soil Types greater than 3A, bedrock (refusal) or signs groundwater evidence are encountered at 6 feet bgs or shallower. Designed systems due to restrictive clay soils, shallow bedrock or shallow groundwater may be required on the site. Additional investigation of individual lots may identify areas where suitable for conventional systems could be used.

In summary, it is our opinion the site is suitable for individual on-site wastewater treatment systems (OWTS) and that contamination of surface and subsurface water resources should not occur provided the OWTS sites are evaluated and installed according to El Paso County and State Guidelines and properly maintained. Based on the testing performed as part of this investigation designed systems will likely be required for the majority of the lots. A Septic Suitability Map is presented in Figure 8. Areas where OWTS sites are not recommended are also indicated on Figure 8. Individual soil testing is required on each lot prior to construction. Absorption fields must be located a minimum of 100 feet from any well, including those on adjacent properties. Absorption fields must also be located a minimum of 50 feet from any drainages, floodplains or ponded areas and 25 feet from dry gulches.

7.0 CLOSURE

It is our opinion that the existing geologic engineering and geologic conditions will impose some minor constraints on development and construction of the site. The majority of these conditions can be avoided by construction. Others can be mitigated through proper engineering design and construction practices. The proposed development and use is consistent with anticipated geologic and engineering geologic conditions.

It should be pointed out that because of the nature of data obtained by random sampling of such variable and non-homogeneous materials as soil and rock, it is important that we be informed of any differences observed between surface and subsurface conditions encountered in construction and those assumed in the body of this report. Individual investigations for building sites and septic systems will be required prior to construction. Construction and design personnel should be made familiar with the contents of this report. Reporting such discrepancies to Entech Engineering, Inc. soon after they are discovered would be greatly appreciated and could possibly help avoid construction and development problems.

This report has been prepared for Richard Elliott for application to the proposed project in accordance with generally accepted geologic soil and engineering practices. No other warranty expressed or implied is made.

We trust that this report has provided you with all the information that you required. Should you require additional information, please do not hesitate to contact Entech Engineering, Inc.

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TABLES

TABLE 1
SUMMARY OF LABORATORY TEST RESULTS

CLIENT RICHARD ELLIOTT
PROJECT FALCON ACRES
JOB NO. 221662

SOIL TYPE	TEST BORING NO.	DEPTH (FT)	WATER (%)	DRY DENSITY (PCF)	PASSING NO. 200 SIEVE (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	SULFATE (WT %)	FHA SWELL (PSF)	SWELL/CONSOL (%)	UNIFIED CLASSIFICATION	SOIL DESCRIPTION
1	2	10			29.4	NV	NP	<0.01			SM	SAND, SILTY
1	4	2-3			36.9						SM	SAND, SILTY
1	5	5			17.2						SM	SAND, SILTY
2	3	5	11.2	110.2	52.9	35	21	0.01		0.0	CL	CLAY, VERY SANDY
3	1	15	33.1	86.6	95.5	60	28	0.01		1.4	MH	SILTSTONE, SANDY
3	3	20			57.3	30	15	0.00			CL	CLAYSTONE, VERY SANDY

TABLE 2

SUMMARY OF LABORATORY TEST RESULTS

CLIENT RICHARD ELLIOTT
 PROJECT FALCON ACRES
 JOB NO. 221662

SOIL TYPE	TEST PIT NO.	DEPTH (FT)	WATER (%)	DRY DENSITY (PCF)	PASSING NO. 200 SIEVE (%)	LIQUID LIMIT (%)	PLASTIC INDEX (%)	SULFATE (WT %)	FHA SWELL (PSF)	SWELL/CONSOL (%)	UNIFIED CLASSIFICATION	SOIL DESCRIPTION
1	TP-3	4			27.0						SM	SAND, SILTY
1	TP-2	2.5			49.1						SC	SAND, VERY CLAYEY
2	TP-1	1			75.1						CL	CLAY, SANDY

Table 3: Summary of Waste Water Treatment Conditions

Test Pit No.	Depth to Refusal (ft.)	LIMITING LAYER	Engineered Design Required (Y/N)
1	>8	4A	Y
2	>8	4A	Y
3	>8	3	N

FIGURES



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VICINITY MAP
FALCON ACRES
CURTIS ROAD AND DAVIS ROAD
EL PASO COUNTY, CO.
FOR: RICHARD ELLIOTT

DRAWN:
JHR

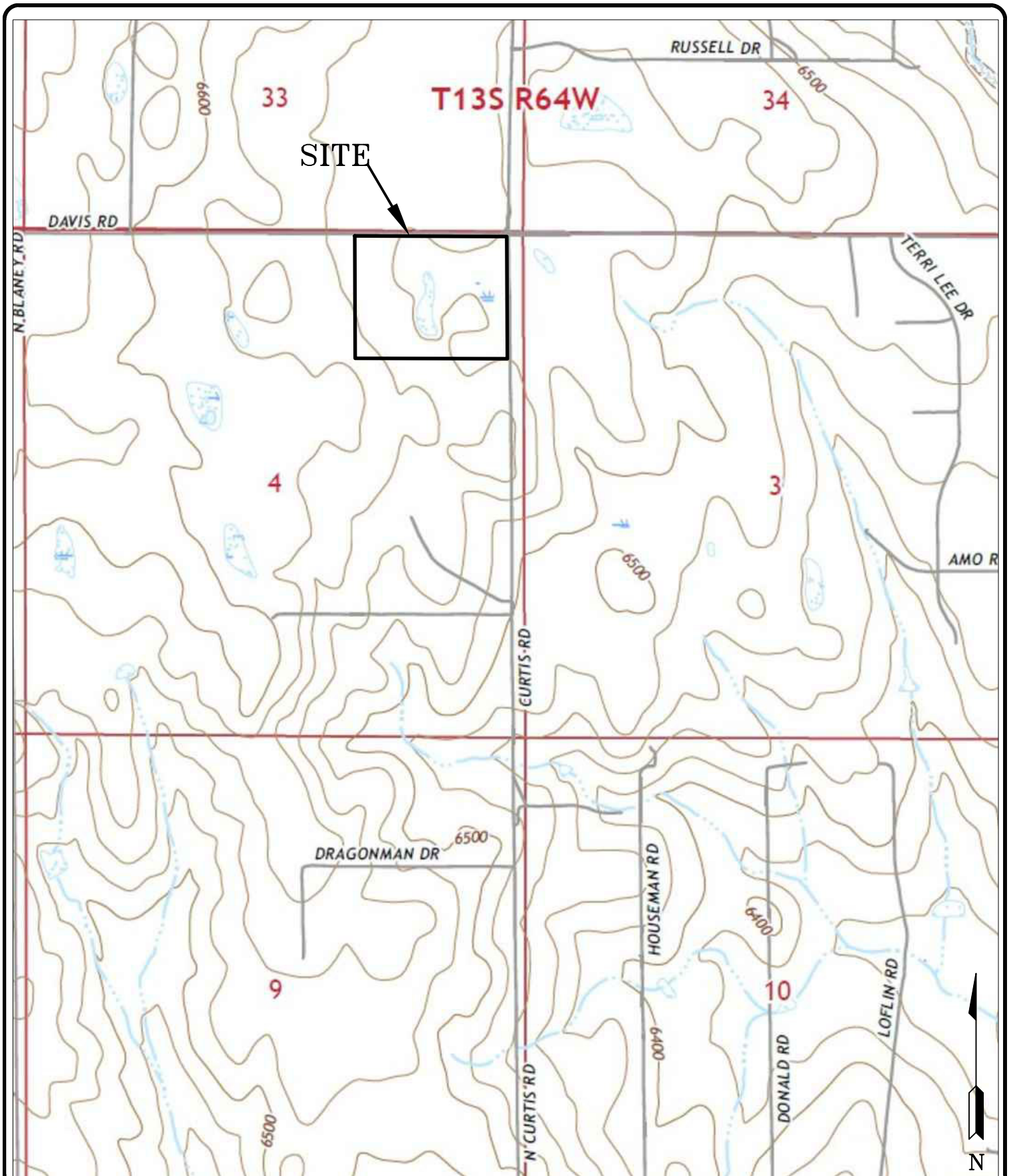
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8/10/22

CHECKED:
LLL

DATE:

JOB NO.:
221662

FIG NO.:
1



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USGS MAP
FALCON ACRES
CURTIS ROAD AND DAVIS ROAD
EL PASO COUNTY, CO.
FOR: RICHARD ELLIOTT

DRAWN:
JHR

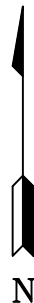
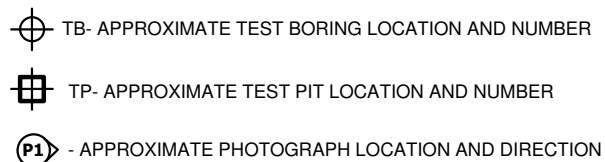
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8/10/22

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LLL

DATE:

JOB NO.:
221662

FIG NO.:
2



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LLL

DATE:
8/10/22

CHECKED:

DATE: _____

FIG NO.:
3



Curtis

N



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SOIL SURVEY MAP
FALCON ACRES
CURTIS ROAD AND DAVIS ROAD
EL PASO COUNTY, CO.
FOR: RICHARD ELLIOTT

DRAWN:
JHR

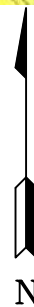
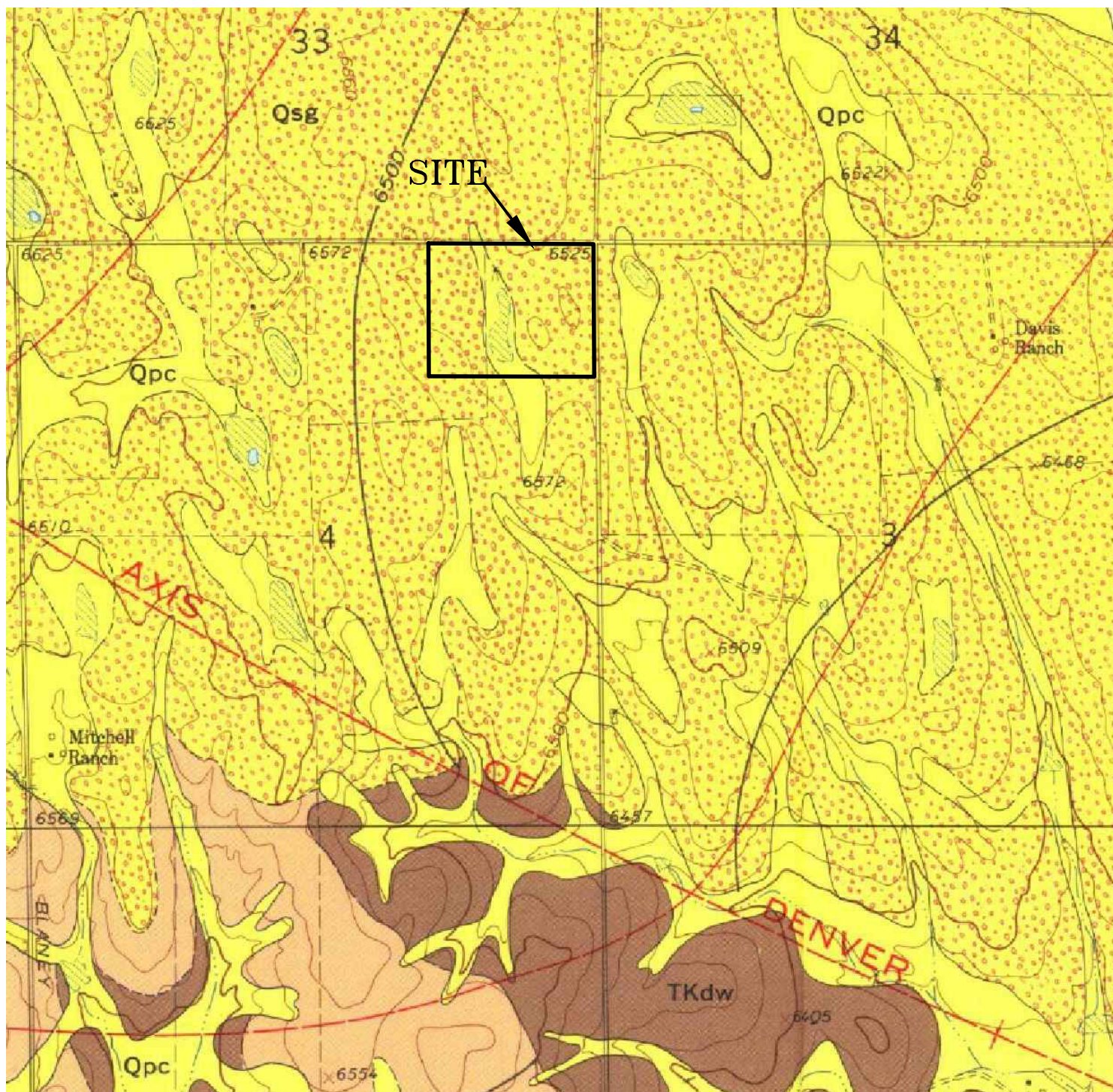
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DATE:

JOB NO.:
221662

FIG NO.:
4



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CORRAL BLUFFS QUADRANGLE GEOLOGIC MAP
FALCON ACRES
NORTH CURTIS ROAD AND DAVIS ROAD
EL PASO COUNTY, CO.
FOR: RICHARD ELLIOTT

DRAWN:
JHR

DATE:
8/10/22

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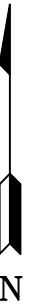
JOB NO.:
221662

FIG NO.:
5



FIG NO.:
6

SITE



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FEMA FLOODPLAIN MAP
FALCON ACRES
CURTIS ROAD AND DAVIS ROAD
EL PASO COUNTY, CO.
FOR: RICHARD ELLIOTT

DRAWN:
LLL

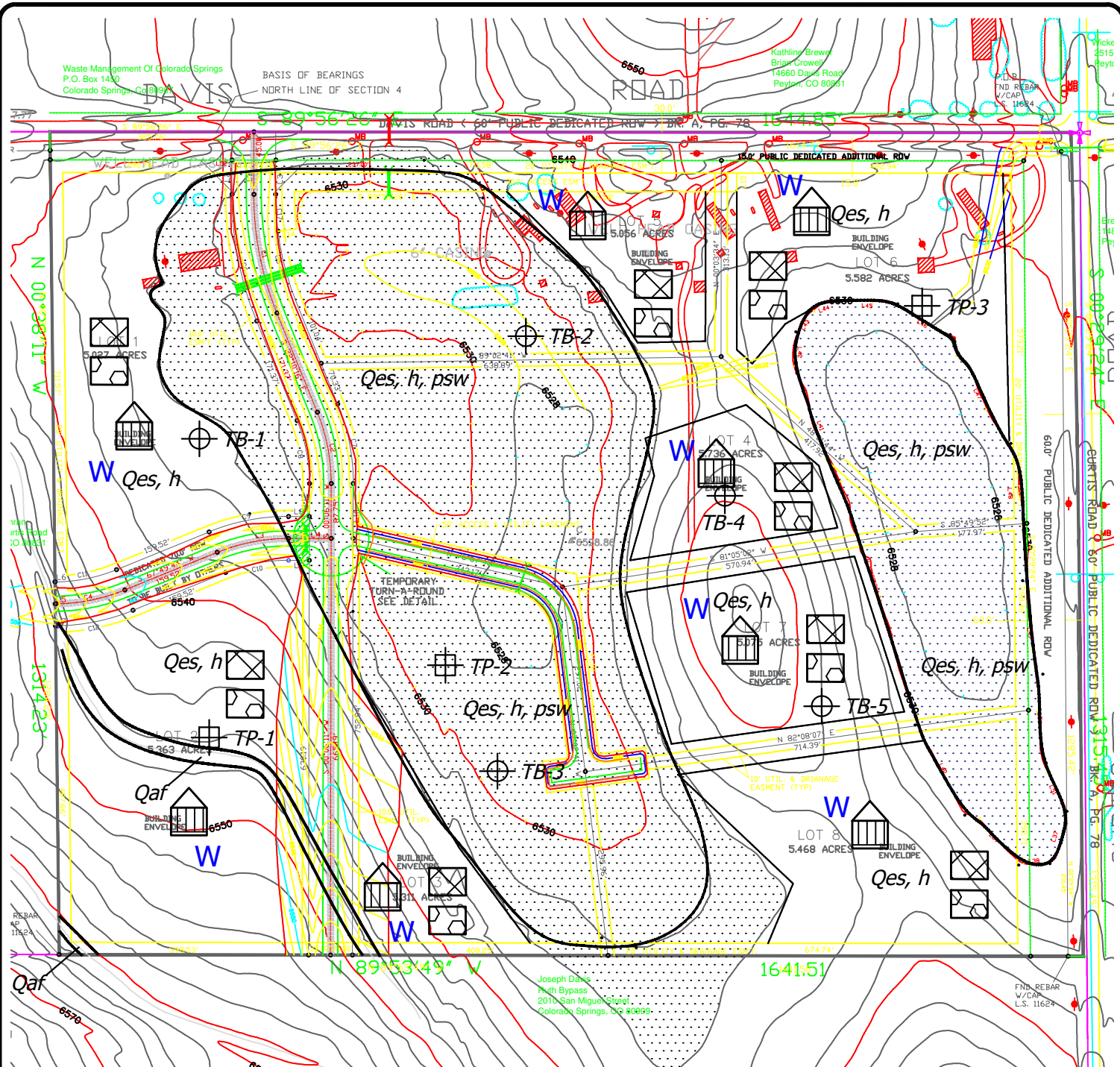
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JOB NO.:
221662

FIG NO.:
7



LEGEND:



- POSSIBLE OWTS LOCATIONS



- POSSIBLE OWTS ALTERNATE LOCATIONS



- POSSIBLE HOUSE LOCATIONS

W

*- WATER WELLS MUST BE A MINIMUM OF 100 FT FROM OWTS ABSORPTION FIELDS



- AREAS WHERE OWTS ARE NOT RECOMMENDED

N



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SEPTIC SUITABILITY MAP
FALCON ACRES
CURTIS ROAD AND DAVIS ROAD
EL PASO COUNTY, CO.
FOR: RICHARD ELLIOTT

DRAWN:
LLL

DATE:
8/10/22

CHECKED:

DATE:

JOB NO.:
221662

FIG NO.:
8

APPENDIX A: Site Photographs



**Looking west from the
central portion of the
site.**

August 5, 2022



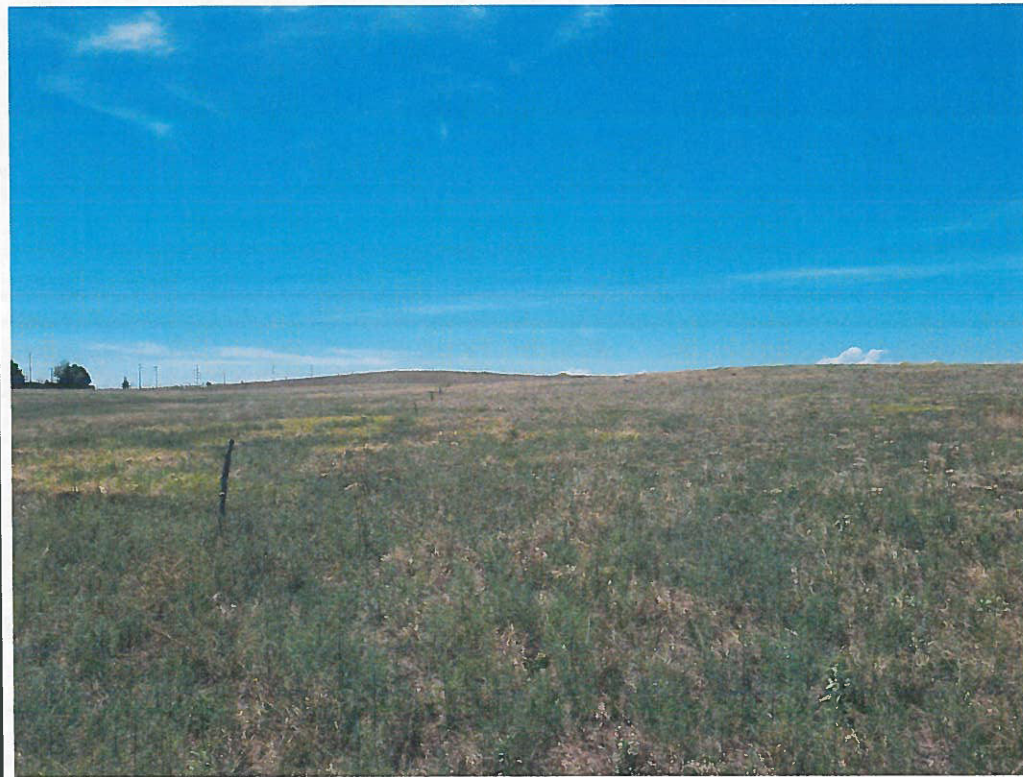
**Looking north from the
southwest portion of
the site.**

August 5, 2022



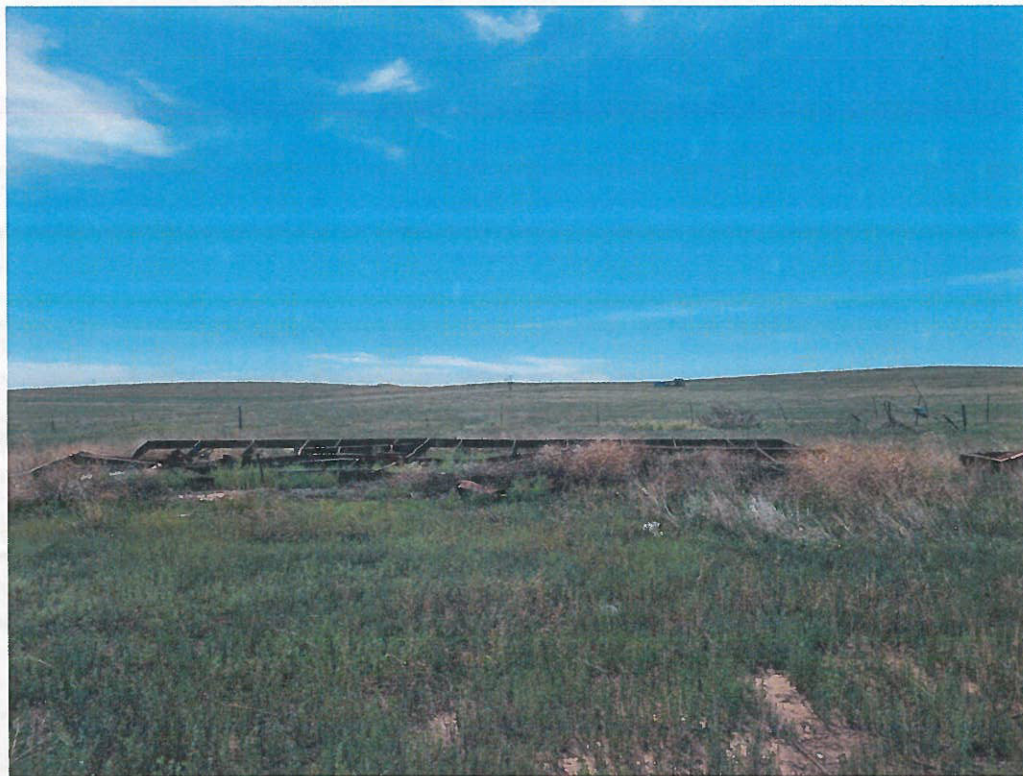
**Looking east from the
southwest corner of
the site.**

August 5, 2022



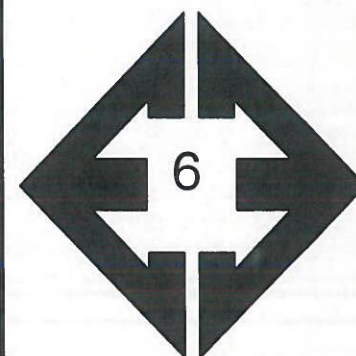
**Looking south from
the southwest site
corner**

August 5, 2022



**Looking south from
the northwest portion
of the site.**

August 5, 2022



**Looking east from the
northwest portion of
the site.**

August 5, 2022

APPENDIX B: Test Boring and Test Pit Logs

TEST BORING NO. 1
 DATE DRILLED 7/26/2022
 Job # 221662

TEST BORING NO. 2
 DATE DRILLED 7/26/2022
 CLIENT RICHARD ELLIOTT
 LOCATION FALCON ACRES

REMARKS

DRY TO 19', 7/28/22

SAND, SILTY, FINE TO COARSE
 GRAINED, BROWN TO TAN,
 MEDIUM DENSE TO LOOSE,
 MOIST

CLAY, SANDY, TAN

SILTSTONE, SANDY, TAN,
 HARD, MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
5			19	9.1	1
5			8	6.0	1
10			25	15.4	1
10					2
15			50	27.6	3
20			50	12.9	3
			9"		

REMARKS

DRY TO 19', 7/28/22

SAND, SILTY, FINE TO COARSE
 GRAINED, BROWN TO TAN,
 MEDIUM DENSE, MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
5			16	4.2	1
5			13	5.7	1
10			13	7.6	1
15			18	11.1	1
20			12	23.6	1



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 COLORADO SPRINGS, COLORADO 80907

TEST BORING LOG

DRAWN:

DATE:

CHECKED:

DATE:

JHR

8-5-22

JOB NO.:
 221662

FIG NO.:

B-1

TEST BORING NO. 3
 DATE DRILLED 7/26/2022
 Job # 221662

TEST BORING NO. 4
 DATE DRILLED 7/23/2022
 CLIENT RICHARD ELLIOTT
 LOCATION FALCON ACRES

REMARKS

DRY TO 19', 7/28/22

SAND, SILTY, FINE TO COARSE
 GRAINED, DARK BROWN, DENSE,
 MOIST

CLAY, SANDY, TAN, VERY
 STIFF, MOIST

SAND, SILTY, FINE TO COARSE
 GRAINED, TAN, MEDIUM DENSE,
 MOIST

CLAYSTONE, VERY SANDY,
 BROWN, HARD, MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
5			48	9.1	1
5			34	8.8	2
10			18	21.3	1
15			50 5"	20.8	3
20			50 4"	14.6	3

REMARKS

DRY TO 20', 7/28/22

SAND, SILTY, FINE TO COARSE
 GRAINED, TAN, MEDIUM DENSE
 TO DENSE, MOIST

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
5			23	4.2	1
5			21	3.1	1
10			17	5.1	1
15			19	6.8	1
20			38	6.4	1



ENTECH
ENGINEERING, INC.

505 ELKTON DRIVE
 COLORADO SPRINGS, COLORADO 80907

TEST BORING LOG

DRAWN:

DATE:

CHECKED:

DATE:

JHR

8-5-22

JOB NO.:
 221662

FIG NO.:

B-2

TEST BORING NO.	
DATE DRILLED	
CLIENT	RICHARD ELLIOTT
LOCATION	FALCON ACRES

REMARKS	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	REMARKS	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
DRY TO 20', 7/28/22 SAND, SILTY, FINE TO COARSE GRAINED, TAN, MEDIUM DENSE TO DENSE, MOIST	5			29	1.7	1		5					
				18	2.9	1							
	10			15	3.1	1		10					
				15	3.8	1		15					
	20			14	11.6	1		20					



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505 ELKTON DRIVE
COLORADO SPRINGS, COLORADO 80907

TEST BORING LOG

DRAWN:

DATE:

CHECKED:

DATE: _____

JOB NO.:
221662

FIG NO.:

B-3

TEST PIT NO. 1
 DATE EXCAVATED 8/5/2022
 Job # 221662

TEST PIT NO. 2
 DATE EXCAVATED 8/5/2022
 CLIENT RICHARD ELLIOT
 LOCATION FALCON ACRES

REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type	REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
topsoil, sandy clay loam, brown, moist	1						Refusal @ 6-feet	1					
	2							2					
sandy clay, fine to medium grained, brown, moist	3			ma		4A	sandy clay, fine to medium grained, dark brown, moist	3			ma		4A
	4							4					
	5			ma		4A	sandy clay, fine to medium grained, dark brown, moist	5			bl	s	4
	6							6					
sandy loam, fine to coarse grained, brown, moist	7			ma		2A		7					
	8							8					
	9							9					
	10							10					

Soil Structure Shape
 granular - gr
 platy - pl
 blocky - bl
 prismatic - pr
 single grain - sg
 massive - ma

Soil Structure Grade
 weak - w
 moderate - m
 strong - s
 loose - l



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505 ELKTON DRIVE
 COLORADO SPRINGS, COLORADO 80907

TEST PIT LOG

DRAWN:
jhr

DATE:
8/10/22

CHECKED:
JHR

DATE:
8-6-22

JOB NO.:
221662

FIG NO.:
B-4

TEST PIT NO. 3
 DATE EXCAVATED 8/5/2022
 Job # 221662

DATE EXCAVATED 8/5/2022
 CLIENT RICHARD ELLIOT
 LOCATION FALCON ACRES

REMARKS

REMARKS

topsoil, sandy clay loam,
 brown, moist

sandy clay loam, fine to
 coarse grained, dark brown,
 moist

Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type
1						1					
2			gr	m	3	2					
3						3					
4			gr	m	3	4					
5						5					
6			gr	m	3	6					
7						7					
8						8					
9						9					
10						10					

Soil Structure Shape
 granular - gr
 platy - pl
 blocky - bl
 prismatic - pr
 single grain - sg
 massive - ma

Soil Structure Grade
 weak - w
 moderate - m
 strong - s
 loose - l



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505 ELKTON DRIVE
 COLORADO SPRINGS, COLORADO 80907

TEST PIT LOG

DRAWN:
jhr

DATE:
8/10/22

CHECKED:
SAR

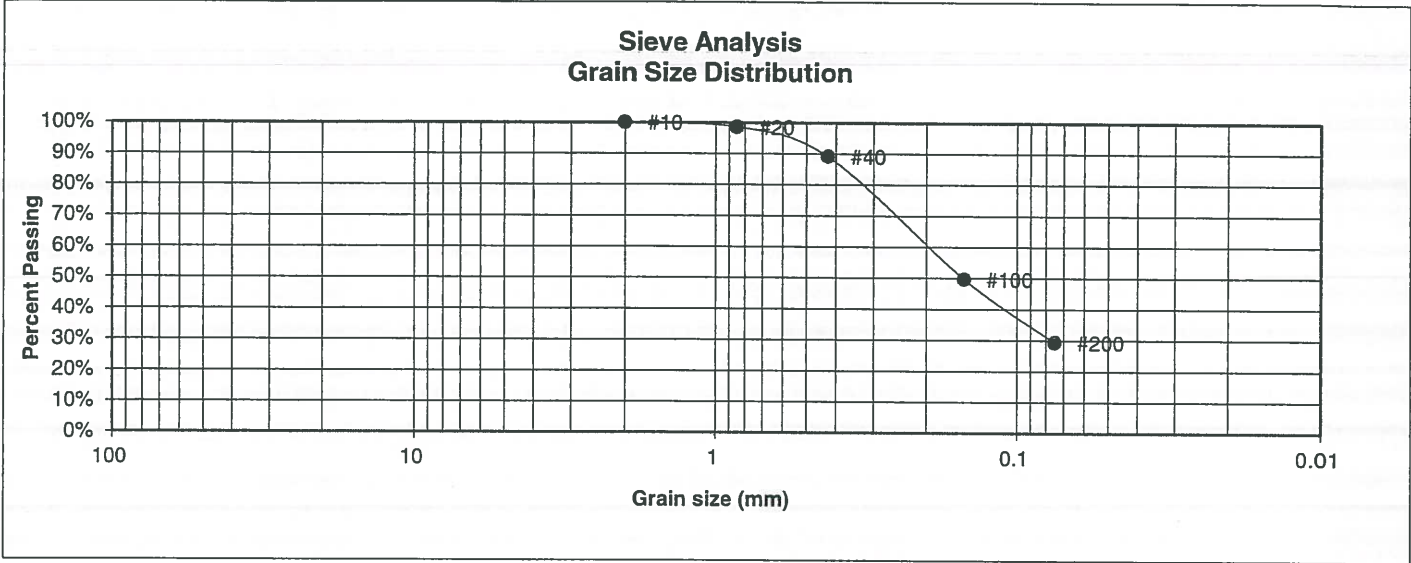
DATE:
8-5-22

JOB NO.:
221662

FIG NO.:
B-5

APPENDIX C: Laboratory Test Results

UNIFIED CLASSIFICATION	SM	CLIENT	RICHARD ELLIOTT
SOIL TYPE #	1	PROJECT	FALCON ACRES
TEST BORING #	2	JOB NO.	221662
DEPTH (FT)	10	TEST BY	BL



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	
10	100.0%
20	98.6%
40	89.1%
100	49.7%
200	29.4%

Atterberg	
Limits	
Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

Swell	
Moisture at start	
Moisture at finish	
Moisture increase	
Initial dry density (pcf)	
Swell (psf)	



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COLORADO SPRINGS, COLORADO 80907

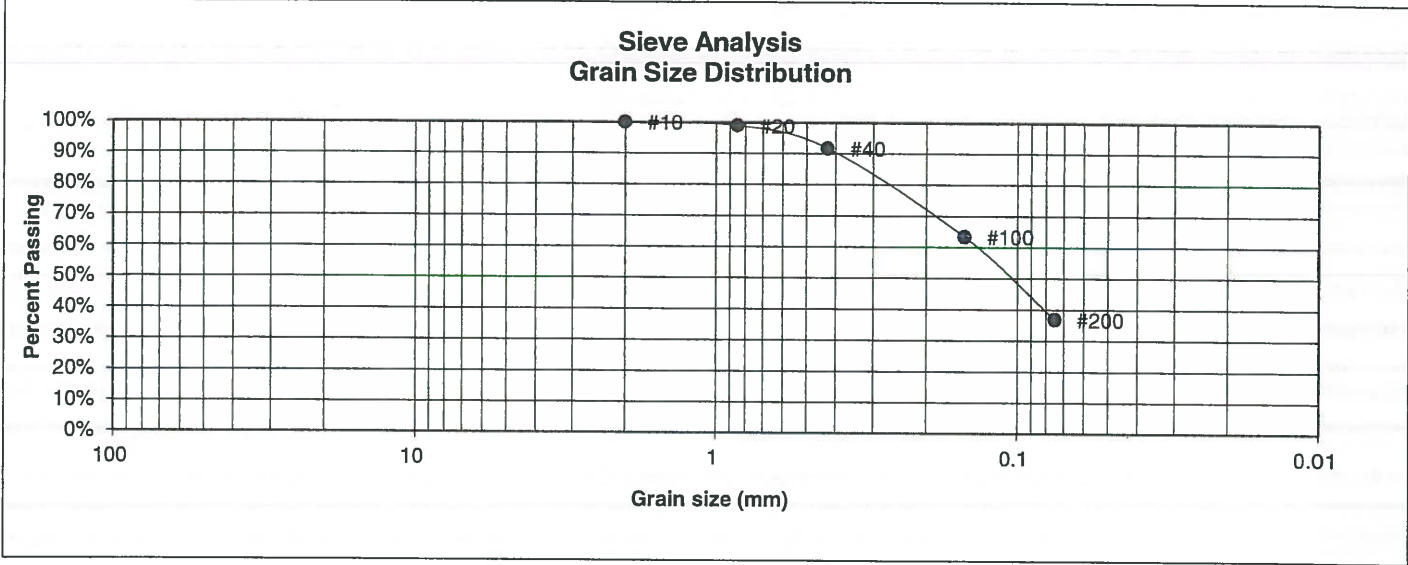
**LABORATORY TEST
RESULTS**

DRAWN:	DATE:	CHECKED: SHR	DATE: 8-5-22
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JOB NO.:
221662

FIG NO.:
C-1

<u>UNIFIED CLASSIFICATION</u>	SM	<u>CLIENT</u>	RICHARD ELLIOTT
<u>SOIL TYPE #</u>	1	<u>PROJECT</u>	FALCON ACRES
<u>TEST BORING #</u>	4	<u>JOB NO.</u>	221662
<u>DEPTH (FT)</u>	2-3	<u>TEST BY</u>	BL



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	
10	100.0%
20	98.9%
40	91.6%
100	63.4%
200	36.9%

Atterberg
Limits
Plastic Limit
Liquid Limit
Plastic Index

Swell
Moisture at start
Moisture at finish
Moisture increase
Initial dry density (pcf)
Swell (psf)



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505 ELKTON DRIVE
COLORADO SPRINGS, COLORADO 80907

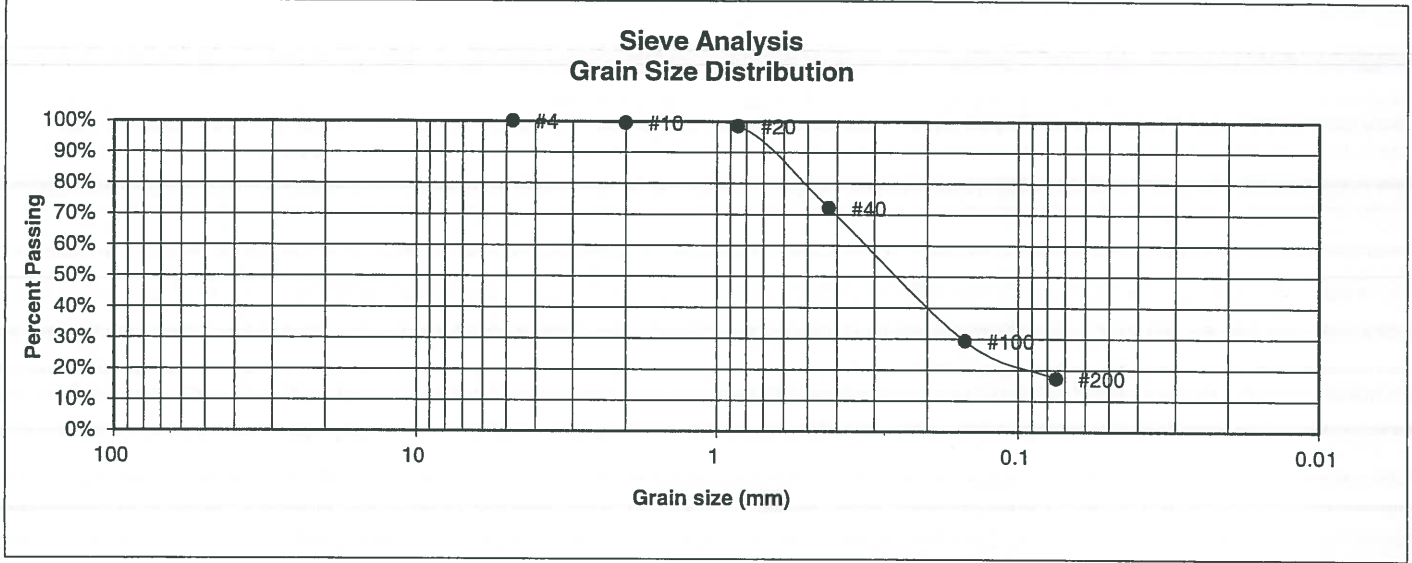
**LABORATORY TEST
RESULTS**

<u>DRAWN:</u>	<u>DATE:</u>	<u>CHECKED:</u> 8-5-22	<u>DATE:</u> JNA
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JOB NO.:
221662

FIG NO.:
C-2

UNIFIED CLASSIFICATION	SM	CLIENT	RICHARD ELLIOTT
SOIL TYPE #	1	PROJECT	FALCON ACRES
TEST BORING #	5	JOB NO.	221662
DEPTH (FT)	5	TEST BY	BL



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	99.5%
20	98.5%
40	72.1%
100	29.3%
200	17.2%

Atterberg
Limits
Plastic Limit
Liquid Limit
Plastic Index

Swell
Moisture at start
Moisture at finish
Moisture increase
Initial dry density (pcf)
Swell (psf)



**ENTECH
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505 ELKTON DRIVE
COLORADO SPRINGS, COLORADO 80907

LABORATORY TEST RESULTS

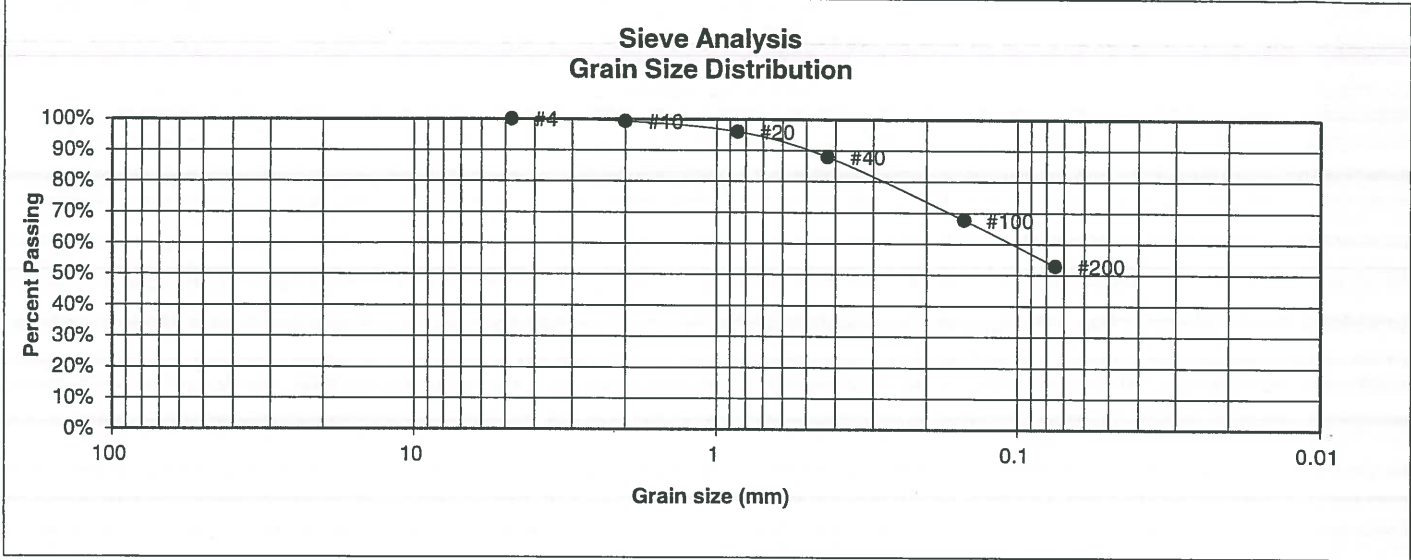
DRAWN:	DATE:	CHECKED: JHR	DATE: 8-5-22
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JOB NO.:
221662

FIG NO.:

C-3

<u>UNIFIED CLASSIFICATION</u>	CL	<u>CLIENT</u>	RICHARD ELLIOTT
<u>SOIL TYPE #</u>	2	<u>PROJECT</u>	FALCON ACRES
<u>TEST BORING #</u>	3	<u>JOB NO.</u>	221662
<u>DEPTH (FT)</u>	5	<u>TEST BY</u>	BL



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	99.3%
20	96.0%
40	87.9%
100	67.7%
200	52.9%

<u>Atterberg Limits</u>	
Plastic Limit	14
Liquid Limit	35
Plastic Index	21

<u>Swell</u>	
Moisture at start	
Moisture at finish	
Moisture increase	
Initial dry density (pcf)	
Swell (psf)	



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ENGINEERING, INC.**
505 ELKTON DRIVE
COLORADO SPRINGS, COLORADO 80907

**LABORATORY TEST
RESULTS**

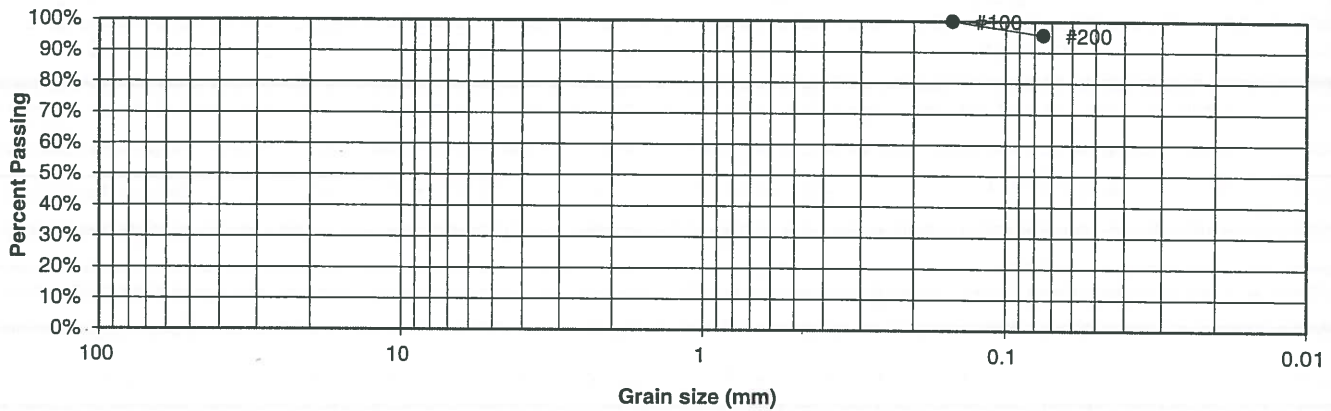
DRAWN:	DATE:	CHECKED:	DATE:
		<i>Snl</i>	<i>8-5-22</i>

JOB NO.:
221662

FIG NO.:
C-4

<u>UNIFIED CLASSIFICATION</u>	MH	<u>CLIENT</u>	RICHARD ELLIOTT
<u>SOIL TYPE #</u>	3	<u>PROJECT</u>	FALCON ACRES
<u>TEST BORING #</u>	1	<u>JOB NO.</u>	221662
<u>DEPTH (FT)</u>	15	<u>TEST BY</u>	BL

**Sieve Analysis
Grain Size Distribution**



<u>U.S. Sieve #</u>	<u>Percent Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	
10	
20	
40	
100	100.0%
200	95.5%

<u>Atterberg Limits</u>	
Plastic Limit	32
Liquid Limit	60
Plastic Index	28

<u>Swell</u>	
Moisture at start	
Moisture at finish	
Moisture increase	
Initial dry density (pcf)	
Swell (psf)	



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ENGINEERING, INC.**

505 ELKTON DRIVE
COLORADO SPRINGS, COLORADO 80907

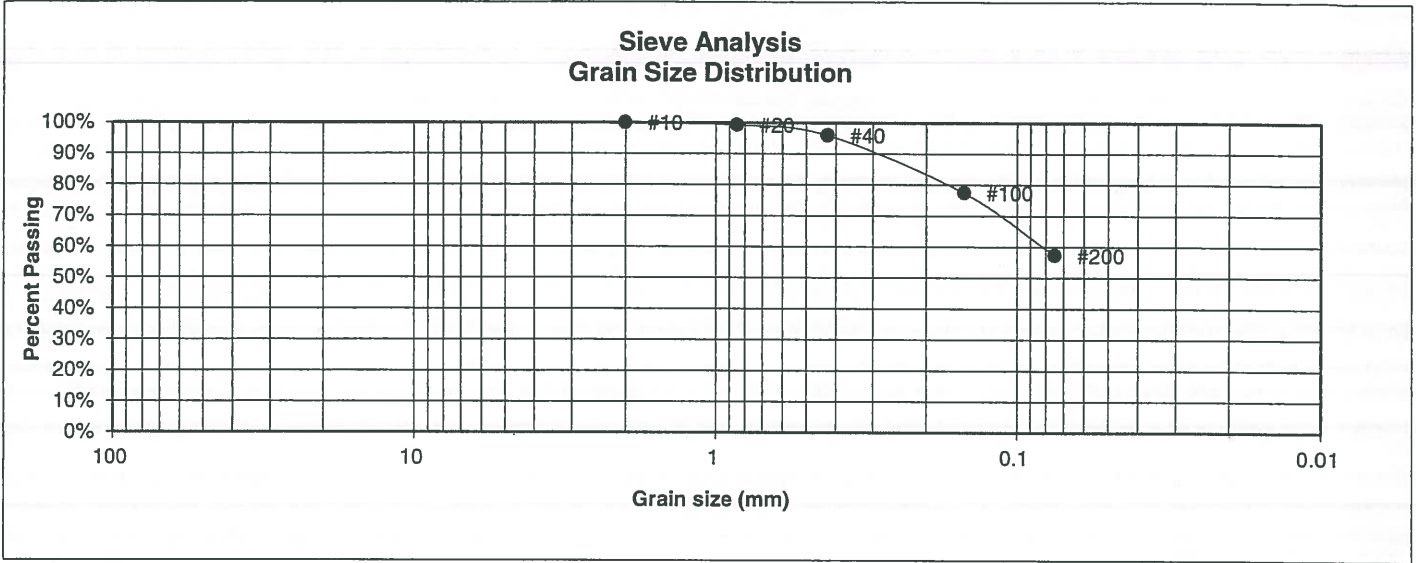
**LABORATORY TEST
RESULTS**

<u>DRAWN:</u>	<u>DATE:</u>	<u>CHECKED:</u> JHR	<u>DATE:</u> 8-5-22
---------------	--------------	------------------------	------------------------

JOB NO.:
221662

FIG NO.:
C-5

UNIFIED CLASSIFICATION	CL	CLIENT	RICHARD ELLIOTT
SOIL TYPE #	3	PROJECT	FALCON ACRES
TEST BORING #	3	JOB NO.	221662
DEPTH (FT)	20	TEST BY	BL



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	
10	100.0%
20	99.3%
40	95.9%
100	77.5%
200	57.3%

Atterberg Limits	
Plastic Limit	15
Liquid Limit	30
Plastic Index	15

Swell	
Moisture at start	
Moisture at finish	
Moisture increase	
Initial dry density (pcf)	
Swell (psf)	



**ENTECH
ENGINEERING, INC.**
505 ELKTON DRIVE
COLORADO SPRINGS, COLORADO 80907

**LABORATORY TEST
RESULTS**

DRAWN:	DATE:	CHECKED:	DATE:
		JHR	8-5-22

JOB NO.:
221662

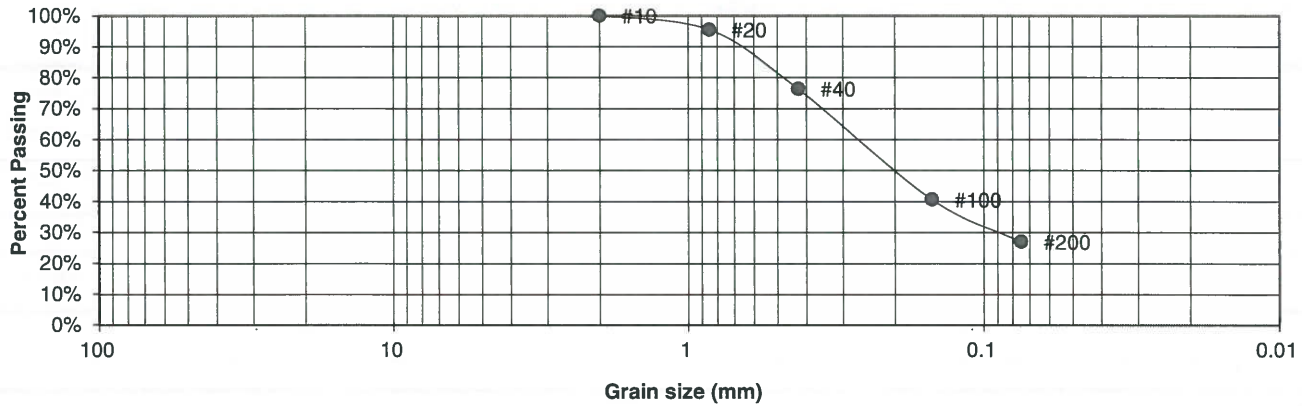
FIG NO.:
C-6

BORING NO. TP-3
 DEPTH(ft) 4
 CLIENT RICHARD ELLIOTT
 PROJECT FALCON ACRES

UNIFIED CLASSIFICATION
 AASHTO CLASSIFICATION

SM
 TEST BY BL
 JOB NO. 221662

Sieve Analysis Grain Size Distribution



U.S.
Sieve #

3"
1 1/2"
3/4"
1/2"
3/8"
4
10
20
40
100
200

Percent
Finer

100.0%
95.5%
76.3%
40.6%
27.0%

Atterberg
Limits

Plastic Limit
Liquid Limit
Plastic Index

Swell

Moisture at start
Moisture at finish
Moisture increase
Initial dry density (pcf)
Swell (psf)



**ENTECH
ENGINEERING, INC.**

505 ELKTON DRIVE
COLORADO SPRINGS, COLORADO 80907

LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:

DATE:

JHL

8-5-22

JOB NO.:
221662

FIG NO.:

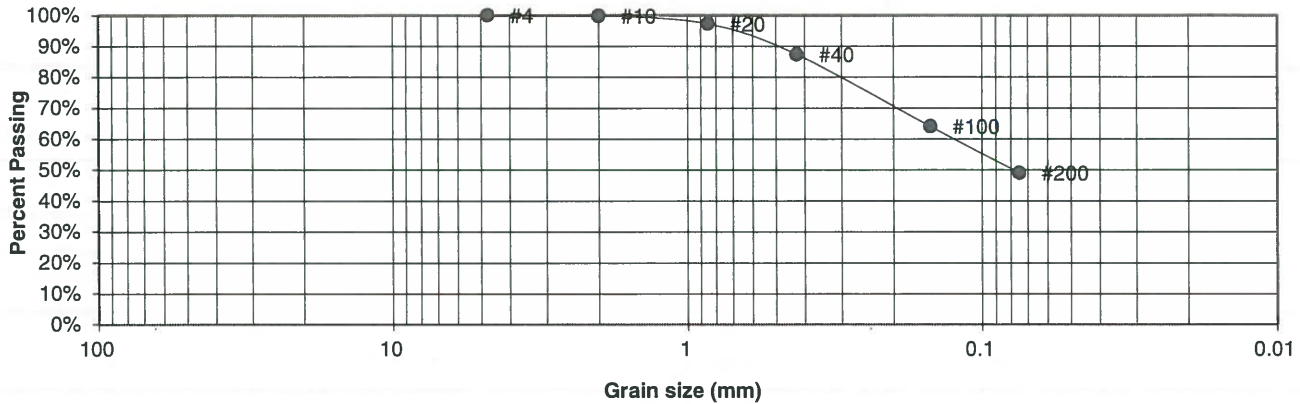
C-7

BORING NO. TP-2
 DEPTH(ft) 2.5
 CLIENT RICHARD ELLIOTT
 PROJECT FALCON ACRES

UNIFIED CLASSIFICATION SC
 AASHTO CLASSIFICATION

TEST BY BL
 JOB NO. 221662

Sieve Analysis Grain Size Distribution



U.S.
Sieve #

3"
1 1/2"
3/4"
1/2"
3/8"

4 100.0%
 10 99.9%
 20 97.3%
 40 87.4%
 100 64.1%
 200 49.1%

Percent
Finer

Atterberg
Limits
Plastic Limit
Liquid Limit
Plastic Index

Swell
Moisture at start
Moisture at finish
Moisture increase
Initial dry density (pcf)
Swell (psf)



**ENTECH
ENGINEERING, INC.**

505 ELKTON DRIVE
 COLORADO SPRINGS, COLORADO 80907

LABORATORY TEST RESULTS

DRAWN:

DATE:

CHECKED:

DATE:

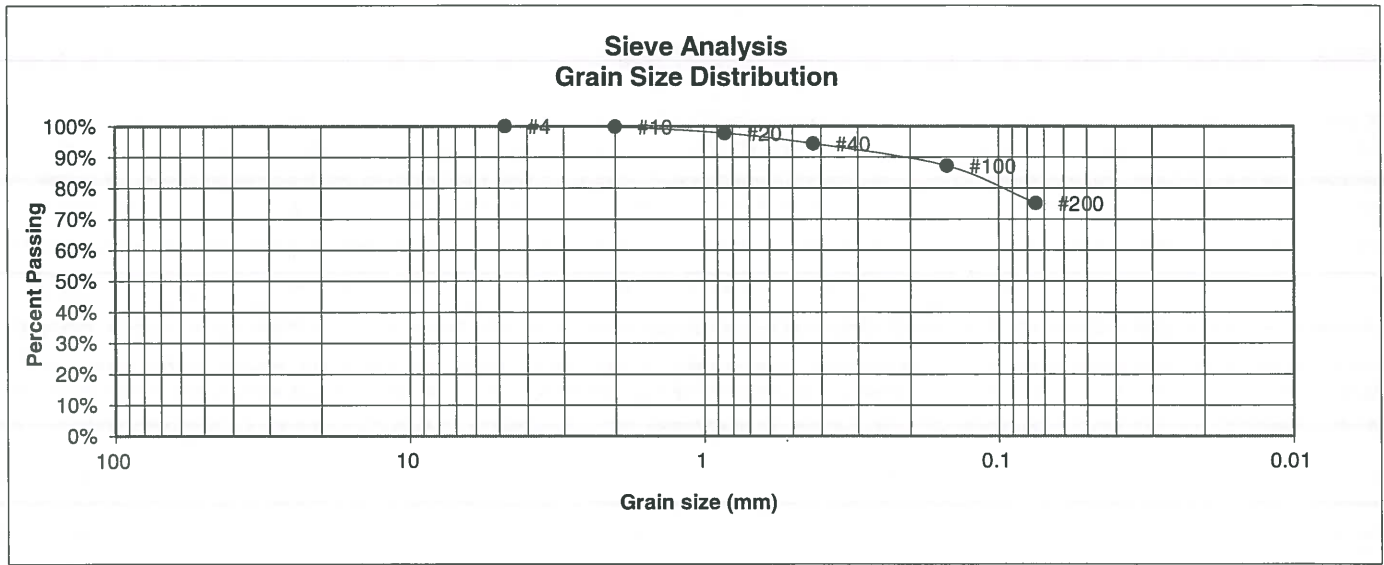
JHL

8-5-22

JOB NO.:
221662

FIG NO.:
C-8

BORING NO.	TP-1	UNIFIED CLASSIFICATION	CL	TEST BY	BL
DEPTH(ft)	5	AASHTO CLASSIFICATION		JOB NO.	221662
CLIENT	RICHARD ELLIOTT				
PROJECT	FALCON ACRES				



U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	99.8%
20	97.7%
40	94.3%
100	87.2%
200	75.1%

Atterberg

Limits

Plastic Limit

Liquid Limit

Plastic Index

Swell

Moisture at start

Moisture at finish

Moisture increase

Initial dry density (pcf)

Swell (psf)



ENTECH
ENGINEERING, INC.
 505 ELKTON DRIVE
 COLORADO SPRINGS, COLORADO 80907

LABORATORY TEST
 RESULTS

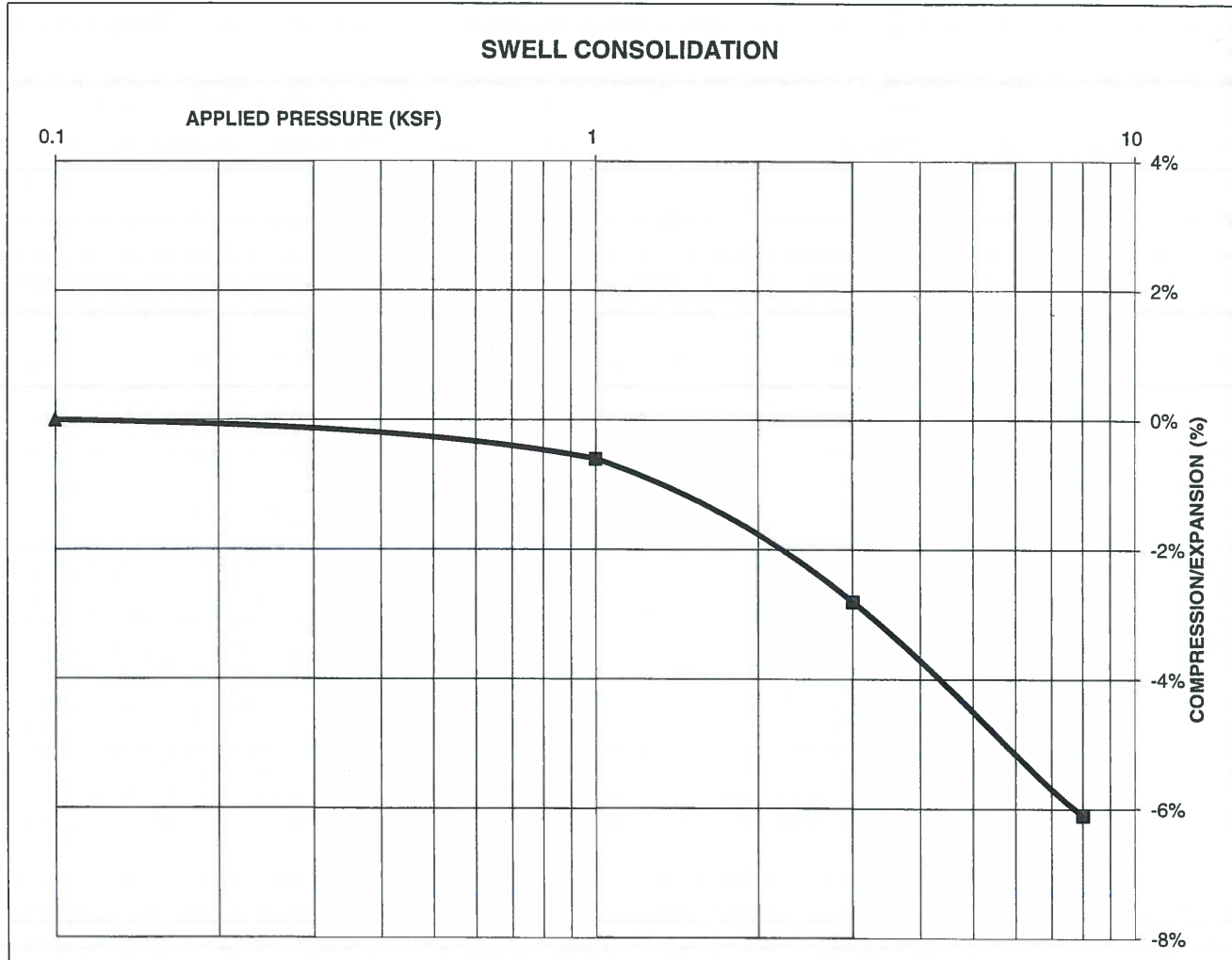
DRAWN:	DATE:	CHECKED:	DATE:
		JHK	8-5-22

JOB NO.:
 221662
 FIG NO.:
 C-9

CONSOLIDATION TEST RESULTS

TEST BORING #	3	DEPTH(ft)	5
DESCRIPTION	CL	SOIL TYPE	2
NATURAL UNIT DRY WEIGHT (PCF)	110		
NATURAL MOISTURE CONTENT	11.2%		
SWELL/CONSOLIDATION (%)	0.0%		

JOB NO. 221662
 CLIENT RICHARD ELLIOTT
 PROJECT FALCON ACRES



ENTECH
ENGINEERING, INC.

505 ELKTON DRIVE
 COLORADO SPRINGS, COLORADO 80907

SWELL CONSOLIDATION TEST RESULTS

DRAWN:

DATE:

CHECKED:

SHR

DATE:

8-5-22

JOB NO.:
 221662

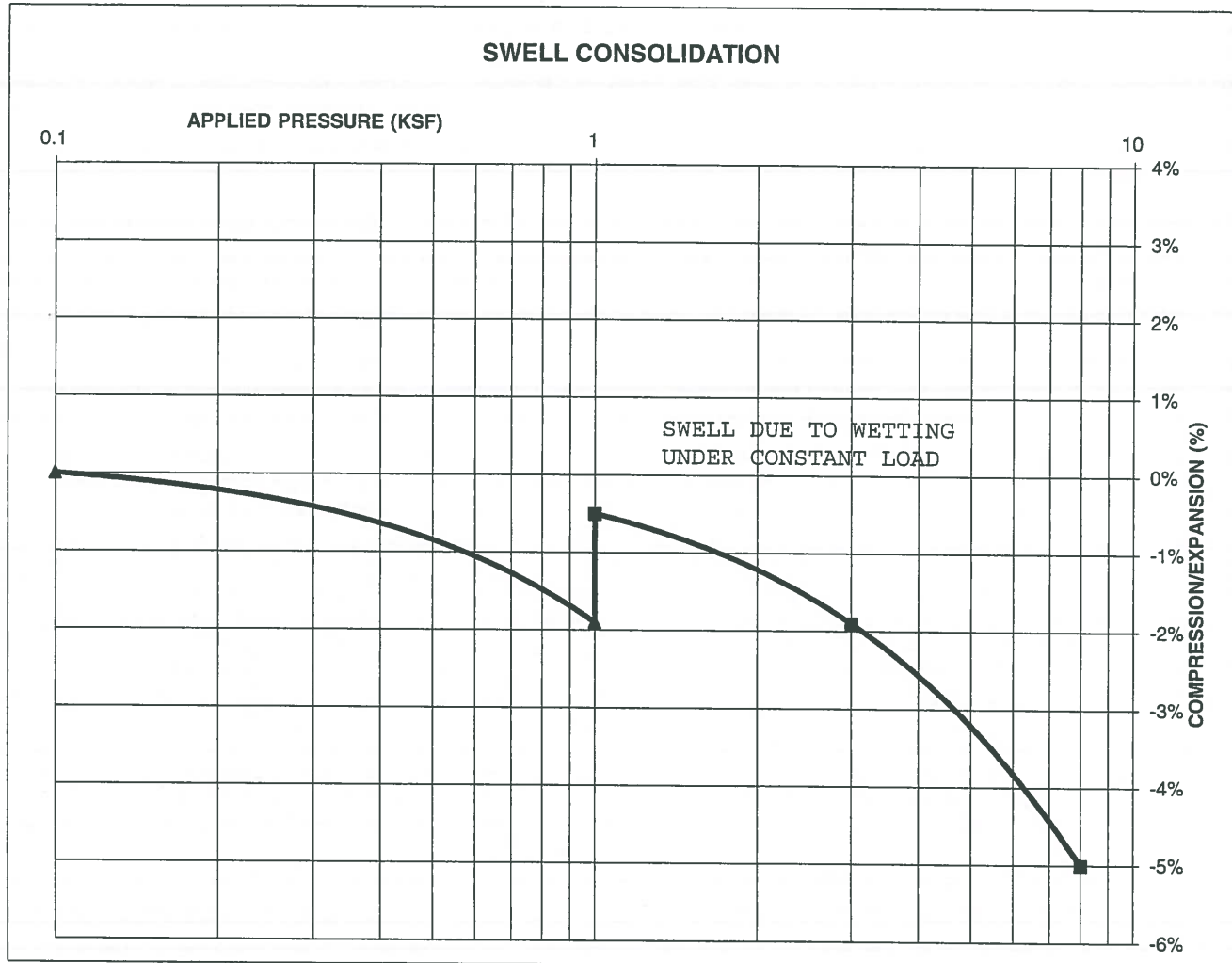
FIG NO.:
C-10

CONSOLIDATION TEST RESULTS

TEST BORING #	1	DEPTH(ft)	15
DESCRIPTION	MH	SOIL TYPE	3
NATURAL UNIT DRY WEIGHT (PCF)	87		
NATURAL MOISTURE CONTENT	33.1%		
SWELL/CONSOLIDATION (%)	1.4%		

JOB NO. 221662
 CLIENT RICHARD ELLIOTT
 PROJECT FALCON ACRES

SWELL CONSOLIDATION



ENTECH
ENGINEERING, INC.

505 ELKTON DRIVE
 COLORADO SPRINGS, COLORADO 80907

SWELL CONSOLIDATION TEST RESULTS

DRAWN:

DATE:

CHECKED:

DATE:

JHR

8-5-22

JOB NO.:
 221662

FIG NO.:
 C-11

CLIENT	<u>RICHARD ELLIOTT</u>	JOB NO.	<u>221662</u>
PROJECT	<u>FALCON ACRES</u>	DATE	<u>8/1/2022</u>
LOCATION	<u>FALCON ACRES</u>	TEST BY	<u>BL</u>

[illegible]

QC BLANK PASS



ENTECH
ENGINEERING, INC.
505 ELKTON DRIVE
COLORADO SPRINGS, COLORADO 80907

LABORATORY TEST SULFATE RESULTS

DRAWN:

DATE: _____

CHECKED:

DATE: _____

JOB NO.:
221662

FIG NO.:

C-12

APPENDIX D: Soil Survey Descriptions

El Paso County Area, Colorado

97—Truckton sandy loam, 3 to 9 percent slopes

Map Unit Setting

National map unit symbol: 2x0j2
Elevation: 5,300 to 6,850 feet
Mean annual precipitation: 14 to 19 inches
Mean annual air temperature: 48 to 52 degrees F
Frost-free period: 85 to 155 days
Farmland classification: Not prime farmland

Map Unit Composition

Truckton and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Truckton

Setting

Landform: Interfluves, hillslopes
Landform position (two-dimensional): Backslope
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Re-worked alluvium derived from arkose

Typical profile

A - 0 to 4 inches: sandy loam
Bt1 - 4 to 12 inches: sandy loam
Bt2 - 12 to 19 inches: sandy loam
C - 19 to 80 inches: sandy loam

Properties and qualities

Slope: 3 to 9 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 1 percent
Maximum salinity: Nonsaline (0.1 to 1.9 mmhos/cm)
Available water supply, 0 to 60 inches: Moderate (about 6.6 inches)

Interpretive groups

Land capability classification (irrigated): 6e
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: A
Ecological site: R049XB210CO - Sandy Foothill
Hydric soil rating: No

Custom Soil Resource Report

Minor Components

Blakeland

Percent of map unit: 8 percent

Landform: Interfluves, hillslopes

Landform position (two-dimensional): Summit, shoulder, backslope

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Convex, linear

Across-slope shape: Convex, linear

Ecological site: R049XB210CO - Sandy Foothill

Hydric soil rating: No

Bresser

Percent of map unit: 7 percent

Landform: Interfluves, low hills

Landform position (two-dimensional): Footslope, toeslope

Landform position (three-dimensional): Base slope

Down-slope shape: Concave, linear

Across-slope shape: Concave, linear

Ecological site: R049XB210CO - Sandy Foothill

Hydric soil rating: No

APPENDIX E: Septic Records

EL PASO COUNTY HEALTH DEPARTMENT
COLORADO SPRINGS, COLORADO

SEWAGE DISPOSAL INSPECTION FORM

#4404000014

DATE

1/24/82

ENVIRONMENTALIST

J. J. Ramis

APPROVAL:

YES ☒ NO ☐

LOCATION (

14655 Davis Rd. B

OCCUPANT

Paul Hunt

LEGAL DESCRIPTION

NE $\frac{1}{4}$, E $\frac{1}{4}$, E $\frac{1}{2}$, NW $\frac{1}{4}$, NE $\frac{1}{4}$, S $\frac{1}{4}$, T14S, R64

TYPE OF CONSTRUCTION

Mobile

NO. OF BEDROOMS

3

SYSTEM INSTALLED BY

Dwayne Lee Pac

COMMERCIAL MFG.

Garden

SIZE

1250g

TYPE OF MATERIAL

Cement

NO. COMPARTMENTS

2

WIDTH 60"

LENGTH

120"

DEPTH (total)

68"

LIQ. CAP.

1250g

DISPOSAL FIELD: BED OR TRENCH DEPTH

24"

WIDTH

36"

LENGTH

235

SQ. FT.

705

DISTANCE BETWEEN LINES

15'

ROCK

1 $\frac{1}{2}$ Quarry

DEPTH

12"

UNDER

6"

OVER

2"

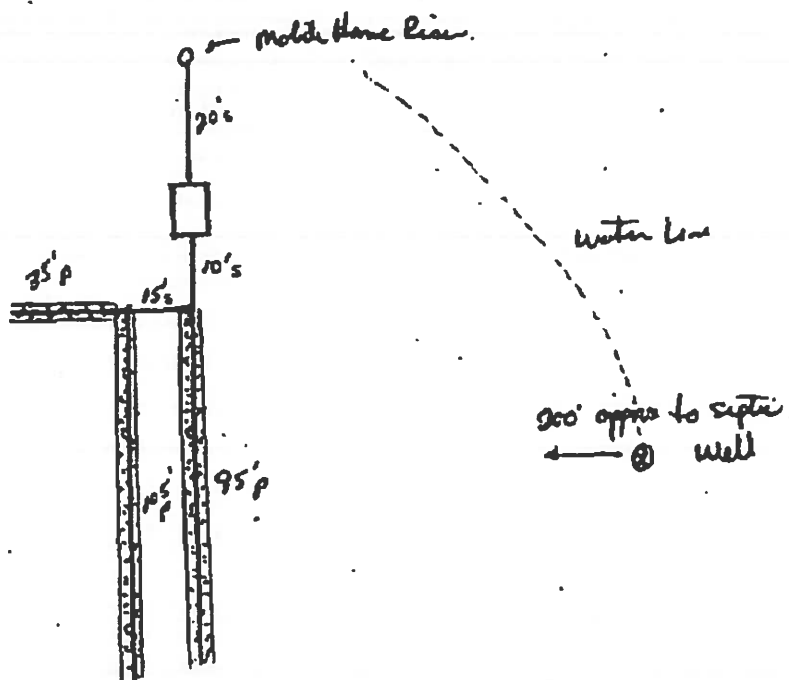
LEACHING PITS (NO.)

LINING MATERIAL

CAPACITY SQ. FT.

Davis Rd.

NORTH



Address 205 50 Permit No. 1847

EL PASO COUNTY • COUNTY HEALTH DEPARTMENT

501 North Foote Avenue • Colorado Springs, Colorado • 636-0125

Water Supply W-2-1 Receipt No. 2814

PERMIT

TO CONSTRUCT, ALTER, REPAIR OR MODIFY ANY INDIVIDUAL SEWAGE DISPOSAL SYSTEM

Issued To Pero Hunt 14655 David Date 11/2/82

Address of Property NE 1/4, E 1/4, NW 1/4, NE 1/4, Section 4, T 14, R 64, CO-Phone
Lot 7 Easton Davis Rd. (Permit valid at this address only)

Sewage-Disposal System work to be performed by Dwayne Dee Exc. Phone _____

This Permit is issued in accordance with 25-10-106 Colorado Revised Statutes 1973, as amended. PERMIT EXPIRES upon completion-installation of sewage-disposal system or at the end of six (6) months from date of issue — whichever occurs first — (unless work is in progress).

— THIS PERMIT DOES NOT DENOTE APPROVAL OF ZONING OR ABREAGRE REQUIREMENTS —

\$130.00
PERMIT FEE

John B. Galt, M.D.
DIRECTOR, CITY-COUNTY HEALTH DEPARTMENT

DATE OF EXPIRATION APR 1 2 1983

ENVIRONMENTALIST

NOTE: LEAVE ENTIRE SEWAGE-DISPOSAL SYSTEM UNCOVERED FOR FINAL INSPECTION. 48 HOUR ADVANCE NOTICE REQUIRED.

SEPTIC TANK SIZE <u>1250</u>	GALLONS	TOTAL ABSORPTION AREA <u>630</u>	SQ. FT.
TRENCH SYSTEM	BED SYSTEM	SEEPAGE PIT SYSTEM	
<u>210</u> ft. of trench <u>36</u> inches wide	<u> </u> ft. x <u> </u> ft.	<u> </u> rings or <u> </u> diam. x <u> </u> w/d	

NOTES:

The Health Office shall assume no responsibility in case of failure or inadequacy of a sewage-disposal system, beyond consulting in good faith with the property owner or representative. Free access to the property shall be authorized at reasonable times for the purpose of making such inspections as are necessary to determine compliance with requirements of this law.

Date 10/5/82

EL PASO COUNTY HEALTH DEPARTMENT
501 NORTH FOOTE AVENUE
COLORADO SPRINGS, COLORADO
636-0125

Tax Schedule # _____

Application for permit to construct, Remodel, or Install a Sewage Disposal System.

Name of Owner Fern Hunt 14655 Davis Rd Phone _____

Address of Property NE 4 E 4 E 2 E 2 NW 4 NE 4 Sub 4, 14, 64

Legal Description of Property 14655 Davis Rd. Lot 7.

Owner's Address (if different) _____ Phone _____

Systems Contractor Dwayne Lee Exc Address _____

Type of Construction Mobile source and type of water supply well

Size of Lot 40 ac

The construction of the Sewage Disposal System will comply with all applicable Laws, Ordinances, Standards or Resolutions.

HEALTH DEPARTMENT USE ONLY

Permit Number _____ Receipt Number _____

Number of Bedrooms 3 Tank Capacity 650 gallons Absorption area 630 Sq. Ft.

Remarks 210 linear ft of 36" trench

APPLICATION IS

(☒) APPROVED

(☐) DENIED

ENVIRONMENTALIST

Jeff Harris

DATE 10/5 19 82

PLOT PLAN WILL INCLUDE THE FOLLOWING

Plot plan may be drawn on the back of this sheet or on a separate sheet.

- | | |
|--|---------------------------------------|
| 1. Streams, Lakes, Ponds, Irrigation Ditches and other Water Courses | 6. Location of Proposed Septic System |
| 2. North Direction | 7. Location of percolation test |
| 3. Location of Property Line | 8. Geographical features |
| 4. Buildings | 9. Other Information as required |
| 5. Wells | |