



See comment letter.

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Lorson Ranch Sketch Plan Amendment 2 Traffic Impact and Access Analysis (LSC #184890) December 17, 2018


Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.



Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.



12/16/18
Date



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December 17, 2018

Mr. Jeff Mark
The Landhuis Company
212 North Wahsatch Avenue, Suite 301
Colorado Springs, CO 80903

RE: Lorson Ranch Sketch Plan Amendment 2
El Paso County, Colorado
Traffic Impact and Access Analysis
LSC #184890

Dear Mr. Mark:

LSC Transportation Consultants, Inc. has prepared this traffic impact analysis for the currently proposed amendment to the Lorson Ranch Sketch Plan. Lorson Ranch is located east of the intersection of Marksheffel Road and Fountain Boulevard in El Paso County, Colorado. The site location is shown on Figure 1.

LSC prepared a Traffic Technical Memorandum for Lorson Ranch dated April 15, 2016. Since completion of that report LSC has prepared several traffic impact studies and transportation memorandums for developments within Lorson Ranch. These reports include:

- *Carriage Meadows South at Lorson Ranch Filing No 1 Updated Traffic Impact and Access Analysis*, August 14, 2017
- *Lorson Ranch East Updated Traffic Impact and Access Analysis*, November 9, 2017 (Revised January 8, 2018)
 - *Lorson Ranch East Filing No 1 Transportation Memorandum*, May 2, 2018
 - *Lorson Ranch East Filing No 2 Transportation Memorandum*, September 24, 2018
- *Carriage Meadows North at Lorson Ranch Filing No 1 Updated Traffic Impact Analysis*, January 29, 2018
- *Lorson Ranch PK-8 School Traffic Impact and Access Analysis*, March 11, 2018
- *Creekside at Lorson Ranch Filing No 1 Traffic Impact and Access Analysis*, October 25, 2018

REPORT CONTENTS

This report contains the following:

- \$ Recent/current street and traffic conditions in the vicinity of the site and the recent reports completed by LSC for Lorson Ranch for identification of existing and planned street widths, lane geometries, traffic controls, posted speed limits, street classification, etc.
- \$ Existing traffic volumes at the key intersections in the vicinity of the site and estimates of 2040 background traffic volumes.
- \$ The projected average weekday and peak-hour vehicle trips to be generated by all of the existing and planned future land uses within Lorson Ranch and a comparison to the estimate assumed in the 2016 Sketch Plan report.
- \$ The assignment of the projected trips to the existing and planned street system.
- \$ The resulting 2040 total traffic volumes on the street system.
- \$ The resulting traffic impacts. The traffic impacts have been quantified by determining the future levels of service at the intersections of Marksheffel Road/Fontaine Boulevard and Marksheffel Road/Lorson Boulevard.
- \$ Recommendations for street functional classification and traffic controls for the key street segments and intersections in the vicinity of the site.

SITE DEVELOPMENT AND LAND USE

The sketch plan area is shown in Figure 1. At buildout, the Lorson Ranch Sketch Plan is approved for up to 6,500 residential dwelling units and up to 70 acres of commercial land use. With this amendment, these would still apply. The purpose of the Lorson Ranch Sketch Plan Amendment No. 2 is to revise and increase the existing approved land use designations on several of the remaining undeveloped areas within Lorson Ranch to increase the permitted density, most notably in the far eastern portions of the Lorson Ranch development. The total number of dwelling units is not planned to be changed, however, this amendment will allow for smaller single-family units such as patio homes, cluster homes, courtyard homes, and duplexes and more multifamily dwelling units such as townhomes and apartments. The proposed Sketch Plan Amendment 2 is attached.

LAND USES ASSUMED IN THE TRAFFIC ANALYSIS

Table 1 shows the land use assumptions assumed in this report and a comparison to the land uses assumed in the *Lorson Ranch Sketch Plan Traffic Technical Memorandum* by LSC dated April 15, 2016. Figure 2 shows the land use zones used for comparison purposes.

Residential Land Uses

Although the previous Lorson Ranch Sketch Plan Amendment allowed for up to 6,500 residential dwelling units the analysis assumed in the *Lorson Ranch Sketch Plan Traffic Technical Memorandum* by LSC dated April 15, 2016 was based on an actual anticipated buildout of 5,301

dwelling units. This number reflected the anticipated buildout accounting for the proposed maximum number of dwelling units that could be developed east of Jimmy Camp Creek in order to keep the future creek crossing structures on Fontaine Boulevard as two lanes. This report is also based on a total buildout of 5,301 dwelling units. Although the total number of dwelling units assumed in this report has not changed, the percentage of multifamily dwelling units has been increased. The 2016 technical memorandum assumed 118 multifamily dwelling units and 5,183 single-family dwelling units. Based on the currently proposed Sketch Plan amendment this report assumes 894 multifamily dwelling units and 4,407 single-family dwelling units.

School Land Uses

Needs to be based on the total allowed by the SKP since the purpose is to shift density to the east.

The Sketch Plan includes a 25-acre school site located northeast of the future intersection of Fontaine Boulevard and Lamprey Drive. The size and location of the school site are consistent with what was assumed in the 2016 traffic technical memorandum, however, the number of students assumed to attend the school has been updated based on the *Lorson Ranch PK-8 School Traffic Impact and Access Analysis* by LSC dated May 11, 2018.

Commercial Land Uses

Although the Lorson Ranch Sketch Plan is approved for up to 70 acres of commercial development, the currently approved amendment and the currently proposed Amendment 2 both show only about 25.3 acres used for commercial uses. The commercial parcels will be located adjacent to Marksheffel Road north and south of Fontaine Boulevard, as shown in the attached Sketch Plan exhibit. This is consistent with what was assumed in the 2016 traffic technical memorandum. A site-specific traffic study would address whatever commercial land use is proposed. The timing and type of commercial development will depend on market demand.

Land use assumptions are shown as part of Table 1.

ROADWAY AND TRAFFIC CONDITIONS

Area Roadways

Figure 1 shows the roadways in the vicinity of the site. The major roadways are identified below followed by a brief description of each.

§ **Marksheffel Road** extends north from the Link Road/C&S Road intersection in Fountain, Colorado to north of Woodmen Road. Marksheffel Road is shown as a future four-lane Expressway on the County *Major Transportation Corridors Plan (MTCP)*. The posted speed limit on Marksheffel Road at Fontaine Boulevard is 55 miles per hour (mph). The PPRTA has completed the Marksheffel Road upgrade between Mesa Ridge Parkway and Bradley Road. This included intersection improvements at the Fontaine Boulevard intersection.

§ **Fontaine Boulevard** is designated as a four-lane Urban Principal Arterial east of Marksheffel Road and it has been constructed as such from Marksheffel Road east to Old Glory Drive. As part of the Lorson Ranch East development Fontaine Boulevard will be extended east from Old Glory Drive. In the interim, an Urban Non-Residential Collector Street will be constructed east of Stingray Lane as development progresses. The posted speed limit on Fontaine Boulevard is 35 mph just east of (and a short distance west of) Marksheffel Road. The speed limit increases to 45 mph just east of the bridge over Jimmy Camp Creek.

§ **Lorson Boulevard** is a planned future roadway that will extend from Marksheffel Road about one-half mile south of Fontaine Boulevard. Lorson Boulevard will be classified as an Urban Non-Residential Collector Street (modified for a 44-foot street width rather than the standard 52-foot street width) with an 80-foot-wide right-of-way. East of Stingray Lane, Lorson Boulevard will be classified as an Urban Non-Residential Collector Street (modified for a 44-foot street width rather than the standard 52-foot street width). The right-of-way will vary from 64 feet to 72 feet to accommodate anticipated future right-turn deceleration lanes. The right-of-way not adjacent to right-turn lanes would be 64 feet. Also, tracts adjacent to the right-of-way will allow for future right-of-way expansion to 80 feet if ever needed. The proposed cross section includes two 14-foot "shared-use" travel lanes, a striped two-way left-turn lane and right-turn deceleration lanes where warranted.

Existing Traffic Volumes

Figure 3 shows the recent traffic volumes at the intersection of Marksheffel Road/Fontaine Boulevard. These traffic volumes were based on traffic counts conducted by LSC in March 2018. The traffic count reports are attached.

Existing Levels of Service

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection. Level of service is indicated on a scale from "A" to "F." LOS A represents control delay of less than 10 seconds for unsignalized and signalized intersections. LOS F represents control delay of more than 50 seconds for unsignalized intersections and more than 80 seconds for signalized intersections. Table 2 shows the level of service delay ranges.

existing and

| Table 2 | | | |
|--|--|--------------------------|--|
| Intersection Levels of Service Delay Ranges | | | |
| Level of Service | Signalized Intersections | | Unsignalized Intersections |
| | Average Control Delay (seconds per vehicle) | V/C⁽¹⁾ | Average Control Delay (seconds per vehicle)⁽²⁾ |
| A | 10.0 sec or less | less than 0.60 | 10.0 sec or less |
| B | 10.1-20.0 sec | 0.60-0.69 | 10.1-15.0 sec |
| C | 20.1-35.0 sec | 0.70-0.79 | 15.1-25.0 sec |
| D | 35.1-55.0 sec | 0.80-0.89 | 25.1-35.0 sec |
| E | 55.1-80.0 sec | 0.90-0.99 | 35.1-50.0 sec |
| F | 80.1 sec or more | 1.00 and greater | 50.1 sec or more |

(1) Source: *Transportation Research Circular 212*
 (2) For unsignalized intersections if V/C ratio is greater than 1.0 the level of service is LOS F regardless of the projected average control delay per vehicle.

The intersection of Marksheffel/Fontaine was analyzed to determine the existing levels of service using Synchro. Figure 3 shows the level of service analysis results. As shown on the figure, all movements at this intersection are level of service C or better during the peak hours. The level of service (LOS) reports are attached.

2040 BASELINE TRAFFIC

Figure 4 shows the projected 2040 baseline traffic volumes. The 2040 baseline traffic volumes are based on traffic volumes shown in the *Marksheffel Road South Corridor Preservation Plan* dated July 2014. The 2040 baseline volumes assume Meridian Road has not been extended south to Fontaine Boulevard. The baseline traffic volumes exclude existing traffic currently generated by land uses within Lorson Ranch and traffic projected to be generated by future land uses.

TRIP GENERATION

Estimates of the traffic volumes expected to be generated by all of the existing, approved, and proposed land uses within Lorson Ranch have been made using the nationally published trip generation rates found in *Trip Generation, 10th Edition, 2017* by the Institute of Transportation Engineers (ITE). Table 3 shows the results of the trip generation estimates. Table 3 also shows a comparison of the trip generation estimate for this same area assumed in the *Lorson Ranch Sketch Plan Traffic Technical Memorandum* by LSC dated April 15, 2016. The estimate contained in the Sketch Plan memorandum was made using the 9th edition of the *Trip Generation* manual.

These estimates are not intended to represent the overall maximum trip generation allowed under the Lorson Ranch approvals (6,500 dwelling units and 25.3 acres of commercial). Rather these estimates reflect the anticipated buildout land uses accounting for the proposed maximum number of dwelling units that could be developed east of Jimmy Camp Creek in order to keep the future creek crossing structures, the proposed Lorson Boulevard, and the Fontaine extension as

They need to be if the Sketch Plan is reserving the right to develop 6500 DU by shifting density to the undeveloped ares to the east.

two lanes (with right-of-way widths as recommended in this study and shown on the Sketch Plan amendment).

The table presents the raw unadjusted ITE trip generation for the land uses then adjusted trip generation to account for internal trips within Lorson Ranch and pass-by trips associated with planned commercial uses. A percentage of the trips generated by the commercial parcels would be "pass-by trips." A pass-by trip is one made by a motorist who would already be on an adjacent roadway regardless of the proposed development, but who stops in at the site while passing by. The motorist would then continue on his or her way to a final destination in the original direction. The pass-by percentages shown were based on the *Trip Generation Handbook - An ITE Proposed Recommended Practice, 3rd Edition, 2017* by ITE.

The internal portion of the total trips for each traffic analysis zone has been paired with internal trips from other traffic analysis zones. Appendix Table 1 shows the internal trip assumptions and calculations.

As shown in Table 3, at buildout Lorson Ranch is projected to generate about 46,083 new external vehicle-trips on the average weekday, with about one-half of the vehicles entering and one-half of the vehicles exiting in a 24-hour period. This includes trips estimated to be generated by all land uses within the Lorson Ranch Sketch Plan area including existing land uses. The *Lorson Ranch Sketch Plan Traffic Technical Memorandum* by LSC dated April 15, 2016 assumed Lorson Ranch would generate about 1,801 more vehicle-trips than are currently estimated. The reduction in the estimated number of vehicle-trips is due partly to a change in trip generation rates from the 9th edition to the 10th edition and partly due to an increase in the number of multifamily dwelling units, which have a lower trip generation rate.

TRIP DISTRIBUTION AND ASSIGNMENT

The directional distribution of the site-generated traffic volumes on the street and roadway system serving the site is one of the most important factors in determining the site's traffic impacts. Figure 5 shows the external trip distribution estimates (external to Lorson Ranch). The directional distribution estimates have been based on the location of the site with respect to the regional residential employment, commercial, and activity centers; the land use proposed; the access/roadway connections assumed; the roadway network; and the most recent traffic counts conducted at the intersection of Marksheffel/Fontaine.

Figure 6 shows the long-term site-generated traffic volume estimates. These estimates include traffic generated by existing land uses within Lorson Ranch. These volumes were determined by first assigning the internal vehicle-trips to the street network based on the location of the planned school site located northeast of the intersection of Fontaine Boulevard and Lamprey Drive and the future retail sites located near the intersection of Fontaine Boulevard and Carriage Meadows Drive.

The external vehicle-trips were then assigned to the street network by applying the trip distribution percentages (from Figure 5) to the external trip generation estimates. The internal

and external site-generated traffic volumes were then summed to determine the total site-generated traffic volumes.

PROJECTED TOTAL TRAFFIC

Figure 7 shows the 2040 total traffic volumes. These volumes are the sum of the 2040 background traffic volumes (from Figure 4) plus the site-generated traffic volumes (from Figure 6).

PROJECTED LEVELS OF SERVICE

The intersections of Marksheffel Road/Fontaine Boulevard and Marksheffel Road/Lorson Boulevard have been analyzed to determine the projected levels of service for the total traffic volumes based on the signalized method of analysis from Synchro and the unsignalized method of analysis procedures outlined in the *Highway Capacity Manual, 6th Edition* by the Transportation Research Board. The level of service reports are attached. The results of the analysis are shown in Figure 7.

Marksheffel/Fontaine

The signal-controlled Marksheffel Road/Fontaine Boulevard intersection is projected to continue to operate at level of service D or better for all movements based on the short-term total traffic volumes. By 2040 this intersection is projected to operate at an overall LOS D or better during the peak hours; however, the southbound left-turn and westbound left-turn movements are projected to operate at LOS E during the afternoon peak.

Marksheffel/Lorson

Based on the projected 2040 total traffic volumes, the westbound left-turn movement at the intersection of Marksheffel/Lorson is projected to operate at LOS F during the morning and afternoon peak hours if this intersection remains a conventional, stop-sign-controlled, full-movement intersection. Assuming a conventional, signal-controlled intersection, all movements are projected to operate at LOS B or better during the peak hours based on the 2040 total traffic volumes. Alternative intersection control such as a “channelized T” type intersection or a modern roundabout has been considered for this intersection. Please see Appendix A for a detailed analysis of these alternate options.

ROADWAY CLASSIFICATIONS

Figure 8 presents the recommended buildout roadway classifications for the Sketch Plan. These have been based in-part on the projected buildout site-generated traffic volumes shown on Figure 6. Note: Corridor preservation per the MTCP 2060 plan will be needed for Fontaine and this will need to be shown on any plans for development submitted that are adjacent to Fontaine.

FONTAINE BOULEVARD TRANSPORTATION CORRIDOR PRESERVATION

The MTCP Corridor Preservation Plan shows Fontaine as a potential future four-lane Principal Arterial. The ECM standard right-of-way for a four-lane Principal is 130 feet, however, “corridor preservation” means that adjacent developments would not need to dedicate right-of-way necessary to achieve this 130 feet, but rather just preserve land in the Fontaine roadway corridor in a “no build” area. The intent of a preservation area would be to include plat notes and restrictions to facilitate an easier acquisition process and with lower cost (when compared to the cost of acquiring land for right-of-way upon which structures and other property improvements have been built) in the future by the County to achieve the ultimate 130 feet (if ever needed). Clear notes would need to be added to the plats adjacent to Fontaine indicating specific strips of land or tracts designated as “transportation corridor potential future right-of-way.”

ROADWAY IMPROVEMENT FEE PROGRAM

The developments within Lorson Ranch will be required to participate in the El Paso County Road Improvement Fee Program. The fee will be determined with each Lorson Ranch development application.

CONCLUSIONS AND RECOMMENDATIONS

Trip Generation

- At buildout Lorson Ranch is projected to generate about 46,083 new external vehicle-trips on the average weekday, with about one-half of the vehicles entering and one-half of the vehicles exiting in a 24-hour period. This includes trips estimated to be generated by all land uses within the Lorson Ranch Sketch Plan area including existing land uses. The *Lorson Ranch Sketch Plan Traffic Technical Memorandum* by LSC dated April 15, 2016 assumed Lorson Ranch would generate about 1,801 more vehicle-trips than are currently estimated. The reduction in the estimated number of vehicle-trips is due partly to a change in trip generation rates from the 9th edition to the 10th edition and partly due to an increase in the number of multifamily dwelling units, which have a lower trip generation rate.

Projected Levels of Service

- The signal-controlled Marksheffel Road/Fontaine Boulevard intersection is projected to operate at an overall level of service D or better based on the 2040 background and total traffic conditions. The southbound left-turn and westbound left-turn movements are projected to operate at LOS E during the afternoon peak hour based on both the background and total traffic volumes.
- This report includes discussion and general analysis of intersection traffic control/intersection type alternatives for Lorson Boulevard/Marksheffel Road.

* * * * *

Please contact me if you have any questions or need further assistance.

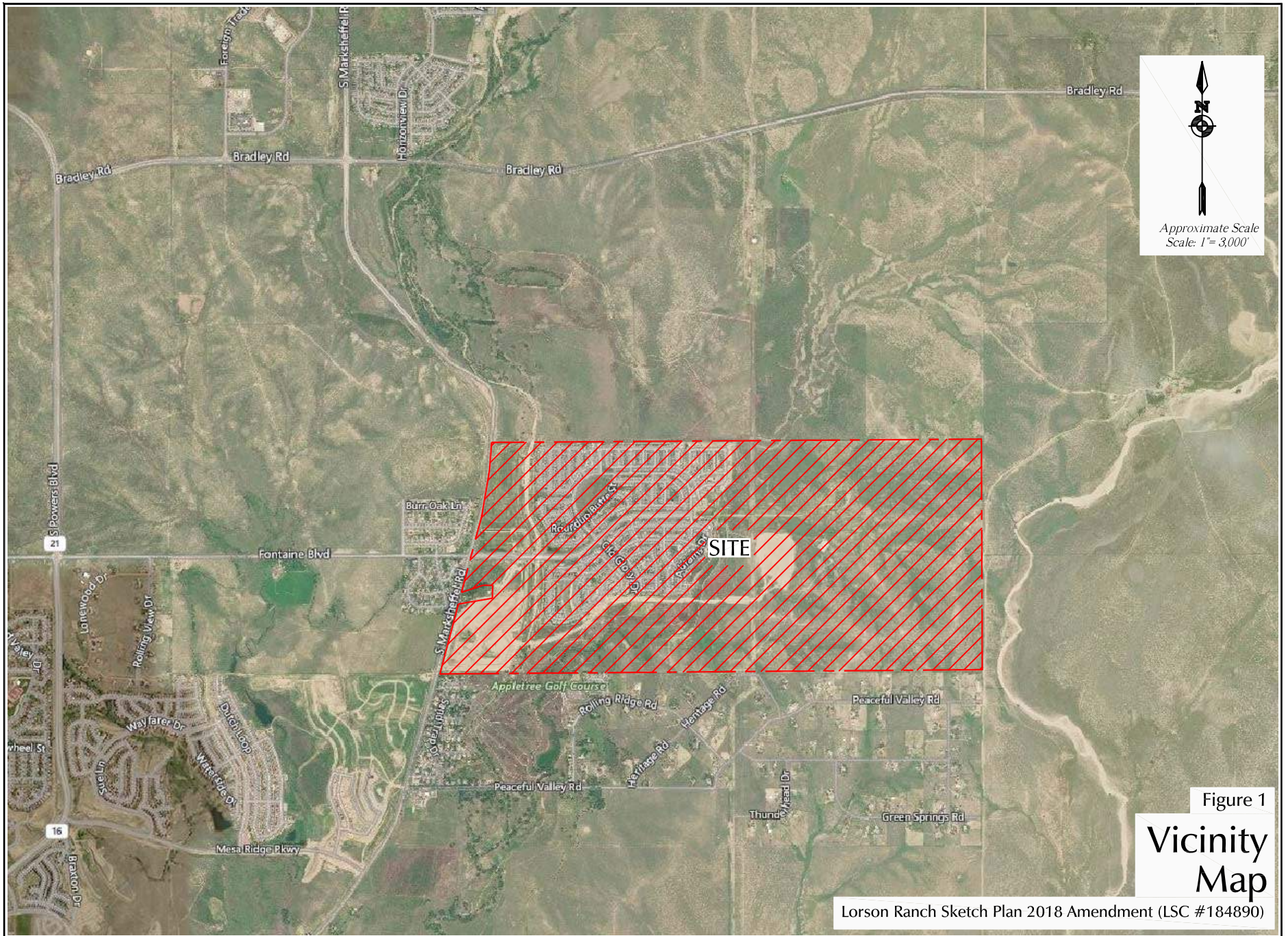
Sincerely,

LSC TRANSPORTATION CONSULTANTS, INC.

By: Jeffrey C. Hodsdon, P.E., PTOE
Principal

JCH:KDF:bjwb

Enclosures: Tables 1 and 3
Appendix Table 1
Figures 1-8
Current Sketch Plan Amendment
Traffic Count Reports
Level of Service Reports
Appendix A – Marksheffel/Lorson Traffic Control Analysis




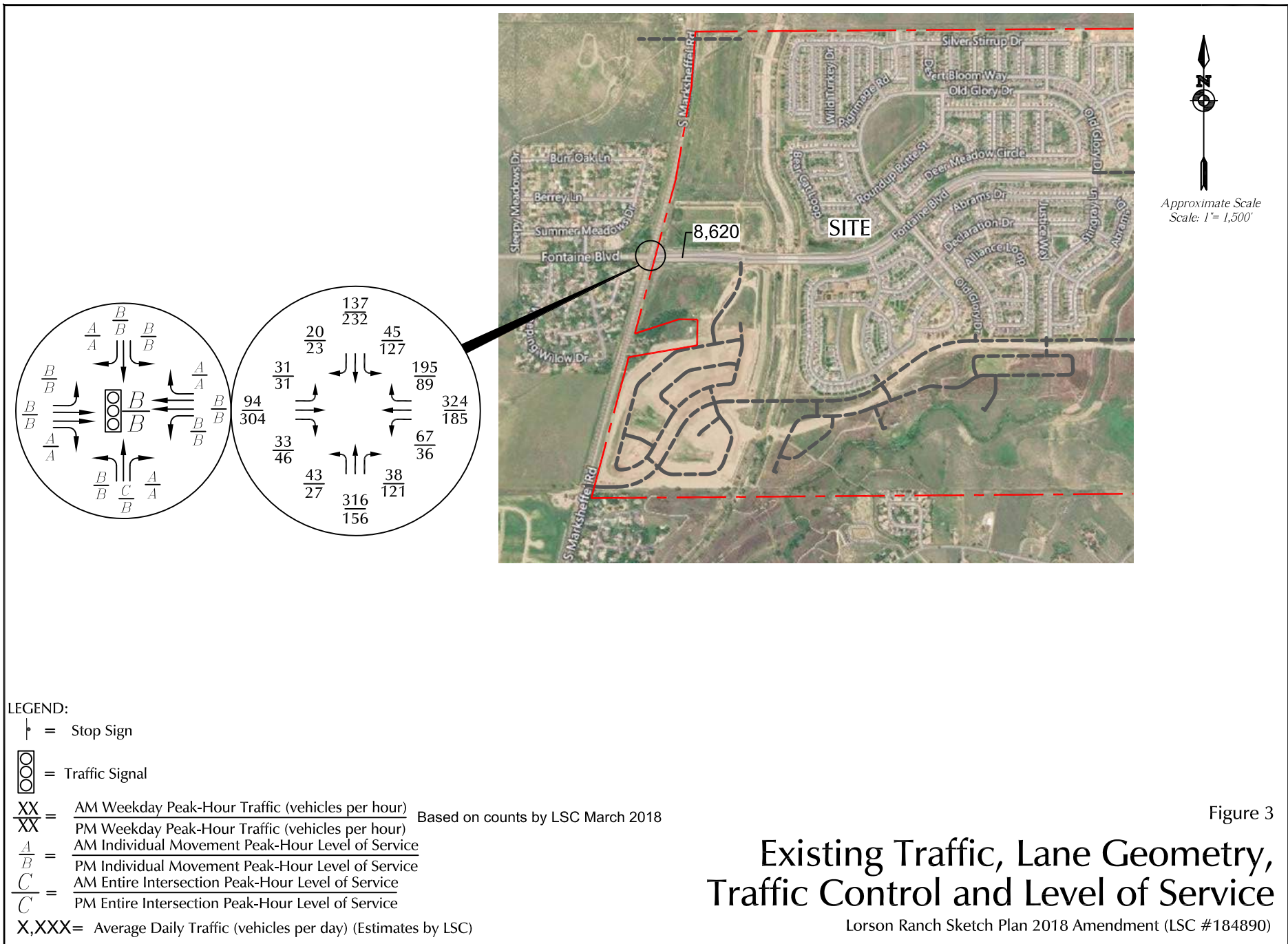
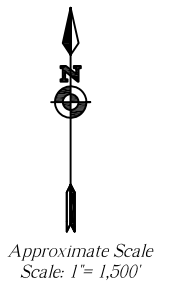
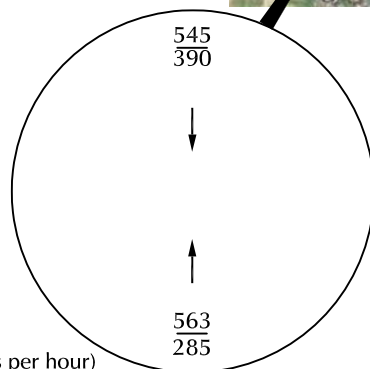
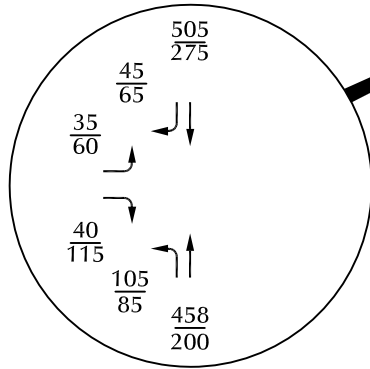
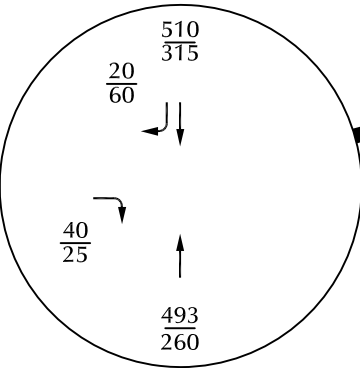
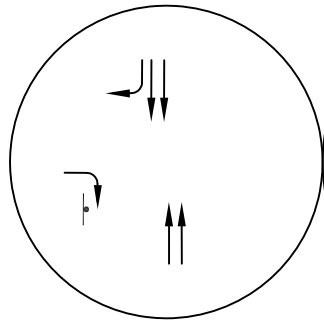
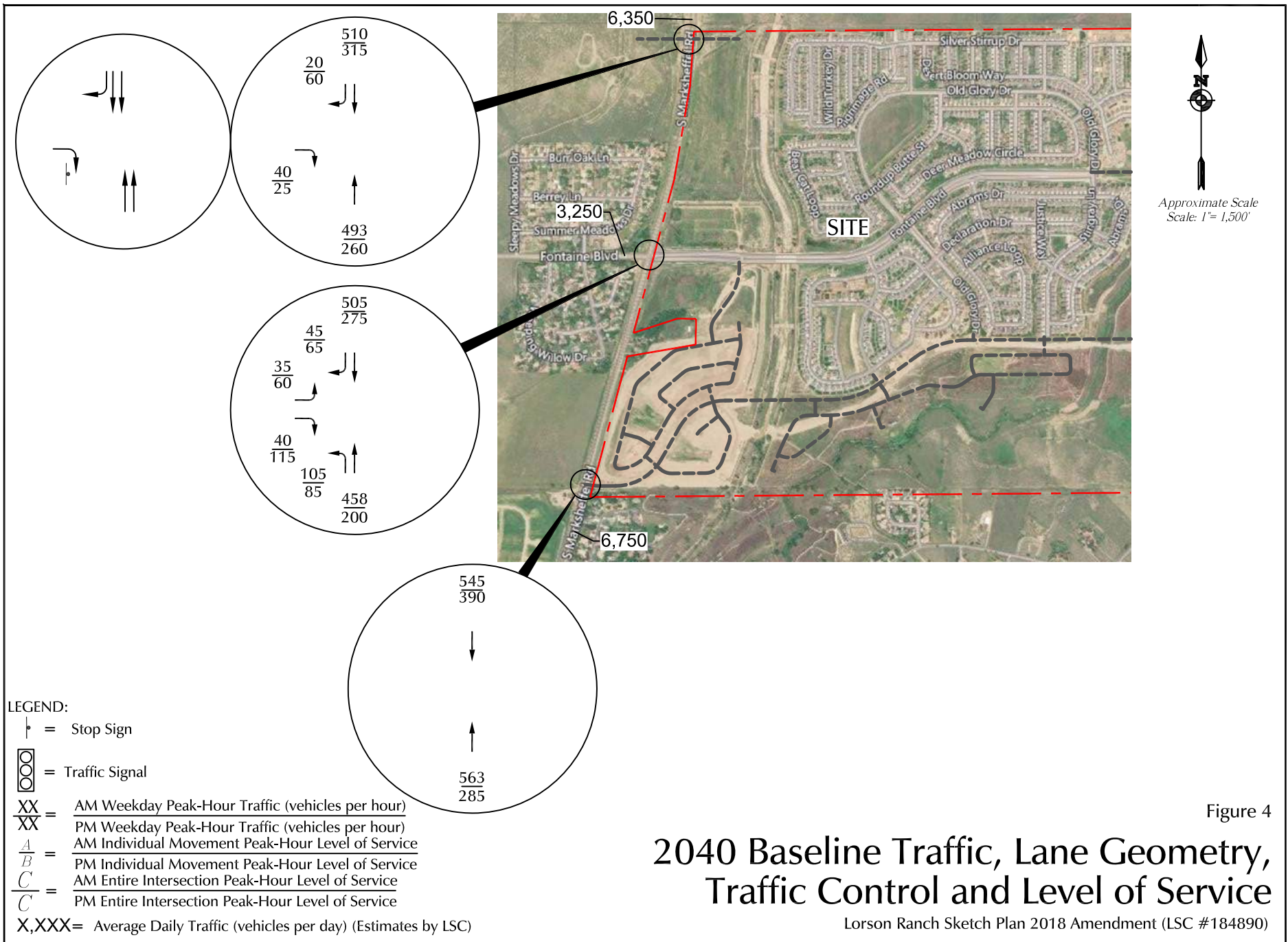

 Approximate Scale
 Scale: 1" = 3,000'

Figure 1

Vicinity Map

Lorson Ranch Sketch Plan 2018 Amendment (LSC #184890)



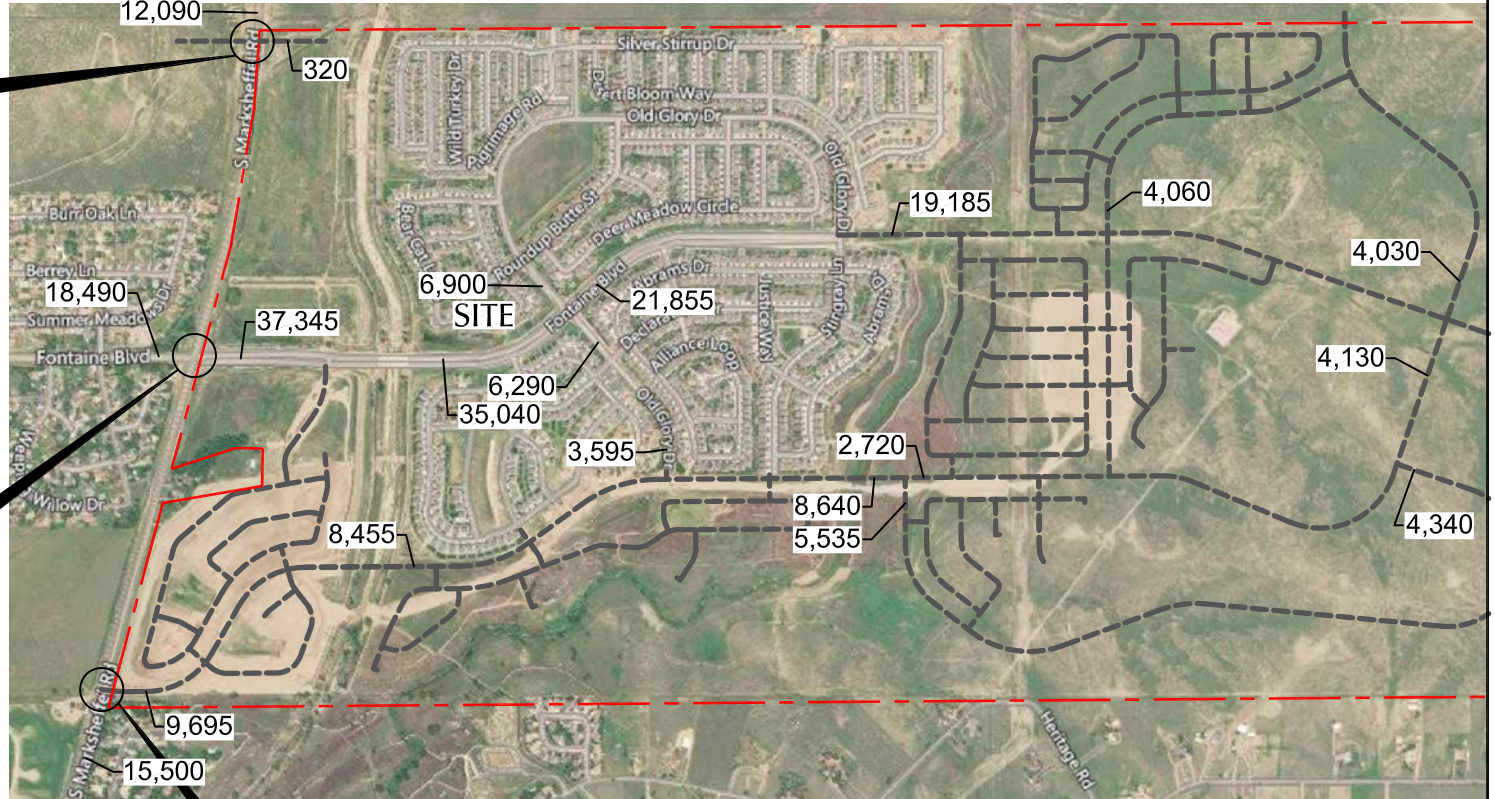
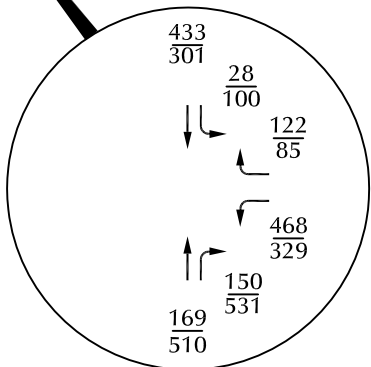
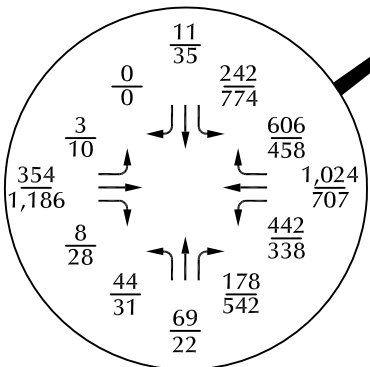
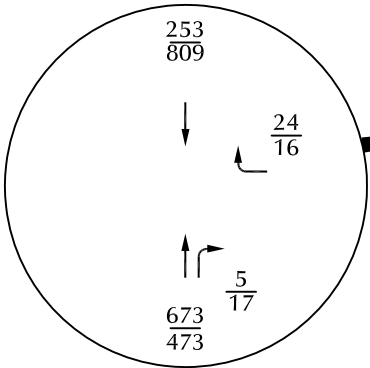
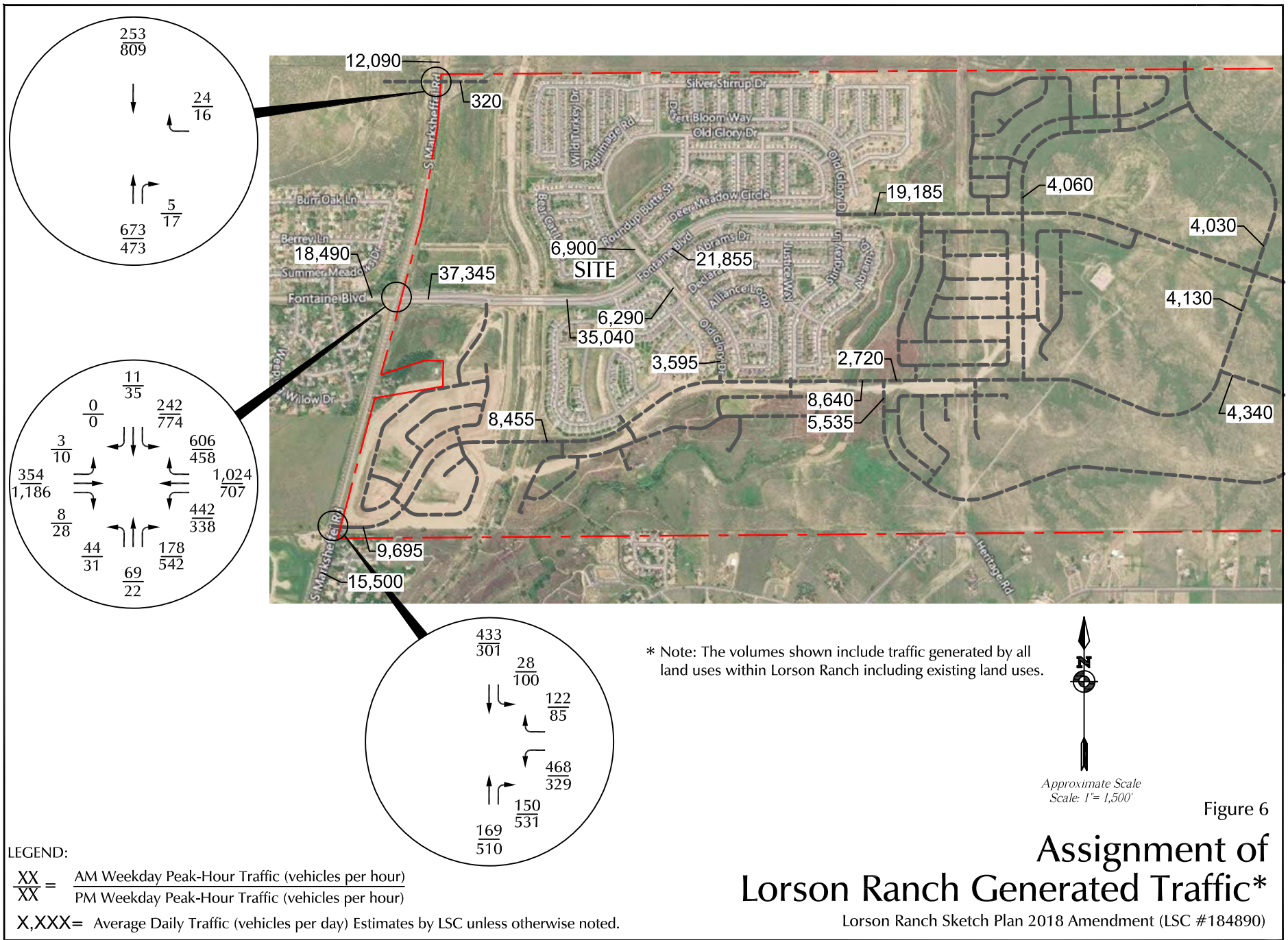




Approximate Scale
Scale: 1" = 1,500'

LEGEND:
 XX%
 XX%
 XX%
 XX% = Zone 1 (Residential) Percent Directional Distribution
 Zone 2 (Residential) Percent Directional Distribution
 Zone 3 (Residential) Percent Directional Distribution
 Zone 4 (Non-Residential) Percent Directional Distribution

Figure 5
**Directional Distribution
 of Site-Generated Traffic**
 Lorson Ranch Sketch Plan 2018 Amendment (LSC #184890)



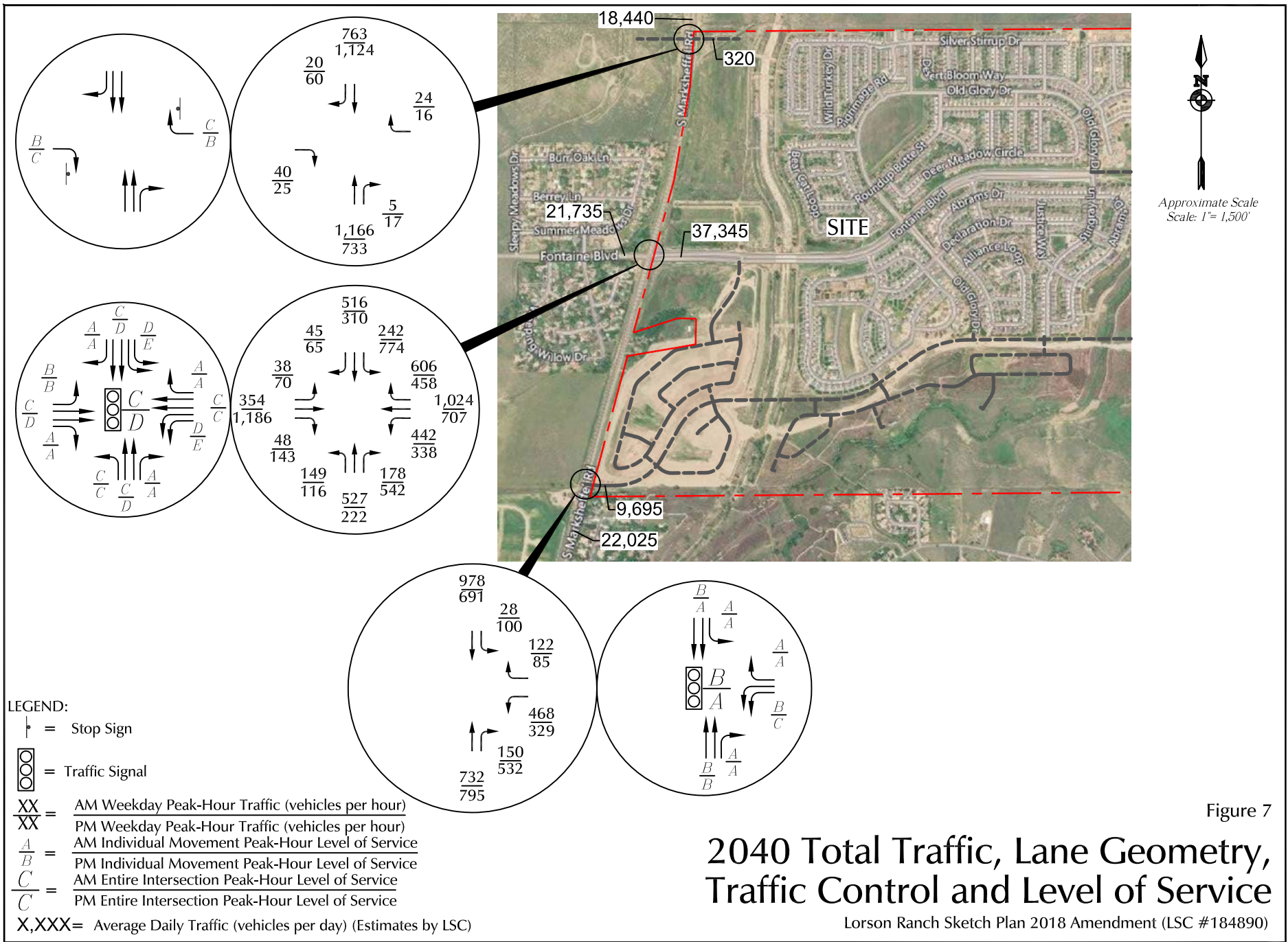
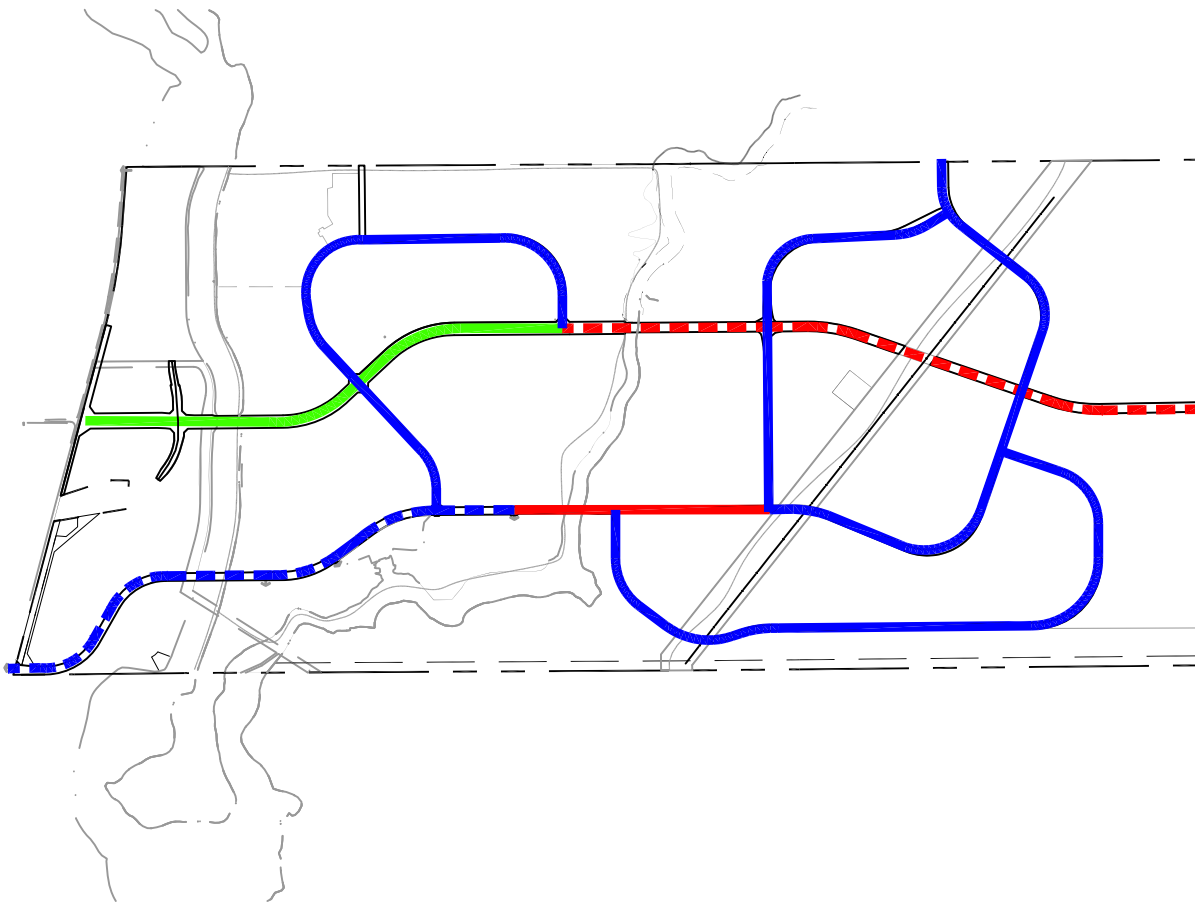



Figure 7




 Approximate Scale
 Scale: 1" = 2,000'



LEGEND:






-  = 4 Lane Principal Arterial
-  = 4 Lane Principal Arterial (Ultimate Classification)
Interim/Lorson Ranch Buildout 2-Lane Urban Non-Residential Collector
-  = Urban Non-Residential Collector (80' ROW) w/ Modified Street Cross-Section Per Deviation #DEV-17-008
-  = Urban Residential Collector (64' to 72' ROW)
-  = Urban Residential Collector (60' ROW).

Figure 8

Recommended Classifications

Lorson Ranch Sketch Plan 2018 Amendment (LSC #184890)

**Table 1
Lorson Ranch Sketch Plan Amendment 2
Land Use Comparison**

| Land Use Zone | Description | Lorson Ranch Sketch Plan Traffic Technical Memorandum 4/15/2016 | | | Current Land Use Assumption | | | Change | | |
|---------------|---|---|------------------|--------------|-----------------------------|------------------|--------------|-------------------|------------------|-----------|
| | | Single-Family DUs ⁽¹⁾ | Multi-family DUs | Total DUs | Single-Family DUs | Multi-family DUs | Total DUs | Single-Family DUs | Multi-family DUs | Total DUs |
| 1 | Adjacent to Marksheffel (Carriage Meadows North and South) | 434 | 72 | 506 | 389 | 56 | 445 | -45 | -16 | -61 |
| 2 | Between the Creeks and North of Lorson Blvd | 1,405 | 46 | 1,451 | 1,373 | 195 | 1,568 | -32 | 149 | 117 |
| 3 | Between the Creeks and South of Lorson Blvd (Creekside at Lorson Ranch Fil 1) | 449 | 0 | 449 | 235 | 0 | 235 | -214 | 0 | -214 |
| 4 | Lorson Ranch East | 783 | 0 | 783 | 826 | 0 | 826 | 43 | 0 | 43 |
| 5 | Between the Lorson Ranch East and the DSD Easment North of Fontaine | 131 | 0 | 131 | 0 | 299 | 299 | -131 | 299 | 168 |
| 6 | Between the East Tributary and Lorson Ranch East South of Lorson | 556 | 0 | 556 | 215 | 97 | 312 | -341 | 97 | -244 |
| 7 | East of the DSD Easment and North of Fontaine | 498 | 0 | 498 | 369 | 247 | 616 | -129 | 247 | 118 |
| 8 | East of the DSD Easment and South of Fontaine | 927 | 0 | 927 | 1,001 | 0 | 1,001 | 74 | 0 | 74 |
| TOTAL | | 5,183 | 118 | 5,301 | 4,408 | 894 | 5,302 | -775 | 776 | 1 |

Notes:

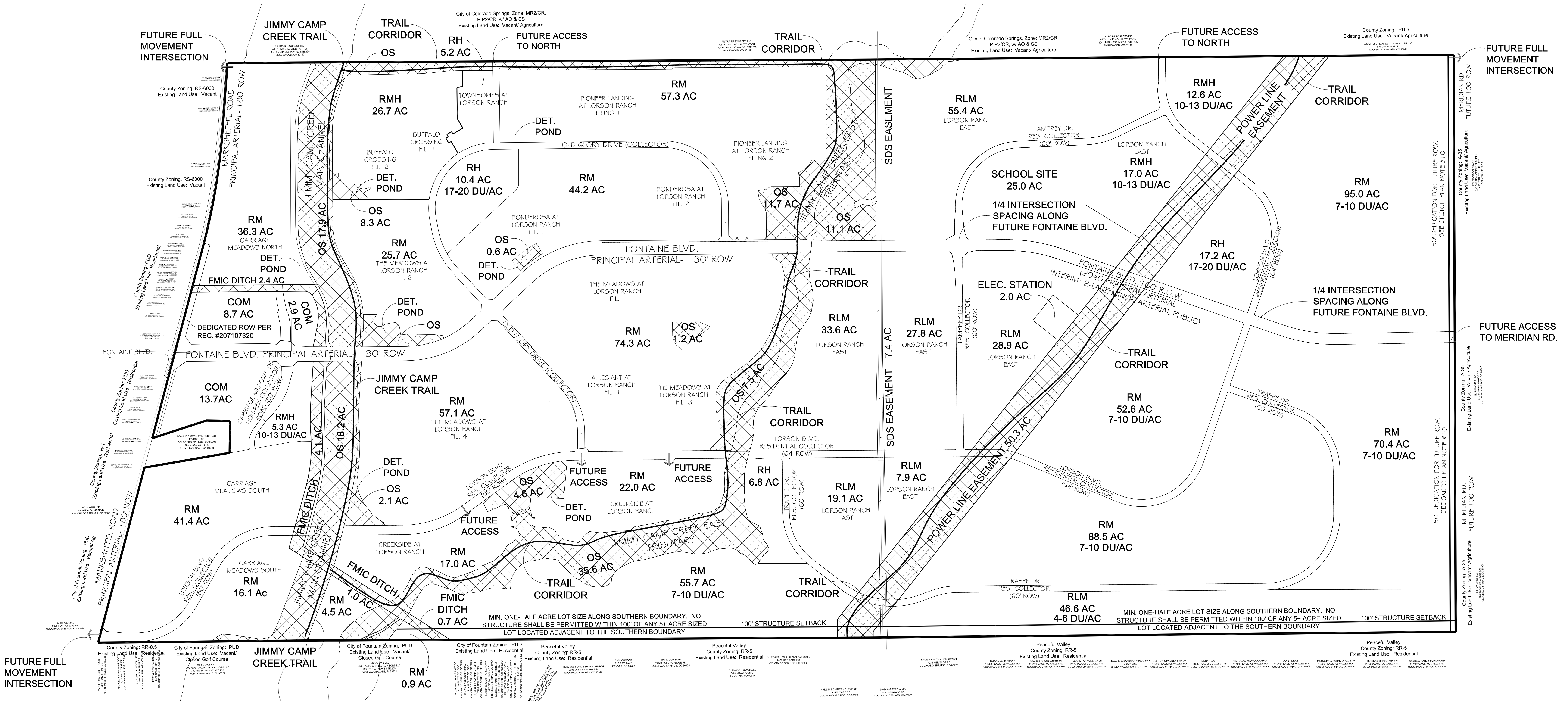
(1) DU = dwelling Unit

LSC Transportation Consultants, Inc.

6,500 now?

Table 3
Lorson Ranch Sketch Plan Amendment 2
Trip Generation Estimate

| Land Use Data | | Trip Generation Rates ⁽¹⁾ | | | | | | | | Raw ITE Trip Generation (Individual Driveway Trips) | | | | | School Internal Trips ⁽²⁾ | | | | | Retail Internal Trips ⁽²⁾ | | | | | Pass-by ⁽³⁾ (%) | Pass-by Trips | | | | | Total New External Trips | | | | | | |
|--|---|--------------------------------------|--------------|-----------|-------------------|--------------|------|------|--------------|---|---------------|--------------|--------------|--------------|--------------------------------------|--------------|-----------|--------------|--------------|--------------------------------------|--------------|--------------|-----------|--------------|----------------------------|---------------|-------|--------------|--------------|-----|--------------------------|---------------|--------------|--------------|--------------|------------|-----|
| | | ITE Code | Quantity | Unit | Daily | AM Peak Hour | | | PM Peak Hour | | Daily | AM Peak Hour | PM Peak Hour | Daily | AM Peak Hour | PM Peak Hour | Daily | AM Peak Hour | PM Peak Hour | Daily | AM Peak Hour | PM Peak Hour | Daily | AM Peak Hour | | PM Peak Hour | Daily | AM Peak Hour | PM Peak Hour | | | | | | | | |
| | | | | | | In | Out | In | Out | In | | | | | | | | | | | | | | | | | | | | Out | In | Out | In | Out | In | Out | In |
| RESIDENTIAL | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| All Residential North of Lorson Boulevard "Between the Creeks" | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 8 | Ponderosa | Single-Family Detached Housing | 210 | 102 | DU ⁽⁴⁾ | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 963 | 19 | 57 | 64 | 37 | 27 | 2 | 5 | 1 | 1 | 103 | 0 | 2 | 5 | 2 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 833 | 17 | 50 | 58 | 34 |
| 9 | Ponderosa | Single-Family Detached Housing | 210 | 102 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 963 | 19 | 57 | 64 | 37 | 27 | 2 | 5 | 1 | 1 | 103 | 0 | 2 | 5 | 2 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 833 | 17 | 50 | 58 | 34 |
| 10 | Meadows Fil 1 | Single-Family Detached Housing | 210 | 97 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 916 | 18 | 54 | 60 | 36 | 26 | 2 | 5 | 1 | 1 | 98 | 0 | 2 | 5 | 2 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 792 | 16 | 47 | 54 | 33 |
| 11 | Meadows Fil 3 | Single-Family Detached Housing | 210 | 51 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 481 | 9 | 28 | 32 | 19 | 14 | 1 | 2 | 1 | 0 | 51 | 0 | 1 | 3 | 1 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 416 | 8 | 25 | 28 | 18 |
| 12 | Meadows Fil 3 | Single-Family Detached Housing | 210 | 87 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 821 | 16 | 48 | 54 | 32 | 23 | 2 | 4 | 1 | 0 | 87 | 0 | 1 | 4 | 2 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 711 | 14 | 43 | 49 | 30 |
| 3 | The Meadows Fil 2 | Single-Family Detached Housing | 210 | 109 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 1,029 | 20 | 60 | 68 | 40 | 29 | 3 | 5 | 1 | 1 | 110 | 1 | 2 | 5 | 3 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 890 | 16 | 53 | 62 | 36 |
| 13 | Allegiant Fil 1 | Single-Family Detached Housing | 210 | 97 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 916 | 18 | 54 | 60 | 36 | 26 | 2 | 5 | 1 | 1 | 98 | 0 | 2 | 5 | 2 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 792 | 16 | 47 | 54 | 33 |
| 5 | Buffalo Crossing | Single-Family Detached Housing | 210 | 204 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 1,926 | 38 | 113 | 127 | 75 | 54 | 5 | 10 | 2 | 1 | 205 | 1 | 3 | 10 | 5 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 1,667 | 32 | 100 | 115 | 69 |
| | Townhomes at Lorson Ranch | Multifamily Housing | 210 | 46 | DU | 7.32 | 0.11 | 0.35 | 0.35 | 0.21 | 337 | 5 | 16 | 16 | 10 | 10 | 1 | 2 | 0 | 0 | 36 | 0 | 1 | 2 | 1 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 291 | 4 | 13 | 14 | 9 |
| 6 | Pioneer Landing | Single-Family Detached Housing | 210 | 59 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 557 | 11 | 33 | 37 | 22 | 16 | 1 | 3 | 1 | 0 | 59 | 0 | 1 | 3 | 1 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 482 | 10 | 29 | 33 | 21 |
| 7 | Pioneer Landing | Single-Family Detached Housing | 210 | 59 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 557 | 11 | 33 | 37 | 22 | 16 | 1 | 3 | 1 | 0 | 59 | 0 | 1 | 3 | 1 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 482 | 10 | 29 | 33 | 21 |
| 15 | Meadows Future Fil 4 West | Single-Family Detached Housing | 210 | 110 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 1,038 | 20 | 61 | 69 | 40 | 29 | 3 | 5 | 1 | 1 | 111 | 1 | 2 | 5 | 3 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 898 | 16 | 54 | 63 | 36 |
| 16 | Meadows Future Fil 4 East | Single-Family Detached Housing | 210 | 126 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 1,189 | 23 | 70 | 79 | 46 | 34 | 3 | 6 | 1 | 1 | 127 | 1 | 2 | 6 | 3 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 1,028 | 19 | 62 | 72 | 42 |
| 18 | Ponderosa Future Fil | Multifamily Housing | 210 | 149 | DU | 7.32 | 0.11 | 0.35 | 0.35 | 0.21 | 1,091 | 16 | 53 | 53 | 31 | 31 | 3 | 5 | 1 | 1 | 116 | 1 | 2 | 6 | 3 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 944 | 12 | 46 | 46 | 27 |
| 39 | Pioneer Landing Fil 2 | Single-Family Detached Housing | 210 | 170 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 1,605 | 31 | 94 | 106 | 62 | 45 | 4 | 8 | 2 | 1 | 171 | 1 | 3 | 8 | 4 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 1,389 | 26 | 83 | 96 | 57 |
| Total All Residential "Between the Creeks" | | | 1,568 | DU | | | | | | | 14,389 | 274 | 831 | 926 | 545 | 407 | 35 | 73 | 16 | 10 | 1,534 | 6 | 27 | 75 | 35 | | | | | | | 12,448 | 233 | 731 | 835 | 500 | |
| Residential Adjacent to Marksheffel | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 1 | Carriage Meadows North | Single-Family Detached Housing | 210 | 155 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 1,463 | 29 | 86 | 97 | 57 | 41 | 4 | 7 | 2 | 1 | 156 | 1 | 2 | 8 | 4 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 1,266 | 24 | 77 | 87 | 52 |
| 147 | Future Multifamily | Multifamily Housing | 210 | 56 | DU | 7.32 | 0.11 | 0.35 | 0.35 | 0.21 | 410 | 6 | 20 | 20 | 12 | 12 | 1 | 2 | 0 | 0 | 44 | 0 | 1 | 2 | 1 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 354 | 5 | 17 | 18 | 11 |
| 47 | Carriage Meadows South | Single-Family Detached Housing | 210 | 86 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 812 | 16 | 48 | 54 | 32 | 23 | 2 | 4 | 1 | 0 | 87 | 0 | 1 | 4 | 2 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 702 | 14 | 43 | 49 | 30 |
| 247 | Carriage Meadows South | Single-Family Detached Housing | 210 | 51 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 481 | 9 | 28 | 32 | 19 | 14 | 1 | 2 | 1 | 0 | 51 | 0 | 1 | 3 | 1 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 416 | 8 | 25 | 28 | 18 |
| 347 | Carriage Meadows South | Single-Family Detached Housing | 210 | 97 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 916 | 18 | 54 | 60 | 36 | 26 | 2 | 5 | 1 | 1 | 98 | 0 | 2 | 5 | 2 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 792 | 16 | 47 | 54 | 33 |
| Total All Residential Adjacent to Marksheffel | | | 445 | DU | | | | | | | 4,082 | 78 | 236 | 263 | 156 | 116 | 10 | 20 | 5 | 2 | 436 | 1 | 7 | 22 | 10 | | | | | | 3,530 | 67 | 209 | 236 | 144 | | |
| Total All Residential "Between the Creeks" and Adjacent to Marksheffel | | | 2,013 | DU | | | | | | | 18,471 | 352 | 1,067 | 1,189 | 701 | 523 | 45 | 93 | 21 | 12 | 1,970 | 7 | 34 | 97 | 45 | | | | | | 15,978 | 300 | 940 | 1,071 | 644 | | |
| Lorson Ranch East | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 42 | North of Fontaine | Single-Family Detached Housing | 210 | 277 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 2,615 | 51 | 154 | 173 | 101 | 74 | 6 | 13 | 3 | 2 | 279 | 1 | 4 | 14 | 6 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 2,262 | 44 | 137 | 156 | 93 |
| 37 | East of Lamprey | Single-Family Detached Housing | 210 | 122 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 1,152 | 23 | 68 | 76 | 45 | 33 | 3 | 6 | 1 | 1 | 123 | 1 | 2 | 6 | 3 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 996 | 19 | 60 | 69 | 41 |
| 27 | West of Lamprey | Single-Family Detached Housing | 210 | 303 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 2,860 | 56 | 168 | 189 | 111 | 81 | 7 | 14 | 3 | 2 | 305 | 1 | 5 | 15 | 7 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 2,474 | 48 | 149 | 171 | 102 |
| 127 | South of Lorson - West | Single-Family Detached Housing | 210 | 76 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 717 | 14 | 42 | 47 | 28 | 20 | 2 | 4 | 1 | 0 | 76 | 0 | 1 | 4 | 2 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 621 | 12 | 37 | 42 | 26 |
| 227 | South of Lorson - East | Single-Family Detached Housing | 210 | 48 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 453 | 9 | 27 | 30 | 18 | 13 | 1 | 2 | 0 | 0 | 48 | 0 | 1 | 2 | 1 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 392 | 8 | 24 | 28 | 17 |
| Total Lorson Ranch East | | | 826 | DU | | | | | | | 7,797 | 153 | 459 | 515 | 303 | 221 | 19 | 39 | 8 | 5 | 831 | 3 | 13 | 41 | 19 | | | | | | 6,745 | 131 | 407 | 466 | 279 | | |
| Total All Residential "Between the Creeks", Adjacent to Marksheffel & Lorson Ranch East | | | 2,839 | DU | | | | | | | 26,268 | 505 | 1,526 | 1,704 | 1,004 | 744 | 64 | 132 | 29 | 17 | 2,801 | 10 | 47 | 138 | 64 | | | | | | 22,723 | 364 | 1,138 | 1,301 | 779 | | |
| Creekside at Lorson Ranch Filing No. 1 | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 26 | Creekside East | Single-Family Detached Housing | 210 | 97 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 916 | 18 | 54 | 60 | 36 | 26 | 2 | 5 | 1 | 1 | 98 | 0 | 2 | 5 | 2 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 792 | 16 | 47 | 54 | 33 |
| 126 | Creekside West | Single-Family Detached Housing | 210 | 138 | DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 1,303 | 26 | 77 | 86 | 51 | 37 | 3 | 7 | 1 | 1 | 139 | 1 | 2 | 7 | 3 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 1,127 | 22 | 68 | 78 | 47 |
| Creekside at Lorson Ranch Filing No. 1 | | | 235 | DU | | | | | | | 2,219 | 44 | 131 | 146 | 87 | 63 | 5 | 12 | 2 | 2 | 237 | 1 | 4 | 12 | 5 | | | | | | 1,919 | 38 | 115 | 132 | 80 | | |
| Short-Term Total | | | 3,074 | DU | | | | | | | 28,487 | 549 | 1,657 | 1,850 | 1,091 | 807 | 69 | 144 | 31 | 19 | 3,038 | 11 | 51 | 150 | 69 | | | | | | 24,642 | 469 | 1,462 | 1,669 | 1,003 | | |
| All Other Future Residential West of the Power Line | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 43 | North of Fontaine and South of Lamprey | Multifamily Housing | 210 | 176 | DU | 7.32 | 0.11 | 0.35 | 0.35 | 0.21 | 1,288 | 19 | 62 | 62 | 36 | 36 | 3 | 6 | 1 | 1 | 137 | 1 | 2 | 7 | 3 | 0% | 0 | 0 | 0 | 0 | 0 | 0 | 1,115 | 15 | 54 | 54 | 32 |
| 45 | North of Fontaine and NE Lamprey/Lorson | Multifamily Housing | 210 | 123 | DU</ | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |



FUTURE FULL MOVEMENT INTERSECTION

FUTURE FULL MOVEMENT INTERSECTION

County Zoning: RR-0.5
Existing Land Use: Residential

City of Fountain Zoning: PUD
Existing Land Use: Vacant/
Closed Golf Course

City of Fountain Zoning: PUD
Existing Land Use: Vacant/
Closed Golf Course

City of Fountain Zoning: Residential
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

County Zoning: RS-6000
Existing Land Use: Vacant

County Zoning: PUD
Existing Land Use: Residential

County Zoning: PUD
Existing Land Use: Vacant/
Closed Golf Course

County Zoning: Residential
Existing Land Use: Residential

County Zoning: Residential
Existing Land Use: Residential

County Zoning: Residential
Existing Land Use: Residential

County Zoning: Residential
Existing Land Use: Residential

County Zoning: Residential
Existing Land Use: Residential

County Zoning: Residential
Existing Land Use: Residential

County Zoning: Residential
Existing Land Use: Residential

County Zoning: RR-0.5
Existing Land Use: Residential

City of Fountain Zoning: PUD
Existing Land Use: Vacant/
Closed Golf Course

City of Fountain Zoning: PUD
Existing Land Use: Vacant/
Closed Golf Course

City of Fountain Zoning: Residential
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

Peaceful Valley County Zoning: RR-5
Existing Land Use: Residential

COUNTER MEASURES INC.

1889 YORK STREET
DENVER, COLORADO
303-333-7409

File Name : Marksheffel Rd - Fontaine Blvd AM
Site Code : 00174850
Start Date : 3/1/2018
Page No : 1

N/S STREET:
E/W STREET:
CITY:
COUNTY:

Groups Printed- VEHICLES

| Start Time | Marksheffel Rd Southbound | | | | Fontaine Blvd Westbound | | | | Marksheffel Rd Northbound | | | | Fontaine Blvd Eastbound | | | | Int. Total |
|-------------|---------------------------|------|-------|------|-------------------------|------|-------|------|---------------------------|------|-------|------|-------------------------|------|-------|------|------------|
| | Left | Thru | Right | Peds | Left | Thru | Right | Peds | Left | Thru | Right | Peds | Left | Thru | Right | Peds | |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | |
| 06:30 AM | 5 | 30 | 4 | 0 | 21 | 74 | 38 | 0 | 10 | 80 | 7 | 0 | 8 | 18 | 6 | 0 | 301 |
| 06:45 AM | 7 | 37 | 2 | 0 | 15 | 104 | 45 | 0 | 8 | 72 | 8 | 0 | 4 | 22 | 6 | 0 | 330 |
| Total | 12 | 67 | 6 | 0 | 36 | 178 | 83 | 0 | 18 | 152 | 15 | 0 | 12 | 40 | 12 | 0 | 631 |
| 07:00 AM | 9 | 28 | 4 | 0 | 20 | 86 | 65 | 0 | 12 | 96 | 11 | 0 | 15 | 18 | 8 | 0 | 372 |
| 07:15 AM | 14 | 32 | 7 | 0 | 12 | 84 | 45 | 0 | 9 | 74 | 11 | 0 | 5 | 29 | 12 | 0 | 334 |
| 07:30 AM | 15 | 40 | 7 | 0 | 20 | 50 | 40 | 0 | 14 | 74 | 8 | 0 | 7 | 25 | 7 | 0 | 307 |
| 07:45 AM | 14 | 20 | 2 | 0 | 13 | 59 | 25 | 0 | 5 | 42 | 12 | 0 | 7 | 38 | 5 | 0 | 242 |
| Total | 52 | 120 | 20 | 0 | 65 | 279 | 175 | 0 | 40 | 286 | 42 | 0 | 34 | 110 | 32 | 0 | 1255 |
| 08:00 AM | 13 | 37 | 2 | 0 | 20 | 93 | 38 | 0 | 8 | 53 | 10 | 0 | 6 | 32 | 3 | 0 | 315 |
| 08:15 AM | 6 | 34 | 4 | 0 | 18 | 96 | 23 | 0 | 12 | 39 | 6 | 0 | 5 | 22 | 9 | 0 | 274 |
| Grand Total | 83 | 258 | 32 | 0 | 139 | 646 | 319 | 0 | 78 | 530 | 73 | 0 | 57 | 204 | 56 | 0 | 2475 |
| Apprch % | 22.3 | 69.2 | 8.6 | 0.0 | 12.6 | 58.5 | 28.9 | 0.0 | 11.5 | 77.8 | 10.7 | 0.0 | 18.0 | 64.4 | 17.7 | 0.0 | |
| Total % | 3.4 | 10.4 | 1.3 | 0.0 | 5.6 | 26.1 | 12.9 | 0.0 | 3.2 | 21.4 | 2.9 | 0.0 | 2.3 | 8.2 | 2.3 | 0.0 | |

COUNTER MEASURES INC.

1889 YORK STREET
DENVER, COLORADO
303-333-7409

File Name : Marksheffel Rd - Fontaine Blvd PM
Site Code : 00174850
Start Date : 3/1/2018
Page No : 1

N/S STREET:
E/W STREET:
CITY:
COUNTY:

Groups Printed- VEHICLES

| Start Time | Marksheffel Rd Southbound | | | | Fontaine Blvd Westbound | | | | Marksheffel Rd Northbound | | | | Marksheffel Blvd Eastbound | | | | Int. Total |
|-------------|---------------------------|------|-------|------|-------------------------|------|-------|------|---------------------------|------|-------|------|----------------------------|------|-------|------|------------|
| | Left | Thru | Right | Peds | Left | Thru | Right | Peds | Left | Thru | Right | Peds | Left | Thru | Right | Peds | |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | |
| 04:00 PM | 31 | 70 | 8 | 0 | 5 | 35 | 16 | 0 | 4 | 46 | 19 | 0 | 11 | 68 | 12 | 0 | 325 |
| 04:15 PM | 42 | 74 | 11 | 0 | 4 | 40 | 19 | 0 | 4 | 51 | 21 | 0 | 10 | 77 | 10 | 0 | 363 |
| 04:30 PM | 35 | 59 | 12 | 0 | 8 | 45 | 20 | 0 | 11 | 46 | 23 | 0 | 10 | 70 | 14 | 0 | 353 |
| 04:45 PM | 30 | 67 | 15 | 0 | 6 | 34 | 14 | 0 | 4 | 35 | 34 | 0 | 13 | 72 | 8 | 0 | 332 |
| Total | 138 | 270 | 46 | 0 | 23 | 154 | 69 | 0 | 23 | 178 | 97 | 0 | 44 | 287 | 44 | 0 | 1373 |
| 05:00 PM | 27 | 54 | 8 | 0 | 6 | 40 | 22 | 0 | 4 | 37 | 35 | 0 | 6 | 54 | 18 | 0 | 311 |
| 05:15 PM | 30 | 60 | 4 | 0 | 8 | 44 | 22 | 0 | 7 | 42 | 26 | 0 | 12 | 76 | 12 | 0 | 343 |
| 05:30 PM | 33 | 65 | 6 | 0 | 9 | 42 | 25 | 0 | 9 | 41 | 23 | 0 | 4 | 103 | 10 | 0 | 370 |
| 05:45 PM | 37 | 53 | 5 | 0 | 13 | 59 | 20 | 0 | 7 | 36 | 37 | 0 | 9 | 71 | 6 | 0 | 353 |
| Total | 127 | 232 | 23 | 0 | 36 | 185 | 89 | 0 | 27 | 156 | 121 | 0 | 31 | 304 | 46 | 0 | 1377 |
| Grand Total | 265 | 502 | 69 | 0 | 59 | 339 | 158 | 0 | 50 | 334 | 218 | 0 | 75 | 591 | 90 | 0 | 2750 |
| Apprch % | 31.7 | 60.0 | 8.3 | 0.0 | 10.6 | 61.0 | 28.4 | 0.0 | 8.3 | 55.5 | 36.2 | 0.0 | 9.9 | 78.2 | 11.9 | 0.0 | |
| Total % | 9.6 | 18.3 | 2.5 | 0.0 | 2.1 | 12.3 | 5.7 | 0.0 | 1.8 | 12.1 | 7.9 | 0.0 | 2.7 | 21.5 | 3.3 | 0.0 | |

Timings
1: Marksheffel Rd & Fontaine Blvd

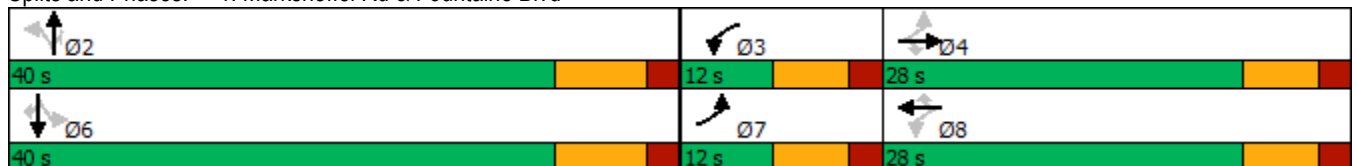
Existing Traffic
AM Peak Hour

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 31 | 94 | 33 | 67 | 324 | 195 | 43 | 316 | 38 | 45 | 137 | 20 |
| Future Volume (vph) | 31 | 94 | 33 | 67 | 324 | 195 | 43 | 316 | 38 | 45 | 137 | 20 |
| Turn Type | pm+pt | NA | Perm | pm+pt | NA | Perm | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases | 7 | 4 | | 3 | 8 | | | 2 | | | 6 | |
| Permitted Phases | 4 | | 4 | 8 | | 8 | 2 | | 2 | 6 | | 6 |
| Detector Phase | 7 | 4 | 4 | 3 | 8 | 8 | 2 | 2 | 2 | 6 | 6 | 6 |
| Switch Phase | | | | | | | | | | | | |
| Minimum Initial (s) | 5.0 | 20.0 | 20.0 | 5.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 |
| Minimum Split (s) | 11.5 | 26.5 | 26.5 | 11.5 | 26.5 | 26.5 | 27.5 | 27.5 | 27.5 | 27.5 | 27.5 | 27.5 |
| Total Split (s) | 12.0 | 28.0 | 28.0 | 12.0 | 28.0 | 28.0 | 40.0 | 40.0 | 40.0 | 40.0 | 40.0 | 40.0 |
| Total Split (%) | 15.0% | 35.0% | 35.0% | 15.0% | 35.0% | 35.0% | 50.0% | 50.0% | 50.0% | 50.0% | 50.0% | 50.0% |
| Yellow Time (s) | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 6.5 | 6.5 | 6.5 | 6.5 | 6.5 | 6.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead/Lag | Lead | Lag | Lag | Lead | Lag | Lag | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | Yes | Yes | | | | | | |
| Recall Mode | None | None | None | None | None | None | None | None | None | None | None | None |
| Act Effct Green (s) | 24.4 | 20.2 | 20.2 | 27.0 | 24.9 | 24.9 | 21.8 | 21.8 | 21.8 | 21.8 | 21.8 | 21.8 |
| Actuated g/C Ratio | 0.37 | 0.31 | 0.31 | 0.41 | 0.38 | 0.38 | 0.33 | 0.33 | 0.33 | 0.33 | 0.33 | 0.33 |
| v/c Ratio | 0.07 | 0.09 | 0.06 | 0.15 | 0.28 | 0.30 | 0.13 | 0.61 | 0.07 | 0.18 | 0.22 | 0.03 |
| Control Delay | 11.2 | 18.3 | 0.2 | 11.6 | 16.5 | 4.6 | 17.0 | 24.0 | 0.2 | 18.5 | 17.7 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 11.2 | 18.3 | 0.2 | 11.6 | 16.5 | 4.6 | 17.0 | 24.0 | 0.2 | 18.5 | 17.7 | 0.1 |
| LOS | B | B | A | B | B | A | B | C | A | B | B | A |
| Approach Delay | | 13.1 | | | 12.0 | | | 21.0 | | | 16.1 | |
| Approach LOS | | B | | | B | | | C | | | B | |

Intersection Summary

Cycle Length: 80
 Actuated Cycle Length: 65.3
 Natural Cycle: 70
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.61
 Intersection Signal Delay: 15.5
 Intersection LOS: B
 Intersection Capacity Utilization 74.8%
 ICU Level of Service D
 Analysis Period (min) 15

Splits and Phases: 1: Marksheffel Rd & Fontaine Blvd



Timings

1: Marksheffel Rd & Fontaine Blvd

Existing Traffic

PM Peak Hour

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 31 | 304 | 46 | 36 | 185 | 89 | 27 | 156 | 121 | 127 | 232 | 23 |
| Future Volume (vph) | 31 | 304 | 46 | 36 | 185 | 89 | 27 | 156 | 121 | 127 | 232 | 23 |
| Turn Type | pm+pt | NA | Perm | pm+pt | NA | Perm | Perm | NA | Perm | Perm | NA | Perm |
| Protected Phases | 7 | 4 | | 3 | 8 | | | 2 | | | 6 | |
| Permitted Phases | 4 | | 4 | 8 | | 8 | 2 | | 2 | 6 | | 6 |
| Detector Phase | 7 | 4 | 4 | 3 | 8 | 8 | 2 | 2 | 2 | 6 | 6 | 6 |
| Switch Phase | | | | | | | | | | | | |
| Minimum Initial (s) | 5.0 | 20.0 | 20.0 | 5.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 | 20.0 |
| Minimum Split (s) | 11.5 | 26.5 | 26.5 | 11.5 | 26.5 | 26.5 | 27.5 | 27.5 | 27.5 | 27.5 | 27.5 | 27.5 |
| Total Split (s) | 12.0 | 28.0 | 28.0 | 12.0 | 28.0 | 28.0 | 40.0 | 40.0 | 40.0 | 40.0 | 40.0 | 40.0 |
| Total Split (%) | 15.0% | 35.0% | 35.0% | 15.0% | 35.0% | 35.0% | 50.0% | 50.0% | 50.0% | 50.0% | 50.0% | 50.0% |
| Yellow Time (s) | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 4.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 | 5.5 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 6.5 | 6.5 | 6.5 | 6.5 | 6.5 | 6.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 | 7.5 |
| Lead/Lag | Lead | Lag | Lag | Lead | Lag | Lag | | | | | | |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | Yes | Yes | | | | | | |
| Recall Mode | None | None | None | None | None | None | None | None | None | None | None | None |
| Act Effct Green (s) | 22.2 | 20.2 | 20.2 | 22.2 | 20.2 | 20.2 | 20.2 | 20.2 | 20.2 | 20.2 | 20.2 | 20.2 |
| Actuated g/C Ratio | 0.38 | 0.34 | 0.34 | 0.38 | 0.34 | 0.34 | 0.34 | 0.34 | 0.34 | 0.34 | 0.34 | 0.34 |
| v/c Ratio | 0.08 | 0.31 | 0.09 | 0.08 | 0.15 | 0.14 | 0.07 | 0.24 | 0.19 | 0.33 | 0.39 | 0.04 |
| Control Delay | 10.1 | 16.2 | 0.3 | 10.2 | 15.2 | 1.5 | 16.1 | 16.8 | 4.0 | 18.9 | 18.4 | 0.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 10.1 | 16.2 | 0.3 | 10.2 | 15.2 | 1.5 | 16.1 | 16.8 | 4.0 | 18.9 | 18.4 | 0.1 |
| LOS | B | B | A | B | B | A | B | B | A | B | B | A |
| Approach Delay | | 13.8 | | | 10.7 | | | 11.6 | | | 17.5 | |
| Approach LOS | | B | | | B | | | B | | | B | |

Intersection Summary

Cycle Length: 80

Actuated Cycle Length: 58.8

Natural Cycle: 70

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.39

Intersection Signal Delay: 13.7

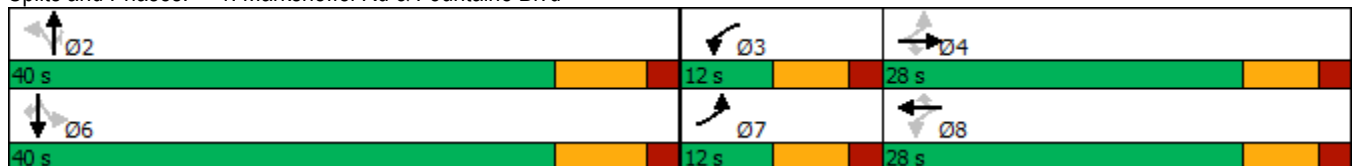
Intersection LOS: B

Intersection Capacity Utilization 77.5%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: Marksheffel Rd & Fontaine Blvd



Timings

2040 Total Traffic

1: Marksheffel Rd & Fountaine Blvd

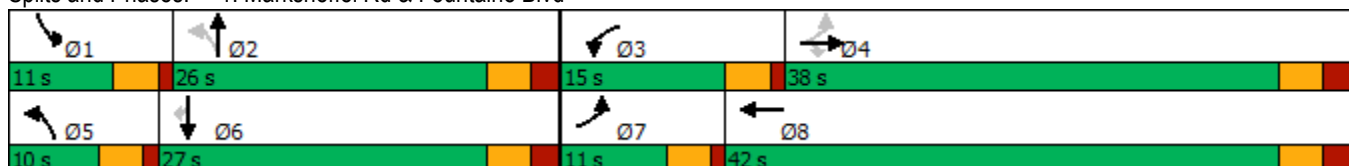
AM Peak Hour

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------|-------|-------|-------|-------|-------|------|-------|-------|------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 38 | 354 | 48 | 442 | 1024 | 606 | 149 | 527 | 178 | 242 | 516 | 45 |
| Future Volume (vph) | 38 | 354 | 48 | 442 | 1024 | 606 | 149 | 527 | 178 | 242 | 516 | 45 |
| Turn Type | pm+pt | NA | Perm | Prot | NA | Free | pm+pt | NA | Free | Prot | NA | Perm |
| Protected Phases | 7 | 4 | | 3 | 8 | | 5 | 2 | | 1 | 6 | |
| Permitted Phases | 4 | | 4 | | | Free | 2 | | Free | | | 6 |
| Detector Phase | 7 | 4 | 4 | 3 | 8 | | 5 | 2 | | 1 | 6 | 6 |
| Switch Phase | | | | | | | | | | | | |
| Minimum Initial (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 |
| Minimum Split (s) | 9.0 | 9.0 | 9.0 | 9.0 | 9.0 | | 9.0 | 9.0 | | 9.0 | 9.0 | 9.0 |
| Total Split (s) | 11.0 | 38.0 | 38.0 | 15.0 | 42.0 | | 10.0 | 26.0 | | 11.0 | 27.0 | 27.0 |
| Total Split (%) | 12.2% | 42.2% | 42.2% | 16.7% | 46.7% | | 11.1% | 28.9% | | 12.2% | 30.0% | 30.0% |
| Yellow Time (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 |
| All-Red Time (s) | 1.0 | 2.0 | 2.0 | 1.0 | 2.0 | | 1.0 | 2.0 | | 1.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 4.0 | 5.0 | 5.0 | 4.0 | 5.0 | | 4.0 | 5.0 | | 4.0 | 5.0 | 5.0 |
| Lead/Lag | Lead | Lag | Lag | Lead | Lag | | Lead | Lag | | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | Yes | | Yes | Yes | | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | | None | None | | None | None | None |
| Act Effct Green (s) | 28.9 | 21.3 | 21.3 | 11.5 | 31.2 | 75.6 | 24.1 | 16.8 | 75.6 | 7.3 | 17.8 | 17.8 |
| Actuated g/C Ratio | 0.38 | 0.28 | 0.28 | 0.15 | 0.41 | 1.00 | 0.32 | 0.22 | 1.00 | 0.10 | 0.24 | 0.24 |
| v/c Ratio | 0.16 | 0.36 | 0.10 | 0.87 | 0.74 | 0.40 | 0.55 | 0.71 | 0.12 | 0.75 | 0.65 | 0.10 |
| Control Delay | 11.5 | 21.9 | 0.4 | 53.9 | 23.5 | 0.8 | 27.8 | 34.0 | 0.2 | 52.9 | 31.5 | 0.4 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 11.5 | 21.9 | 0.4 | 53.9 | 23.5 | 0.8 | 27.8 | 34.0 | 0.2 | 52.9 | 31.5 | 0.4 |
| LOS | B | C | A | D | C | A | C | C | A | D | C | A |
| Approach Delay | | 18.5 | | | 23.1 | | | 25.9 | | | 36.1 | |
| Approach LOS | | B | | | C | | | C | | | D | |

Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 75.6
 Natural Cycle: 60
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 0.87
 Intersection Signal Delay: 25.7
 Intersection LOS: C
 Intersection Capacity Utilization 69.2%
 ICU Level of Service C
 Analysis Period (min) 15

Splits and Phases: 1: Marksheffel Rd & Fountaine Blvd



Timings
5: Marksheffel Rd & Lorson Blvd

2040 Total Traffic
AM Peak Hour

| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------|-------|-------|-------|------|-------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 468 | 122 | 732 | 150 | 28 | 978 |
| Future Volume (vph) | 468 | 122 | 732 | 150 | 28 | 978 |
| Turn Type | Prot | Perm | NA | Free | pm+pt | NA |
| Protected Phases | 8 | | 2 | | 1 | 6 |
| Permitted Phases | | 8 | | Free | 6 | |
| Detector Phase | 8 | 8 | 2 | | 1 | 6 |
| Switch Phase | | | | | | |
| Minimum Initial (s) | 4.0 | 4.0 | 4.0 | | 4.0 | 4.0 |
| Minimum Split (s) | 20.0 | 20.0 | 20.0 | | 9.0 | 20.0 |
| Total Split (s) | 20.0 | 20.0 | 60.0 | | 10.0 | 70.0 |
| Total Split (%) | 22.2% | 22.2% | 66.7% | | 11.1% | 77.8% |
| Yellow Time (s) | 3.0 | 3.0 | 3.0 | | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | | 5.0 | 5.0 |
| Lead/Lag | | | Lag | | Lead | |
| Lead-Lag Optimize? | | | Yes | | Yes | |
| Recall Mode | None | None | None | | None | None |
| Act Effct Green (s) | 12.4 | 12.4 | 20.1 | 44.4 | 21.7 | 21.7 |
| Actuated g/C Ratio | 0.28 | 0.28 | 0.45 | 1.00 | 0.49 | 0.49 |
| v/c Ratio | 0.51 | 0.24 | 0.48 | 0.10 | 0.08 | 0.64 |
| Control Delay | 16.8 | 5.2 | 10.5 | 0.1 | 6.5 | 10.6 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 16.8 | 5.2 | 10.5 | 0.1 | 6.5 | 10.6 |
| LOS | B | A | B | A | A | B |
| Approach Delay | 14.4 | | 8.8 | | | 10.5 |
| Approach LOS | B | | A | | | B |

Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 44.4
 Natural Cycle: 50
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.64
 Intersection Signal Delay: 10.8
 Intersection Capacity Utilization 48.7%
 Analysis Period (min) 15
 Intersection LOS: B
 ICU Level of Service A

Splits and Phases: 5: Marksheffel Rd & Lorson Blvd



HCM 6th TWSC
 6: Marksheffel Rd & Peaceful Ridge Access/North Lorson Access

2040 Total Traffic
 AM Peak Hour

| Intersection | | | | | | | | | | | | |
|--------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Int Delay, s/veh | 0.4 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | | ↗ | | | ↗ | | ↑↑↑ | ↗ | | ↑↑↑ | ↗ |
| Traffic Vol, veh/h | 0 | 0 | 40 | 0 | 0 | 24 | 0 | 1166 | 5 | 0 | 763 | 20 |
| Future Vol, veh/h | 0 | 0 | 40 | 0 | 0 | 24 | 0 | 1166 | 5 | 0 | 763 | 20 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | 0 | - | - | 0 | - | - | 250 | - | - | 250 |
| Veh in Median Storage, # | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 0 | 42 | 0 | 0 | 25 | 0 | 1227 | 5 | 0 | 803 | 21 |

| Major/Minor | Minor2 | | Minor1 | | Major1 | | Major2 | |
|----------------------|--------|---|--------|---|--------|------|--------|---|
| Conflicting Flow All | - | - | 402 | - | - | 614 | - | 0 |
| Stage 1 | - | - | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - | - | - |
| Critical Hdwy | - | - | 7.14 | - | - | 7.14 | - | - |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | - | - |
| Follow-up Hdwy | - | - | 3.92 | - | - | 3.92 | - | - |
| Pot Cap-1 Maneuver | 0 | 0 | 511 | 0 | 0 | 373 | 0 | - |
| Stage 1 | 0 | 0 | - | 0 | 0 | - | 0 | - |
| Stage 2 | 0 | 0 | - | 0 | 0 | - | 0 | - |
| Platoon blocked, % | - | - | - | - | - | - | - | - |
| Mov Cap-1 Maneuver | - | - | 511 | - | - | 373 | - | - |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | - | - |
| Stage 1 | - | - | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - | - | - |

| Approach | EB | WB | NB | SB |
|----------------------|------|------|----|----|
| HCM Control Delay, s | 12.7 | 15.4 | 0 | 0 |
| HCM LOS | B | C | | |

| Minor Lane/Major Mvmt | NBT | NBR | EBLn1WBLn1 | SBT | SBR |
|-----------------------|-----|-----|-------------|-----|-----|
| Capacity (veh/h) | - | - | 511 373 | - | - |
| HCM Lane V/C Ratio | - | - | 0.082 0.068 | - | - |
| HCM Control Delay (s) | - | - | 12.7 15.4 | - | - |
| HCM Lane LOS | - | - | B C | - | - |
| HCM 95th %tile Q(veh) | - | - | 0.3 0.2 | - | - |

Timings

2040 Total Traffic

1: Marksheffel Rd & Fountaine Blvd

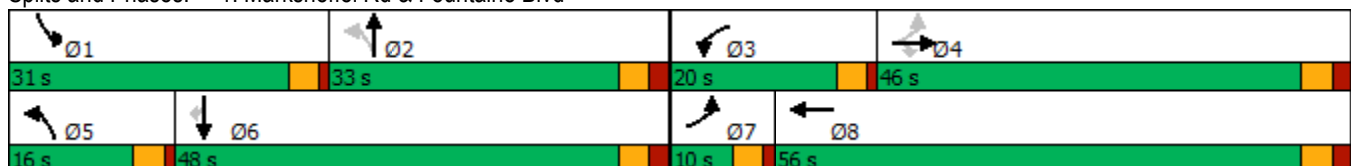
PM Peak Hour

| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | | | | | | | |
| Traffic Volume (vph) | 70 | 1186 | 143 | 338 | 707 | 458 | 116 | 222 | 542 | 774 | 310 | 65 |
| Future Volume (vph) | 70 | 1186 | 143 | 338 | 707 | 458 | 116 | 222 | 542 | 774 | 310 | 65 |
| Turn Type | pm+pt | NA | Perm | Prot | NA | Free | pm+pt | NA | Free | Prot | NA | Perm |
| Protected Phases | 7 | 4 | | 3 | 8 | | 5 | 2 | | 1 | 6 | |
| Permitted Phases | 4 | | 4 | | | Free | 2 | | Free | | | 6 |
| Detector Phase | 7 | 4 | 4 | 3 | 8 | | 5 | 2 | | 1 | 6 | 6 |
| Switch Phase | | | | | | | | | | | | |
| Minimum Initial (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | | 4.0 | 4.0 | | 4.0 | 4.0 | 4.0 |
| Minimum Split (s) | 9.0 | 9.0 | 9.0 | 9.0 | 9.0 | | 9.0 | 9.0 | | 9.0 | 9.0 | 9.0 |
| Total Split (s) | 10.0 | 46.0 | 46.0 | 20.0 | 56.0 | | 16.0 | 33.0 | | 31.0 | 48.0 | 48.0 |
| Total Split (%) | 7.7% | 35.4% | 35.4% | 15.4% | 43.1% | | 12.3% | 25.4% | | 23.8% | 36.9% | 36.9% |
| Yellow Time (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | | 3.0 | 3.0 | | 3.0 | 3.0 | 3.0 |
| All-Red Time (s) | 1.0 | 2.0 | 2.0 | 1.0 | 2.0 | | 1.0 | 2.0 | | 1.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 | | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 4.0 | 5.0 | 5.0 | 4.0 | 5.0 | | 4.0 | 5.0 | | 4.0 | 5.0 | 5.0 |
| Lead/Lag | Lead | Lag | Lag | Lead | Lag | | Lead | Lag | | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | Yes | | Yes | Yes | | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | | None | None | | None | None | None |
| Act Effct Green (s) | 48.0 | 41.0 | 41.0 | 15.2 | 52.3 | 113.8 | 23.7 | 12.6 | 113.8 | 27.0 | 29.5 | 29.5 |
| Actuated g/C Ratio | 0.42 | 0.36 | 0.36 | 0.13 | 0.46 | 1.00 | 0.21 | 0.11 | 1.00 | 0.24 | 0.26 | 0.26 |
| v/c Ratio | 0.21 | 0.95 | 0.22 | 0.75 | 0.44 | 0.30 | 0.42 | 0.58 | 0.35 | 0.97 | 0.34 | 0.13 |
| Control Delay | 15.6 | 51.8 | 4.7 | 59.2 | 22.8 | 0.5 | 29.5 | 54.4 | 0.6 | 68.7 | 35.9 | 1.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 15.6 | 51.8 | 4.7 | 59.2 | 22.8 | 0.5 | 29.5 | 54.4 | 0.6 | 68.7 | 35.9 | 1.1 |
| LOS | B | D | A | E | C | A | C | D | A | E | D | A |
| Approach Delay | | 45.2 | | | 24.2 | | | 18.0 | | | 56.1 | |
| Approach LOS | | D | | | C | | | B | | | E | |

Intersection Summary

Cycle Length: 130
 Actuated Cycle Length: 113.8
 Natural Cycle: 90
 Control Type: Semi Act-Uncoord
 Maximum v/c Ratio: 0.97
 Intersection Signal Delay: 36.5
 Intersection LOS: D
 Intersection Capacity Utilization 85.6%
 ICU Level of Service E
 Analysis Period (min) 15

Splits and Phases: 1: Marksheffel Rd & Fountaine Blvd



Timings
5: Marksheffel Rd & Lorson Blvd

2040 Total Traffic
PM Peak Hour

| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
|----------------------|-------|-------|-------|-------|-------|-------|
| Lane Configurations | | | | | | |
| Traffic Volume (vph) | 329 | 85 | 795 | 532 | 100 | 691 |
| Future Volume (vph) | 329 | 85 | 795 | 532 | 100 | 691 |
| Turn Type | Prot | Perm | NA | Perm | pm+pt | NA |
| Protected Phases | 8 | | 2 | | 1 | 6 |
| Permitted Phases | | 8 | | 2 | 6 | |
| Detector Phase | 8 | 8 | 2 | 2 | 1 | 6 |
| Switch Phase | | | | | | |
| Minimum Initial (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Split (s) | 20.0 | 20.0 | 20.0 | 20.0 | 9.0 | 20.0 |
| Total Split (s) | 20.0 | 20.0 | 60.0 | 60.0 | 10.0 | 70.0 |
| Total Split (%) | 22.2% | 22.2% | 66.7% | 66.7% | 11.1% | 77.8% |
| Yellow Time (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead/Lag | | | Lag | Lag | Lead | |
| Lead-Lag Optimize? | | | Yes | Yes | Yes | |
| Recall Mode | None | None | None | None | None | None |
| Act Effct Green (s) | 10.8 | 10.8 | 21.8 | 21.8 | 29.1 | 29.1 |
| Actuated g/C Ratio | 0.21 | 0.21 | 0.43 | 0.43 | 0.57 | 0.57 |
| v/c Ratio | 0.48 | 0.22 | 0.55 | 0.56 | 0.30 | 0.39 |
| Control Delay | 22.0 | 7.2 | 12.8 | 3.6 | 6.9 | 6.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 22.0 | 7.2 | 12.8 | 3.6 | 6.9 | 6.2 |
| LOS | C | A | B | A | A | A |
| Approach Delay | 19.0 | | 9.1 | | | 6.3 |
| Approach LOS | B | | A | | | A |

Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 50.7
 Natural Cycle: 50
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.56
 Intersection Signal Delay: 9.8
 Intersection Capacity Utilization 49.4%
 Analysis Period (min) 15
 Intersection LOS: A
 ICU Level of Service A

Splits and Phases: 5: Marksheffel Rd & Lorson Blvd



HCM 6th TWSC
 6: Marksheffel Rd & Peaceful Ridge Access/North Lorson Access

2040 Total Traffic
 PM Peak Hour

| Intersection | | | | | | | | | | | | |
|--------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Int Delay, s/veh | 0.3 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | | ↗ | | | ↗ | | ↑↑↑ | ↗ | | ↑↑↑ | ↗ |
| Traffic Vol, veh/h | 0 | 0 | 25 | 0 | 0 | 16 | 0 | 733 | 17 | 0 | 1124 | 60 |
| Future Vol, veh/h | 0 | 0 | 25 | 0 | 0 | 16 | 0 | 733 | 17 | 0 | 1124 | 60 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | 0 | - | - | 0 | - | - | 250 | - | - | 250 |
| Veh in Median Storage, # | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, % | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 0 | 26 | 0 | 0 | 17 | 0 | 772 | 18 | 0 | 1183 | 63 |

| Major/Minor | Minor2 | | Minor1 | | Major1 | | Major2 | |
|----------------------|--------|---|--------|---|--------|------|--------|---|
| Conflicting Flow All | - | - | 592 | - | - | 386 | - | 0 |
| Stage 1 | - | - | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - | - | - |
| Critical Hdwy | - | - | 7.14 | - | - | 7.14 | - | - |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | - | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | - | - |
| Follow-up Hdwy | - | - | 3.92 | - | - | 3.92 | - | - |
| Pot Cap-1 Maneuver | 0 | 0 | 385 | 0 | 0 | 523 | 0 | - |
| Stage 1 | 0 | 0 | - | 0 | 0 | - | 0 | - |
| Stage 2 | 0 | 0 | - | 0 | 0 | - | 0 | - |
| Platoon blocked, % | - | - | - | - | - | - | - | - |
| Mov Cap-1 Maneuver | - | - | 385 | - | - | 523 | - | - |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | - | - |
| Stage 1 | - | - | - | - | - | - | - | - |
| Stage 2 | - | - | - | - | - | - | - | - |

| Approach | EB | WB | NB | SB |
|----------------------|----|------|----|----|
| HCM Control Delay, s | 15 | 12.1 | 0 | 0 |
| HCM LOS | C | B | | |

| Minor Lane/Major Mvmt | NBT | NBR | EBLn1WBLn1 | SBT | SBR |
|-----------------------|-----|-----|------------|-------|-----|
| Capacity (veh/h) | - | - | 385 | 523 | - |
| HCM Lane V/C Ratio | - | - | 0.068 | 0.032 | - |
| HCM Control Delay (s) | - | - | 15 | 12.1 | - |
| HCM Lane LOS | - | - | C | B | - |
| HCM 95th %tile Q(veh) | - | - | 0.2 | 0.1 | - |

LORSON RANCH SKETCH PLAN AMENDMENT 2

APPENDIX A

MARKSHEFFEL ROAD AND LORSON BOULEVARD TRAFFIC CONTROL ANALYSIS

The future intersection of Marksheffel Road/Lorson Boulevard has been analyzed to determine the projected levels of service for the total traffic volumes based on the signalized method of analysis from Synchro and the unsignalized method of analysis procedures outlined in the *Highway Capacity Manual, 6th Edition* by the Transportation Research Board. The level of service reports are attached. The projected 2040 total traffic volumes and the results of the analysis are shown in Appendix Figure 1.

Unsignalized (Stop-Sign-Controlled) and Signalized Intersection Traffic Control

The westbound left-turn movement at the intersection of Marksheffel/Lorson is projected to operate at LOS F during the morning and afternoon peak hours if this intersection remains a conventional, stop-sign-controlled, full-movement intersection. Assuming a conventional, signal-controlled intersection, all movements are projected to operate at LOS B or better during the peak hours based on the 2040 total traffic volumes.

Alternative Intersection Configuration/Traffic Control

The following are two potential alternatives to a conventional full-movement intersection (stop-sign controlled or signal controlled, for which analysis results are presented in the preceding paragraph). These include modern roundabout and “channelized T” type intersections.

Modern Roundabout Intersection

A modern roundabout intersection at Lorson/Marksheffel (hypothetically) would initially be a single-lane roundabout but would need to be designed to be expandable to a two-lane roundabout.

By 2040 it was assumed that the intersection would be expanded to a two-lane roundabout. Based on the 2040 total traffic volumes the westbound approach is projected to operate at LOS D (25.5 seconds control delay) during the peak hour.

Advantages

- Generally, modern roundabouts have safety advantages over conventional four-leg signal-controlled intersections. This is because crashes tend to be lower speed, there are fewer conflict points, and the types (angles) of crashes tend to be those that generally result in less severe accidents. Granted, as a conventional T intersection (which would be the case until (and if) a fourth leg is added) this intersection would have significantly fewer conflict points than a four-leg conventional intersection.

- A roundabout may be more aesthetically appealing than a traditional signal-controlled intersection and generally lower traffic noise levels.
- Long-term operation and maintenance cost is likely to be lower with a roundabout than a traffic signal.

Disadvantages

- The travel speed through the intersection compared with a signalized intersection during the signal green phase would be slower for through traffic on Marksheffel Road. This may affect travel times along the corridor if, in the future, other Marksheffel intersections to the north and south are controlled by a series of coordinated traffic signals. However, the average intersection delay should be factored into the overall corridor travel time. This analysis may show no overall disadvantage.

Channelized T Intersection

The channelized T type intersection allows for an intersection with generally lower overall and side-street delay than with a conventional T intersection and with fewer stops for the through traffic on the major roadway when compared to a conventional signalized T intersection. An example of a channelized T-type intersection is at the intersection of US Highway 24 and Garrett Road near Falcon (El Paso County). That particular intersection is signalized with a “directional signal,” but a channelized T can also operate as an unsignalized intersection with Stop-sign control on the minor street. Whether signalized or unsignalized, the raised median configuration would allow for “free” (no stopping) movement for the southbound through movement through the intersection. The westbound left turn would cross the northbound lanes and into a channelized southbound left-turn acceleration lane for merging into southbound through traffic.

By 2040 the delay for the westbound left-turn movement is projected to be LOS F during the morning peak hour even with the channelized T. If the channelized T intersection were signalized with a “directional signal,” the delay for the westbound left-turn movement is projected to be 21.3 seconds (LOS C).

Advantages

- The intersection of Marksheffel/Lorson could likely operate at a satisfactory level of service as a Stop-sign-controlled intersection for longer as an unsignalized, channelized T intersection than if it were to remain a conventional T intersection.
- Once signal control would be required to maintain an acceptable level of service, the channelized T configuration would result in lower delay for through traffic especially for the southbound traffic, which would operate freely. The overall intersection delay is projected to be lower with a channelized T intersection. Based on the 2040 total morning peak-hour volumes, the projected overall intersection delay is 7.9 seconds per vehicle (LOS A) with a signal-controlled channelized T intersection and 10.8 seconds per

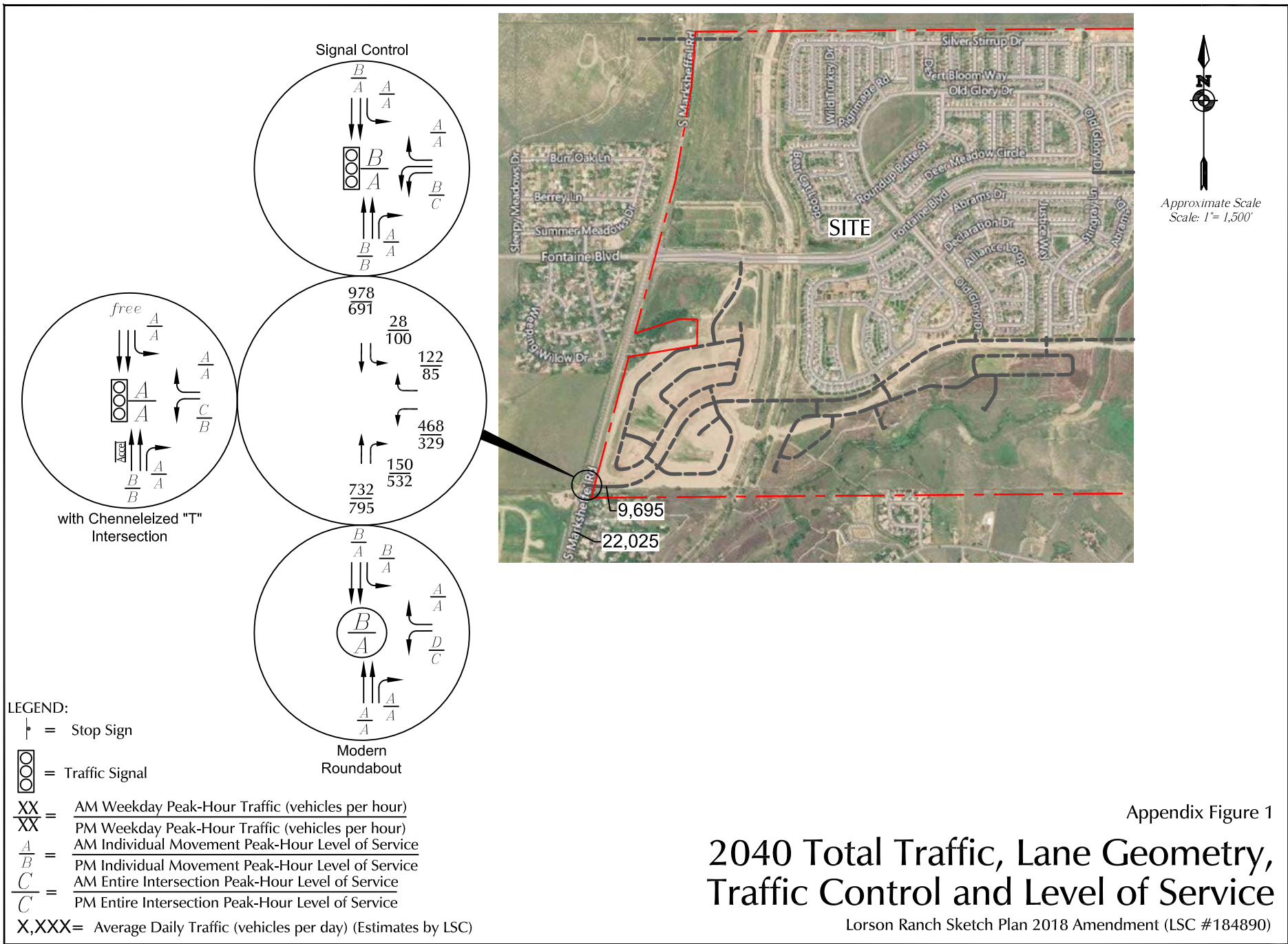
vehicle (LOS B) with a conventional signal-controlled intersection. Based on the 2040 total afternoon peak-hour volumes the projected overall intersection delay is 6.1 seconds per vehicle (LOS A) with a signal-controlled channelized T intersection and 9.8 seconds per vehicle (LOS A) with a conventional signal-controlled intersection.

- There is the potential, depending on the time of day and traffic volumes, to allow for a longer side-street signal phase due to one-way signal progression and no red phase for southbound traffic.

Disadvantages

- The channelized T configuration would only work on an interim basis prior to the addition of a potential fourth leg of this intersection. It is anticipated that development of the Singer property on the west side of Marksheffel Road would result in a request for a full-movement-capable, fourth/west leg of this intersection. If/when that occurs, many of the channelized T improvements would need to be removed or modified.
- The channelized T configuration may be confusing for some drivers and the merging movement into southbound traffic requires a more complex movement than with a signal. However, most motorists entering the intersection from the east would be regular users and would quickly learn to navigate the intersection.
- A channelized T intersection would require the construction of raised channelizing medians on Marksheffel Road and the ongoing maintenance of those medians.
- The section of Marksheffel Road between Lorson Boulevard and Poa Annua would need to be designed to accommodate a southbound left-turn acceleration lane from Lorson Boulevard, a taper, and a southbound left-turn lane approaching Poa Annua. Based on a posted speed limit of 55 mph, the El Paso County *Engineering Criteria Manual* (ECM) requires a 960-foot-long acceleration lane plus a 222-foot taper. Based on a design speed of 60 mph the ECM requires a 290-foot-long left-turn lane approach Poa Annua plus 50 to 75 feet of storage length. The total length of the acceleration lane, taper, and left-turn lane would be between 1,522 and 1,547 feet. The total distance between Lorson Boulevard and Poa Annua street is about 1,025 feet centerline to centerline. The construction of a channelized T intersection would therefore require a deviation(s) to the ECM.
- A channelized T can be more difficult for pedestrians than a conventional signalized intersection. However, there may be ways to better accommodate pedestrians – such as adding a pedestrian-only phase for southbound traffic. More research would be needed regarding pedestrian accommodation.

Enclosures: Figure 1
Level of Service Reports



| Intersection | | | | | | |
|-----------------------------|-------|-------|-------|-------|-------|-------|
| Intersection Delay, s/veh | 12.7 | | | | | |
| Intersection LOS | B | | | | | |
| Approach | WB | | NB | | SB | |
| Entry Lanes | 2 | | 2 | | 2 | |
| Conflicting Circle Lanes | 2 | | 2 | | 2 | |
| Adj Approach Flow, veh/h | 621 | | 929 | | 1058 | |
| Demand Flow Rate, veh/h | 634 | | 947 | | 1080 | |
| Vehicles Circulating, veh/h | 786 | | 30 | | 503 | |
| Vehicles Exiting, veh/h | 191 | | 1553 | | 917 | |
| Ped Vol Crossing Leg, #/h | 0 | | 0 | | 0 | |
| Ped Cap Adj | 1.000 | | 1.000 | | 1.000 | |
| Approach Delay, s/veh | 21.7 | | 5.9 | | 13.4 | |
| Approach LOS | C | | A | | B | |
| Lane | Left | Right | Left | Right | Left | Right |
| Designated Moves | L | TR | LT | TR | LT | TR |
| Assumed Moves | L | TR | LT | TR | LT | TR |
| RT Channelized | | | | | | |
| Lane Util | 0.793 | 0.207 | 0.470 | 0.530 | 0.470 | 0.530 |
| Follow-Up Headway, s | 2.667 | 2.535 | 2.667 | 2.535 | 2.667 | 2.535 |
| Critical Headway, s | 4.645 | 4.328 | 4.645 | 4.328 | 4.645 | 4.328 |
| Entry Flow, veh/h | 503 | 131 | 445 | 502 | 508 | 572 |
| Cap Entry Lane, veh/h | 655 | 728 | 1313 | 1384 | 850 | 926 |
| Entry HV Adj Factor | 0.980 | 0.977 | 0.981 | 0.980 | 0.979 | 0.981 |
| Flow Entry, veh/h | 493 | 128 | 436 | 492 | 497 | 561 |
| Cap Entry, veh/h | 642 | 711 | 1288 | 1357 | 832 | 908 |
| V/C Ratio | 0.768 | 0.180 | 0.339 | 0.363 | 0.598 | 0.618 |
| Control Delay, s/veh | 25.5 | 7.1 | 5.9 | 6.0 | 13.5 | 13.2 |
| LOS | D | A | A | A | B | B |
| 95th %tile Queue, veh | 7 | 1 | 2 | 2 | 4 | 4 |

Timings
5: Marksheffel Rd & Lorson Blvd

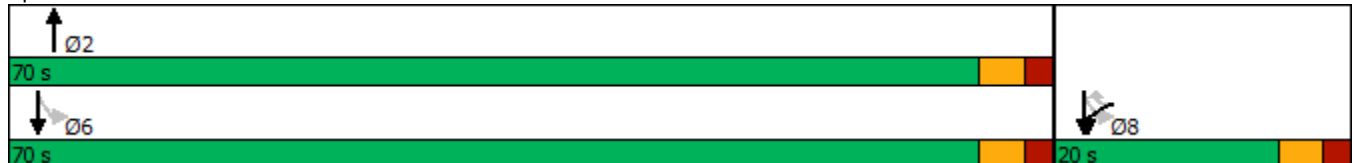
2040 Total Traffic With Signal Controlled Channelized T
AM Peak Hour

| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT | Ø6 |
|----------------------|-------|-------|-------|------|------|------|------|
| Lane Configurations | | | | | | | |
| Traffic Volume (vph) | 468 | 122 | 732 | 150 | 28 | 978 | |
| Future Volume (vph) | 468 | 122 | 732 | 150 | 28 | 978 | |
| Turn Type | Prot | Perm | NA | Free | Perm | NA | |
| Protected Phases | 8! | | 2 | | | 8 6! | 6 |
| Permitted Phases | | 8 | | Free | 8 6! | | |
| Detector Phase | 8 | 8 | 2 | | 8 6 | 8 6 | |
| Switch Phase | | | | | | | |
| Minimum Initial (s) | 4.0 | 4.0 | 4.0 | | | | 4.0 |
| Minimum Split (s) | 20.0 | 20.0 | 20.0 | | | | 20.0 |
| Total Split (s) | 20.0 | 20.0 | 70.0 | | | | 70.0 |
| Total Split (%) | 22.2% | 22.2% | 77.8% | | | | 78% |
| Yellow Time (s) | 3.0 | 3.0 | 3.0 | | | | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | | | | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | | | | |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | | | | |
| Lead/Lag | | | | | | | |
| Lead-Lag Optimize? | | | | | | | |
| Recall Mode | None | None | None | | | | None |
| Act Effct Green (s) | 15.1 | 15.1 | 14.5 | 39.6 | 39.6 | 39.6 | |
| Actuated g/C Ratio | 0.38 | 0.38 | 0.37 | 1.00 | 1.00 | 1.00 | |
| v/c Ratio | 0.73 | 0.19 | 0.60 | 0.10 | 0.05 | 0.31 | |
| Control Delay | 21.3 | 3.5 | 12.2 | 0.1 | 0.2 | 0.3 | |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| Total Delay | 21.3 | 3.5 | 12.2 | 0.1 | 0.2 | 0.3 | |
| LOS | C | A | B | A | A | A | |
| Approach Delay | 17.7 | | 10.1 | | | 0.2 | |
| Approach LOS | B | | B | | | A | |

Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 39.6
 Natural Cycle: 45
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.73
 Intersection Signal Delay: 7.9
 Intersection LOS: A
 Intersection Capacity Utilization 61.3%
 ICU Level of Service B
 Analysis Period (min) 15
 ! Phase conflict between lane groups.

Splits and Phases: 5: Marksheffel Rd & Lorson Blvd



| Intersection | | | | | | |
|-----------------------------|-------|-------|-------|-------|-------|-------|
| Intersection Delay, s/veh | 9.9 | | | | | |
| Intersection LOS | A | | | | | |
| Approach | WB | | NB | | SB | |
| Entry Lanes | 2 | | 2 | | 2 | |
| Conflicting Circle Lanes | 2 | | 2 | | 2 | |
| Adj Approach Flow, veh/h | 435 | | 1397 | | 832 | |
| Demand Flow Rate, veh/h | 444 | | 1425 | | 849 | |
| Vehicles Circulating, veh/h | 854 | | 107 | | 353 | |
| Vehicles Exiting, veh/h | 678 | | 1095 | | 945 | |
| Ped Vol Crossing Leg, #/h | 0 | | 0 | | 0 | |
| Ped Cap Adj | 1.000 | | 1.000 | | 1.000 | |
| Approach Delay, s/veh | 14.6 | | 9.5 | | 8.3 | |
| Approach LOS | B | | A | | A | |
| Lane | Left | Right | Left | Right | Left | Right |
| Designated Moves | L | TR | LT | TR | LT | TR |
| Assumed Moves | L | TR | LT | TR | LT | TR |
| RT Channelized | | | | | | |
| Lane Util | 0.795 | 0.205 | 0.470 | 0.530 | 0.470 | 0.530 |
| Follow-Up Headway, s | 2.667 | 2.535 | 2.667 | 2.535 | 2.667 | 2.535 |
| Critical Headway, s | 4.645 | 4.328 | 4.645 | 4.328 | 4.645 | 4.328 |
| Entry Flow, veh/h | 353 | 91 | 670 | 755 | 399 | 450 |
| Cap Entry Lane, veh/h | 615 | 687 | 1223 | 1297 | 976 | 1052 |
| Entry HV Adj Factor | 0.980 | 0.978 | 0.980 | 0.981 | 0.981 | 0.980 |
| Flow Entry, veh/h | 346 | 89 | 657 | 741 | 391 | 441 |
| Cap Entry, veh/h | 603 | 672 | 1199 | 1272 | 957 | 1031 |
| V/C Ratio | 0.574 | 0.132 | 0.548 | 0.582 | 0.409 | 0.428 |
| Control Delay, s/veh | 16.6 | 6.8 | 9.3 | 9.6 | 8.4 | 8.2 |
| LOS | C | A | A | A | A | A |
| 95th %tile Queue, veh | 4 | 0 | 3 | 4 | 2 | 2 |

Timings
5: Marksheffel Rd & Lorson Blvd

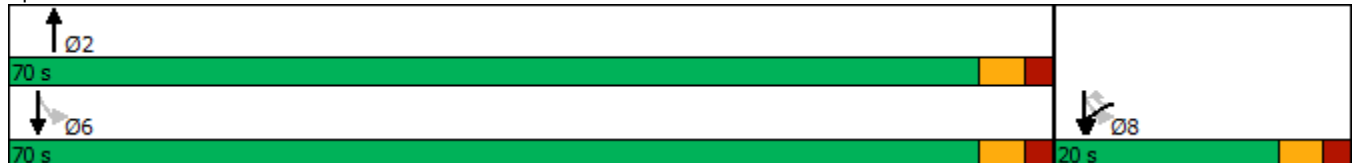
2040 Total Traffic With Signal Controlled Channelized T
PM Peak Hour

| | ↙ | ↖ | ↑ | ↗ | ↘ | ↓ | Ø6 |
|----------------------|-------|-------|-------|------|------|------|------|
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT | Ø6 |
| Lane Configurations | ↙ | ↖ | ↑↑ | ↗ | ↘ | ↑↑ | |
| Traffic Volume (vph) | 329 | 85 | 795 | 532 | 100 | 691 | |
| Future Volume (vph) | 329 | 85 | 795 | 532 | 100 | 691 | |
| Turn Type | Prot | Perm | NA | Free | Perm | NA | |
| Protected Phases | 8! | | 2 | | | 8 6! | 6 |
| Permitted Phases | | 8 | | Free | 8 6! | | |
| Detector Phase | 8 | 8 | 2 | | 8 6 | 8 6 | |
| Switch Phase | | | | | | | |
| Minimum Initial (s) | 4.0 | 4.0 | 4.0 | | | | 4.0 |
| Minimum Split (s) | 20.0 | 20.0 | 20.0 | | | | 20.0 |
| Total Split (s) | 20.0 | 20.0 | 70.0 | | | | 70.0 |
| Total Split (%) | 22.2% | 22.2% | 77.8% | | | | 78% |
| Yellow Time (s) | 3.0 | 3.0 | 3.0 | | | | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | | | | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | | | | |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | | | | |
| Lead/Lag | | | | | | | |
| Lead-Lag Optimize? | | | | | | | |
| Recall Mode | None | None | None | | | | None |
| Act Effct Green (s) | 15.1 | 15.1 | 15.8 | 40.9 | 40.9 | 40.9 | |
| Actuated g/C Ratio | 0.37 | 0.37 | 0.39 | 1.00 | 1.00 | 1.00 | |
| v/c Ratio | 0.53 | 0.14 | 0.61 | 0.35 | 0.21 | 0.22 | |
| Control Delay | 14.7 | 3.9 | 12.1 | 0.6 | 0.9 | 0.2 | |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | |
| Total Delay | 14.7 | 3.9 | 12.1 | 0.6 | 0.9 | 0.2 | |
| LOS | B | A | B | A | A | A | |
| Approach Delay | 12.5 | | 7.5 | | | 0.3 | |
| Approach LOS | B | | A | | | A | |

Intersection Summary

Cycle Length: 90
 Actuated Cycle Length: 40.9
 Natural Cycle: 40
 Control Type: Actuated-Uncoordinated
 Maximum v/c Ratio: 0.61
 Intersection Signal Delay: 6.1
 Intersection Capacity Utilization 58.2%
 Analysis Period (min) 15
 ! Phase conflict between lane groups.

Splits and Phases: 5: Marksheffel Rd & Lorson Blvd



Markup Summary

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Needs to be based on the total allowed by the SKP since the purpose is to shift density to the east.



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They need to be if the Sketch Plan is reserving the right to develop 6500 DU by shifting density to the undeveloped ares to the east.

jchodsdon (2)



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