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# James Irwin Charter Academy Traffic Impact Study <br> (LSD \#S224370) PCD File Nos. COM-2222, PPR-239 <br> April 30, 2023 

ACCEPTED for FILE Engineering Review 05/25/2023 1:09:23 PM Elizabeth Nijkamp, PE EPC Department of Public Works

## Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.


## Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.


# James Irwin Charter Academy Traffic Impact Analysis 

Prepared for:
Jeremy Hammers | Project Executive
Elder Construction
4870 Centennial Boulevard, Suite 100
Colorado Springs, CO 80919

APRIL 30, 2023

LSC Transportation Consultants
Prepared by: Jeffrey C. Hodsdon, P.E.

LSC \#S224370

PCD File No. COM-2222 \& PPR-239

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April 30, 2023

Jeremy Hammers | Project Executive
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RE: James Irwin Charter School<br>Traffic Impact Study<br>El Paso County, Colorado<br>PCD File No. COM-2222 \& PPR-239<br>LSC \#S224370

Dear Mr. Hammers,

LSC Transportation Consultants, Inc. has prepared this traffic impact study (TIS) for the proposed James Irwin Charter School in El Paso County, Colorado. The site is located northeast of the intersection of Powers Boulevard/Waynoka Road at 2460 Waynoka Place (El Paso County parcel ID 5331301024). Access to the site would be to Waynoka Place. No direct access is proposed to Powers Boulevard or Waynoka Road.

This report has been prepared for submittal to El Paso County.

## REPORT CONTENTS

- Inventory of the existing adjacent and nearby area road system. This included surface conditions, functional classifications, roadway widths, lane configurations, traffic control, posted speed limits, pavement markings, intersection and access spacing, roadway and intersection alignments, auxiliary left-turn and right-turn lanes, intersection sight distances, etc.
- Morning, mid-afternoon, and late afternoon peak-hour turning-movement traffic counts at the following "study-area" intersections:
- Powers Boulevard/Waynoka Road
- Waynoka Road/Waynoka Place
- Waynoka Place/all site accesses
- Constitution Avenue/Tutt Boulevard/Waynoka Place (morning peak only)
- Palmer Park Road/Waynoka Road
- Review of previously-completed traffic studies in the vicinity of this site for information and findings relative to this development. Other recent studies completed in the area and
any applicable data/transferrable information/analysis etc. from previous LSC studies adjacent to the site were also utilized;
- Evaluation of intersection/access sight distance at the proposed access points based on current criteria in the County's Engineering Criteria Manual (ECM);
- Estimates of average weekday and peak-hour trip generation for the proposed development;
- Estimation of directional distribution of site-generated vehicle trips on the area road system, at the study-area intersections, and at the proposed site-access points.
- Projections of site-generated turning-movement traffic volumes at the following "study-area" intersections:
- Powers Boulevard/Waynoka Road
- Waynoka Road/Waynoka Place
- Waynoka Place/all site accesses
- Constitution Avenue/Tutt Boulevard/Waynoka Place
- Palmer Park Road/Waynoka Road
- Estimates of short- and long-term background traffic volumes at the study-area intersections and access points;
- Short-term and long-term total traffic projections at the study-area intersections;
- Level of service (LOS) analysis at the study-area intersections;
- Queuing analysis at the site-access points;
- On-site queue length necessary for parent drop-off/pick-up operations;
- Evaluation of existing, short- and long-term total projected intersection volumes with respect to criteria for auxiliary right-/left-turn lanes on Waynoka Road and Waynoka Place, based on the criteria in the County's Engineering Criteria Manual;
- The recommended access plan for the school and recommendations for roadway striping along Waynoka Place north of Waynoka Road;
- A short-term, pedestrian/non-motorized accessibility plan.
- Other recommendations and the El Paso County Road Impact Fee Program requirement; and
- Summary of compiled data, analysis, findings, and recommendations.


## SCHOOL LOCATION, ACCESS, AND CIRCULATION

## Site Location

Figure 1 shows the location of the proposed James Irwin Charter School site relative to the adjacent and nearby streets. The site is in unincorporated El Paso County, adjacent to the city limits of the City of Colorado Springs, Colorado. The site is located at 2460 Waynoka Place (El Paso County parcel ID 5331301024 and is bordered by Powers Boulevard to the west, Waynoka Place to the east, Waynoka Road to the south, and a shopping center to the north. The school campus plan, including buildings, access points, parking areas, and circulation, is shown in Figure 2.

## Previous Land Use

The site was originally developed as a large, single-user manufacturing facility. That facility is no longer in operation and existing buildings and parking lots are vacant.

## Site Access Plan

Access to the site would be provided via three accesses to Waynoka Place. No direct access would be provided to Powers Boulevard.

- Proposed north access: 588 feet north of Waynoka Place/Waynoka Road (entrance only - proposed new access point - recommended by LSC). Only entering turning movements will be permitted at the proposed north access, which will be the primary access for parent drop-off/pick-up and student drivers.
- Middle access: 375 feet north of Waynoka Place/Waynoka Road (main exit, entrance only for buses, visitors, and drop off/pick up of students using inter-school bus service to other James Irwin schools, bus access - existing access to the property).
- South access: 156 feet north of Waynoka Place/Waynoka Road (staff parking lot only existing access to the property). Staff parking and primarily overflow parking would be served by the south access.

Figure 16 shows the recommended site access and circulation plan. The area is to be used for parent pick-up and drop-off "car line." The figure also shows a concept for the bus loading and unloading area.

## EXISTING AND PROJECTED FUTURE STUDENT ENROLLMENT

## School Enrollment and Operations

During the opening school year (2023-2024), James Irwin Charter Academy will serve 359 students. Maximum future enrollment is planned to be about 720 students in grades 6 through 14. Projected enrollment by school year is shown in Table 1, as well as the projected number of buses serving the school and faculty/staff numbers.

Table 1: James Irwin Charter Academy Projected Enrollment, Number of Buses, and Staff

| School Year | Student Enrollment | Staff | Buses ${ }^{1}$ |
| :---: | :---: | :---: | :---: |
| 2023-2024 | 359 | 39 | 2 |
| 2024-2025 | 395 | 43 | 3 |
| 2025-2026 | 489 | 55 | 3 |
| 2026-2027 | 525 | 58 | 3 |
| 2027-2028 | 574 | 62 | 3 |
| 2028-2029 | 623 | 68 | 3 |
| Max Enrollment | 720 | 80 | 3 |
| ${ }^{1} 2$ buses $=8$ trips daily ( 2 in +2 out during AM, 2 in +2 out during PM) <br> 3 buses $=12$ trips daily ( 3 in +3 out during AM, 3 in +3 out during PM) |  |  |  |

## School Operations

Students in grades 11-12 will generally spent 50-75 percent of their time at the proposed campus in this report, with the remainder of their schedule split between either an internship or at Pikes Peak Community College (PPCC). Each academic classroom will have 25 student desks and one teacher workstation to accommodate 20-25 students at any given time.

Although eligible to enroll at the school, students in grades 13-14 will never attend classes at this proposed campus (studied in this report), as they will attend PPCC full-time. Enrollment numbers for grades 13-14 were not included in Table 1.

## SCHOOL BELL AND BUS OPERATIONS

The school day would begin at 7:30 a.m. and would end at 3:15 p.m. As with most charter schools, this campus would not have bus-route service as with public schools. Rather, these buses would essentially act as "shuttle buses" for purposes of providing intercampus transportation for students to travel to and from other James Irwin campuses that may be closer to their homes.).

One empty bus will arrive around 7:00 a.m. to take students from Waynoka to the main campus and to the Howard location. This bus would leave by about 7:20 a.m. Bus riders would generally consist of siblings of students that attend PTEC, but who themselves attend another JICS school. Depending on the number of students, this will could be a minibus.

At least one full bus of students will arrive from the Astrozon location with students that attend PTEC but were dropped off at the Astrozon location. In years two and later, there will likely be a full bus and second minibus or full bus, depending on enrollment. It is unlikely that there would be more than two buses on the premises at any given time. Corresponding bus trips would occur during the afternoon following dismissal.

LSC has analyzed the following peak-hour periods to coincide with the arrival/dismissal of students during the school day and the peak hour of adjacent street traffic:

- AM peak hour - 7:00 a.m. to 8:00 a.m.
- Mid-day school peak hour - 2:30 p.m. to 3:30 p.m.
- PM peak hour - 4:00 p.m. to 5:00 p.m.


## AREA PEDESTRIAN AND BICYCLE FACILITIES

Sidewalks exist along Waynoka Place, but generally not along Waynoka Road. Sidewalks exist along Constitution Avenue to the north and along Palmer Park Boulevard east of Waynoka.

Future extensions of two major regional trails (Sand Creek Trail and the Rock Island Trail) are planned in close proximity to the site. These future major regional trail connections would provide connectivity to other trails and intersecting roadways (most with sidewalks and some with bicycle lanes).

Please refer to the section below entitled "SCHOOL ZONE SIGNAGE \& PAVEMENT MARKINGS"

- Also, please refer to Figure 17, which presents a detailed short-term, pedestrian/non-motorized accessibility plan.


## ROADWAY AND TRAFFIC CONDITIONS

## Study Area

The study area is bordered by Constitution Avenue on the north, Palmer Park Boulevard on the south, Powers Boulevard on the west, and a combination of Sand Creek, the Rock Island Trail ROW and the Cherokee Ridge Golf Course on the east. Per the multi-jurisdictional project meeting, the study-area intersections added include Waynoka/Palmer Park and Constitution/Tutt/Waynoka Place.

## Area Roadways

Figure 1 shows the roadways in the vicinity of the site. Major roadways are identified below, followed by a brief description.

Powers Boulevard (State Highway 21) classified by CDOT as a 6-lane F-W: Freeway in the vicinity of the site. Adjacent to the site, Powers has a posted speed limit of 55 miles per hour (mph). No auxiliary turn lanes currently exist at the stop-sign-controlled, right-in/right-out (RIRO) intersection of Powers Boulevard/Waynoka Road. Note: Pursuant to a recent meeting with CDOT and El Paso County, CDOT will require the permanent closure of the Waynoka Road connection to Powers Boulevard with this project. A cul-de-sac turn-around will be constructed on Waynoka Road just west of Waynoka Place/Waynoka Road intersection.

Constitution Avenue is shown on the El Paso County Major Transportation Corridors Plan (MTCP) as a four-lane Principal Arterial (County portion). Overall, Constitution extends east-to-west between Paseo Road and US Highway (Hwy) 24. The intersection of Constitution Avenue/Tutt Boulevard/Waynoka Place is signalized. This intersection is within the City of Colorado Springs.
Waynoka Road is shown on the MTCP as a two-lane Collector (the street is an Urban, NonResidential Collector). Waynoka Road extends generally north/south for 1.1 miles between Powers Boulevard and Palmer Park Boulevard. The posted speed limit on Waynoka Road is 30 mph . Note: Pursuant to a recent meeting with CDOT and EI Paso County, CDOT will require the permanent closure of the Waynoka Road connection to Powers Boulevard with this project. A cul-de-sac turn-around will be constructed on Waynoka Road just west of Waynoka Place/Waynoka Road intersection.

Waynoka Place is a local road that extends generally north/south for 0.4 miles between Waynoka Road and Constitution Avenue. No auxiliary turn lanes are striped/marked at the stop-sign-controlled T-intersection of Waynoka Road/Waynoka Place. The Powers Boulevard Environmental Assessment (EA) (Chapter 4, Tables 4-4 and 4-5) indicates closure of the direct connection to Powers Boulevard (required by CDOT as part of this project) and instead to [a connection to] a northbound frontage road. Obviously, in the short term, Waynoka Road and Waynoka Place will locally function as a frontage road on the east side of Powers between Palmer Park and Constitution.

Palmer Park Boulevard extends from west of Union Boulevard east to Shawnee Drive. Classified as a Principal Arterial between Powers Boulevard and Peterson Boulevard, Palmer Park Boulevard has two through lanes in each direction plus a center two-way left-turn lane and a posted speed limit of 35 miles per hour (mph). The intersection of Palmer Park Boulevard/Waynoka Place is a stop sign-controlled, full-movement T-intersection.

## ACCESS SIGHT-DISTANCE ANALYSIS

Both existing site-access points and the proposed north site-access point have been evaluated for intersection and stopping sight distance. Please refer to Figure 3a-3c, which show the detailed access sight-distance analysis.

Site improvements (existing-to-remain and proposed new) must not impede sight-distance lines of sight, as the access points will need to meet El Paso County's Engineering Criteria Manual (ECM) standards for sight distance.

Criteria from the American Association of State Highway and Transportation Officials (AASHTO) was used to analyze sight distance for the left-turn movements from the major street. AASHTO provides a different set of criteria for northbound-left turning movements from the "major street" (Waynoka Place) into the school access points. These criteria are different from left turns from the "minor street" (stop-sign-controlled approaches - traffic exiting the school) and the
criteria for left turns from the major street are not provided within the ECM. Per AASHTO criteria, 285 feet of intersection sight distance would be required for left turns from Waynoka Road.

Existing site landscaping, lower tree branches, bushes, signs, buildings, parking areas, etc. should be removed, if necessary, and new site improvements should not be placed within the ECM-required line of sight "triangles."

## North Access

Exiting turning movements would not be permitted at the north site access, while the southbound-right turn would be a "free" movement. The northbound-left turning movement would have conflicting turning movements and, thus, would be required to meet intersection sight distance for left turns from a major street. Per AASHTO, 285 feet of intersection sight distance would be required for left turns from Waynoka Road, which would be provided, as shown in Figure 3a.

## Middle Access

With a $30-\mathrm{mph}$ posted speed limit on Waynoka Place, the minimum required entering/intersection sight distance for both approaches at the proposed middle site-access location is 300 feet for passenger vehicles (per Table 2-35 of the County's Engineering Criteria Manual). Per Table 2-36, the design vehicle is single-unit trucks (for buses on a residential, school-bus route). Sight distances for both approaches at the proposed middle site-access location to Waynoka Road meet the required 300-foot requirement, as shown in Figure 3b.

Looking to the south, a 360 -foot line of sight for intersection sight distance would be provided for school buses. This assumes a $10-\mathrm{mph}$ speed for an approaching vehicle turning westbound-right from Waynoka Road to Waynoka Place at the south end of the line of site.

Assuming a "worst case" scenario in which a sight-distance easement would not be provided across private property, a 270 -foot line of sight for intersection sight distance would be provided to the south from the center of the eastbound-left exiting turning lane. This assumes the speed of an approaching vehicle heading northbound on Waynoka Place would be less than 30 mph at the south end of the line of sight.

## South Access

With a $30-\mathrm{mph}$ posted speed limit on Waynoka Place, the minimum required entering/intersection sight distance for both approaches at the proposed middle site-access location is 300 feet for passenger vehicles (per Table 2-35 of the County's Engineering Criteria Manual). Per Table 2-36, the design vehicle is single-unit trucks (for buses on a residential, school-bus route). Sight distances for both approaches at the proposed middle site-access location to Waynoka Road meet the required 300-foot requirement, as shown in Figure 3c.

Exiting turning movements would not be permitted at the north site access, while the southbound-right turn would be a "free" movement. The northbound-left turning movement would have conflicting turning movements and, thus, would be required to meet intersection sight distance for left turns from a major street. Per AASHTO, 285 feet of intersection sight distance would be required for left turns from Waynoka Road, which would be provided, as shown in Figure 3a.

Assuming a "worst case" scenario in which a sight-distance easement would not be provided across private property, a 100-foot line of sight for intersection sight distance would be provided to the south from the center of the eastbound-left exiting turning lane. This assumes a $10-\mathrm{mph}$ speed for an approaching vehicle turning westbound-right from Waynoka Road to Waynoka Place at the south end of the line of site.
Assuming a "worst case" scenario in which a sight-distance easement would not be provided across private property, a 100-foot line of sight for intersection sight distance would be provided to the south from the center of the eastbound-left exiting turning lane. This assumes a $15-\mathrm{mph}$ speed for an approaching vehicle turning eastbound-left from Waynoka Road to Waynoka Place at the south end of the line of site.

## Existing Traffic Volumes

Existing traffic volumes at the following intersections are shown in Figure 4. Detailed traffic count reports are attached.

- Powers Road/Waynoka Road
- Thursday, June 9, 2022, from 6:45-8:00 a.m.
- Thursday, June 9, 2022, from 2:30-3:30 p.m.
- Thursday, June 9, 2022, from 4:00-6:00 p.m.
- Waynoka Road/Waynoka Place
- Thursday, June 9, 2022, from 6:45-8:00 a.m.
- Thursday, June 9, 2022, from 2:30-3:30 p.m.
- Thursday, June 9, 2022, from 4:00-6:00 p.m.
- Constitution Avenue/Waynoka Place/Tutt Boulevard
- Tuesday, June 28, 2022, from 6:30-8:30 a.m.
- Palmer Park Boulevard/Waynoka Road
- Tuesday, July 19, 2022, from 6:30-8:30 a.m.
- Tuesday, July 19, 2022, from 1:30-3:30 p.m.
- Tuesday, July 19, 2022, from 4:00-6:00 p.m.


## TRIP GENERATION

Estimates of the existing and projected vehicle trips to be generated by a site are typically made using the following nationally published average trip-generation rates in Trip Generation, 11 th Edition, 2021 by the Institute of Transportation Engineers (ITE). LSC used rates for ITE land-use code " 538 - Charter

School (K-12)" to estimate the school trip generation. LSC has also included a comparison to the trip generation for the previous land use at this site (estimated), for reference.

## Short Term (2023-2024 School Year)

Table 2 below presents a summary of the estimated site trip generation for the 2023-2024 school year using ITE rates. The detailed short-term trip-generation estimate for the school is presented in Table 3 (attached).

Table 2: Estimated Site Vehicle-Trip Generation (2023-2024 School Year)

| Analysis Period | Weekday |  |  |
| :---: | :---: | :---: | :---: |
|  | In | Out | Total |
| Morning Peak Hour | 179 | 159 | 338 |
| Mid-Day Peak Hour | 131 | 131 | 262 |
| Evening Peak Hour | 7 | 7 | 14 |

Based on the ITE estimate for the proposed James Irwin Charter Academy, the site would generate about 785 external vehicle trips on the average weekday during the initial 2023-2024 school year. During the weekday morning peak hour, approximately 179 vehicles would enter and 159 vehicles would exit the site. Approximately 131 entering vehicles and 131 exiting vehicles are projected for the weekday school mid-afternoon peak hour. During the weekday late-afternoon "commuter" peak hour, approximately 7 vehicles would enter and 7 vehicles would exit the site.

## Long Term (Maximum Enrollment)

Table 3 below presents a summary of the estimated site trip generation for the maximum enrollment school year using ITE rates. The detailed long-term trip-generation estimate for the school is presented in Table 9 (attached).

Table 3: Estimated Site Vehicle-Trip Generation (Maximum Enrollment School Year)

| Analysis Period | Weekday |  |  |
| :---: | :---: | :---: | :---: |
|  | In | Out | Total |
| Morning Peak Hour | 359 | 318 | 677 |
| Mid-Day Peak Hour | 263 | 263 | 526 |
| Evening Peak Hour | 13 | 13 | 26 |

Based on the long-term ITE trip estimate for the proposed James Irwin Charter Academy, about 359 vehicles would enter and 318 vehicles would exit the site during the morning peak hour. Approximately 263 entering vehicles and 263 exiting vehicles are projected for the weekday school mid-afternoon peak hour. During the weekday late-afternoon "commuter" peak hour, approximately 13 vehicles would enter and 13 vehicles would exit the site.

## Comparison to Previous Land Use

## Short Term

During the opening year, compared to the previous land use for the site (an 82,235-square-foot manufacturing building), the proposed James Irwin Charter Academy would generate:

- AM peak hour - 69 additional entering and 144 additional exiting trips
- Mid-day peak hour - 121 additional entering and 82 additional exiting trips
- PM peak hour - 14 fewer entering and 92 fewer exiting trips


## Long Term

During the long term, compared to the previous land use for the site (an 82,235-square-foot office building), the proposed James Irwin Charter Academy would generate:

- AM peak hour - 249 additional entering and 303 additional exiting trips
- Mid-day peak hour - 253 additional entering and 214 additional exiting trips
- PM peak hour - 7 fewer entering and 85 fewer exiting trips


## TRIP DISTRIBUTION AND ASSIGNMENT

## Trip Directional Distribution

Estimating the directional distribution of site-generated vehicle trips to the study-area roads and intersections is a necessary component in determining the site's traffic impacts. Figure 5 shows the percentages of the site-generated vehicle trips projected to be oriented to and from the site's major approaches. Estimates have been based on the following factors: the proposed new land use, the area street and road system serving the site, and the site's geographic location relative to the balance of the City of Colorado Springs and unincorporated areas of El Paso County.

Additionally, the applicant provided a list of zip codes in which currently-enrolled students reside. LSC utilized these data as part of the trip distribution estimate. Please refer to Appendix Figure 1 for more details.

## Site-Generated Traffic (Short Term)

Figure 6 shows the projected site-generated traffic volumes for the weekday morning and evening peak hours for the short term. Short-term site-generated traffic volumes have been calculated by applying directional-distribution percentages estimated by LSC (from Figure 5) to the short-term trip-generation estimates (from Table 3). The 2022-2023 school year estimates have been used for the short-term school site-generated traffic-volume estimates.

## Site-Generated Traffic (Long Term)

Figure 7 shows the projected site-generated traffic volumes for the weekday morning and evening peak hours for the maximum enrollment year. Long-term site-generated traffic volumes have been calculated by applying directional-distribution percentages estimated by LSC (from Figure 5) to the long-term trip-generation estimates (from Table 3). The maximum enrollment school year estimates have been used for the long-term school site-generated traffic-volume estimates.

## SHORT-TERM SCENARIO, BASELINE, AND TOTAL TRAFFIC

The Waynoka Road connection to Powers Boulevard will be permanently closed with this project, as required by CDOT.

Figure 8 shows the estimated short-term baseline traffic volumes, which reflect adjustments and rerouting of existing traffic to account for the planned closure of the Waynoka Road/Powers Boulevard intersection.

Figure 9 shows the projected short-term total traffic volumes, which are the sum of short-term baseline (adjusted existing traffic, from Figure 8) plus estimated James Irwin Charter Academy short-term (2023-2024 school year) site-generated traffic (from Figure 6).

## FUTURE LONG-TERM TRAFFIC SCENARIO

Several potential future changes to the area roadway network will affect future traffic volumes in the study area.

- Powers Boulevard is planned as a future freeway. Although Powers Boulevard volumes are likely to continue to increase, the corridor already currently carries high volumes.
- The Waynoka Road connection to Powers Boulevard will be permanently closed with this project, as required by CDOT.
- Waynoka Road south of Waynoka Place and Waynoka Place will likely combine to form portions of the planned future Powers Boulevard frontage road.
- Some currently-vacant parcels along Waynoka Road may be developed in the future. Although this will add some additional traffic to Waynoka, the roadway is under-capacity and will be able to accommodate additional trips.


## 2042 BACKGROUND TRAFFIC

Figure 10 shows the background traffic volumes for the year 2042. Background traffic is the traffic estimated to be on the adjacent roadway system without consideration of the proposed school. Background traffic includes the through traffic and the traffic generated by adjacent developments (existing and anticipated future) but assumes zero traffic generated by the site.

## 2042 TOTAL TRAFFIC

Figure 11 shows the total traffic volumes for the year 2042 at the study-area intersections, which are the sum of the 2042 background traffic volumes (from Figure 10) plus the long-term site-generated traffic volumes (from Figure 7).

## LEVEL OF SERVICE ANALYSIS

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection and is indicated on a scale from "A" to "F." LOS A is indicative of little congestion or delay. LOS F indicates a high level of congestion or delay. Table 4 shows the level of service delay ranges for signalized and unsignalized intersections.

Table 4: Intersection Levels of Service Delay Ranges

|  | Signalized Intersections | Unsignalized Intersections |
| :---: | :---: | :---: |
| Level of Service | Average Control Delay <br> (seconds per vehicle) | Average Control Delay <br> (seconds per vehicle) ${ }^{(1)}$ |
| A | 10.0 sec or less | 10.0 sec or less |
| B | $10.1-20.0 \mathrm{sec}$ | $10.1-15.0 \mathrm{sec}$ |
| C | $20.1-35.0 \mathrm{sec}$ | $15.1-25.0 \mathrm{sec}$ |
| D | $35.1-55.0 \mathrm{sec}$ | $25.1-35.0 \mathrm{sec}$ |
| E | $55.1-80.0 \mathrm{sec}$ | $35.1-50.0 \mathrm{sec}$ |
| F | 80.1 sec or more | 50.1 sec or more |

(1) For unsignalized intersections, if $\mathrm{V} / \mathrm{C}$ ratio is greater than 1.0 the level of service is LOS F, regardless of the projected average control delay per vehicle.

Detailed Synchro reports are attached. A summary of LOS during the weekday morning and evening peak hours for the study-area intersections is shown in the following figures:

- Figure 4: Existing Traffic, Lane Geometry, Traffic Control, and LOS
- Figure 8: Short-Term Baseline Traffic, Lane Geometry, Traffic Control, and LOS
- Figure 9: Short-Term Baseline + Site Traffic, Lane Geometry, Traffic Control, and LOS
- Figure 10: 2042 Background Traffic, Lane Geometry, Traffic Control, and LOS
- Figure 11: 2042 Background + Site Traffic, Lane Geometry, Traffic Control, and LOS


## Powers Boulevard/Waynoka Road

The westbound-right turning movement at Powers Boulevard/Waynoka Road currently operates at LOS D during the morning peak hour but LOS F during the mid-day and PM peak hours. Pursuant to a recent meeting with CDOT and El Paso County, CDOT has indicated that this intersection must be closed prior to the opening of the proposed school. As such, no short- or long-term analysis has been included in this report.

## Waynoka Place/Site Access Points

LSC has assumed that Waynoka Road would be restriped with a painted left-turn median. This would either be striping for dedicated left-turn bays or a center two-way left-turn lane (TWLTL) in conjunction with the opening of the charter school.

## Waynoka Place/North Site Access

All individual turning movements at the proposed north site access are projected to operate at LOS A through the long-term during all peak periods. Only northbound-left and southbound-right entering movements by parents and students would be permitted at this access.

## Waynoka Place/Middle Site Access

All individual turning movements at the proposed north site access are projected to operate at LOS D or better through the long-term during all peak periods. Separate eastbound-left and eastbound-right exiting movements would be provided. This access will be the primary exit for the school, including parent and student vehicles. Buses would be permitted to enter (and exit) at this access, as would visitors, and parents dropping off/picking up students using inter-school bus service to other James Irwin schools.

## Waynoka Place/South Site Access

All individual turning movements at the proposed north site access are projected to operate at LOS B or better through the long-term during all peak periods. Only staff/faculty and overflow parking would be permitted at this full-movement access.

## Waynoka Road/Waynoka Place

All single-lane approaches at this intersection currently operate at and are projected to remain at LOS B or better during all peak periods, with or without the addition of site-generated traffic. Note: This analysis has been conducted based on the current laneage of single-lane approaches. Please refer to the following Auxiliary Turn-Lane Needs Analysis section of this report.

## Constitution Avenue/Tutt Boulevard/Waynoka Place

Note: LSC did not modify the City's existing signal timings at this intersection when analyzing any short-term or long-term scenario, with or without the addition of site-generated traffic. Based on projected volume increases on Tutt Boulevard and Waynoka Place, the City may opt to adjust existing signal timings to provide more green time to the northbound and southbound approaches in order to improve LOS on the minor-street approaches.

## Short Term

Based on existing signal timings, all individual turning movements currently operate at and are projected to remain at LOS D or better during the short term, with or without the addition of site-generated traffic from the proposed charter school.

## Long Term

Using existing signal timings, the following individual turning movements are projected to operate at LOS E or worse during the long term, with or without the addition of site-generated traffic from the proposed charter school: southbound-through and northbound-left. The northbound-through/right shared turn lane is projected to operate at LOS E during the maximum enrollment school year.

It is unlikely that LOS on the northbound approach would improve without modifying the existing signal timings to provide more green time on minor-street approaches. There is not sufficient room to add separate northbound-through and northbound-right lanes, so this turning movement is likely to remain a single northbound-through/right shared turn lane in the future. Additionally, the eastbound-right turning movement already exceeds the City's threshold for requiring a right-turn deceleration lane, but this improvement is not feasible due to geometric constraints on the southwest corner of Constitution/Tutt/Waynoka.

## Palmer Park Boulevard/Waynoka Road

## Short Term

Assuming the existing stop-sign control, all individual turning movements would continue to operate at LOS D or better during the short term, with or without the addition of site-generated traffic from the proposed charter school.

## Long Term

Based on the long term projected total volumes, the southbound left-turning movement is projected to operate at LOSE (with a v/c ratio below 1.0) during the morning peak hour, assuming (the current) stop-sign traffic control. This intersection is a short distance east of the Powers/Palmer Park signalized intersection. Relatively long traffic gaps in eastbound traffic are created by this upstream signal to the west and the signal at the adjacent Wendy's/shopping center signal, which benefit southbound-left-turning motorists at Waynoka Road/Palmer Park Boulevard.

Right-of-way has been reserved for potential future realignment of Waynoka Road north of Palmer Park. The Powers Environmental Assessment envisions this realignment such that Waynoka Road would align with the rear access to the former Kmart shopping center, and this
four-leg intersection would be signalized. The property owner on the north side of Palmer Park is (and has been) evaluating other short-term and long-term access and traffic-control alternatives for this section of Palmer Park Boulevard. Regarding the cross-section of Waynoka on the southbound approach to Palmer Park, there is already sufficient width for separate right-and left-turn movements. Adding additional width to the existing cross section of Waynoka north of Palmer Park would not change the level of service.

## QUEUING ANALYSIS - ACCESS POINT INTERSECTIONS AND OFFSITE INTERSECTIONS

A queuing analysis was performed for the eastbound approach at the middle site access to Waynoka Place. Queuing analyses have been run for the short-term total and the 2042 total traffic volumes.
"Upstream block time" represents the percent of time during the peak hour in which the entry point for a turn lane upstream of the subject intersection is blocked by a queue in the adjacent through lane. "Storage block time" is the proportion of time in which the turn lane's queue exceeds the available storage length and left-turning vehicles overspill the turn lane in the model and into the adjacent through lane.
"Maximum queue" represents the maximum queue length observed for each individual lane during the 15-minute analysis period. SimTraffic records the maximum back of queue observed for every two-minute period. In SimTraffic, a vehicle is considered queued whenever it is behind another vehicle traveling at less than 10 feet/second (approximately 7 mph ) or at a stop bar. The maximum observed queue may not occur during the same interval in which the highest upstream block time (percent) or storage block time (percent) occurs. LSC has analyzed the highest value for each metric for each turn lane/approach, regardless of whether or not they occur in the same 15-minute interval.

Reported queue lengths for auxiliary turn lanes in SimTraffic is generally limited by the turn-lane length. SimTraffic simply reports the maximum observed queue length during simulations. Any spillover from a left-turn lane is reported in the adjacent lane queue length. Please refer to Figure 12 for more details.

## North Access

Analysis has been run to estimate the maximum queue length of the northbound-left lanes at the north access that would extend to the middle access. A 180-foot dedicated northbound-left turn lane is recommended on Waynoka Place between the north and middle site accesses.

As shown in Table 5, SimTraffic simulation reports indicate that the northbound-left queue is projected to reach a maximum of about 105 feet, which would not exceed the 180 feet of stacking distance for this turn lane.

Table 5: Projected 2042 Queues at North Access

| North Access -- NBL |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Analysis Period | Storage Length | Taper <br> Length | Total Stacking | Queue <br> SimTraffic | Exceeds <br> Stacking? |
| AM Peak Hour | 145' | $35^{\prime}$ | $180{ }^{\prime}$ | 105' | No |
| Mid-Day Peak Hour |  |  |  | $73^{\prime}$ | No |
| PM Peak Hour |  |  |  | $14^{\prime}$ | No |

## Middle Access

## Eastbound Approach (Exiting Traffic)

Analysis has been run to estimate the maximum queue length of the eastbound-left and eastbound-right exiting lanes at the middle access to determine the minimum on-site stacking length for parent and student vehicles after pick-up and drop-off operations. These lanes would be striped separately in their entirety after the parent loading zone.

During the mid-afternoon peak hour, simulation reports indicate that the eastbound-left queue is projected to reach a maximum of about 263 feet, while the eastbound-right queue is projected to reach a maximum of about 273 feet during the long-term afternoon release period.

LSC recommends that the eastbound exiting turn lanes at the middle access each be striped for a minimum of 285 feet, which would accommodate approximately 12-14 vehicles.

Northbound-Left Turn Bay (Limited Entry - Buses, Visitors, and Inter-Campus Bus "Terminal")
The 175 -foot northbound-left turn bay ( 90 feet plus 85 -foot taper) at the middle access would accommodate the projected 95th-percentile queue length of 105 feet. The middle access would serve as an exit-only, except for buses, visitors, and parents dropping off/picking up students using inter-school bus service to other James Irwin schools. These would be the only vehicles allowed entry at this middle access. Parents will be required to use the north access for the main parent pickup and drop-off "carpool" lane.

Please refer to the attached SimTraffic reports for projected maximum and $95^{\text {th }}$ percentile midday peak-hour queue lengths. Please refer to Table 6 for a summary of projected queues at the middle site access during all peak analysis periods.

Table 6: Projected 2042 Queues at Middle Access

| Middle Access -- EBL |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Peak | Storage <br> Length | Taper <br> Length | Total Stacking | Queue SimTraffic | Exceeds <br> Stacking? |
| AM Peak Hour | $260{ }^{\prime}$ | $25^{\prime}$ | 285 | 263' | No |
| Mid-Day Peak Hour |  |  |  | $273{ }^{\prime}$ | No |
| PM Peak Hour |  |  |  | $31^{\prime}$ | No |
| Middle Access -- EBR |  |  |  |  |  |
| Peak | Storage <br> Length | Taper Length | Total Stacking | Queue SimTraffic | Exceeds Stacking? |
| AM Peak Hour | $190{ }^{\prime}$ | $50^{\prime}$ | $240{ }^{\prime}$ | $136{ }^{\prime}$ | No |
| Mid-Day Peak Hour |  |  |  | $65^{\prime}$ | No |
| PM Peak Hour |  |  |  | $22^{\prime}$ | No |
| Middle Access -- NBL |  |  |  |  |  |
| Peak | Storage Length | Taper Length | Total Stacking | Queue SimTraffic | Exceeds Stacking? |
| AM Peak Hour | $90^{\prime}$ | $85^{\prime}$ | 175' | 105' | No |
| Mid-Day Peak Hour |  |  |  | $16^{\prime}$ | No |
| PM Peak Hour |  |  |  | $0^{\prime}$ | No |

## South Access

Analysis has been run to estimate the maximum queue length of the northbound-left lanes at the north access that would extend to the middle access. A 100-foot dedicated northbound-left turn lane is recommended on Waynoka Place between the south site access and intersection of Waynoka Road/Waynoka Place.

As shown in Table 7, SimTraffic simulation reports indicate that the northbound-left queue is projected to reach a maximum of about 35 feet, which would not exceed the 100 feet of stacking distance for this turn lane.

Table 7: Projected 2042 Queues at South Access

| South Access -- NBL |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Peak | Storage <br> Length | Taper Length | Total Stacking | Queue <br> SimTraffic | Exceeds Stacking? |
| AM Peak Hour | $50^{\prime}$ | $50^{\prime}$ | $100 '$ | $35^{\prime}$ | No |
| Mid-Day Peak Hour |  |  |  | $33^{\prime}$ | No |
| PM Peak Hour |  |  |  | $18{ }^{\prime}$ | No |

## Waynoka Road/Waynoka Place - Southbound Approach

Analysis has been run to estimate the maximum queue length of the single-lane southbound approach on Waynoka Place to determine if it would extend to the south access. As shown in

Table 8, SimTraffic simulation reports indicate that the northbound-left queue is projected to reach a maximum of about 93 feet, which would not exceed the 100 feet of stacking distance for the southbound approach of Waynoka between Waynoka Road/Waynoka Place and the south site access.

Table 8: Projected 2042 Queues at Waynoka Road/Waynoka Place (Southbound Approach)

| Waynoka Rd + Waynoka PI -- SB Approach |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Peak | Storage Length | Taper <br> Length | Total Stacking | Queue <br> SimTraffic | Exceeds <br> Stacking? |
| AM Peak Hour | $100 '$ | $0^{\prime}$ | $100 '$ | $93^{\prime}$ | No |
| Mid-Day Peak Hour |  |  |  | $92^{\prime}$ | No |
| PM Peak Hour |  |  |  | $68^{\prime}$ | No |

## AUXILIARY TURN-LANE NEED ANALYSIS

Please refer to the following exhibits (attached) for proposed striping plans at each site access:

- Figure 12: Projected Queue Lengths
- Figure 13: Proposed North Access Laneage
- Figure 14: Proposed Middle Access Laneage
- Figure 15: Proposed South Access Laneage


## Powers Boulevard/Waynoka Road

Powers Boulevard is classified as "F-W: Freeway" with a posted speed limit of 55 mph in the vicinity of the site. Waynoka Road is classified as a Non-Residential Collector. No auxiliary right-turn lanes currently exist on Powers Boulevard at Waynoka Road. However, CDOT has indicated that this RIRO intersection will be closed in conjunction with the opening of the proposed school.

## Waynoka Road at Waynoka Place

Following the closure of the Waynoka Road/Powers intersection, nearly all vehicles would be westbound-right-turning (to head northbound on Waynoka Place), so there would be no need for a separate right-turn deceleration lane on the westbound approach. As shown in Table 7, SimTraffic simulation reports indicate that the northbound-left queue at the south access is projected to reach a maximum of about 35 feet, which would not extend back to the intersection of Waynoka Road/Waynoka Place (100 feet of stacking distance available).

Additionally, LSC recommends that the southbound approach on Waynoka Place approaching Waynoka Road should remain a single-lane approach. Striping for a short northbound-left-turn bay is recommended just north of this intersection at the south access (staff/overflow parking lot), and that is achievable with the southbound single-lane approach on Waynoka Place at Waynoka Road.

## Waynoka Place/Site Access Points

LSC recommends that Waynoka Place be striped with a painted left-turn median. Striping for a center two-way left-turn lane (TWLTL) (except the segment just north of Waynoka Road) is shown in the figures. The preliminary recommended configuration of the access points and associated laneage, striping of Waynoka Place, etc. is shown in Figure 12. Also, please refer to the queuing tables presented in the previous section.

## Waynoka Place/North Site Access

Only entering turning movements will be permitted at the north access, which will be the ingress for parent and student drivers. Based on projected traffic volumes, the ECM threshold for southbound-right turn lane would be met at the main access, as the highest peak-hour volume at this access ( 186 vph ) would exceed the $E C M^{\prime} \mathrm{s} 50-\mathrm{vph}$ threshold requiring a right-turn lane on a Collector. LSC recommends the following southbound-right turn-lane dimensions at the north access:

- 280-foot southbound right-turn deceleration lane (190 feet with 90-foot taper)

Based on projected traffic volumes, the ECM threshold for northbound-left turn lane would be met at the main access, as the highest peak-hour volume at this access ( 134 vph ) would exceed the ECM's 25 -vph threshold requiring a left-turn lane on a Collector. The northbound left-turn lane will be provided with the restriping for a TWLTL

## Waynoka Place/Middle Site Access

The middle access would primarily serve exiting traffic. Parents entering for the pick-up/drop-off "carline" for this school would enter at the north access and exit at this middle access. Therefore, -the only entering traffic permitted at this middle access will be buses, visitors, and parents dropping off/picking up students using inter-school bus service to other James Irwin schools. These vehicles would be allowed entry at this middle access to access the visitor parking and the inter-campus bus "terminal."

LSC recommends that separate eastbound-left and eastbound-right turn lanes with at least 340 feet of stacking distance per lane be provided to accommodate projected exiting queues.

Based on projected traffic volumes, the ECM threshold for a southbound-right turn lane on Waynoka Place would not be met at the middle access, as the highest peak-hour volume at this access ( 7 vph ) would not exceed the ECM's 50-vph threshold for requiring a right-turn lane on a Collector. The 7 vph may increase depending on the level of use of the inter-campus bus service by parents. However, it is unlikely that the 50 vph threshold would be exceeded.

Based on projected traffic volumes, the ECM threshold for northbound-left turn lane would not be met at the middle access, as the highest peak-hour volume at this access ( 7 vph ) would not
exceed the ECM's 25 -vph threshold requiring a left-turn lane on a Collector. However, as this middle access would serve as an entrance for a limited number of buses and parents/visitors, a northbound-left turn will be provided with the restriping for a TWLTL. The 7 vph may increase depending on the level of use of the inter-campus bus service by parents. However, a higher volume would primarily be associated with parents dropping off/picking up students traveling to/from other campuses on the inter-campus bus program. It is unlikely that the $95^{\text {th }}$ percentile queue would extend south past the upstream (south) access-point intersection.

## Waynoka Place/South Site Access

Staff parking and primarily overflow parking would be served by the south access.,

Based on projected traffic volumes, the ECM threshold for southbound-right turn lane would not be met at the south access, as the highest peak-hour volume at this access ( 22 vph ) would not exceed the ECM's 50 -vph threshold for requiring a right-turn lane on a Collector.

Based on projected traffic volumes, the ECM threshold for northbound left-turn lane would not be met at the south access, as the highest peak-hour volume at this access ( 16 vph ) would not exceed the ECM's 25 -vph threshold requiring a left-turn lane on a Collector. However, due to the short distance on Waynoka Place between the south access and the stop bar at Waynoka Road, and because this south access would serve as the main staff parking lot, LSC recommends that a short 100-foot dedicated northbound-left turn lane (rather than TWLTL) be striped to prevent blockages on the northbound approach of Waynoka Place extending back to Waynoka Road. This northbound left-turn bay would consist of 50 feet of storage plus a 50-foot taper.

## Palmer Park Boulevard/Waynoka Road

## Westbound-Right Turn Movement

Based on projected traffic volumes, the westbound right-turn movement would exceed the ECM threshold requiring a separate westbound right-turn deceleration lane.

## Eastbound-Left Turn Movement

No modifications would be required to the existing cross-section of Palmer Park Boulevard, which includes a striped, center left-turn median (TWLTL). There is about 400' of back-to-back stacking distance between this intersection and the main access to the shopping center on the south side of Palmer Park. There is a service access to the south within this 400' distance.

## Constitution Avenue/Tutt Boulevard/Waynoka Place

The City of Colorado Springs required morning peak-hour traffic data and analysis only.

Based on the counts, projections and LSC analysis, and the City Traffic Criteria Manual, turn-lane thresholds prescribing separate right-turn lanes are currently exceeded on the northbound and eastbound approaches. However, there is not sufficient room to add separate northbound-through and northbound-right lanes at this off-site intersection, so this turning movement is likely to remain a single northbound-through/right shared turn lane. Additionally, although the eastbound-right turning movement already exceeds the City's threshold for requiring a right-turn deceleration lane, this improvement is not feasible due to geometric constraints on the southwest corner of Constitution/Tutt/Waynoka.

## ON-SITE TRAFFIC OPERATIONS

The north access will be an entry only for the main "car line" - for parents dropping off and picking up students attending this school. No egress is planned at this north location. Parents will enter and follow the carline shown in Figure 16a within the north parking lot. These are concepts only and may be modified if needed, provided sufficient queue distance is provided to prevent queues from backing onto Waynoka Place (into the public right-of-way). The next section presents the required queue distance.

The middle access would primarily serve exiting traffic. The only entering traffic permitted at this middle access will be buses, visitors, and parents dropping off/picking up students using inter-school bus service to other James Irwin schools. These vehicles would be allowed entry at this middle access to access the visitor parking and the inter-campus bus "terminal." The intent for this is to separate bus traffic from the main entering carline traffic. Also, to encourage use of inter-campus bus transportation (by allowing these parents easier access to the bus "terminal" without needing to enter the queue in the main car line).

As shown in Figure 16a (opening year)/16b (max enrollment), the entering buses and associated parent vehicles would need to turn left across the exiting main parent car line. LSC recommends staff traffic control be stationed at this point to stop the exiting line of parent vehicles to create a gap for buses (and associated inter-campus parent vehicle drop off/pick up) to be able to turn left into the bus terminal. A staff member also stationed to the south (at the point where buses and parents will exit the bus terminal lanes and enter the exiting traffic lanes approaching Waynoka Place) would be beneficial to operations and to encourage use of the inter-campus bus transportation. Encouraging inter-campus bus transportation would reduce vehicle miles traveled by private vehicles on the area roadways

## ON-SITE QUEUING REQUIREMENT (FOR PARENT PICK-UP/DROP-OFF "CAR-LINE")

## School On-Site Queueing for Parent Drop-off and Pick-up

The North Carolina Municipal School Transportation Assistance (MSTA) performs studies that address the safety concerns with the overall pedestrian safety and traffic operations on a school campus, and how traffic affects adjacent roadways. To calculate school operations, MSTA has
developed a database of specific data related to school operations, including required queue lengths and trip-generation estimates by mode (parent drop-off/pick-up, bus, etc.). LSC has used the MSTA's spreadsheet in several similar school operations studies, as it has typically been required by jurisdictions as a preferred alternative to ITE rates for schools.

Data indicates that AM traffic operations on a school campus usually operate safely and efficiently due to parent traffic arriving at a broader range of times. PM traffic operations are quite different, as parents often arrive well before the school dismissal and park or queue (back up) along campus driveways. The PM queue often results with vehicles stopped in the roadway or along the shoulder of a major through route, which increase the chances of accidents and similar traffic-related safety concerns.

Per information from the City of Colorado Springs Traffic Engineering Division, the required "high-demand" stacking length on-site in the proposed parent drop-off/pick-up loop for the maximum enrollment ( 720 students) would be 1,650 feet. The school "carpool" plan will need to show this length of on-site stacking/queuing distance for parent drop-off/pick-up plus 175 feet of active loading/unloading zone distance (NC MSTA guidelines). Depending on the site operational characteristics, the necessary on-site queue lengths could potentially be adjusted.

This queue distance is exclusive of a recommended 5-7-vehicle-long drop-off/pick-up zone (the 175 -foot distance). The empirical formula adds an additional 30 percent to a base queue-length calculation of required total queue length as a precaution for atypical events, including bad weather, school performances, and other special events. Formula-generated queue lengths are based on afternoon school peak-hour empirical queuing data.

As shown in Figure 16, 1,750 feet of on-site stacking distance would be provided, which exceeds the City's 1,650 -foot requirement for parent drop-off/pick-up stacking operations. Shortly after entering the single-lane north access, a second parent queue lane would provide additional stacking distance for much of the remainder of the circulation loop. Vehicles would merge back to a single lane before the 175-foot parent drop-off/pick-up loading zone. Please refer to Figure 16 for more details.

## ROADWAY CLASSIFICATIONS

Powers Boulevard is a designated Freeway, Waynoka Road is a Non-Residential Collector, and Waynoka Place is a Local Road. However, Waynoka Place should likely be considered a Non-Residential Collector, as the ROW is 80 -feet wide, and even without this school development, Waynoka Place provides the north/south connection up to Constitution and north/south continuity via Tutt Boulevard. Moreover, the previous land use was consistent with one for which the Non-Residential Collector roadway type was intended to serve.

## SCHOOL ZONE SIGNAGE \& PAVEMENT MARKINGS

Although charter schools draw from a larger geographic area than traditional neighborhood public schools, school-zone speed limits, signage and pavement markings have been implemented at several charter schools within the Colorado Springs area (notably ones adjacent to or within residential neighborhoods). This school site is not within or adjacent to a residential neighborhood. However, this is not to suggest that no students would walk or bike to school. There are apartment complexes on Tutt Boulevard about a half-mile to the north across Constitution Avenue as well as planned residential development east of Tutt Boulevard. Please refer to Figure 17, which shows a detailed short-term, pedestrian/non-motorized accessibility plan. The travel distance between the school and the nearest homes within the Constitution Hills neighborhood to the east is about three-quarters of a mile.

Given the above, LSC recommends the following measures occur prior to and following with the beginning of the opening of the school in the fall of 2023.

- Identify through school enrollment records, the locations of students who could potentially walk to school (residing within a reasonable walking distance up to 1 to 1.5 miles, possibly up to two miles), and assess the potential associated requirements based on that information and the plan shown in Figure 17.
- LSC has consulted with the City of Colorado Springs Public Works Traffic Engineering Division regarding the need for school pedestrian crossing warning signs, pavement markings and potentially school speed zone signs/flashers at the intersection of Constitution/Tutt Boulevard/Waynoka Place. Figure 17 presents the details.
- The school should direct any pedestrian traffic to cross to the west side of Waynoka Place at the Constitution/Tutt Boulevard/Waynoka Place intersection instead of walking south from Constitution along the east side of Waynoka Place, so they would not need to cross Waynoka Place adjacent to the school. Again, refer to any requirements per Figure 17.
- Although the school carpool plan should specifically prohibit parents from dropping off and picking up students along the east side of Waynoka Place and Waynoka Road, should this occur such that students walk across either of these streets, the school should evaluate options to control this, and/or develop and implement school-zone speed-limit signage/school pedestrian crossing signs to enhance safety even if parents are notified not to drop off/pick up in those locations outside of the provided on-site carpool lanes.

Future Rock Island and Sand Creek regional trail connections will likely improve the viability of non-motorized trips to/from the school. At that time, school-zone and school pedestrian signs/markings should be revisited, considering these trail extensions and connector facilities to the sidewalks near to/adjacent to the school.

## CONFORMANCE WITH THE MTCP

No reimbursable roadway improvement projects have been identified as being needed by the year 2040, per Map 13 and Table 4 of El Paso County's 2016 MTCP. See the attached MTCP maps for reference.

## COUNTY ROAD IMPROVEMENT FEE PROGRAM

The applicant will be required to participate in this program.

## MULTI-MODAL/TRANSPORTATION DEMAND MANAGEMENT (TDM) OPPORTUNITIES

No multi-modal/transportation demand management (TDM) roadway improvement projects have been identified as being needed by the year 2040 per Map 15 and Table 5 of El Paso County's 2016 MTCP.

Please refer to the Pedestrian and bicycle section above for details on sidewalk facilities and two nearby future regional trail extensions/connections.

## SUMMARY

## Trip Generation

- During the morning peak hour, about 359 vehicles would enter and 318 vehicles would exit the site.
- Approximately 263 vehicles would enter and 263 vehicles would exit the site during the school afternoon peak hour.
- During the PM peak hour, about 13 vehicles would enter and 13 vehicles would exit the site.


## Pedestrian and Bicycle Accessibility

- Please refer to the section of the report for details on existing sidewalk locations in the area. Two planned major regional trail corridors intersect near the site. This will provide excellent pedestrian and bicycle accessibility in the future once these trails are established.


## Projected Levels of Service

- All individual turning movements and single-lane approaches at the proposed site-access points are projected to operate at LOS D or better through the 20-year horizon following the opening of the charter school. Please refer to the "Level of Service" section for details.
- Please refer to the "Level of Service" section for analysis and results at the two offsite intersections analyzed.


## Auxiliary Turn-Lane Needs Analysis

- Please refer to the "Auxiliary Turn-Lane Analysis" section for details.
- Regarding the site-access points and adjacent section of Waynoka Place (see Figure 12)
- Figure 13 - Figure 15 show a preliminary laneage concept. LSC will assist the design team with the detailed configuration of the access points, access radii, alignment and width, associated laneage, striping of Waynoka Place, etc. at the design stage.


## On-Site Traffic Operations Concepts

Please refer to Figures 16a (opening year)/16b (max enrollment) for preliminary concepts for the parent car line stacking, drop-off and pick-up zones, and inter-campus bus routing.

Please contact me if you have any questions regarding this report.

Respectfully Submitted,
LSC TRANSPORTATION CONSULTANTS, INC.

By: Jeffrey C. Hodsdon, P.E.
Principal
JCH/JAB:jas

Enclosures: Table 9
Figure 1 - Figure 17
Traffic Count Reports
Synchro Level of Service Reports
Queuing Reports
MTCP Maps
Appendix Figure 1 - ZIP Code Data

Table 9

Table 9: Detailed Trip Generation Estimate

| School Year | ITE |  | Inputs |  | Trip Generation Rates ${ }^{4}$ |  |  |  |  |  |  | Driveway Trips Generated |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Code | Description | Values | Units | Average Weekday | A.M. |  | Mid-Day ${ }^{5}$ |  | P.M. ${ }^{6}$ |  | Average Weekday | A.M. |  | Mid-Day |  | P.M. |  |
|  |  |  |  |  |  | In | Out | In | Out | In | Out |  | In | Out | In | Out | In | Out |
| Previous Land Use |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| - | 710 | General Office | 82.235 | KSF | 10.84 | 1.34 | 0.18 | 0.12 | 0.60 | 0.24 | 1.20 | 891 | 110 | 15 | 10 | 49 | 20 | 98 |
| Based on ITE Rates |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2023-2024 (Short Term) | 538 | Charter School ( $\mathrm{K}-12$ ) | 359 | Students | - | 0.50 | 0.44 | 0.37 | 0.37 | 0.02 | 0.02 | - | 179 | 159 | 131 | 131 | 7 | 7 |
| 2024-2025 | 538 | Charter School ( $\mathrm{K}-12$ ) | 395 | Students | - | 0.50 | 0.44 | 0.37 | 0.37 | 0.02 | 0.02 | - | 197 | 175 | 144 | 144 | 7 | 7 |
| 2025-2026 | 538 | Charter School ( $\mathrm{K}-12$ ) | 489 | Students | - | 0.50 | 0.44 | 0.37 | 0.37 | 0.02 | 0.02 | - | 244 | 216 | 178 | 178 | 9 | 9 |
| 2026-2027 | 538 | Charter School (K-12) | 525 | Students | - | 0.50 | 0.44 | 0.37 | 0.37 | 0.02 | 0.02 | - | 262 | 232 | 192 | 192 | 10 | 10 |
| 2027-2028 | 538 | Charter School ( K -12) | 574 | Students | - | 0.50 | 0.44 | 0.37 | 0.37 | 0.02 | 0.02 | - | 286 | 254 | 210 | 210 | 10 | 10 |
| 2028-2029 | 538 | Charter School (K-12) | 623 | Students | - | 0.50 | 0.44 | 0.37 | 0.37 | 0.02 | 0.02 | - | 310 | 275 | 227 | 227 | 11 | 11 |
| Max Enrollment (Long Term) | 538 | Charter School (K-12) | 720 | Students | - | 0.50 | 0.44 | 0.37 | 0.37 | 0.02 | 0.02 | - | 359 | 318 | 263 | 263 | 13 | 13 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Trip Generation Comparison -- Opening Year |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| - - | 710 | General Office | 82.235 | KSF | 10.84 | 1.34 | 0.18 | 0.12 | 0.60 | 0.24 | 1.20 | 891 | 110 | 15 | 10 | 49 | 20 | 98 |
| 2023-2024 (Short Term) | 538 | Charter School (K-12) | 359 | Students | - | 0 | 0 | 0 | 0 | 0 | 0 | - | 179 | 159 | 131 | 131 | 7 | 7 |
|  |  |  |  |  |  |  |  |  |  | Diff | rence | - | 69 | 144 | 121 | 82 | -14 | -92 |
| Trip Generation Comparison -- Max Enrollment Year |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| - - | 710 | General Office | 82.235 | KSF | 10.84 | 1.34 | 0.18 | 0.12 | 0.60 | 0.24 | 1.20 | 891 | 110 | 15 | 10 | 49 | 20 | 98 |
| Max Enrollment (Long Term) | 538 | Charter School (K-12) | 720 | Students | - | 0 | 0 | 0 | 0 | 0 | 0 | - | 359 | 318 | 263 | 263 | 13 | 13 |
|  |  |  |  |  |  |  |  |  |  | Diff | rence | - | 249 | 303 | 253 | 214 | -7 | -85 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ${ }^{1}$ Assumes 1.5 students per vehicle for on-campus students |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ${ }^{2}$ Does not include approximately 100 students who will be transported from/to other campuses to this site at the start/end of each school day from 2 buses and 2 vans off-camp |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ${ }^{3}$ KSF $=1,000$ square feet |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ${ }^{4}$ Source: Trip Generation, 11th Edition (2021) by the Institute of Transportation Engineers (ITE) |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ${ }^{5}$ Assumes PM peak trip generation is 5\% of School PM (mid-day) trip generation |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ${ }^{6}$ Assumes mid-day peak trip generation is $50 \%$ of PM trip generation |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Figures 1-17










Figure 7
Site-Generated Traffic (Full-Enrollment)






Figure 12






$=\mathrm{=}=\mathrm{=}=$ Major Roadway (Unincorporated El Paso County) ${ }^{(4)}$

- Pedestrian/Non-motorized Trips Requiring bussing per the City of Colorado Springs ${ }^{(2)}$

Signalized intersection with pedestrian crossing(s)

## Traffic Counts

# LSC Transportation Consultants, Inc. <br> 2504 E. Pikes Peak Ave, Suite 304 <br> Colorado Springs, CO 80909 <br> 719-633-2868 

File Name : Waynoka PI - Waynoka Rd AM
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Groups Printed- Unshifted

|  | Waynoka PI Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Int. Total |
| 06:45 | 2 | 0 | 25 | 0 | 27 | 13 | 3 | 0 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 1 | 0 | 7 | 50 |
| Total | 2 | 0 | 25 | 0 | 27 | 13 | 3 | 0 | 0 | 16 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 1 | 0 | 7 | 50 |


| $07: 00$ | 5 | 0 | 21 | 0 | 26 | 5 | 9 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 4 | 0 | 21 | 61 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $07: 15$ | 5 | 0 | 16 | 0 | 21 | 9 | 5 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 4 | 0 | 8 | 43 |
| $07: 30$ | 6 | 0 | 18 | 0 | 24 | 17 | 9 | 0 | 0 | 26 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 6 | 0 | 11 | 61 |
| $07: 45$ | 3 | 0 | 41 | 0 | 44 | 17 | 2 | 0 | 0 | 19 | 0 | 0 | 1 | 1 | 2 | 0 | 8 | 6 | 0 | 14 | 79 |
| Total | 19 | 0 | 96 | 0 | 115 | 48 | 25 | 0 | 0 | 73 | 0 | 0 | 1 | 1 | 2 | 1 | 33 | 20 | 0 | 54 | 244 |
| Grand Total | 21 | 0 | 121 | 0 | 142 | 61 | 28 | 0 | 0 | 89 | 0 | 0 | 1 | 1 | 2 | 1 | 39 | 21 | 0 | $61 \mid$ | 294 |
| Apprch \% | 14.8 | 0 | 85.2 | 0 |  | 68.5 | 31.5 | 0 | 0 |  | 0 | 0 | 50 | 50 |  | 1.6 | 63.9 | 34.4 | 0 |  |  |
| Total \% | 7.1 | 0 | 41.2 | 0 | 48.3 | 20.7 | 9.5 | 0 | 0 | 30.3 | 0 | 0 | 0.3 | 0.3 | 0.7 | 0.3 | 13.3 | 7.1 | 0 | 20.7 |  |

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|  | Waynoka PI Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 6:45:00 AM to 7:45:00 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 7:00:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:00:00 AM | 5 | 0 | 21 | 0 | 26 | 5 | 9 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 4 | 0 | 21 | 61 |
| 7:15:00 AM | 5 | 0 | 16 | 0 | 21 | 9 | 5 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 4 | 0 | 8 | 43 |
| 7:30:00 AM | 6 | 0 | 18 | 0 | 24 | 17 | 9 | 0 | 0 | 26 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 6 | 0 | 11 | 61 |
| 7:45:00 AM | 3 | 0 | 41 | 0 | 44 | 17 | 2 | 0 | 0 | 19 | 0 | 0 | 1 | 1 | 2 | 0 | 8 | 6 | 0 | 14 | 79 |
| Total Volume | 19 | 0 | 96 | 0 | 115 | 48 | 25 | 0 | 0 | 73 | 0 | 0 | 1 | 1 | 2 | 1 | 33 | 20 | 0 | 54 | 244 |
| \% App. Total | 16.5 | 0 | 83.5 | 0 |  | 65.8 | 34.2 | 0 | 0 |  | 0 | 0 | 50 | 50 |  | 1.9 | 61.1 | 37 | 0 |  |  |
| PHF | . 792 | . 000 | . 585 | . 000 | . 653 | . 706 | . 694 | . 000 | . 000 | . 702 | . 000 | . 000 | . 250 | . 250 | . 250 | . 250 | . 485 | . 833 | . 000 | . 643 | 772 |



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|  | Waynoka PI Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toala | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toala |  |
| Peak Hour Analysis From 6:45:00 AM to 7:45:00 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| +0 mins. | $\begin{array}{\|cc\|} \hline 7,00: 00 \mathrm{AM} \\ 5 \end{array}$ | 0 | 21 | 0 | 26 | 7:00:00 An 5 | 9 | 0 | 0 | 14 | ${ }^{\text {770:000 AM }}$ | 0 | 0 | 0 | 0 | ${ }^{\text {7:00:00 Al }}$ | 17 | 4 | 0 | 21 |  |
| +5 mins. | 5 | 0 | 16 | 0 | 21 | 9 | 5 | 0 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 1 | 3 | 4 | 0 | 8 |  |
| +10 mins. | 6 | 0 | 18 | 0 | 24 | 17 | 9 | 0 | 0 | 26 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 6 | 0 | 11 |  |
| +15 mins. | 3 | 0 | 41 | 0 | 44 | 17 | 2 | 0 | 0 | 19 | 0 | 0 | 1 | 1 | 2 | 0 | 8 | 6 | 0 | 14 |  |
| Total Volume | 19 | 0 | 96 | 0 | 115 | 48 | 25 | 0 | 0 | 73 | 0 | 0 | 1 | 1 | 2 | , | 33 | 20 | 0 | 54 |  |
| \% App. Total | 16.5 | 0 | 83.5 | 0 |  | 65.8 | 34.2 | 0 | 0 |  | 0 | 0 | 50 | 50 |  | 1.9 | 61.1 | 37 | 0 |  |  |
| PHF | . 792 | . 000 | . 585 | . 000 | . 653 | . 706 | . 694 | . 000 | . 000 | . 702 | . 000 | . 000 | . 250 | . 250 | . 250 | . 250 | . 485 | . 833 | . 000 | . 643 |  |


|  |  |  |
| :---: | :---: | :---: |
|  | Peak Hour Data <br> Unshifted |  |
|  | In - Peak Hour: 07:00 |  |

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File Name : Waynoka PI - Waynoka Rd Mid
Site Code : S224370
Start Date : 6/9/2022
Page No : 1

Groups Printed- Unshifted

|  | Waynoka PI Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Int. Total |
| 14:30 | 5 | 0 | 20 | 0 | 25 | 20 | 9 | 0 | 0 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 9 | 0 | 15 | 69 |
| 14:45 | 3 | 0 | 24 | 0 | 27 | 16 | 6 | 0 | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 11 | 0 | 17 | 66 |
| Total | 8 | 0 | 44 | 0 | 52 | 36 | 15 | 0 | 0 | 51 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 20 | 0 | 32 | 135 |
| 15:00 | 8 | 0 | 32 | 0 | 40 | 38 | 13 | 0 | 0 | 51 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 7 | 0 | 11 | 102 |
| 15:15 | 3 | 0 | 31 | 0 | 34 | 28 | 7 | 0 | 0 | 35 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 17 | 0 | 20 | 89 |
| Grand Total | 19 | 0 | 107 | 0 | 126 | 102 | 35 | 0 | 0 | 137 | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 44 | 0 | 63 | 326 |
| Apprch \% | 15.1 | 0 | 84.9 | 0 |  | 74.5 | 25.5 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 30.2 | 69.8 | 0 |  |  |
| Total \% | 5.8 | 0 | 32.8 | 0 | 38.7 | 31.3 | 10.7 | 0 | 0 | 42 | 0 | 0 | 0 | 0 | 0 | 0 | 5.8 | 13.5 | 0 | 19.3 |  |

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|  | Waynoka PI Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toat | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal |  |
| Peak Hour Analysis From 2:30:00 PM to 3:15:00 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 2:30:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:30:00 PM | 5 | 0 | 20 | 0 | 25 | 20 | 9 | 0 | 0 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 9 | 0 | 15 | 69 |
| 2:45:00 PM | 3 | 0 | 24 | 0 | 27 | 16 | 6 | 0 | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 11 | 0 | 17 | 66 |
| 3:00:00 PM | 8 | 0 | 32 | 0 | 40 | 38 | 13 | 0 | 0 | 51 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 7 | 0 | 11 | 102 |
| 3:15:00 PM | 3 | 0 | 31 | 0 | 34 | 28 | 7 | 0 | 0 | 35 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 17 | 0 | 20 | 89 |
| Total Volume | 19 | 0 | 107 | 0 | 126 | 102 | 35 | 0 | 0 | 137 | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 44 | 0 | 63 | 326 |
| \% App. Total | 15.1 | 0 | 84.9 | 0 |  | 74.5 | 25.5 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 30.2 | 69.8 | 0 |  |  |
| PHF | . 594 | . 000 | . 836 | . 000 | . 788 | . 671 | . 673 | . 000 | . 000 | . 672 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 792 | . 647 | . 000 | . 788 | 799 |



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Start Date : 6/9/2022
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|  | Waynoka PI Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total |  |
| Peak Hour Analysis From 2:30:00 PM to 3:15:00 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| +0 mins | $\begin{gathered} \text { 2:30:00 PM } \\ 5 \end{gathered}$ | 0 | 20 | 0 | 25 | 2:30:00 рм $20$ | 9 | 0 | 0 | 29 | 2:30:00 PM | 0 | 0 | 0 | 0 | 2:30:00 P1 | 6 | 9 | 0 | 15 |  |
| +5 mins. | 3 | 0 | 24 | 0 | 27 | 16 | 6 | 0 | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 11 | 0 | 17 |  |
| +10 mins. | 8 | 0 | 32 | 0 | 40 | 38 | 13 | 0 | 0 | 51 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 7 | 0 | 11 |  |
| +15 mins. | 3 | 0 | 31 | 0 | 34 | 28 | 7 | 0 | 0 | 35 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 17 | 0 | 20 |  |
| Total Volume | 19 | 0 | 107 | 0 | 126 | 102 | 35 | 0 | 0 | 137 | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 44 | 0 | 63 |  |
| \% App. Total | 15.1 | 0 | 84.9 | 0 |  | 74.5 | 25.5 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 30.2 | 69.8 | 0 |  |  |
| PHF | . 594 | . 000 | . 836 | . 000 | . 788 | . 671 | . 673 | . 000 | . 000 | . 672 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 792 | . 647 | . 000 | . 788 |  |



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Groups Printed- Unshifted

|  | Waynoka PI Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toala | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | int. Total |
| 16:00 | 4 | 0 | 20 | 0 | 24 | 31 | 6 | 0 | 0 | 37 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 25 | 0 | 32 | 93 |
| 16:15 | 3 | 0 | 25 | 0 | 28 | 47 | 8 | 0 | 0 | 55 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 19 | 0 | 26 | 109 |
| 16:30 | 2 | 0 | 24 | 0 | 26 | 44 | 3 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 23 | 0 | 28 | 101 |
| 16:45 | 4 | 0 | 21 | 1 | 26 | 43 | 4 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 34 | 0 | 40 | 113 |
| Total | 13 | 0 | 90 | 1 | 104 | 165 | 21 | 0 | 0 | 186 | 0 | 0 | 0 | 0 | 0 | 0 | 25 | 101 | 0 | 126 | 416 |
| 17:00 | 2 | 0 | 23 | 0 | 25 | 32 | 1 | 0 | 0 | 33 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 50 | 0 | 59 | 117 |
| 17:15 | 3 | 0 | 20 | 0 | 23 | 40 | 3 | 0 | 0 | 43 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 18 | 0 | 26 | 92 |
| 17:30 | 3 | 0 | 23 | 0 | 26 | 42 | 1 | 0 | 0 | 43 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 8 | 0 | 11 | 80 |
| 17:45 | 3 | 0 | 28 | 0 | 31 | 18 | 2 | 0 | 0 | 20 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 14 | 0 | 17 | 68 |
| Total | 11 | 0 | 94 | 0 | 105 | 132 | 7 | 0 | 0 | 139 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 90 | 0 | 113 | 357 |
| Grand Total | 24 | 0 | 184 | 1 | 209 | 297 | 28 | 0 | 0 | 325 | 0 | 0 | 0 | 0 | 0 | 0 | 48 | 191 | 0 | 239 | 773 |
| Apprch \% | 11.5 | 0 | 88 | 0.5 |  | 91.4 | 8.6 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 20.1 | 79.9 | 0 |  |  |
| Total \% | 3.1 | 0 | 23.8 | 0.1 | 27 | 38.4 | 3.6 | 0 | 0 | 42 | 0 | 0 | 0 | 0 | 0 | 0 | 6.2 | 24.7 | 0 | 30.9 |  |

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Start Date: 6/9/2022
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|  | Waynoka PI Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 4:00:00 PM to 5:45:00 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 4:15:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:15:00 PM | 3 | 0 | 25 | 0 | 28 | 47 | 8 | 0 | 0 | 55 | 0 | 0 | 0 | 0 | 0 | 0 | 7 | 19 | 0 | 26 | 109 |
| 4:30:00 PM | 2 | 0 | 24 | 0 | 26 | 44 | 3 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 23 | 0 | 28 | 101 |
| 4:45:00 PM | 4 | 0 | 21 | 1 | 26 | 43 | 4 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 34 | 0 | 40 | 113 |
| 5:00:00 PM | 2 | 0 | 23 | 0 | 25 | 32 | 1 | 0 | 0 | 33 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 50 | 0 | 59 | 117 |
| Total Volume | 11 | 0 | 93 | 1 | 105 | 166 | 16 | 0 | 0 | 182 | 0 | 0 | 0 | 0 | 0 | 0 | 27 | 126 | 0 | 153 | 440 |
| \% App. Total | 10.5 | 0 | 88.6 | 1 |  | 91.2 | 8.8 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 17.6 | 82.4 | 0 |  |  |
| PHF | . 688 | . 000 | . 930 | . 250 | . 938 | . 883 | . 500 | . 000 | . 000 | . 827 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 750 | . 630 | . 000 | . 648 | . 940 |



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|  | Waynoka PI Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total |  |
| Peak Hour Analysis From 4:00:00 PM to 5:45:00 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| +0 mins. | $\begin{gathered} \text { 4:15:00 P } \\ 3 \end{gathered}$ | 0 | 25 | 0 | 28 | $\begin{array}{\|c} \text { 4:00:00 PM } \\ 31 \end{array}$ | 6 | 0 | 0 | 37 | $\begin{gathered} \text { 4:00:00 PM } \\ 0 \end{gathered}$ | 0 | 0 | 0 | 0 | $\begin{gathered} \text { 4:15:0 PN } \\ 0 \end{gathered}$ | 7 | 19 | 0 | 26 |  |
| +5 mins. | 2 | 0 | 24 | 0 | 26 | 47 | 8 | 0 | 0 | 55 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 23 | 0 | 28 |  |
| +10 mins. | 4 | 0 | 21 | 1 | 26 | 44 | 3 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 34 | 0 | 40 |  |
| +15 mins. | 2 | 0 | 23 | 0 | 25 | 43 | 4 | 0 | 0 | 47 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 50 | 0 | 59 |  |
| Total Volume | 11 | 0 | 93 | 1 | 105 | 165 | 21 | 0 | 0 | 186 | 0 | 0 | 0 | 0 | 0 | 0 | 27 | 126 | 0 | 153 |  |
| \% App. Total | 10.5 | 0 | 88.6 | 1 |  | 88.7 | 11.3 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 17.6 | 82.4 | 0 |  |  |
| PHF | . 688 | . 000 | . 930 | . 250 | . 938 | . 878 | . 656 | . 000 | . 000 | . 845 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 750 | . 630 | . 000 | . 648 |  |



# LSC Transportation Consultants, Inc. <br> 2504 E. Pikes Peak Ave, Suite 304 <br> Colorado Springs, CO 80909 <br> 719-633-2868 

File Name : Waynoka Rd - Palmer Park Blvd AM Mid Site Code : S224370
Start Date :7/19/2022
Page No : 1

Groups Printed- Unshifted

|  | Waynoka Rd Southbound |  |  |  |  | Palmer Park Blvd Westbound |  |  |  |  | Northbound |  |  |  |  | Palmer Park Blvd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Int. Total |
| 06:30 | 6 | 0 | 4 | 0 | 10 | 5 | 116 | 0 | 0 | 121 | 0 | 0 | 0 | 0 | 0 | 0 | 70 | 19 | 0 | 89 | 220 |
| 06:45 | 6 | 0 | 6 | 0 | 12 | 1 | 81 | 0 | 0 | 82 | 0 | 0 | 0 | 0 | 0 | 0 | 119 | 20 | 0 | 139 | 233 |
| Total | 12 | 0 | 10 | 0 | 22 | 6 | 197 | 0 | 0 | 203 | 0 | 0 | 0 | 0 | 0 | 0 | 189 | 39 | 0 | 228 | 453 |
| 07:00 | 16 | 0 | 10 | 0 | 26 | 4 | 128 | 0 | 0 | 132 | 0 | 0 | 0 | 0 | 0 | 0 | 88 | 20 | 0 | 108 | 266 |
| 07:15 | 8 | 0 | 6 | 0 | 14 | 7 | 141 | 0 | 0 | 148 | 0 | 0 | 0 | 0 | 0 | 0 | 98 | 12 | 0 | 110 | 272 |
| 07:30 | 12 | 0 | 6 | 0 | 18 | 7 | 168 | 0 | 0 | 175 | 0 | 0 | 0 | 0 | 0 | 0 | 90 | 15 | 0 | 105 | 298 |
| 07:45 | 12 | 0 | 9 | 0 | 21 | 7 | 126 | 0 | 0 | 133 | 0 | 0 | 0 | 0 | 0 | 0 | 111 | 24 | 0 | 135 | 289 |
| Total | 48 | 0 | 31 | 0 | 79 | 25 | 563 | 0 | 0 | 588 | 0 | 0 | 0 | 0 | 0 | 0 | 387 | 71 | 0 | 458 | 1125 |
| 08:00 | 15 | 0 | 5 | 0 | 20 | 7 | 103 | 0 | 0 | 110 | 0 | 0 | 0 | 0 | 0 | 0 | 67 | 12 | 0 | 79 | 209 |
| 08:15 | 18 | 0 | 8 | 0 | 26 | 2 | 99 | 0 | 0 | 101 | 0 | 0 | 0 | 0 | 0 | 0 | 87 | 8 | 0 | 95 | 222 |
| *** BREAK |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Total | 33 | 0 | 13 | 0 | 46 | 9 | 202 | 0 | 0 | 211 | 0 | 0 | 0 | 0 | 0 | 0 | 154 | 20 | 0 | 174 | 431 |

*** BREAK ***

| 13:30 | 17 | 0 | 7 | 0 | 24 | 7 | 85 | 0 | 0 | 92 | 0 | 0 | 0 | 0 | 0 | 0 | 105 | 13 | 0 | 118 | 234 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 13:45 | 17 | 0 | 8 | 0 | 25 | 4 | 87 | 0 | 0 | 91 | 0 | 0 | 0 | 0 | 0 | 0 | 96 | 17 | 0 | 113 | 229 |
| Total | 34 | 0 | 15 | 0 | 49 | 11 | 172 | 0 | 0 | 183 | 0 | 0 | 0 | 0 | 0 | 0 | 201 | 30 | 0 | 231 | 463 |
| 14:00 | 24 | 0 | 8 | 0 | 32 | 8 | 89 | 0 | 0 | 97 | 0 | 0 | 0 | 0 | 0 | 0 | 83 | 19 | 0 | 102 | 231 |
| 14:15 | 20 | 0 | 11 | 0 | 31 | 8 | 103 | 0 | 0 | 111 | 0 | 0 | 0 | 0 | 0 | 0 | 116 | 26 | 0 | 142 | 284 |
| 14:30 | 16 | 0 | 10 | 0 | 26 | 8 | 87 | 0 | 0 | 95 | 0 | 0 | 0 | 0 | 0 | 0 | 119 | 18 | 0 | 137 | 258 |
| 14:45 | 25 | 0 | 13 | 0 | 38 | 10 | 77 | 0 | 0 | 87 | 0 | 0 | 0 | 0 | 0 | 1 | 148 | 21 | 0 | 170 | 295 |
| Total | 85 | 0 | 42 | 0 | 127 | 34 | 356 | 0 | 0 | 390 | 0 | 0 | 0 | 0 | 0 | 1 | 466 | 84 | 0 | 551 | 1068 |
| 15:00 | 40 | 0 | 13 | 0 | 53 | 9 | 110 | 0 | 0 | 119 | 0 | 0 | 0 | 0 | 0 | 0 | 123 | 8 | 0 | 131 | 303 |
| 15:15 | 23 | 0 | 6 | 0 | 29 | 13 | 118 | 0 | 1 | 132 | 0 | 0 | 0 | 0 | 0 | 0 | 153 | 20 | 0 | 173 | 334 |
| Grand Total | 275 | 0 | 130 | 0 | 405 | 107 | 1718 | 0 | 1 | 1826 | 0 | 0 | 0 | 0 | 0 | 1 | 1673 | 272 | 0 | 1946 | 4177 |
| Apprch \% | 67.9 | 0 | 32.1 | 0 |  | 5.9 | 94.1 | 0 | 0.1 |  | 0 | 0 | 0 | 0 |  | 0.1 | 86 | 14 | 0 |  |  |
| Total \% | 6.6 | 0 | 3.1 | 0 | 9.7 | 2.6 | 41.1 | 0 | 0 | 43.7 | 0 | 0 | 0 | 0 | 0 |  | 40.1 | 6.5 | 0 | 46.6 |  |

## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd - Palmer Park Blvd AM Mid
Site Code : S224370
Start Date: 7/19/2022
Page No : 2


## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd - Palmer Park Blvd AM Mid
Site Code : S224370
Start Date : 7/19/2022
Page No : 3

|  | Waynoka Rd Southbound |  |  |  |  | Palmer Park Blvd Westbound |  |  |  |  | Northbound |  |  |  |  | Palmer Park Blvd Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total |  |
| Peak Hour Analysis From 6:30:00 AM to 3:15:00 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 2:30:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:30:00 PM | 16 | 0 | 10 | 0 | 26 | 8 | 87 | 0 | 0 | 95 | 0 | 0 | 0 | 0 | 0 | 0 | 119 | 18 | 0 | 137 | 258 |
| 2:45:00 PM | 25 | 0 | 13 | 0 | 38 | 10 | 77 | 0 | 0 | 87 | 0 | 0 | 0 | 0 | 0 | 1 | 148 | 21 | 0 | 170 | 295 |
| 3:00:00 PM | 40 | 0 | 13 | 0 | 53 | 9 | 110 | 0 | 0 | 119 | 0 | 0 | 0 | 0 | 0 | 0 | 123 | 8 | 0 | 131 | 303 |
| 3:15:00 PM | 23 | 0 | 6 | 0 | 29 | 13 | 118 | 0 | 1 | 132 | 0 | 0 | 0 | 0 | 0 | 0 | 153 | 20 | 0 | 173 | 334 |
| Total Volume | 104 | 0 | 42 | 0 | 146 | 40 | 392 | 0 | 1 | 433 | 0 | 0 | 0 | 0 | 0 | 1 | 543 | 67 | 0 | 611 | 1190 |
| \% App. Total | 71.2 | 0 | 28.8 | 0 |  | 9.2 | 90.5 | 0 | 0.2 |  | 0 | 0 | 0 | 0 |  | 0.2 | 88.9 | 11 | 0 |  |  |
| PHF | . 650 | . 000 | . 808 | . 000 | . 689 | . 769 | . 831 | . 000 | . 250 | . 820 | . 000 | . 000 | . 000 | . 000 | . 000 | . 250 | . 887 | . 798 | . 000 | . 883 | . 891 |



## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd - Palmer Park Blvd AM Mid
Site Code : S224370
Start Date: 7/19/2022
Page No : 4

|  | Waynoka Rd Southbound |  |  |  |  | Palmer Park Blvd Westbound |  |  |  |  | Northbound |  |  |  |  | Palmer Park Blvd Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total |  |
| Peak Hour Analysis From 6:30:00 AM to 3:15:00 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | 2:15:00 PM |  |  |  |  | 7:00:00 AM |  |  |  |  | 6:30:00 AM |  |  |  |  | 2:30:00 PM |  |  |  |  |  |
| +0 mins. | 20 | 0 | 11 | 0 | 31 | 4 | 128 | 0 | 0 | 132 | 0 | 0 | 0 | 0 | 0 | 0 | 119 | 18 | 0 | 137 |  |
| +5 mins. | 16 | 0 | 10 | 0 | 26 | 7 | 141 | 0 | 0 | 148 | 0 | 0 | 0 | 0 | 0 | 1 | 148 | 21 | 0 | 170 |  |
| +10 mins. | 25 | 0 | 13 | 0 | 38 | 7 | 168 | 0 | 0 | 175 | 0 | 0 | 0 | 0 | 0 | 0 | 123 | 8 | 0 | 131 |  |
| +15 mins. | 40 | 0 | 13 | 0 | 53 | 7 | 126 | 0 | 0 | 133 | 0 | 0 | 0 | 0 | 0 | 0 | 153 | 20 | 0 | 173 |  |
| Total Volume | 101 | 0 | 47 | 0 | 148 | 25 | 563 | 0 | 0 | 588 | 0 | 0 | 0 | 0 | 0 | 1 | 543 | 67 | 0 | 611 |  |
| \% App. Total | 68.2 | 0 | 31.8 | 0 |  | 4.3 | 95.7 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0.2 | 88.9 | 11 | 0 |  |  |
| PHF | . 631 | . 000 | . 904 | . 000 | . 698 | . 893 | . 838 | . 000 | . 000 | . 840 | . 000 | . 000 | . 000 | . 000 | . 000 | . 250 | . 887 | . 798 | . 000 | . 883 |  |


|  |  |  |
| :---: | :---: | :---: |
|  | Peak Hour Data <br> Unshifted |  |
|  | In - Peak Hour: 06:30 |  |

# LSC Transportation Consultants, Inc. 

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd - Palmer Park Blvd PM
Site Code : S224370
Start Date : 7/21/2022
Page No : 1

Groups Printed- Unshifted

|  | Waynoka Rd Southbound |  |  |  |  | Palmer Park Blvd Westbound |  |  |  |  | Northbound |  |  |  |  | Palmer Park BIvd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Int. Total |
| 16:00 | 24 | 0 | 12 | 0 | 36 | 16 | 113 | 0 | 0 | 129 | 0 | 0 | 0 | 0 | 0 | 0 | 177 | 18 | 0 | 195 | 360 |
| 16:15 | 18 | 0 | 16 | 0 | 34 | 9 | 111 | 0 | 0 | 120 | 0 | 0 | 0 | 0 | 0 | 0 | 179 | 22 | 0 | 201 | 355 |
| 16:30 | 21 | 0 | 9 | 0 | 30 | 11 | 135 | 0 | 0 | 146 | 0 | 0 | 0 | 0 | 0 | 0 | 179 | 21 | 0 | 200 | 376 |
| 16:45 | 20 | 0 | 9 | 0 | 29 | 10 | 105 | 0 | 0 | 115 | 0 | 0 | 0 | 0 | 0 | 0 | 190 | 14 | 0 | 204 | 348 |
| Total | 83 | 0 | 46 | 0 | 129 | 46 | 464 | 0 | 0 | 510 | 0 | 0 | 0 | 0 | 0 | 0 | 725 | 75 | 0 | 800 | 1439 |
| 17:00 | 25 | 0 | 6 | 0 | 31 | 14 | 143 | 0 | 0 | 157 | 0 | 0 | 0 | 0 | 0 | 0 | 181 | 22 | 0 | 203 | 391 |
| 17:15 | 19 | 0 | 11 | 0 | 30 | 4 | 128 | 0 | 0 | 132 | 0 | 0 | 0 | 0 | 0 | 0 | 203 | 17 | 0 | 220 | 382 |
| 17:30 | 16 | 0 | 11 | 0 | 27 | 1 | 110 | 0 | 0 | 111 | 0 | 0 | 0 | 0 | 0 | 0 | 186 | 10 | 0 | 196 | 334 |
| 17:45 | 26 | 0 | 10 | 0 | 36 | 6 | 96 | 0 | 0 | 102 | 0 | 0 | 0 | 0 | 0 | 0 | 162 | 12 | 0 | 174 | 312 |
| Total | 86 | 0 | 38 | 0 | 124 | 25 | 477 | 0 | 0 | 502 | 0 | 0 | 0 | 0 | 0 | 0 | 732 | 61 | 0 | 793 | 1419 |
| Grand Total | 169 | 0 | 84 | 0 | 253 | 71 | 941 | 0 | 0 | 1012 | 0 | 0 | 0 | 0 | 0 | 0 | 1457 | 136 | 0 | 1593 | 2858 |
| Apprch \% | 66.8 | 0 | 33.2 | 0 |  | 7 | 93 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 91.5 | 8.5 | 0 |  |  |
| Total \% | 5.9 | 0 | 2.9 | 0 | 8.9 | 2.5 | 32.9 | 0 | 0 | 35.4 | 0 | 0 | 0 | 0 | 0 | 0 | 51 | 4.8 | 0 | 55.7 |  |



## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd - Palmer Park Blvd PM
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|  | Waynoka Rd Southbound |  |  |  |  | Palmer Park Blvd Westbound |  |  |  |  | Northbound |  |  |  |  | Palmer Park Blvd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 4:00:00 PM to 5:45:00 PM - Peak 1 of 1 <br> Peak Hour for Entire Intersection Begins at 4:30:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:30:00 PM | 21 | 0 | 9 | 0 | 30 | 11 | 135 | 0 | 0 | 146 | 0 | 0 | 0 | 0 | 0 | 0 | 179 | 21 | 0 | 200 | 376 |
| 4:45:00 PM | 20 | 0 | 9 | 0 | 29 | 10 | 105 | 0 | 0 | 115 | 0 | 0 | 0 | 0 | 0 | 0 | 190 | 14 | 0 | 204 | 348 |
| 5:00:00 PM | 25 | 0 | 6 | 0 | 31 | 14 | 143 | 0 | 0 | 157 | 0 | 0 | 0 | 0 | 0 | 0 | 181 | 22 | 0 | 203 | 391 |
| 5:15:00 PM | 19 | 0 | 11 | 0 | 30 | 4 | 128 | 0 | 0 | 132 | 0 | 0 | 0 | 0 | 0 | 0 | 203 | 17 | 0 | 220 | 382 |
| Total Volume | 85 | 0 | 35 | 0 | 120 | 39 | 511 | 0 | 0 | 550 | 0 | 0 | 0 | 0 | 0 | 0 | 753 | 74 | 0 | 827 | 1497 |
| \% App. Total | 70.8 | 0 | 29.2 | 0 |  | 7.1 | 92.9 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 91.1 | 8.9 | 0 |  |  |
| PHF | . 850 | . 000 | . 795 | . 000 | . 968 | . 696 | . 893 | . 000 | . 000 | . 876 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 927 | . 841 | . 000 | . 940 | . 957 |



## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd - Palmer Park Blvd PM
Site Code : S224370
Start Date : 7/21/2022
Page No : 3

|  | Waynoka Rd Southbound |  |  |  |  | Palmer Park Blvd Westbound |  |  |  |  | Northbound |  |  |  |  | Palmer Park Blvd Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total |  |
| Peak Hour Analysis From 4:00:00 PM to 5:45:00 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | 4:00:00 PM |  |  |  |  | 4:30:00 P1 |  |  |  |  | 4:00:00 PM |  |  |  |  | 4:30:00 PM |  |  |  |  |  |
| +0 mins. | 24 | 0 | 12 | 0 | 36 | 11 | 135 | 0 | 0 | 146 | 0 | 0 | 0 | 0 | 0 | 0 | 179 | 21 | 0 | 200 |  |
| +5 mins. | 18 | 0 | 16 | 0 | 34 | 10 | 105 | 0 | 0 | 115 | 0 | 0 | 0 | 0 | 0 | 0 | 190 | 14 | 0 | 204 |  |
| +10 mins. | 21 | 0 | 9 | 0 | 30 | 14 | 143 | 0 | 0 | 157 | 0 | 0 | 0 | 0 | 0 | 0 | 181 | 22 | 0 | 203 |  |
| +15 mins. | 20 | 0 | 9 | 0 | 29 | 4 | 128 | 0 | 0 | 132 | 0 | 0 | 0 | 0 | 0 | 0 | 203 | 17 | 0 | 220 |  |
| Total Volume | 83 | 0 | 46 | 0 | 129 | 39 | 511 | 0 | 0 | 550 | 0 | 0 | 0 | 0 | 0 | 0 | 753 | 74 | 0 | 827 |  |
| \% App. Total | 64.3 | 0 | 35.7 | 0 |  | 7.1 | 92.9 | 0 | 0 |  | 0 | 0 | 0 | 0 |  | 0 | 91.1 | 8.9 | 0 |  |  |
| PHF | . 865 | . 000 | . 719 | . 000 | . 896 | . 696 | . 893 | . 000 | . 000 | . 876 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 927 | . 841 | . 000 | . 940 |  |


|  |  |  |
| :---: | :---: | :---: |
|  | Peak Hour Data <br> Unshifted |  |
|  |  |  |

# LSC Transportation Consultants, Inc. <br> 2504 E. Pikes Peak Ave, Suite 304 <br> Colorado Springs, CO 80909 <br> 719-633-2868 

File Name : Waynoka Rd - Driveway Accesses AM
Site Code : S224370
Start Date : 6/9/2022
Page No : 1

Groups Printed- Bank 1

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Driveway Accesses Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toala | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Int. Total |
| 06:45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 |


| $07: 00$ | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 5 | 1 | 0 | 1 | 0 | 2 | 2 | 0 | 0 | 0 | 2 | 9 |
| ---: | ---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $07: 15$ | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 2 | 0 | 1 | 0 | 3 | 1 | 0 | 0 | 0 | 1 | 5 |
| $07: 30$ | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 3 |
| $07: 45$ | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 2 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 8 | 4 | 0 | 2 | 0 | 6 | 5 | 0 | 0 | 0 | 5 | 19 |


| Grand Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 11 | 4 | 0 | 2 | 0 | 6 | 5 | 0 | 0 | 0 | 5 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Apprch \% | 0 | 0 | 0 | 0 |  | 0 | 0 | 100 | 0 |  | 66.7 | 0 | 33.3 | 0 |  | 100 | 0 | 0 | 0 | 2 |

## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd - Driveway Accesses AM
Site Code : S224370
Start Date : 6/9/2022
Page No : 2

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Driveway Accesses Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toial | Right | Thru | Left | Peds | App. Toala | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toaal | Int. Total |
| Peak Hour Analysis From 6:45:00 AM to 7:45:00 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 6:45:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 6:45:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 |
| 7:00:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 5 | 1 | 0 | 1 | 0 | 2 | 2 | 0 | 0 | 0 | 2 | 9 |
| 7:15:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 2 | 0 | 1 | 0 | 3 | 1 | 0 | 0 | 0 | 1 | 5 |
| 7:30:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 3 |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 10 | 4 | 0 | 2 | 0 | 6 | 4 | 0 | 0 | 0 | 4 | 20 |
| \% App. Total | 0 | 0 | 0 | 0 |  | 0 | 0 | 100 | 0 |  | 66.7 | 0 | 33.3 | 0 |  | 100 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 500 | . 000 | . 500 | . 500 | . 000 | . 500 | . 000 | . 500 | . 500 | . 000 | . 000 | . 000 | . 500 | . 556 |



## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd - Driveway Accesses AM
Site Code : S224370
Start Date : 6/9/2022
Page No : 3

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Driveway Accesses Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Total |  |
| Peak Hour Analysis From 6:45:00 AM to 7:45:00 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| +0 mins. | $\begin{gathered} \text { 6:45:00 AM } \\ 0 \end{gathered}$ | 0 | 0 | 0 | 0 | ${ }^{\text {6,45:00 AN }}$ | 0 | 3 | 0 | 3 | ${ }^{\text {6.45:00 an }}$ | 0 | 0 | 0 | 0 | ${ }^{\text {7:00:00 an }}$ | 0 | 0 | 0 | 2 |  |
| +5 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 5 | 1 | 0 | 1 | 0 | 2 | 1 | 0 | 0 | 0 | 1 |  |
| +10 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 2 | 0 | 1 | 0 | 3 | 1 | 0 | 0 | 0 | 1 |  |
| +15 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 1 |  |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 10 | 4 | 0 | 2 | 0 | 6 | 5 | 0 | 0 | 0 | 5 |  |
| \% App. Total | 0 | 0 | 0 | 0 |  | 0 | 0 | 100 | 0 |  | 66.7 | 0 | 33.3 | 0 |  | 100 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 500 | . 000 | . 500 | . 500 | . 000 | . 500 | . 000 | . 500 | . 625 | . 000 | . 000 | . 000 | . 625 |  |



# LSC Transportation Consultants, Inc. <br> 2504 E. Pikes Peak Ave, Suite 304 <br> Colorado Springs, CO 80909 <br> 719-633-2868 

File Name : Waynoka Rd - Driveway Accesses Mid
Site Code : S224370
Start Date: 6/9/2022
Page No : 1

Groups Printed- Bank 1

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Driveway Accesses Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| 14:30 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 2 | 0 | 1 | 0 | 3 | 2 | 0 | 0 | 0 | 2 | 9 |
| 14:45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 3 | 0 | 5 | 0 | 8 | 2 | 0 | 0 | 0 | 2 | 14 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 8 | 5 | 0 | 6 | 0 | 11 | 4 | 0 | 0 | 0 | 4 | 23 |
| 15:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 2 | 0 | 5 | 0 | 7 | 1 | 0 | 0 | 0 | 1 | 11 |
| 15:15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 6 | 0 | 0 | 3 | 0 | 3 | 6 | 0 | 0 | 0 | 6 | 15 |
| Grand Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 0 | 17 | 7 | 0 | 14 | 0 | 21 | 11 | 0 | 0 | 0 | 11 | 49 |
| Apprch \% | 0 | 0 | 0 | 0 |  | 0 | 0 | 100 | 0 |  | 33.3 | 0 | 66.7 | 0 |  | 100 | 0 | 0 | 0 |  |  |
| Total \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 34.7 | 0 | 34.7 | 14.3 | 0 | 28.6 | 0 | 42.9 | 22.4 | 0 | 0 | 0 | 22.4 |  |

## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd-Driveway Accesses Mid
Site Code : S224370
Start Date: 6/9/2022
Page No :2

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Driveway Accesses Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 2:30:00 PM to 3:15:00 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 2:30:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:30:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 2 | 0 | 1 | 0 | 3 | 2 | 0 | 0 | 0 | 2 | 9 |
| 2:45:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 3 | 0 | 5 | 0 | 8 | 2 | 0 | 0 | 0 | 2 | 14 |
| 3:00:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 2 | 0 | 5 | 0 | 7 | 1 | 0 | 0 | 0 | 1 | 11 |
| 3:15:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 6 | 0 | 0 | 3 | 0 | 3 | 6 | 0 | 0 | 0 | 6 | 15 |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 0 | 17 | 7 | 0 | 14 | 0 | 21 | 11 | 0 | 0 | 0 | 11 | 49 |
| \% App. Total | 0 | 0 | 0 | 0 |  | 0 | 0 | 100 | 0 |  | 33.3 | 0 | 66.7 | 0 |  | 100 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 708 | . 000 | . 708 | . 583 | . 000 | . 700 | . 000 | . 656 | . 458 | . 000 | . 000 | . 000 | . 458 | . 817 |



## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd-Driveway Accesses Mid
Site Code : S224370
Start Date: 6/9/2022
Page No : 3

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Driveway Accesses Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total |  |
| Peak Hour Analysis From 2:30:00 PM to 3:15:00 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| +0 mins | 2:30:00 PM | 0 | 0 | 0 | 0 | 2:30:00 P1 | 0 | 4 | 0 | 4 | 2:30:00 PM | 0 | 1 | 0 | 3 | 2:30:00 P | 0 | 0 | 0 | 2 |  |
| +5 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 3 | 0 | 5 | 0 | 8 | 2 | 0 | 0 | 0 | 2 |  |
| +10 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 3 | 2 | 0 | 5 | 0 | 7 | 1 | 0 | 0 | 0 | 1 |  |
| +15 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 6 | 0 | 0 | 3 | 0 | 3 | 6 | 0 | 0 | 0 | 6 |  |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 0 | 17 | 7 | 0 | 14 | 0 | 21 | 11 | 0 | 0 | 0 | 11 |  |
| \% App. Total | 0 | 0 | 0 | 0 |  | 0 | 0 | 100 | 0 |  | 33.3 | 0 | 66.7 | 0 |  | 100 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 708 | . 000 | . 708 | . 583 | . 000 | . 700 | . 000 | . 656 | . 458 | . 000 | . 000 | . 000 | . 458 |  |



# LSC Transportation Consultants, Inc. <br> 2504 E. Pikes Peak Ave, Suite 304 <br> Colorado Springs, CO 80909 <br> 719-633-2868 

File Name : Waynoka Rd - Driveway Accesses PM
Site Code : S224370
Start Date : 6/9/2022
Page No : 1

Groups Printed- Bank 1

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Driveway Accesses Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Int. Total |
| 16:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 5 | 2 | 0 | 0 | 0 | 2 | 4 | 0 | 0 | 0 | 4 | 11 |
| 16:15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 5 | 0 | 0 | 0 | 5 | 3 | 0 | 0 | 0 | 3 | 10 |
| 16:30 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 1 | 0 | 6 | 2 | 0 | 0 | 0 | 2 | 8 |
| 16:45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 5 | 0 | 2 | 0 | 7 | 4 | 0 | 0 | 0 | 4 | 15 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 11 | 17 | 0 | 3 | 0 | 20 | 13 | 0 | 0 | 0 | 13 | 44 |
| 17:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 1 | 0 | 7 | 4 | 0 | 0 | 0 | 4 | 11 |
| 17:15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 4 | 0 | 5 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 11 |
| 17:30 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 4 | 0 | 3 | 0 | 7 | 5 | 0 | 0 | 0 | 5 | 14 |
| 17:45 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 5 | 0 | 2 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 9 |
| Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 6 | 19 | 0 | 11 | 0 | 30 | 9 | 0 | 0 | 0 | 9 | 45 |
| Grand Total | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 17 | 0 | 17 | 36 | 0 | 14 | 0 | 50 | 22 | 0 | 0 | 0 | 22 | 89 |
| Apprch \% | 0 | 0 | 0 | 0 |  | 0 | 0 | 100 | 0 |  | 72 | 0 | 28 | 0 |  | 100 | 0 | 0 | 0 |  |  |
| Total \% | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 19.1 | 0 | 19.1 | 40.4 | 0 | 15.7 | 0 | 56.2 | 24.7 | 0 | 0 | 0 | 24.7 |  |

## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd - Driveway Accesses PM
Site Code : S224370
Start Date : 6/9/2022
Page No : 2

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Driveway Accesses Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Total |  |
| Peak Hour Analysis From 4:00:00 PM to 5:45:00 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 4:45:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:45:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 5 | 0 | 2 | 0 | 7 | 4 | 0 | 0 | 0 | 4 | 15 |
| 5:00:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 1 | 0 | 7 | 4 | 0 | 0 | 0 | 4 | 11 |
| 5:15:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 4 | 0 | 5 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 11 |
| 5:30:00 PM | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 4 | 0 | 3 | 0 | 7 | 5 | 0 | 0 | 0 | 5 | 14 |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 8 | 19 | 0 | 11 | 0 | 30 | 13 | 0 | 0 | 0 | 13 | 51 |
| \% App. Total | 0 | 0 | 0 | 0 |  | 0 | 0 | 100 | 0 |  | 63.3 | 0 | 36.7 | 0 |  | 100 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 500 | . 000 | . 500 | . 792 | . 000 | . 550 | . 000 | . 833 | . 650 | . 000 | . 000 | . 000 | 650 | 850 |



## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Waynoka Rd - Driveway Accesses PM
Site Code : S224370
Start Date : 6/9/2022
Page No : 3

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Driveway Accesses Northbound |  |  |  |  | Waynoka Rd Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Appo Total | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toala |  |
| Peak Hour Analysis From 4:00:00 PM to 5:45:00 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| +0 mins. | $\begin{array}{\|c} 4.0000 \mathrm{Pm} \\ 0 \end{array}$ | 0 | 0 | 0 | 0 | ${ }^{\text {4.00:00 PM }} 0$ | 0 | 5 | 0 | 5 | $\begin{aligned} & \text { 4.45:00 PM } \\ & 5 \end{aligned}$ | 0 | 2 | 0 | 7 | 4.00:00 P1 <br> 4 | 0 | 0 | 0 | 4 |  |
| +5 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 2 | 6 | 0 | 1 | 0 | 7 | 3 | 0 | 0 | 0 | 3 |  |
| +10 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 5 | 0 | 9 | 2 | 0 | 0 | 0 | 2 |  |
| +15 mins. | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 4 | 4 | 0 | 3 | 0 | 7 | 4 | 0 | 0 | 0 | 4 |  |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 0 | 11 | 19 | 0 | 11 | 0 | 30 | 13 | 0 | 0 | 0 | 13 |  |
| \% App. Total | 0 | 0 | 0 | 0 |  | 0 | 0 | 100 | 0 |  | 63.3 | 0 | 36.7 | 0 |  | 100 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 000 | . 550 | . 000 | . 550 | . 792 | . 000 | . 550 | . 000 | . 833 | . 813 | . 000 | . 000 | . 000 | 813 |  |

# LSC Transportation Consultants, Inc. <br> 2504 E. Pikes Peak Ave, Suite 304 <br> Colorado Springs, CO 80909 <br> 719-633-2868 

File Name: Tutt Blvd - Constitution Ave AM
Site Code : S224370
Start Date : 6/28/2022
Page No : 1

Groups Printed- Unshifted

|  | Tutt Blvd Southbound |  |  |  |  | Constitution Av Westbound |  |  |  |  | Waynoka PI Northbound |  |  |  |  | Constitution Av Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | Apo. Total | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | po. Toal | Int. Total |
| 06:30 | 28 | 8 | 11 | 0 | 47 | 16 | 160 | 16 | 0 | 192 | 3 | 6 | 8 | 0 | 17 | 18 | 74 | 10 | 0 | 102 | 358 |
| 06:45 | 27 | 6 | 19 | 0 | 52 | 27 | 162 | 24 | 1 | 214 | 4 | 7 | 9 | 0 | 20 | 26 | 92 | 7 | 0 | 125 | 411 |
| Total | 55 | 14 | 30 | 0 | 99 | 43 | 322 | 40 | 1 | 406 | 7 | 13 | 17 | 0 | 37 | 44 | 166 | 17 | 0 | 227 | 769 |
| 07:00 | 28 | 9 | 16 | 0 | 53 | 21 | 186 | 15 | 0 | 222 | 4 | 4 | 8 | 0 | 16 | 12 | 76 | 8 | 0 | 96 | 387 |
| 07:15 | 33 | 10 | 15 | 0 | 58 | 31 | 218 | 24 | 0 | 273 | 4 | 5 | 8 | 0 | 17 | 16 | 99 | 17 | 0 | 132 | 480 |
| 07:30 | 31 | 13 | 20 | 0 | 64 | 25 | 218 | 16 | 0 | 259 | 5 | 6 | 6 | 0 | 17 | 19 | 92 | 14 | 0 | 125 | 465 |
| 07:45 | 23 | 13 | 23 | 0 | 59 | 27 | 201 | 12 | 0 | 240 | 5 | 7 | 4 | 0 | 16 | 23 | 95 | 16 | 0 | 134 | 449 |
| Total | 115 | 45 | 74 | 0 | 234 | 104 | 823 | 67 | 0 | 994 | 18 | 22 | 26 | 0 | 66 | 70 | 362 | 55 | 0 | 487 | 1781 |
| 08:00 | 19 | 12 | 18 | 0 | 49 | 18 | 176 | 15 | 0 | 209 | 5 | 3 | 12 | 0 | 20 | 13 | 87 | 17 | 0 | 117 | 395 |
| 08:15 | 29 | 8 | 22 | 0 | 59 | 16 | 171 | 16 | 0 | 203 | 5 | 8 | 12 | 0 | 25 | 16 | 109 | 20 | 0 | 145 | 432 |
| Grand Total | 218 | 79 | 144 | 0 | 441 | 181 | 1492 | 138 | 1 | 1812 | 35 | 46 | 67 | 0 | 148 | 143 | 724 | 109 | 0 | 976 | 3377 |
| Apprch \% | 49.4 | 17.9 | 32.7 | 0 |  | 10 | 82.3 | 7.6 | 0.1 |  | 23.6 | 31.1 | 45.3 | 0 |  | 14.7 | 74.2 | 11.2 | 0 |  |  |
| Total \% | 6.5 | 2.3 | 4.3 | 0 | 13.1 | 5.4 | 44.2 | 4.1 | 0 | 53.7 | 1 | 1.4 | 2 |  | 4.4 | 4.2 | 21.4 | 3.2 | 0 | 28.9 |  |

# LSC Transportation Consultants, Inc. 

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name: Tutt Blvd - Constitution Ave AM
Site Code : S224370
Start Date : 6/28/2022
Page No :2

|  | Tutt BIvd Southbound |  |  |  |  | Constitution Av Westbound |  |  |  |  | Waynoka PI Northbound |  |  |  |  | Constitution Av Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 6:30:00 AM to 8:15:00 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 7:15:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:15:00 AM | 33 | 10 | 15 | 0 | 58 | 31 | 218 | 24 | 0 | 273 | 4 | 5 | 8 | 0 | 17 | 16 | 99 | 17 | 0 | 132 | 480 |
| 7:30:00 AM | 31 | 13 | 20 | 0 | 64 | 25 | 218 | 16 | 0 | 259 | 5 | 6 | 6 | 0 | 17 | 19 | 92 | 14 | 0 | 125 | 465 |
| 7:45:00 AM | 23 | 13 | 23 | 0 | 59 | 27 | 201 | 12 | 0 | 240 | 5 | 7 | 4 | 0 | 16 | 23 | 95 | 16 | 0 | 134 | 449 |
| 8:00:00 AM | 19 | 12 | 18 | 0 | 49 | 18 | 176 | 15 | 0 | 209 | 5 | 3 | 12 | 0 | 20 | 13 | 87 | 17 | 0 | 117 | 395 |
| Total Volume | 106 | 48 | 76 | 0 | 230 | 101 | 813 | 67 | 0 | 981 | 19 | 21 | 30 | 0 | 70 | 71 | 373 | 64 | 0 | 508 | 1789 |
| \% App. Total | 46.1 | 20.9 | 33 | 0 |  | 10.3 | 82.9 | 6.8 | 0 |  | 27.1 | 30 | 42.9 | 0 |  | 14 | 73.4 | 12.6 | 0 |  |  |
| PHF | . 803 | . 923 | . 826 | . 000 | . 898 | . 815 | . 932 | . 698 | . 000 | . 898 | . 950 | . 750 | . 625 | . 000 | . 875 | . 772 | . 942 | . 941 | . 000 | . 948 | . 932 |



# LSC Transportation Consultants, Inc. 

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name: Tutt Blvd - Constitution Ave AM
Site Code : S224370
Start Date : 6/28/2022
Page No : 3

|  | Tutt Blvd Southbound |  |  |  |  | Constitution Av Westbound |  |  |  |  | Waynoka PI Northbound |  |  |  |  | Constitution Av Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | Apo. | Right | Thru | Left | Peds | App. Toal |  |

Peak Hour Analysis From 6:30:00 AM to 8:15:00 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 7:00:00 AM |  |  |  |  | 7:00:00 AM |  |  |  |  | 7:30:00 AM |  |  |  |  | 7:30:00 AM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 28 | 9 | 16 | 0 | 53 | 21 | 186 | 15 | 0 | 222 | 5 | 6 | 6 | 0 | 17 | 19 | 92 | 14 | 0 | 125 |
| +5 mins. | 33 | 10 | 15 | 0 | 58 | 31 | 218 | 24 | 0 | 273 | 5 | 7 | 4 | 0 | 16 | 23 | 95 | 16 | 0 | 134 |
| +10 mins. | 31 | 13 | 20 | 0 | 64 | 25 | 218 | 16 | 0 | 259 | 5 | 3 | 12 | 0 | 20 | 13 | 87 | 17 | 0 | 117 |
| +15 mins. | 23 | 13 | 23 | 0 | 59 | 27 | 201 | 12 | 0 | 240 | 5 | 8 | 12 | 0 | 25 | 16 | 109 | 20 | 0 | 145 |
| Total Volume | 115 | 45 | 74 | 0 | 234 | 104 | 823 | 67 | 0 | 994 | 20 | 24 | 34 | 0 | 78 | 71 | 383 | 67 | 0 | 521 |
| \% App. Total | 49.1 | 19.2 | 31.6 | 0 |  | 10.5 | 82.8 | 6.7 | 0 |  | 25.6 | 30.8 | 43.6 | 0 |  | 13.6 | 73.5 | 12.9 | 0 |  |
| PHF | . 871 | . 865 | . 804 | 000 | . 914 | . 839 | . 944 | . 698 | . 000 | . 910 | 1.000 | . 750 | . 708 | 000 | . 780 | . 772 | . 878 | . 838 |  | . 898 |


|  |  |  |
| :---: | :---: | :---: |
|  | Peak Hour Data <br> Unshifted |  |
|  |  |  |

# LSC Transportation Consultants, Inc. <br> 2504 E. Pikes Peak Ave, Suite 304 <br> Colorado Springs, CO 80909 <br> 719-633-2868 

File Name : Powers Blvd - Waynoka Rd AM
Site Code : S224370
Start Date : 6/9/2022
Page No : 1

Groups Printed- Unshifted

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Powers Blvd Northbound |  |  |  |  | Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toala | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Toala | int. Total |
| 06:45 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 7 | 19 | 451 | 0 | 0 | 470 | 0 | 0 | 0 | 0 | 0 | 477 |
| Total | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 7 | 19 | 451 | 0 | 0 | 470 | 0 | 0 | 0 | 0 | 0 | 477 |
| 07:00 | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 10 | 23 | 455 | 0 | 0 | 478 | 0 | 0 | 0 | 0 | 0 | 488 |
| 07:15 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 3 | 7 | 484 | 0 | 0 | 491 | 0 | 0 | 0 | 0 | 0 | 494 |
| 07:30 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 8 | 10 | 536 | 0 | 0 | 546 | 0 | 0 | 0 | 0 | 0 | 554 |
| 07:45 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 5 | 16 | 524 | 0 | 0 | 540 | 0 | 0 | 0 | 0 | 0 | 545 |
| Total | 0 | 0 | 0 | 0 | 0 | 26 | 0 | 0 | 0 | 26 | 56 | 1999 | 0 | 0 | 2055 | 0 | 0 | 0 | 0 | 0 | 2081 |
| Grand Total | 0 | 0 | 0 | 0 | 0 | 33 | 0 | 0 | 0 | 33 | 75 | 2450 | 0 | 0 | 2525 | 0 | 0 | 0 | 0 | 0 | 2558 |
| Apprch \% | 0 | 0 | 0 | 0 |  | 100 | 0 | 0 | 0 |  | 3 | 97 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| Total \% | 0 | 0 | 0 | 0 | 0 | 1.3 | 0 | 0 | 0 | 1.3 | 2.9 | 95.8 | 0 | 0 | 98.7 | 0 | 0 | 0 | 0 | 0 |  |

# LSC Transportation Consultants, Inc. <br> 2504 E. Pikes Peak Ave, Suite 304 <br> Colorado Springs, CO 80909 <br> 719-633-2868 

File Name : Powers Blvd - Waynoka Rd AM
Site Code : S224370
Start Date : 6/9/2022
Page No : 2

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Powers Blvd Northbound |  |  |  |  | Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal |  |
| Peak Hour Analysis From 6:45:00 AM to 7:45:00 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 7:00:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:00:00 AM | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 10 | 23 | 455 | 0 | 0 | 478 | 0 | 0 | 0 | 0 | 0 | 488 |
| 7:15:00 AM | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 3 | 7 | 484 | 0 | 0 | 491 | 0 | 0 | 0 | 0 | 0 | 494 |
| 7:30:00 AM | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 8 | 10 | 536 | 0 | 0 | 546 | 0 | 0 | 0 | 0 | 0 | 554 |
| 7:45:00 AM | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 5 | 16 | 524 | 0 | 0 | 540 | 0 | 0 | 0 | 0 | 0 | 545 |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 26 | 0 | 0 | 0 | 26 | 56 | 1999 | 0 | 0 | 2055 | 0 | 0 | 0 | 0 | 0 | 2081 |
| \% App. Total | 0 | 0 | 0 | 0 |  | 100 | 0 | 0 | 0 |  | 2.7 | 97.3 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 650 | . 000 | . 000 | . 000 | . 650 | . 609 | . 932 | . 000 | . 000 | . 941 | . 000 | . 000 | . 000 | . 000 | . 000 | . 939 |

## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Powers Blvd - Waynoka Rd AM
Site Code : S224370
Start Date: 6/9/2022
Page No : 3

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Powers Blvd Northbound |  |  |  |  | Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total |  |
| Peak Hour Analysis From 6:45:00 AM to 7:45:00 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | 6:45:00 AM |  |  |  |  | 6:45:00 AM |  |  |  |  | 7:00:00 AM |  |  |  |  | 6:45:00 AM |  |  |  |  |  |
| +0 mins. | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 7 | 23 | 455 | 0 | 0 | 478 | 0 | 0 | 0 | 0 | 0 |  |
| +5 mins. | 0 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 0 | 10 | 7 | 484 | 0 | 0 | 491 | 0 | 0 | 0 | 0 | 0 |  |
| +10 mins. | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 3 | 10 | 536 | 0 | 0 | 546 | 0 | 0 | 0 | 0 | 0 |  |
| +15 mins. | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 8 | 16 | 524 | 0 | 0 | 540 | 0 | 0 | 0 | 0 | 0 |  |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 28 | 0 | 0 | 0 | 28 | 56 | 1999 | 0 | 0 | 2055 | 0 | 0 | 0 | 0 | 0 |  |
| \% App. Total | 0 | 0 | 0 | 0 |  | 100 | 0 | 0 | 0 |  | 2.7 | 97.3 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 700 | . 000 | . 000 | . 000 | . 700 | . 609 | . 932 | . 000 | . 000 | . 941 | . 000 | . 000 | . 000 | . 000 | . 000 |  |



# LSC Transportation Consultants, Inc. <br> 2504 E. Pikes Peak Ave, Suite 304 <br> Colorado Springs, CO 80909 <br> 719-633-2868 

File Name : Powers Blvd - Waynoka Rd Mid Site Code : S224370
Start Date : 6/9/2022
Page No : 1

Groups Printed- Unshifted

|  | Driveway Accesses Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Powers Blvd Northbound |  |  |  |  | Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toala | Int. Total |
| 14:30 | 0 | 0 | 0 | 0 | 0 | 13 | 0 | 0 | 0 | 13 | 13 | 621 | 0 | 0 | 634 | 0 | 0 | 0 | 0 | 0 | 647 |
| 14:45 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 8 | 16 | 672 | 0 | 0 | 688 | 0 | 0 | 0 | 0 | 0 | 696 |
| Total | 0 | 0 | 0 | 0 | 0 | 21 | 0 | 0 | 0 | 21 | 29 | 1293 | 0 | 0 | 1322 | 0 | 0 | 0 | 0 | 0 | 1343 |
| 15:00 | 0 | 0 | 0 | 0 | 0 | 21 | 0 | 0 | 0 | 21 | 17 | 722 | 0 | 0 | 739 | 0 | 0 | 0 | 0 | 0 | 760 |
| 15:15 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 7 | 22 | 729 | 0 | 0 | 751 | 0 | 0 | 0 | 0 | 0 | 758 |
| Grand Total | 0 | 0 | 0 | 0 | 0 | 49 | 0 | 0 | 0 | 49 | 68 | 2744 | 0 | 0 | 2812 | 0 | 0 | 0 | 0 | 0 | 2861 |
| Apprch \% | 0 | 0 | 0 | 0 |  | 100 | 0 | 0 | 0 |  | 2.4 | 97.6 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| Total \% | 0 | 0 | 0 | 0 | 0 | 1.7 | 0 | 0 | 0 | 1.7 | 2.4 | 95.9 | 0 | 0 | 98.3 | 0 | 0 | 0 | 0 | 0 |  |

## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name: Powers Blvd - Waynoka Rd Mid Site Code : S224370
Start Date: 6/9/2022
Page No : 2

|  | Driveway Accesses Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Powers Blvd Northbound |  |  |  |  | Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 2:30:00 PM to 3:15:00 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 2:30:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 2:30:00 PM | 0 | 0 | 0 | 0 | 0 | 13 | 0 | 0 | 0 | 13 | 13 | 621 | 0 | 0 | 634 | 0 | 0 | 0 | 0 | 0 | 647 |
| 2:45:00 PM | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 8 | 16 | 672 | 0 | 0 | 688 | 0 | 0 | 0 | 0 | 0 | 696 |
| 3:00:00 PM | 0 | 0 | 0 | 0 | 0 | 21 | 0 | 0 | 0 | 21 | 17 | 722 | 0 | 0 | 739 | 0 | 0 | 0 | 0 | 0 | 760 |
| 3:15:00 PM | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 7 | 22 | 729 | 0 | 0 | 751 | 0 | 0 | 0 | 0 | 0 | 758 |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 49 | 0 | 0 | 0 | 49 | 68 | 2744 | 0 | 0 | 2812 | 0 | 0 | 0 | 0 | 0 | 2861 |
| \% App. Total | 0 | 0 | 0 | 0 |  | 100 | 0 | 0 | 0 |  | 2.4 | 97.6 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 583 | . 000 | . 000 | . 000 | . 583 | . 773 | . 941 | . 000 | . 000 | . 936 | . 000 | . 000 | . 000 | . 000 | . 000 | . 941 |



## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name: Powers Blvd - Waynoka Rd Mid Site Code : S224370
Start Date: 6/9/2022
Page No : 3

|  | Driveway Accesses Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Powers Blvd Northbound |  |  |  |  | Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total |  |
| Peak Hour Analysis From 2:30:00 PM to 3:15:00 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | 2:30:00 PM |  |  |  |  | 2:30:00 PM |  |  |  |  | 2:30:00 PM |  |  |  |  | 2:30:00 PM |  |  |  |  |  |
| +0 mins. | 0 | 0 | 0 | 0 | 0 | 13 | 0 | 0 | 0 | 13 | 13 | 621 | 0 | 0 | 634 | 0 | 0 | 0 | 0 | 0 |  |
| +5 mins. | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 8 | 16 | 672 | 0 | 0 | 688 | 0 | 0 | 0 | 0 | 0 |  |
| +10 mins. | 0 | 0 | 0 | 0 | 0 | 21 | 0 | 0 | 0 | 21 | 17 | 722 | 0 | 0 | 739 | 0 | 0 | 0 | 0 | 0 |  |
| +15 mins. | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 7 | 22 | 729 | 0 | 0 | 751 | 0 | 0 | 0 | 0 | 0 |  |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 49 | 0 | 0 | 0 | 49 | 68 | 2744 | 0 | 0 | 2812 | 0 | 0 | 0 | 0 | 0 |  |
| \% App. Total | 0 | 0 | 0 | 0 |  | 100 | 0 | 0 | 0 |  | 2.4 | 97.6 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 583 | . 000 | . 000 | . 000 | . 583 | . 773 | . 941 | . 000 | . 000 | . 936 | . 000 | . 000 | . 000 | . 000 | . 000 |  |



# LSC Transportation Consultants, Inc. <br> 2504 E. Pikes Peak Ave, Suite 304 <br> Colorado Springs, CO 80909 <br> 719-633-2868 

File Name : Powers Blvd - Waynoka Rd PM
Site Code : S224370
Start Date: 6/9/2022
Page No : 1

Groups Printed- Unshifted

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Powers Blvd Northbound |  |  |  |  | Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal | int. Total |
| 16:00 | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 7 | 34 | 746 | 0 | 0 | 780 | 0 | 0 | 0 | 0 | 0 | 787 |
| 16:15 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 6 | 26 | 718 | 0 | 0 | 744 | 0 | 0 | 0 | 0 | 0 | 750 |
| 16:30 | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 5 | 25 | 734 | 0 | 0 | 759 | 0 | 0 | 0 | 0 | 0 | 764 |
| 16:45 | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 4 | 44 | 689 | 0 | 0 | 733 | 0 | 0 | 0 | 0 | 0 | 737 |
| Total | 0 | 0 | 0 | 0 | 0 | 22 | 0 | 0 | 0 | 22 | 129 | 2887 | 0 | 0 | 3016 | 0 | 0 | 0 | 0 | 0 | 3038 |
| 17:00 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 3 | 48 | 650 | 0 | 0 | 698 | 0 | 0 | 0 | 0 | 0 | 701 |
| 17:15 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 6 | 24 | 649 | 0 | 0 | 673 | 0 | 0 | 0 | 0 | 0 | 679 |
| 17:30 | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 6 | 16 | 627 | 0 | 0 | 643 | 0 | 0 | 0 | 0 | 0 | 649 |
| 17:45 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 3 | 13 | 714 | 0 | 0 | 727 | 0 | 0 | 0 | 0 | 0 | 730 |
| Total | 0 | 0 | 0 | 0 | 0 | 18 | 0 | 0 | 0 | 18 | 101 | 2640 | 0 | 0 | 2741 | 0 | 0 | 0 | 0 | 0 | 2759 |
| Grand Total | 0 | 0 | 0 | 0 | 0 | 40 | 0 | 0 | 0 | 40 | 230 | 5527 | 0 | 0 | 5757 | 0 | 0 | 0 | 0 | 0 | 5797 |
| Apprch \% | 0 | 0 | 0 | 0 |  | 100 | 0 | 0 | 0 |  | 4 | 96 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| Total \% | 0 | 0 | 0 | 0 | 0 | 0.7 | 0 | 0 | 0 | 0.7 | 4 | 95.3 | 0 | 0 | 99.3 | 0 | 0 | 0 | 0 | 0 |  |

## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Powers Blvd - Waynoka Rd PM
Site Code : S224370
Start Date : 6/9/2022
Page No : 2

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Powers Blvd Northbound |  |  |  |  | Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total | Right | Thru | Left | Peds | App. Total |  |
| Peak Hour Analysis From 4:00:00 PM to 5:45:00 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 4:00:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:00:00 PM | 0 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 0 | 7 | 34 | 746 | 0 | 0 | 780 | 0 | 0 | 0 | 0 | 0 | 787 |
| 4:15:00 PM | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 6 | 26 | 718 | 0 | 0 | 744 | 0 | 0 | 0 | 0 | 0 | 750 |
| 4:30:00 PM | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 5 | 25 | 734 | 0 | 0 | 759 | 0 | 0 | 0 | 0 | 0 | 764 |
| 4:45:00 PM | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 4 | 44 | 689 | 0 | 0 | 733 | 0 | 0 | 0 | 0 | 0 | 737 |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 22 | 0 | 0 | 0 | 22 | 129 | 2887 | 0 | 0 | 3016 | 0 | 0 | 0 | 0 | 0 | 3038 |
| \% App. Total | 0 | 0 | 0 | 0 |  | 100 | 0 | 0 | 0 |  | 4.3 | 95.7 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 786 | . 000 | . 000 | . 000 | . 786 | . 733 | . 967 | . 000 | . 000 | . 967 | . 000 | . 000 | . 000 | . 000 | . 000 | . 965 |

## LSC Transportation Consultants, Inc.

2504 E. Pikes Peak Ave, Suite 304
Colorado Springs, CO 80909
719-633-2868
File Name : Powers Blvd - Waynoka Rd PM
Site Code : S224370
Start Date : 6/9/2022
Page No : 3

|  | Southbound |  |  |  |  | Waynoka Rd Westbound |  |  |  |  | Powers Blvd Northbound |  |  |  |  | Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toala | Right | Thru | Left | Peds | App. Toal | Right | Thru | Left | Peds | App. Toal |  |
| Peak Hour Analysis From 4:00:00 PM to 5:45:00 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| +0 mins. | 4:00.00 P11 | 0 | 0 | 0 | 0 | 4.0000 PM | 0 | 0 | 0 | 7 | 4000.00 PM | 746 | 0 | 0 | 780 | 4.00:00 PM <br> 0 | 0 | 0 | 0 | 0 |  |
| +5 mins. | 0 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 0 | 6 | 26 | 718 | 0 | 0 | 744 | 0 | 0 | 0 | 0 | 0 |  |
| +10 mins. | 0 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 5 | 25 | 734 | 0 | 0 | 759 | 0 | 0 | 0 | 0 | 0 |  |
| +15 mins. | 0 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 0 | 4 | 44 | 689 | 0 | 0 | 733 | 0 | 0 | 0 | 0 | 0 |  |
| Total Volume | 0 | 0 | 0 | 0 | 0 | 22 | 0 | 0 | 0 | 22 | 129 | 2887 | 0 | 0 | 3016 | 0 | 0 | 0 | 0 | 0 |  |
| \% App. Total | 0 | 0 | 0 | 0 |  | 100 | 0 | 0 | 0 |  | 4.3 | 95.7 | 0 | 0 |  | 0 | 0 | 0 | 0 |  |  |
| PHF | . 000 | . 000 | . 000 | . 000 | . 000 | . 786 | . 000 | . 000 | . 000 | . 786 | . 733 | . 967 | . 000 | . 000 | . 967 | . 000 | . 000 | . 000 | . 000 | . 000 |  |



|  | 4 | $\rightarrow$ | 7 | 7 | － | 4 | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 恌信 |  | \％ | 个种 | F＇ | \％ | $\hat{F}$ |  | \％ | $\uparrow$ | F |
| Traffic Volume（vph） | 55 | 362 | 70 | 67 | 823 | 104 | 26 | 22 | 18 | 74 | 45 | 115 |
| Future Volume（vph） | 55 | 362 | 70 | 67 | 823 | 104 | 26 | 22 | 18 | 74 | 45 | 115 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length（ft） | 140 |  | 0 | 185 |  | 185 | 50 |  | 0 | 165 |  | 150 |
| Storage Lanes | 1 |  | 0 | 1 |  | 1 | 1 |  | 0 | 1 |  | 1 |
| Taper Length（ft） | 90 |  |  | 130 |  |  | 25 |  |  | 110 |  |  |
| Lane Util．Factor | 1.00 | 0.91 | 0.91 | 1.00 | 0.91 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.963 |  |  |  | 0.850 |  | 0.937 |  |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（prot） | 1770 | 4897 | 0 | 1770 | 5085 | 1583 | 1770 | 1745 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.288 |  |  | 0.432 |  |  | 0.719 |  |  | 0.649 |  |  |
| Satd．Flow（perm） | 536 | 4897 | 0 | 805 | 5085 | 1583 | 1339 | 1745 | 0 | 1209 | 1863 | 1583 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  | 73 |  |  |  | 109 |  | 20 |  |  |  | 125 |
| Link Speed（mph） |  | 40 |  |  | 40 |  |  | 30 |  |  | 30 |  |
| Link Distance（ft） |  | 497 |  |  | 574 |  |  | 515 |  |  | 591 |  |
| Travel Time（s） |  | 8.5 |  |  | 9.8 |  |  | 11.7 |  |  | 13.4 |  |
| Peak Hour Factor | 0.95 | 0.95 | 0.56 | 0.77 | 0.95 | 0.95 | 0.53 | 0.63 | 0.73 | 0.92 | 0.78 | 0.92 |
| Adj．Flow（vph） | 58 | 381 | 125 | 87 | 866 | 109 | 49 | 35 | 25 | 80 | 58 | 125 |


| Shared Lane Traffic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group Flow（vph） | 58 | 506 | 0 | 87 | 866 | 109 | 49 | 60 | 0 | 80 | 58 | 125 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width（ft） |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset（ft） |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width（ft） |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  | Yes |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed（mph） | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors | 1 | 2 |  | 1 | 2 | 1 | 1 | 2 |  | 1 | 2 | 1 |
| Detector Template | Left | Thru |  | Left | Thru | Right | Left | Thru |  | Left | Thru | Right |
| Leading Detector（ft） | 20 | 100 |  | 20 | 100 | 20 | 20 | 100 |  | 20 | 100 | 20 |
| Trailing Detector（tt） | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Position（ft） | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Size（ft） | 20 | 6 |  | 20 | 6 | 20 | 20 | 6 |  | 20 | 6 | 20 |
| Detector 1 Type | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl＋Ex | Cl＋Ex | Cl＋Ex |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 2 Position（ft） |  | 94 |  |  | 94 |  |  | 94 |  |  | 94 |  |
| Detector 2 Size（ft） |  | 6 |  |  | 6 |  |  | 6 |  |  | 6 |  |
| Detector 2 Type |  | Cl＋Ex |  |  | Cl＋Ex |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl＋Ex |  |


| Detector 2 Channel |  | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Detector 2 Extend（s） | $\mathrm{pm}+\mathrm{pt}$ | NA | $\mathrm{pm+pt}$ | NA | Perm | pm＋pt | NA | pm＋pt | NA | Perm |
| Turn Type | 5 | 2 | 1 | 6 |  | 7 | 4 | 3 | 8 | 8 |
| Protected Phases | 2 |  | 6 |  | 6 | 4 |  | 8 |  | 8 |
| Permitted Phases | 2 |  |  |  |  |  |  |  |  |  |


|  | $\rangle$ |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Detector Phase | 5 | 2 |  | 1 | 6 | 6 | 7 | 4 |  | 3 | 8 | 8 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 20.0 |  | 4.0 | 20.0 | 20.0 | 4.0 | 4.0 |  | 4.0 | 4.0 | 4.0 |
| Minimum Split (s) | 10.5 | 26.0 |  | 10.5 | 26.0 | 26.0 | 10.5 | 24.5 |  | 10.5 | 10.5 | 10.5 |
| Total Split (s) | 25.0 | 71.0 |  | 25.0 | 71.0 | 71.0 | 25.0 | 25.0 |  | 25.0 | 25.0 | 25.0 |
| Total Split (\%) | 17.1\% | 48.6\% |  | 17.1\% | 48.6\% | 48.6\% | 17.1\% | 17.1\% |  | 17.1\% | 17.1\% | 17.1\% |
| Maximum Green (s) | 20.0 | 65.0 |  | 20.0 | 65.0 | 65.0 | 20.0 | 18.5 |  | 20.0 | 18.5 | 18.5 |
| Yellow Time (s) | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 | 3.0 | 4.5 |  | 3.0 | 4.5 | 4.5 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 6.0 |  | 5.0 | 6.0 | 6.0 | 5.0 | 6.5 |  | 5.0 | 6.5 | 6.5 |
| Lead/Lag | Lead | Lag |  | Lead | Lag | Lag | Lead | Lag |  | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes |  | Yes | Yes | Yes | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 |
| Recall Mode | None | C-Max |  | None | C-Max | C-Max | None | Max |  | None | Max | Max |
| Walk Time (s) |  | 7.0 |  |  | 7.0 | 7.0 |  | 7.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 11.0 |  |  | 11.0 | 11.0 |  | 11.0 |  |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 |  |  | 0 | 0 |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 85.0 | 76.7 |  | 87.9 | 79.8 | 79.8 | 38.1 | 28.2 |  | 42.7 | 32.3 | 32.3 |
| Actuated g/C Ratio | 0.58 | 0.53 |  | 0.60 | 0.55 | 0.55 | 0.26 | 0.19 |  | 0.29 | 0.22 | 0.22 |
| v/c Ratio | 0.16 | 0.19 |  | 0.16 | 0.31 | 0.12 | 0.13 | 0.17 |  | 0.20 | 0.14 | 0.28 |
| Control Delay | 11.9 | 15.9 |  | 11.7 | 19.0 | 3.2 | 37.2 | 36.5 |  | 38.2 | 49.3 | 9.5 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 11.9 | 15.9 |  | 11.7 | 19.0 | 3.2 | 37.2 | 36.5 |  | 38.2 | 49.3 | 9.5 |
| LOS | B | B |  | B | B | A | D | D |  | D | D | A |
| Approach Delay |  | 15.5 |  |  | 16.8 |  |  | 36.8 |  |  | 27.0 |  |
| Approach LOS |  | B |  |  | B |  |  | D |  |  | C |  |
| Queue Length 50th (ft) | 20 | 76 |  | 31 | 165 | 0 | 33 | 32 |  | 55 | 46 | 0 |
| Queue Length 95th (tt) | 39 | 102 |  | 46 | 201 | 31 | 38 | 47 |  | 99 | 78 | 56 |
| Internal Link Dist (ft) |  | 417 |  |  | 494 |  |  | 435 |  |  | 511 |  |
| Turn Bay Length (ft) | 140 |  |  | 185 |  | 185 | 50 |  |  | 165 |  | 150 |
| Base Capacity (vph) | 503 | 2606 |  | 633 | 2779 | 914 | 471 | 353 |  | 449 | 412 | 447 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Spill back Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.12 | 0.19 |  | 0.14 | 0.31 | 0.12 | 0.10 | 0.17 |  | 0.18 | 0.14 | 0.28 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

Area Type: Other
Cycle Length: 146
Actuated Cycle Length: 146
Offset: $65(45 \%)$, Referenced to phase 2:EBTL and 6:WBTL, Start of Yellow
Natural Cycle: 75
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.31
Intersection Signal Delay: 18.8 Intersection LOS: B
Intersection Capacity Utilization 45.7\% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 1: Waynoka P//Tutt Blvd \& Constitution Ave



| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 88 | 0 | - | 0 | 147 | 59 |
| Stage 1 | - | - | - | - | 59 | - |
| Stage 2 | - | - | - | - | 88 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1508 | - | - | - | 845 | 1007 |
| Stage 1 | - | - | - | - | 964 | - |
| Stage 2 | - | - | - | - | 935 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1508 | - | - | - | 831 | 1007 |
| Mov Cap-2 Maneuver | - | - | - | - | 831 | - |
| Stage 1 | - | - | - |  | 949 | - |
| Stage 2 | - | - | - |  | 935 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 2.8 |  | 0 |  | 10 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1508 | - | - | - | 856 |
| HCM Lane V/C Ratio |  | 0.016 | - | - | - | 0.162 |
| HCM Control Delay (s) |  | 7.4 | 0 | - | - | 10 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 0.6 |



| Major/Minor | Major1 |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Major2 |  | Minor2 |  |  |  |  |
| Conflicting Flow All | 637 | 0 | - | 0 | 1065 | 319 |
| $\quad$ Stage 1 | - | - | - | - | 621 | - |
| $\quad$ Stage 2 | - | - | - | - | 444 | - |
| Critical Hdwy | 4.14 | - | - | - | 6.84 | 6.94 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.84 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.84 | - |
| Follow-up Hdwy | 2.22 | - | - | - | 3.52 | 3.32 |
| Pot Cap-1 Maneuver | 943 | - | - | - | 218 | 677 |
| $\quad$ Stage 1 | - | - | - | - | 498 | - |
| Stage 2 | - | - | - | - | 614 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 943 | - | - | - | 191 | 677 |
| Mov Cap-2 Maneuver | - | - | - | - | 316 | - |
| Stage 1 | - | - | - | - | 436 | - |
| Stage 2 | - | - | - | - | 614 | - |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 2.1 | 0 | 13.7 |
| HCM LOS |  | B |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 943 | - | - | - | 316 | 677 |
| HCM Lane V/C Ratio | 0.125 | - | - | -0.133 | 0.097 |  |
| HCM Control Delay (s) | 9.4 | - | - | - | 18.1 | 10.9 |
| HCM Lane LOS | A | - | - | - | C | B |
| HCM 95th \%tile Q(veh) | 0.4 | - | - | - | 0.5 | 0.3 |


|  | 7 | 4 | $\dagger$ | $>$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | 「 | 惺家 |  |  |  |
| Trafic Volume（vph） | 0 | 26 | 1999 | 56 | 0 | 0 |
| Future Volume（vph） | 0 | 26 | 1999 | 56 | 0 | 0 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util．Factor | 1.00 | 1.00 | 0.91 | 0.91 | 1.00 | 1.00 |
| Frt |  | 0.865 | 0.996 |  |  |  |
| FIt Protected |  |  |  |  |  |  |
| Satd．Flow（prot） | 0 | 1611 | 5065 | 0 | 0 | 0 |
| Flt Permitted |  |  |  |  |  |  |
| Satd．Flow（perm） | 0 | 1611 | 5065 | 0 | 0 | 0 |
| Link Speed（mph） | 35 |  | 55 |  |  | 55 |
| Link Distance（tt） | 712 |  | 355 |  |  | 265 |
| Travel Time（s） | 13.9 |  | 4.4 |  |  | 3.3 |
| Peak Hour Factor | 0.76 | 0.76 | 0.95 | 0.94 | 0.95 | 0.95 |
| Adj．Flow（vph） | 0 | 34 | 2104 | 60 | 0 | 0 |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |
| Lane Group Flow（vph） | 0 | 34 | 2164 | 0 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width（ft） | 0 |  | 0 |  |  | 0 |
| Link Offset（ft） | 0 |  | 0 |  |  | 0 |
| Crosswalk Width（ft） | 16 |  | 16 |  |  | 16 |
| Two way Left Turn Lane |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed（mph） | 15 | 0 |  | 9 | 15 |  |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type：Other |  |  |  |  |  |  |
| Control Type：Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 49．9\％ICU Level of Service A |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |



| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 165 | 0 | - | 0 | 233 | 104 |
| Stage 1 | - | - | - | - | 104 | - |
| Stage 2 | - | - | - | - | 129 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - |  | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1413 | - | - | - | 755 | 951 |
| Stage 1 | - | - | - | - | 920 | - |
| Stage 2 | - | - | - |  | 897 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1413 | - | - | - | 726 | 951 |
| Mov Cap-2 Maneuver | - | - | - | - | - 726 | - |
| Stage 1 | - | - | - | - | - 885 | - |
| Stage 2 | - | - | - |  | 897 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 5.3 |  | 0 |  | 11 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 |  |  |
| Capacity (veh/h) |  | 1413 | - | - | - | 753 |
| HCM Lane V/C Ratio |  | 0.038 | - | - | - | 0.202 |
| HCM Control Delay (s) |  | 7.6 | 0 | - | - - | 11 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0.1 | - | - | - | 0.8 |

6: Palmer Park Blvd \& Waynoka Rd

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 448 | 0 | - | 0 | 938 | 224 |  |
| Stage 1 | - | - | - | - | 422 | - |  |
| Stage 2 | - | - | - | - | 516 | - |  |
| Critical Hdwy | 4.14 | - | - | - | 6.84 | 6.94 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.84 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.84 | - |  |
| Follow-up Hdwy | 2.22 | - | - | - | 3.52 | 3.32 |  |
| Pot Cap-1 Maneuver | 1109 | - | - | - | 263 | 779 |  |
| Stage 1 | - | - | - | - | 629 | - |  |
| Stage 2 | - | - | - | - | 564 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1109 | - | - | - | 236 | 779 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 365 | - |  |
| Stage 1 | - | - | - | - | 565 | - |  |
| Stage 2 | - | - | - | - | 564 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 1.4 |  | 0 |  | 12.2 |  |  |
| HCM LOS |  |  |  |  | B |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT WBT |  | WBR SBLn1 SBLn2 |  |  |
| Capacity (veh/h) |  | 1109 | - | - | - | 365 | 779 |
| HCM Lane V/C Ratio |  | 0.101 | - | - | - | 0.146 | 0.18 |
| HCM Control Delay (s) |  | 8.6 | - | - | - | 16.5 | 10.6 |
| HCM Lane LOS |  | A | - | - | - | C | B |
| HCM 95th \%tile Q(veh) |  | 0.3 | - | - | - | 0.5 | 0.7 |


|  | 4 |  |  |  |  |  | 4 | 4 |  |  |  | 4 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{*}$ | 惺 |  | ${ }^{*}$ | 444 | 「 | ${ }^{*}$ | $\uparrow$ |  | ${ }^{7}$ | 4 | 7 |
| Traffic Volume (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Future Volume (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 140 |  | 0 | 185 |  | 185 | 50 |  | 0 | 165 |  | 150 |
| Storage Lanes | 1 |  | 0 | 1 |  | 1 | 1 |  | 0 | 1 |  | 1 |
| Taper Length (ft) | 90 |  |  | 130 |  |  | 25 |  |  | 110 |  |  |
| Lane Util. Factor | 1.00 | 0.91 | 0.91 | 1.00 | 0.91 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  |  |  |  |  |  |  |  |  |  |  |  |
| Flt Protected |  |  |  |  |  |  |  |  |  |  |  |  |
| Satd. Flow (prot) | 1863 | 5085 | 0 | 1863 | 5085 | 1863 | 1863 | 1863 | 0 | 1863 | 1863 | 1863 |
| Flt Permitted |  |  |  |  |  |  |  |  |  |  |  |  |
| Satd. Flow (perm) | 1863 | 5085 | 0 | 1863 | 5085 | 1863 | 1863 | 1863 | 0 | 1863 | 1863 | 1863 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  |  |  |  |  |  |  |  |  |  |  |  |
| Link Speed (mph) |  | 40 |  |  | 40 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 497 |  |  | 574 |  |  | 515 |  |  | 591 |  |
| Travel Time (s) |  | 8.5 |  |  | 9.8 |  |  | 11.7 |  |  | 13.4 |  |
| Peak Hour Factor | 0.95 | 0.95 | 0.56 | 0.77 | 0.95 | 0.95 | 0.53 | 0.63 | 0.73 | 0.92 | 0.78 | 0.92 |
| Adj. Flow (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Shared Lane Traffic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  | Yes |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors | 1 | 2 |  | 1 | 2 | 1 | 1 | 2 |  | 1 | 2 | 1 |
| Detector Template | Left | Thru |  | Left | Thru | Right | Left | Thru |  | Left | Thru | Right |
| Leading Detector (ft) | 20 | 100 |  | 20 | 100 | 20 | 20 | 100 |  | 20 | 100 | 20 |
| Trailing Detector (ft) | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Position(ft) | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Size(ft) | 20 | 6 |  | 20 | 6 | 20 | 20 | 6 |  | 20 | 6 | 20 |
| Detector 1 Type | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 2 Position(ft) |  | 94 |  |  | 94 |  |  | 94 |  |  | 94 |  |
| Detector 2 Size(ft) |  | 6 |  |  | 6 |  |  | 6 |  |  | 6 |  |
| Detector 2 Type |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | Cl+Ex |  |  | Cl+Ex |  |  | Cl+Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Turn Type | pm+pt |  |  | pm+pt |  | Perm | pm+pt |  |  | pm+pt |  | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 7 | 4 |  | 3 | 8 |  |
| Permitted Phases | 2 |  |  | 6 |  | 6 | 4 |  |  | 8 |  | 8 |


|  | $4$ | $\rightarrow$ |  | 7 |  |  | 4 | 4 |  |  | 1 | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Detector Phase | 5 | 2 |  | 1 | 6 | 6 | 7 | 4 |  | 3 | 8 | 8 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 20.0 |  | 4.0 | 20.0 | 20.0 | 4.0 | 4.0 |  | 4.0 | 4.0 | 4.0 |
| Minimum Split (s) | 10.5 | 26.0 |  | 10.5 | 26.0 | 26.0 | 10.5 | 24.5 |  | 10.5 | 10.5 | 10.5 |
| Total Split (s) | 25.0 | 71.0 |  | 25.0 | 71.0 | 71.0 | 25.0 | 25.0 |  | 25.0 | 25.0 | 25.0 |
| Total Split (\%) | 17.1\% | 48.6\% |  | 17.1\% | 48.6\% | 48.6\% | 17.1\% | 17.1\% |  | 17.1\% | 17.1\% | 17.1\% |
| Maximum Green (s) | 20.0 | 65.0 |  | 20.0 | 65.0 | 65.0 | 20.0 | 18.5 |  | 20.0 | 18.5 | 18.5 |
| Yellow Time (s) | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 | 3.0 | 4.5 |  | 3.0 | 4.5 | 4.5 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 6.0 |  | 5.0 | 6.0 | 6.0 | 5.0 | 6.5 |  | 5.0 | 6.5 | 6.5 |
| Lead/Lag | Lead | Lag |  | Lead | Lag | Lag | Lead | Lag |  | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes |  | Yes | Yes | Yes | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 |
| Recall Mode | None | C-Max |  | None | C-Max | C-Max | None | Max |  | None | Max | Max |
| Walk Time (s) |  | 7.0 |  |  | 7.0 | 7.0 |  | 7.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 11.0 |  |  | 11.0 | 11.0 |  | 11.0 |  |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 |  |  | 0 | 0 |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated g/C Ratio |  |  |  |  |  |  |  |  |  |  |  |  |
| v/c Ratio |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Delay |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue Delay |  |  |  |  |  |  |  |  |  |  |  |  |
| Total Delay |  |  |  |  |  |  |  |  |  |  |  |  |
| LOS |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach Delay |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach LOS |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue Length 50th (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Queue Length 95th (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Internal Link Dist (ft) |  | 417 |  |  | 494 |  |  | 435 |  |  | 511 |  |
| Turn Bay Length (ft) |  |  |  |  |  |  |  |  |  |  |  |  |
| Base Capacity (vph) |  |  |  |  |  |  |  |  |  |  |  |  |
| Starvation Cap Reductn |  |  |  |  |  |  |  |  |  |  |  |  |
| Spillback Cap Reductn |  |  |  |  |  |  |  |  |  |  |  |  |
| Storage Cap Reductn |  |  |  |  |  |  |  |  |  |  |  |  |
| Reduced v/c Ratio |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

## Area Type: Other

Cycle Length: 146
Actuated Cycle Length: 146
Offset: 65 (45\%), Referenced to phase 2:EBTL and 6:WBTL, Start of Yellow
Natural Cycle: 75
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.00
Intersection Signal Delay: $0.0 \quad$ Intersection LOS: A
Intersection Capacity Utilization 0.0\% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 1: Constitution Ave


|  | $\checkmark$ |  | 4 | $p$ |  | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | 「 | 个个官 |  |  |  |
| Traffic Volume（vph） | 0 | 49 | 2744 | 68 | 0 | 0 |
| Future Volume（vph） | 0 | 49 | 2744 | 68 | 0 | 0 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util．Factor | 1.00 | 1.00 | 0.91 | 0.91 | 1.00 | 1.00 |
| Frt |  | 0.865 | 0.996 |  |  |  |
| Flt Protected |  |  |  |  |  |  |
| Satd．Flow（prot） | 0 | 1611 | 5065 | 0 | 0 | 0 |
| Flt Permitted |  |  |  |  |  |  |
| Satd．Flow（perm） | 0 | 1611 | 5065 | 0 | 0 | 0 |
| Link Speed（mph） | 35 |  | 55 |  |  | 55 |
| Link Distance（ft） | 712 |  | 355 |  |  | 265 |
| Travel Time（s） | 13.9 |  | 4.4 |  |  | 3.3 |
| Peak Hour Factor | 0.76 | 0.76 | 0.95 | 0.94 | 0.95 | 0.95 |
| Adj．Flow（vph） | 0 | 64 | 2888 | 72 | 0 | 0 |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |
| Lane Group Flow（vph） | 0 | 64 | 2960 | 0 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width（ft） | 0 |  | 0 |  |  | 0 |
| Link Offset（ft） | 0 |  | 0 |  |  | 0 |
| Crosswalk Width（ft） | 16 |  | 16 |  |  | 16 |
| Two way Left Turn Lane |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed（mph） | 15 | 9 |  | 9 | 15 |  |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type：Other |  |  |  |  |  |  |
| Control Type：Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 64．5\％ |  |  |  | ICU Level of Service C |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |



| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | :---: |
| Conflicting Flow All | 214 | 0 | - | 0 | 393 |  |
| $\quad$ Stage 1 | - | - | - | - | 119 |  |
| $\quad$ Stage 2 | - | - | - | - | 274 |  |


| Stage 2 | - | - | - | - | 772 | - |
| :---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1356 | - | - | - | 555 | 933 |
| Mov Cap-2 Maneuver | - | - | - | - | 555 | - |
| Stage 1 | - | - | - | - | 823 | - |
| Stage 2 | - | - | - | - | 772 | - |


|  | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Approach |  |  |  |
| HCM Control Delay, s | 6.3 | 0 | 12.8 |
| HCOS |  |  | B |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1356 | - | - | -585 |
| HCM Lane V/C Ratio | 0.09 | - | - | -0.212 |
| HCM Control Delay (s) | 7.9 | 0 | - | -12.8 |
| HCM Lane LOS | A | A | - | - |
| HCM 95th \%tile Q(veh) | 0.3 | - | - | - |
| H | 0.8 |  |  |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 559 | 0 | - | 0 | 1169 | 280 |  |
| Stage 1 | - | - | - | - | 529 | - |  |
| Stage 2 | - | - | - | - | 640 | - |  |
| Critical Hdwy | 4.14 | - | - | - | 6.84 | 6.94 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.84 | - |  |
| Critical Hdwy Stg 2 | - | - | - |  | 5.84 | - |  |
| Follow-up Hdwy | 2.22 | - | - | - | 3.52 | 3.32 |  |
| Pot Cap-1 Maneuver | 1008 | - | - | - | 186 | 717 |  |
| Stage 1 | - | - | - |  | 555 | - |  |
| Stage 2 | - | - | - |  | 487 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1008 | - | - | - | 163 | 717 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 296 | - |  |
| Stage 1 | - | - | - | - | 486 | - |  |
| Stage 2 | - | - | - |  | 487 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 1.3 |  | 0 |  | 13.9 |  |  |
| HCM LOS |  |  |  |  | B |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| Capacity (veh/h) |  | 1008 | - | - | - | 296 | 717 |
| HCM Lane V/C Ratio |  | 0.124 | - | - | - | 0.171 | 0.133 |
| HCM Control Delay (s) |  | 9.1 | - | - | - | 19.7 | 10.8 |
| HCM Lane LOS |  | A | - | - | - | C | B |
| HCM 95th \%tile Q(veh) |  | 0.4 | - | - | - | 0.6 | 0.5 |


|  | 7 | 4 | $\dagger$ | $>$ |  | $\dagger$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations |  | 「 | 惺家 |  |  |  |
| Trafic Volume（vph） | 0 | 22 | 2887 | 129 | 0 | 0 |
| Future Volume（vph） | 0 | 22 | 2887 | 129 | 0 | 0 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lane Util．Factor | 1.00 | 1.00 | 0.91 | 0.91 | 1.00 | 1.00 |
| Frt |  | 0.865 | 0.994 |  |  |  |
| FIt Protected |  |  |  |  |  |  |
| Satd．Flow（prot） | 0 | 1611 | 5055 | 0 | 0 | 0 |
| Flt Permitted |  |  |  |  |  |  |
| Satd．Flow（perm） | 0 | 1611 | 5055 | 0 | 0 | 0 |
| Link Speed（mph） | 35 |  | 55 |  |  | 55 |
| Link Distance（tt） | 712 |  | 355 |  |  | 265 |
| Travel Time（s） | 13.9 |  | 4.4 |  |  | 3.3 |
| Peak Hour Factor | 0.76 | 0.76 | 0.95 | 0.94 | 0.95 | 0.95 |
| Adj．Flow（vph） | 0 | 29 | 3039 | 137 | 0 | 0 |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |
| Lane Group Flow（vph） | 0 | 29 | 3176 | 0 | 0 | 0 |
| Enter Blocked Intersection | No | No | No | No | No | No |
| Lane Alignment | Left | Right | Left | Right | Left | Left |
| Median Width（ft） | 0 |  | 0 |  |  | 0 |
| Link Offset（ft） | 0 |  | 0 |  |  | 0 |
| Crosswalk Width（ft） | 16 |  | 16 |  |  | 16 |
| Two way Left Turn Lane |  |  |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed（mph） | 15 | 0 |  | 9 | 15 |  |
| Sign Control | Stop |  | Free |  |  | Free |
| Intersection Summary |  |  |  |  |  |  |
| Area Type：Other |  |  |  |  |  |  |
| Control Type：Unsignalized |  |  |  |  |  |  |
| Intersection Capacity Utilization 68．6\％ICU Level of Service C |  |  |  |  |  |  |
| Analysis Period（min） 15 |  |  |  |  |  |  |


|  | 4 | $\rightarrow$ | 7 | $\downarrow$ | － | 4 | 4 | $\dagger$ | $p$ | ＊ | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 中性 |  | ${ }^{7}$ | 个个4 | F | ${ }^{7}$ | $\hat{i}$ |  | ${ }^{7}$ | $\uparrow$ | F |
| Traffic Volume（vph） | 55 | 362 | 110 | 67 | 823 | 104 | 40 | 30 | 18 | 74 | 45 | 114 |
| Future Volume（vph） | 55 | 362 | 110 | 67 | 823 | 104 | 40 | 30 | 18 | 74 | 45 | 114 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length（ft） | 140 |  | 0 | 185 |  | 185 | 50 |  | 0 | 165 |  | 150 |
| Storage Lanes | 1 |  | 0 | 1 |  | 1 | 1 |  | 0 | 1 |  | 1 |
| Taper Length（ft） | 90 |  |  | 130 |  |  | 25 |  |  | 110 |  |  |
| Lane Utill．Factor | 1.00 | 0.91 | 0.91 | 1.00 | 0.91 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.949 |  |  |  | 0.850 |  | 0.949 |  |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（prot） | 1770 | 4826 | 0 | 1770 | 5085 | 1583 | 1770 | 1768 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.289 |  |  | 0.396 |  |  | 0.719 |  |  | 0.702 |  |  |
| Satd．Flow（perm） | 538 | 4826 | 0 | 738 | 5085 | 1583 | 1339 | 1768 | 0 | 1308 | 1863 | 1583 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  | 115 |  |  |  | 109 |  | 15 |  |  |  | 124 |
| Link Speed（mph） |  | 40 |  |  | 40 |  |  | 30 |  |  | 30 |  |
| Link Distance（ft） |  | 497 |  |  | 574 |  |  | 515 |  |  | 591 |  |
| Travel Time（s） |  | 8.5 |  |  | 9.8 |  |  | 11.7 |  |  | 13.4 |  |
| Peak Hour Factor | 0.95 | 0.95 | 0.56 | 0.77 | 0.95 | 0.95 | 0.53 | 0.63 | 0.73 | 0.92 | 0.78 | 0.92 |
| Adj．Flow（vph） | 58 | 381 | 196 | 87 | 866 | 109 | 75 | 48 | 25 | 80 | 58 | 124 |


| Shared Lane Trafic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group Flow（vph） | 58 | 577 | 0 | 87 | 866 | 109 | 75 | 73 | 0 | 80 | 58 | 124 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width（ft） |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset（ft） |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width（ft） |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  | Yes |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed（mph） | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors | 1 | 2 |  | 1 | 2 | 1 | 1 | 2 |  | 1 | 2 | 1 |
| Detector Template | Left | Thru |  | Left | Thru | Right | Left | Thru |  | Left | Thru | Right |
| Leading Detector（ft） | 20 | 100 |  | 20 | 100 | 20 | 20 | 100 |  | 20 | 100 | 20 |
| Trailing Detector（ft） | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Position（ft） | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Size（ft） | 20 | 6 |  | 20 | 6 | 20 | 20 | 6 |  | 20 | 6 | 20 |
| Detector 1 Type | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | $\mathrm{Cl}+\mathrm{Ex}$ | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 2 Position（ft） |  | 94 |  |  | 94 |  |  | 94 |  |  | 94 |  |
| Detector 2 Size（ft） |  | 6 |  |  | 6 |  |  | 6 |  |  | 6 |  |
| Detector 2 Type |  | Cl＋Ex |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |


| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Detector 2 Extend（s） |  | 0.0 |  | 0.0 |  |  | 0.0 |  | 0.0 |  |
| Turn Type | pm＋pt | NA | pm＋pt | NA | Perm | pm＋pt | NA | pm＋pt | NA | Perm |
| Protected Phases | 5 | 2 | 1 | 6 |  | 7 | 4 | 3 | 8 |  |
| Permitted Phases | 2 |  | 6 |  | 6 | 4 |  | 8 |  | 8 |


|  | $\rangle$ |  |  | $\dagger$ |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Detector Phase | 5 | 2 |  | 1 | 6 | 6 | 7 | 4 |  | 3 | 8 | 8 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 20.0 |  | 4.0 | 20.0 | 20.0 | 4.0 | 4.0 |  | 4.0 | 4.0 | 4.0 |
| Minimum Split (s) | 10.5 | 26.0 |  | 10.5 | 26.0 | 26.0 | 10.5 | 24.5 |  | 10.5 | 10.5 | 10.5 |
| Total Split (s) | 25.0 | 71.0 |  | 25.0 | 71.0 | 71.0 | 25.0 | 25.0 |  | 25.0 | 25.0 | 25.0 |
| Total Split (\%) | 17.1\% | 48.6\% |  | 17.1\% | 48.6\% | 48.6\% | 17.1\% | 17.1\% |  | 17.1\% | 17.1\% | 17.1\% |
| Maximum Green (s) | 20.0 | 65.0 |  | 20.0 | 65.0 | 65.0 | 20.0 | 18.5 |  | 20.0 | 18.5 | 18.5 |
| Yellow Time (s) | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 | 3.0 | 4.5 |  | 3.0 | 4.5 | 4.5 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 6.0 |  | 5.0 | 6.0 | 6.0 | 5.0 | 6.5 |  | 5.0 | 6.5 | 6.5 |
| Lead/Lag | Lead | Lag |  | Lead | Lag | Lag | Lead | Lag |  | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes |  | Yes | Yes | Yes | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 |
| Recall Mode | None | C-Max |  | None | C-Max | C-Max | None | Max |  | None | Max | Max |
| Walk Time (s) |  | 7.0 |  |  | 7.0 | 7.0 |  | 7.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 11.0 |  |  | 11.0 | 11.0 |  | 11.0 |  |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 |  |  | 0 | 0 |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 85.0 | 76.7 |  | 87.9 | 79.8 | 79.8 | 39.7 | 28.2 |  | 40.3 | 28.5 | 28.5 |
| Actuated g/C Ratio | 0.58 | 0.53 |  | 0.60 | 0.55 | 0.55 | 0.27 | 0.19 |  | 0.28 | 0.20 | 0.20 |
| v/c Ratio | 0.15 | 0.22 |  | 0.17 | 0.31 | 0.12 | 0.19 | 0.21 |  | 0.20 | 0.16 | 0.30 |
| Control Delay | 11.9 | 15.0 |  | 11.8 | 19.0 | 3.2 | 38.1 | 42.0 |  | 38.2 | 51.4 | 10.3 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 11.9 | 15.0 |  | 11.8 | 19.0 | 3.2 | 38.1 | 42.0 |  | 38.2 | 51.4 | 10.3 |
| LOS | B | B |  | B | B | A | D | D |  | D | D | B |
| Approach Delay |  | 14.8 |  |  | 16.8 |  |  | 40.0 |  |  | 27.9 |  |
| Approach LOS |  | B |  |  | B |  |  | D |  |  | C |  |
| Queue Length 50th (ft) | 20 | 82 |  | 31 | 165 | 0 | 52 | 47 |  | 55 | 46 | 0 |
| Queue Length 95th (tt) | 39 | 110 |  | 46 | 201 | 31 | 53 | 63 |  | 99 | 79 | 58 |
| Internal Link Dist (ft) |  | 417 |  |  | 494 |  |  | 435 |  |  | 511 |  |
| Turn Bay Length (ft) | 140 |  |  | 185 |  | 185 | 50 |  |  | 165 |  | 150 |
| Base Capacity (vph) | 504 | 2589 |  | 601 | 2779 | 914 | 471 | 353 |  | 466 | 363 | 408 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Spill back Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.12 | 0.22 |  | 0.14 | 0.31 | 0.12 | 0.16 | 0.21 |  | 0.17 | 0.16 | 0.30 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

Area Type: Other
Cycle Length: 146
Actuated Cycle Length: 146
Offset: $65(45 \%)$, Referenced to phase 2:EBTL and 6:WBTL, Start of Yellow
Natural Cycle: 75
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.31
Intersection Signal Delay: 19.2 Intersection LOS: B
Intersection Capacity Utilization 45.7\% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 1: Waynoka P/ITutt Blvd \& Constitution Ave


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 6.1 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | A |  |  |  |
| Traffic Vol, veh/h | 5 | 5 | 5 | 75 | 125 | 15 |
| Future Vol, veh/h | 5 | 5 | 5 | 75 | 125 | 15 |
| Conflicting Peds, \#hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - None | - | None |  |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 83 | 83 | 83 | 83 | 83 | 83 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 6 | 6 | 6 | 90 | 151 | 18 |


| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |
| :--- | ---: | :--- | :--- | :--- | :--- | ---: |
| Conflicting Flow All | 96 | 0 | - | 0 | 69 | 51 |
| $\quad$ Stage 1 | - | - | - | - | 51 | - |
| Stage 2 | - | - | - | - | 18 | - |
| Critical Hdwy | 4.12 | - | - | -6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | -5.42 | - |  |
| Follow-up Hdwy | 2.218 | - | - | -3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | 1498 | - | - | -936 | 1017 |  |
| $\quad$ Stage 1 | - | - | - | - | 971 | - |
| $\quad$ Stage 2 | - | - | - | -1005 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1498 | - | - | - | 932 | 1017 |
| Mov Cap-2 Maneuver | - | - | - | - | 932 | - |
| Stage 1 | - | - | - | - | 967 | - |
| Stage 2 | - | - | - | -1005 | - |  |


| Approach | EB | WB | SB |
| :--- | :---: | ---: | :---: |
| HCM Control Delay, s | 3.7 | 0 | 9.7 |

HCMLOS A

| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1498 | - | - | -940 |
| HCM Lane V/C Ratio | 0.004 | - | - | -0.179 |
| HCM Control Delay (s) | 7.4 | 0 | - | -9.7 |
| HCM Lane LOS | A | A | - | - |
| HCM 95th \%tile Q(veh) | 0 | - | - | - |
| H | 0.7 |  |  |  |



| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Conflicting Flow All | 637 | 0 | - | 0 | 1163 | 319 |
| Stage 1 | - | - | - | - | 621 | - |
| Stage 2 | - | - | - | - | 542 | - |
| Critical Hdwy | 4.14 | - | - | - | 6.84 | 6.94 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.84 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.84 | - |
| Follow-up Hdwy | 2.22 | - | - | - | 3.52 | 3.32 |
| Pot Cap-1 Maneuver | 943 | - | - | - | 188 | 677 |
| $\quad$ Stage 1 | - | - | - | - | 498 | - |
| Stage 2 | - | - | - | - | 547 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 943 | - | - | - | 155 | 677 |
| Mov Cap-2 Maneuver | - | - | - | - | 283 | - |
| Stage 1 | - | - | - | - | 410 | - |
| Stage 2 | - | - | - | - | 547 | - |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 2.8 | 0 | 14.3 |
| HCM LOS |  | B |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 943 | - | - | - | 283 | 677 |
| HCM Lane V/C Ratio | 0.177 | - | - | -0.148 | 0.101 |  |
| HCM Control Delay (s) | 9.6 | - | - | - | 19.9 | 10.9 |
| HCM Lane LOS | A | - | - | - | C | B |
| HCM 95th \%tile Q(veh) | 0.6 | - | - | - | 0.5 | 0.3 |



| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 187 | 0 | - | 0 | 145 | 103 |
| Stage 1 | - | - | - |  | 103 | - |
| Stage 2 | - | - | - | - | 42 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1387 | - | - | - | 847 | 952 |
| Stage 1 | - | - | - |  | 921 | - |
| Stage 2 | - | - | - | - | 980 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1387 | - | - | - | 839 | 952 |
| Mov Cap-2 Maneuver | - | - | - | - | 839 | - |
| Stage 1 | - | - | - | - | 913 | - |
| Stage 2 | - | - | - | - | 980 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 3 |  | 0 |  | 10.3 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1387 | - | - | - | 846 |
| HCM Lane V/C Ratio |  | 0.009 | - | - | - | 0.192 |
| HCM Control Delay (s) |  | 7.6 | 0 | - | - | 10.3 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 0.7 |




| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 1.8 | 0 | 12.5 |
| HCM LOS |  | $B$ |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1109 | - | - | - | 334 | 779 |
| HCM Lane V/C Ratio | 0.135 | - | - | -0.159 | 0.208 |  |
| HCM Control Delay (s) | 8.8 | - | - | - | 17.8 | 10.8 |
| HCM Lane LOS | A | - | - | - | C | B |
| HCM 95th \%tile Q(veh) | 0.5 | - | - | - | 0.6 | 0.8 |



| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 341 | 0 | - | 0 | 206 | 172 |
| Stage 1 | - | - | - | - | 172 | - |
| Stage 2 | - | - | - | - | 34 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1218 | - | - | - | 782 | 872 |
| Stage 1 | - | - | - | - | 858 | - |
| Stage 2 | - | - | - |  | 988 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1218 | - | - | - | 773 | 872 |
| Mov Cap-2 Maneuver | - | - | - | - | 773 | - |
| Stage 1 | - | - | - | - | 848 | - |
| Stage 2 | - | - | - |  | 988 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 5.6 |  | 0 |  | 10.4 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1218 | - | - | - | 780 |
| HCM Lane V/C Ratio |  | 0.012 | - | - | - | 0.151 |
| HCM Control Delay (s) |  |  | 0 | - | - | 10.4 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 0.5 |



| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 559 | 0 | - | 0 | 1253 | 280 |  |
| Stage 1 | - | - | - |  | 529 | - |  |
| Stage 2 | - | - | - | - | 724 | - |  |
| Critical Hdwy | 4.14 | - | - | - | 6.84 | 6.94 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.84 | - |  |
| Critical Hdwy Stg 2 | - | - | - |  | 5.84 | - |  |
| Follow-up Hdwy | 2.22 | - | - | - | 3.52 | 3.32 |  |
| Pot Cap-1 Maneuver | 1008 | - | - | - | 164 | 717 |  |
| Stage 1 | - | - | - | - | 555 | - |  |
| Stage 2 | - | - | - |  | 441 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1008 | - | - | - | 137 | 717 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 269 | - |  |
| Stage 1 | - | - | - | - | 463 | - |  |
| Stage 2 | - | - | - |  | 441 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 1.6 |  | 0 |  | 14.4 |  |  |
| HCM LOS |  |  |  |  | B |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvm |  | EBL | EBT | WBT | WBR | BLn1 | BLn2 |
| Capacity (veh/h) |  | 1008 | - | - | - | 269 | 717 |
| HCM Lane V/C Ratio |  | 0.165 | - | - | - | 0.188 | 0.144 |
| HCM Control Delay (s) |  | 9.3 | - | - | - | 21.5 | 10.9 |
| HCM Lane LOS |  | A | - | - | - | C | B |
| HCM 95th \%tile Q(veh) |  | 0.6 | - | - | - | 0.7 | 0.5 |


|  | 4 | $\rightarrow$ |  | $\checkmark$ | － |  | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\checkmark$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 性官 |  | ${ }^{7}$ | 个种 | $\stackrel{7}{ }$ | \％ | $\uparrow$ |  | \％ | $\uparrow$ | F |
| Trafic Volume（vph） | 55 | 362 | 148 | 103 | 823 | 104 | 101 | 33 | 32 | 74 | 68 | 115 |
| Future Volume（vph） | 55 | 362 | 148 | 103 | 823 | 104 | 101 | 33 | 32 | 74 | 68 | 115 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length（ft） | 140 |  | 0 | 185 |  | 185 | 50 |  | 0 | 165 |  | 150 |
| Storage Lanes | 1 |  | 0 | 1 |  | 1 | 1 |  | 0 | 1 |  | 1 |
| Taper Length（ft） | 90 |  |  | 130 |  |  | 25 |  |  | 110 |  |  |
| Lane Util．Factor | 1.00 | 0.91 | 0.91 | 1.00 | 0.91 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.950 |  |  |  | 0.850 |  | 0.923 |  |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（prot） | 1770 | 4831 | 0 | 1770 | 5085 | 1583 | 1770 | 1719 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.296 |  |  | 0.387 |  |  | 0.575 |  |  | 0.701 |  |  |
| Satd．Flow（perm） | 551 | 4831 | 0 | 721 | 5085 | 1583 | 1071 | 1719 | 0 | 1306 | 1863 | 1583 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  | 112 |  |  |  | 109 |  | 30 |  |  |  | 125 |
| Link Speed（mph） |  | 40 |  |  | 40 |  |  | 30 |  |  | 30 |  |
| Link Distance（ft） |  | 497 |  |  | 574 |  |  | 515 |  |  | 591 |  |
| Travel Time（s） |  | 8.5 |  |  | 9.8 |  |  | 11.7 |  |  | 13.4 |  |
| Peak Hour Factor | 0.95 | 0.95 | 0.77 | 0.77 | 0.95 | 0.95 | 0.61 | 0.78 | 0.73 | 0.92 | 0.78 | 0.92 |
| Adj．Flow（vph） | 58 | 381 | 192 | 134 | 866 | 109 | 166 | 42 | 44 | 80 | 87 | 125 |


| Shared Lane Trafic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group Flow（vph） | 58 | 573 | 0 | 134 | 866 | 109 | 166 | 86 | 0 | 80 | 87 | 125 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width（t） |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset（ft） |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width（ft） |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  | Yes |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed（mph） | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors | 1 | 2 |  | 1 | 2 | 1 | 1 | 2 |  | 1 | 2 | 1 |
| Detector Template | Left | Thru |  | Left | Thru | Right | Left | Thru |  | Left | Thru | Right |
| Leading Detector（ft） | 20 | 100 |  | 20 | 100 | 20 | 20 | 100 |  | 20 | 100 | 20 |
| Trailing Detector（ft） | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Position（ft） | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Size（ft） | 20 | 6 |  | 20 | 6 | 20 | 20 | 6 |  | 20 | 6 | 20 |
| Detector 1 Type | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl＋Ex | Cl＋Ex | Cl＋Ex | $\mathrm{Cl}+\mathrm{Ex}$ | $\mathrm{Cl}+\mathrm{Ex}$ |  | Cl＋Ex | Cl＋Ex | Cl＋Ex |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 2 Position（ft） |  | 94 |  |  | 94 |  |  | 94 |  |  | 94 |  |
| Detector 2 Size（ft） |  | 6 |  |  | 6 |  |  | 6 |  |  | 6 |  |
| Detector 2 Type |  | Cl＋Ex |  |  | Cl＋Ex |  |  | Cl＋Ex |  |  | $\mathrm{Cl}+\mathrm{Ex}$ |  |


| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Detector 2 Extend（s） |  | 0.0 |  | 0.0 |  |  | 0.0 |  | 0.0 |  |
| Turn Type | pm＋pt | NA | pm＋pt | NA | Perm | pm＋pt | NA | pm＋pt | NA | Perm |
| Protected Phases | 5 | 2 | 1 | 6 |  | 7 | 4 | 3 | 8 |  |
| Permitted Phases | 2 |  | 6 |  | 6 | 4 |  | 8 |  | 8 |


|  | $\rangle$ |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Detector Phase | 5 | 2 |  | 1 | 6 | 6 | 7 | 4 |  | 3 | 8 | 8 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 20.0 |  | 4.0 | 20.0 | 20.0 | 4.0 | 4.0 |  | 4.0 | 4.0 | 4.0 |
| Minimum Split (s) | 10.5 | 26.0 |  | 10.5 | 26.0 | 26.0 | 10.5 | 24.5 |  | 10.5 | 10.5 | 10.5 |
| Total Split (s) | 25.0 | 71.0 |  | 25.0 | 71.0 | 71.0 | 25.0 | 25.0 |  | 25.0 | 25.0 | 25.0 |
| Total Split (\%) | 17.1\% | 48.6\% |  | 17.1\% | 48.6\% | 48.6\% | 17.1\% | 17.1\% |  | 17.1\% | 17.1\% | 17.1\% |
| Maximum Green (s) | 20.0 | 65.0 |  | 20.0 | 65.0 | 65.0 | 20.0 | 18.5 |  | 20.0 | 18.5 | 18.5 |
| Yellow Time (s) | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 | 3.0 | 4.5 |  | 3.0 | 4.5 | 4.5 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 6.0 |  | 5.0 | 6.0 | 6.0 | 5.0 | 6.5 |  | 5.0 | 6.5 | 6.5 |
| Lead/Lag | Lead | Lag |  | Lead | Lag | Lag | Lead | Lag |  | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes |  | Yes | Yes | Yes | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 |
| Recall Mode | None | C-Max |  | None | C-Max | C-Max | None | Max |  | None | Max | Max |
| Walk Time (s) |  | 7.0 |  |  | 7.0 | 7.0 |  | 7.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 11.0 |  |  | 11.0 | 11.0 |  | 11.0 |  |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 |  |  | 0 | 0 |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 83.3 | 75.0 |  | 89.4 | 79.8 | 79.8 | 44.7 | 28.2 |  | 34.6 | 22.8 | 22.8 |
| Actuated g/C Ratio | 0.57 | 0.51 |  | 0.61 | 0.55 | 0.55 | 0.31 | 0.19 |  | 0.24 | 0.16 | 0.16 |
| v/c Ratio | 0.15 | 0.23 |  | 0.26 | 0.31 | 0.12 | 0.41 | 0.24 |  | 0.23 | 0.30 | 0.36 |
| Control Delay | 12.0 | 15.9 |  | 12.6 | 19.0 | 3.2 | 42.1 | 35.5 |  | 39.0 | 59.2 | 12.0 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 12.0 | 15.9 |  | 12.6 | 19.0 | 3.2 | 42.1 | 35.5 |  | 39.0 | 59.2 | 12.0 |
| LOS | B | B |  | B | B | A | D | D |  | D | E | B |
| Approach Delay |  | 15.5 |  |  | 16.7 |  |  | 39.9 |  |  | 33.5 |  |
| Approach LOS |  | B |  |  | B |  |  | D |  |  | C |  |
| Queue Length 50th (ft) | 20 | 84 |  | 49 | 165 | 0 | 121 | 45 |  | 55 | 74 | 0 |
| Queue Length 95th (tt) | 39 | 114 |  | 67 | 201 | 31 | 120 | 82 |  | 99 | 115 | 61 |
| Internal Link Dist (ft) |  | 417 |  |  | 494 |  |  | 435 |  |  | 511 |  |
| Turn Bay Length (ft) | 140 |  |  | 185 |  | 185 | 50 |  |  | 165 |  | 150 |
| Base Capacity (vph) | 510 | 2534 |  | 593 | 2779 | 914 | 425 | 356 |  | 449 | 291 | 352 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Spill back Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.11 | 0.23 |  | 0.23 | 0.31 | 0.12 | 0.39 | 0.24 |  | 0.18 | 0.30 | 0.36 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

Area Type: Other
Cycle Length: 146
Actuated Cycle Length: 146
Offset: $65(45 \%)$, Referenced to phase 2:EBTL and 6:WBTL, Start of Yellow
Natural Cycle: 75
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.41
Intersection Signal Delay: 21.1 Intersection LOS: C
Intersection Capacity Utilization 49.2\% ICU Level of Service A
Analysis Period (min) 15

Splits and Phases: 1: Waynoka P//Tutt Blvd \& Constitution Ave


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |




| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 808 | 268 | 361 | 0 | - | 0 |  |
| Stage 1 | 268 | - | - | - | - | - |  |
| Stage 2 | 540 | - | - | - | - | - |  |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |  |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |  |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |  |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |  |
| Pot Cap-1 Maneuver | 350 | 771 | 1198 | - | - | - |  |
| Stage 1 | 777 | - | - | - | - | - |  |
| Stage 2 | 584 | - | - | - | - | - |  |
| Platoon blocked, \% |  |  |  | - | - | - |  |
| Mov Cap-1 Maneuver | 311 | 771 | 1198 | - | - | - |  |
| Mov Cap-2 Maneuver | 429 | - | - | - | - | - |  |
| Stage 1 | 690 | - | - | - | - | - |  |
| Stage 2 | 584 | - | - | - | - | - |  |
|  |  |  |  |  |  |  |  |
| $\begin{aligned} & \text { Approach } \\ & \hline \text { HCM Control Delay, s } \\ & \text { HCM LOS } \end{aligned}$ | EB |  | NB |  | SB |  |  |
|  | 0 |  | 2.8 |  | 0 |  |  |
|  | A |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT | n1 | SBT | SBR |  |
| Capacity (veh/h) |  | 1198 | - | - | - | - |  |
| HCM Lane V/C Ratio |  | 0.112 | - | - | - | - |  |
| HCM Control Delay (s) |  | 8.4 | - | 0 | - | - |  |
| HCM Lane LOS |  | A | - | A | - | - |  |
| HCM 95th \%tile Q(veh) |  | 0.4 | - | - | - | - |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 284 | 0 | - | 0 | 163 | 145 |
| Stage 1 | - | - | - |  | 145 | - |
| Stage 2 | - | - | - | - | 18 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - |  | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1278 | - | - | - | 828 | 902 |
| Stage 1 | - | - | - |  | 882 | - |
| Stage 2 | - | - | - |  | 1005 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1278 | - | - | - | 824 | 902 |
| Mov Cap-2 Maneuver | - | - | - |  | 824 | - |
| Stage 1 | - | - | - | - | 878 | - |
| Stage 2 | - | - | - |  | 1005 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 3.9 |  | 0 |  | 11.9 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1278 | - | - | - | 828 |
| HCM Lane V/C Ratio |  | 0.005 | - | - | - | 0.37 |
| HCM Control Delay (s) |  | 7.8 | 0 | - | - | 11.9 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 1.7 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| $l$ |  |  |  |  |  |  |



| Intersection |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | :--- |




| Major/Minor $\quad$ N | Minor2 |  | Major1 |  | Major2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 777 | 243 | 311 | 0 | - | 0 |  |
| Stage 1 | 243 | - | - | - | - | - |  |
| Stage 2 | 534 | - | - | - | - | - |  |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |  |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |  |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |  |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |  |
| Pot Cap-1 Maneuver | 365 | 796 | 1249 | - | - | - |  |
| Stage 1 | 797 | - | - | - | - | - |  |
| Stage 2 | 588 | - | - | - | - | - |  |
| Platoon blocked, \% |  |  |  | - | - | - |  |
| Mov Cap-1 Maneuver | 337 | 796 | 1249 | - | - | - |  |
| Mov Cap-2 Maneuver | 448 | - | - | - | - | - |  |
| Stage 1 | 735 | - | - | - | - | - |  |
| Stage 2 | 588 | - | - | - | - | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | NB |  | SB |  |  |
| HCM Control Delay, s | 0 |  | 1.8 |  | 0 |  |  |
| HCM LOS | A |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT | 1 | SBT | SBR |  |
| Capacity (veh/h) |  | 1249 | - | - | - | - |  |
| HCM Lane V/C Ratio |  | 0.078 | - | - | - | - |  |
| HCM Control Delay (s) |  | 8.1 | - | 0 | - | - |  |
| HCM Lane LOS |  | A | - | A | - | - |  |
| HCM 95th \%tile Q(veh) |  | 0.3 | - | - | - | - |  |



|  | Minor2 |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | :--- |
| Major/Minor | Major1 | Major2 |  |  |  |
| Conflicting Flow All | 531 | 201 | 209 | 0 | - |
| $\quad$ Stage 1 | 201 | - | - | - | - |
| $\quad$ Stage 2 | 330 | - | - | - | - |


| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 10.8 | 0.3 | 0 |
| HCM LOS | B |  |  |


| Minor Lane/Major Mvmt | NBL | NBT EBLn1 | SBT | SBR |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1362 | -653 | - | - |
| HCM Lane V/C Ratio | 0.009 | -0.043 | - | - |
| HCM Control Delay (s) | 7.7 | -10.8 | - | - |
| HCM Lane LOS | A | - | B | - |
| HCM 95th \%tile Q(veh) | 0 | - | 0.1 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor $\quad$ N | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 334 | 0 | - | 0 | 222 | 177 |
| Stage 1 | - | - | - | - | 177 | - |
| Stage 2 | - | - | - | - | 45 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1225 | - | - | - | 766 | 866 |
| Stage 1 | - | - | - | - | 854 | - |
| Stage 2 | - | - | - | - | 977 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1225 | - | - | - | 758 | 866 |
| Mov Cap-2 Maneuver | - | - | - | - | 758 | - |
| Stage 1 | - | - | - | - | 845 | - |
| Stage 2 | - | - | - | - | 977 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 3.2 |  | 0 |  | 12.1 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1225 | - | - | - | 763 |
| HCM Lane V/C Ratio |  | 0.01 | - | - | - | 0.336 |
| HCM Control Delay (s) |  | 8 | 0 | - | - | 12.1 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 1.5 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor $\quad$ N | Major1 |  | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 487 | 0 | - | 0 | 1135 | 244 |  |
| Stage 1 | - | - | - | - | 455 | - |  |
| Stage 2 | - | - | - | - | 680 | - |  |
| Critical Hdwy | 4.14 | - | - | - | 6.84 | 6.94 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.84 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.84 | - |  |
| Follow-up Hdwy | 2.22 | - | - | - | 3.52 | 3.32 |  |
| Pot Cap-1 Maneuver | 1072 | - | - | - | 196 | 757 |  |
| Stage 1 | - | - | - | - | 606 | - |  |
| Stage 2 | - | - | - | - | 465 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1072 | - | - | - | 161 | 757 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 292 | - |  |
| Stage 1 | - | - | - | - | 496 | - |  |
| Stage 2 | - | - | - | - | 465 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |  |
| HCM Control Delay, s | 2.3 |  | 0 |  | 14.4 |  |  |
| HCM LOS |  |  |  |  | B |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| Capacity (veh/h) |  | 1072 | - | - | - | 292 | 757 |
| HCM Lane V/C Ratio |  | 0.181 | - | - | - | 0.269 | 0.237 |
| HCM Control Delay (s) |  | 9.1 | - | - | - | 21.8 | 11.2 |
| HCM Lane LOS |  | A | - | - | - | C | B |
| HCM 95th \%tile Q(veh) |  | 0.7 | - | - | - | 1.1 | 0.9 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 495 | 122 | 125 | 0 | - | 0 |  |
| Stage 1 | 122 | - | - | - | - | - |  |
| Stage 2 | 373 | - | - | - | - | - |  |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |  |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |  |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |  |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |  |
| Pot Cap-1 Maneuver | 534 | 929 | 1462 | - | - | - |  |
| Stage 1 | 903 | - | - | - | - | - |  |
| Stage 2 | 696 | - | - | - | - | - |  |
| Platoon blocked, \% |  |  |  | - | - | - |  |
| Mov Cap-1 Maneuver | 532 | 929 | 1462 | - | - | - |  |
| Mov Cap-2 Maneuver | 589 | - | - | - | - | - |  |
| Stage 1 | 900 | - | - | - | - | - |  |
| Stage 2 | 696 | - | - | - | - | - |  |
|  |  |  |  |  |  |  |  |
| $\begin{aligned} & \text { Approach } \\ & \text { HCM Control Delay, s } \\ & \text { HCM LOS } \end{aligned}$ | EB |  | NB |  | SB |  |  |
|  | 0 |  | 0.1 |  | 0 |  |  |
|  | A |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT | n1 | SBT | SBR |  |
| Capacity (veh/h) |  | 1462 | - | - | - | - |  |
| HCM Lane V/C Ratio |  | 0.003 | - | - | - | - |  |
| HCM Control Delay (s) |  | 7.5 | - | 0 | - | - |  |
| HCM Lane LOS |  | A | - | A | - | - |  |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | - |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | r |  | 1 | 4 | 个 |  |
| Traffic Vol, veh/h | 0 | 0 | 0 | 308 | 100 | 0 |
| Future Vol, veh/h | 0 | 0 | 0 | 308 | 100 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | 50 | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 50 | 50 | 50 | 85 | 92 | 50 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 0 | 0 | 362 | 109 | 0 |


| Major/Minor | Minor2 |  | Major1 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 471 | 109 | 109 | 0 | - | 0 |
| Stage 1 | 109 | - | - | - | - | - |
| Stage 2 | 362 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |
| Pot Cap-1 Maneuver | 551 | 945 | 1481 | - | - | - |
| Stage 1 | 916 | - | - | - | - | - |
| Stage 2 | 704 | - | - | - | - | - |
| Platoon blocked, \% |  |  |  | - | - | - |
| Mov Cap-1 Maneuver | 551 | 945 | 1481 | - | - | - |
| Mov Cap-2 Maneuver | 601 | - | - | - | - | - |
| Stage 1 | 916 | - | - | - | - | - |
| Stage 2 | 704 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | NB |  | B |  |
| HCM Control Delay, s | 0 |  | 0 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT EBLn1 |  | 1 SBT |  |
| Capacity (veh/h) |  | 1481 | - | - | - | - |
| HCM Lane V/C Ratio |  | - | - | - | - | - |
| HCM Control Delay (s) |  | 0 | - | 0 | - | - |
| HCM Lane LOS |  | A | - | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 330 | 0 | - | 0 | 203 | 167 |
| Stage 1 | - | - | - | - | 167 | - |
| Stage 2 | - | - | - | - | 36 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1229 | - | - | - | 786 | 877 |
| Stage 1 | - | - | - | - | 863 | - |
| Stage 2 | - | - | - | - | 986 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1229 | - | - | - | 777 | 877 |
| Mov Cap-2 Maneuver | - | - | - | - | 777 | - |
| Stage 1 | - | - | - | - | 853 | - |
| Stage 2 | - | - | - | - | 986 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 5.6 |  | 0 |  | 10.3 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1229 | - | - | - | 785 |
| HCM Lane V/C Ratio |  | 0.013 | - | - | - | 0.142 |
| HCM Control Delay (s) |  | 8 | 0 | - | - | 10.3 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 0.5 |




|  | $\stackrel{ }{ }$ |  |  | 7 |  |  | 4 | $\dagger$ | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 惺 |  | \％ | 个种 | F | \％ | 1 |  | \％ | $\uparrow$ | F |
| Traffic Volume（vph） | 100 | 650 | 100 | 75 | 1300 | 150 | 65 | 75 | 25 | 100 | 90 | 230 |
| Future Volume（vph） | 100 | 650 | 100 | 75 | 1300 | 150 | 65 | 75 | 25 | 100 | 90 | 230 |
| Ideal Flow（vphpl） | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length（ft） | 140 |  | 0 | 185 |  | 185 | 50 |  | 0 | 165 |  | 150 |
| Storage Lanes | 1 |  | 0 | 1 |  | 1 | 1 |  | 0 | 1 |  | 1 |
| Taper Length（ft） | 90 |  |  | 130 |  |  | 25 |  |  | 110 |  |  |
| Lane Util．Factor | 1.00 | 0.91 | 0.91 | 1.00 | 0.91 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.969 |  |  |  | 0.850 |  | 0.967 |  |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd．Flow（prot） | 1770 | 4928 | 0 | 1770 | 5085 | 1583 | 1770 | 1801 | 0 | 1770 | 1863 | 1583 |
| Flt Permitted | 0.134 |  |  | 0.281 |  |  | 0.615 |  |  | 0.563 |  |  |
| Satd．Flow（perm） | 250 | 4928 | 0 | 523 | 5085 | 1583 | 1146 | 1801 | 0 | 1049 | 1863 | 1583 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd．Flow（RTOR） |  | 58 |  |  |  | 118 |  | 8 |  |  |  | 250 |
| Link Speed（mph） |  | 40 |  |  | 40 |  |  | 30 |  |  | 30 |  |
| Link Distance（ft） |  | 497 |  |  | 574 |  |  | 515 |  |  | 591 |  |
| Travel Time（s） |  | 8.5 |  |  | 9.8 |  |  | 11.7 |  |  | 13.4 |  |
| Peak Hour Factor | 0.95 | 0.95 | 0.56 | 0.77 | 0.95 | 0.95 | 0.53 | 0.63 | 0.73 | 0.92 | 0.78 | 0.92 |
| Adj．Flow（vph） | 105 | 684 | 179 | 97 | 1368 | 158 | 123 | 119 | 34 | 109 | 115 | 250 |
| Shared Lane Traffic（\％） |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow（vph） | 105 | 863 | 0 | 97 | 1368 | 158 | 123 | 153 | 0 | 109 | 115 | 250 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width（ft） |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset（ft） |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width（ft） |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  | Yes |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed（mph） | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 | 15 |  | 9 |
| Number of Detectors | 1 | 2 |  | ， | 2 | 1 | ， | 2 |  | 1 | 2 | 1 |
| Detector Template | Left | Thru |  | Left | Thru | Right | Left | Thru |  | Left | Thru | Right |
| Leading Detector（ft） | 20 | 100 |  | 20 | 100 | 20 | 20 | 100 |  | 20 | 100 | 20 |
| Trailing Detector（tt） | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Position（ft） | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Size（ft） | 20 | 6 |  | 20 | 6 | 20 | 20 | 6 |  | 20 | 6 | 20 |
| Detector 1 Type | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | Cl＋Ex | Cl＋Ex | Cl＋Ex | Cl＋Ex |  | Cl＋Ex | Cl＋Ex | Cl＋Ex |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay（s） | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 2 Position（ft） |  | 94 |  |  | 94 |  |  | 94 |  |  | 94 |  |
| Detector 2 Size（ft） |  | 6 |  |  | 6 |  |  | 6 |  |  | 6 |  |
| Detector 2 Type |  | Cl＋Ex |  |  | Cl＋Ex |  |  | Cl＋Ex |  |  | Cl＋Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend（s） |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Turn Type | pm＋pt | NA |  | pm＋pt | NA | Perm | pm＋pt | NA |  | pm＋pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 7 | 4 |  | 3 | 8 |  |
| Permitted Phases | 2 |  |  | 6 |  | 6 | 4 |  |  | 8 |  | 8 |


|  | 4 |  |  |  |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Detector Phase | 5 | 2 |  | 1 | 6 | 6 | 7 | 4 |  | 3 | 8 | 8 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 20.0 |  | 4.0 | 20.0 | 20.0 | 4.0 | 4.0 |  | 4.0 | 4.0 | 4.0 |
| Minimum Split (s) | 10.5 | 26.0 |  | 10.5 | 26.0 | 26.0 | 10.5 | 24.5 |  | 10.5 | 10.5 | 10.5 |
| Total Split (s) | 25.0 | 71.0 |  | 25.0 | 71.0 | 71.0 | 25.0 | 25.0 |  | 25.0 | 25.0 | 25.0 |
| Total Split (\%) | 17.1\% | 48.6\% |  | 17.1\% | 48.6\% | 48.6\% | 17.1\% | 17.1\% |  | 17.1\% | 17.1\% | 17.1\% |
| Maximum Green (s) | 20.0 | 65.0 |  | 20.0 | 65.0 | 65.0 | 20.0 | 18.5 |  | 20.0 | 18.5 | 18.5 |
| Yellow Time (s) | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 | 3.0 | 4.5 |  | 3.0 | 4.5 | 4.5 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 6.0 |  | 5.0 | 6.0 | 6.0 | 5.0 | 6.5 |  | 5.0 | 6.5 | 6.5 |
| Lead/Lag | Lead | Lag |  | Lead | Lag | Lag | Lead | Lag |  | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes |  | Yes | Yes | Yes | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 |
| Recall Mode | None | C-Max |  | None | C-Max | C-Max | None | Max |  | None | Max | Max |
| Walk Time (s) |  | 7.0 |  |  | 7.0 | 7.0 |  | 7.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 11.0 |  |  | 11.0 | 11.0 |  | 11.0 |  |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 |  |  | 0 | 0 |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 86.3 | 76.3 |  | 85.7 | 76.0 | 76.0 | 40.9 | 26.3 |  | 39.1 | 25.4 | 25.4 |
| Actuated g/C Ratio | 0.59 | 0.52 |  | 0.59 | 0.52 | 0.52 | 0.28 | 0.18 |  | 0.27 | 0.17 | 0.17 |
| V/c Ratio | 0.44 | 0.33 |  | 0.26 | 0.52 | 0.18 | 0.33 | 0.46 |  | 0.32 | 0.35 | 0.52 |
| Control Delay | 17.0 | 19.2 |  | 12.8 | 24.0 | 6.1 | 40.4 | 56.9 |  | 40.3 | 57.9 | 10.4 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 17.0 | 19.2 |  | 12.8 | 24.0 | 6.1 | 40.4 | 56.9 |  | 40.3 | 57.9 | 10.4 |
| LOS | B | B |  | B | C | A | D | E |  | D | E | B |
| Approach Delay |  | 18.9 |  |  | 21.6 |  |  | 49.5 |  |  | 28.8 |  |
| Approach LOS |  | B |  |  | C |  |  | D |  |  | C |  |
| Queue Length 50th (ft) | 38 | 156 |  | 35 | 304 | 18 | 87 | 125 |  | 77 | 97 | 0 |
| Queue Length 95th (tt) | 64 | 195 |  | 50 | 360 | 58 | 80 | 136 |  | 128 | 143 | 83 |
| Internal Link Dist (ft) |  | 417 |  |  | 494 |  |  | 435 |  |  | 511 |  |
| Turn Bay Length (tt) | 140 |  |  | 185 |  | 185 | 50 |  |  | 165 |  | 150 |
| Base Capacity (vph) | 364 | 2604 |  | 496 | 2648 | 880 | 436 | 331 |  | 418 | 324 | 482 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.29 | 0.33 |  | 0.20 | 0.52 | 0.18 | 0.28 | 0.46 |  | 0.26 | 0.35 | 0.52 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

Area Type: Other
Cycle Length: 146
Actuated Cycle Length: 146
Offset: 65 (45\%), Referenced to phase 2:EBTL and 6:WBTL, Start of Yellow
Natural Cycle: 75
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 0.52
Intersection Signal Delay: $24.1 \quad$ Intersection LOS: C
Intersection Capacity Utilization $57.5 \%$ ICU Level of Service B
Analysis Period (min) 15

Splits and Phases: 1: Waynoka P//Tutt Blvd \& Constitution Ave


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.7 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | A |  |  |  |
| Traffic Vol, veh/h | 5 | 5 | 5 | 100 | 135 | 15 |
| Future Vol, veh/h | 5 | 5 | 5 | 100 | 135 | 15 |
| Conflicting Peds, \#hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 83 | 83 | 83 | 83 | 83 | 83 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 6 | 6 | 6 | 120 | 163 | 18 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 126 | 0 | - | 0 | 84 | 66 |
| Stage 1 | - | - | - |  | 66 | - |
| Stage 2 | - | - | - | - | 18 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1460 | - | - | - | 918 | 998 |
| Stage 1 | - | - | - | - | 957 | - |
| Stage 2 | - | - | - | - | 1005 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1460 | - | - | - | 914 | 998 |
| Mov Cap-2 Maneuver | - | - | - | - | 914 | - |
| Stage 1 | - | - | - | - | 953 | - |
| Stage 2 | - | - | - | - | 1005 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 3.7 |  | 0 |  | 9.9 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1460 | - | - | - | 922 |
| HCM Lane V/C Ratio |  | 0.004 | - | - | - | 0.196 |
| HCM Control Delay (s) |  | 7.5 | 0 | - | - | 9.9 |
| HCM Lane LOS |  | A | A | - | - | A |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 0.7 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 337 | 0 | - | 0 | 220 | 178 |
| Stage 1 | - | - | - | - | 178 | - |
| Stage 2 | - | - | - | - | 42 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1222 | - | - | - | 768 | 865 |
| Stage 1 | - | - | - | - | 853 | - |
| Stage 2 | - | - | - | - | 980 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1222 | - | - | - | 760 | 865 |
| Mov Cap-2 Maneuver | - | - | - | - | 760 | - |
| Stage 1 | - | - | - |  | 844 | - |
| Stage 2 | - | - | - |  | 980 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 3.2 |  | 0 |  | 11.1 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1222 | - | - | - | 766 |
| HCM Lane V/C Ratio |  | 0.01 | - | - | - | 0.236 |
| HCM Control Delay (s) |  | 8 | 0 | - | - | 11.1 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 0.9 |




| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 1.6 | 0 | 12.9 |
| HCM LOS |  | $B$ |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 999 | - | - | -379 | 712 |  |
| HCM Lane V/C Ratio | 0.15 | - | - | -0.167 | 0.247 |  |
| HCM Control Delay (s) | 9.2 | - | - | - | 16.4 | 11.7 |
| HCM Lane LOS | A | - | - | - | C | B |
| HCM 95th \%tile Q(veh) | 0.5 | - | - | - | 0.6 | 1 |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds $300 s \quad+$ : Computation Not Defined $\quad$ : All major volume in platoon

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.1 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | A |  |  |  |
| Traffic Vol, veh/h | 12 | 5 | 3 | 308 | 107 | 8 |
| Future Vol, veh/h | 12 | 5 | 3 | 308 | 107 | 8 |
| Conflicting Peds, \#hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 83 | 83 | 87 | 87 | 83 | 83 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 14 | 6 | 3 | 354 | 129 | 10 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 357 | 0 | - | 0 | 214 | 180 |
| Stage 1 | - | - | - | - | 180 | - |
| Stage 2 | - | - | - | - | 34 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1202 | - | - | - | 774 | 863 |
| Stage 1 | - | - | - | - | 851 | - |
| Stage 2 | - | - | - | - | 988 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1202 | - | - | - | 765 | 863 |
| Mov Cap-2 Maneuver | - | - | - | - | 765 | - |
| Stage 1 | - | - | - |  | 841 | - |
| Stage 2 | - | - | - |  | 988 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 5.7 |  | 0 |  | 10.7 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1202 | - | - | - | 771 |
| HCM Lane V/C Ratio |  | 0.012 | - | - | - | 0.18 |
| HCM Control Delay (s) |  |  | 0 | - | - | 10.7 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 0.7 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 1.5 | 0 | 13.9 |
| HCM LOS |  | $B$ |  |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 947 | - | - | - | 329 | 680 |
| HCM Lane V/C Ratio | 0.173 | - | - | - | 0.2 | 0.211 |
| HCM Control Delay (s) | 9.6 | - | - | - | 18.7 | 11.7 |
| HCM Lane LOS | A | - | - | - | C | B |
| HCM 95th \%tile Q(veh) | 0.6 | - | - | - | 0.7 | 0.8 |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds $300 s \quad+$ : Computation Not Defined $\quad$ : All major volume in platoon

|  | 4 |  |  | 7 | $\leftarrow$ |  | 4 | $\dagger$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \% | 慛 |  | \% | 䖮 | F | \% | $\uparrow$ |  | ${ }^{7}$ | $\uparrow$ | F |
| Trafic Volume (vph) | 100 | 650 | 175 | 147 | 1300 | 150 | 245 | 97 | 54 | 100 | 137 | 230 |
| Future Volume (vph) | 100 | 650 | 175 | 147 | 1300 | 150 | 245 | 97 | 54 | 100 | 137 | 230 |
| Ideal Flow (vphpl) | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Storage Length (ft) | 140 |  | 0 | 185 |  | 185 | 50 |  | 0 | 165 |  | 150 |
| Storage Lanes | 1 |  | 0 | 1 |  | 1 | 1 |  | 0 | 1 |  | 1 |
| Taper Length (ft) | 90 |  |  | 130 |  |  | 25 |  |  | 110 |  |  |
| Lane Util. Factor | 1.00 | 0.91 | 0.91 | 1.00 | 0.91 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Frt |  | 0.963 |  |  |  | 0.850 |  | 0.944 |  |  |  | 0.850 |
| Flt Protected | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  | 0.950 |  |  |
| Satd. Flow (prot) | 1770 | 4897 | 0 | 1770 | 5085 | 1583 | 1770 | 1758 | 0 | 1770 | 1863 | 1583 |
| FIt Permitted | 0.141 |  |  | 0.246 |  |  | 0.307 |  |  | 0.616 |  |  |
| Satd. Flow (perm) | 263 | 4897 | 0 | 458 | 5085 | 1583 | 572 | 1758 | 0 | 1147 | 1863 | 1583 |
| Right Turn on Red |  |  | Yes |  |  | Yes |  |  | Yes |  |  | Yes |
| Satd. Flow (RTOR) |  | 74 |  |  |  | 118 |  | 17 |  |  |  | 229 |
| Link Speed (mph) |  | 40 |  |  | 40 |  |  | 30 |  |  | 30 |  |
| Link Distance (ft) |  | 497 |  |  | 574 |  |  | 515 |  |  | 591 |  |
| Travel Time (s) |  | 8.5 |  |  | 9.8 |  |  | 11.7 |  |  | 13.4 |  |
| Peak Hour Factor | 0.95 | 0.95 | 0.77 | 0.77 | 0.95 | 0.95 | 0.61 | 0.78 | 0.73 | 0.92 | 0.78 | 0.92 |
| Adj. Flow (vph) | 105 | 684 | 227 | 191 | 1368 | 158 | 402 | 124 | 74 | 109 | 176 | 250 |
| Shared Lane Trafic (\%) |  |  |  |  |  |  |  |  |  |  |  |  |
| Lane Group Flow (vph) | 105 | 911 | 0 | 191 | 1368 | 158 | 402 | 198 | 0 | 109 | 176 | 250 |
| Enter Blocked Intersection | No | No | No | No | No | No | No | No | No | No | No | No |
| Lane Alignment | Left | Left | Right | Left | Left | Right | Left | Left | Right | Left | Left | Right |
| Median Width(ft) |  | 12 |  |  | 12 |  |  | 12 |  |  | 12 |  |
| Link Offset(ft) |  | 0 |  |  | 0 |  |  | 0 |  |  | 0 |  |
| Crosswalk Width(ft) |  | 16 |  |  | 16 |  |  | 16 |  |  | 16 |  |
| Two way Left Turn Lane |  |  |  |  |  |  |  | Yes |  |  |  |  |
| Headway Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Speed (mph) | 15 |  | 9 | 15 |  | 9 | 15 |  | , | 15 |  | 9 |
| Number of Detectors | 1 | 2 |  | 1 | 2 | 1 | 1 | 2 |  | 1 | 2 | 1 |
| Detector Template | Left | Thru |  | Left | Thru | Right | Left | Thru |  | Left | Thru | Right |
| Leading Detector (ft) | 20 | 100 |  | 20 | 100 | 20 | 20 | 100 |  | 20 | 100 | 20 |
| Trailing Detector (ft) | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Position(ft) | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Detector 1 Size(ft) | 20 | 6 |  | 20 | 6 | 20 | 20 | 6 |  | 20 | 6 | 20 |
| Detector 1 Type | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex | Cl+Ex |  | Cl+Ex | Cl+Ex | Cl+Ex |
| Detector 1 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 1 Extend (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Queue (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 1 Delay (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Detector 2 Position(tt) |  | 94 |  |  | 94 |  |  | 94 |  |  | 94 |  |
| Detector 2 Size(ft) |  | 6 |  |  | 6 |  |  | 6 |  |  | 6 |  |
| Detector 2 Type |  | Cl+Ex |  |  | Cl+Ex |  |  | Cl+Ex |  |  | Cl+Ex |  |
| Detector 2 Channel |  |  |  |  |  |  |  |  |  |  |  |  |
| Detector 2 Extend (s) |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |  | 0.0 |  |
| Turn Type | pm+pt | NA |  | pm+pt | NA | Perm | pm+pt | NA |  | pm+pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 7 | 4 |  | 3 | 8 |  |
| Permitted Phases | 2 |  |  | 6 |  | 6 | 4 |  |  | 8 |  | 8 |


|  | $\rangle$ |  |  |  |  |  | 4 | $\uparrow$ |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Detector Phase | 5 | 2 |  | 1 | 6 | 6 | 7 | 4 |  | 3 | 8 | 8 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 20.0 |  | 4.0 | 20.0 | 20.0 | 4.0 | 4.0 |  | 4.0 | 4.0 | 4.0 |
| Minimum Split (s) | 10.5 | 26.0 |  | 10.5 | 26.0 | 26.0 | 10.5 | 24.5 |  | 10.5 | 10.5 | 10.5 |
| Total Split (s) | 25.0 | 71.0 |  | 25.0 | 71.0 | 71.0 | 25.0 | 25.0 |  | 25.0 | 25.0 | 25.0 |
| Total Split (\%) | 17.1\% | 48.6\% |  | 17.1\% | 48.6\% | 48.6\% | 17.1\% | 17.1\% |  | 17.1\% | 17.1\% | 17.1\% |
| Maximum Green (s) | 20.0 | 65.0 |  | 20.0 | 65.0 | 65.0 | 20.0 | 18.5 |  | 20.0 | 18.5 | 18.5 |
| Yellow Time (s) | 3.0 | 4.0 |  | 3.0 | 4.0 | 4.0 | 3.0 | 4.5 |  | 3.0 | 4.5 | 4.5 |
| All-Red Time (s) | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |  | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 6.0 |  | 5.0 | 6.0 | 6.0 | 5.0 | 6.5 |  | 5.0 | 6.5 | 6.5 |
| Lead/Lag | Lead | Lag |  | Lead | Lag | Lag | Lead | Lag |  | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes |  | Yes | Yes | Yes | Yes | Yes |  | Yes | Yes | Yes |
| Vehicle Extension (s) | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |  | 3.0 | 3.0 | 3.0 |
| Recall Mode | None | C-Max |  | None | C-Max | C-Max | None | Max |  | None | Max | Max |
| Walk Time (s) |  | 7.0 |  |  | 7.0 | 7.0 |  | 7.0 |  |  | 7.0 | 7.0 |
| Flash Dont Walk (s) |  | 11.0 |  |  | 11.0 | 11.0 |  | 11.0 |  |  | 11.0 | 11.0 |
| Pedestrian Calls (\#/hr) |  | 0 |  |  | 0 | 0 |  | 0 |  |  | 0 | 0 |
| Act Effct Green (s) | 82.8 | 72.8 |  | 89.2 | 76.0 | 76.0 | 44.7 | 26.3 |  | 32.2 | 18.5 | 18.5 |
| Actuated g/C Ratio | 0.57 | 0.50 |  | 0.61 | 0.52 | 0.52 | 0.31 | 0.18 |  | 0.22 | 0.13 | 0.13 |
| v/c Ratio | 0.44 | 0.37 |  | 0.49 | 0.52 | 0.18 | 1.19 | 0.60 |  | 0.36 | 0.75 | 0.62 |
| Control Delay | 17.3 | 21.2 |  | 16.2 | 24.0 | 6.1 | 149.4 | 59.4 |  | 41.5 | 81.0 | 17.0 |
| Queue Delay | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |  | 0.0 | 0.0 | 0.0 |
| Total Delay | 17.3 | 21.2 |  | 16.2 | 24.0 | 6.1 | 149.4 | 59.4 |  | 41.5 | 81.0 | 17.0 |
| LOS | B | C |  | B | C | A | F | E |  | D | F | B |
| Approach Delay |  | 20.8 |  |  | 21.5 |  |  | 119.7 |  |  | 43.0 |  |
| Approach LOS |  | C |  |  | C |  |  | F |  |  | D |  |
| Queue Length 50th (ft) | 38 | 173 |  | 73 | 304 | 18 | ~359 | 160 |  | 77 | 164 | 18 |
| Queue Length 95th (ft) | 64 | 219 |  | 92 | 360 | 58 | \#295 | 215 |  | 128 | 213 | 108 |
| Internal Link Dist (ft) |  | 417 |  |  | 494 |  |  | 435 |  |  | 511 |  |
| Turn Bay Length (ft) | 140 |  |  | 185 |  | 185 | 50 |  |  | 165 |  | 150 |
| Base Capacity (vph) | 370 | 2480 |  | 465 | 2648 | 880 | 339 | 330 |  | 399 | 236 | 400 |
| Starvation Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 |  | 0 | 0 | 0 | 0 | 0 |  | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.28 | 0.37 |  | 0.41 | 0.52 | 0.18 | 1.19 | 0.60 |  | 0.27 | 0.75 | 0.63 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

Area Type: Other
Cycle Length: 146
Actuated Cycle Length: 146
Offset: $65(45 \%)$, Referenced to phase 2:EBTL and 6:WBTL, Start of Yellow
Natural Cycle: 75
Control Type: Actuated-Coordinated
Maximum v/c Ratio: 1.19
Intersection Signal Delay: $39.5 \quad$ Intersection LOS: D
Intersection Capacity Utilization 70.2\% ICU Level of Service C
Analysis Period (min) 15
~ Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 1: Waynoka P//Tutt Blvd \& Constitution Ave


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Minor2 | Major1 |  |  | Major2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 1373 | 379 | 565 | 0 | - | 0 |  |
| Stage 1 | 379 | - | - | - | - | - |  |
| Stage 2 | 994 | - | - | - | - | - |  |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |  |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |  |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |  |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |  |
| Pot Cap-1 Maneuver | 161 | 668 | 1007 | - | - | - |  |
| Stage 1 | 692 | - | - | - | - | - |  |
| Stage 2 | 358 | - | - | - | - | - |  |
| Platoon blocked, \% |  |  |  | - | - | - |  |
| Mov Cap-1 Maneuver | 118 | 668 | 1007 | - | - | - |  |
| Mov Cap-2 Maneuver | 243 | - | - | - | - | - |  |
| Stage 1 | 508 | - | - | - | - | - |  |
| Stage 2 | 358 | - | - | - | - | - |  |
|  |  |  |  |  |  |  |  |
| Approach | EB |  | NB |  | SB |  |  |
| HCM Control Delay, s | 0 |  | 3.6 |  | 0 |  |  |
| HCM LOS | A |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT | n1 | SBT | SBR |  |
| Capacity (veh/h) |  | 1007 | - | - | - | - |  |
| HCM Lane V/C Ratio |  | 0.266 | - | - | - | - |  |
| HCM Control Delay (s) |  | 9.9 | - | 0 | - | - |  |
| HCM Lane LOS |  | A | - | A | - | - |  |
| HCM 95th \%tile Q(veh) |  | 1.1 | - | - | - | - |  |




| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 11.8 | 0.9 | 0 |
| HCM LOS | B |  |  |


| Minor Lane/Major Mvmt | NBL | NBT EBLn1 | SBT | SBR |
| :--- | ---: | ---: | ---: | :--- |
| Capacity (veh/h) | 1249 | -602 | - | - |
| HCM Lane V/C Ratio | 0.026 | -0.116 | - | - |
| HCM Control Delay (s) | 8 | -11.8 | - | - |
| HCM Lane LOS | A | - | B | - |
| HCM 95th \%tile Q(veh) | 0.1 | - | 0.4 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 6.4 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | A |  |  |  |
| Traffic Vol, veh/h | 5 | 5 | 5 | 251 | 253 | 15 |
| Future Vol, veh/h | 5 | 5 | 5 | 251 | 253 | 15 |
| Conflicting Peds, \#hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 63 | 75 | 78 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 6 | 6 | 6 | 398 | 337 | 19 |


| Major/Minor | Major1 | Major2 |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Approach | EB | WB | SB |
| :--- | :---: | :---: | :---: |
| HCM Control Delay, s | 4.1 | 0 | 13.7 |

HCMLOS B

| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1155 | - | - | -765 |
| HCM Lane V/C Ratio | 0.006 | - | - | -0.466 |
| HCM Control Delay (s) | 8.1 | 0 | - | -13.7 |
| HCM Lane LOS | A | A | - | - |
| HCM 95th \%tile Q(veh) | 0 | - | - | - |
| H |  |  |  |  |







| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 1359 | 323 | 459 | 0 | - | 0 |  |
| Stage 1 | 323 | - | - | - | - | - |  |
| Stage 2 | 1036 | - | - | - | - | - |  |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |  |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |  |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |  |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |  |
| Pot Cap-1 Maneuver | 164 | 718 | 1102 | - | - | - |  |
| Stage 1 | 734 | - | - | - | - | - |  |
| Stage 2 | 342 | - | - | - | - | - |  |
| Platoon blocked, \% |  |  |  | - | - | - |  |
| Mov Cap-1 Maneuver | 134 | 718 | 1102 | - | - | - |  |
| Mov Cap-2 Maneuver | 254 | - | - | - | - | - |  |
| Stage 1 | 602 | - | - | - | - | - |  |
| Stage 2 | 342 | - | - | - | - | - |  |
|  |  |  |  |  |  |  |  |
| $\frac{\text { Approach }}{\text { HCM Control Delay, s }}$HCM LOS | EB |  | NB |  | SB |  |  |
|  | 0 |  | 2.1 |  | 0 |  |  |
|  | A |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT | n1 | SBT | SBR |  |
| Capacity (veh/h) |  | 1102 | - | - | - | - |  |
| HCM Lane V/C Ratio |  | 0.18 | - | - | - | - |  |
| HCM Control Delay (s) |  | 9 | - | 0 | - | - |  |
| HCM Lane LOS |  | A | - | A | - | - |  |
| HCM 95th \%tile Q(veh) |  | 0.7 | - | - | - | - |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.5 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | A |  | M |  |
| Traffic Vol, ven/h | 10 | 15 | 15 | 375 | 237 | 10 |
| Future Vol, veh/h | 10 | 15 | 15 | 375 | 237 | 10 |
| Conflicting Peds, \#hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 77 | 79 | 78 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 13 | 19 | 19 | 487 | 300 | 13 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 506 | 0 | - | 0 | 308 | 263 |
| Stage 1 | - | - | - |  | 263 | - |
| Stage 2 | - | - | - | - | 45 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1059 | - | - | - | 684 | 776 |
| Stage 1 | - | - | - | - | 781 | - |
| Stage 2 | - | - | - | - | 977 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1059 | - | - | - | 676 | 776 |
| Mov Cap-2 Maneuver | - | - | - | - | 676 | - |
| Stage 1 | - | - | - | - | 772 | - |
| Stage 2 | - | - | - | - | 977 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 3.4 |  | 0 |  | 14.7 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1059 | - | - | - | 680 |
| HCM Lane V/C Ratio |  | 0.012 | - | - | - | 0.46 |
| HCM Control Delay (s) |  | 8.4 | 0 | - | - | 14.7 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 2.4 |






| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 553 | 147 | 154 | 0 | - | 0 |  |
| Stage 1 | 147 | - | - | - | - | - |  |
| Stage 2 | 406 | - | - | - | - | - |  |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |  |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |  |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |  |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |  |
| Pot Cap-1 Maneuver | 494 | 900 | 1426 | - | - | - |  |
| Stage 1 | 880 | - | - | - | - | - |  |
| Stage 2 | 673 | - | - | - | - | - |  |
| Platoon blocked, \% |  |  |  | - | - | - |  |
| Mov Cap-1 Maneuver | 491 | 900 | 1426 | - | - | - |  |
| Mov Cap-2 Maneuver | 561 | - | - | - | - | - |  |
| Stage 1 | 874 | - | - | - | - | - |  |
| Stage 2 | 673 | - | - | - | - | - |  |
|  |  |  |  |  |  |  |  |
| $\frac{\text { Approach }}{\text { HCM Control Delay, s }}$HCM LOS | EB |  | NB |  | SB |  |  |
|  | 0 |  | 0.2 |  | 0 |  |  |
|  | A |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBL | NBT | n1 | SBT | SBR |  |
| Capacity (veh/h) |  | 1426 | - | - | - | - |  |
| HCM Lane V/C Ratio |  | 0.007 | - | - | - | - |  |
| HCM Control Delay (s) |  | 7.5 | - | 0 | - | - |  |
| HCM Lane LOS |  | A | - | A | - | - |  |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | - |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 608 | 0 | - | 0 | 342 | 306 |
| Stage 1 | - | - | - | - | 306 | - |
| Stage 2 | - | - | - | - | 36 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - |  | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 970 | - | - | - | 654 | 734 |
| Stage 1 | - | - | - |  | 747 | - |
| Stage 2 | - | - | - | - | 986 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 970 | - | - | - | 644 | 734 |
| Mov Cap-2 Maneuver | - | - | - | - | 644 | - |
| Stage 1 | - | - | - |  | 735 | - |
| Stage 2 | - | - | - | - | 986 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 6.2 |  | 0 |  | 12 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 970 | - | - | - | 650 |
| HCM Lane V/C Ratio |  | 0.016 | - | - | - | 0.205 |
| HCM Control Delay (s) |  | 8.8 | 0 | - | - | 12 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | - | 0.8 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 1.5 | 0 | 14 |
| HCM LOS |  |  | B |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 SBLn2 |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 946 | - | - | - | 332 |
| 679 |  |  |  |  |  |
| HCM Lane V/C Ratio | 0.176 | - | - | -0.205 | 0.207 |
| HCM Control Delay (s) | 9.6 | - | - | - | 18.6 |
| 11.7 |  |  |  |  |  |
| HCM Lane LOS | A | - | - | - | C |
| HCM 95th \%tile Q(veh) | 0.6 | - | - | - | 0.8 |

## Notes

$\sim$ : Volume exceeds capacity $\$$ : Delay exceeds $300 s \quad+$ : Computation Not Defined $\quad$ : All major volume in platoon

## Queuing Reports

Intersection: 2: Waynoka PI \& Middle Access, Interval \#1

| Movement | EB | EB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | $R$ | L |
| Maximum Queue (ft) | 62 | 56 | 6 |
| Average Queue (ft) | 41 | 31 | 1 |
| 95th Queue (ft) | 69 | 60 | 10 |
| Link Distance (ft) | 988 | 988 |  |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: Waynoka PI \& Middle Access, Interval \#2

| Movement | EB | EB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | R | L |
| Maximum Queue (ft) | 71 | 57 | 6 |
| Average Queue (ft) | 46 | 36 | 1 |
| 95th Queue (ft) | 78 | 60 | 9 |
| Link Distance (ft) | 988 | 988 |  |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  | 90 |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: Waynoka PI \& Middle Access, Interval \#3

| Movement | EB | EB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | R | L |
| Maximum Queue (ft) | 236 | 139 | 6 |
| Average Queue (ft) | 139 | 63 | 1 |
| 95th Queue (ft) | 263 | 136 | 9 |
| Link Distance (ft) | 988 | 988 |  |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 2: Waynoka PI \& Middle Access, Interval \#4

| Movement | EB | EB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | $R$ | L |
| Maximum Queue (ft) | 127 | 68 | 6 |
| Average Queue (ft) | 57 | 36 | 0 |
| 95th Queue (ft) | 116 | 68 | 0 |
| Link Distance (ft) | 988 | 988 |  |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 2: Waynoka PI \& Middle Access, All Intervals

| Movement | EB | EB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | R | L |
| Maximum Queue (ft) | 236 | 147 | 18 |
| Average Queue (ft) | 71 | 41 | 1 |
| 95th Queue (ft) | 168 | 90 | 8 |
| Link Distance (ft) | 988 | 988 |  |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 3: Waynoka PI \& North Access, Interval \#1

| Movement | NB | SB |
| :--- | ---: | ---: |
| Directions Served | L | TR |
| Maximum Queue (ft) | 40 | 4 |
| Average Queue (ft) | 16 | 1 |
| 95th Queue $(\mathrm{ft})$ | 45 | 6 |
| Link Distance (ft) |  |  |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 100 |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Intersection: 3: Waynoka PI \& North Access, Interval \#2

| Movement | NB |
| :--- | ---: |
| Directions Served | L |
| Maximum Queue (ft) | 54 |
| Average Queue (ft) | 22 |
| 95th Queue (ft) | 56 |
| Link Distance (ft) |  |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) | 100 |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 3: Waynoka PI \& North Access, Interval \#3

| Movement | NB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | T | TR |
| Maximum Queue (ft) | 96 | 33 | 28 |
| Average Queue (ft) | 63 | 8 | 7 |
| 95th Queue (ft) | 105 | 62 | 26 |
| Link Distance (ft) |  | 156 |  |
| Upstream Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 1 |  |
| Storage Bay Dist (ft) | 100 |  |  |
| Storage Blk Time (\%) | 2 | 0 |  |
| Queuing Penalty (veh) | 8 | 0 |  |

## Intersection: 3: Waynoka PI \& North Access, Interval \#4

| Movement | NB | SB |
| :--- | ---: | ---: |
| Directions Served | L | TR |
| Maximum Queue (ft) | 49 | 9 |
| Average Queue (ft) | 21 | 2 |
| 95th Queue (ft) | 53 | 12 |
| Link Distance (ft) |  |  |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 100 |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Intersection: 3: Waynoka PI \& North Access, All Intervals

| Movement | NB | NB | SB |  |
| :--- | ---: | ---: | ---: | :---: |
| Directions Served | L | T | TR |  |
| Maximum Queue (ft) | 96 | 33 | 28 |  |
| Average Queue (ft) | 31 | 2 | 2 |  |
| 95th Queue (ft) | 77 | 30 | 14 |  |
| Link Distance (ft) | 156 |  |  |  |
| Upstream Blk Time (\%) |  | 0 |  |  |
| Queuing Penalty (veh) |  | 0 |  |  |
| Storage Bay Dist (ft) | 100 |  |  |  |
| Storage Blk Time (\%) | 0 | 0 |  |  |
| Queuing Penalty (veh) | 2 | 0 |  |  |

## Intersection: 4: Waynoka PI \& South Access, Interval \#1

| Movement | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | L | TR |
| Maximum Queue (ft) | 33 | 25 | 6 |
| Average Queue (ft) | 14 | 4 | 1 |
| 95th Queue (ft) | 39 | 22 | 9 |
| Link Distance (ft) | 250 |  | 138 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |

Intersection: 4: Waynoka PI \& South Access, Interval \#2

| Movement | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | L | TR |
| Maximum Queue (ft) | 32 | 12 | 14 |
| Average Queue (ft) | 18 | 2 | 2 |
| 95th Queue (ft) | 42 | 14 | 20 |
| Link Distance (ft) | 250 |  | 138 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  | 50 |  |
| Storage Bay Dist (ft) |  | 0 |  |
| Storage Blk Time (\%) |  | 0 |  |

Intersection: 4: Waynoka PI \& South Access, Interval \#3

| Movement | EB | NB | SB |
| :--- | :---: | ---: | ---: |
| Directions Served | LR | L | TR |
| Maximum Queue (ft) | 52 | 31 | 14 |
| Average Queue (ft) | 35 | 11 | 2 |
| 95th Queue (ft) | 57 | 35 | 21 |
| Link Distance (ft) | 250 |  | 138 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  | 50 |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |

Intersection: 4: Waynoka PI \& South Access, Interval \#4

| Movement | EB | NB |
| :--- | ---: | ---: |
| Directions Served | LR | L |
| Maximum Queue (ft) | 28 | 24 |
| Average Queue (ft) | 16 | 4 |
| 95th Queue (ft) | 38 | 20 |
| Link Distance (ft) | 250 |  |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  | 50 |
| Storage Bay Dist (ft) |  | 0 |
| Storage Blk Time (\%) |  | 0 |
| Queuing Penalty (veh) |  | 0 |

Intersection: 4: Waynoka PI \& South Access, All Intervals

| Movement | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | L | TR |
| Maximum Queue (ft) | 52 | 31 | 27 |
| Average Queue (ft) | 21 | 5 | 1 |
| 95th Queue (tt) | 48 | 24 | 15 |
| Link Distance (ft) | 250 |  | 138 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  | 50 |  |
| Storage Bay Dist (ft) |  | 0 |  |
| Storage Blk Time (\%) |  | 0 |  |
| Queuing Penalty (veh) |  | 0 |  |

Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#1

| Movement | SB |
| :--- | :---: |
| Directions Served | LR |
| Maximum Queue (ft) | 67 |
| Average Queue (ft) | 47 |
| 95th Queue (ft) | 70 |
| Link Distance (ft) | 114 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#2

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | LT | LR |
| Maximum Queue (ft) | 6 | 68 |
| Average Queue (ft) | 1 | 47 |
| 95th Queue (ft) | 9 | 72 |
| Link Distance (ft) |  | 114 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#3

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | LT | LR |
| Maximum Queue (ft) | 18 | 90 |
| Average Queue (ft) | 3 | 60 |
| 95th Queue (ft) | 17 | 93 |
| Link Distance (ft) | 114 |  |
| Upstream Blk Time (\%) | 0 |  |
| Queuing Penalty (veh) | 0 |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#4

| Movement | SB |
| :--- | :---: |
| Directions Served | LR |
| Maximum Queue (ft) | 61 |
| Average Queue (ft) | 41 |
| 95th Queue (ft) | 63 |
| Link Distance (ft) | 114 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

Intersection: 5: Waynoka Rd \& Waynoka PI, All Intervals

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | LT | LR |
| Maximum Queue (ft) | 25 | 93 |
| Average Queue (ft) | 1 | 49 |
| 95th Queue (ft) | 9 | 77 |
| Link Distance (ft) | 114 |  |
| Upstream Blk Time (\%) |  | 0 |
| Queuing Penalty (veh) | 0 |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Intersection: 2: Waynoka PI \& Middle Access, Interval \#1

| Movement | EB | EB |
| :--- | ---: | ---: |
| Directions Served | L | R |
| Maximum Queue (ft) | 60 | 39 |
| Average Queue (ft) | 38 | 29 |
| 95th Queue (ft) | 60 | 46 |
| Link Distance (ft) | 988 | 988 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: Waynoka PI \& Middle Access, Interval \#2

| Movement | EB | EB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | R | L |
| Maximum Queue (ft) | 58 | 35 | 6 |
| Average Queue (ft) | 35 | 26 | 0 |
| 95th Queue (ft) | 56 | 46 | 0 |
| Link Distance (ft) | 988 | 988 |  |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  | 90 |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 2: Waynoka PI \& Middle Access, Interval \#3

| Movement | EB | EB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | R | L |
| Maximum Queue (ft) | 226 | 59 | 12 |
| Average Queue (ft) | 143 | 42 | 3 |
| 95th Queue (ft) | 273 | 65 | 16 |
| Link Distance (ft) | 988 | 988 |  |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

Intersection: 2: Waynoka PI \& Middle Access, Interval \#4

| Movement | EB | EB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | R | L |
| Maximum Queue (ft) | 115 | 48 | 12 |
| Average Queue (ft) | 47 | 29 | 2 |
| 95th Queue (ft) | 112 | 54 | 13 |
| Link Distance (ft) | 988 | 988 |  |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 2: Waynoka PI \& Middle Access, All Intervals

| Movement | EB | EB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | R | L |
| Maximum Queue (ft) | 226 | 64 | 24 |
| Average Queue (ft) | 66 | 31 | 1 |
| 95th Queue (ft) | 170 | 55 | 10 |
| Link Distance (ft) | 988 | 988 |  |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

## Intersection: 3: Waynoka PI \& North Access, Interval \#1

| Movement | NB | SB |
| :--- | :---: | ---: |
| Directions Served | L | TR |
| Maximum Queue (ft) | 31 | 4 |
| Average Queue (ft) | 14 | 1 |
| 95th Queue (ft) | 39 | 7 |
| Link Distance (ft) |  |  |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 100 |  |
| Storage Blk Time (\%) |  |  |

Intersection: 3: Waynoka PI \& North Access, Interval \#2

| Movement | NB |
| :--- | :---: |
| Directions Served | L |
| Maximum Queue (ft) | 31 |
| Average Queue (ft) | 12 |
| 95th Queue (ft) | 37 |
| Link Distance (ft) |  |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) | 100 |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 3: Waynoka PI \& North Access, Interval \#3

| Movement | NB | SB |
| :--- | ---: | ---: |
| Directions Served | L | TR |
| Maximum Queue (ft) | 72 | 23 |
| Average Queue (ft) | 45 | 5 |
| 95th Queue (ft) | 73 | 22 |
| Link Distance (ft) |  |  |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 100 |  |
| Storage Blk Time (\%) | 1 |  |
| Queuing Penalty (veh) | 3 |  |

## Intersection: 3: Waynoka PI \& North Access, Interval \#4

| Movement | NB | SB |
| :--- | ---: | ---: |
| Directions Served | L | TR |
| Maximum Queue (ft) | 39 | 4 |
| Average Queue (ft) | 15 | 1 |
| 95th Queue (ft) | 42 | 7 |
| Link Distance (ft) |  |  |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 100 |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Intersection: 3: Waynoka PI \& North Access, All Intervals

| Movement | NB | SB |
| :--- | ---: | ---: |
| Directions Served | L | TR |
| Maximum Queue (ft) | 72 | 28 |
| Average Queue (ft) | 22 | 1 |
| 95th Queue (ft) | 56 | 12 |
| Link Distance (ft) |  |  |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) | 100 |  |
| Storage Blk Time (\%) | 0 |  |
| Queuing Penalty (veh) | 1 |  |

## Intersection: 4: Waynoka PI \& South Access, Interval \#1

| Movement | EB | NB |
| :--- | ---: | ---: |
| Directions Served | LR | L |
| Maximum Queue (ft) | 28 | 12 |
| Average Queue (ft) | 12 | 2 |
| 95th Queue (ft) | 35 | 14 |
| Link Distance (ft) | 250 |  |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  | 50 |
| Storage Bay Dist (ft) |  | 0 |
| Storage Blk Time (\%) |  | 0 |

Intersection: 4: Waynoka PI \& South Access, Interval \#2

| Movement | EB |
| :--- | :---: |
| Directions Served | LR |
| Maximum Queue (ft) | 37 |
| Average Queue (ft) | 14 |
| 95th Queue (ft) | 41 |
| Link Distance (ft) | 250 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

Intersection: 4: Waynoka PI \& South Access, Interval \#3

| Movement | EB | NB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | L | T |
| Maximum Queue (ft) | 49 | 29 | 6 |
| Average Queue (ft) | 27 | 9 | 1 |
| 95th Queue (ft) | 56 | 33 | 9 |
| Link Distance (ft) | 250 |  | 114 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  | 50 |  |
| Storage Bay Dist (ft) |  | 0 |  |
| Storage Blk Time (\%) |  | 1 |  |
| Queuing Penalty (veh) |  | 1 |  |

Intersection: 4: Waynoka PI \& South Access, Interval \#4

| Movement | EB | NB |
| :--- | ---: | ---: |
| Directions Served | LR | L |
| Maximum Queue (ft) | 28 | 12 |
| Average Queue (ft) | 12 | 3 |
| 95th Queue (ft) | 35 | 17 |
| Link Distance (ft) | 250 |  |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  | 50 |
| Storage Bay Dist (ft) |  | 0 |
| Storage Blk Time (\%) |  | 0 |
| Queuing Penalty (veh) |  | 0 |

Intersection: 4: Waynoka PI \& South Access, All Intervals

| Movement | EB | NB | NB |
| :--- | ---: | ---: | ---: |
| Directions Served | LR | L | T |
| Maximum Queue (ft) | 49 | 29 | 6 |
| Average Queue (ft) | 16 | 3 | 0 |
| 95th Queue (ft) | 44 | 19 | 5 |
| Link Distance (ft) | 250 |  | 114 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  | 50 |  |
| Storage Bay Dist (ft) |  | 0 |  |
| Storage Blk Time (\%) |  | 0 |  |

Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#1

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | LT | LR |
| Maximum Queue (ft) | 30 | 68 |
| Average Queue (ft) | 4 | 42 |
| 95th Queue (ft) | 22 | 68 |
| Link Distance (ft) | 632 | 114 |
| Upstream Blk Time (\%) |  | 0 |
| Queuing Penalty (veh) |  | 0 |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#2

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | LT | LR |
| Maximum Queue (ft) | 19 | 64 |
| Average Queue (ft) | 4 | 44 |
| 95th Queue (ft) | 20 | 67 |
| Link Distance (ft) | 632 | 114 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#3

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | LT | LR |
| Maximum Queue (ft) | 25 | 85 |
| Average Queue (ft) | 8 | 57 |
| 95th Queue (ft) | 29 | 92 |
| Link Distance (ft) | 632 | 114 |
| Upstream Blk Time (\%) |  | 0 |
| Queuing Penalty (veh) |  | 1 |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#4

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LR |
| Maximum Queue (ft) | 22 | 4 | 78 |
| Average Queue (ft) | 3 | 1 | 47 |
| 95th Queue (ft) | 18 | 7 | 78 |
| Link Distance (ft) | 632 | 316 | 114 |
| Upstream Blk Time (\%) |  |  | 0 |
| Queuing Penalty (veh) |  |  | 0 |
| Storage Bay Dist (ft) |  |  |  |

Intersection: 5: Waynoka Rd \& Waynoka PI, All Intervals

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LR |
| Maximum Queue (ft) | 31 | 4 | 94 |
| Average Queue (ft) | 5 | 0 | 47 |
| 95th Queue (ft) | 23 | 3 | 78 |
| Link Distance (ft) | 632 | 316 | 114 |
| Upstream Blk Time (\%) |  |  | 0 |
| Queuing Penalty (veh) |  |  | 0 |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |

Intersection: 2: Waynoka PI \& Middle Access, Interval \#1

| Movement | EB | EB |
| :--- | ---: | ---: |
| Directions Served | L | R |
| Maximum Queue (ft) | 17 | 17 |
| Average Queue (ft) | 4 | 3 |
| 95th Queue (ft) | 21 | 17 |
| Link Distance (ft) | 988 | 988 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: Waynoka PI \& Middle Access, Interval \#2

| Movement | EB | EB |
| :--- | ---: | ---: |
| Directions Served | L | R |
| Maximum Queue (ft) | 23 | 6 |
| Average Queue (ft) | 7 | 2 |
| 95th Queue (ft) | 26 | 12 |
| Link Distance (ft) | 988 | 988 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: Waynoka PI \& Middle Access, Interval \#3

| Movement | EB | EB |
| :--- | ---: | ---: |
| Directions Served | L | R |
| Maximum Queue (ft) | 29 | 30 |
| Average Queue (ft) | 9 | 4 |
| 95th Queue (ft) | 31 | 22 |
| Link Distance (ft) | 988 | 988 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Intersection: 2: Waynoka PI \& Middle Access, Interval \#4

| Movement | EB | EB |
| :--- | ---: | ---: |
| Directions Served | L | R |
| Maximum Queue (ft) | 17 | 23 |
| Average Queue (ft) | 4 | 5 |
| 95th Queue (ft) | 19 | 22 |
| Link Distance (ft) | 988 | 988 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 2: Waynoka PI \& Middle Access, All Intervals

| Movement | EB | EB |
| :--- | ---: | ---: |
| Directions Served | L | R |
| Maximum Queue (ft) | 29 | 30 |
| Average Queue (ft) | 6 | 3 |
| 95th Queue (ft) | 25 | 19 |
| Link Distance (ft) | 988 | 988 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 3: Waynoka PI \& North Access, Interval \#1

| Movement | NB |
| :--- | ---: |
| Directions Served | L |
| Maximum Queue (ft) | 12 |
| Average Queue (ft) | 2 |
| 95th Queue (ft) | 14 |
| Link Distance (ft) |  |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) | 100 |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

Intersection: 3: Waynoka PI \& North Access, Interval \#2

| Movement |
| :--- |
| Directions Served |
| Maximum Queue (ft) |
| Average Queue (ft) |
| 95th Queue (ft) |
| Link Distance (ft) |
| Upstream Blk Time (\%) |
| Queuing Penalty (veh) |
| Storage Bay Dist (ft) |
| Storage Blk Time (\%) |
| Queuing Penalty (veh) |

## Intersection: 3: Waynoka PI \& North Access, Interval \#3

| Movement | NB |
| :--- | ---: |
| Directions Served | L |
| Maximum Queue (ft) | 12 |
| Average Queue (ft) | 2 |
| 95th Queue (ft) | 14 |
| Link Distance (ft) |  |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) | 100 |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 3: Waynoka PI \& North Access, Interval \#4

Movement
Directions Served
Maximum Queue (ft)
Average Queue (ft)
95th Queue (ft)
Link Distance (ft)
Upstream Blk Time (\%)
Queuing Penalty (veh)
Storage Bay Dist (ft)
Storage Blk Time (\%)
Queuing Penalty (veh)

Intersection: 3: Waynoka PI \& North Access, All Intervals

| Movement | NB |
| :--- | ---: |
| Directions Served | L |
| Maximum Queue (ft) | 19 |
| Average Queue (ft) | 1 |
| 95th Queue (ft) | 9 |
| Link Distance (ft) |  |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) | 100 |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 4: Waynoka PI \& South Access, Interval \#1

| Movement | EB |
| :--- | ---: |
| Directions Served | LR |
| Maximum Queue (ft) | 11 |
| Average Queue (ft) | 2 |
| 95th Queue (ft) | 12 |
| Link Distance (ft) | 250 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

Intersection: 4: Waynoka PI \& South Access, Interval \#2

| Movement | EB |
| :--- | ---: |
| Directions Served | LR |
| Maximum Queue (ft) | 23 |
| Average Queue (ft) | 3 |
| 95th Queue (ft) | 18 |
| Link Distance (ft) | 250 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

Intersection: 4: Waynoka PI \& South Access, Interval \#3

| Movement | EB |
| :--- | ---: |
| Directions Served | LR |
| Maximum Queue (ft) | 17 |
| Average Queue (ft) | 2 |
| 95th Queue (ft) | 15 |
| Link Distance (ft) | 250 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

## Intersection: 4: Waynoka PI \& South Access, Interval \#4

| Movement | EB |  |
| :--- | ---: | :--- |
| Directions Served | LR |  |
| Maximum Queue (ft) | 11 |  |
| Average Queue (ft) | 3 |  |
| 95th Queue (ft) | 18 |  |
| Link Distance (ft) | 250 |  |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 4: Waynoka PI \& South Access, All Intervals

| Movement | EB |
| :--- | ---: |
| Directions Served | LR |
| Maximum Queue (ft) | 23 |
| Average Queue (ft) | 3 |
| 95th Queue (ft) | 16 |
| Link Distance (ft) | 250 |
| Upstream Blk Time (\%) |  |
| Queuing Penalty (veh) |  |
| Storage Bay Dist (ft) |  |
| Storage Blk Time (\%) |  |
| Queuing Penalty (veh) |  |

Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#1

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LR |
| Maximum Queue (ft) | 30 | 9 | 58 |
| Average Queue (ft) | 4 | 1 | 37 |
| 95th Queue (ft) | 22 | 10 | 56 |
| Link Distance (ft) | 632 | 316 | 114 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

## Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#2

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | LT | LR |
| Maximum Queue (ft) | 25 | 63 |
| Average Queue (ft) | 5 | 35 |
| 95th Queue (ft) | 24 | 60 |
| Link Distance (ft) | 632 | 114 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

## Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#3

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LR |
| Maximum Queue (ft) | 31 | 14 | 65 |
| Average Queue (ft) | 13 | 3 | 42 |
| 95th Queue (ft) | 37 | 24 | 68 |
| Link Distance (ft) | 632 | 316 | 114 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 5: Waynoka Rd \& Waynoka PI, Interval \#4

| Movement | EB | SB |
| :--- | ---: | ---: |
| Directions Served | LT | LR |
| Maximum Queue (ft) | 18 | 53 |
| Average Queue (ft) | 2 | 34 |
| 95th Queue (ft) | 14 | 54 |
| Link Distance (ft) | 632 | 114 |
| Upstream Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |
| Storage Bay Dist (ft) |  |  |
| Storage Blk Time (\%) |  |  |
| Queuing Penalty (veh) |  |  |

Intersection: 5: Waynoka Rd \& Waynoka PI, All Intervals

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LT | TR | LR |
| Maximum Queue (ft) | 31 | 23 | 76 |
| Average Queue (ft) | 6 | 1 | 37 |
| 95th Queue (ft) | 26 | 12 | 60 |
| Link Distance (ft) | 632 | 316 | 114 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 6: Palmer Park Blvd \& Waynoka Rd, Interval \#2

| Movement | EB | SB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | L | L | R |
| Maximum Queue (ft) | 48 | 76 | 62 |
| Average Queue (ft) | 33 | 39 | 38 |
| 95th Queue (ft) | 57 | 82 | 67 |
| Link Distance (ft) |  | 403 | 403 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) | 100 |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |

Intersection: 6: Palmer Park Blvd \& Waynoka Rd, Interval \#3

| Movement | EB | WB | SB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | L | T | L | R |
| Maximum Queue (ft) | 54 | 4 | 105 | 65 |
| Average Queue (ft) | 37 | 1 | 64 | 44 |
| 95th Queue (ft) | 59 | 7 | 114 | 73 |
| Link Distance (ft) |  | 361 | 403 | 403 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) | 100 |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |

Intersection: 6: Palmer Park Blvd \& Waynoka Rd, Interval \#4

| Movement | EB | WB | SB | SB |
| :--- | ---: | ---: | ---: | ---: |
| Directions Served | L | T | L | R |
| Maximum Queue (ft) | 53 | 4 | 81 | 67 |
| Average Queue (ft) | 30 | 1 | 48 | 40 |
| 95th Queue (ft) | 55 | 6 | 94 | 63 |
| Link Distance (ft) |  | 361 | 403 | 403 |
| Upstream Blk Time (\%) |  |  |  |  |
| Queuing Penalty (veh) |  |  |  |  |
| Storage Bay Dist (ft) | 100 |  |  |  |
| Storage Blk Time (\%) |  |  |  |  |

## MTCP Maps

## Map 13: Improvements Map




Map 14: 2040 Roadway Plan (Classification and Lanes)


## El PASOCOUITTY 2016 MajorTransportation Corridors Plan Update

## Roadway Plan

A total of 67 different projects have been identified as being needed by the year 2040. These projects are shown on Map 13 and listed on Table 4, with each project numbered within each improvement category. Capacity improvement projects are concentrated in the developing urban/suburban western part of the County, while paving projects are spread throughout the County with several in the eastern part.

Map 14 shows the 2040 Roadway Plan that results from the implementation of the improvements described above. The map shows road laneage and classification envisioned in 2040 if all 67 projects are implemented.

Table 4: 2040 Roadway Improvement Projects

| $\begin{array}{\|c} \text { Project } \\ \text { ID } \end{array}$ | Road Segment | Segment |  | PPRTA <br> Project | Urban vs. Rural | Existing Conditions |  | Future Conditions |  | Total Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Beginning | End |  |  | Lanes | Functional Class | Lanes | Functional Class |  |
| Paving Improvement Projects |  |  |  |  |  |  |  |  |  |  |
| P1 | Black Forest Rd | Walker Rd | County Line Rd |  | Rural | 2 | Gravel Road | 2 | Unimproved County Rd | \$1,954,000 |
| P2 | Walker Rd | Black Forest Rd | Meridian Rd |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$4,899,000 |
| P3 | Sweet Rd | Peyton Hwy | Ellicott Hwy |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$6,431,000 |
| P4 | Harrisville Rd | Blasingame Rd | Ramah Hwy |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$1,601,000 |
| P5 | Funk Rd | Calhan Hwy | Ramah Hwy |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$7,913,000 |
| P6 | Eastonville Rd | Eastonville Loop | Londonderry Dr |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$1,284,000 |
| P7 | Blaney Rd S | Meridian Rd | Hoofbeat Rd |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$1,125,000 |
| P8 | Drennan Rd | Curtis Rd | Ellicott Hwy |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$7,148,000 |

: El PASOCOU|IT| 2016 Major Transportation Corridors Plan Update

Table 4: 2040 Roadway Improvement Projects

| Project ID | Road Segment | Segment |  | PPRTA <br> Project | Urban vs. Rural | Existing Conditions |  | Future Conditions |  | Total Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Beginning | End |  |  | Lanes | Functional Class | Lanes | Functional Class |  |
| P9 | Sanborn Rd | Ellicott Hwy | Baggett Rd |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$1,566,000 |
| P10 | Log Rd | 90-degree bend | SH 94 |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$1,550,000 |
| P11 | Latigo Blvd | Eastonville Rd | Elbert Rd |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$1,297,000 |
| P12 | Hoofbeat | Blaney Rd S | SH 94 |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$2,756,000 |
| P13 | Soap Weed Rd | South of US 24 | Beg. of Paved section |  | Rural | 2 | Gravel Road | 2 | Unimproved County Road | \$2,495,000 |
|  |  |  |  |  |  |  |  |  | ving Projects Total | \$42,019,000 |
|  |  |  |  | Resurfa | ing Proj |  |  |  |  |  |
| R1 | Boone Rd | Fossinger Rd | Myers Rd |  | Rural | 2 | Unimproved County Road | 2 | Unimproved County Road | \$11,647,000 |
| R2 | Sweet Rd | Elbert Rd | Peyton Hwy |  | Rural | 2 | Unimproved County Road | 2 | Unimproved County Road | \$1,633,000 |
| R3 | Murphy Rd | Eastonville Rd | Bradshaw Rd |  | Rural | 2 | Unimproved County Road | 2 | Unimproved County Road | \$1,622,000 |
| R4 | Chamberlin South | B St | End of street | B | Rural | 2 | Unimproved County Road | 2 | Unimproved County Road | \$112,000 |
| R5 | Fountain Mesa Rd | Caballero Ave | Fontaine Blvd | B | Rural | 2 | Unimproved County Road | 2 | Unimproved County Road | \$355,000 |
| Resurfacing Projects Total Cost |  |  |  |  |  |  |  |  |  | \$15,369,000 |

## El PASOCOUITY <br> 2016 MajorTransportation Corridors Plan Update

Table 4: 2040 Roadway Improvement Projects

| Project ID | Road Segment | Segment |  | PPRTA <br> Project | Urban vs. Rural | Existing Conditions |  | Future Conditions |  | Total Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Beginning | End |  |  | Lanes | Functional Class | Lanes | Functional Class |  |
| County Road Upgrades |  |  |  |  |  |  |  |  |  |  |
| U1 | Curtis Rd | Judge Orr Rd. | SH 94 |  | Rural | 2 | Unimproved County Road | 2 | Principal Arterial | \$35,549,000 |
| U2 | Curtis Rd | SH 94 | Drennan Rd |  | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$23,379,000 |
| U3 | Bradley Rd | COS City Limit | Curtis Rd |  | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$24,252,000 |
| U4 | Old Pueblo Rd | Fountain City Limits | 1-25 | B | Rural | 2 | Unimproved County Road | 2 | Collector | \$16,722,000 |
| U5 | Falcon Hwy | US 24 | 1 mi east of Curtis Rd |  | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$16,509,000 |
| U6 | Hodgen Rd | Goshawk Rd | Meridian Rd. | B | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$7,698,000 |
| U7 | Baptist Rd | Desiree Dr | Roller Coaster Rd |  | Rural | 2 | Unimproved County Road | 2 | Collector | \$5,286,000 |
| U8 | Hodgen Rd | Black Forest Rd | Bar X Rd | B | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$5,053,000 |
| U9 | Hodgen Rd | Roller Coaster Rd | SH 83 |  | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$3,518,000 |
| U10 | Meridian Rd | Hodgen Rd | Murphy Rd | B | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$7,763,000 |
| U11 | Black Forest Rd | Hodgen Rd | Stapleton Dr | B | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$22,714,000 |
| U12 | Vollmer Rd | Stapleton Dr | Shoup Rd | B | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$11,691,000 |

Table 4: 2040 Roadway Improvement Projects

| Project <br> ID | Road Segment | Segment |  | PPRTA Project | Urban vs. Rural | Existing Conditions |  | Future Conditions |  | Total Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Beginning | End |  |  | Lanes | Functional Class | Lanes | Functional Class |  |
| U13 | Shoup Rd | SH 83 | Black Forest Rd |  | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$15,019,000 |
| U14 | Milam Rd | Shoup Rd | Old Ranch Rd |  | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$9,447,000 |
| U15 | Walker Rd | Steppler Rd | Black Forest Rd |  | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$6,783,000 |
| U16 | Roller Coaster Rd | Hodgen Rd | Old Northgate Rd |  | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$11,697,000 |
| U17 | Higby Rd | Cloverleaf Rd | Roller Coaster Rd |  | Urban | 2 | Unimproved County Road | 2 | Minor Arterial | \$6,514,000 |
| U18 | Beacon Lite Rd | SH 105 | County Line Rd | A | Rural | 2 | Unimproved County Road | 2 | Collector | \$5,321,000 |
| U19 | Eastonville Rd | Mclaughlin Rd | Latigo Blvd | A | Rural | 2 | Unimproved County Road | 2 | Minor Arterial | \$18,420,000 |
| U20 | Monument Hill | Woodmoor Dr | County Line Rd | A | Rural | 2 | Unimproved County Road | 2 | Collector | \$5,224,000 |
| U21 | Deer Creek Rd | Monument Hill | Woodmen Dr | A | Rural | 2 | Unimproved County Road | 2 | Collector | \$879,000 |
| County Road Upgrade Projects Total Costs |  |  |  |  |  |  |  |  |  | \$259,437.000 |

## EL PASOCOUITIY 2016 Major Transportation Corridors Plan Update

Table 4: 2040 Roadway Improvement Projects

| Project <br> ID | Road Segment | Segment |  | PPRTA Project | Urban vs. Rural | Existing Conditions |  | Future Conditions |  | Total Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Beginning | End |  |  | Lanes | Functional Class | Lanes | Functional Class |  |
| State Highway Improvements |  |  |  |  |  |  |  |  |  |  |
| SH1 | SH94 | City Limits | Slocum Rd |  | Rural | 2 | Principal Arterial | 4 | Principal Arterial | \$31,129,000 |
| SH2 | US 83 | Shoup Rd | Northgate Rd |  | Rural | 4 | Principal Arterial | 6 | Principal Arterial | \$5,953,000 |
| SH3 | US 24 West | 31st St | Manitou Interchange |  | Urban | 4 | Principal Arterial | 4 | Freeway | \$9,045,000 |
| SH4 | US 24 | Marksheffel Rd | Constitution |  | Urban | 4 | Principal Arterial | 6 | Expressway | \$4,591,000 |
| SH5 | US 24 | Garratt Rd | Woodmen Rd |  | Rural | 4 | Principal Arterial | 6 | Principal Arterial | \$7,995,000 |
| SH6 | US 83 | Northgate | Hodgen Rd |  | Rural | 2 | Principal Arterial | 4 | Principal Arterial | \$10,742,000 |
| State Highway Capacity Projects Total Costs |  |  |  |  |  |  |  |  |  | \$69,455,000 |

Table 4: 2040 Roadway Improvement Projects

| Project ID | Road Segment | Segment |  | PPRTA Project | Urban vs. Rural | Existing Conditions |  | Future Conditions |  | Total Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Beginning | End |  |  | Lanes | Functional Class | Lanes | Functional Class |  |
| County Road Capacity Improvements |  |  |  |  |  |  |  |  |  |  |
| C1 | Enoch Rd | SH 94 | Schriever |  | Rural | 2 | Collector | 4 | Principal Arterial | \$8,208,000 |
| C2 | Marksheffel Rd | Stetson Hills | 2000 ft north |  | Urban | 2 | Principal Arterial | 4 | Principal Arterial | \$3,526,000 |
| C3 | Marksheffel Rd | Barnes Rd | Carefree Cir. N |  | Urban | 2 | Principal Arterial | 4 | Principal Arterial | \$8,864,000 |
| C4 | Marksheffel Rd | 0.5 mi . north of Fontaine | Link Rd |  | Rural | 2 | Minor Arterial | 4 | Expressway | \$20,816,000 |
| C5 | Fontaine | Marksheffel Rd | Easy St |  | Urban | 2 | Minor Arterial | 4 | Minor Arterial | \$42,449,000 |
| C6 | Bradley Rd | Academy Blvd | Hancock Expy |  | Urban | 2 | Principal Arterial | 4 | Principal Arterial | \$18,301,000 |
| C7 | Academy Blvd | I-25 | Bradley Rd | A | Urban | 4 | Expressway | 6 | Expressway | \$22,733,000 |
| C8 | Woodmen Rd | Marksheffel Rd | Banning Lewis |  | Urban | 4 | Principal Arterial | 6 | Expressway | \$19,316,000 |
| C9 | Walker Rd | SH 83 | Steppler Rd |  | Rural | 2 | Collector | 4 | Minor Arterial | \$15,126,000 |
| C10 | Meridian Rd | Murphy Rd | Rex Rd | B | Rural | 2 | Collector | 4 | Minor Arterial | \$21,081,000 |
| C11 | Black Forest Rd | Stapleton Dr | 1300 ft south of Silver Pond Heights | B | Urban | 2 | Minor Arterial | 4 | Minor Arterial | \$7,507,000 |
| C12 | Stapleton Dr | Towner | Judge Orr Rd. | B | Urban | 2 | Principal Arterial | 4 | Principal Arterial | \$41,076,000 |
| C13 | Vollmer Rd | Marksheffel Rd | Stapleton Dr |  | Rural | 2 | Collector | 4 | Minor Arterial | \$9,599,000 |
| C14 | Judge Orr Rd | Eastonville Rd | Peyton Hwy |  | Rural | 2 | Minor Arterial | 4 | Minor Arterial | \$38,248,000 |
| C15 | Hwy 105 | Knollwood Blvd | SH 83 |  | Rural | 2 | Principal Arterial | 4 | Principal Arterial | \$28,297,000 |
| C16 | Grinnell St | Powers Blvd | Bradley Rd | B | Rural | 2 | Minor Arterial | 4 | Minor Arterial | \$3,807,000 |
| County Road Capacity Projects Total Costs |  |  |  |  |  |  |  |  |  | \$319,856,000 |

Table 4: 2040 Roadway Improvement Projects

| Project ID | Road Segment | Segment |  | PPRTA <br> Project | Urban vs. Rural | Existing Conditions |  | Future Conditions |  | Total Cost |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Beginning | End |  |  | Lanes | Functional Class | Lanes | Functional Class |  |
| New Road Connections |  |  |  |  |  |  |  |  |  |  |
| N1 | Roller Coaster Rd | Eliminate jog in alignment |  |  | Rural |  |  | 2 | Minor Arterial | \$4,118,000 |
| N2 | Black Forest Rd | Eliminate jog in alignment |  |  | Rural |  |  | 2 | Minor Arterial | \$2,585,000 |
| N3 | Hodgen Rd | Eastonville Rd | Elbert Rd |  | Rural |  |  | 2 | Collector | \$4,470,000 |
| N4 | Rex Rd | Rex Rd | Eastonville Rd |  | Urban |  |  | 2 | Collector | \$6,359,000 |
| N5 | Stapleton Dr | Towner Rd | Black Forest Rd |  | Urban |  |  | 4 | Principal Arterial | \$55,771,000 |
| N6 | Woodmen Hills Rd | Stapleton Dr | Raygor Rd |  | Urban |  |  | 2 | Collector | \$12,296,000 |
| N7 | Peyton Hwy | Judge Orr Rd | Peyton Hwy |  | Rural |  |  | 2 | Collector | \$8,365,000 |
| N8 | Howell Lane | Bridge over Kettle Creek |  |  | Rural |  |  | 2 | Collector | \$8,130,000 |
| N9 | Meridian Rd | Bradley Rd | Mesa Ridge Pkwy |  | Rural |  |  | 2 | Minor Arterial | \$11,312,000 |
| N10 | Mesa Ridge Pkwy | Marksheffel Rd | Meridian Rd |  | Rural |  |  | 2 | Minor Arterial | \$5,216,000 |
| N11 | Fontaine Blvd | Fontaine Blvd | Meridian Rd |  | Urban |  |  | 4 | Principal Arterial | \$11,217,000 |
| N12 | Marksheffel Rd | Woodmen Rd | Research Pkwy |  | Urban |  |  | 4 | Principal Arterial | \$40,262,000 |
| N13 | Banning Lewis | Woodmen Rd | Stapleton |  | Urban |  |  | 4 | Principal Arterial | \$11,131,000 |
| N14 | Mesa Ridge Pkwy | Powers Blvd | Marksheffel Rd | A | Rural |  |  | 4 | Principal Arterial | \$14,170,000 |
| N15 | Tutt Blvd Extension | Dublin Blvd | Templeton Gap | A | Urban |  |  | 4 | Principal Arterial | \$4,506,000 |
| N16 | Furrow Rd Ext | Lamplighter Dr | Higby Rd |  | Urban |  |  | 2 | Collector | \$1,078,000 |
| N17 | Bradley Rd | Grinnell St. | Powers Blvd | B | Urban |  |  | 2 | Minor Arterial | \$10,335,000 |
| New Road Connections Total Project Costs |  |  |  |  |  |  |  |  |  | \$208,915,000 |
| Total Project Cost of County Improvements |  |  |  |  |  |  |  |  |  | \$845,596,000 |
| Total Cost for PPRTA A List Projects |  |  |  |  |  |  |  |  |  | \$68,847,000 |
| Total State Highway Improvements Cost |  |  |  |  |  |  |  |  |  | \$69,455,000 |
| Total Cost of All Projects |  |  |  |  |  |  |  |  |  | \$915,051,000 |

## El PASOCOUITII 2016 MajorTransportation Corridors Plan Update

Table 5: 2040 Multi-modal Improvement Projects

| Project ID | Road Name | Improvement Type | Beginning (South, West) | End (North, East) | Length |
| :---: | :---: | :---: | :---: | :---: | :---: |
| M1 | S. Peyton Highway | Bicycle | Squirrel Creek Road | Falcon Highway | 15.93 |
| M2 | S. Ellicott Highway | Bicycle \& Primary Regional Trail | Squirrel Creek Road | Farmer Road | 1.93 |
| M3 | Squirrel Creek Road | Bicycle \& Primary Regional Trail | Shumway Road | S. Ellicott Highway | 14.06 |
| M4 | Falcon Highway | Bicycle \& Secondary Regional Trail | Meridian Road | S. Peyton Highway | 6.95 |
| M5 | Meridian Road | Bicycle | Blaney Road | Falcon Highway | 2.98 |
| M6 | Peyton Highway | Bicycle | Falcon Highway | US 24 | 7.00 |
| M7 | Elbert Road | Bicycle | Judge Orr Road | US 24 | 2.32 |
| M8 | Judge Orr Road | Bicycle | Eastonville Road | Peyton Highway | 2.98 |
| M9 | Stapleton Dr | Bicycle | Meridian Road | US 24 | 2.56 |
| M10 | Hodgen Road | Bicycle | Meridian Road | Eastonville Road | 1.67 |
| M11 | Vollmer Road | Bicycle \& Primary Regional Trail | Marksheffel Road | Shoup Road | 4.51 |
| M12 | Hodgen Road | Bicycle \& Primary Regional Trail | Highway 105 | US 83 | 4.07 |
| M13 | US 24 | Primary Regional Trail | Manitou | Cascade | 3.44 |
| M14 | Shoup Road | Bicycle | US 83 | Vollmer Road | 6.24 |



