Final Drainage Report Rock Island Trail Sand Creek to Constitution Capital Project Colorado Springs, Colorado

Prepared for: City of Colorado Springs Parks, Recreation and Cultural Services Department 1401 Recreation Way Colorado Springs, CO 80905 (719) 385-6951



1604 South 21st Street Colorado Springs, Colorado 80904 Ph: (719)630-7342

Kiowa Project No. 16028 SWENT File: STM-REV24-0295 EPC Project Number CDR193

August 7, 2024

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Drainage Conditions Map

ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Kiowa Engineering Corporation, 1604 South 21st Street, Colorado Springs, Colorado 80904



CITY PROJECT MANAGER'S STATEMENT:

I hereby certify that the drainage for Rock Island Trail shall be constructed according to the design presented in this report. I further understand that field changes must be reviewed by the City Review Engineer to ensure conformance with the original design intent. I am employed by and perform engineering services solely for the City of Colorado Springs, and therefore am exempt from Colorado Revised Statute Title 12, Article 25, Part 1 according to § 12-25-103(1), C.R.S.

Name of City Project Manager: <u>Emily Duncan</u>

Signature: ____

_Date: _____

EL PASO COUNTY STATEMENT:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 & 2, El Paso County Engineering Criteria Manual, and Land Development Code, as amended.

Joshua Palmer, P.E. El Paso County Engineer/ECM Administrator Date

Conditions:

I. GENERAL LOCATION AND DESCRIPTION

The purpose of this Drainage Letter is to identify on-site and off-site drainage patterns, storm sewers, culvert and inlet locations, areas tributary to the site, and to safely route developed storm water to adequate outfalls for Rock Island Trail.

Rock Island Trail is comprised of 36.2 acres, located in southeast Colorado Springs, Colorado between Sand Creek and Constitution Ave. The property is bordered multiple commercial and residential lots and city property.

The property is located in Sections 5 and 6, Township 14, Range 65 of the 6th Principal Meridian, in Colorado Springs, El Paso County, Colorado. The vegetation in the site consists of native grasses. A vicinity map showing the general location of the site is presented in Appendix A.

The property is primarily the abandoned Rock Island Railroad alignment. And is now an almost 2 mile long narrow strip of land that is city owned open space consisting of approximately 36.8 acres. The total disturbed area associated with this project is approximately 18.2 acres. There is no proposed development within any streamside buffer zone or in any designated floodplain, as indicated on FEMA panel 08041C0752G. A FEMA firmette for the site is located in Appendix A.

II. GENERAL CONCEPT

A. EXISTING DRAINAGE PATTERNS

In the existing condition, the site generally drains from north to the south and from east to west. The site sheet flows south offsite onto the developed properties to the south. Sand Creek is at the west end of the project which intercepts flows and conveys them West to Fountain Creek.

Because of the long and narrow shape of the project most of the storm runoff leaves the property as sheet flow onto the numerous properties adjacent to the property. There are not points on the property where flows leave in a concentrated manner. The following is a description of the existing drainage sub-basins.

Sub-basin E-1: Sub-basin E-1 is 0.87 acres, with 5 and 100-year runoff of 0.3 and 1.9 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the south side of the site and consists of open space and trails. The runoff from this sub-basin flows south across the basin as sheet flow and does not concentrate. The design point is depicted as Design Point E1. The flow ultimately gets to Sand Creek through numerous paths.

<u>Sub-basin E-2</u>: Sub-basin E-2 is 7.99 acres, with 5 and 100-year runoff of 3.3 and 22 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the north side of the site and consists of open space and trails. The runoff from this sub-basin flows north across the basin as sheet flow and does not concentrate. The design point is depicted as Design Point E2. The flow enters a concrete trapezoidal ditch that borders the project to the north. This ditch discharges directly to Sand Creek and the west end of the project.

Sub-basin E-3: Sub-basin E-3 is 10.64 acres, with 5 and 100-year runoff of 4.1 and 27 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the south side of the site and consists of open space and trails. The runoff from this sub-basin flows south across the basin as sheet flow and does not concentrate. The design point is depicted as Design Point E3. The flow ultimately gets to Sand Creek through numerous paths.

Sub-basin E-4: Sub-basin E-4 is 6.13 acres, with 5 and 100-year runoff of 2.5 and 17 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the north side of the site and consists of open space and trails. The runoff from this sub-basin flows north across the basin as sheet flow and does not concentrate. The design point is depicted as Design Point E4. The flow ultimately gets to Sand Creek through numerous paths.

Sub-basin E-5: Sub-basin E-5 is 9.69 acres, with 5 and 100-year runoff of 4.0 and 27 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the south side of the site and consists of open space and trails. The runoff from this sub-basin flows south across the basin as sheet flow and does not concentrate. The design point is depicted as Design Point E5. The flow ultimately gets to Sand Creek through numerous paths.

Sub-basin E-6: Sub-basin E-6 is 0.65 acres, with 5 and 100-year runoff of 0.3 and 1.7 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the south side of the site and consists of open space and trails. The runoff from this sub-basin flows south across the basin as sheet flow and does not concentrate. The design point is depicted as Design Point E6. The flow discharges into basin E-5.

Sub-basin E-7: Sub-basin E-7 is 0.67 acres, with 5 and 100-year runoff of 0.3 and 2.1 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the west end of the site and consists of open space and trails. The runoff from this sub-basin flows west into Sand Creek and does not concentrate. The design point is depicted as Design Point E7.

Sub-basin E-8: Sub-basin E-8 is 0.17 acres, with 5 and 100-year runoff of 0.5 and 1.0 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the west end of the site and consists of sidewalk and open space. The runoff from this sub-basin flows north into Constitution Ave as concentrated flow. The design point is depicted as Design Point E8. The flow ultimately gets to Sand Creek using Constitution Ave Curb and Gutter.

B. PROPOSED DRAINAGE PATTERNS

Similar to the existing conditions, the proposed drainage will generally travel to the west into Sand Creek, then ultimately flow into the Fountain creek drainage basin.

The runoff in the developed condition will be the same as the existing condition. The basin areas do not change as a result of this project, the times of concentration do not change as a result of this project. The C values do not change as a result of this project. The site remains "undeveloped historic greenbelts"

Sub-basin P-1: Sub-basin P-1 is 0.87 acres, with 5 and 100-year runoff of 0.3 and 1.9 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the south side of the site and consists of open space and trails. The runoff from this sub-basin flows south across the basin as sheet flow and does not concentrate. The design point is depicted as Design Point P1. The flow ultimately gets to Sand Creek through numerous paths.

<u>Sub-basin P-2</u>: Sub-basin P-2 is 7.99 acres, with 5 and 100-year runoff of 3.3 and 22 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the north side of the site and consists of open space and trails. The runoff from this sub-basin flows north across the basin as sheet flow and does not concentrate. The design point is depicted as Design Point E2. The flow enters a concrete trapezoidal ditch that borders the project to the north. This ditch discharges directly to Sand Creek and the west end of the project.

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Sub-basin P-4: Sub-basin P-4 is 6.13 acres, with 5 and 100-year runoff of 2.5 and 17 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the north side of the site and consists of open space and trails. The runoff from this sub-basin flows north across the basin as sheet flow and does not concentrate. The design point is depicted as Design Point P4. The flow ultimately gets to Sand Creek through numerous paths.

Sub-basin P-5: Sub-basin P-5 is 9.69 acres, with 5 and 100-year runoff of 4.0 and 27 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the south side of the site and consists of open space and trails. The runoff from this sub-basin flows south across the basin as sheet flow and does not concentrate. The design point is depicted as Design Point P5. The flow ultimately gets to Sand Creek through numerous paths.

Sub-basin P-6: Sub-basin P-6 is 0.65 acres, with 5 and 100-year runoff of 0.3 and 1.7 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the south side of the site and consists of open space and trails. The runoff from this sub-basin flows south across the basin as sheet flow and concentrates at design point 6. The flow discharges into basin P-5 thru a proposed 18" culvert. (This culvert is the only pipe conveyance in the project.) There will be riop rap at eh culvert outlet to dissipate the flows. In basin 5 these flows will travel overland over flat not steep (2%) terrain to infiltrate in a pervious (RPA) area.

Sub-basin P-7: Sub-basin P-7 is 0.67 acres, with 5 and 100-year runoff of 0.3 and 2.1 CFS respectively. It is not expected to receive any offsite flow. The sub-basin includes a portion of the west end of the site and consists of open space and trails. The runoff from this sub-basin flows west into Sand Creek and does not concentrate. The design point is depicted as Design Point P7.

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III. DRAINAGE DESIGN CRITERIA

This report followed the criteria and format included in "Colorado Springs Drainage Criteria Manual (DCM) Volume 1", "Volume 2" and "Colorado Springs Engineering Criteria Manual".

The report also followed the "Master Development Drainage Plan Drennan Subdivision Filing No. 1." And Amendment 1. The design of this site is in conformance with the MDDP.

Hydrologic and hydraulic calculations for the site were performed using the methods outlined in the *Colorado Springs Drainage Criteria Manual*. Topography for the site was compiled using a one-foot contour interval and is presented on the Drainage Plan.

The hydrologic calculations were made for the historic and developed site conditions. The Drainage Plan presents the drainage patterns for the site, including the sub-basins. The peak flow rates for the sub-basins were estimated using the Rational Method. The 5-year (Minor Storm) and 100-year (Major Storm) recurrence intervals were determined. The one-hour rainfall depth was determined

Review C1: Calculations of proposed inlet and rip rap are required. Review C2: Unresolved. Please include calculations of proposed, inlet, culvert, and riprap. 3 Kiowa Engineering Corporation from Table 6-2 of the Drainage Criteria Manual. These depths are shown in the runoff calculations spreadsheet.

This report and findings are in general conformance with the Sand Creek Drainage Basin Planning Study (DBPS) Final Report, dated January 2021, prepared by Stantec, HDR and DewBerry. There are no other known drainage reports for this site. This is primarily because the site is an old unplatted railroad corridor.

For both existing and proposed condition undeveloped greenbelt curve numbers were used. The site will remain a greenbelt with the trail improvements.

I. OFF-SITE RUNOFF CONSIDERATION

Most of the storm runoff leaves the site as sheet flows onto the adjacent properties. The amount of flow going onto any one property is negligible.

No significant off-site flows are expected to enter the site. No off-site flows will enter the site from the south due to the topography. No off-site flows will enter the site from the northwest of Peterson Blvd due to a concrete ditch separating the project site from all the properties north of the site. Some residential back yards will drain onto the site east of Peterson on the north side of the site. But this flow is negligible and unconcentrated flow.

II. HYDROLOGIC AND HYDRAULIC CALCULATIONS

Hydrologic and hydraulic calculations for the site were performed using the methods outlined in the *Colorado Springs Drainage Criteria Manual*. Topography for the site was compiled using a one-foot contour interval and is presented on the Drainage Plan.

The hydrologic calculations were made for the historic and developed site conditions. The Drainage Plan presents the drainage patterns for the site, including the sub-basins. The peak flow rates for the sub-basins were estimated using the Rational Method. The 5-year (Minor Storm) and 100-year (Major Storm) recurrence intervals were determined. The one-hour rainfall depth was determined from Table 6-2 of the *Drainage Criteria Manual*. These depths are shown in the runoff calculations spreadsheet.

Collection of the runoff will be accomplished through a combination of sheet flow, gutter flow, creek flow, and off-site storm flow.

The peak flow data generated using the rational method was used to verify no increase in cfs of the site due to proposed site development within the site.

III. SOILS CONSIDERATIONS

The onsite soils were considered to be Hydrologic Soil Group A, based on the *Soil Survey*. For existing conditions, runoff coefficients were determined using a land use of pasture/meadow. The land use for the proposed development will be Commercial.

A Grading and Erosion Control plan is required for this project since the area of disturbance 18.2 acre. A Grading and Erosion Control plan will be submitted to SWENT fand EPC or review and approval with the development of the construction drawings.

IV. WATER QUALITY METHODOLOGY (4-STEP PROCESS):

The Four Step Process is a method of mitigating the impact of new development on receiving waters by reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainageways, and implementing long-term source controls. The Four Step Process primarily focuses

on smaller, frequently occurring storm events, as opposed to larger storms for which the drainage facilities are sized.

Exclusion

This site is a trail "shown as Urban Trails in the Parks, Recreation, and Cultural Services Park System Master Plan." This project is claiming exclusion from disturbance area to reduce or eliminate the amount of disturbance area associated with a project for the purposes of applying the 4 Step Process and detention requirements. Per section 2.0 of the 4-step process in the DCM.

The Grading and Erosion Control Plan will be submitted to Stormwater Enterprise for review and approval prior to construction.

STEP 1: VOLUME REDUCTION

This project is a Roadway / trail specific projects not associated with new development. We are using Runoff Reduction tab of the MHFD-BMP workbook.

Using the UD-BMP runoff reduction spreadsheet we identified that the site would provide 5187 CF of WQCV thru its pervious areas.

The runoff reduction measures taken resulted in a 100% reduction in WQCV using MHFD UD-BMP Runoff Reduction spreadsheet. The calculations can be found in Appendix C.

STEP 2: VOLUME TREATMENT

With 100% reduction in step 1 and the trail exemption, step 2 is not necessary.

STEP 3: STABILIZE DRAINAGEWAYS

With 100% reduction in step 1 and the trail exemption, step 3 is not necessary.

STEP 4: SOURCE CONTROL

With 100% reduction in step 1 and the trail exemption, step 4 is not necessary.

IV. DRAINAGE BASIN FEES

The site is city property and will not be required to pay drainage fees.

V. SUMMARY

The site runoff proposed for Rock Island Trail will not increase and not adversely affect the downstream and surrounding developments. This report and findings are in general conformance with the Sand Creek Drainage Basin Planning Study (DBPS).

Runoff Summary

Design Point	Exi	sting	Proposed					
	5 yr Flow (cfs)	100-yr Flow (cfs)	5 yr Flow (cfs)	100 yr flow (cfs)				
E-1 / P-1	0.3	1.9	0.3	1.9				
E-2 / P-2	3.3	22	3.3	22				
E-3 / P-3	4.1	27	4.1	27				
E-4 / P-4	2.5	17	2.5	17				
E-5 / P-5	4.2	30	4.2	30				
E-6 / P-6	0.2	1.6	0.2	1.6				
E-7 / P-7	0.3	2.1	0.3	2.1				
E-8 / P-8	0.4	0.9	0.4	0.9				

The flows do not change from existing to proposed because the "greenbelt" curve numbers are used for both the existing and developed conditions.

A. AGENCY REQUIREMENTS

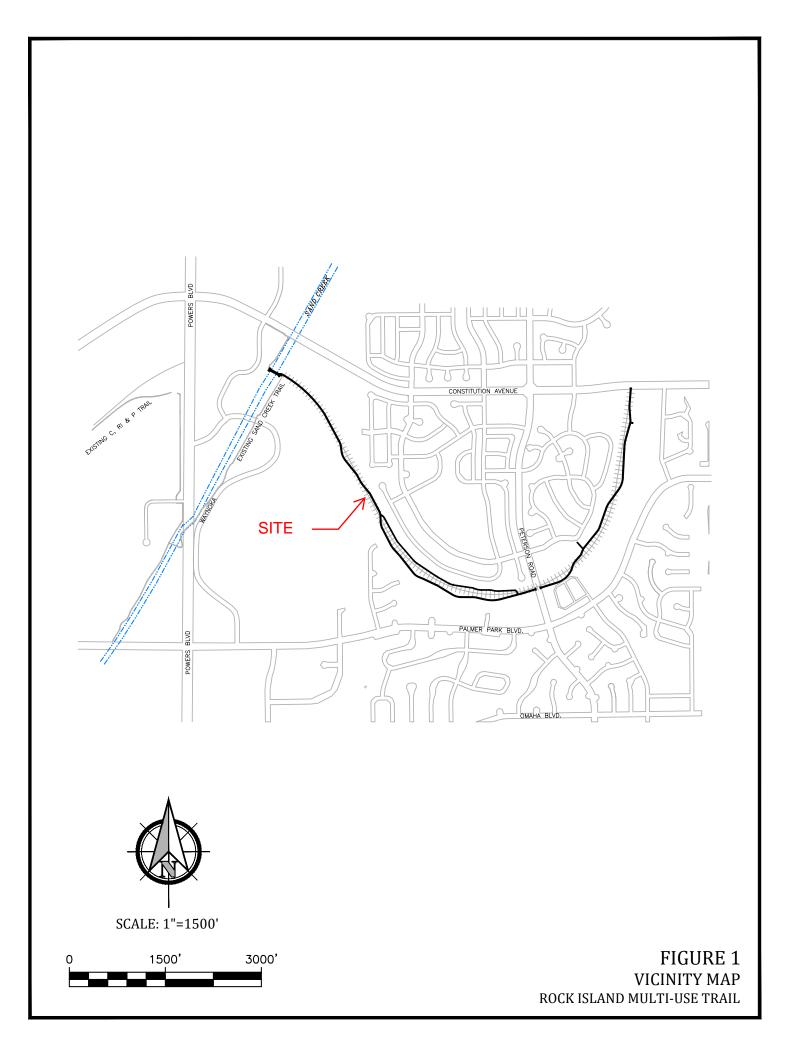
I. FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA)

The subject property limits are shown on Flood Insurance Rate Map (FIRM) 08041C0752G with effective dates of December 7, 2018 that are included in Appendix A. The FIRMs also show that the property to be developed is located outside of the FEMA regulated floodplain.

VI. REFERENCES

- 1) Sand Creek Drainage Basin Planning Study Final Report, dated January 2021, prepared by Stantec, HDR and DewBerry.
- 2) <u>Colorado Springs Drainage Criteria Manual Volume 1,</u> dated July 2014, Revised January 2021.
- 3) <u>Colorado Springs Drainage Criteria Manual Volume 2</u>, dated July 2014, revised December 2020.
- 4) <u>Colorado Springs Engineering Criteria Manual</u>, dated July 2019.
- 5) <u>National Flood Insurance Hazard layer FIRMette portion of panels 08041C0741G</u>, Federal Emergency Management Agency, both Effective Date 12/7/2018.

APPENDIX A Figure 1: Vicinity Map Figure 2: Soils Map Figure 3: FEMA Flood Insurance Rate Map



NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The horizontal datum was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988 (NAVD88). These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website a http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12

National Geodetic Survey SSMC-3, #9202

1315 East-West Highway Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, City of Fountain, Bureau of Land Management, National Oceanic and Atmospheric Administration, United States Geological Survey, and Anderson Consulting Engineers, Inc. These data are current as of 2006.

This map reflects more detailed and up-to-date stream channel configurations and floodplain delineations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channe distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile aselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact FEMA Map Service Center (MSC) via the FEMA Map Information eXchange (FMIX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website a http://www.msc.fema.gov/.

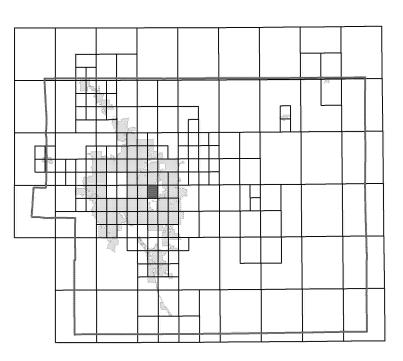
f you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip.

> El Paso County Vertical Datum Offset Table Vertical Datum

Flooding Source Offset (ft) REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY

FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION

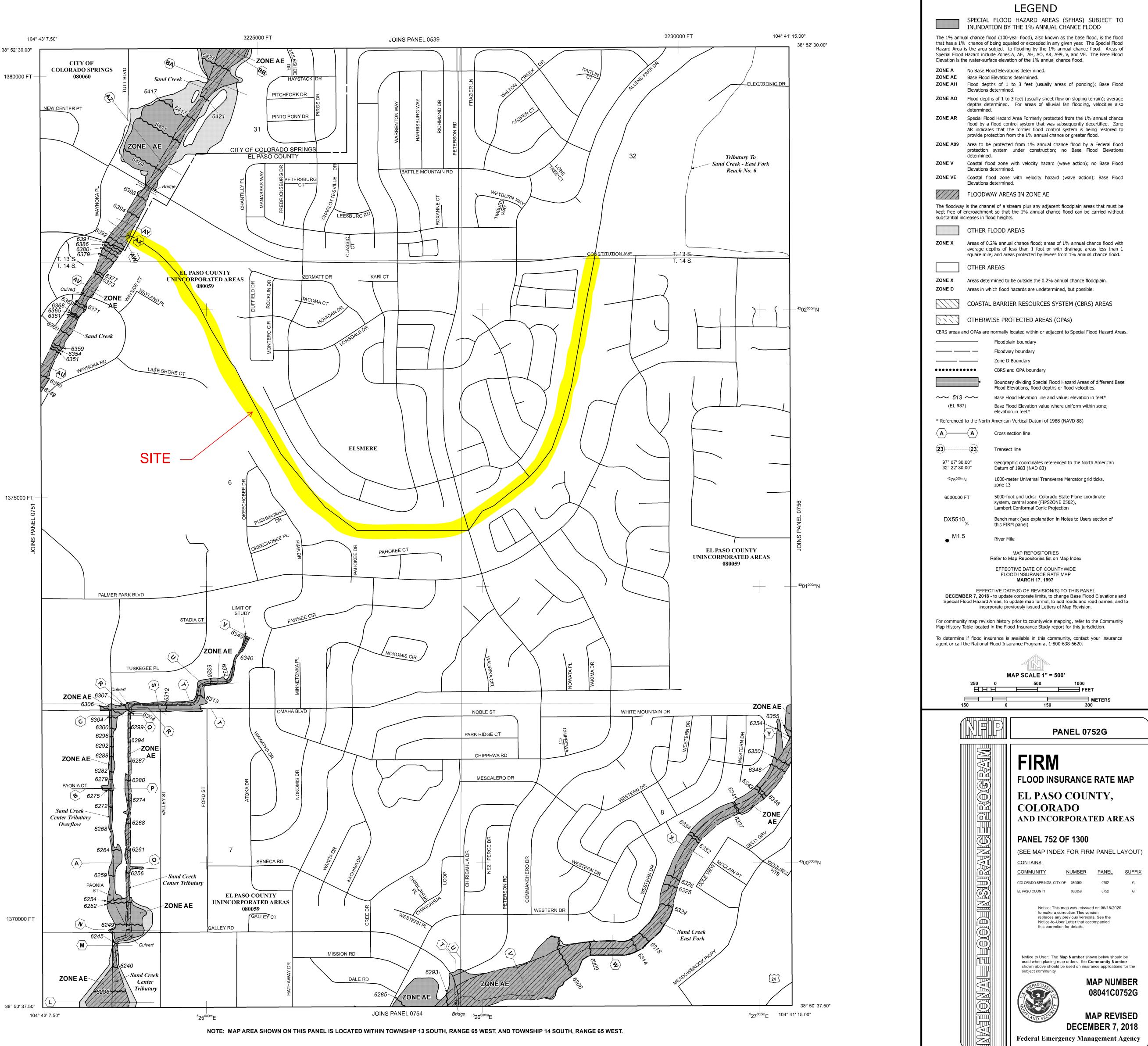
Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).



Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.





United States Department of Agriculture

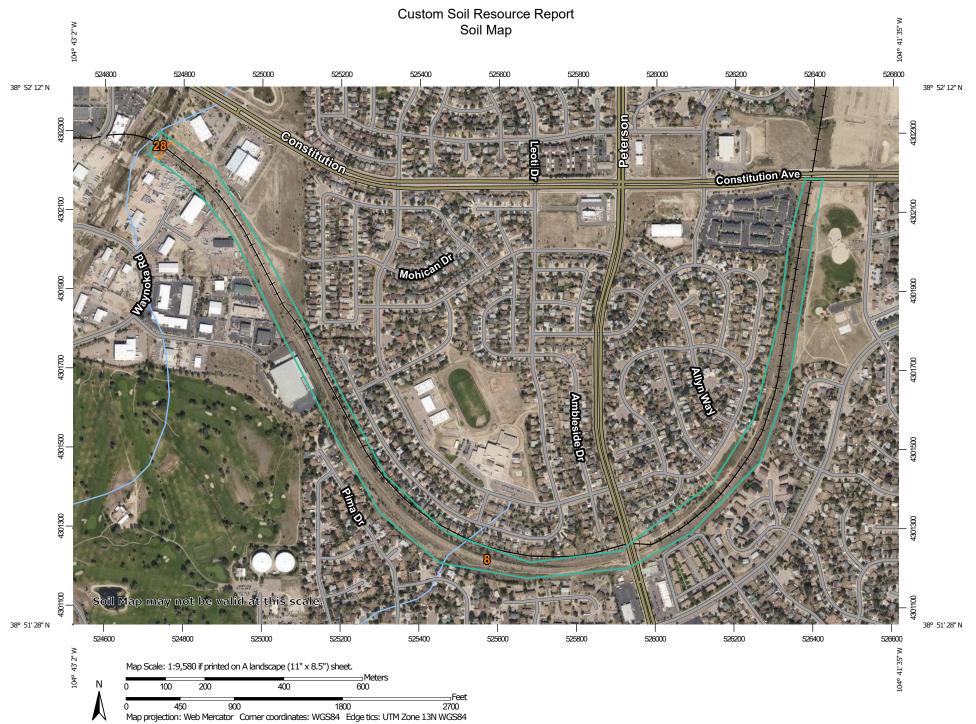
Natural Resources Conservation

Service

A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for El Paso County Area, Colorado





	MAP L	EGEND		MAP INFORMATION
	terest (AOI) Area of Interest (AOI)	8	Spoil Area Stony Spot	The soil surveys that comprise your AOI were mapped at 1:24,000.
Soils	Soil Map Unit Polygons Soil Map Unit Lines	Ø V	Very Stony Spot Wet Spot	Warning: Soil Map may not be valid at this scale.
	Soil Map Unit Points Point Features		Other Special Line Features	Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of
ල හ	Blowout Borrow Pit	Water Fea	tures Streams and Canals	contrasting soils that could have been shown at a more detailed scale.
×	Clay Spot		ation Rails	Please rely on the bar scale on each map sheet for map measurements.
×	Gravel Pit Gravelly Spot	~	Interstate Highways US Routes	Source of Map: Natural Resources Conservation Service Web Soil Survey URL: Coordinate System: Web Mercator (EPSG:3857)
 Θ Λ.	Landfill Lava Flow	~	Major Roads Local Roads	Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts
人 小 次	Marsh or swamp Mine or Quarry	Backgrou	Aerial Photography	distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.
0	Miscellaneous Water Perennial Water			This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.
~ +	Rock Outcrop Saline Spot			Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 21, Aug 24, 2023
··· •·	Sandy Spot Severely Eroded Spot			Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.
 ۵	Sinkhole Slide or Slip			Date(s) aerial images were photographed: Aug 19, 2018—Sep 23, 2018
ju K	Sodic Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

		[
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	41.7	98.8%
28	Ellicott loamy coarse sand, 0 to 5 percent slopes	0.5	1.2%
Totals for Area of Interest		42.2	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

El Paso County Area, Colorado

8-Blakeland loamy sand, 1 to 9 percent slopes

Map Unit Setting

National map unit symbol: 369v Elevation: 4,600 to 5,800 feet Mean annual precipitation: 14 to 16 inches Mean annual air temperature: 46 to 48 degrees F Frost-free period: 125 to 145 days Farmland classification: Not prime farmland

Map Unit Composition

Blakeland and similar soils: 98 percent Minor components: 2 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Blakeland

Setting

Landform: Hills, flats Landform position (three-dimensional): Side slope, talf Down-slope shape: Linear Across-slope shape: Linear Parent material: Alluvium derived from sedimentary rock and/or eolian deposits derived from sedimentary rock

Typical profile

A - 0 to 11 inches: loamy sand AC - 11 to 27 inches: loamy sand C - 27 to 60 inches: sand

Properties and qualities

Slope: 1 to 9 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Low
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 5 percent
Available water supply, 0 to 60 inches: Low (about 4.5 inches)

Interpretive groups

Land capability classification (irrigated): 3e Land capability classification (nonirrigated): 6e Hydrologic Soil Group: A Ecological site: R049XB210CO - Sandy Foothill Hydric soil rating: No

Minor Components

Other soils Percent of map unit: 1 percent

Hydric soil rating: No

Pleasant

Percent of map unit: 1 percent Landform: Depressions Hydric soil rating: Yes

28—Ellicott loamy coarse sand, 0 to 5 percent slopes

Map Unit Setting

National map unit symbol: 3680 Elevation: 5,500 to 6,500 feet Mean annual precipitation: 13 to 15 inches Mean annual air temperature: 47 to 50 degrees F Frost-free period: 125 to 145 days Farmland classification: Not prime farmland

Map Unit Composition

Ellicott and similar soils: 97 percent *Minor components:* 3 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Ellicott

Setting

Landform: Flood plains, stream terraces Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Parent material: Sandy alluvium

Typical profile

A - 0 to 4 inches: loamy coarse sand *C - 4 to 60 inches:* stratified coarse sand to sandy loam

Properties and qualities

Slope: 0 to 5 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Somewhat excessively drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: Frequent
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 4.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7w Hydrologic Soil Group: A *Ecological site:* R069XY031CO - Sandy Bottomland *Other vegetative classification:* SANDY BOTTOMLAND (069AY031CO) *Hydric soil rating:* No

Minor Components

Fluvaquentic haplaquoll

Percent of map unit: 1 percent Landform: Swales Hydric soil rating: Yes

Other soils

Percent of map unit: 1 percent Hydric soil rating: No

Pleasant

Percent of map unit: 1 percent Landform: Depressions Hydric soil rating: Yes

APPENDIX B Rational Calculations

Rock Island Trail

Final Drainage Report Area Runoff Coefficient Summary - EXISTING

			1	DEVELOPEI)	U	NDEVELOPE	WEIGHTED		
BASIN	BASIN TOTAL AREA		AREA	AREA C ₅		AREA	<i>C</i> ₅	C 100	<i>C</i> ₅	C 100
	(SF)	(Acres)	(Acres)			(Acres)				
E-1	38,091	0.87		0.53	0.68	0.87	0.09	0.36	0.09	0.36
E-2	348,229	7.99		0.53	0.68	7.99	0.09	0.36	0.09	0.36
E-3	463,313	10.64		0.53	0.68	10.64	0.09	0.36	0.09	0.36
E-4	266,935	6.13		0.53	0.68	6.13	0.09	0.36	0.09	0.36
E-5	422,019	9.69		0.53	0.68	9.69	0.09	0.36	0.09	0.36
E-6	28,408	0.65		0.53	0.68	0.65	0.09	0.36	0.09	0.36
E-7	29,092	0.67		0.53	0.68	0.67	0.09	0.36	0.09	0.36
E-8	7,374	0.17	0.17	0.53	0.68	0.00	0.09	0.36	0.53	0.68
	1,603,461	36.8								

Calculated by: CKC Date: 5/15/2024 Checked by: TC

Rock Island Trail

Final Drainage Report Area Runoff Coefficient Summary - PROPOSED

			1	DEVELOPEL)	U	NDEVELOPE	WEIGHTED		
BASIN	TOTAL	AREA	AREA	C 5	C 100	AREA	<i>C</i> ₅	C 100	<i>C</i> 5	C 100
	(SF)	(Acres)	(Acres)			(Acres)				
P-1	38,091	0.87		0.53	0.68	0.87	0.09	0.36	0.09	0.36
P-2	348,229	7.99		0.53	0.68	7.99	0.09	0.36	0.09	0.36
P-3	463,313	10.64		0.53	0.68	10.64	0.09	0.36	0.09	0.36
P-4	266,935	6.13		0.53	0.68	6.13	0.09	0.36	0.09	0.36
P-5	422,019	9.69		0.53	0.68	9.69	0.09	0.36	0.09	0.36
P-6	28,408	0.65		0.53	0.68	0.65	0.09	0.36	0.09	0.36
P-7	29,092	0.67		0.53	0.68	0.67	0.09	0.36	0.09	0.36
P-8	7,374	0.17	0.17	0.53	0.68	0.00	0.09	0.36	0.53	0.68
										1

Calculated by: CKC

Date:	5/15/2024	
Checked by:	TC	

Rock Island Trail Final Drainage Report Area Drainage Summary - EXISTING

		WEIG	HTED		OVER	LAND		ST	REET /	CHAN	NEL FLO	W	T_t	6	^C A	INTE	NSITY	TOTAL	FLOW
BASIN	AREA TOTAL	C 5	C 100	<i>C</i> 5	Length	Height	T _C	Grass/ Paved	Length	Slope	Velocity	T_t	TOTAL	CA ₅	CA 100	I 5	I 100	Q 5	Q 100
	(Acres)	* For Calcs See	Runoff Summary		(ft)	(ft)	(min)		(ft)	(%)	(fps)	(min)	(min)			(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
E-1	0.87	0.09	0.36	0.09	90	2.0	13.8	Grass Grass	0 0	1.0% 1.0%	1.3 1.3	0.0 0.0	13.8	0.08	0.31	3.6	6.1	0.3	1.9
E-2	7.99	0.09	0.36	0.09	80	10.0	7.3	Grass Grass	0 0	1.0% 1.0%	1.3 1.3	0.0 0.0 0.0	7.3	0.72	2.88	4.6	7.7	3.3	22
E-3	10.64	0.09	0.36	0.09	160	30.0	9.1	Grass Grass	0 0	1.0% 1.0%	1.3 1.3	0.0 0.0 0.0	9.1	0.96	3.83	4.3	7.2	4.1	27
E-4	6.13	0.09	0.36	0.09	90	14.0	7.2	Grass Grass	0 0	1.0% 1.0%	1.3 1.3	0.0 0.0 0.0	7.2	0.55	2.21	4.6	7.7	2.5	17
E-5	9.69	0.09	0.36	0.09	100	17.0	7.4	Grass Grass	0 0	1.0% 1.0%	1.3 1.3	0.0 0.0 0.0	7.4	0.87	3.49	4.6	7.7	4.0	27
E-6	0.65	0.09	0.36	0.09	25	1.0	6.0	Grass Grass	430 0	2.0% 1.0%	1.3 1.3	5.4 0.0 0.0	11.4	0.06	0.23	3.9	6.6	0.2	1.6
E-7	0.67	0.09	0.36	0.09	25	1.0	6.0	Grass Grass	25 0	2.0% 1.0%	1.3 1.3	0.3 0.0 0.0	6.3	0.06	0.24	4.8	8.1	0.3	1.9
E-8	0.17	0.53	0.68	0.09	25	1.0	6.0	Grass Grass	50 0	2.0% 1.0%	1.3 1.3	0.6 0.0 0.0	6.6	0.09	0.12	4.8	8.0	0.4	0.9

Calculated by: CKC Date: 5/15/2024 Checked by: TC

Rock Island Trail Final Drainage Report Area Drainage Summary - PROPOSED

		WEIG	HTED		OVER	LAND		<i>S1</i>	REET /	CHAN	NEL FLO	W	T_t	6	A	INTE	SITY	TOTAL	FLOW
BASIN	AREA TOTAL	<i>C</i> ₅	С 100	<i>C</i> ₅	Length	Height	T _C	Grass/ Paved	Length	Slope	Velocity	T_t	TOTAL	CA 5	CA 100	I 5	I 100	Q 5	Q 100
	(Acres)	* For Calcs See	e Runoff Summary		(ft)	(ft)	(min)		(ft)	(%)	(fps)	(min)	(min)			(in/hr)	(in/hr)	(c.f.s.)	(c.f.s.)
P-1	0.87	0.09	0.36	0.09	90	2.0	13.8	Grass	0	1.0%	1.3	0.0	13.8	0.08	0.31	3.6	6.1	0.3	1.9
								Grass	0	1.0%	1.3	0.0							
P-2	7.99	0.09	0.36	0.09	80	10.0	7.3	Grass	0	1.0%	1.3	0.0	7.3	0.72	2.88	4.6	7.7	3.3	22
								Grass	0	1.0%	1.3	0.0							ľ
												0.0							ľ
P-3	10.64	0.09	0.36	0.09	160	30.0	9.1	Grass	0	1.0%	1.3	0.0	9.1	0.96	3.83	4.3	7.2	4.1	27
								Grass	0	1.0%	1.3	0.0							ľ
												0.0							ľ
P-4	6.13	0.09	0.36	0.09	90	14.0	7.2	Grass	0	1.0%	1.3	0.0	7.2	0.55	2.21	4.6	7.7	2.5	17
								Grass	0	1.0%	1.3	0.0							ľ
												0.0							
P-5	9.69	0.09	0.36	0.09	100	17.0	7.4	Grass	0	1.0%	1.3	0.0	7.4	0.87	3.49	4.6	7.7	4.0	27
								Grass	0	1.0%	1.3	0.0							ľ
												0.0							
P-6	0.65	0.09	0.36	0.09	25	1.0	6.0	Grass	430	2.0%	1.3	5.4	11.4	0.06	0.23	3.9	6.6	0.2	1.6
								Grass	0	1.0%	1.3	0.0							ľ
												0.0							
P- 7	0.67	0.09	0.36	0.09	25	1.0	6.0	Grass	25	2.0%	1.3	0.3	6.3	0.06	0.24	4.8	8.1	0.3	1.9
								Grass	0	1.0%	1.3	0.0							ľ
												0.0							
P-8	0.17	0.53	0.68	0.09	25	1.0	6.0	Grass	50	2.0%	1.3	0.6	6.6	0.09	0.12	4.8	8.0	0.4	0.9
								Grass	0	1.0%	1.3	0.0							
												0.0							

Calculated by: CKC Date: 5/15/2024 Checked by: TC

Rock Island Trail Final Drainage Report Surface Routing Summary

Design	Contributing Basins &				STR	eet / Ch	ANNEL FL	.OW	T_t	INTE	NSITY	FL	OW
Points	Design Points	Equivalent CA 5	Equivalent CA 100	Maximum T _C	Length (ft)	Slope (%)	Velocity (fps)	T _t (min)	TOTAL (min)	I_5	I 100	Q_5	Q 100
<i>E5</i>	E6 E5	0.06	0.23 3.49	11.4 7.4	200	2.0%	2.8	1.2	7.4				
	ES	0.87	3.72	/.4					7.4	4.5	8.0	4.2	30
P5	P6	0.06	0.23	11.4	200	2.0%	2.8	1.2					
	Р5	0.9	3.5 3.72	7.4					7.4	4.5	8.0	4.2	30
					}								
									-				

Calculated by:	CKC
Date:	5/15/2024
Checked by:	TC

Land Use or Surface	Durant						Runoff Co	oefficients					
Characteristics	Percent Impervious	2-у	ear	5-y	ear	10-1	year	ر-25	/ear	י-50	/ear	100-	year
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

Table 6-6. Runoff Coefficients for Rational Method (Source: UDFCD 2001)

3.2 Time of Concentration

One of the basic assumptions underlying the Rational Method is that runoff is a function of the average rainfall rate during the time required for water to flow from the hydraulically most remote part of the drainage area under consideration to the design point. However, in practice, the time of concentration can be an empirical value that results in reasonable and acceptable peak flow calculations.

For urban areas, the time of concentration (t_c) consists of an initial time or overland flow time (t_i) plus the travel time (t_i) in the storm sewer, paved gutter, roadside drainage ditch, or drainage channel. For nonurban areas, the time of concentration consists of an overland flow time (t_i) plus the time of travel in a concentrated form, such as a swale or drainageway. The travel portion (t_i) of the time of concentration can be estimated from the hydraulic properties of the storm sewer, gutter, swale, ditch, or drainageway. Initial time, on the other hand, will vary with surface slope, depression storage, surface cover, antecedent rainfall, and infiltration capacity of the soil, as well as distance of surface flow. The time of concentration is represented by Equation 6-7 for both urban and non-urban areas.

APPENDIX C Runoff Reduction

			Desig	gn Procedu	re Form: F		uction					Shoot 4 -5 1
Designer:	Todd Cartwrig	ght PE		OD-RWb (A6	ersion 3.07, Mai	rcn 2018)						Sheet 1 of 1
Company:	Kiowa Engine	-									-	
Date:	August 15, 20	24									-	
Project:	16028 Rock Is	and Trail									-	
Location:	Basins 1 & 2										-	
											•	
SITE INFORMATION (Use	WQCV F	Rainfall Depth	0.60 0.43	inches inches (for W	/atersheds Ou	itside of the D	enver Region	, Figure 3-1 i	n USDCM Vol	. 3)		
Area Type	UIA:RPA		UIA:RPA	SPA	UIA:RPA	SPA	UIA:RPA	SPA	UIA:RPA			
Area ID	P1-UIA1		P2-UIA1	P2-SPA2	P2-UIA3	P2-SPA4	P2-UIA5	P2-SPA6	P2-UIA7			
Downstream Design Point ID	P1		P2	P2	P2	P2	P2	P2	P2			
Downstream BMP Type	None		None	None	None	None	None	None	None			
DCIA (ft ²)												
UIA (ft ²)			9,160		1,178		6,406		393			
RPA (ft ²)	29,062		30,859		12,961		19,857		5,267			
SPA (ft ²)				18,953		17,914		225,261				
HSG A (%)			100%	100%	100%	100%	100%	100%	100%			
HSG B (%)	0%		0%	0%	0%	0%	0%	0%	0%			
HSG C/D (%)			0%	0%	0%	0%	0%	0%	0%			
Average Slope of RPA (ft/ft)			0.050		0.050		0.050		0.050			
UIA:RPA Interface Width (ft)	1500.00	l	1500.00		600.00		550.00		50.00		l	
CALCULATED RUNOFF Area ID UIA:RPA Area (ft ²)	RESULTS P1-UIA1 38,091		P2-UIA1 40,019	P2-SPA2	P2-UIA3 14,139	P2-SPA4	P2-UIA5 26,263	P2-SPA6	P2-UIA7 5,660			
L / W Ratio	0.06		0.06		0.06		0.09		2.26			
UIA / Area	0.2370		0.2289		0.0833		0.2439		0.0694			
Runoff (in)	0.00		0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Runoff (ft ³)			0	0	0	0	0	0	0			
Runoff Reduction (ft ³)			382	948	49	896	267	11263	16			
CALCULATED WQCV RE	син те											
Area ID			P2-UIA1	P2-SPA2	P2-UIA3	P2-SPA4	P2-UIA5	P2-SPA6	P2-UIA7			
WQCV (ft ³)	376		382	0	49	0	267	0	12-0147			
WQCV Reduction (ft ³)	376		382	0	49	0	267	0	16			
WQCV Reduction (%)			100%	0%	100%	0%	100%	0%	100%			
Untreated WQCV (ft ³)			0	0	0	0	0	0	0			
				•					•			•
CALCULATED DESIGN F Downstream Design Point ID	POINT RESUL	-TS (sums re P2	sults from a	II columns wi	th the same	Downstream	Design Poin	t ID)				
DOWNSTEAM Design Found D DCIA (ft ²)		0										
UIA (ft ²)	9,029	17,137										
RPA (ft ²)		68,944										
SPA (ft ²)		262,128										
Total Area (ft ²)		348,209		1		-			1		1	1
Total Impervious Area (ft ²)		17,137										
WQCV (ft ³)	376	714										
WQCV Reduction (ft ³)	376	714										
WQCV Reduction (%)	100%	100%										
Untreated WQCV (ft ³)	0	0										
CALCULATED SITE RES Total Area (ft ²)		results from	all columns	in worksheet	t)							
Total Impervious Area (ft ²)												
WQCV (ft ³)	1,090											
WQCV Reduction (ft ³)												
	4000/	1										
WQCV Reduction (%)	100%											
WQCV Reduction (%) Untreated WQCV (ft ³)												

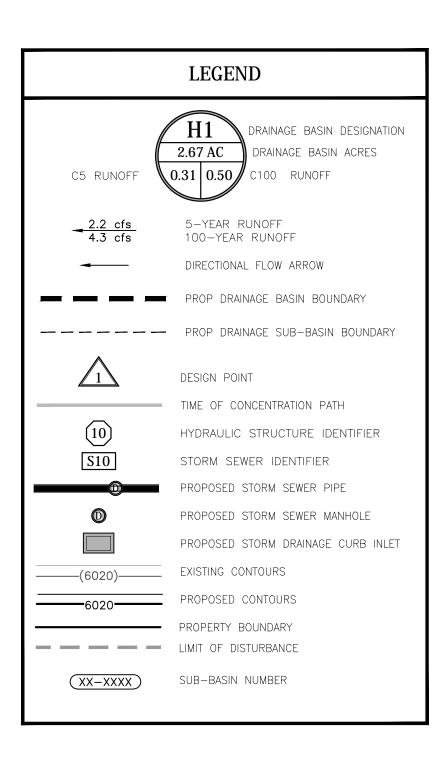
Designer	Todd Corturni	nht DE		OD-RWA (Ae	ersion 3.07, Ma	rch 2018)						Sheet 1 of 1
Designer: Company:	Todd Cartwrig Kiowa Engine	-									-	
Date:	August 15, 20	-									-	
Project:	16028 Rock Is										-	
Location:	Basins 3										-	
Location.	Dasins 5										-	
SITE INFORMATION (Use	er Input in Bl	ue Cells)		-								
		Rainfall Depth		inches								
Depth of Average Ru	noff Producin	g Storm, d ₆ =	0.43	inches (for W	/atersheds Ou	utside of the D	enver Region	i, Figure 3-1 ir	n USDCM Vol	. 3)		
Area Type	SPA	UIA:RPA	UIA:RPA	SPA	SPA	UIA:RPA	UIA:RPA	UIA:RPA	UIA:RPA			
Area ID	P3-UIA1	P3-SPA2	P3-UIA3	P3-SPA4	P3-SPA5	P3-UIA6A	P3-UIA6B	P3-UIA6C	P3-UIA7			
ownstream Design Point ID	P3	P3	P3	P3	P3	P3	P3	P3	P3			
Downstream BMP Type	None	None	None	None	None	None	None	None	None			
DCIA (ft ²)												
UIA (ft ²)		6,795	1,168			11,849	11,849	11,849	294			
RPA (ft ²)		31,927	5,080			55,759	55,759	55,759	5,079	1	1	1
SPA (ft ²)	22,952			16,089	171,042					1	1	1
HSG A (%)	100%	100%	100%	100%	100%	100%	100%	100%	100%		1	1
HSG B (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%		İ	Ì
HSG C/D (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%		İ	
Average Slope of RPA (ft/ft)		0.050	0.050			0.050	0.050	0.050	0.050			
JIA:RPA Interface Width (ft)		800.00	200.00			1000.00	1000.00	1000.00	50.00			
CALCULATED RUNOFF												
Area ID	P3-UIA1	P3-SPA2	P3-UIA3	P3-SPA4	P3-SPA5	P3-UIA6A	P3-UIA6B	P3-UIA6C	P3-UIA7			
UIA:RPA Area (ft ²)		38,722	6,248			67,608	67,608	67,608	5,373			
L / W Ratio		0.06	0.16			0.07	0.07	0.07	2.15			
UIA / Area		0.1755	0.1869			0.1753	0.1753	0.1753	0.0547			
Runoff (in)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00			
Runoff (ft ³)	0	0	0	0	0	0	0	0	0			
Runoff Reduction (ft ³)	1148	283	49	804	8552	494	494	494	12			
CALCULATED WQCV RE		D0.0D40	DO LUAO	D0.0044					D0 1 11 4 7		r	
Area ID	P3-UIA1	P3-SPA2 283	P3-UIA3	P3-SPA4 0	P3-SPA5 0	P3-UIA6A 494	P3-UIA6B 494	P3-UIA6C 494	P3-UIA7 12			
WQCV (ft ³)	0	283	49 49	0	0	494	494	494	12			
WQCV Reduction (ft ³)	0%	283	49	0%	0%	494 100%	494 100%	494 100%	12			
WQCV Reduction (%) Untreated WQCV (ft ³)	0%	0	0	0%	0%	0	0	0	0		-	
	0	0	0	0	0	0	0	0	0			
CALCULATED DESIGN F		TS (sums re	sults from a	ll columns wi	ith the same	Downstream	Design Poin	t ID)				
ownstream Design Point ID	P3		Suits Hom a			Downstream	Designition	(10)				
DCIA (ft ²)	0						-				1	1
UIA (ft ²)	43,804										İ	1
RPA (ft ²)	209,363										İ	1
SPA (it) SPA (it ²)	210,083										İ	1
Total Area (ft ²)	463,250											1
Total Impervious Area (ft ²)	43,804											1
WQCV (ft ³)	1,825											1
WQCV Reduction (ft ³)	1,825											1
WQCV Reduction (%)	100%											1
Untreated WQCV (ft ³)	0			1	1		-	1	-	1	1	1
CALCULATED SITE RES	ULTS (sums	results from	all columns	in worksheet	t)							
Total Area (ft ²)	463,250											
Total Impervious Area (ft ²)	43,804	1										
WQCV (ft ³)	1,825	1										
	1,825	1										
WQCV Reduction (ft ³)		1										
WQCV Reduction (ft ³) WQCV Reduction (%)	100%											
	100% 0											

			Desig	jn Procedu	re Form: I	Runoff Red	uction					
				UD-BMP (Ve	ersion 3.07, Ma	rch 2018)						Sheet 1 of 1
-	Todd Cartwrig	-									•	
	Kiowa Engine	-										
Date:	August 15, 20											
Project:	16028 Rock Is	sland I rail										
Location:	Basins 4											
SITE INFORMATION (Use	WQCV F	Rainfall Depth	0.60 0.43	inches inches (for W	/atersheds Ou	utside of the D	enver Regior	ı, Figure 3-1 i	n USDCM Vo	l. 3)		
Area Type	UIA:RPA	SPA	SPA	UIA:RPA	SPA							
Area ID	P4-UIA1	P4-SPA2	P4-SPA3	P4-UIA4	P4-SPA5							
Downstream Design Point ID	P4	P4	P4	P4	P4							
Downstream BMP Type	None	None	None	None	None							
DCIA (ft ²)												
UIA (ft ²)	702		-	12,561								
RPA (ft ²)	12,065			42,611								
SPA (ft ²)	-	34,092	44,522		120,382							
HSG A (%)	100%	100%	100%	100%	100%							
HSG B (%)	0%	0%	0%	0%	0%							
HSG C/D (%)	0%	0%	0%	0%	0%							
Average Slope of RPA (ft/ft)	0.050			0.050								
UIA:RPA Interface Width (ft)	160.00			1050.00								
CALCULATED RUNOFF I Area ID UIA:RPA Area (ft ²)	P4-UIA1 12,767	P4-SPA2	P4-SPA3	P4-UIA4 55,172	P4-SPA5							
L / W Ratio	0.50			0.06								
UIA / Area	0.0550			0.2277								
Runoff (in)	0.00	0.00	0.00	0.00	0.00							
Runoff (ft ³)	0.00	0.00	0.00	0.00	0.00							
Runoff Reduction (ft ³)	29	1705	2226	523	6019							
CALCULATED WQCV RE Area ID	P4-UIA1	P4-SPA2	P4-SPA3	P4-UIA4	P4-SPA5							
WQCV (ft ³)	29	0	0	523	0							
WQCV Reduction (ft ³)	29	0	0	523	0							
WQCV Reduction (%) Untreated WQCV (ft ³)	100% 0	0% 0	0% 0	100% 0	0%							
CALCULATED DESIGN P Downstream Design Point ID						Downstream	Design Poin	t ID)				
DCIA (ft ²)	0											
UIA (ft ²)	13,263											
RPA (ft ²)	54,676											
SPA (ft ²)	198,996											
Total Area (ft ²)	266,935											
Total Impervious Area (ft ²)	13,263											
WQCV (ft ³)	553											
WQCV Reduction (ft ³)	553											
WQCV Reduction (%)	100%											
Untreated WQCV (ft ³)	0											
CALCULATED SITE RES Total Area (ft ²) Total Impervious Area (ft ²) WQCV (ft ³) WQCV Reduction (ft ³) WQCV Reduction (%) Untreated WQCV (ft ³)	ULTS (sums 266,935 13,263 553 553 100% 0	results from	all columns	in workshee	t)							

Designer:	Todd Cartwrig	nht PE		OD-RWL (A6	ersion 3.07, Ma	ui 2016)				Sheet 1 of
-	Kiowa Engine	-								-
Date:	August 15, 20									-
Project:	16028 Rock Is									-
Location:	Basins 5									_
200440111										-
SITE INFORMATION (Use	er Input in Bl	ue Cells)								
		Rainfall Depth		inches						
Depth of Average Ru	noff Producin	g Storm, d ₆ =	0.43	inches (for W	/atersheds Ou	itside of the D	enver Region	i, Figure 3-1 ir	n USDCM Vol. 3)	
Area Tura		SD4	UIA:RPA	SD4	UIA:RPA	SPA				
Area Type Area ID	UIA:RPA P5-UIA1	SPA P5-SPA2	P5-UIA3	SPA P5-SPA4	P5-UIA5	PR-SPA	UIA:RPA PB-UIA7A	UIA:RPA P5-UIA7B		
ownstream Design Point ID	P5	P5	P5	P5	P5	P5	P5	P5		
Downstream BMP Type	None	None	None	None	None	None	None	None		
DCIA (ft ²)										
UIA (ft ²)	694		4,787		2,890		10,125	10,126		
RPA (ft ²)	10,765		29,493	-	22,377		42,331	42,332		
SPA (ft ²)		7,202		110,011		128,886				
HSG A (%)	100%	100%	100%	100%	100%	100%	100%	100%		
HSG B (%)	0%	0%	0%	0%	0%	0%	0%	0%		
HSG C/D (%)	0%	0%	0%	0%	0%	0%	0%	0%		↓
Average Slope of RPA (ft/ft)	0.050		0.050		0.050		0.050	0.050	├ ── ├ ──	+ $+$ $+$ $$
JIA:RPA Interface Width (ft)	100.00		1050.00		450.00		825.00	825.00		
CALCULATED RUNOFF	RESULTS									
Area ID	P5-UIA1	P5-SPA2	P5-UIA3	P5-SPA4	P5-UIA5	PR-SPA6	PB-UIA7A	P5-UIA7B		
UIA:RPA Area (ft ²)	11,459		34,280		25,267		52,456	52,458		
L / W Ratio	1.15		0.06		0.12		0.08	0.08		
UIA / Area	0.0606		0.1396		0.1144		0.1930	0.1930		
Runoff (in)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00		
Runoff (ft ³)	0	0	0	0	0	0	0	0		
Runoff Reduction (ft ³)	29	360	199	5501	120	6444	422	422		
CALCULATED WQCV RE									г – т	<u>т т</u>
Area ID	P5-UIA1	P5-SPA2	P5-UIA3	P5-SPA4	P5-UIA5	PR-SPA6	PB-UIA7A	P5-UIA7B		
WQCV (ft ³)	29 29	0	199 199	0	120 120	0	422 422	422 422		
WQCV Reduction (ft ³) WQCV Reduction (%)	100%	0%	100%	0%	120	0%	100%	100%		
Untreated WQCV (ft ³)	0	0 %	0	0%	0	0%	0	0		
	0	Ū	0	v	Ŭ	0	Ū	Ŭ		
CALCULATED DESIGN F		TS (sums re	sults from al	l columns wi	ith the same	Downstream	Design Poin	t ID)		
ownstream Design Point ID	P5						Ŭ			
DCIA (ft ²)	0									
UIA (ft ²)	28,622									
RPA (ft ²)	147,298									
SPA (ft ²)	246,099									
Total Area (ft ²)	422,019									
Total Impervious Area (ft ²)	28,622				ļ			ļ		↓ ↓ ↓
WQCV (ft ³)	1,193								├ ── ├ ──	├
WQCV Reduction (ft ³)	1,193								<u> </u>	+
WQCV Reduction (%) Untreated WQCV (ft ³)	100% 0								<u>├──</u>	<u>├ </u>
	U				1			1	<u> </u>	<u> </u>
CALCULATED SITE RES	ULTS (sume	results from	all columns	in worksheet	6					
Total Area (ft ²)	422,019				-,					
Total Impervious Area (ft ²)	28,622									
WQCV (ft ³)	1,193									
WQCV Reduction (ft ³)	1,193									
WQCV Reduction (%)	100%									
WQCV Reduction (70)										
Untreated WQCV (ft ³)	0									

			Desig	gn Procedu	re Form: F	Runoff Red	luction					
				UD-BMP (Ve	ersion 3.07, Ma	rch 2018)						Sheet 1 of 1
Designer:	Todd Cartwrig										-	
Company:	Kiowa Engine										-	
Date:	August 15, 20										-	
Project:	16028 Rock Is	aland Trail									-	
Location:	Basins 6 - 8										-	
SITE INFORMATION (Use Depth of Average Ru	WQCV F	Rainfall Depth	0.60 0.43	inches inches (for W	atersheds Ou	utside of the D	enver Region	n, Figure 3-1 i	n USDCM Vo	l. 3)		
Area Type	UIA:RPA		UIA:RPA	SPA		UIA:RPA						
Area ID	P6-UIA1		P7-UIA1	P7-SPA2		P8-UIA1						
Downstream Design Point ID	P6		P7	P7		P8						
Downstream BMP Type	None		None	None		None						
DCIA (ft ²)												
UIA (ft ²)	2,648		5,583			4,400						
RPA (ft ²)	25,760		14,316			2,974						
SPA (ft ²)				9,193								
HSG A (%)			100%	100%		100%						
HSG B (%)	0%		0%	0%		0%						
HSG C/D (%)			0%	0%		0%		ļ	ļ	ļ		
Average Slope of RPA (ft/ft)			0.050			0.050						
UIA:RPA Interface Width (ft)	450.00		450.00			400.00						
CALCULATED RUNOFF Area ID UIA:RPA Area (ft ²) L / W Ratio			P7-UIA1 19,899 0.10	P7-SPA2 		P8-UIA1 7,374 0.06						
UIA / Area	0.0932		0.2806			0.5967						
Runoff (in)			0.00	0.00		0.00						
Runoff (ft ³)	0		0	0		0						
Runoff Reduction (ft ³)	110		233	460		183						
CALCULATED WQCV RE	ени те											
Area ID			P7-UIA1	P7-SPA2		P8-UIA1				1		
WQCV (ft ³)			233	0		183						
WQCV Reduction (ft ³)			233	0		183						
WQCV Reduction (%)			100%	0%		100%						
Untreated WQCV (ft ³)			0	0		0						
CALCULATED DESIGN F Downstream Design Point ID		P7	P8	ll columns wi	th the same	Downstream	Design Poir	nt ID)		1		
DOWNSTICATIN Design Form 1D DCIA (ft ²)		0	0					1	ł	1		
UIA (ft ²)	2,648	5,583	4,400					İ		l		
RPA (ft ²)		14,316	2,974					İ		l		
SPA (ft ²)		9,193	0									
Total Area (ft ²)		29,092	7,374			1	1	1	1	1	1	
Total Impervious Area (ft ²)		5,583	4,400			1	1	1	1	1	1	
WQCV (ft ³)	110	233	183			1	1	1	1	1	1	
WQCV Reduction (ft ³)		233	183					İ		İ		
WQCV Reduction (%)		100%	100%									
Untreated WQCV (ft ³)	0	0	0									
CALCULATED SITE RES Total Area (ft ²) Total Impervious Area (ft ²) WQCV (ft ³) WQCV Reduction (ft ³) WQCV Reduction (%)	64,874 12,631 526 526	results from	all columns	in worksheef)							
Untreated WQCV (ft ³)]										





	UIA	
ł	PIA/RPA	
	SPA	
-	DCIA	

Runoff Reduction Calculations

Total Site Area (ac)	36.8
Total Triburary Area (ac)	36.8
Total Disturbed Area (ac)	18.2
Total Site Impervious Area (ac)	2.5
Total Site Percent Impervious	6.8%
Upstream Impervious Area (ac)	0.0
Planned Infiltration Area (ac)	0.0
WQCV (cf)	5576
Stormwater Volume Reduction (cf)	5576
Stormwater Volume Reduction as % of WQCV	100%
-	

ROC Sand Cr No Revisions: RUNC TAC Designer: Revised: JDC Detailer: Void: 8/15/20 Date:

Runoff Reduction Legend

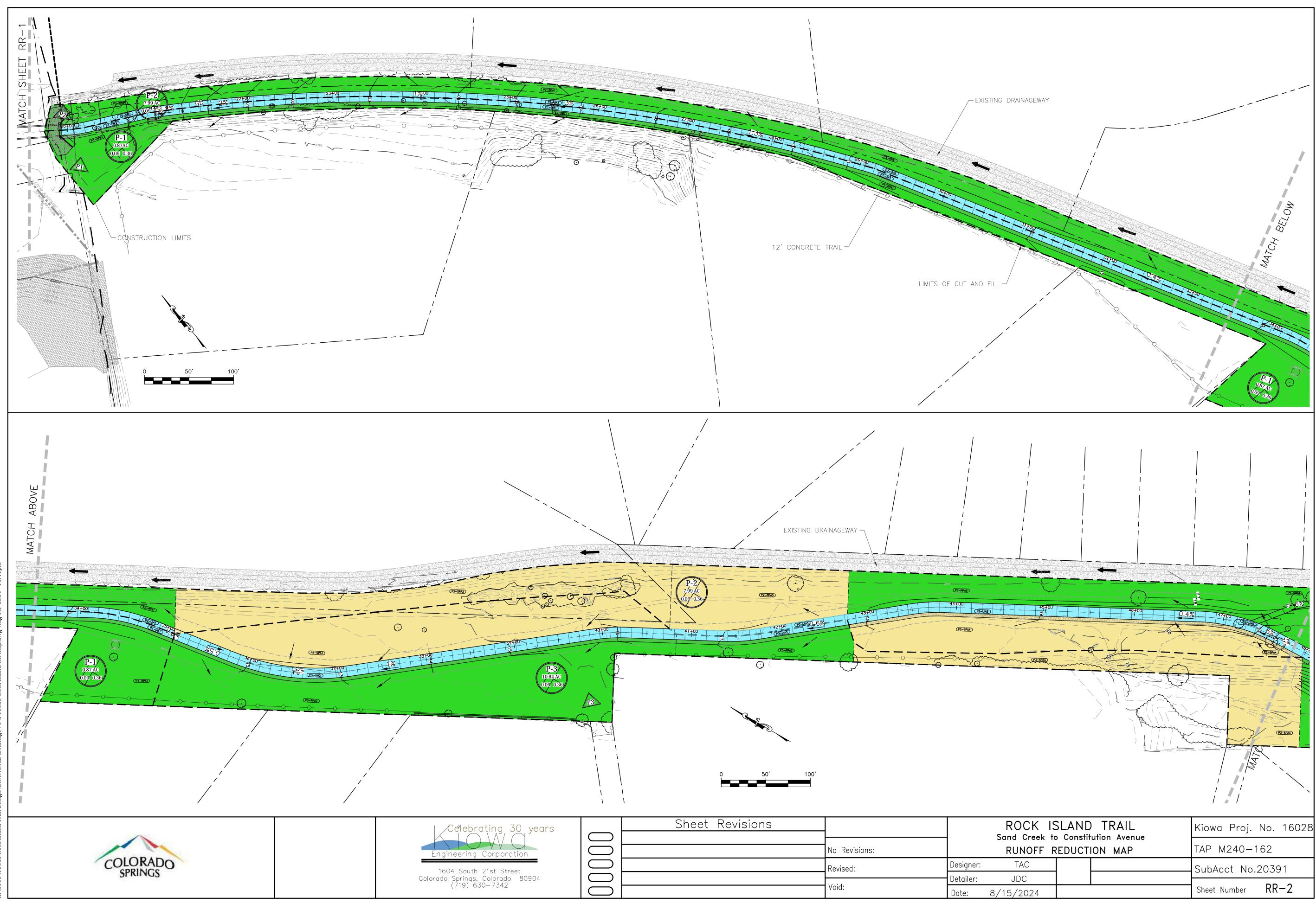
UIA (Unconnected Impervious Area)

PIA (Planned Infiltration Area) RPA (Receiving Pervious Area)

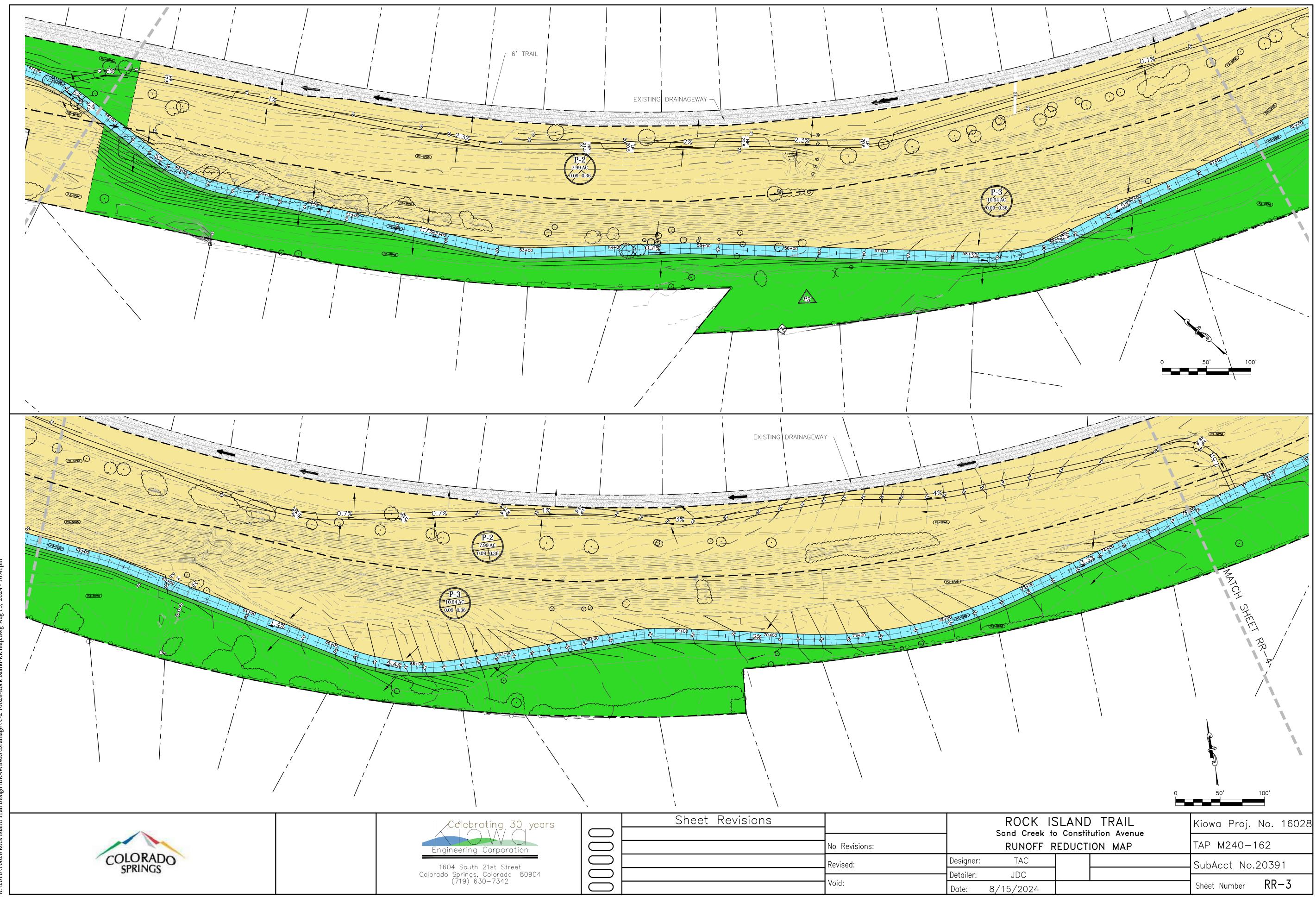
SPA (Separate Pervious Area)

DCIA (Directly-Connected Impervious Area)

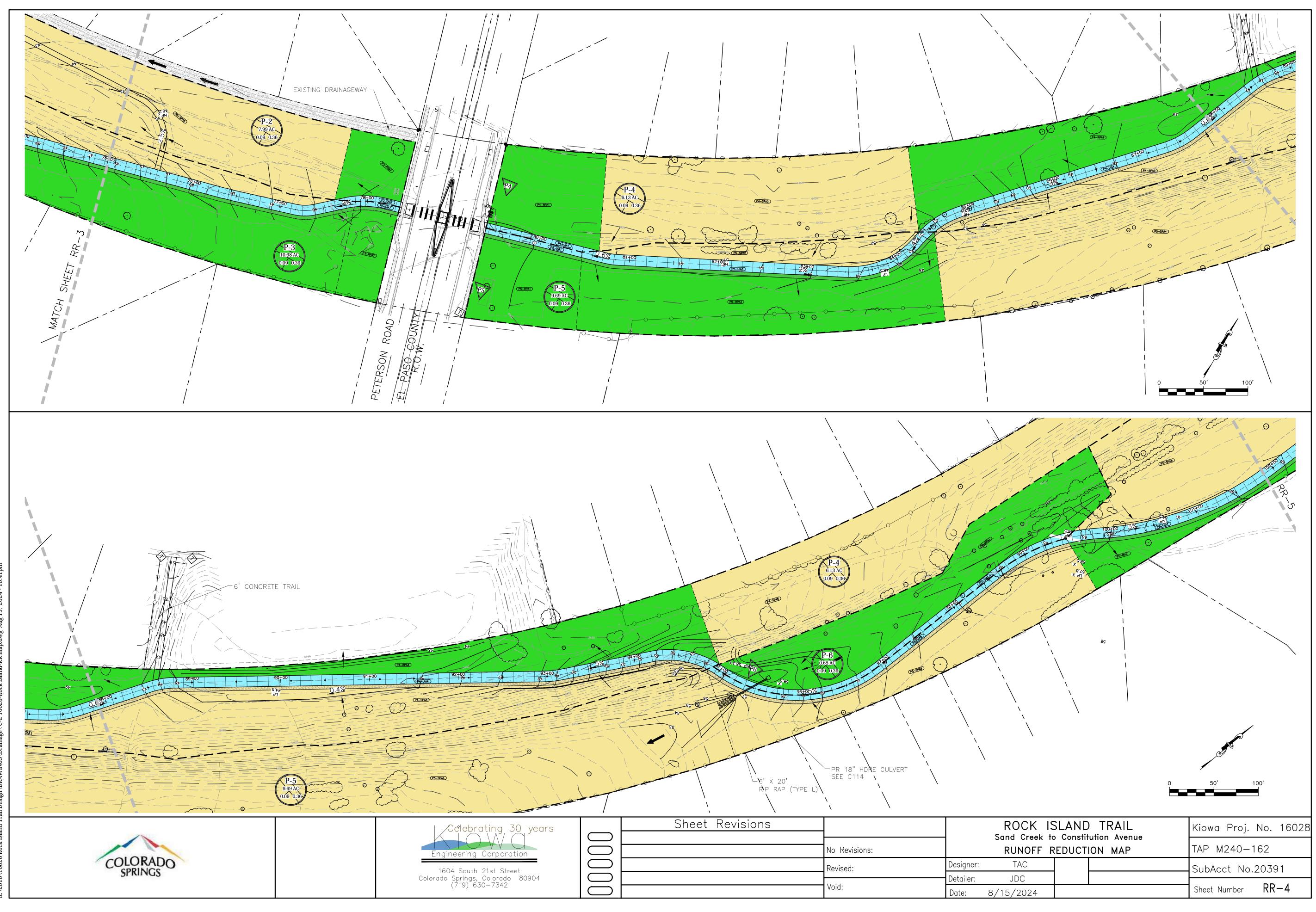
CK ISLAND TRAIL Creek to Constitution Avenue	Kiowa Proj. No. 16028					
	TAP M240-162					
C	SubAcct No.20391					
024	Sheet Number RR-1					



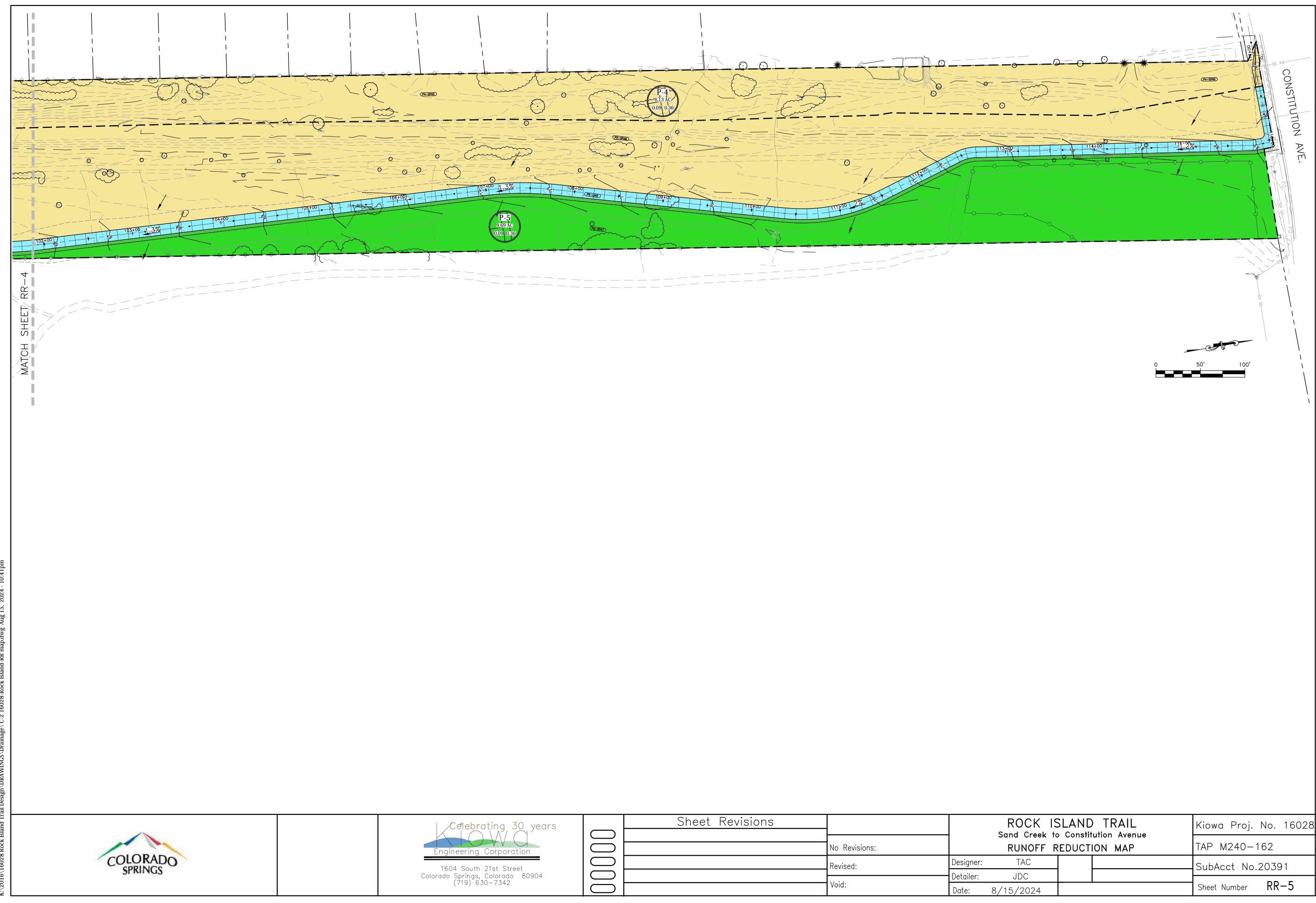
	 v INAIL	Kiowa Proj. No. 16028 TAP M240-162					
	ION MAP						
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brating 30 years		Sheet Revisions		ROCK ISLAND TRAIL Sand Creek to Constitution Avenue	Proj. No. 16028
			No Revisions:	RUNOFF REDUCTION MAP TAP M2	40-162
uth 21st Street	\bigcirc		Revised:	Designer: TAC SubAcct	No.20391
ngs, Colorado 80904) 630-7342			Void:	Detailer: JDC Sheet Num	

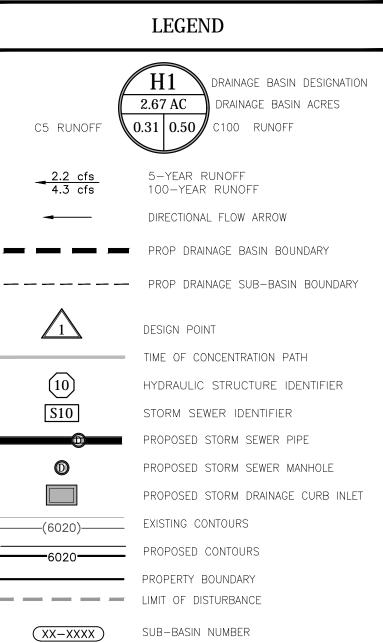
<u>APPENDIX D</u> Drainage Maps

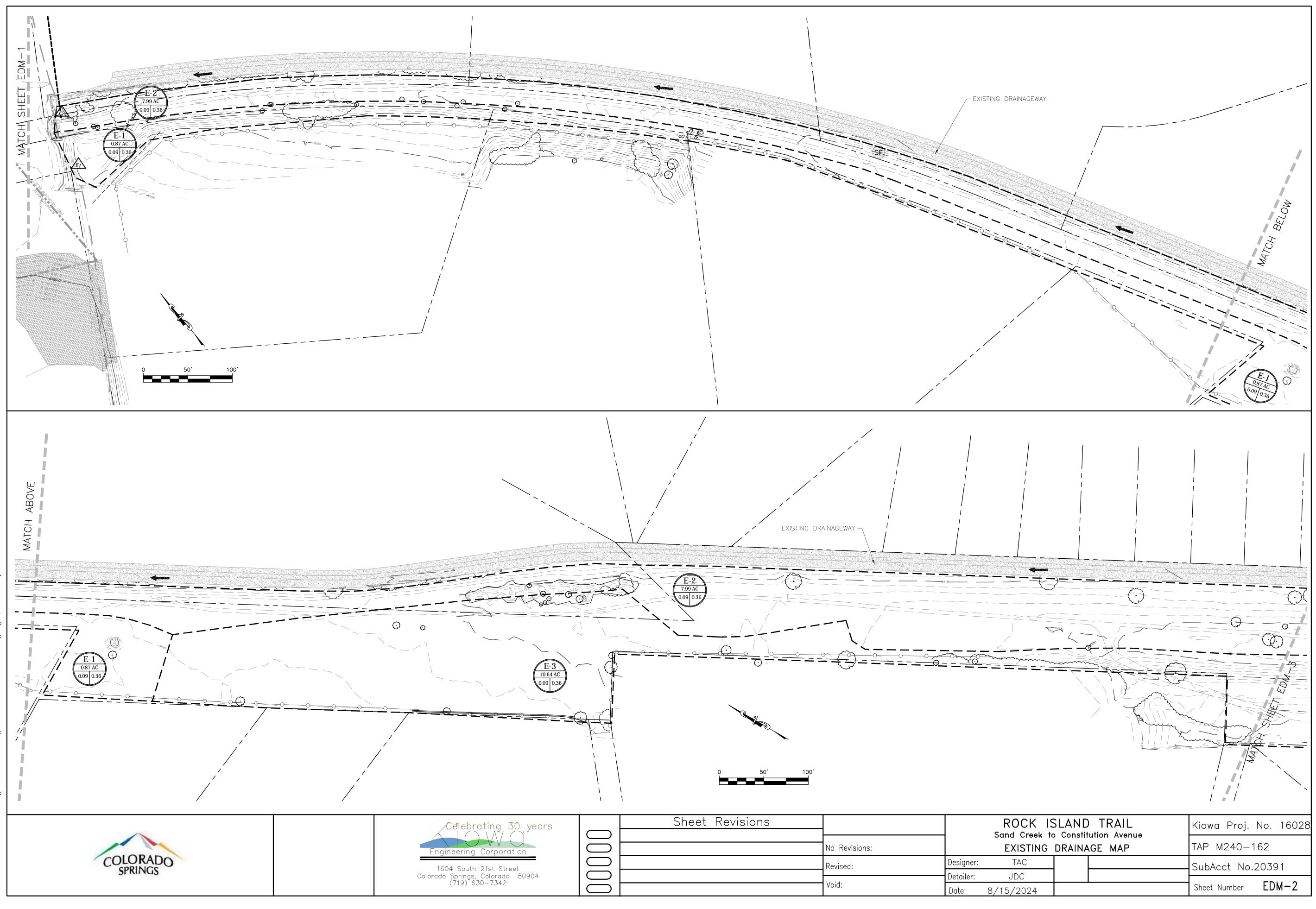




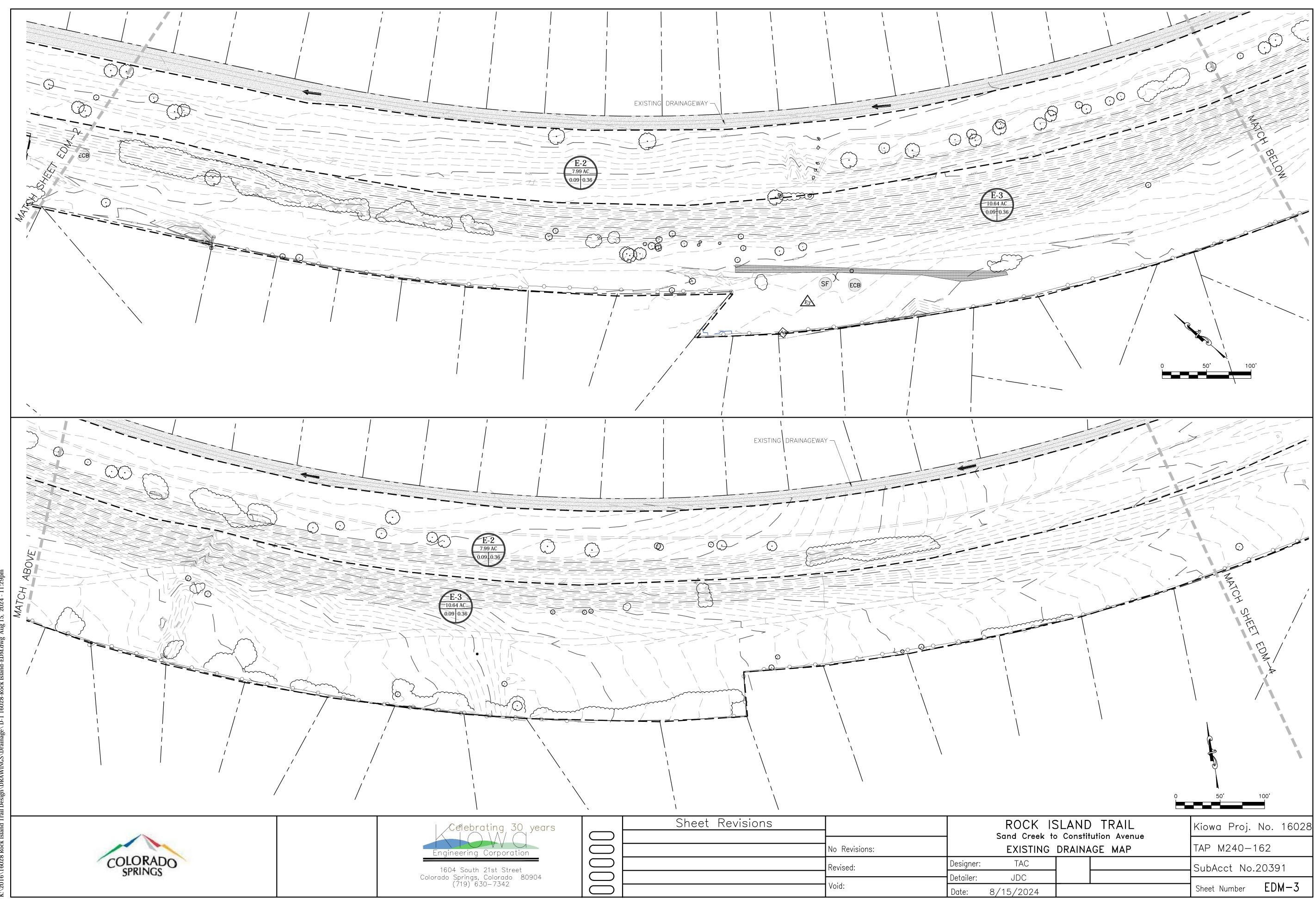
SUB-BASIN AND DESIGN POINT DISCHARGES								
DESIGN POINT	CONTRIBUTING BASINS	AREA	5-YR RUNOFF COEF.	100-YR RUNOFF COEF.	5-YR FLOW	100-YR FLOW		
E1	E-1	0.87 ac	0.09	0.36	0.3 cfs	1.9 cfs		
E2	E-2	7.99 ac	0.09	0.36	3.3 cfs	22 cfs		
E3	E-3	10.64 ac	0.09	0.36	4.1 cfs	27 cfs		
E4	E-4	6.13 ac	0.09	0.36	2.5 cfs	17 cfs		
E5	E-5 & E-5	9.69 ac	0.09	0.36	4.2 CFS	30 CFS		
E6	E-6	0.65 AC	0.09	0.36	0.2 CFS	1.6 CFS		
E7	E-7	0.67 AC	0.09	0.36	0.3 CFS	1.9 CFS		
E8	E-8	0.17 AC	0.53	0.68	0.4 CFS	10.9 CFS		

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ebrating 30 years		Sheet Revisions		ROCK ISLAND TRAIL Sand Creek to Constitution Avenue	Kiowa Proj. No. 16028
ing Corporation	$\left \right $		No Revisions:	EXISTING DRAINAGE MAP	TAP M240-162
buth 21st Street			Revised:	Designer: TAC	SubAcct No.20391
ngs, Colorado 80904) 630-7342			Void:	Detailer: JDC Date: 8/15/2024	Sheet Number EDM-1

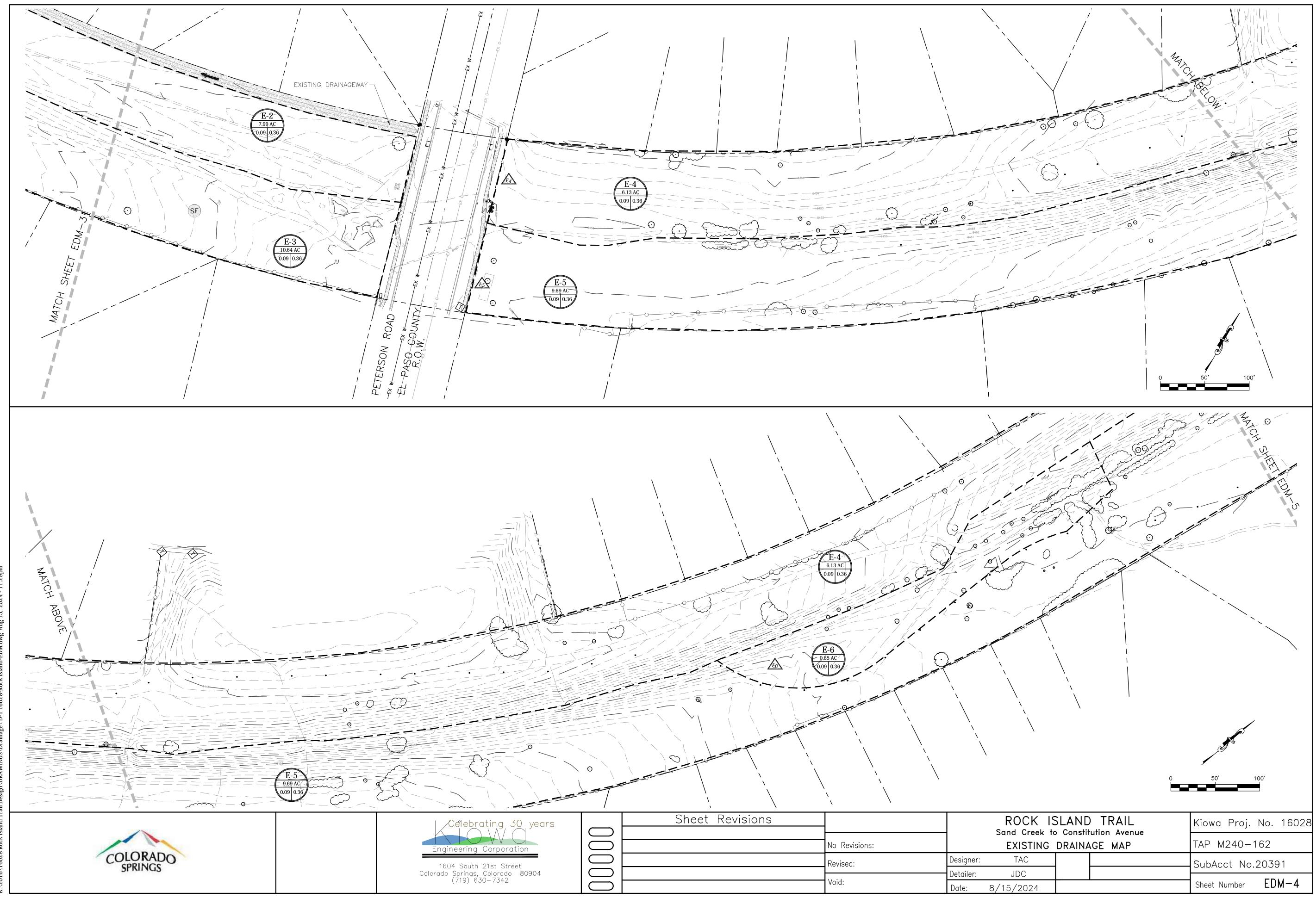


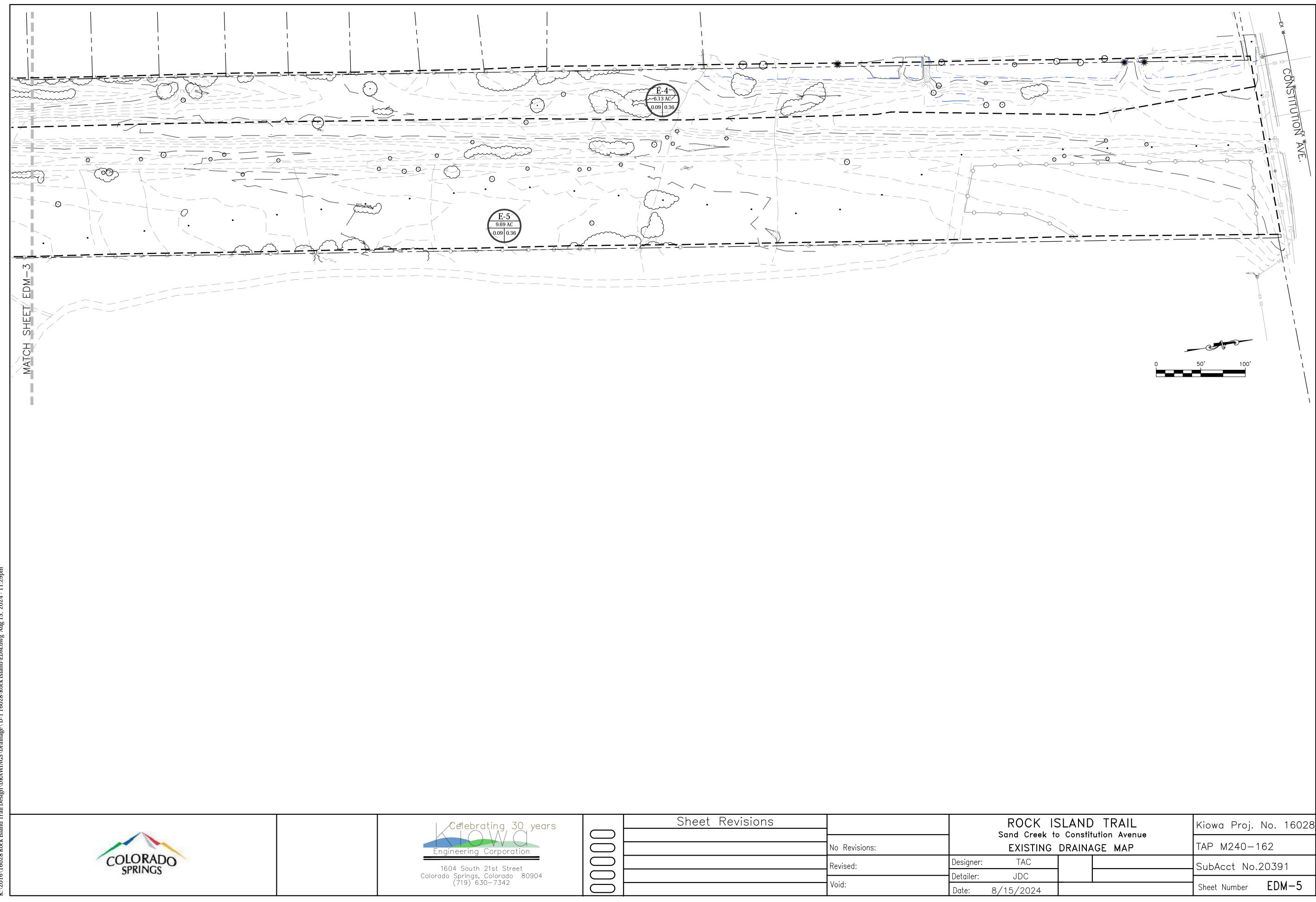


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		Sheet	Numt	ber	EDM



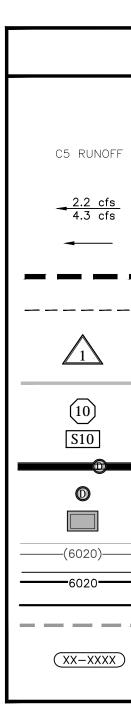
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Corporation	$\left(\right) \left(\right)$		No Revisions:		EXISTIN
n 21st Street	\bigcirc		Revised:	Designer:	TAC
, Colorado 80904	\bigcirc			Detailer:	JDC
530-7342	\bigcirc		Void:	Date:	8/15/2024





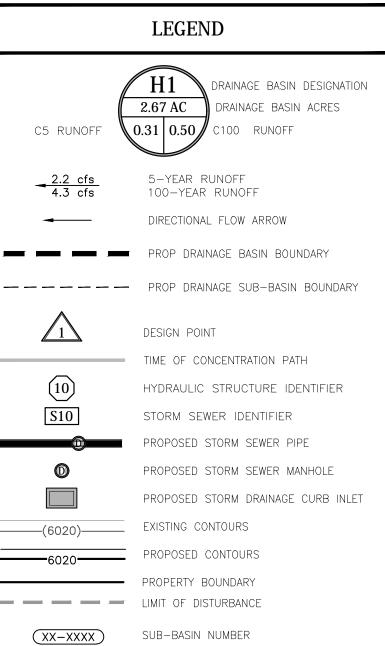
ebrating 30 years	Sheet Revisions		ROCK ISLAND TRAIL Sand Creek to Constitution Avenue	Kiowa Proj. No. 16028
ng Corporation		No Revisions:	EXISTING DRAINAGE MAP	TAP M240-162
uth 21st Street		Revised:	Designer: TAC	SubAcct No.20391
ngs, Colorado 80904) 630-7342		Void:	Detailer: JDC Date: 8/15/2024	Sheet Number EDM-5

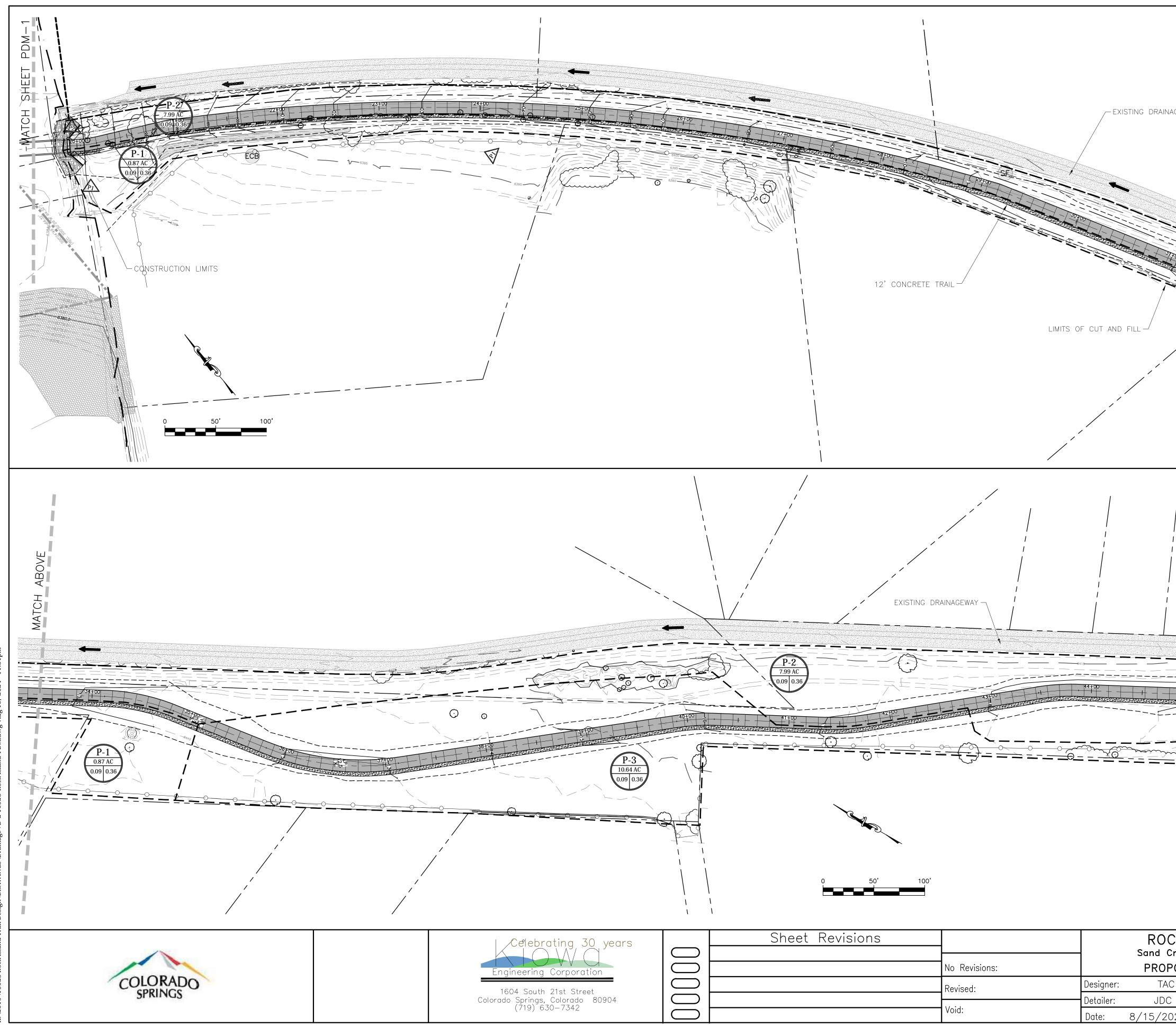




	SUB-BASIN AND DESIGN POINT DISCHARGES							
DESIGN POINT	CONTRIBUTING BASINS	AREA	5-YR RUNOFF COEF.	100-YR RUNOFF COEF.	5-YR FLOW	100-YR FLOW		
P1	P-1	0.87 ac	0.09	0.36	0.3 cfs	1.9 cfs		
P2	P-2	7.99 ac	0.09	0.36	3.3 cfs	22 cfs		
P3	P-3	10.64 ac	0.09	0.36	4.1 cfs	27 cfs		
P4	P-4	6.13 ac	0.09	0.36	2.5 cfs	17 cfs		
P5	P-5 & P-6	9.69 ac	0.09	0.36	4.2 cfs	30 cfs		
P6	P-6	0.65 AC	0.09	0.36	0.2 CFS	1.6 CFS		
P7	P-7	0.67 AC	0.09	0.36	0.3 CFS	1.9 CFS		
P8	P-8	0.17 AC	0.53	0.68	0.4 CFS	10.9 CFS		

The Construction of Deserver Leaver	Developed to be been been been been been been been	ALTERNINGAL DALIDALIYAL DALIYASIYA DALIYASIYA DALIMAN DALIMAN DALIMAN DALIMAN DALIMAN DALIMAN DALIMAN DALIMAN D			
ebrating, 30 years		Sheet Revisions			Kiowa Proj. No. 16028
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outh 21st Street	$\left \right $		Revised:	Designer: TAC	SubAcct No.20391
ngs, Colorado 80904 9) 630-7342	$\left \begin{array}{c} \\ \\ \\ \end{array} \right $		Void:	Detailer: JDC Date: 8/15/2024	Sheet Number PDM-1

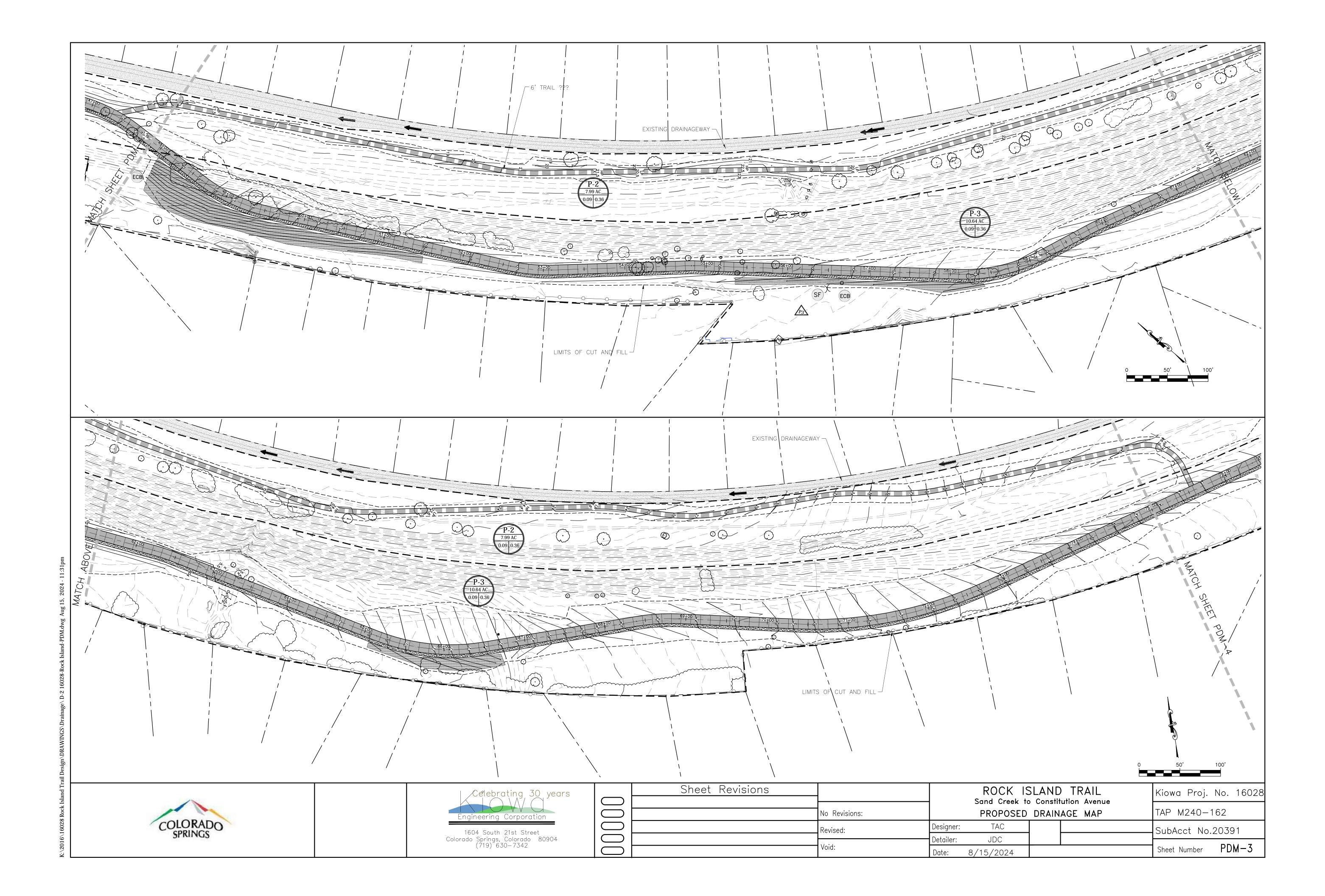


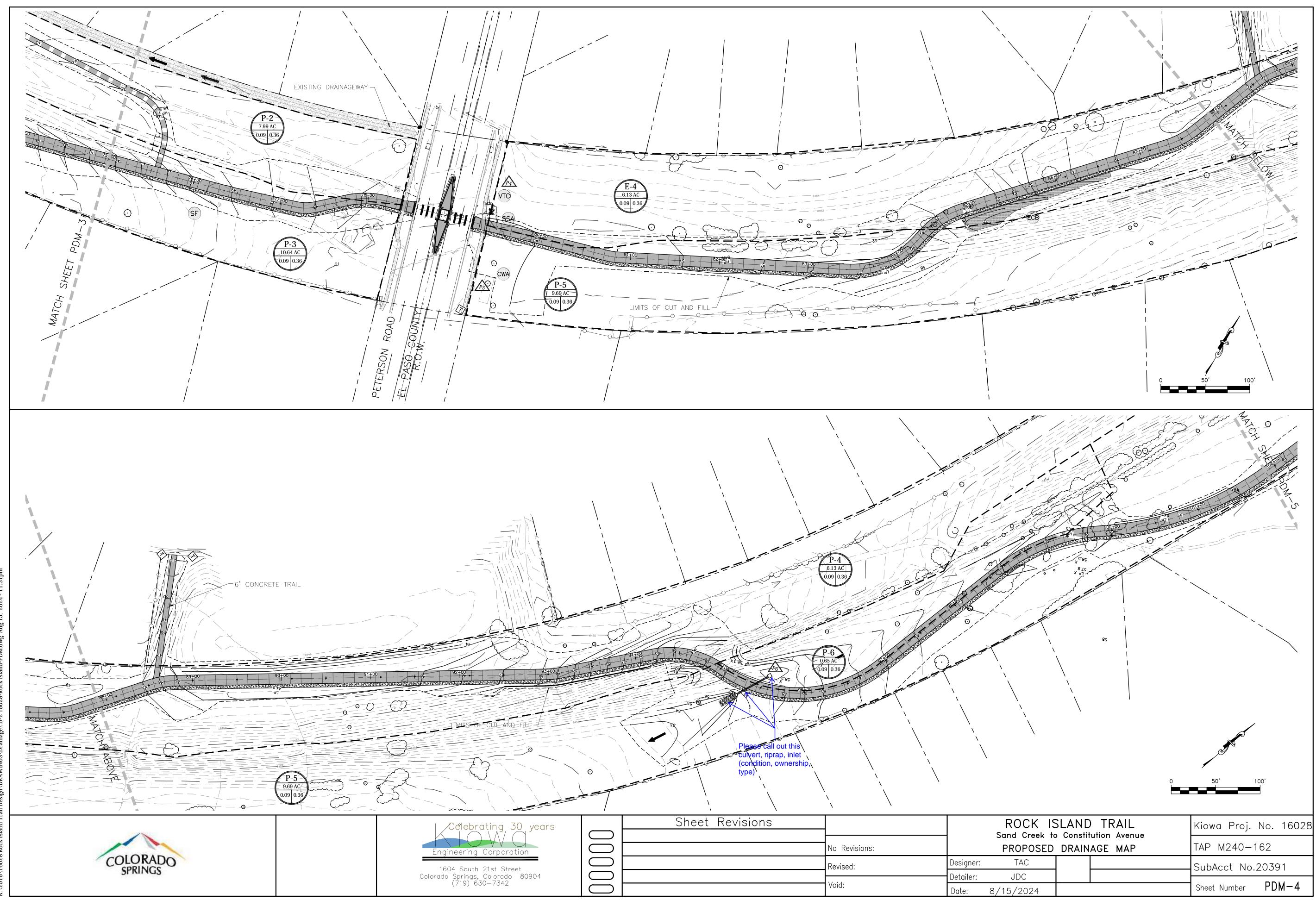


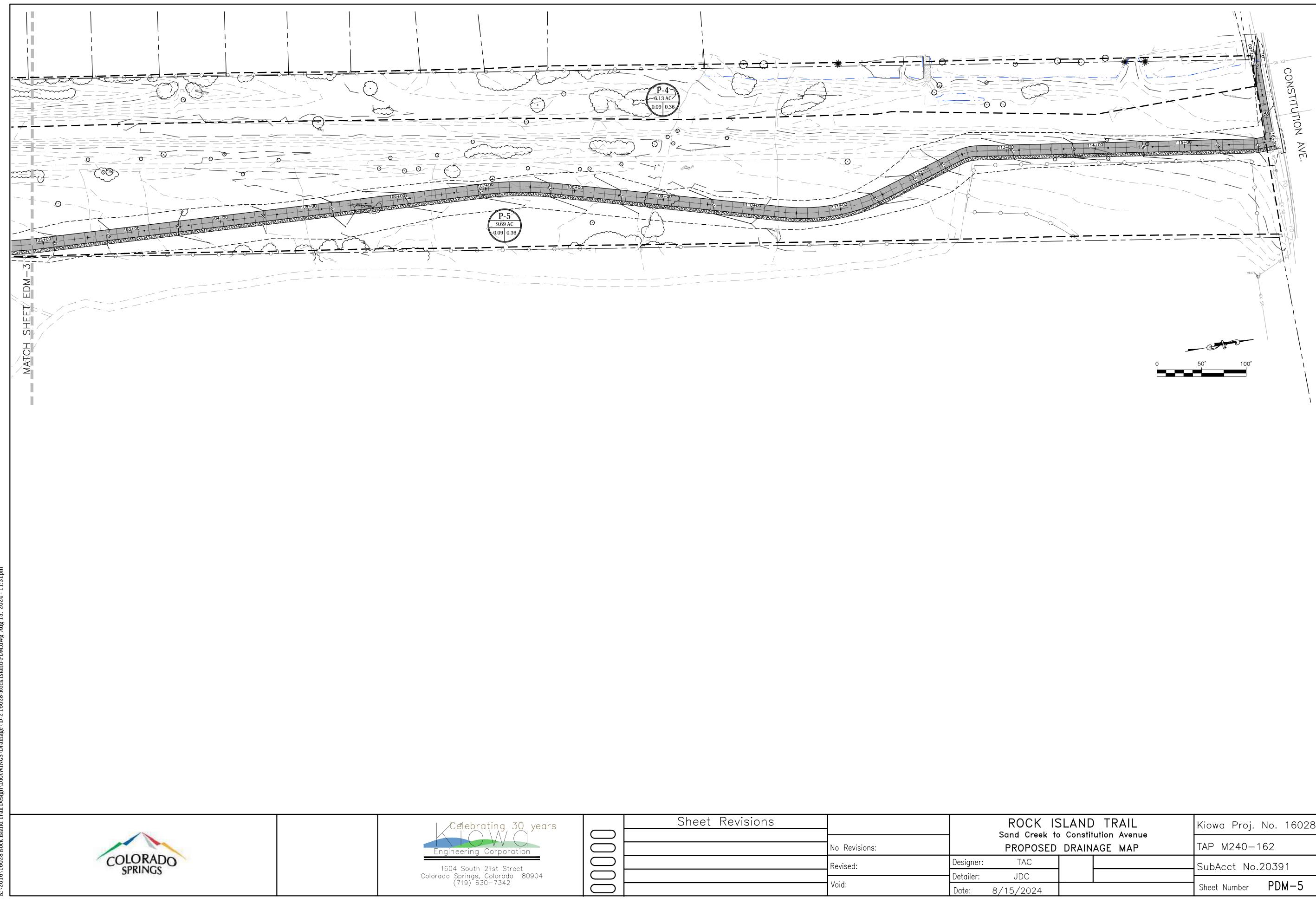
6/16028 Rock Island Trail Design/DRAWINGS/Drainage/ D-2 16028-Rock Island-PDM.dwg Aug 15, 2024 - 11:3

AGEWAY	March Brian
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Creek to Constitution Avenu	KIOWA Proj. No. 16020				
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ing Corporation			No Revisions:	PROPOSED DRAINAGE MAP	TAP M240-162
uth 21st Street			Revised:	Designer: TAC	SubAcct No.20391
ngs, Colorado 80904) 630-7342			Void:	Detailer: JDC Date: 8/15/2024	Sheet Number PDM-5