

# TRAFFIC IMPACT STUDY

For

**3980 Walker Road  
El Paso County, Colorado**  
PCD File No. PPR2350

April 2024  
Revised June 2024

Prepared for:

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23-071949

**Traffic Engineer's Statement**

The attached traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.



06/26/2024

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Fred Lantz, P.E. #23410

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Date

**Developer's Statement**

I, the Developer, have read and will comply with all commitments made on my behalf within this report.



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06/26/2024

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Valens Capital, LLC  
3980 Walker Road  
Colorado Springs, CO 80908

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Date

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## I. Introduction

### Project Overview

This traffic impact study is provided as a planning document and addresses the capacity, geometric, and control requirements associated with the development entitled 3980 Walker Road. The development is located near the northeast corner of Walker Road and Kerry Run Road in El Paso County, Colorado.

This traffic impact study has been revised to reflect an updated density.

### Study Area Boundaries

The study area to be examined in this analysis encompasses the segment of Walker Road bounded by State Highway 83 and Kerry Run Road.

Figure 1 illustrates location of the site and study intersections.

### Site Description

Land for the development is currently occupied by one single-family home and storage area for farm equipment and is surrounded by a mix of institutional and residential land uses.

The proposed development is conceptual in nature as no specific user has been determined. However, for purposes of this analysis, the proposed development is understood to entail the new construction of an approximate 11,100 square foot modular building intended to be used as a religious institution, such as for bible study groups, supporting a maximum 95 attendees, as allowed within the County's RR-5 (Rural Residential) zoning district.

Existing access to the development is provided via one full-movement access onto Walker Road (referred to as Site Access). The Site Access intersection with Walker Road was excluded from analysis as internal intersection operations are expected to be comparable to, or better than, those projected for the above-described intersections.

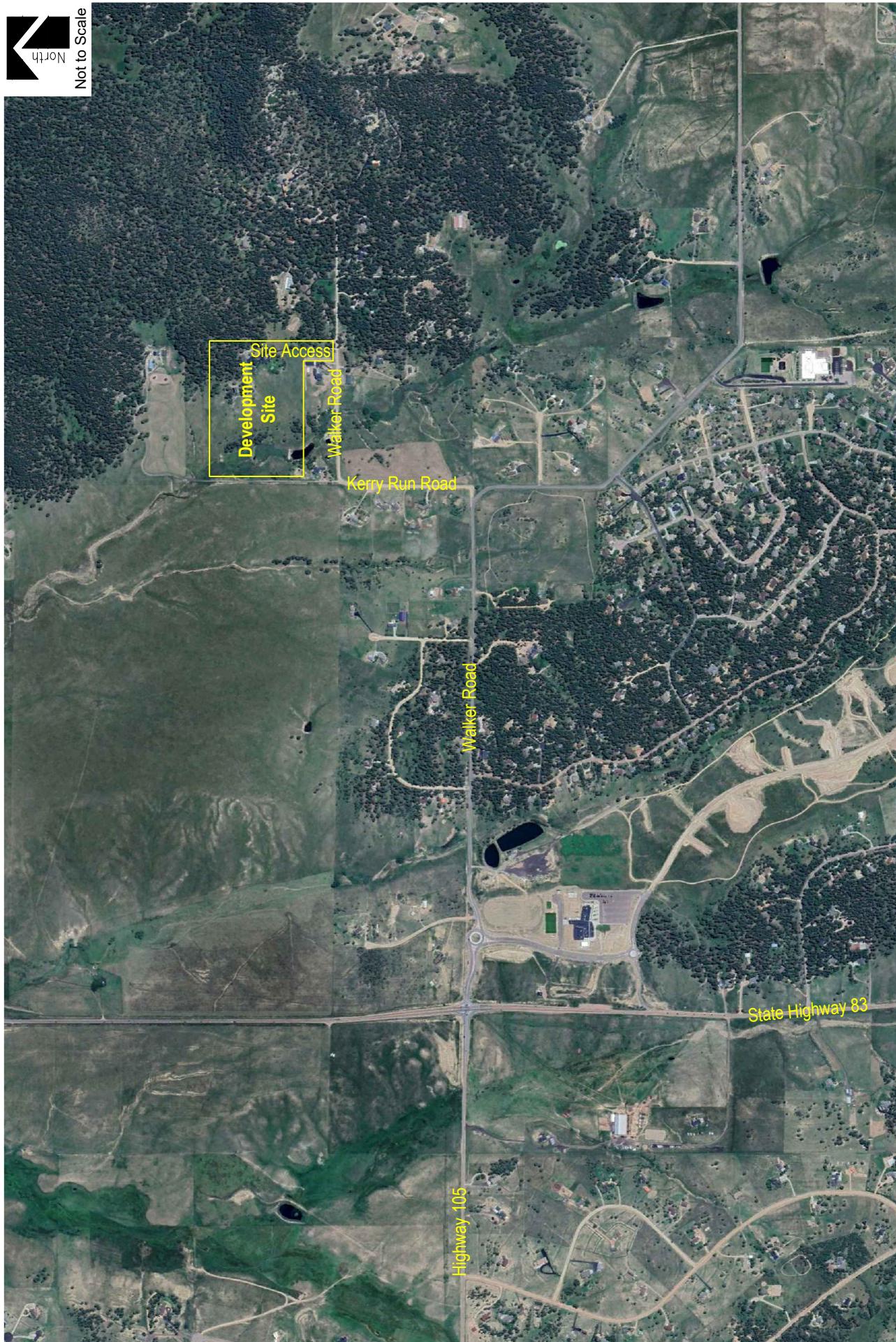
For purposes of this study, it is anticipated that development construction would be completed by end of Year 2026.

General site and access locations are shown on Figure 1.

A conceptual site plan, as prepared by Valens Capital, LLC, is shown on Figure 2. This plan is provided for illustrative purposes only.



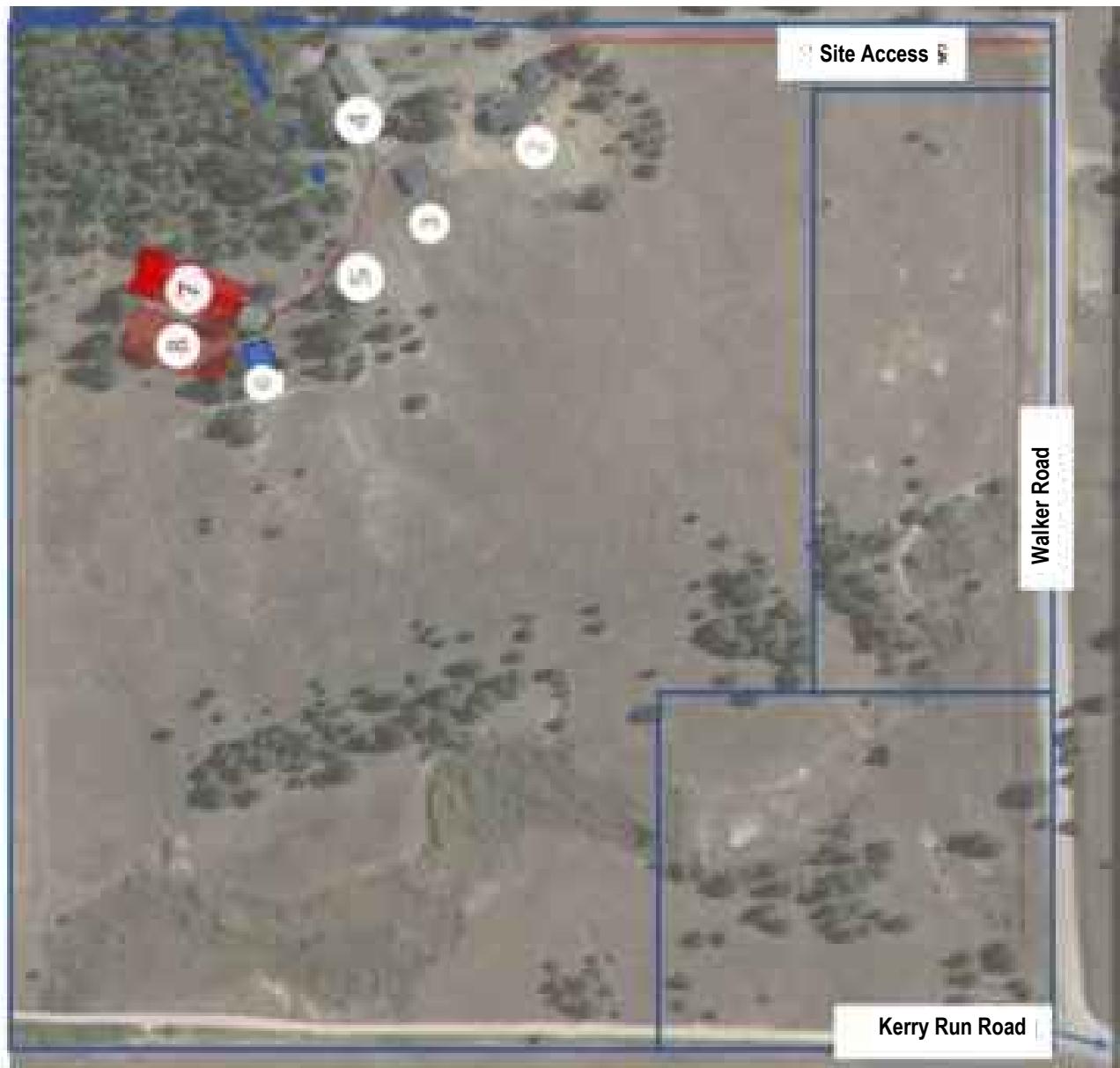
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3980 WALKER ROAD  
Traffic Impact Study



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3980 WALKER ROAD  
Traffic Impact Study

**SM ROCHA, LLC**  
Traffic and Transportation Consultants



## Existing and Committed Surface Transportation Network

Within the study area, Kerry Run Road is the primary roadway that will accommodate traffic to and from the proposed development. The secondary roadways include Walker Road, State Highway 83, and Highway 105. A brief description of each roadway, based on El Paso County's 2016 Major Transportation Corridors Plan Update (MTCP)<sup>1</sup>, is provided below:

Kerry Run Road is a north-south rural, local, gravel roadway having two through lanes (one lane in each direction) with shared turn lanes at the intersection within the study area. Kerry Run Road does not provide a posted speed limit within the study area. In accordance with the County's Engineering Criteria Manual (ECM)<sup>2</sup>, Kerry Run Road is assumed to provide a posted speed limit of 30 MPH.

Walker Road is an east-west collector roadway having two through lanes (one lane in each direction) with a combination of shared and exclusive turn lanes at the intersections within the study area. Walker Road provides a posted speed limit of 35 MPH. Walker Road ends at State Highway 83 and continues west as Highway 105.

East of Kerry Run Road and adjacent to the development, Walker Road is an east-west rural, local, gravel roadway that provides two through lanes with shared turn lanes at the intersections within the study area. Walker Road does not provide a posted speed limit within the study area. In accordance with the County's ECM, Walker Road is assumed to provide a posted speed limit of 30 MPH.

Highway 105 is an east-west principal arterial roadway having two through lanes (one lane in each direction) with a combination of shared and exclusive turn lanes at the intersection within the study area. Highway 105 provides a posted speed limit of 50 MPH. Highway 105 ends at State Highway 83 and continues east as Walker Road.

State Highway 83 is a north-south state roadway having two through lanes (one lane in each direction) with exclusive turn lanes at the intersection within the study area. The Colorado Department of Transportation (CDOT) categorizes the adjacent segment of State Highway 83 as a Regional Highway (R-A) and provides a posted speed limit of 60 MPH.

The study intersection of Walker Road and State Highway 83 is signalized. The intersection of Kerry Run Road and Walker Road operates under a stop-controlled condition. A stop-controlled intersection is defined as a roadway intersection where vehicle rights-of-way are controlled by one or more "STOP" signs.

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<sup>1</sup> El Paso County 2016 Major Transportation Corridors Plan Update, Felsburg Holt & Ullevig, December 2016.

<sup>2</sup> El Paso County Engineering Criteria Manual, El Paso County, October 2020.

Pursuant to County's MTCP, Highway 105 and Walker Road (collector) will be widened from two to four through lanes from Knollwood Boulevard to Steppler Boulevard. However, the County's MTCP plan does not mention when this will occur.

No other regional or specific improvements for the above described roadways are known to be planned or committed at this time. All other study area roadways appear to be built to their ultimate cross-sections.

A sight distance evaluation was considered for the intersection of Kerry Run Road and Walker Road. However, upon visual inspection of the existing intersection and roadway, it is observed that no significant landscaping is present, and the terrain is relatively flat. As such, it is believed that sight distances are satisfactory.

## II. Existing Traffic Conditions

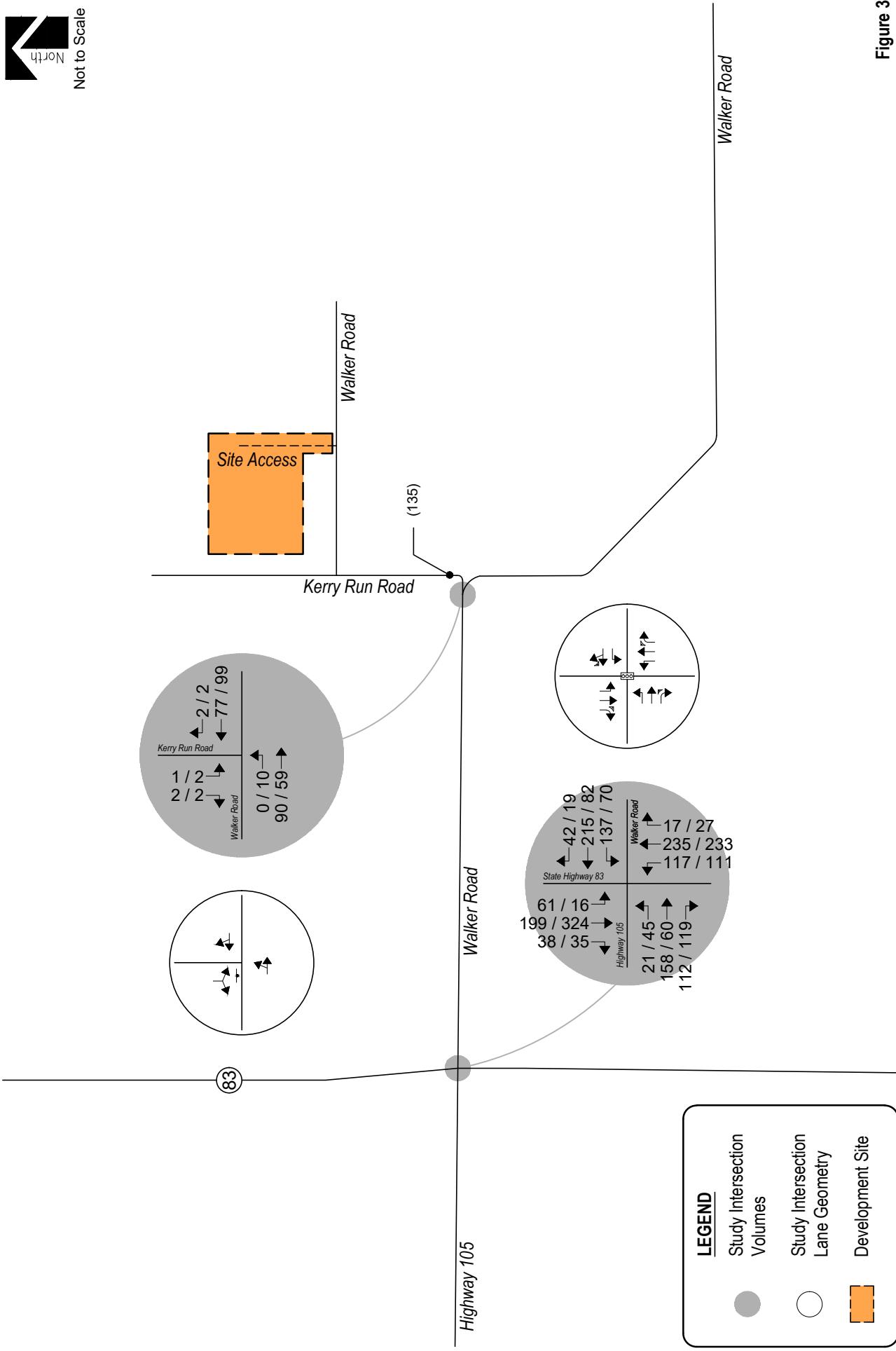
Morning (AM) and afternoon (PM) peak hour traffic counts were collected at the Walker Road intersections with Kerry Run Road and State Highway 83. Average daily traffic (ADT) volumes were collected over a 24-hour period on Kerry Run Road. Counts were collected on Wednesday, January 31, 2024, with AM peak hour counts being collected during the period of 7:00 a.m. to 9:00 a.m. and PM peak hour counts being collected during the period of 4:00 p.m. to 6:00 p.m.

Existing volumes and intersection geometry are shown on Figure 3. Traffic count data is included for reference in Appendix A.

Existing signal timing parameters for State Highway 83 and Walker Road were obtained from CDOT and used throughout this study to the best extent possible in order to remain consistent with existing signal coordination plans. CDOT signal timing information received is included for reference in Appendix A.



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**Figure 3**  
**EXISTING TRAFFIC**  
Volumes & Intersection Geometry  
AM / PM Peak Hour  
(ADT) : Average Daily Traffic

### **Peak Hour Intersection Levels of Service – Existing Traffic**

The Signalized and Unsignalized Intersection Analysis techniques, as published in the Highway Capacity Manual (HCM), 6<sup>th</sup> Edition, by the Transportation Research Board and as incorporated into the SYNCHRO computer program, were used to analyze the study intersections for existing and future traffic conditions. These nationally accepted techniques allow for the determination of intersection level of service (LOS) based on the congestion and delay of each traffic movement.

Pursuant to Section B.4.1.A of the County's ECM, the design objective for each scenario of this study shall be level of service "D". Level of service is a method of measurement used by transportation professionals to quantify a driver's perception of travel conditions that include travel time, number of stops, and total amount of stopped delay experienced on a roadway network. The HCM categorizes level of service into a range from "A" which indicates little, if any, vehicle delay, to "F" which indicates a level of operation considered unacceptable to most drivers. These levels of service grades with brief descriptions of the operating condition, for unsignalized and signalized intersections, are included for reference in Appendix B and have been used throughout this study.

The level of service analyses results for existing conditions are summarized in Table 1.

Intersection capacity worksheets developed for this study are provided in Appendix C.

**Table 1 – Intersection Capacity Analysis Summary – Existing Traffic**

INTERSECTION LANE GROUPS	LEVEL OF SERVICE	
	AM PEAK HOUR	PM PEAK HOUR
State Highway 83 / Highway 105 / Walker Road (Signalized)	C (20.6)	B (15.3)
Walker Road / Kerry Run Road (Stop-Controlled) Eastbound Left and Through Southbound Left and Right	A A	A A

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)  
Stop-Controlled Intersection: Level of Service

### **Existing Traffic Analysis Results**

Under existing conditions, operational analysis shows that the signalized intersection of State Highway 83 with Highway 105 and Walker Road has overall operations at LOS C during the morning peak traffic hour and LOS B during the afternoon peak traffic hour.

The unsignalized intersection of Walker Road and Kerry Run Road has turning movement operations at LOS A during the morning and afternoon peak traffic hours.

### III. Future Traffic Conditions Without Proposed Development

Background traffic is the traffic projected to be on area roadways without consideration of the proposed development. Background traffic includes traffic generated by development of vacant parcels in the area.

To account for projected increases in background traffic for Years 2026 and 2040, a compounded annual growth rate was determined using historical traffic data for the surrounding area provided by CDOT's Online Transportation Information System (OTIS) along the adjacent segment of State Highway 83 which shows a 20-year growth rate between one and two percent. Therefore, in order to provide for a conservative analysis, a growth rate of approximately two percent was applied to existing traffic volumes.

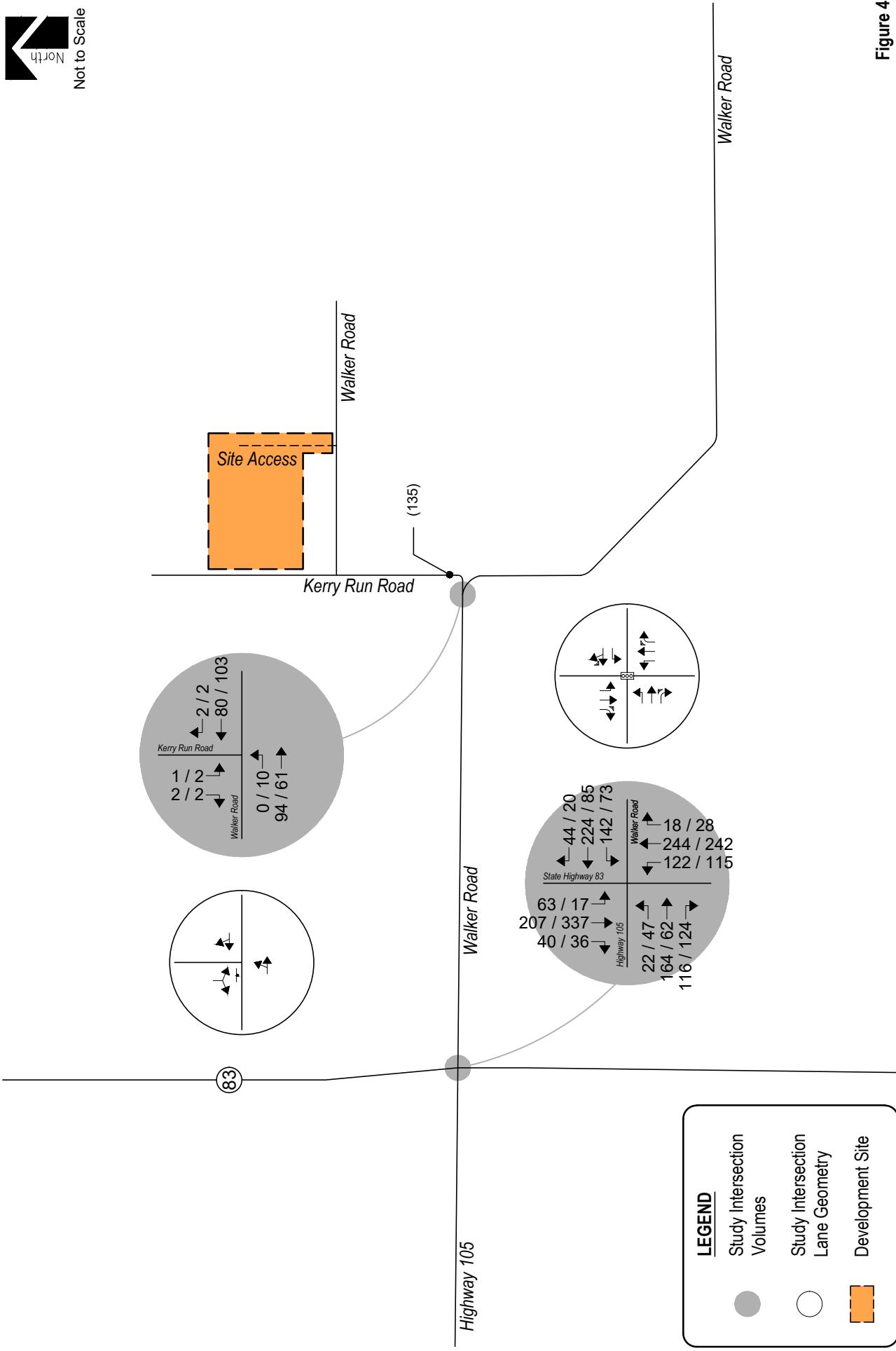
It is important to note that ingress and egress traffic volumes along Kerry Run Road are not subject to annual growth patterns since this access drive does not provide connection to other roadways, therefore does not serve regional traffic.

As discussed in Section I, the expansion of Highway 105 and Walker Road from two through lanes to four through lanes is planned in order to accommodate regional transportation demands. However, in order to provide a conservative analysis, Year 2026 and Year 2040 background traffic conditions assume no roadway improvements to accommodate regional transportation demands. Year 2040 does, however, assume existing signal timing parameters for State Highway 83 and Walker Road with optimized intersection splits in effort to better long-term intersection performance.

Projected background traffic volumes and intersection geometry for Years 2026 and 2040 are shown on Figure 4 and Figure 5, respectively.



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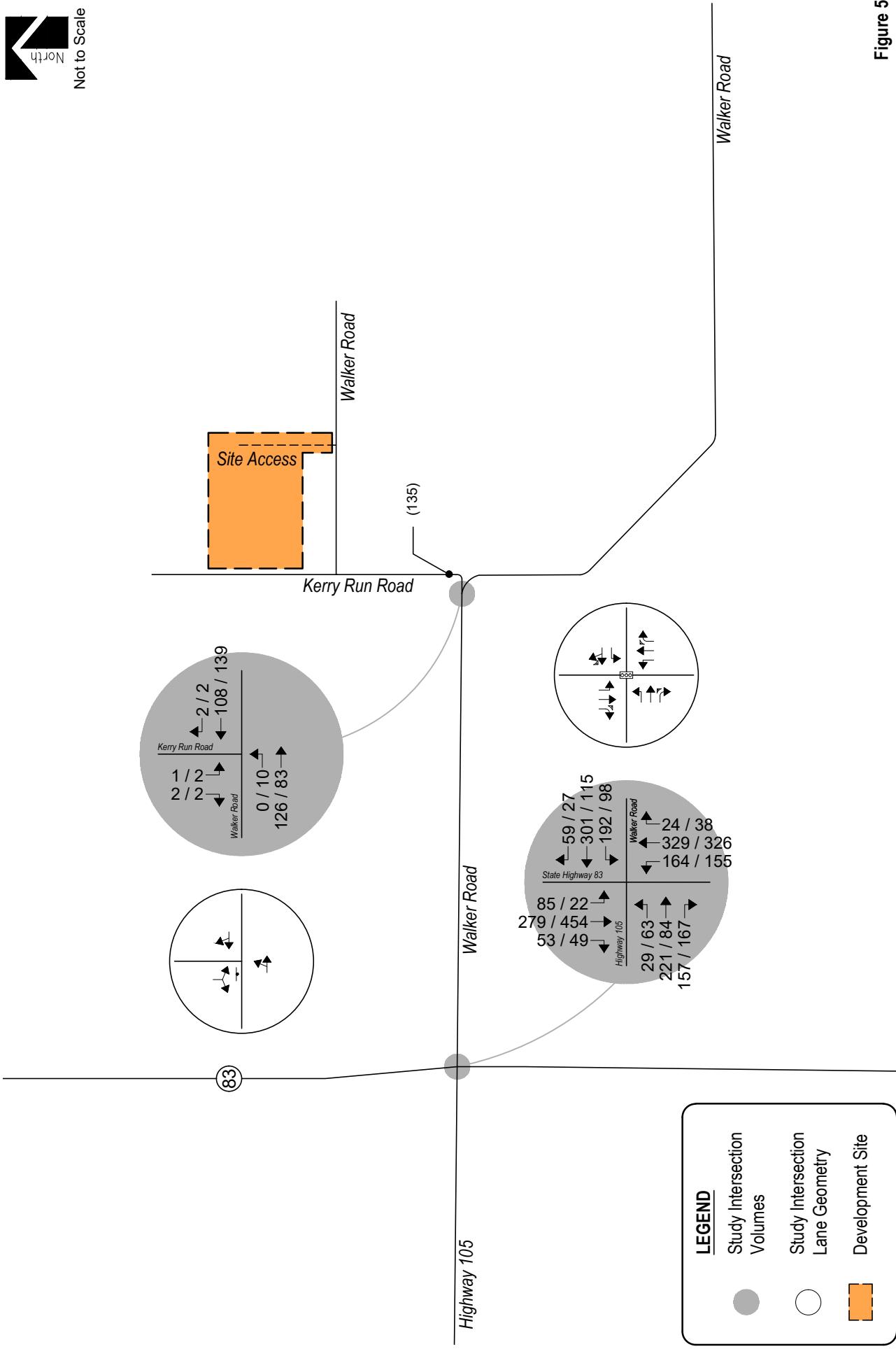
**Figure 4**  
**BACKGROUND TRAFFIC - YEAR 2026**  
Volumes & Intersection Geometry

AM / PM Peak Hour  
(ADT) : Average Daily Traffic

June 2024  
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### Peak Hour Intersection Levels of Service – Background Traffic

As with existing traffic conditions, the operations of study intersections were analyzed under background conditions, without the proposed development, using the SYNCHRO computer program.

Background traffic level of service analysis results for Year 2026 are listed in Table 2. Year 2040 operational results are summarized in Table 3.

Definitions of levels of service are given in Appendix B. Intersection capacity worksheets are provided in Appendix C.

**Table 2 – Intersection Capacity Analysis Summary – Background Traffic – Year 2026**

INTERSECTION LANE GROUPS	LEVEL OF SERVICE	
	AM PEAK HOUR	PM PEAK HOUR
State Highway 83 / Highway 105 / Walker Road (Signalized)	C (21.2)	B (15.5)
Walker Road / Kerry Run Road (Stop-Controlled)		
Eastbound Left and Through	A	A
Southbound Left and Right	A	A

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)

Stop-Controlled Intersection: Level of Service

### Background Traffic Analysis Results – Year 2026

Year 2026 background traffic analysis indicates that the signalized intersection of State Highway 83 with Highway 105 and Walker Road continues to project overall operations at LOS C during the AM peak traffic hour and LOS B during the PM peak traffic hour.

The unsignalized intersection of Walker Road and Kerry Run Road continues to have turning movement operations at LOS A during both AM and PM peak traffic periods.

**Table 3 – Intersection Capacity Analysis Summary – Background Traffic – Year 2040**

INTERSECTION LANE GROUPS	LEVEL OF SERVICE	
	AM PEAK HOUR	PM PEAK HOUR
State Highway 83 / Highway 105 / Walker Road (Signalized)	C (24.1)	B (18.1)
Walker Road / Kerry Run Road (Stop-Controlled)		
Eastbound Left and Through	A	A
Southbound Left and Right	A	A

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)

Stop-Controlled Intersection: Level of Service

### Background Traffic Analysis Results – Year 2040

By Year 2040 and without the proposed development, the study intersection of State Highway 83 with Highway 105 and Walker Road continues to experience LOS C operations during the AM peak traffic hour and LOS B operations during the PM peak traffic hour.

The unsignalized intersection of Walker Road and Kerry Run Road continues to project turning movement operations at LOS A during both the morning and afternoon peak traffic hour.

These intersection operations are similar to existing conditions.

## IV. Proposed Project Traffic

### Trip Generation

Standard traffic generation characteristics compiled by the Institute of Transportation Engineers (ITE) in their report entitled Trip Generation Manual, 11<sup>th</sup> Edition, were considered to the proposed land use in order to estimate average daily traffic (ADT), AM Peak Hour, and PM Peak Hour vehicle trips. A vehicle trip is defined as a one-way vehicle movement from a point of origin to a point of destination.

Proposed facility operations, as described by the developer, were instead evaluated to estimate average daily and weekday peak hour trip information for the proposed land use. It is expected that the proposed development will have a maximum capacity of 95 attendees supported by a total of 38 off-street parking spaces.

Using the above information, the maximum number of daily and peak hour trips were then calculated. Considering how religious land uses are considered as family destinations, a single-occupant vehicle (SOV) rate of 4.0 was used.

Table 4 illustrates projected ADT, AM Peak Hour, and PM Peak Hour traffic volumes likely generated by the proposed development upon build-out.

**Table 4 – Trip Generation Summary**

ITE CODE	LAND USE	SIZE	TOTAL TRIPS GENERATED									
			24 HOUR	AM PEAK HOUR			PM PEAK HOUR			ENTER	EXIT	TOTAL
				ENTER	EXIT	TOTAL	ENTER	EXIT	TOTAL			
-	Religious Institution	95 ATT	48	24	0	24	0	24	24			
		Total:	48	24	0	24	0	24	24			

Key: ATT = Attendees.

Note: All data and calculations above are subject to being rounded to nearest value.

Upon build-out, Table 4 illustrates that the proposed development has the potential to generate approximately 48 daily vehicle trips with 24 of those occurring during the morning peak hour and 24 during the afternoon peak hour. It is important to note that the above volumes are based on the expected worst-case conditions that occur during a typical weekday. Furthermore, it is understood that the proposed development will not always be at maximum occupancy, nor are operations expected to occur outside of peak hours of adjacent street traffic.

Unresolved: Per ECM section B.3.3.A, trip generation shall be calculated from the latest data contained within the Institute of Transportation Engineers' Trip Generation Manual. Please provide a basis for the trip generation. Coordinate with the applicant to provide more clarity as to the intended uses so that a more accurate trip generation can be provided. Per meeting on 8-13-2024, please fully define the parameters of the use moving forward, submit with a comparable use from the ITE, pinpoint exactly what you are asking the County to approve. Phasing of the project could be used as a limitation.

### **Adjustments to Trip Generation Rates**

A development of this type is not likely to attract trips from within area land uses nor pass-by or diverted link trips from the adjacent roadway system, therefore no trip reduction was taken in this analysis.

### **Trip Distribution**

The overall directional distribution of site-generated traffic was determined based on the location of development site within the County, proposed and existing area land uses, allowed turning movements, available roadway network, and in reference to distribution patterns of existing traffic count data.

Overall trip distribution patterns for the development are shown on Figure 6.

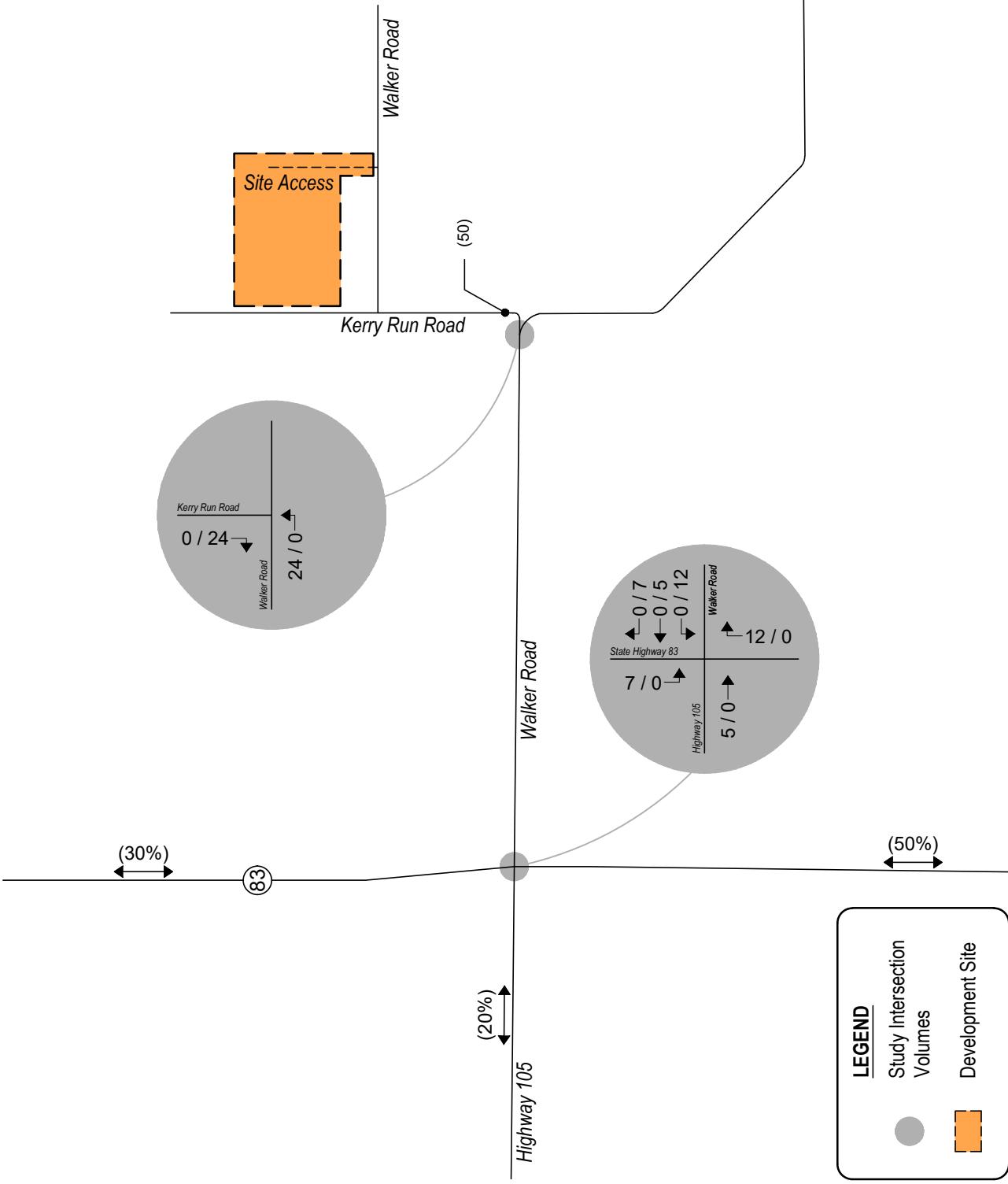
### **Trip Assignment**

Traffic assignment is how generated and distributed vehicle trips are expected to be loaded onto the available roadway network.

Applying trip distribution patterns to site-generated traffic provides the overall site-generated trip assignments shown on Figure 6.



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**Figure 6**  
**SITE DEVELOPMENT DISTRIBUTION (%) :** Overall  
**SITE-GENERATED TRIPS**  
AM / PM Peak Hour

## V. Future Traffic Conditions With Proposed Developments

Total traffic is the traffic projected to be on area roadways with consideration of the proposed development. Total traffic includes background traffic projections for Years 2026 and 2040 with consideration of site-generated traffic. For analysis purposes, it was assumed that development construction would be completed by end of Year 2026.

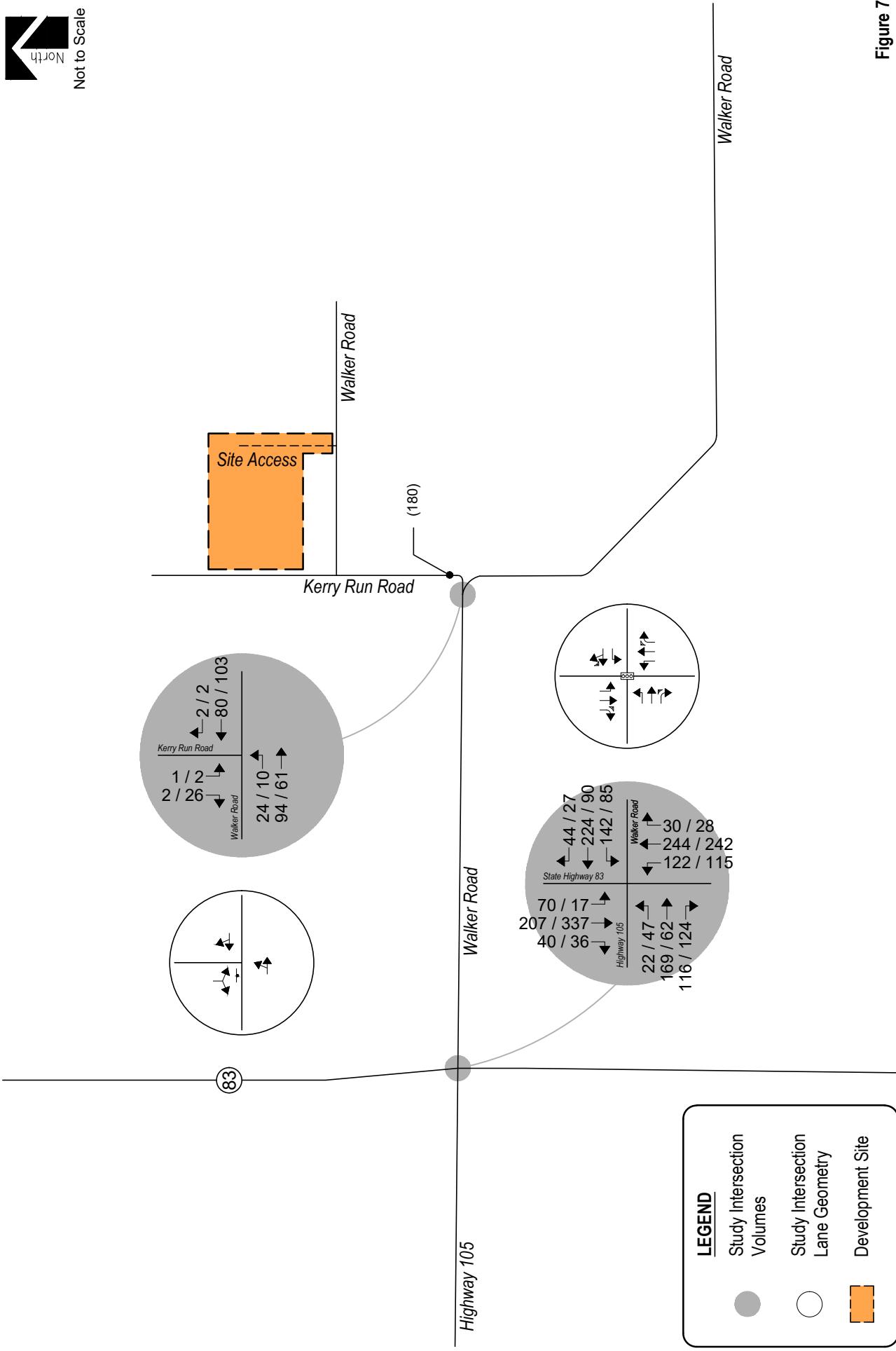
Pursuant to area roadway improvement discussions provided in Section III, Year 2026 total traffic conditions assume no roadway improvements to accommodate regional transportation demands. Year 2040 total traffic conditions assume that Highway 105 and Walker Road are expanded to four through lanes to accommodate regional transportation demands. Roadway improvements associated with site development are expected to be limited to site access and frontage as required by the governing agency.

Projected Year 2026 total traffic volumes and intersection geometry are shown in Figure 7.

Figure 8 shows projected total traffic volumes and intersection geometry for Year 2040.



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**Figure 7**  
**TOTAL TRAFFIC - YEAR 2026**  
Volumes & Intersection Geometry

AM / PM Peak Hour  
(ADT) : Average Daily Traffic

June 2024  
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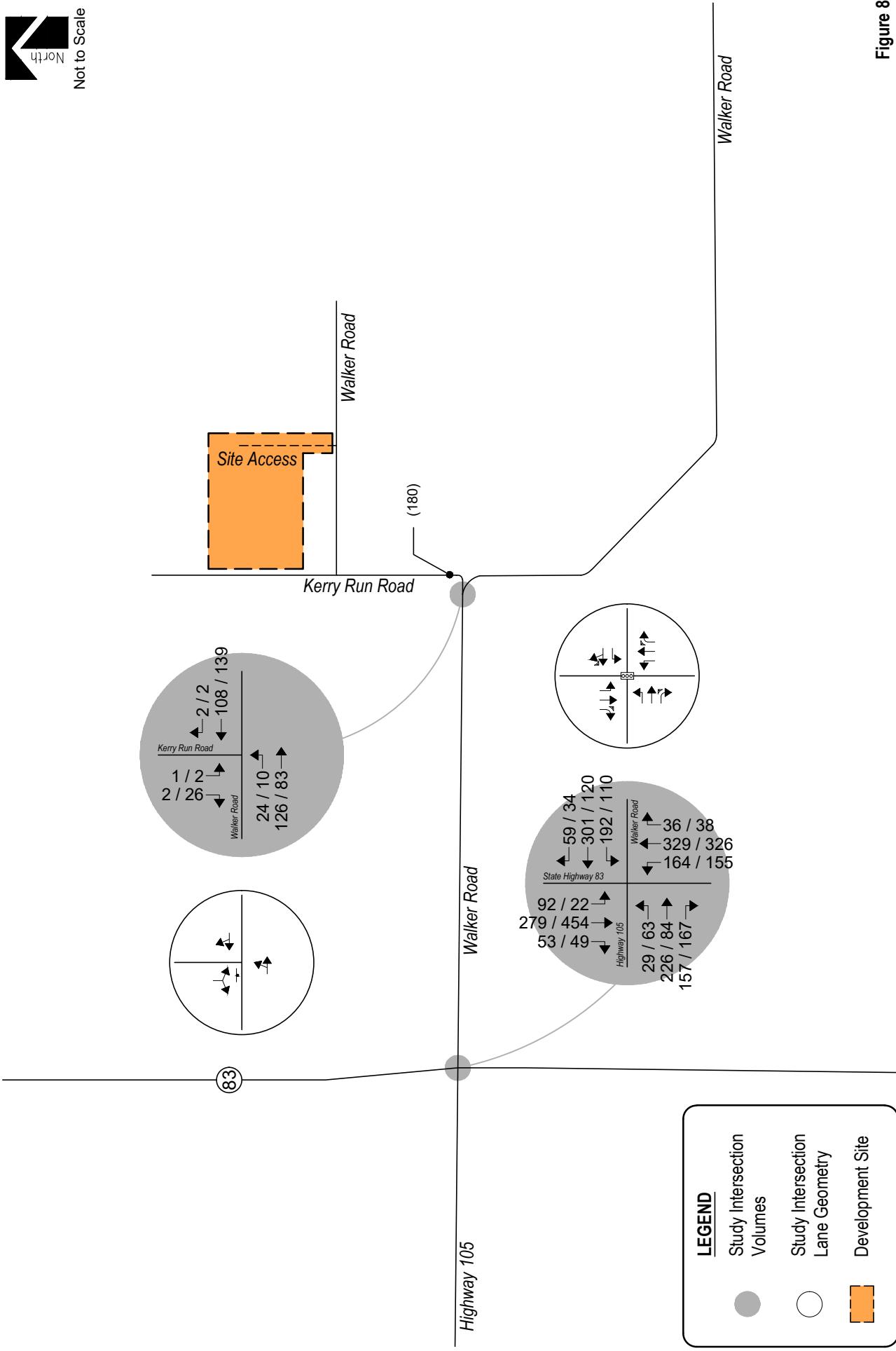


Figure 8  
**TOTAL TRAFFIC - YEAR 2040**  
Volumes & Intersection Geometry  
AM / PM Peak Hour  
(ADT) : Average Daily Traffic

## VI. Project Impacts

The analyses and procedures described in this study were performed in accordance with the latest (HCM) and are again based upon the worst-case conditions that occur during a typical weekday upon build-out of site development and analyzed land uses. Therefore, study intersections are likely to operate with traffic conditions better than those described within this study, which represent the peak hours of weekday operations only.

### **Peak Hour Intersection Levels of Service – Total Traffic**

As with background traffic, the operations of the study intersections were analyzed under projected total traffic conditions using the SYNCHRO computer program. Total traffic level of service analysis results for Years 2026 and 2040 are summarized in Table 5 and Table 6, respectively.

Definitions of levels of service are given in Appendix B. Intersection capacity worksheets are provided in Appendix C.

**Table 5 – Intersection Capacity Analysis Summary – Total Traffic – Year 2026**

INTERSECTION LANE GROUPS	LEVEL OF SERVICE	
	AM PEAK HOUR	PM PEAK HOUR
State Highway 83 / Highway 105 / Walker Road (Signalized)	C (21.1)	B (15.8)
Walker Road / Kerry Run Road (Stop-Controlled) Eastbound Left and Through Southbound Left and Right	A A	A A

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)  
Stop-Controlled Intersection: Level of Service

**Table 6 – Intersection Capacity Analysis Summary – Total Traffic – Year 2040**

INTERSECTION LANE GROUPS	LEVEL OF SERVICE	
	AM PEAK HOUR	PM PEAK HOUR
State Highway 83 / Highway 105 / Walker Road (Signalized)	C (24.1)	B (18.6)
Walker Road / Kerry Run Road (Stop-Controlled) Eastbound Left and Through Southbound Left and Right	A A	A A

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)  
Stop-Controlled Intersection: Level of Service

## Total Traffic Analysis Results Upon Development Build-Out

Table 6 illustrates how, by Year 2040 and upon development build-out, the signalized intersection of State Highway 83 with Highway 105 and Walker Road continues to show an overall LOS C operation during the morning peak traffic hour and LOS B operation during the afternoon peak traffic hour.

The stop-controlled intersection of Walker Road and Kerry Run Road continues to project turning movement operations at LOS A for the morning and afternoon peak traffic hours.

These intersection operations are similar to existing and background conditions.

### Queue Length Analysis

Queue lengths for the study intersections were analyzed using Year 2040 total traffic conditions. The analysis yields estimate of 95<sup>th</sup> percentile queue lengths, which have only a five percent probability of being exceeded during the analysis time period. An average vehicle length of 25 feet was assumed. Queue lengths were modeled and are included with the Synchro worksheets in Appendix C.

Table 7 summarizes the 95<sup>th</sup> percentile queue results in comparison to the projected storage requirements for turn movements within study area for Year 2040.

**Table 7 – Turn Lane Queues and Storage Requirements – Total Traffic – Year 2040**

Intersection	Turn Movement	Existing Turn Lane Length (feet)	AM Peak Hour		PM Peak Hour		Recommended Turn Lane Length (feet)	
			95th Percentile Queue Length (feet)	Vehicle Equivalent (vehicles)	95th Percentile Queue Length (feet)	Vehicle Equivalent (vehicles)		
Signalized Intersections								
State Highway 83 / Highway 105 / Walker Road	EB	L	250'	28'	2	59'	3	250'
		T	-	194'	8	91'	4	-
		R	350'	0'	0	0'	0	350'
	WB	L	270'	128'	6	94'	4	270'
		T,R	-	332'	14	143'	6	-
	NB	L	410'	91'	4	63'	3	410'
		T	-	240'	10	189'	8	-
		R	370'	0'	0	0'	0	370'
	SB	L	430'	55'	3	14'	1	430'
		T	-	200'	8	295'	12	-
		R	430'	0'	0	0'	0	430'
Signalized Intersections								
Walker Road / Kerry Run Road	EB	L,T	-	3'	1	0'	0	-
	WB	T,R	-	0'	0	0'	0	-
	SB	L,R	-	0'	0	3'	1	-

Note: Turn Lane Length does not include taper length.

As Table 7 shows, all turn lane lengths have sufficient storage to accommodate future traffic volumes.

## Total Traffic Auxiliary Lane Analysis

Auxiliary lanes for Walker Road are to be based on the County's ECM.

Considering development build-out, an evaluation of auxiliary lane requirements, pursuant to Section 2.3.7.D of the County's ECM, reveals that a left turn deceleration lane at Kerry Run Road along Walker Road is not required since the projected peak hour left turn ingress volume is less than the County's threshold of 25 vehicles per hour.

## Recommended Improvements

Roadway adequacy for Walker Road and Kerry Run Road were assessed pursuant to the County's ECM. As discussed in Section I, Walker Road and Kerry Run Road adjacent to the property have rural, local, gravel roadway classifications.

Pursuant to Section 2.3.2, Table 2-5 of the County's ECM, rural, local, gravel roadways have a design ADT of 200 trips/day. Therefore, in review of long-term total traffic volumes along Walker Road and Kerry Run Road, no roadway improvements are necessary nor recommended as projected ADT volumes are within the County's 200 trips/day threshold.

Furthermore, pursuant to projected intersection operations illustrated in Tables 6 and 7, no improvements to the existing Kerry Run Road and Walker Road intersection are recommended nor believed to be needed in order to improve operations. Anticipated level of service and 95<sup>th</sup> percentile queuing results indicate the study intersection will have long-term operations within the County's expectations.

Unresolved: Is the private road up to adequate standard to accommodate the additional traffic generated without an improvements?

Unresolved: This will need to be determined once a use has been defined and the trip generation is more representative for the property

## VII. Conclusion

This traffic impact study addressed the capacity, geometric, and control requirements associated with the development entitled 3980 Walker Road. This proposed development consists of an approximate 11,100 square foot modular building intended to be used as a religious institution supporting a maximum of 95 attendees, as allowed within the County's RR-5 zoning district. The development is located near the northeast corner of Walker Road and Kerry Run Road in El Paso County, Colorado.

The study area to be examined in this analysis encompassed the segment of Walker Road bounded by State Highway 83 and Kerry Run Road

Analysis was conducted for critical AM Peak Hour and PM Peak Hour traffic operations for existing traffic conditions, Year 2026 and Year 2040 background traffic conditions, and Year 2026 and Year 2040 total traffic conditions.

Analysis of existing traffic conditions indicates that the signalized intersection of Walker Road with State Highway 83 and Highway 105 has operations at LOS C or better during morning and afternoon peak traffic hours.

The stop-controlled intersection of Walker Road and Kerry Run Road has turning movement operations at LOS A during the morning and afternoon peak traffic hour.

Without the proposed development, Year 2026 background operational analysis shows that the signalized intersection of Walker Road with State Highway 83 and Highway 105 continues to project operations at LOS C or better during morning and afternoon peak traffic hours.

The stop-controlled intersection of Walker Road and Kerry Run Road continues to project turning movement operations at LOS A during the morning and afternoon peak traffic hours.

By Year 2040 and without the proposed development, the signalized intersection of Walker Road with State Highway 83 and Highway 105 continues to expect overall operations at LOS C or better during the morning and afternoon peak traffic hours.

The stop-controlled intersection of Walker Road and Kerry Run Road continues to expect turning movement operations at LOS A during the morning and afternoon peak traffic hours.

Analysis of future traffic conditions indicates that the addition of site-generated traffic is expected to create no negative impact to traffic operations for the existing and surrounding roadway system upon roadway improvements assumed within this analysis. With all conservative assumptions defined in this analysis, the study intersections are projected to operate at future levels of service comparable to Year 2040 background traffic conditions. Kerry Run Road has long-term operations at LOS A during peak traffic periods and upon build-out.

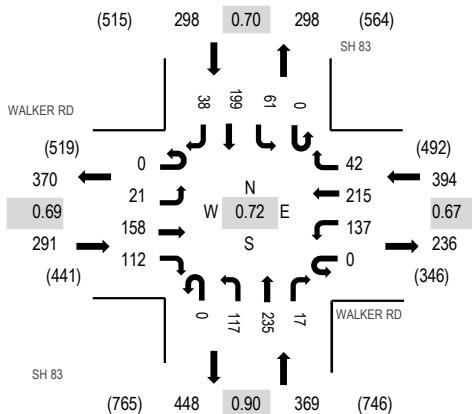
**APPENDIX A**

**Traffic Count Data**

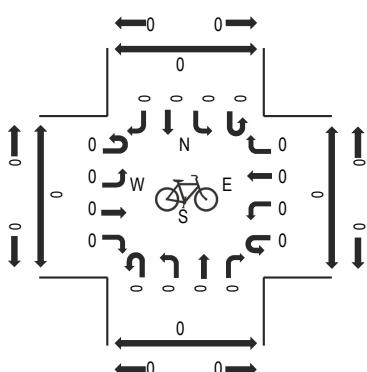
**Signal Timing Information**

**Location:** 1 SH 83 & WALKER RD AM  
**Date:** Wednesday, January 31, 2024  
**Peak Hour:** 07:00 AM - 08:00 AM  
**Peak 15-Minutes:** 07:30 AM - 07:45 AM

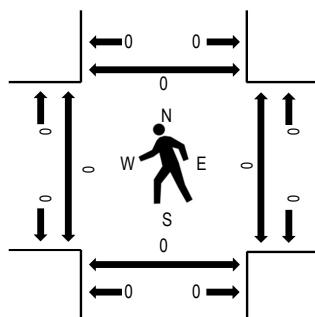
### Peak Hour - Motorized Vehicles



### Peak Hour - Bicycles



### Peak Hour - Pedestrians



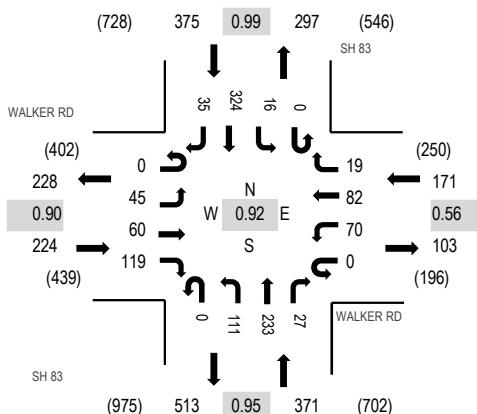
Note: Total study counts contained in parentheses.

### Traffic Counts - Motorized Vehicles

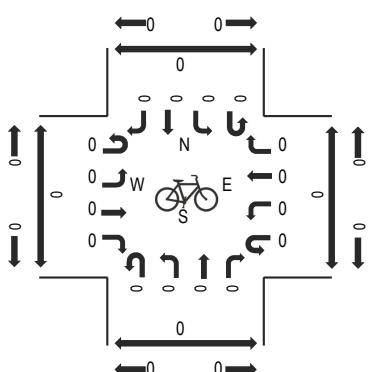
Interval Start Time	WALKER RD Eastbound				WALKER RD Westbound				SH 83 Northbound				SH 83 Southbound				Rolling Hour	Pedestrian Crossings				
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	West	East	South	North	
7:00 AM	0	7	11	22	0	11	17	2	0	24	56	1	0	5	45	9	210	1,352	0	0	0	0
7:15 AM	0	4	50	26	0	13	42	15	0	25	50	4	0	14	48	17	308	1,334	0	0	0	0
7:30 AM	0	6	72	28	0	49	87	12	0	35	69	3	0	35	64	7	467	1,222	0	0	0	0
7:45 AM	0	4	25	36	0	64	69	13	0	33	60	9	0	7	42	5	367	963	0	0	0	0
8:00 AM	0	5	8	29	0	8	8	4	0	24	45	9	0	2	47	3	192	842	0	0	0	0
8:15 AM	0	4	13	16	0	7	7	3	0	27	65	7	0	4	39	4	196		0	0	0	0
8:30 AM	0	4	10	21	0	11	5	3	0	22	60	15	0	6	46	5	208		0	0	0	0
8:45 AM	0	4	11	25	0	20	14	8	0	25	61	17	0	8	48	5	246		0	0	0	0
Count Total	0	38	200	203	0	183	249	60	0	215	466	65	0	81	379	55	2,194		0	0	0	0
Peak Hour	0	21	158	112	0	137	215	42	0	117	235	17	0	61	199	38	1,352		0	0	0	0

**Location:** 1 SH 83 & WALKER RD PM  
**Date:** Wednesday, January 31, 2024  
**Peak Hour:** 04:00 PM - 05:00 PM  
**Peak 15-Minutes:** 04:00 PM - 04:15 PM

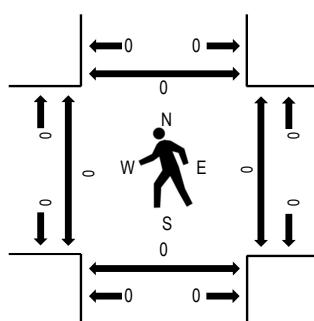
### Peak Hour - Motorized Vehicles



### Peak Hour - Bicycles



### Peak Hour - Pedestrians



Note: Total study counts contained in parentheses.

### Traffic Counts - Motorized Vehicles

Interval Start Time	WALKER RD Eastbound				WALKER RD Westbound				SH 83 Northbound				SH 83 Southbound				Rolling Hour	Pedestrian Crossings				
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	Total	West	East	South	North	
4:00 PM	0	11	17	32	0	29	39	8	0	31	53	7	0	3	74	5	309	1,141	0	0	0	0
4:15 PM	0	16	13	21	0	14	12	3	0	28	53	12	0	3	87	6	268	1,103	0	0	0	0
4:30 PM	0	8	12	31	0	18	17	6	0	26	59	3	0	3	83	13	279	1,071	0	0	0	0
4:45 PM	0	10	18	35	0	9	14	2	0	26	68	5	0	7	80	11	285	1,036	0	0	0	0
5:00 PM	0	9	16	37	0	7	6	2	0	25	63	8	0	8	81	9	271	978	0	0	0	0
5:15 PM	0	8	10	33	0	9	7	1	0	32	43	9	0	1	75	8	236	0	0	0	0	0
5:30 PM	0	9	9	33	0	11	13	6	0	26	40	7	0	5	76	9	244	0	0	0	0	0
5:45 PM	0	18	11	22	0	9	5	3	0	25	47	6	0	3	69	9	227	0	0	0	0	0
Count Total	0	89	106	244	0	106	113	31	0	219	426	57	0	33	625	70	2,119	0	0	0	0	0
Peak Hour	0	45	60	119	0	70	82	19	0	111	233	27	0	16	324	35	1,141	0	0	0	0	0

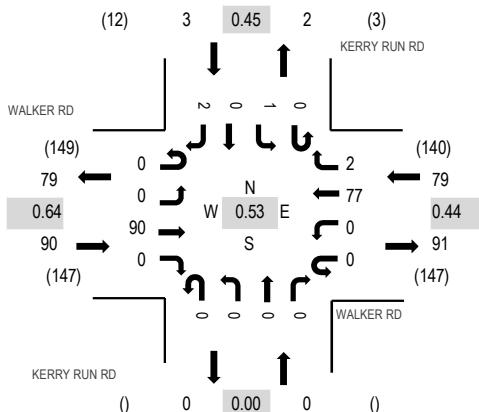
**Location:** 2 KERRY RUN RD & WALKER RD AM

**Date:** Wednesday, January 31, 2024

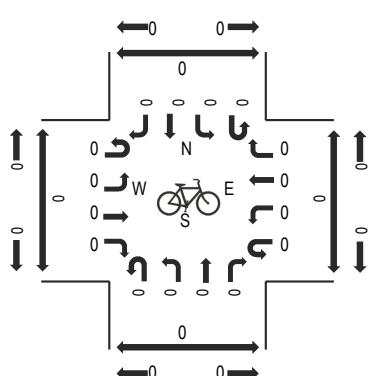
**Peak Hour:** 08:00 AM - 09:00 AM

**Peak 15-Minutes:** 08:45 AM - 09:00 AM

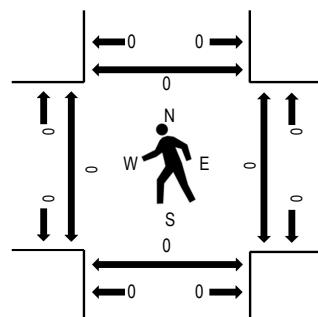
### Peak Hour - Motorized Vehicles



### Peak Hour - Bicycles



### Peak Hour - Pedestrians



Note: Total study counts contained in parentheses.

### Traffic Counts - Motorized Vehicles

Interval Start Time	WALKER RD Eastbound				WALKER RD Westbound				KERRY RUN RD Northbound				KERRY RUN RD Southbound				Rolling Hour	Pedestrian Crossings				
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		West	East	South	North	
7:00 AM	0	0	3	0	0	0	13	0	0	0	0	0	0	0	0	0	18	127	0	0	0	
7:15 AM	0	0	12	0	0	0	25	0	0	0	0	0	0	0	0	0	37	135	0	0	0	
7:30 AM	0	0	12	0	0	0	14	0	0	0	0	0	0	0	0	0	31	126	0	0	0	
7:45 AM	0	1	29	0	0	0	9	0	0	0	0	0	0	0	0	0	41	132	0	0	0	
8:00 AM	0	0	15	0	0	0	9	0	0	0	0	0	0	0	0	0	26	172	0	0	0	
8:15 AM	0	0	18	0	0	0	10	0	0	0	0	0	0	0	0	0	28	0	0	0	0	
8:30 AM	0	0	22	0	0	0	13	2	0	0	0	0	0	0	0	0	37	0	0	0	0	
8:45 AM	0	0	35	0	0	0	45	0	0	0	0	0	0	0	1	0	0	81	0	0	0	0
Count Total	0	1	146	0	0	0	138	2	0	0	0	0	0	1	0	11	299	0	0	0	0	
Peak Hour	0	0	90	0	0	0	77	2	0	0	0	0	0	1	0	2	172	0	0	0	0	

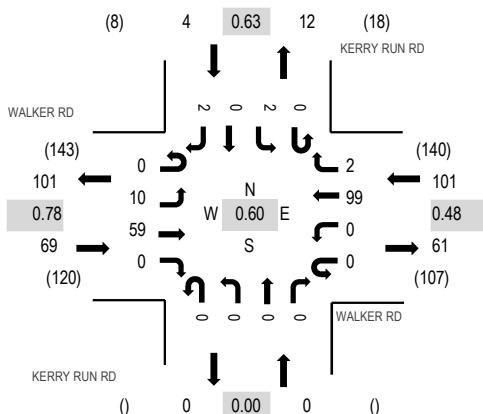
**Location:** 2 KERRY RUN RD & WALKER RD PM

**Date:** Wednesday, January 31, 2024

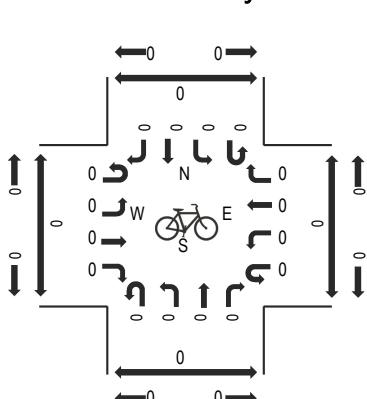
**Peak Hour:** 04:00 PM - 05:00 PM

**Peak 15-Minutes:** 04:00 PM - 04:15 PM

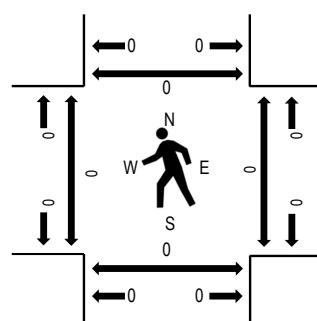
### Peak Hour - Motorized Vehicles



### Peak Hour - Bicycles



### Peak Hour - Pedestrians



Note: Total study counts contained in parentheses.

### Traffic Counts - Motorized Vehicles

Interval Start Time	WALKER RD Eastbound				WALKER RD Westbound				KERRY RUN RD Northbound				KERRY RUN RD Southbound				Rolling Hour	Pedestrian Crossings			
	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right	U-Turn	Left	Thru	Right		West	East	South	North
4:00 PM	0	1	19	0	0	0	52	1	0	0	0	0	0	0	0	0	73	174	0	0	0
4:15 PM	0	3	14	0	0	0	18	0	0	0	0	0	0	1	0	1	37	125	0	0	0
4:30 PM	0	3	7	0	0	0	19	1	0	0	0	0	0	0	0	0	30	112	0	0	0
4:45 PM	0	3	19	0	0	0	10	0	0	0	0	0	0	0	1	0	34	110	0	0	0
5:00 PM	0	2	13	0	0	0	9	0	0	0	0	0	0	0	0	0	24	94	0	0	0
5:15 PM	0	0	12	0	0	0	9	1	0	0	0	0	0	0	0	2	24	0	0	0	0
5:30 PM	0	1	14	0	0	0	12	0	0	0	0	0	0	0	1	1	28	0	0	0	0
5:45 PM	0	2	7	0	0	0	8	0	0	0	0	0	0	0	0	1	18	0	0	0	0
Count Total	0	15	105	0	0	0	137	3	0	0	0	0	0	2	0	6	268	0	0	0	0
Peak Hour	0	10	59	0	0	0	99	2	0	0	0	0	0	2	0	2	174	0	0	0	0

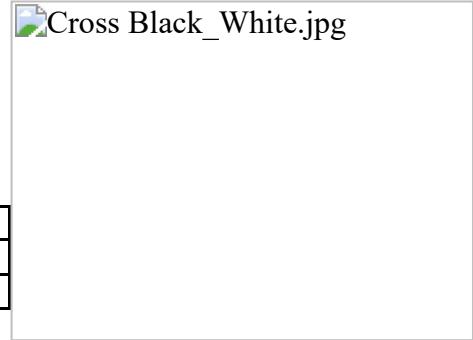
Site Code: 3  
 Station ID: 3  
 KERRY RUN RD N.O. WALKER RD

Start Time	31-Jan-24 Wed	NB	SB	Total
12:00 AM		0	0	0
01:00		0	0	0
02:00		0	0	0
03:00		0	0	0
04:00		0	1	1
05:00		0	2	2
06:00		0	3	3
07:00		1	9	10
08:00		2	3	5
09:00		7	6	13
10:00		5	7	12
11:00		2	1	3
12:00 PM		10	5	15
01:00		5	7	12
02:00		0	7	7
03:00		5	4	9
04:00		12	4	16
05:00		6	4	10
06:00		2	1	3
07:00		3	3	6
08:00		2	0	2
09:00		4	0	4
10:00		0	0	0
11:00		0	0	0
Total	66	67	67	133
Percent	49.6%	50.4%	50.4%	
AM Peak Vol.	-	09:00	07:00	-
PM Peak Vol.	-	16:00	13:00	-
Grand Total Percent	66	67	67	
	49.6%	50.4%	50.4%	
ADT	ADT 133			AADT 133

## MaxTime Timing Sheet

2.0.16 update

## **Administration**



## Unit Information

Controller ID	0
Main St.	Hwy 83
Side St.	Walker

Adapter	IP Address	Subnet Mask	Default Gateway	ARP	Mode
1				Disable	Static
2				Disable	Static

## **Serial Ports:**

Port	Description	Function	Address	Baud	Bits	Stop	Parity	Flow	CTS	RTS
1	Port 2/C21S	None	1	9600	8	1	None	None	0	0
2	Aux P3/C22S	None	1	9600	8	1	None	None	0	0
3	SDLC Port 1	None	1	9600	8	1	None	None	0	0
4	Com A/C50S	None	1	9600	8	1	None	None	0	0
5	FIO	None	1	9600	8	1	None	None	0	0
6	DISPLAY/C60M	None	1	9600	8	1	None	None	0	0
7	SP7	None	1	9600	8	1	None	None	0	0
8	SP8/Com B	None	1	9600	8	1	None	None	0	0

## Unit Parameters

Startup Flash	0	Auto Ped Clr	Enable	Red Revert	4.0	Backup Time	600	Ext Mode	Disable
All Red Exit	0	Grn Flash Freq.	60	Yel Flash Freq.	60	MCE Enable	Enable	Free Seq.	1
MCE Seq.	1	Start Yellow	0.0	Start Red	0.0	Start Clear Hold	6		
Master By TOD	Disable	Flash CVM	Disable	All Red Flash	Enable	3 Phs Dia Seq			
				Auto Ped Clear	Enable	4 Phs Dia Seq			

## Phase Parameters

Ped Service Limit	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Pre Green	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Pre Clearance	0.0	6.0	0.0	0.0	0.0	6.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

Phases	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
Pre Clearance 2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Clear Ext Max	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Red Clear Ext Pass	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Queue Jump	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Adv Warning Ext	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

## Phase Options

Phases	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
Enable	X	X	X	X	X	X	X	X												
Auto Flash Ent.		X				X														
Auto Flash Exit		X				X														
Non Actuated I																				
Non Actuated II																				
Non Lock Mem	X	X	X	X	X	X	X	X												
Min Veh Recall																				
Max Veh Recall																				
Ped Recall																				
Soft Veh Recall																				
Dual Entry			X				X													
Sim Gap Dis																				
Guaranteed Pass																				
Act Rest Walk																				
Cond Service																				
Add Initial																				
Ped Clr During Yel																				
Ped Clr During Red																				
Cond Reservice																				
Yel Min Override																				
No Startup Call																				
Adv. Warn Flasher		X				X														
No Ped Str Up Call																				
Ped Clr OVTG																				
Flash Exit Call																				
Flash Exit Ped Call																				
MinGreen2																				
MaxGreen2																				
MaxGreen3																				
Ped2																				
Ped Clear Pre Clear																				
Ped NA+ Mode																				
Red Rest																				
Serve Evy Oth Even																				
Serve Evy Oth Odd																				
Coord Ped Yield																				
Ped Recycle																				
Coutdown																				

## No Serve Phases

Sequence 1		Sequence 2		Sequence 3		Sequence 4	
Ph.	No Serve Phases						
1		1		1		1	
2		2		2		2	

3
4
5
6
7
8

3
4
5
6
7
8

3
4
5
6
7
8

3
4
5
6
7
8

Sequence 1
9
10
11
12
13
14
15
16

Sequence 2
9
10
11
12
13
14
15
16

Sequence 3
9
10
11
12
13
14
15
16

Sequence 4
9
10
11
12
13
14
15
16

### Phase Configuration

Ph.	Startup	Ring	Concurrent	Startup Min	Description
1	Phase Not On	1	5,6	0	SB LT
2	Green No Walk	1	5,6	0	NB
3	Phase Not On	1	7,8	0	WB LT
4	Phase Not On	1	7,8	0	EB
5	Phase Not On	2	1,2	0	NB LT
6	Green No Walk	2	1,2	0	SB
7	Phase Not On	2	3,4	0	EB LT
8	Phase Not On	2	3,4	0	WB
9	None	0		0	
10	None	0		0	
11	None	0		0	
12	None	0		0	
13	None	0		0	
14	None	0		0	
15	None	0		0	
16	None	0		0	
17	None	0		0	
18	None	0		0	
19	None	0		0	
20	None	0		0	

### Sequence Configuration

Sequence 1	
Ring	Phases
1	1,2,a,3,4,b
2	5,6,a,7,8,b
3	
4	
5	
6	
7	
8	
9	
10	
11	
12	
13	
14	

Sequence 2	
Ring	Phases
1	2,1,a,3,4,b
2	5,6,a,7,8,b
3	
4	
5	
6	
7	
8	
9	
10	
11	
12	
13	
14	

Sequence 3	
Ring	Phases
1	1,2,a,4,3,b
2	5,6,a,7,8,b
3	
4	
5	
6	
7	
8	
9	
10	
11	
12	
13	
14	

Sequence 4	
Ring	Phases
1	2,1,a,4,3,b
2	5,6,a,7,8,b
3	
4	
5	
6	
7	
8	
9	
10	
11	
12	
13	
14	





16	6	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
17	6	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
18	6	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
19	6	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
20	6	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
21	3	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
22	8	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
23	8	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
24	8	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
25	8	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
26	8	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
27	5	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
28	7	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
29	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
30	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
31	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0
32	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0

Det.	Call	Call	Call	Add Call	Sw			Queue	Ext	No	Max	Erratic	Failed	Failed	Fail	Description
	Phs	Ped	Ovl			Delay	Extend	Limit	Hold	Activity	Pres	Counts	Time	Recall	Link	
33	1	0	0		0	0.0	0.0	0	0.0	0	0	0	15	Min	0	
34	2	0	0		0	0.0	0.0	0	0.0	0	0	0	15	Min	0	
35	3	0	0		0	0.0	0.0	0	0.0	0	0	0	15	Min	0	
36	4	0	0		0	0.0	0.0	0	0.0	0	0	0	15	Min	0	
37	5	0	0		0	0.0	0.0	0	0.0	0	0	0	15	Min	0	
38	6	0	0		0	0.0	0.0	0	0.0	0	0	0	15	Min	0	
39	7	0	0		0	0.0	0.0	0	0.0	0	0	0	15	Min	0	
40	8	0	0		0	0.0	0.0	0	0.0	0	0	0	15	Min	0	
41	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
42	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
43	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
44	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
45	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
46	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
47	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
48	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
49	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
50	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
51	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
52	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
53	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
54	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
55	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
56	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
57	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
58	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
59	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
60	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
61	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
62	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
63	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
64	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
65	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
66	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
67	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
68	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
69	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	
70	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	None	0	

71	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0	
72	0	0	0		0	0.0	0.0	0	0.0	0	0	0	0	0	None	0	

**Vehicle Detection Options**

Detector	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20
Volume Detector																				
Occupancy																				
Yellow Lock Call																				
Red Lock call																				
Extend	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	
Added Initial																				
Queue																				
Call	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	
Terminate																				
Min Green 2																				
Protected Perm																				
Disable Dly Lead																				
Disable TS2 Diag																				

Detector	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40
Volume Detector																				
Occupancy																				
Yellow Lock Call																				
Red Lock call																				
Extend	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	
Added Initial																				
Queue																				
Call	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	
Terminate																				
Min Green 2																				
Protected Perm																				
Disable Dly Lead																				
Disable TS2 Diag																				

Detector	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60
Volume Detector																				
Occupancy																				
Yellow Lock Call																				
Red Lock call																				
Extend	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	
Added Initial																				
Queue																				
Call	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	X	
Terminate																				
Min Green 2																				
Protected Perm																				
Disable Dly Lead																				
Disable TS2 Diag																				

Detector	61	62	63	64	65	66	67	68	69	70	71	72
Volume Detector												
Occupancy												
Yellow Lock Call												
Red Lock call												
Extend	X	X	X	X	X	X	X	X	X	X	X	X
Added Initial												
Queue												

Data Collection Period	0
Number of Periods	1

Call	X	X	X	X	X	X	X	X	X	X	X	X	X
Terminate													
Min Green 2													
Protected Perm													
Disable Dly Lead													
Disable TS2 Diag													

### Speed Detectors

Det	Enable	Type	Units	Min	Max	Car	Det	Trail	Trap
				Log	Log	Length	Length	Det	Length
1		Single	Inches	5	80	0	0	0	0
2		Single	Inches	5	80	0	0	0	0
3		Single	Inches	5	80	0	0	0	0
4		Single	Inches	5	80	0	0	0	0
5		Single	Inches	5	80	0	0	0	0
6		Single	Inches	5	80	0	0	0	0
7		Single	Inches	5	80	0	0	0	0
8		Single	Inches	5	80	0	0	0	0

### Pedestrian Detectors

Det	Call	Call	Add	Walk	Clear	No	Max	Erratic
	Phs	Ovlp	Call Phs	2	2	Act	Pres	Count
1	0	0		0	0	0	0	0
2	2	0		0	0	0	0	0
3	0	0		0	0	0	0	0
4	4	0		0	0	0	0	0
5	0	0		0	0	0	0	0
6	6	0		0	0	0	0	0
7	0	0		0	0	0	0	0
8	8	0		0	0	0	0	0
9	0	0		0	0	0	0	0
10	0	0		0	0	0	0	0
11	0	0		0	0	0	0	0
12	0	0		0	0	0	0	0
13	0	0		0	0	0	0	0
14	0	0		0	0	0	0	0
15	0	0		0	0	0	0	0
16	0	0		0	0	0	0	0
17	0	0		0	0	0	0	0
18	0	0		0	0	0	0	0
19	0	0		0	0	0	0	0
20	0	0		0	0	0	0	0

Det	Call	Call	Add	Walk	Clear	No	Max	Erratic
	Phs	Ovlp	Call Phs	2	2	Act	Pres	Count
21	0	0		0	0	0	0	0
22	0	0		0	0	0	0	0
23	0	0		0	0	0	0	0
24	0	0		0	0	0	0	0
25	0	0		0	0	0	0	0
26	0	0		0	0	0	0	0
27	0	0		0	0	0	0	0
28	0	0		0	0	0	0	0
29	0	0		0	0	0	0	0
30	0	0		0	0	0	0	0
31	0	0		0	0	0	0	0
32	0	0		0	0	0	0	0
33	0	0		0	0	0	0	0
34	0	0		0	0	0	0	0
35	0	0		0	0	0	0	0
36	0	0		0	0	0	0	0
37	0	0		0	0	0	0	0
38	0	0		0	0	0	0	0
39	0	0		0	0	0	0	0
40	0	0		0	0	0	0	0

### Pri/Pre Detectors

Det	Description	Low Call	High Call	Low	high	Lead/Trail	Arrival	Pri	Ext	Min	Pri	No	Max	Erratic
				Num	Num			Dlay		On	Olvp	Act	Pres	Count
1		None	None	0	0	None	0	0	0	0	0	0	0	0
2		None	None	0	0	None	0	0	0	0	0	0	0	0
3		None	None	0	0	None	0	0	0	0	0	0	0	0
4		None	None	0	0	None	0	0	0	0	0	0	0	0
5		None	None	0	0	None	0	0	0	0	0	0	0	0
6		None	None	0	0	None	0	0	0	0	0	0	0	0
7		None	None	0	0	None	0	0	0	0	0	0	0	0
8		None	None	0	0	None	0	0	0	0	0	0	0	0

### Overlaps

OLP	Enabled	Type	Included Phs	Modifier Phs	Modifier Ovlps	Neg Phases	Inhibit Neg Phs	Neg Ovlps
1	Enabled	FYA - 4 Sec	2	1				
2	Enabled	Off						
3	Enabled	FYA - 4 Sec	4	3				

4	Enabled	Off					
5	Enabled	FYA - 4 Sec	6	5			
6	Enabled	Off					
7	Enabled	FYA - 4 Sec	8	7			
8	Enabled	Off					
9	Enabled	Off					
10	Enabled	Off					
11	Enabled	Off					
12	Enabled	Off					
13	Enabled	Off					
14	Enabled	Off					
15	Enabled	Off					
16	Enabled	Off					

OLP	TrG Omit Phs	Negative Peds	Neg Ped Ovlps	Grn Sup Phs	N Ped Phs Calls	Description
1						
2						
3						
4						
5						
6						

OLP	TrG Omit Phs	Negative Peds	Neg Ped Ovlps	Grn Sup Phs	N Ped Phs Calls	Description
7						
8						
9						
10						
11						
12						
13						
14						
15						
16						

OLP	Min	Phase Control							Mx Grn	Red Revert	Flash Inactive	Flash Alt	Walk Rest			
		Trail GRN	Trail YEL	Trail RED	Walk 1	Ped Clr 1	Walk 2	Ped Clr 2	Delay	Flash						
1	0	0.0	0.0	0	0	0	0	0	0.0	On	0	0	0.0	Off	Off	Off
2	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off
3	0	0.0	0.0	0	0	0	0	0	0.0	On	0	0	0.0	Off	Off	Off
4	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off
5	0	0.0	0.0	0	0	0	0	0	0.0	On	0	0	0.0	Off	Off	Off
6	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off
7	0	0.0	0.0	0	0	0	0	0	0.0	On	0	0	0.0	Off	Off	Off
8	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off
9	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off
10	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off
11	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off
12	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off
13	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off
14	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off
15	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off
16	0	0.0	0.0	0	0	0	0	0	0.0	Off	0	0	0.0	Off	Off	Off

No Veh Reserv					FYA Prot. Red Cl					
No Hold Trail Exit					Phs Intvl Override					
Ped Recycle					Queue Jump					
No Yellow Protect					No FYA Ped Wlk					
No Bridging					Term After Call					
LRT Prepare Go										

### Custom Overlap Rules

Rule	Custom Ovlp	Incl. State	Mod. State	Neg. State	Output	Flash
1	Disable	Any	Any	Any	Not Set	Not Set
2	Disable	Any	Any	Any	Not Set	Not Set
3	Disable	Any	Any	Any	Not Set	Not Set
4	Disable	Any	Any	Any	Not Set	Not Set
5	Disable	Any	Any	Any	Not Set	Not Set
6	Disable	Any	Any	Any	Not Set	Not Set
7	Disable	Any	Any	Any	Not Set	Not Set
8	Disable	Any	Any	Any	Not Set	Not Set
9	Disable	Any	Any	Any	Not Set	Not Set
10	Disable	Any	Any	Any	Not Set	Not Set

### Coordination Parameters

Operational Mode	Manual Free	Maximum Mode	Per Pattern	Max Cyc Limit %	15
Coordination Mode	Pattern	Force Mode	Per Pattern	Min Cyc Limit %	15
Correction Mode	Shortway (Auto)	Transition Cover Ped	Pattern	Max Dwell	0

Patterns	Offset					Ref Col	Coord Mode	Force Mode	Max Mode	Trans Ped	Min Perm	Phs Pln	Det Pln	Ped Pln	Ovlp Pln	Pri Pln	Description	
	Patt.	Cycle	1	2	3	Split	Seq											
1	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
2	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
3	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
4	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
5	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
6	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
7	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
8	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
9	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
10	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
11	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
12	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
13	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
14	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
15	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
16	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
17	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
18	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
19	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1
20	0	0	0	0	0	0	0	Yel	Auto	Fixed	Inh	Phase	Phs Only	1	1	1	1	1

### Split Parameters

Split 1				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode			Pri Min	Pri Max	Pri F. Off
PH.	Time	Min	Max										
1	0	0	0				Fix	None			0	0	Float
2	0	0	0				Fix	None			0	0	Float
3	0	0	0				Fix	None			0	0	Float
4	0	0	0				Fix	None			0	0	Float
5	0	0	0				Fix	None			0	0	Float
6	0	0	0				Fix	None			0	0	Float
7	0	0	0				Fix	None			0	0	Float

8	0	0	0						Fix	None	0	0	Float
9	0	0	0						Fix	None	0	0	Float
10	0	0	0						Fix	None	0	0	Float
11	0	0	0						Fix	None	0	0	Float
12	0	0	0						Fix	None	0	0	Float
13	0	0	0						Fix	None	0	0	Float
14	0	0	0						Fix	None	0	0	Float
15	0	0	0						Fix	None	0	0	Float
16	0	0	0						Fix	None	0	0	Float

Split 2				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode	Pri	Pri	Pri	
PH.	Time	Min	Max						Min	Max	F. Off	
1	0	0	0					Fix	None	0	0	Float
2	0	0	0					Fix	None	0	0	Float
3	0	0	0					Fix	None	0	0	Float
4	0	0	0					Fix	None	0	0	Float
5	0	0	0					Fix	None	0	0	Float
6	0	0	0					Fix	None	0	0	Float
7	0	0	0					Fix	None	0	0	Float
8	0	0	0					Fix	None	0	0	Float
9	0	0	0					Fix	None	0	0	Float
10	0	0	0					Fix	None	0	0	Float
11	0	0	0					Fix	None	0	0	Float
12	0	0	0					Fix	None	0	0	Float
13	0	0	0					Fix	None	0	0	Float
14	0	0	0					Fix	None	0	0	Float
15	0	0	0					Fix	None	0	0	Float
16	0	0	0					Fix	None	0	0	Float

Split 3				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode	Pri	Pri	Pri	
PH.	Time	Min	Max						Min	Max	F. Off	
1	0	0	0					Fix	None	0	0	Float
2	0	0	0					Fix	None	0	0	Float
3	0	0	0					Fix	None	0	0	Float
4	0	0	0					Fix	None	0	0	Float
5	0	0	0					Fix	None	0	0	Float
6	0	0	0					Fix	None	0	0	Float
7	0	0	0					Fix	None	0	0	Float
8	0	0	0					Fix	None	0	0	Float
9	0	0	0					Fix	None	0	0	Float
10	0	0	0					Fix	None	0	0	Float
11	0	0	0					Fix	None	0	0	Float
12	0	0	0					Fix	None	0	0	Float
13	0	0	0					Fix	None	0	0	Float
14	0	0	0					Fix	None	0	0	Float
15	0	0	0					Fix	None	0	0	Float
16	0	0	0					Fix	None	0	0	Float

Split 4				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode	Pri	Pri	Pri	
PH.	Time	Min	Max						Min	Max	F. Off	
1	0	0	0					Fix	None	0	0	Float
2	0	0	0					Fix	None	0	0	Float
3	0	0	0					Fix	None	0	0	Float
4	0	0	0					Fix	None	0	0	Float
5	0	0	0					Fix	None	0	0	Float
6	0	0	0					Fix	None	0	0	Float
7	0	0	0					Fix	None	0	0	Float

8	0	0	0				Fix	None	0	0	Float
9	0	0	0				Fix	None	0	0	Float
10	0	0	0				Fix	None	0	0	Float
11	0	0	0				Fix	None	0	0	Float
12	0	0	0				Fix	None	0	0	Float
13	0	0	0				Fix	None	0	0	Float
14	0	0	0				Fix	None	0	0	Float
15	0	0	0				Fix	None	0	0	Float
16	0	0	0				Fix	None	0	0	Float

Split 5				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode	Pri	Pri	Pri
PH.	Time	Min	Max						Min	Max	F. Off
1	0	0	0				Fix	None	0	0	Float
2	0	0	0				Fix	None	0	0	Float
3	0	0	0				Fix	None	0	0	Float
4	0	0	0				Fix	None	0	0	Float
5	0	0	0				Fix	None	0	0	Float
6	0	0	0				Fix	None	0	0	Float
7	0	0	0				Fix	None	0	0	Float
8	0	0	0				Fix	None	0	0	Float
9	0	0	0				Fix	None	0	0	Float
10	0	0	0				Fix	None	0	0	Float
11	0	0	0				Fix	None	0	0	Float
12	0	0	0				Fix	None	0	0	Float
13	0	0	0				Fix	None	0	0	Float
14	0	0	0				Fix	None	0	0	Float
15	0	0	0				Fix	None	0	0	Float
16	0	0	0				Fix	None	0	0	Float

Split 6				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode	Pri	Pri	Pri
PH.	Time	Min	Max						Min	Max	F. Off
1	0	0	0				Fix	None	0	0	Float
2	0	0	0				Fix	None	0	0	Float

Split 6				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode	Pri	Pri	Pri
PH.	Time	Min	Max						Min	Max	F. Off
3	0	0	0				Fix	None	0	0	Float
4	0	0	0				Fix	None	0	0	Float
5	0	0	0				Fix	None	0	0	Float
6	0	0	0				Fix	None	0	0	Float
7	0	0	0				Fix	None	0	0	Float
8	0	0	0				Fix	None	0	0	Float
9	0	0	0				Fix	None	0	0	Float
10	0	0	0				Fix	None	0	0	Float
11	0	0	0				Fix	None	0	0	Float
12	0	0	0				Fix	None	0	0	Float
13	0	0	0				Fix	None	0	0	Float
14	0	0	0				Fix	None	0	0	Float
15	0	0	0				Fix	None	0	0	Float
16	0	0	0				Fix	None	0	0	Float

Split 7				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode	Pri	Pri	Pri
PH.	Time	Min	Max						Min	Max	F. Off
1	0	0	0				Fix	None	0	0	Float
2	0	0	0				Fix	None	0	0	Float
3	0	0	0				Fix	None	0	0	Float
4	0	0	0				Fix	None	0	0	Float
5	0	0	0				Fix	None	0	0	Float

6	0	0	0				Fix	None	0	0	Float
7	0	0	0				Fix	None	0	0	Float
8	0	0	0				Fix	None	0	0	Float
9	0	0	0				Fix	None	0	0	Float
10	0	0	0				Fix	None	0	0	Float
11	0	0	0				Fix	None	0	0	Float
12	0	0	0				Fix	None	0	0	Float
13	0	0	0				Fix	None	0	0	Float
14	0	0	0				Fix	None	0	0	Float
15	0	0	0				Fix	None	0	0	Float
16	0	0	0				Fix	None	0	0	Float

Split 8				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode	Pri	Pri	Pri
PH.	Time	Min	Max						Min	Max	F. Off
1	0	0	0				Fix	None	0	0	Float
2	0	0	0				Fix	None	0	0	Float
3	0	0	0				Fix	None	0	0	Float
4	0	0	0				Fix	None	0	0	Float
5	0	0	0				Fix	None	0	0	Float
6	0	0	0				Fix	None	0	0	Float
7	0	0	0				Fix	None	0	0	Float
8	0	0	0				Fix	None	0	0	Float
9	0	0	0				Fix	None	0	0	Float
10	0	0	0				Fix	None	0	0	Float
11	0	0	0				Fix	None	0	0	Float
12	0	0	0				Fix	None	0	0	Float
13	0	0	0				Fix	None	0	0	Float
14	0	0	0				Fix	None	0	0	Float
15	0	0	0				Fix	None	0	0	Float
16	0	0	0				Fix	None	0	0	Float

Split 9				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode	Pri	Pri	Pri
PH.	Time	Min	Max						Min	Max	F. Off
1	0	0	0				Fix	None	0	0	Float
2	0	0	0				Fix	None	0	0	Float
3	0	0	0				Fix	None	0	0	Float
4	0	0	0				Fix	None	0	0	Float

Split 9				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode	Pri	Pri	Pri
PH.	Time	Min	Max						Min	Max	F. Off
5	0	0	0				Fix	None	0	0	Float
6	0	0	0				Fix	None	0	0	Float
7	0	0	0				Fix	None	0	0	Float
8	0	0	0				Fix	None	0	0	Float
9	0	0	0				Fix	None	0	0	Float
10	0	0	0				Fix	None	0	0	Float
11	0	0	0				Fix	None	0	0	Float
12	0	0	0				Fix	None	0	0	Float
13	0	0	0				Fix	None	0	0	Float
14	0	0	0				Fix	None	0	0	Float
15	0	0	0				Fix	None	0	0	Float
16	0	0	0				Fix	None	0	0	Float

Split 10				Coord PH	Ref PH	Cover Ped	Force Off Mode	Mode	Pri	Pri	Pri
PH.	Time	Min	Max						Min	Max	F. Off
1	0	0	0				Fix	None	0	0	Float
2	0	0	0				Fix	None	0	0	Float











Sp F9					Sp F11					Sp F13					Sp F15				
Sp F10					Sp F12					Sp F14					Sp F16				

### Preemption Configuration

Preempt	1	2	3	4	5	6	7	8
Enabled	Disabled							
Type	Rail Road	Rail Road	Emerg Veh					
Description								
Track Phases								
Track 2 Phases								
Track Overlaps								
Track 2 Overlaps								
Dwell Phase								
Dwell Ped								
Dwell Overlaps								
Cycling Phases								
Cycling Peds								
Cycling Overlaps								
Exit Phases								
Exit Overlaps								
Exit Veh Calls								
Exit Ped Calls								
Recovery Omit Phs								
Max Pres Action	0	0	0	0	0	0	0	0
Exit Type	Exit Phases							
Exit Max Mode	Disabled							

### Cabinet Config

Run ITS on NEMA port 1	No	Enable TS2/ATC Stop Time		Disable TS2 Fault Flash	X
Run ITS on 2070-1C C13S	No	Disable TS2 Startup Call		Disable TS2 Cab. Alarms	X

### IO Modules

IO Mod	TYPE
1	Caltrans 332
2	TS2 DR3 BIU
3	None
4	None
5	None
6	None
7	None
8	None
9	None
10	None

### Channel Configuration

Chan	Ctrl Type	Source	MMU Channel
1	Phs Veh	1	1
2	Phs Veh	2	2
3	Phs Veh	3	3
4	Phs Veh	4	4
5	Phs Veh	5	5
6	Phs Veh	6	6
7	Phs Veh	7	7
8	Phs Veh	8	8
9	Wrn Flash	2	9
10	None	3	10

Chan	Ctrl Type	Source	MMU Channel
11	Wrn Flash	6	11
12	None	7	12
13	None	2	13
14	None	2	14
15	None	6	15
16	None	6	16
17	None	0	17
18	None	0	18
19	None	0	19
20	None	0	20

### Channel Options

Channel	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16
Flash Yellow	X				X											
Flash Red	X		X	X	X		X	X	X	X	X	X				
Alt Flash	X		X	X			X									
Channel	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32
Flash Yellow																
Flash Red																
Alt Flash																

### Phase Intervals

Interval	Description	Red	Yel	Grn	Type
1	Not Act	On	Off	Off	Red

Interval	Description	Red	Yel	Grn	Type
7	Pre Clr	Off	Off	On	Green

2	Dly Grn	On	Off	Off	Red
3	Pre Grn	Off	Off	On	Green
4	Min Grn	Off	Off	On	Green
5	Grn Ext	Off	Off	On	Green
6	Grn Dwell	Off	Off	On	Green

8	Yel Change	Off	On	Off	Yellow
9	Red Clr	On	Off	Off	Red
10	Red Dwell	On	Off	Off	Red
11	Barrier	On	Off	Off	Red
12	Pre Clr 2	Off	Off	Off	Not Def

## Pedestrian Intervals

Interval	Description	DWK	CLR	Wlk	Type
1	Not Active	On	Off	Off	Dont Walk
2	Dly Ped	On	Off	Off	Dont Walk
3	Walk	Off	Off	On	Walk
4	Walk Dwell	Off	Off	On	Walk
5	Fish DWalk	Flash	Off	Off	Ped Clear
6	DWalk	On	Off	Off	Dont Walk

## Alarm Config

Alarm	Name
1	
2	
3	
4	
5	

Alarm	Name
6	
7	
8	
9	
10	

## **Manual Control Phase Groups**

## Prioritor Settings

Enabled	Lock Out Time	PRS Time to Live
No	0	300

Prioritor	Enabled	Priority	Priority Phs	Skipped Phs	Skip Peds	Delay Time	Est Travel	Mx Presence	Output Dly
1	On	0				0	0	0	0
2	On	0				0	0	0	0
3	On	0				0	0	0	0
4	On	0				0	0	0	0
5	On	0				0	0	0	0
6	On	0				0	0	0	0
7	On	0				0	0	0	0
8	On	0				0	0	0	0

Prioritor	Res. Lockout	Free Pri Min	Free Pri Max	Description
1	0	Min Green	Max Green	
2	0	Min Green	Max Green	
3	0	Min Green	Max Green	
4	0	Min Green	Max Green	
5	0	Min Green	Max Green	
6	0	Min Green	Max Green	
7	0	Min Green	Max Green	
8	0	Min Green	Max Green	

Prioritor	1	2	3	4	5	6	7	8
Lock After First Serve								
Pres. Only Check-in								
Extend Walk Rest								

PRS Reservice Times

## Peer Configuration

Ctrl	Peer ID	Device Type	IP address	IP Port	Http Port	Serial Port	Serial Addr.	Master Sect.	P2P TO	Description
1	0	Peer MaxTime		161	80	0	0	0	15	
2	0	Peer MaxTime		161	80	0	0	0	15	
3	0	Peer MaxTime		161	80	0	0	0	15	
4	0	Peer MaxTime		161	80	0	0	0	15	
5	0	Peer MaxTime		161	80	0	0	0	15	
6	0	Peer MaxTime		161	80	0	0	0	15	
7	0	Peer MaxTime		161	80	0	0	0	15	
8	0	Peer MaxTime		161	80	0	0	0	15	
9	0	Peer MaxTime		161	80	0	0	0	15	
10	0	Peer MaxTime		161	80	0	0	0	15	

## Master Section Configuration

Section	Control	Poll	Req #	Fail Time	Algorithm Period	Description
1	None	60	1	300	240	
2	None	60	1	300	240	
3	None	60	1	300	240	
4	None	60	1	300	240	
5	None	60	1	300	240	
6	None	60	1	300	240	
7	None	60	1	300	240	
8	None	60	1	300	240	
9	None	60	1	300	240	
10	None	60	1	300	240	
11	None	60	1	300	240	
12	None	60	1	300	240	
13	None	60	1	300	240	
14	None	60	1	300	240	
15	None	60	1	300	240	
16	None	60	1	300	240	

## User Program Info

Pgrm	Description	Pgrm	Description
1		17	
2		18	
3		19	
4		20	
5		21	
6		22	
7		23	
8		24	
9		25	
10		26	
11		27	
12		28	
13		29	
14		30	
15		31	
16		32	

**APPENDIX B**

**Level of Service Definitions**

The following information is referenced from the [Highway Capacity Manual: A Guide for Multimodal Mobility Analysis](#), 6<sup>th</sup> Edition, Transportation Research Board, 2016: Chapter 19 – Signalized Intersections.

### **Motorized Vehicle Level of Service (LOS) for Signalized Intersections**

Levels of service are defined to represent reasonable ranges in control delay.

**LOS A** Describes operations with a control delay of 10 s/veh or less and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

**LOS B** Describes operations with control delay between 10 and 20 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.

**LOS C** Describes operations with control delay between 20 and 35 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual *cycle failures* (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

**LOS D** Describes operations with control delay between 35 and 55 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

**LOS E** Describes operations with control delay between 55 and 80 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

**LOS F** Describes operations with control delay exceeding 80 s/veh or a volume-to-capacity ratio greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

Control Delay (s/veh)	<u>LOS by Volume-to-Capacity Ratio<sup>a</sup></u>	
	v/c ≤ 1.0	v/c > 1.0
≤ 10	A	F
> 10 – 20	B	F
> 20 – 35	C	F
> 35 – 55	D	F
> 55 – 80	E	F
> 80	F	F

Note: <sup>a</sup>For approach-based and intersectionwide assessments, LOS is defined solely by control delay.

The following information is referenced from the [Highway Capacity Manual: A Guide for Multimodal Mobility Analysis](#), 6<sup>th</sup> Edition, Transportation Research Board, 2016: Chapter 20 – Two-Way Stop-Controlled Intersections, Chapter 21 – All-Way Stop-Controlled Intersections, and Chapter 22 - Roundabouts.

### **Motorized Vehicle Level of Service (LOS) for Unsignalized & Roundabout Intersections**

LOS is a quantitative stratification of performance measure(s) representing quality of service. Quality of service describes how well a transportation facility or service operates from a traveler's perspective. LOS is measured on an A – F scale, with LOS A representing the best operating conditions from a traveler's perspective.

Control Delay (s/veh)	<u>LOS by Volume-to-Capacity Ratio<sup>a</sup></u>	
	v/c ≤ 1.0	v/c > 1.0
0 – 10	A	F
> 10 – 15	B	F
> 15 – 25	C	F
> 25 – 35	D	F
> 35 – 50	E	F
> 50	F	F

Note: The LOS criteria apply to each lane on a given approach and to each approach on the minor street. LOS is not calculated for major-street approaches or for the intersection as a whole.

<sup>a</sup>For approaches and intersectionwide assessment, LOS is defined solely by control delay.

**APPENDIX C**

**Capacity Worksheets**

## Timings

## 1: State Highway 83 &amp; Highway 105/Walker Road

## Existing Traffic Volumes

AM Peak Hour

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	21	158	112	137	215	42	117	235	17	61	199	38
Future Volume (vph)	21	158	112	137	215	42	117	235	17	61	199	38
Satd. Flow (prot)	1770	1863	1583	1770	1816	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.567			0.488			0.616			0.573		
Satd. Flow (perm)	1056	1863	1583	909	1816	0	1147	1863	1583	1067	1863	1583
Satd. Flow (RTOR)				254			11			165		165
Lane Group Flow (vph)	23	172	122	149	280	0	127	255	18	66	216	41
Turn Type	pm+pt	NA	Free	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free	8			2		2	6		6
Detector Phase	7	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	6.0		5.0	6.0		5.0	30.0	30.0	5.0	30.0	30.0
Minimum Split (s)	10.0	12.0		10.0	12.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (s)	12.0	26.0		12.0	26.0		12.0	36.0	36.0	12.0	36.0	36.0
Total Split (%)	14.0%	30.2%		14.0%	30.2%		14.0%	41.9%	41.9%	14.0%	41.9%	41.9%
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0		5.0	7.0	7.0	5.0	7.0	7.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Act Effct Green (s)	21.0	13.7	77.6	25.1	21.5		37.0	29.6	29.6	36.6	29.4	29.4
Actuated g/C Ratio	0.27	0.18	1.00	0.32	0.28		0.48	0.38	0.38	0.47	0.38	0.38
v/c Ratio	0.07	0.52	0.08	0.40	0.55		0.21	0.36	0.03	0.12	0.31	0.06
Control Delay	17.7	35.3	0.1	22.5	29.9		11.6	21.3	0.1	11.0	20.7	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	17.7	35.3	0.1	22.5	29.9		11.6	21.3	0.1	11.0	20.7	0.2
LOS	B	D	A	C	C		B	C	A	B	C	A
Approach Delay		20.5			27.4			17.2			16.1	
Approach LOS		C			C			B			B	
Queue Length 50th (ft)	8	79	0	53	105		28	89	0	14	74	0
Queue Length 95th (ft)	23	138	0	96	216		65	171	0	38	145	0
Internal Link Dist (ft)		557			487			435			486	
Turn Bay Length (ft)	250		350				410		370	430		430
Base Capacity (vph)	362	487	1583	372	514		603	711	706	570	706	702
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.06	0.35	0.08	0.40	0.54		0.21	0.36	0.03	0.12	0.31	0.06

## Intersection Summary

Cycle Length: 86

Actuated Cycle Length: 77.6

Natural Cycle: 75

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.55

## Timings

### 1: State Highway 83 & Highway 105/Walker Road

## Existing Traffic Volumes

AM Peak Hour

Intersection Signal Delay: 20.6

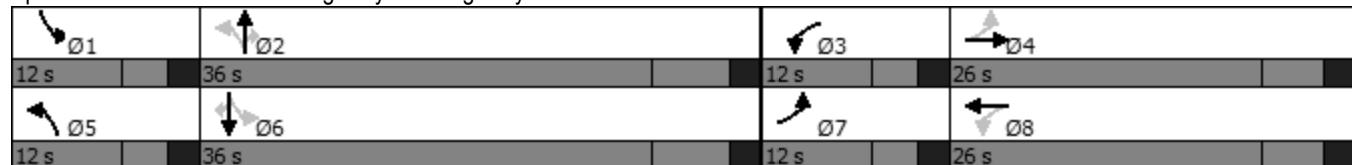
Intersection LOS: C

Intersection Capacity Utilization 68.7%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: State Highway 83 & Highway 105/Walker Road



HCM 6th TWSC  
2: Walker Road & Kerry Run Road

Existing Traffic Volumes  
AM Peak Hour

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	0	90	77	2	1	2
Future Vol, veh/h	0	90	77	2	1	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	98	84	2	1	2
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	86	0	-	0	183	85
Stage 1	-	-	-	-	85	-
Stage 2	-	-	-	-	98	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1510	-	-	-	806	974
Stage 1	-	-	-	-	938	-
Stage 2	-	-	-	-	926	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1510	-	-	-	806	974
Mov Cap-2 Maneuver	-	-	-	-	806	-
Stage 1	-	-	-	-	938	-
Stage 2	-	-	-	-	926	-
Approach	EB	WB	SB			
HCM Control Delay, s	0	0	9			
HCM LOS			A			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1510	-	-	-	911	
HCM Lane V/C Ratio	-	-	-	-	0.004	
HCM Control Delay (s)	0	-	-	-	9	
HCM Lane LOS	A	-	-	-	A	
HCM 95th %tile Q(veh)	0	-	-	-	0	

## Timings

## 1: State Highway 83 &amp; Highway 105/Walker Road

## Existing Traffic Volumes

PM Peak Hour



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	45	60	119	70	82	19	111	233	27	16	324	35
Future Volume (vph)	45	60	119	70	82	19	111	233	27	16	324	35
Satd. Flow (prot)	1770	1863	1583	1770	1809	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.686			0.620			0.453			0.602		
Satd. Flow (perm)	1278	1863	1583	1155	1809	0	844	1863	1583	1121	1863	1583
Satd. Flow (RTOR)			254		13				165			165
Lane Group Flow (vph)	49	65	129	76	110	0	121	253	29	17	352	38
Turn Type	pm+pt	NA	Free	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free	8			2		2	6		6
Detector Phase	7	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	6.0		5.0	6.0		5.0	30.0	30.0	5.0	30.0	30.0
Minimum Split (s)	10.0	12.0		10.0	12.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (s)	12.0	26.0		12.0	26.0		12.0	36.0	36.0	12.0	36.0	36.0
Total Split (%)	14.0%	30.2%		14.0%	30.2%		14.0%	41.9%	41.9%	14.0%	41.9%	41.9%
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0		5.0	7.0	7.0	5.0	7.0	7.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Act Effct Green (s)	14.9	9.2	70.9	16.0	11.5		44.0	42.7	42.7	40.3	35.8	35.8
Actuated g/C Ratio	0.21	0.13	1.00	0.23	0.16		0.62	0.60	0.60	0.57	0.50	0.50
v/c Ratio	0.16	0.27	0.08	0.24	0.36		0.20	0.23	0.03	0.02	0.37	0.04
Control Delay	20.3	32.5	0.1	21.4	29.8		9.0	12.4	0.0	8.2	18.6	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	20.3	32.5	0.1	21.4	29.8		9.0	12.4	0.0	8.2	18.6	0.1
LOS	C	C	A	C	C		A	B	A	A	B	A
Approach Delay		12.8			26.4			10.5			16.4	
Approach LOS		B			C			B			B	
Queue Length 50th (ft)	16	28	0	26	43		24	58	0	3	122	0
Queue Length 95th (ft)	40	63	0	56	89		53	148	0	12	213	0
Internal Link Dist (ft)		557			487			435			486	
Turn Bay Length (ft)	250		350				410		370	430		430
Base Capacity (vph)	328	541	1583	329	534		618	1122	1019	716	941	881
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.12	0.08	0.23	0.21		0.20	0.23	0.03	0.02	0.37	0.04

## Intersection Summary

Cycle Length: 86

Actuated Cycle Length: 70.9

Natural Cycle: 70

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.37

## Timings

### 1: State Highway 83 & Highway 105/Walker Road

## Existing Traffic Volumes

PM Peak Hour

Intersection Signal Delay: 15.3

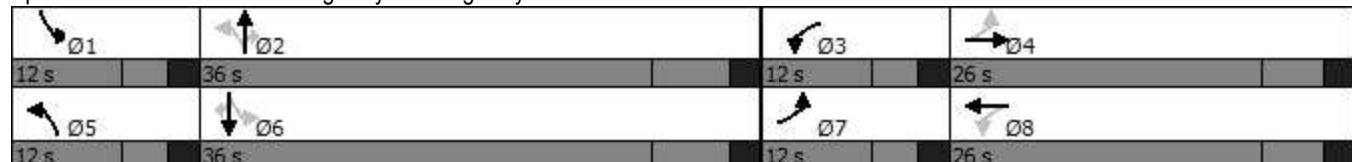
Intersection LOS: B

Intersection Capacity Utilization 56.7%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: State Highway 83 & Highway 105/Walker Road



HCM 6th TWSC  
2: Walker Road & Kerry Run Road

Existing Traffic Volumes  
PM Peak Hour

Intersection						
Int Delay, s/veh	0.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	10	59	99	2	2	2
Future Vol, veh/h	10	59	99	2	2	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	11	64	108	2	2	2
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	110	0	-	0	195	109
Stage 1	-	-	-	-	109	-
Stage 2	-	-	-	-	86	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1480	-	-	-	794	945
Stage 1	-	-	-	-	916	-
Stage 2	-	-	-	-	937	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1480	-	-	-	788	945
Mov Cap-2 Maneuver	-	-	-	-	788	-
Stage 1	-	-	-	-	909	-
Stage 2	-	-	-	-	937	-
Approach	EB	WB	SB			
HCM Control Delay, s	1.1	0	9.2			
HCM LOS			A			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1480	-	-	-	859	
HCM Lane V/C Ratio	0.007	-	-	-	0.005	
HCM Control Delay (s)	7.5	0	-	-	9.2	
HCM Lane LOS	A	A	-	-	A	
HCM 95th %tile Q(veh)	0	-	-	-	0	

## Timings

### 1: State Highway 83 & Highway 105/Walker Road

## Background Traffic Volumes

AM Peak Hour - Year 2026

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	22	164	116	142	224	44	122	244	18	63	207	40
Future Volume (vph)	22	164	116	142	224	44	122	244	18	63	207	40
Satd. Flow (prot)	1770	1863	1583	1770	1816	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.525			0.473			0.573			0.587		
Satd. Flow (perm)	978	1863	1583	881	1816	0	1067	1863	1583	1093	1863	1583
Satd. Flow (RTOR)			254		11				165			165
Lane Group Flow (vph)	24	178	126	154	291	0	133	265	20	68	225	43
Turn Type	pm+pt	NA	Free	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free	8			2		2	6		6
Detector Phase	7	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	6.0		5.0	6.0		5.0	30.0	30.0	5.0	30.0	30.0
Minimum Split (s)	10.0	12.0		10.0	12.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (s)	12.0	26.0		12.0	26.0		12.0	36.0	36.0	12.0	36.0	36.0
Total Split (%)	14.0%	30.2%		14.0%	30.2%		14.0%	41.9%	41.9%	14.0%	41.9%	41.9%
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0		5.0	7.0	7.0	5.0	7.0	7.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Act Effct Green (s)	21.2	14.0	80.1	25.2	21.6		39.3	31.8	31.8	37.6	29.1	29.1
Actuated g/C Ratio	0.26	0.17	1.00	0.31	0.27		0.49	0.40	0.40	0.47	0.36	0.36
v/c Ratio	0.07	0.55	0.08	0.43	0.59		0.23	0.36	0.03	0.12	0.33	0.06
Control Delay	17.8	36.4	0.1	23.4	31.3		11.6	21.2	0.1	11.0	21.4	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	17.8	36.4	0.1	23.4	31.3		11.6	21.2	0.1	11.0	21.4	0.2
LOS	B	D	A	C	C		B	C	A	B	C	A
Approach Delay		21.1			28.6			17.1			16.6	
Approach LOS		C			C			B			B	
Queue Length 50th (ft)	8	82	0	55	111		30	94	0	15	78	0
Queue Length 95th (ft)	23	142	0	98	226		68	177	0	39	151	0
Internal Link Dist (ft)		557			487			435			486	
Turn Bay Length (ft)	250		350				410		370	430		430
Base Capacity (vph)	338	466	1583	355	497		584	739	727	579	677	680
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.38	0.08	0.43	0.59		0.23	0.36	0.03	0.12	0.33	0.06

#### Intersection Summary

Cycle Length: 86

Actuated Cycle Length: 80.1

Natural Cycle: 75

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.59

## Timings

### 1: State Highway 83 & Highway 105/Walker Road

## Background Traffic Volumes

AM Peak Hour - Year 2026

Intersection Signal Delay: 21.2

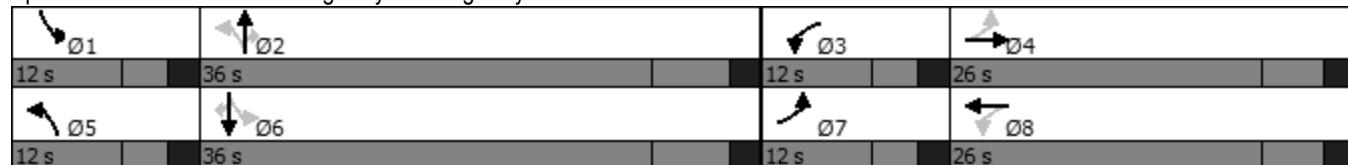
Intersection LOS: C

Intersection Capacity Utilization 69.6%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: State Highway 83 & Highway 105/Walker Road



HCM 6th TWSC  
2: Walker Road & Kerry Run Road

Background Traffic Volumes  
AM Peak Hour - Year 2026

Intersection						
Int Delay, s/veh	0.2					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	0	94	80	2	1	2
Future Vol, veh/h	0	94	80	2	1	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	102	87	2	1	2
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	89	0	-	0	190	88
Stage 1	-	-	-	-	88	-
Stage 2	-	-	-	-	102	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1506	-	-	-	799	970
Stage 1	-	-	-	-	935	-
Stage 2	-	-	-	-	922	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1506	-	-	-	799	970
Mov Cap-2 Maneuver	-	-	-	-	799	-
Stage 1	-	-	-	-	935	-
Stage 2	-	-	-	-	922	-
Approach	EB	WB	SB			
HCM Control Delay, s	0	0	9			
HCM LOS			A			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1506	-	-	-	905	
HCM Lane V/C Ratio	-	-	-	-	0.004	
HCM Control Delay (s)	0	-	-	-	9	
HCM Lane LOS	A	-	-	-	A	
HCM 95th %tile Q(veh)	0	-	-	-	0	

## Timings

## 1: State Highway 83 &amp; Highway 105/Walker Road

## Background Traffic Volumes

PM Peak Hour - Year 2026

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	47	62	124	73	85	20	115	242	28	17	337	36
Future Volume (vph)	47	62	124	73	85	20	115	242	28	17	337	36
Satd. Flow (prot)	1770	1863	1583	1770	1809	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.684			0.611			0.440			0.597		
Satd. Flow (perm)	1274	1863	1583	1138	1809	0	820	1863	1583	1112	1863	1583
Satd. Flow (RTOR)				254		13				165		165
Lane Group Flow (vph)	51	67	135	79	114	0	125	263	30	18	366	39
Turn Type	pm+pt	NA	Free	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free		8		2		2	6		6
Detector Phase	7	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	6.0		5.0	6.0		5.0	30.0	30.0	5.0	30.0	30.0
Minimum Split (s)	10.0	12.0		10.0	12.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (s)	12.0	26.0		12.0	26.0		12.0	36.0	36.0	12.0	36.0	36.0
Total Split (%)	14.0%	30.2%		14.0%	30.2%		14.0%	41.9%	41.9%	14.0%	41.9%	41.9%
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0		5.0	7.0	7.0	5.0	7.0	7.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Act Effct Green (s)	15.0	9.3	70.3	16.1	11.7		43.5	42.3	42.3	39.8	35.5	35.5
Actuated g/C Ratio	0.21	0.13	1.00	0.23	0.17		0.62	0.60	0.60	0.57	0.50	0.50
v/c Ratio	0.16	0.27	0.09	0.25	0.37		0.21	0.23	0.03	0.03	0.39	0.04
Control Delay	20.3	32.5	0.1	21.4	29.9		9.2	12.6	0.1	8.4	19.0	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	20.3	32.5	0.1	21.4	29.9		9.2	12.6	0.1	8.4	19.0	0.1
LOS	C	C	A	C	C		A	B	A	A	B	A
Approach Delay		12.7			26.4			10.7			16.8	
Approach LOS		B			C			B			B	
Queue Length 50th (ft)	17	29	0	27	45		25	61	0	3	128	0
Queue Length 95th (ft)	41	64	0	57	92		55	155	0	13	223	0
Internal Link Dist (ft)		557			487			435			486	
Turn Bay Length (ft)	250		350				410		370	430		430
Base Capacity (vph)	333	551	1583	332	544		605	1120	1017	710	939	880
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.12	0.09	0.24	0.21		0.21	0.23	0.03	0.03	0.39	0.04

## Intersection Summary

Cycle Length: 86

Actuated Cycle Length: 70.3

Natural Cycle: 70

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.39

## Timings

### 1: State Highway 83 & Highway 105/Walker Road

## Background Traffic Volumes

PM Peak Hour - Year 2026

Intersection Signal Delay: 15.5

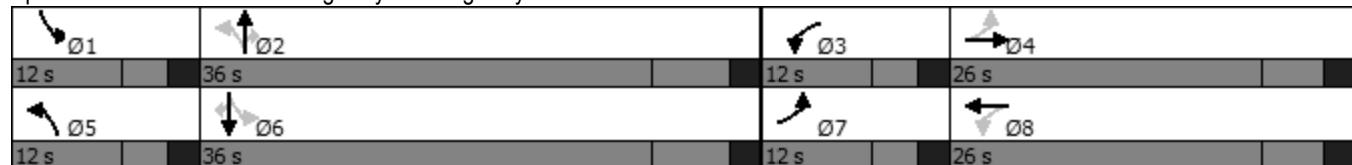
Intersection LOS: B

Intersection Capacity Utilization 57.1%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: State Highway 83 & Highway 105/Walker Road



Intersection						
Int Delay, s/veh	0.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	10	61	103	2	2	2
Future Vol, veh/h	10	61	103	2	2	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	11	66	112	2	2	2
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	114	0	-	0	201	113
Stage 1	-	-	-	-	113	-
Stage 2	-	-	-	-	88	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1475	-	-	-	788	940
Stage 1	-	-	-	-	912	-
Stage 2	-	-	-	-	935	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1475	-	-	-	782	940
Mov Cap-2 Maneuver	-	-	-	-	782	-
Stage 1	-	-	-	-	905	-
Stage 2	-	-	-	-	935	-
Approach	EB	WB	SB			
HCM Control Delay, s	1.1	0	9.2			
HCM LOS			A			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1475	-	-	-	854	
HCM Lane V/C Ratio	0.007	-	-	-	0.005	
HCM Control Delay (s)	7.5	0	-	-	9.2	
HCM Lane LOS	A	A	-	-	A	
HCM 95th %tile Q(veh)	0	-	-	-	0	

## Timings

## 1: State Highway 83 &amp; Highway 105/Walker Road

## Background Traffic Volumes

AM Peak Hour - Year 2040

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	29	221	157	192	301	59	164	329	24	85	279	53
Future Volume (vph)	29	221	157	192	301	59	164	329	24	85	279	53
Satd. Flow (prot)	1770	1863	1583	1770	1816	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.356			0.368			0.484			0.456		
Satd. Flow (perm)	663	1863	1583	685	1816	0	902	1863	1583	849	1863	1583
Satd. Flow (RTOR)			254		11				165			165
Lane Group Flow (vph)	32	240	171	209	391	0	178	358	26	92	303	58
Turn Type	pm+pt	NA	Free	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free	8			2		2	6		6
Detector Phase	7	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	6.0		5.0	6.0		5.0	30.0	30.0	5.0	30.0	30.0
Minimum Split (s)	10.0	12.0		10.0	12.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (s)	10.0	26.0		13.0	29.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (%)	11.6%	30.2%		15.1%	33.7%		11.6%	43.0%	43.0%	11.6%	43.0%	43.0%
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0		5.0	7.0	7.0	5.0	7.0	7.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Act Effct Green (s)	22.4	16.4	82.6	29.3	23.7		38.2	32.3	32.3	37.1	30.1	30.1
Actuated g/C Ratio	0.27	0.20	1.00	0.35	0.29		0.46	0.39	0.39	0.45	0.36	0.36
v/c Ratio	0.13	0.65	0.11	0.60	0.74		0.38	0.49	0.04	0.21	0.45	0.09
Control Delay	17.8	39.0	0.1	26.8	37.2		15.5	23.8	0.1	13.0	23.4	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	17.8	39.0	0.1	26.8	37.2		15.5	23.8	0.1	13.0	23.4	0.2
LOS	B	D	A	C	D		B	C	A	B	C	A
Approach Delay		22.5			33.6			20.1			18.3	
Approach LOS		C			C			C			B	
Queue Length 50th (ft)	10	115	0	76	191		52	154	0	26	126	0
Queue Length 95th (ft)	28	189	0	128	#332		91	240	0	51	200	0
Internal Link Dist (ft)		557			487			435			486	
Turn Bay Length (ft)	250		350				410		370	430		430
Base Capacity (vph)	247	452	1583	348	529		469	727	718	437	678	681
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.53	0.11	0.60	0.74		0.38	0.49	0.04	0.21	0.45	0.09

## Intersection Summary

Cycle Length: 86

Actuated Cycle Length: 82.6

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.74

## Timings

### 1: State Highway 83 & Highway 105/Walker Road

## Background Traffic Volumes

AM Peak Hour - Year 2040

Intersection Signal Delay: 24.1

Intersection LOS: C

Intersection Capacity Utilization 76.8%

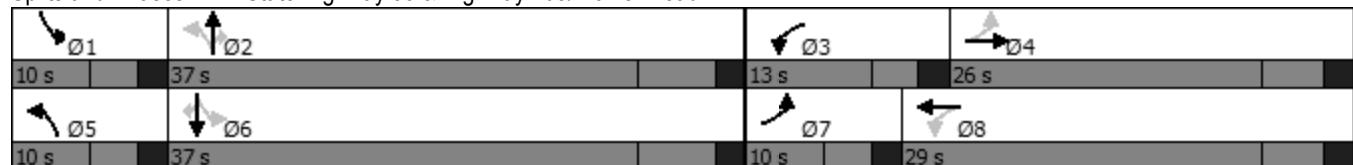
ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

#### Splits and Phases: 1: State Highway 83 & Highway 105/Walker Road



Intersection						
Int Delay, s/veh	0.1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	0	126	108	2	1	2
Future Vol, veh/h	0	126	108	2	1	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	137	117	2	1	2
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	119	0	-	0	255	118
Stage 1	-	-	-	-	118	-
Stage 2	-	-	-	-	137	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1469	-	-	-	734	934
Stage 1	-	-	-	-	907	-
Stage 2	-	-	-	-	890	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1469	-	-	-	734	934
Mov Cap-2 Maneuver	-	-	-	-	734	-
Stage 1	-	-	-	-	907	-
Stage 2	-	-	-	-	890	-
Approach	EB	WB	SB			
HCM Control Delay, s	0	0	9.2			
HCM LOS			A			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1469	-	-	-	856	
HCM Lane V/C Ratio	-	-	-	-	0.004	
HCM Control Delay (s)	0	-	-	-	9.2	
HCM Lane LOS	A	-	-	-	A	
HCM 95th %tile Q(veh)	0	-	-	-	0	

## Timings

## 1: State Highway 83 &amp; Highway 105/Walker Road

## Background Traffic Volumes

PM Peak Hour - Year 2040

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	63	84	167	98	115	27	155	326	38	22	454	49
Future Volume (vph)	63	84	167	98	115	27	155	326	38	22	454	49
Satd. Flow (prot)	1770	1863	1583	1770	1811	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.637			0.653			0.329			0.549		
Satd. Flow (perm)	1187	1863	1583	1216	1811	0	613	1863	1583	1023	1863	1583
Satd. Flow (RTOR)				254		11				165		165
Lane Group Flow (vph)	68	91	182	107	154	0	168	354	41	24	493	53
Turn Type	pm+pt	NA	Free	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free		8		2		2	6		6
Detector Phase	7	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	6.0		5.0	6.0		5.0	30.0	30.0	5.0	30.0	30.0
Minimum Split (s)	10.0	12.0		10.0	12.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (s)	10.0	18.0		11.0	19.0		12.0	47.0	47.0	10.0	45.0	45.0
Total Split (%)	11.6%	20.9%		12.8%	22.1%		14.0%	54.7%	54.7%	11.6%	52.3%	52.3%
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0		5.0	7.0	7.0	5.0	7.0	7.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Act Effct Green (s)	14.9	10.1	81.8	16.5	10.8		51.0	46.4	46.4	45.3	38.2	38.2
Actuated g/C Ratio	0.18	0.12	1.00	0.20	0.13		0.62	0.57	0.57	0.55	0.47	0.47
v/c Ratio	0.27	0.40	0.11	0.38	0.62		0.35	0.34	0.04	0.04	0.57	0.06
Control Delay	26.7	39.5	0.1	28.7	43.0		9.2	13.0	0.1	7.2	20.2	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	26.7	39.5	0.1	28.7	43.0		9.2	13.0	0.1	7.2	20.2	0.1
LOS	C	D	A	C	D		A	B	A	A	C	A
Approach Delay		15.9			37.2			10.9			17.8	
Approach LOS		B			D			B			B	
Queue Length 50th (ft)	28	45	0	44	72		35	88	0	5	191	0
Queue Length 95th (ft)	59	91	0	85	133		63	189	0	14	295	0
Internal Link Dist (ft)		557			487			435			486	
Turn Bay Length (ft)	250		350				410		370	430		430
Base Capacity (vph)	252	274	1583	285	299		482	1056	968	612	870	827
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.27	0.33	0.11	0.38	0.52		0.35	0.34	0.04	0.04	0.57	0.06

## Intersection Summary

Cycle Length: 86

Actuated Cycle Length: 81.8

Natural Cycle: 70

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.62

## Timings

### 1: State Highway 83 & Highway 105/Walker Road

## Background Traffic Volumes

PM Peak Hour - Year 2040

Intersection Signal Delay: 18.1

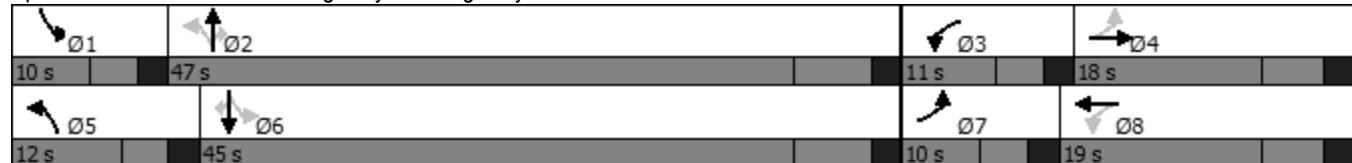
Intersection LOS: B

Intersection Capacity Utilization 64.6%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: State Highway 83 & Highway 105/Walker Road



Intersection						
Int Delay, s/veh	0.5					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	10	83	139	2	2	2
Future Vol, veh/h	10	83	139	2	2	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	11	90	151	2	2	2
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	153	0	-	0	264	152
Stage 1	-	-	-	-	152	-
Stage 2	-	-	-	-	112	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1428	-	-	-	725	894
Stage 1	-	-	-	-	876	-
Stage 2	-	-	-	-	913	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1428	-	-	-	719	894
Mov Cap-2 Maneuver	-	-	-	-	719	-
Stage 1	-	-	-	-	869	-
Stage 2	-	-	-	-	913	-
Approach	EB	WB	SB			
HCM Control Delay, s	0.8	0	9.5			
HCM LOS			A			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1428	-	-	-	797	
HCM Lane V/C Ratio	0.008	-	-	-	0.005	
HCM Control Delay (s)	7.5	0	-	-	9.5	
HCM Lane LOS	A	A	-	-	A	
HCM 95th %tile Q(veh)	0	-	-	-	0	

## Timings

## 1: State Highway 83 &amp; Highway 105/Walker Road

Total Traffic Volumes

AM Peak Hour - Year 2026

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	22	169	116	142	224	44	122	244	30	70	207	40
Future Volume (vph)	22	169	116	142	224	44	122	244	30	70	207	40
Satd. Flow (prot)	1770	1863	1583	1770	1816	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.525			0.463			0.574			0.584		
Satd. Flow (perm)	978	1863	1583	862	1816	0	1069	1863	1583	1088	1863	1583
Satd. Flow (RTOR)				254		11				165		165
Lane Group Flow (vph)	24	184	126	154	291	0	133	265	33	76	225	43
Turn Type	pm+pt	NA	Free	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free	8			2		2	6		6
Detector Phase	7	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	6.0		5.0	6.0		5.0	30.0	30.0	5.0	30.0	30.0
Minimum Split (s)	10.0	12.0		10.0	12.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (s)	12.0	26.0		12.0	26.0		12.0	36.0	36.0	12.0	36.0	36.0
Total Split (%)	14.0%	30.2%		14.0%	30.2%		14.0%	41.9%	41.9%	14.0%	41.9%	41.9%
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0		5.0	7.0	7.0	5.0	7.0	7.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Act Effct Green (s)	21.3	14.1	80.2	25.3	21.7		39.2	31.7	31.7	37.7	29.1	29.1
Actuated g/C Ratio	0.27	0.18	1.00	0.32	0.27		0.49	0.40	0.40	0.47	0.36	0.36
v/c Ratio	0.07	0.56	0.07	0.43	0.58		0.22	0.36	0.04	0.13	0.33	0.06
Control Delay (s/veh)	17.7	36.7	0.0	23.4	31.2		11.6	21.2	0.1	11.1	21.4	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	17.7	36.7	0.0	23.4	31.2		11.6	21.2	0.1	11.1	21.4	0.1
LOS	B	D	A	C	C		B	C	A	B	C	A
Approach Delay (s/veh)		21.6			28.6			16.7			16.5	
Approach LOS		C			C			B			B	
Queue Length 50th (ft)	8	85	0	55	111		30	94	0	17	78	0
Queue Length 95th (ft)	23	146	0	98	226		68	177	0	43	151	0
Internal Link Dist (ft)		557			487			435			486	
Turn Bay Length (ft)	250		350				410		370	430		430
Base Capacity (vph)	339	466	1583	351	499		584	736	725	576	675	679
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.07	0.39	0.08	0.44	0.58		0.23	0.36	0.05	0.13	0.33	0.06

## Intersection Summary

Cycle Length: 86

Actuated Cycle Length: 80.2

Natural Cycle: 75

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.58

## Timings

### 1: State Highway 83 & Highway 105/Walker Road

Total Traffic Volumes

AM Peak Hour - Year 2026

Intersection Signal Delay (s/veh): 21.1

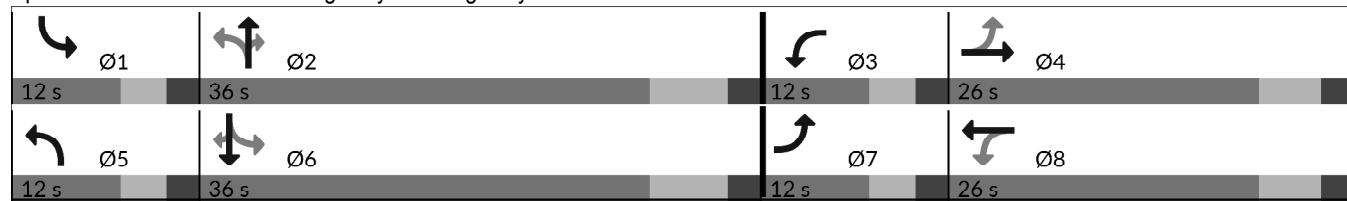
Intersection LOS: C

Intersection Capacity Utilization 69.6%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: State Highway 83 & Highway 105/Walker Road



HCM 6th TWSC  
2: Walker Road & Kerry Run Road

Total Traffic Volumes  
AM Peak Hour - Year 2026

Intersection						
Int Delay, s/veh	1					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	24	94	80	2	1	2
Future Vol, veh/h	24	94	80	2	1	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	26	102	87	2	1	2
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	89	0	-	0	242	88
Stage 1	-	-	-	-	88	-
Stage 2	-	-	-	-	154	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1506	-	-	-	746	970
Stage 1	-	-	-	-	935	-
Stage 2	-	-	-	-	874	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1506	-	-	-	733	970
Mov Cap-2 Maneuver	-	-	-	-	733	-
Stage 1	-	-	-	-	918	-
Stage 2	-	-	-	-	874	-
Approach	EB	WB	SB			
HCM Control Delay, s/v	1.5	0	9.1			
HCM LOS			A			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1506	-	-	-	876	
HCM Lane V/C Ratio	0.017	-	-	-	0.004	
HCM Control Delay (s/veh)	7.4	0	-	-	9.1	
HCM Lane LOS	A	A	-	-	A	
HCM 95th %tile Q (veh)	0.1	-	-	-	0	

## Timings

## 1: State Highway 83 &amp; Highway 105/Walker Road

Total Traffic Volumes

PM Peak Hour - Year 2026

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	47	62	124	85	90	27	115	242	28	17	337	36
Future Volume (vph)	47	62	124	85	90	27	115	242	28	17	337	36
Satd. Flow (prot)	1770	1863	1583	1770	1799	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.676			0.620			0.438			0.597		
Satd. Flow (perm)	1259	1863	1583	1155	1799	0	816	1863	1583	1112	1863	1583
Satd. Flow (RTOR)				254		16				165		165
Lane Group Flow (vph)	51	67	135	92	127	0	125	263	30	18	366	39
Turn Type	pm+pt	NA	Free	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free		8		2		2	6		6
Detector Phase	7	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	6.0		5.0	6.0		5.0	30.0	30.0	5.0	30.0	30.0
Minimum Split (s)	10.0	12.0		10.0	12.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (s)	12.0	26.0		12.0	26.0		12.0	36.0	36.0	12.0	36.0	36.0
Total Split (%)	14.0%	30.2%		14.0%	30.2%		14.0%	41.9%	41.9%	14.0%	41.9%	41.9%
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0		5.0	7.0	7.0	5.0	7.0	7.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Act Effct Green (s)	15.3	9.8	69.0	16.4	12.1		42.2	41.3	41.3	38.6	34.7	34.7
Actuated g/C Ratio	0.22	0.14	1.00	0.24	0.18		0.61	0.60	0.60	0.56	0.50	0.50
v/c Ratio	0.15	0.25	0.08	0.27	0.38		0.20	0.23	0.02	0.02	0.39	0.04
Control Delay (s/veh)	20.0	31.8	0.1	21.6	29.6		9.4	12.9	0.0	8.5	19.4	0.0
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	20.0	31.8	0.1	21.6	29.6		9.4	12.9	0.0	8.5	19.4	0.0
LOS	C	C	A	C	C		A	B	A	A	B	A
Approach Delay (s/veh)		12.5			26.3			11.0			17.2	
Approach LOS		B			C			B			B	
Queue Length 50th (ft)	17	29	0	32	49		25	62	0	3	130	0
Queue Length 95th (ft)	41	64	0	65	100		56	158	0	13	227	0
Internal Link Dist (ft)		557			487			435			486	
Turn Bay Length (ft)	250		350				410		370	430		430
Base Capacity (vph)	347	584	1583	350	575		603	1116	1014	706	936	878
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.15	0.11	0.09	0.26	0.22		0.21	0.24	0.03	0.03	0.39	0.04

## Intersection Summary

Cycle Length: 86

Actuated Cycle Length: 69

Natural Cycle: 70

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.39

## Timings

### 1: State Highway 83 & Highway 105/Walker Road

## Total Traffic Volumes

PM Peak Hour - Year 2026

Intersection Signal Delay (s/veh): 15.8

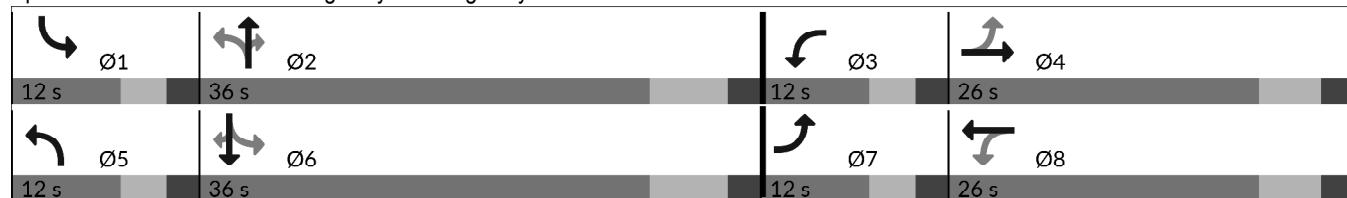
Intersection LOS: B

Intersection Capacity Utilization 57.7%

ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: State Highway 83 & Highway 105/Walker Road



HCM 6th TWSC  
2: Walker Road & Kerry Run Road

Total Traffic Volumes  
PM Peak Hour - Year 2026

Intersection						
Int Delay, s/veh	1.6					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	10	61	103	2	2	26
Future Vol, veh/h	10	61	103	2	2	26
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	11	66	112	2	2	28
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	114	0	-	0	201	113
Stage 1	-	-	-	-	113	-
Stage 2	-	-	-	-	88	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1475	-	-	-	788	940
Stage 1	-	-	-	-	912	-
Stage 2	-	-	-	-	935	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1475	-	-	-	782	940
Mov Cap-2 Maneuver	-	-	-	-	782	-
Stage 1	-	-	-	-	905	-
Stage 2	-	-	-	-	935	-
Approach	EB	WB	SB			
HCM Control Delay, s/v	1.1	0	9			
HCM LOS			A			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1475	-	-	-	927	
HCM Lane V/C Ratio	0.007	-	-	-	0.033	
HCM Control Delay (s/veh)	7.5	0	-	-	9	
HCM Lane LOS	A	A	-	-	A	
HCM 95th %tile Q (veh)	0	-	-	-	0.1	

## Timings

## 1: State Highway 83 &amp; Highway 105/Walker Road

Total Traffic Volumes

AM Peak Hour - Year 2040

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	29	226	157	192	301	59	164	329	36	92	279	53
Future Volume (vph)	29	226	157	192	301	59	164	329	36	92	279	53
Satd. Flow (prot)	1770	1863	1583	1770	1816	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.357			0.360			0.483			0.456		
Satd. Flow (perm)	665	1863	1583	671	1816	0	900	1863	1583	849	1863	1583
Satd. Flow (RTOR)				254		11				165		165
Lane Group Flow (vph)	32	246	171	209	391	0	178	358	39	100	303	58
Turn Type	pm+pt	NA	Free	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free	8			2		2	6		6
Detector Phase	7	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	6.0		5.0	6.0		5.0	30.0	30.0	5.0	30.0	30.0
Minimum Split (s)	10.0	12.0		10.0	12.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (s)	10.0	26.0		13.0	29.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (%)	11.6%	30.2%		15.1%	33.7%		11.6%	43.0%	43.0%	11.6%	43.0%	43.0%
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0		5.0	7.0	7.0	5.0	7.0	7.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Act Effct Green (s)	22.5	16.5	82.7	29.4	23.8		38.2	32.3	32.3	37.1	30.1	30.1
Actuated g/C Ratio	0.27	0.20	1.00	0.36	0.29		0.46	0.39	0.39	0.45	0.36	0.36
v/c Ratio	0.12	0.66	0.10	0.60	0.73		0.38	0.49	0.05	0.22	0.44	0.08
Control Delay (s/veh)	17.7	39.4	0.1	27.0	37.0		15.5	23.8	0.1	13.1	23.4	0.2
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	17.7	39.4	0.1	27.0	37.0		15.5	23.8	0.1	13.1	23.4	0.2
LOS	B	D	A	C	D		B	C	A	B	C	A
Approach Delay (s/veh)		22.9			33.6			19.7			18.3	
Approach LOS		C			C			B			B	
Queue Length 50th (ft)	10	118	0	76	191		52	154	0	28	126	0
Queue Length 95th (ft)	28	194	0	128	#332		91	240	0	55	200	0
Internal Link Dist (ft)		557			487			435			486	
Turn Bay Length (ft)	250		350				410		370	430		430
Base Capacity (vph)	248	452	1583	344	529		468	726	718	436	677	680
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.13	0.54	0.11	0.61	0.74		0.38	0.49	0.05	0.23	0.45	0.09

## Intersection Summary

Cycle Length: 86

Actuated Cycle Length: 82.7

Natural Cycle: 80

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.74

## Timings

### 1: State Highway 83 & Highway 105/Walker Road

## Total Traffic Volumes

AM Peak Hour - Year 2040

Intersection Signal Delay (s/veh): 24.1

Intersection LOS: C

Intersection Capacity Utilization 76.8%

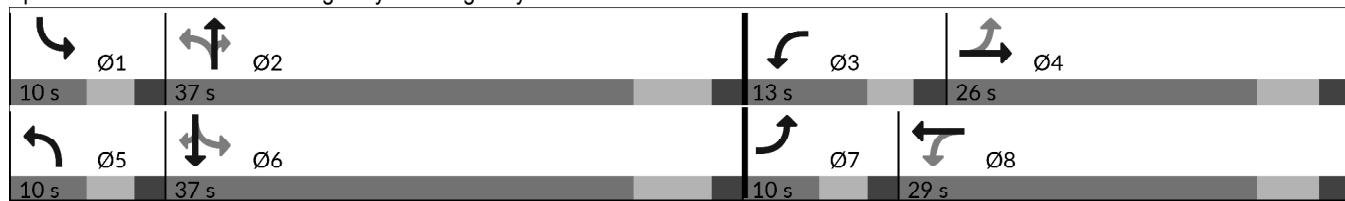
ICU Level of Service D

Analysis Period (min) 15

# 95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Splits and Phases: 1: State Highway 83 & Highway 105/Walker Road



HCM 6th TWSC  
2: Walker Road & Kerry Run Road

Total Traffic Volumes  
AM Peak Hour - Year 2040

Intersection						
Int Delay, s/veh	0.8					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	24	126	108	2	1	2
Future Vol, veh/h	24	126	108	2	1	2
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	26	137	117	2	1	2
Major/Minor	Major1	Major2	Minor2			
Conflicting Flow All	119	0	-	0	307	118
Stage 1	-	-	-	-	118	-
Stage 2	-	-	-	-	189	-
Critical Hdwy	4.12	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.218	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1469	-	-	-	685	934
Stage 1	-	-	-	-	907	-
Stage 2	-	-	-	-	843	-
Platoon blocked, %	-	-	-	-	-	-
Mov Cap-1 Maneuver	1469	-	-	-	672	934
Mov Cap-2 Maneuver	-	-	-	-	672	-
Stage 1	-	-	-	-	890	-
Stage 2	-	-	-	-	843	-
Approach	EB	WB	SB			
HCM Control Delay, s/v	1.2	0	9.4			
HCM LOS			A			
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1469	-	-	-	827	
HCM Lane V/C Ratio	0.018	-	-	-	0.004	
HCM Control Delay (s/veh)	7.5	0	-	-	9.4	
HCM Lane LOS	A	A	-	-	A	
HCM 95th %tile Q (veh)	0.1	-	-	-	0	

## Timings

## 1: State Highway 83 &amp; Highway 105/Walker Road

Total Traffic Volumes

PM Peak Hour - Year 2040

Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑	↑
Traffic Volume (vph)	63	84	167	110	120	34	155	326	38	22	454	49
Future Volume (vph)	63	84	167	110	120	34	155	326	38	22	454	49
Satd. Flow (prot)	1770	1863	1583	1770	1801	0	1770	1863	1583	1770	1863	1583
Flt Permitted	0.589			0.654			0.328			0.549		
Satd. Flow (perm)	1097	1863	1583	1218	1801	0	611	1863	1583	1023	1863	1583
Satd. Flow (RTOR)			254		14				165			165
Lane Group Flow (vph)	68	91	182	120	167	0	168	354	41	24	493	53
Turn Type	pm+pt	NA	Free	pm+pt	NA		pm+pt	NA	Perm	pm+pt	NA	Perm
Protected Phases	7	4		3	8		5	2		1	6	
Permitted Phases	4		Free		8		2		2	6		6
Detector Phase	7	4		3	8		5	2	2	1	6	6
Switch Phase												
Minimum Initial (s)	5.0	6.0		5.0	6.0		5.0	30.0	30.0	5.0	30.0	30.0
Minimum Split (s)	10.0	12.0		10.0	12.0		10.0	37.0	37.0	10.0	37.0	37.0
Total Split (s)	10.0	18.0		11.0	19.0		12.0	47.0	47.0	10.0	45.0	45.0
Total Split (%)	11.6%	20.9%		12.8%	22.1%		14.0%	54.7%	54.7%	11.6%	52.3%	52.3%
Yellow Time (s)	3.0	4.0		3.0	4.0		3.0	5.0	5.0	3.0	5.0	5.0
All-Red Time (s)	2.0	2.0		2.0	2.0		2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)	5.0	6.0		5.0	6.0		5.0	7.0	7.0	5.0	7.0	7.0
Lead/Lag	Lead	Lag		Lead	Lag		Lead	Lag	Lag	Lead	Lag	Lag
Lead-Lag Optimize?	Yes	Yes		Yes	Yes		Yes	Yes	Yes	Yes	Yes	Yes
Recall Mode	None	None		None	None		None	Max	Max	None	Max	Max
Act Effct Green (s)	15.2	10.3	82.1	16.7	11.1		51.1	46.4	46.4	45.3	38.3	38.3
Actuated g/C Ratio	0.19	0.13	1.00	0.20	0.14		0.62	0.57	0.57	0.55	0.47	0.47
v/c Ratio	0.27	0.38	0.11	0.41	0.65		0.35	0.33	0.04	0.03	0.56	0.06
Control Delay (s/veh)	26.9	39.1	0.1	29.6	44.1		9.2	13.0	0.0	7.2	20.3	0.1
Queue Delay	0.0	0.0	0.0	0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0
Total Delay (s/veh)	26.9	39.1	0.1	29.6	44.1		9.2	13.0	0.0	7.2	20.3	0.1
LOS	C	D	A	C	D		A	B	A	A	C	A
Approach Delay (s/veh)		15.9			38.1			11.0			17.9	
Approach LOS		B			D			B			B	
Queue Length 50th (ft)	28	45	0	50	78		36	90	0	5	193	0
Queue Length 95th (ft)	59	91	0	94	143		63	189	0	14	295	0
Internal Link Dist (ft)		557			487			435			486	
Turn Bay Length (ft)	250		350				410		370	430		430
Base Capacity (vph)	244	274	1583	288	298		479	1052	966	610	868	825
Starvation Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Spillback Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Storage Cap Reductn	0	0	0	0	0		0	0	0	0	0	0
Reduced v/c Ratio	0.28	0.33	0.11	0.42	0.56		0.35	0.34	0.04	0.04	0.57	0.06

## Intersection Summary

Cycle Length: 86

Actuated Cycle Length: 82.1

Natural Cycle: 70

Control Type: Actuated-Uncoordinated

Maximum v/c Ratio: 0.65

## Timings

### 1: State Highway 83 & Highway 105/Walker Road

## Total Traffic Volumes

PM Peak Hour - Year 2040

Intersection Signal Delay (s/veh): 18.6

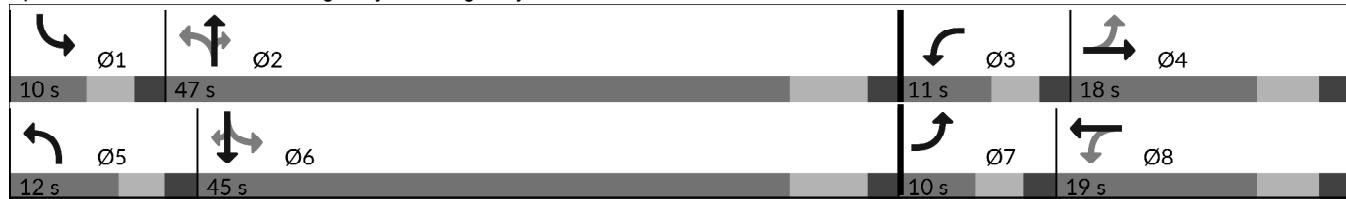
Intersection LOS: B

Intersection Capacity Utilization 65.3%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: State Highway 83 & Highway 105/Walker Road



HCM 6th TWSC  
2: Walker Road & Kerry Run Road

Total Traffic Volumes  
PM Peak Hour - Year 2040

Intersection

Int Delay, s/veh 1.3

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	10	83	139	2	2	26
Future Vol, veh/h	10	83	139	2	2	26
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	11	90	151	2	2	28

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	153	0	-
Stage 1	-	-	-
Stage 2	-	-	112
Critical Hdwy	4.12	-	-
Critical Hdwy Stg 1	-	-	5.42
Critical Hdwy Stg 2	-	-	5.42
Follow-up Hdwy	2.218	-	-
Pot Cap-1 Maneuver	1428	-	-
Stage 1	-	-	876
Stage 2	-	-	913
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1428	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	869
Stage 2	-	-	913

Approach	EB	WB	SB
HCM Control Delay, s/v	0.8	0	9.2
HCM LOS			A

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1428	-	-	-	879
HCM Lane V/C Ratio	0.008	-	-	-	0.035
HCM Control Delay (s/veh)	7.5	0	-	-	9.2
HCM Lane LOS	A	A	-	-	A
HCM 95th %tile Q (veh)	0	-	-	-	0.1

# V4\_TIS.pdf Markup Summary

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8/26/2024 11:04:25 AM (1)

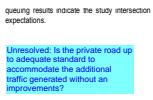


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**Page Label:** 18  
**Author:** Bret  
**Date:** 8/26/2024 11:04:25 AM  
**Status:**  
**Color:**   
**Layer:**  
**Space:**

Unresolved: Per ECM section B.3.3.A, trip generation shall be calculated from the latest data contained within the Institute of Transportation Engineers' Trip Generation Manual. Please provide a basis for the trip generation. Coordinate with the applicant to provide more clarity as to the intended uses so that a more accurate trip generation can be provided. Per meeting on 8-13-2024, please fully define the parameters of the use moving forward, submit with a comparable use from the ITE, pinpoint exactly what you are asking the County to approve. Phasing of the project could be used as a limitation.

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8/26/2024 11:05:11 AM (1)

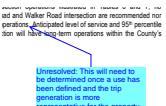


**Subject:** Engineer  
**Page Label:** 26  
**Author:** Bret  
**Date:** 8/26/2024 11:05:11 AM  
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Unresolved: Is the private road up to adequate standard to accommodate the additional traffic generated without an improvements?

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8/26/2024 11:05:26 AM (1)



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**Page Label:** 26  
**Author:** Bret  
**Date:** 8/26/2024 11:05:26 AM  
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Unresolved: This will need to be determined once a use has been defined and the trip generation is more representative for the property