



October 31, 2022

Gilbert LaForce, PE
El Paso County
2880 International Circle, Suite 110
Colorado Springs, CO 8010

Re: **Winsome Filing No. 2 Certification Letter**
El Paso County, Colorado

Dear Mr. LaForce:

This letter serves as the certification for Winsome Filing No. 2, El Paso County, CO consistent with El Paso County Engineering Manual ("ECM") Section 5.10.B, which states, *"Engineering Record Drawings shall be accompanied by a certification letter from the Engineer of Record which shall state that the site and adjacent properties (as affected by work performed under the County permit) are stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover, and that the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements. For sites including detention and/or water quality facilities, the certification letter shall include a statement that the facilities provide the required storage volume and will meet the required release rates, as documented by an attached UDFCD design form submitted with the original application, the stage areas, elevations and outlet dimensions."*

Based upon this information and information gathered during periodic site visits to the site during significant/key phases of the infrastructure installation, *Kimley-Horn & Associates, Inc.* is of the opinion that the work performed under the County Permit, per Section 5.10.B of the ECM, have been constructed in general compliance with the approved Construction Plans as filed with the County dated November 16, 2021.

Statement Of Engineer In Responsible Charge:

I, Kevin R. Kofford, a registered Professional Engineer in the State of Colorado, in accordance with Sections 5.2 and 5.3 of the Bylaws and Rules of the State Board of Registration for Professional Engineers and Professional Land Surveyors, do hereby certify that I or a person under my responsible charge periodically observed the construction of the above-mentioned project. Based on the on-site field observations and review of pertinent documentation, it is my professional opinion that the required infrastructure improvements have been installed and are in general compliance with the approved Construction Plans as filed with El Paso County. For BMPs with a Water Quality Capture Volume (WQCV), I have attached the post-construction As-Built drawings. The As-Built drawings accurately depict the final installation of the stormwater BMPs and verify the WQCV.

Kevin R. Kofford, P.E.
Colorado No. 57234



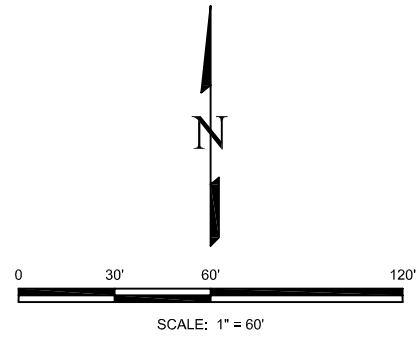




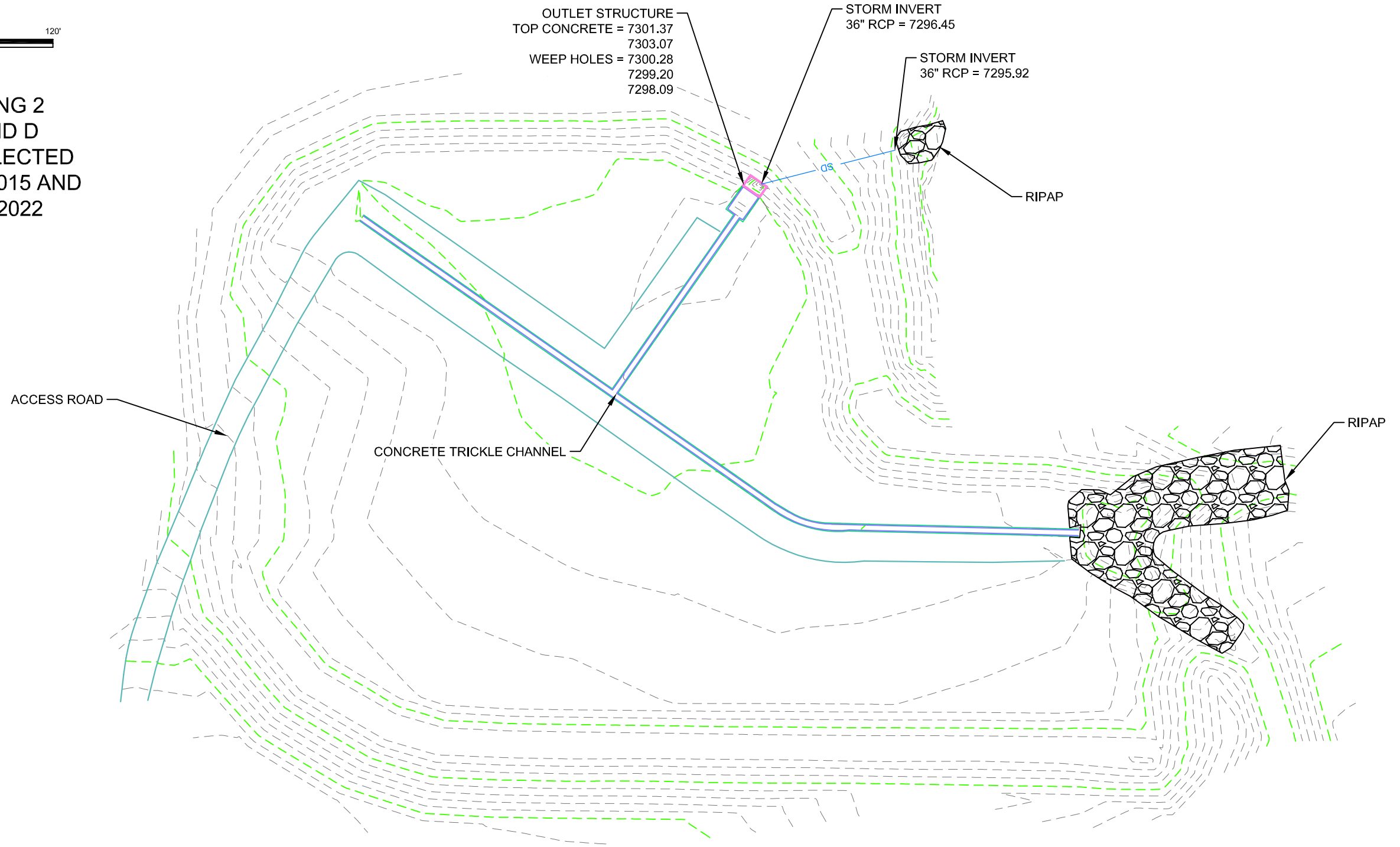




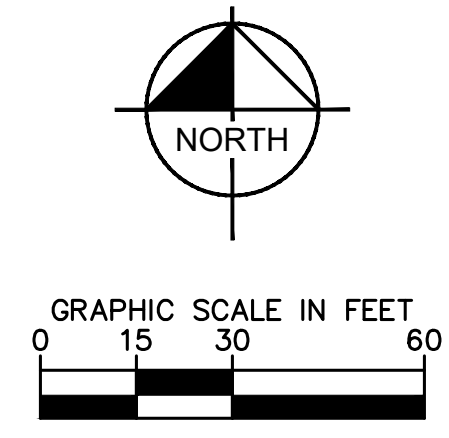
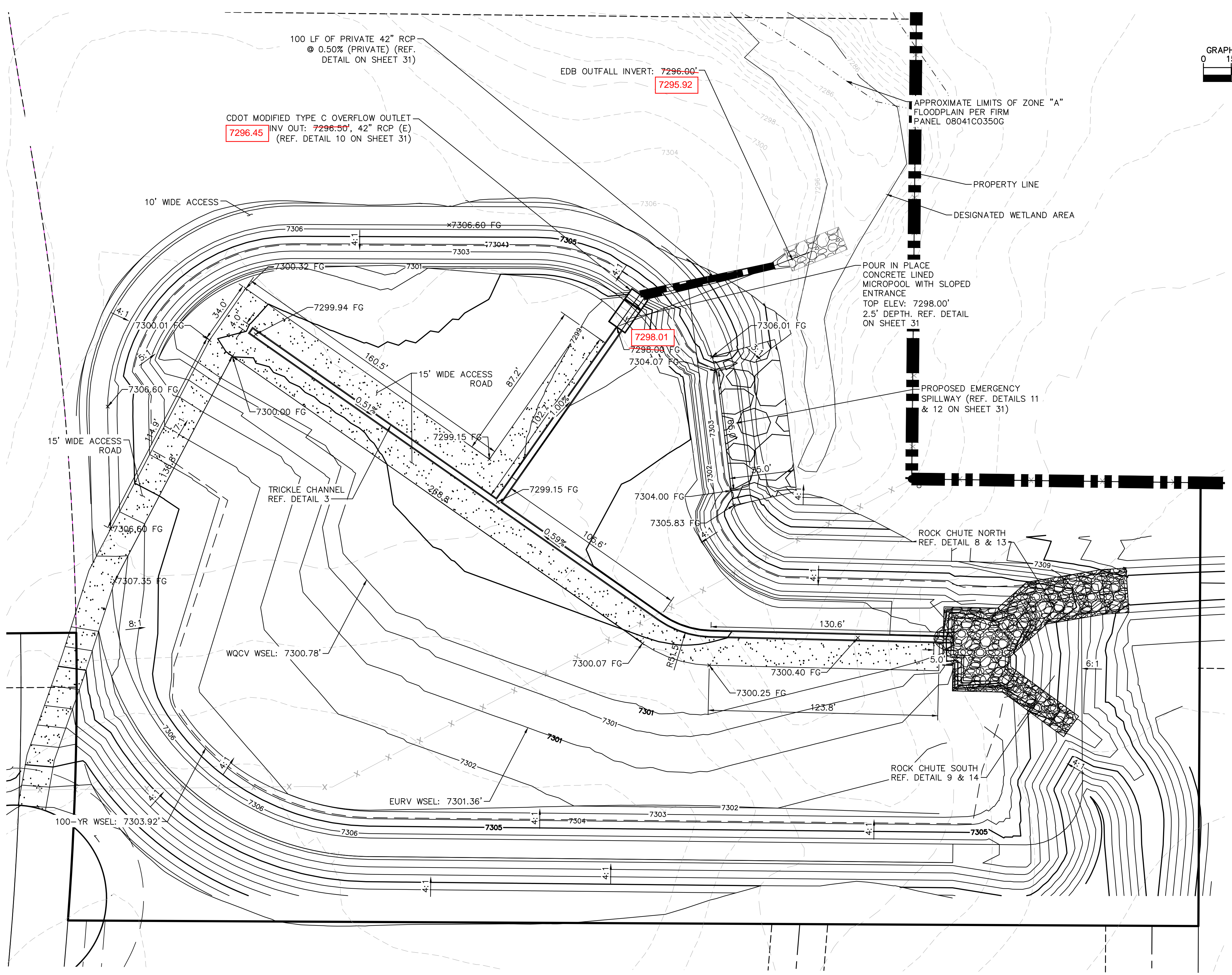




WINSOME FILING 2
 AS-BUILT POND D
 FIELD DATA COLLECTED
 SEPTEMBER 15, 2015 AND
 OCTOBER 27, 2022



Plotted By: Kofford, Kevin Sheet: WINSOME_P2_Layout:POND 5 GRADING November 08, 2021 03:02:19pm K:\DEN_Civil\196106000_Winsome_P2\CADD\PlanSheets\196106000_POND_5.dwg
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LEGEND

FG	FINISH GRADE
FBT	TOP OF FOREBAY AT FINISHED GRADE
FBB	BOTTOM OF FOREBAY AT FINISHED GRADE
TCT	TOP OF TRICKLE CHANNEL AT FINISHED GRADE
TCB	BOTTOM OF TRICKLE CHANNEL AT FINISHED GRADE
MPT	TOP OF MICROPOOL AT FINISHED GRADE
MPB	BOTTOM OF MICROPOOL AT FINISHED GRADE
GRATE	OUTLET STRUCTURE GRATE ELEVATION
ME	MATCH EXISTING
PT	TOP OF STEEL PLATE AT FINISHED GRADE
PB	BOTTOM OF STEEL PLATE AT FINISHED GRADE
	PROPERTY LINE
	TOP OF POND
	PROPOSED STORM SEWER

No.	REVISIONS	DATE	BY
4	COUNTY SUBMITTAL #4	10/29/21	KRK
3	COUNTY SUBMITTAL #3	9/15/21	KRK
2	COUNTY SUBMITTAL #2	8/25/21	KRK

Kimley»Horn
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 2 N NEVADA AVE SUITE 300, COLORADO SPRINGS, CO 80903
 PHONE: 719-453-0180
 WWW.KIMLEY-HORN.COM

LICENSED PROFESSIONAL	KEVIN KOFFORD
KHA PROJECT	019610600
DATE	08/25/2021
SCALE	AS SHOWN
DESIGNED BY	KHTAM
DRAWN BY	KHTAM
CHECKED BY	TLC DATE:
CO LICENSE NUMBER	57234

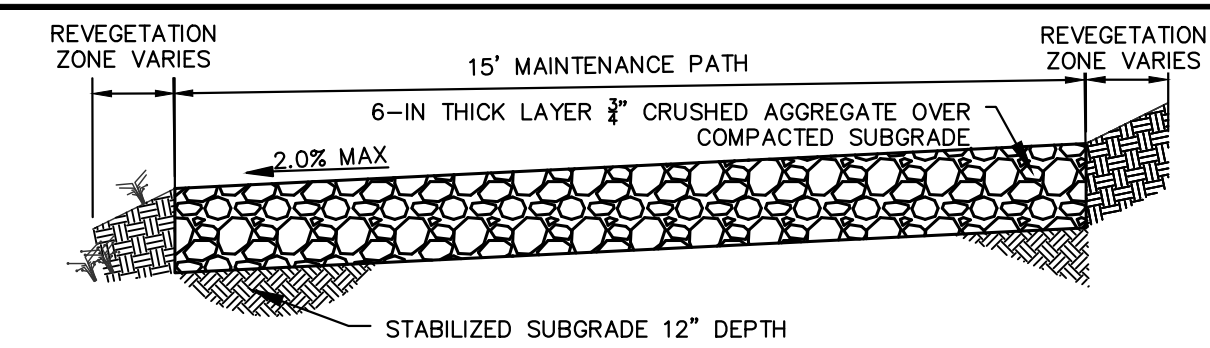
POND 5 OVERVIEW

WINSOME FILING NO. 2
 PREPARED FOR
WINSOME LLC
 EL PASO COUNTY CO

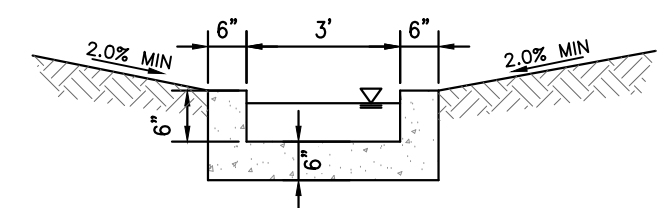


EPC 11/16/2021

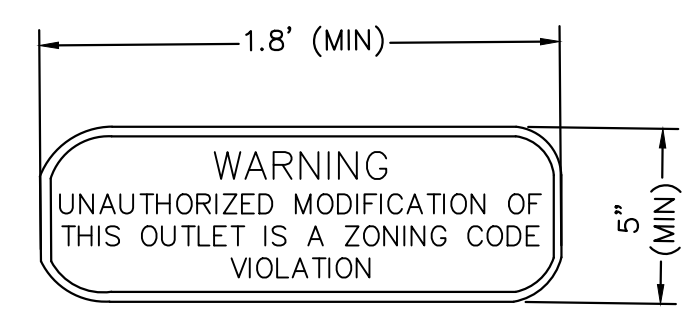
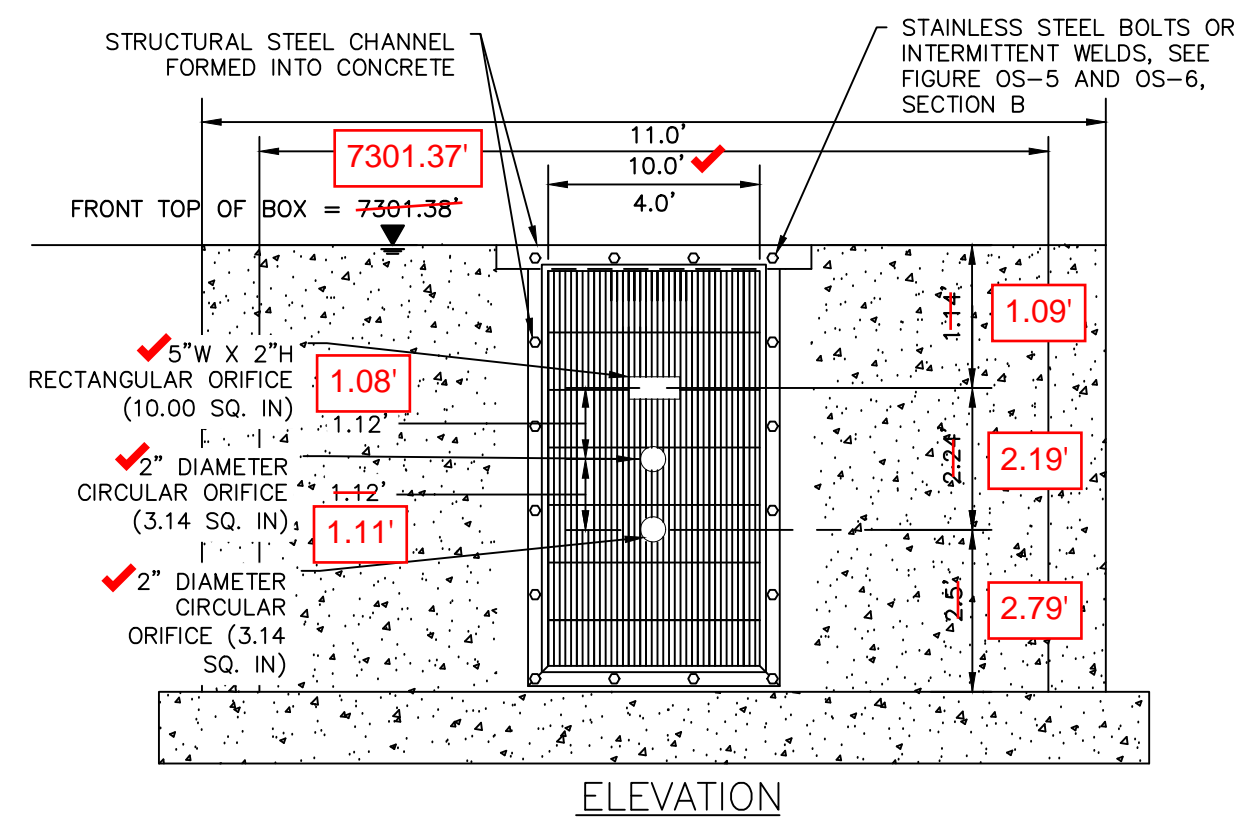
Plotted By: Kkoford, Kevin - Sheet - SWSOME.F2 - Layout: POND 5 DETAILS (1) - November 08, 2021 - 03:02:37pm - K:\VEN_Civil\196106000 - Winsome.F2\CADD\PlanSheets\0196106000_POND_5.dwg
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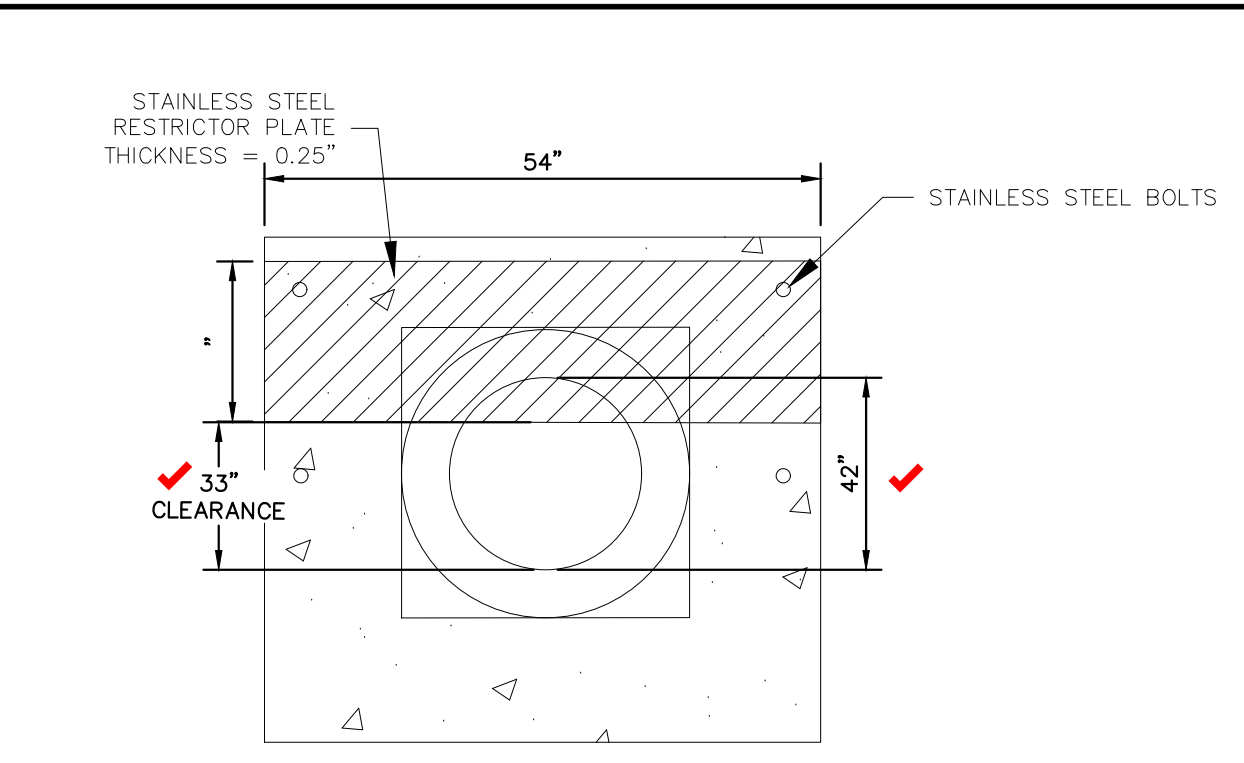
2 MAINTENANCE ROAD
 N.T.S.
MAINTENANCE PATH NOTES
 1. MAINTENANCE PATH SHALL INCLUDE SUBGRADE PREPARATION, CRUSHED 3/4" AGGREGATE, AND COMPACTION.



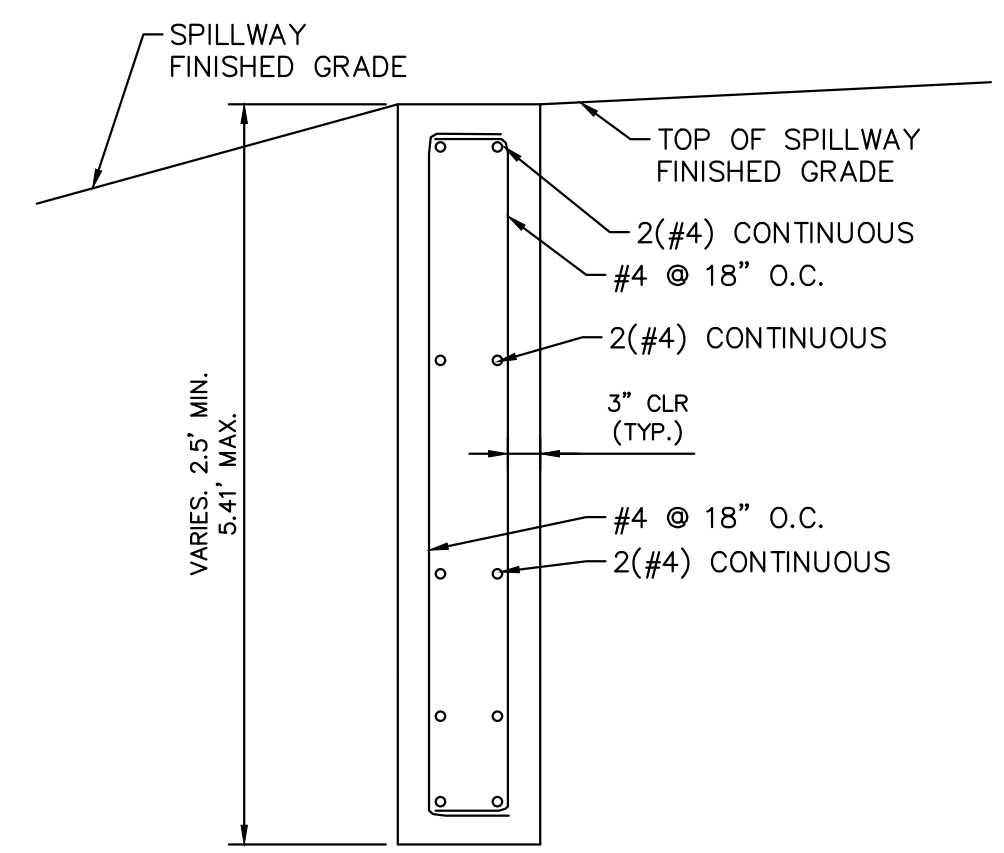
3 CONCRETE TRICKLE CHANNEL
 N.T.S.



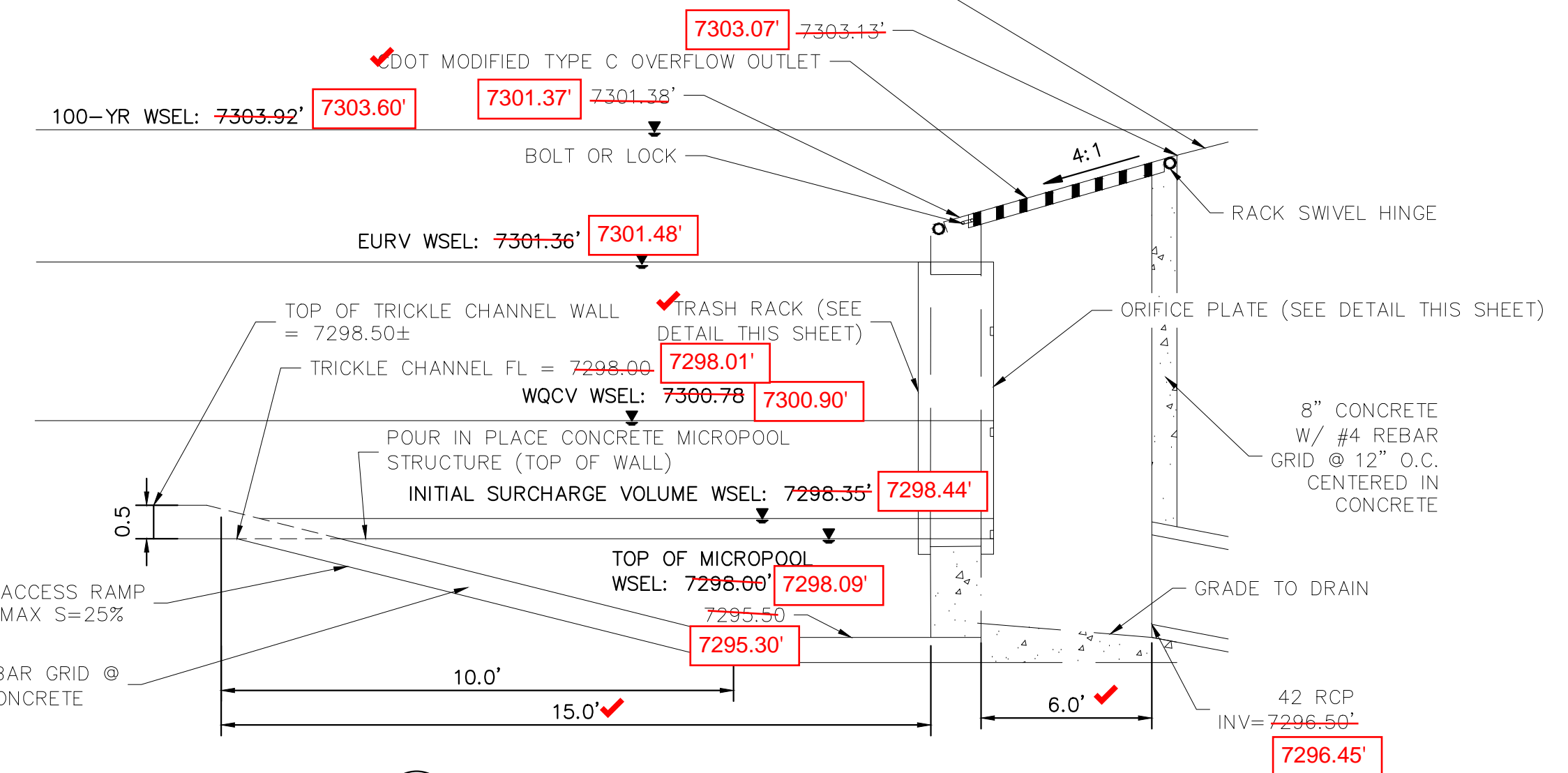
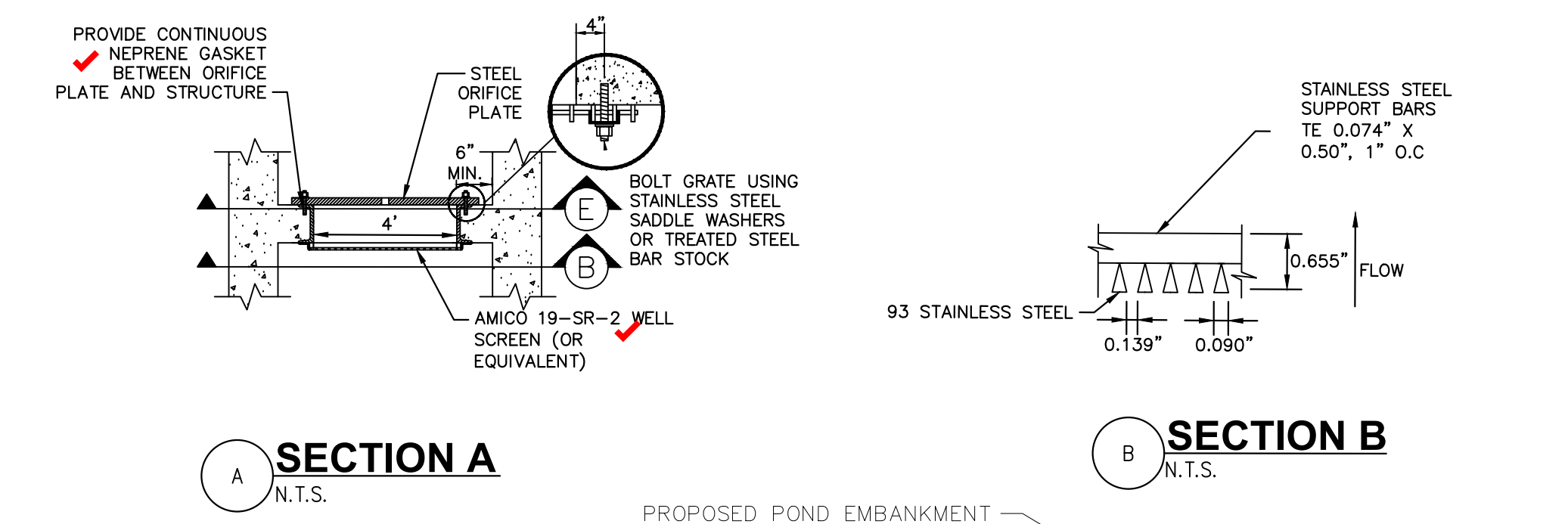
7 OUTLET SIGNAGE
 N.T.S.
OUTLET SIGNAGE NOTES
 1. SIGN SHALL BE A MINIMUM OF 0.75 SQUARE FEET AND SHALL BE ATTACHED TO THE OUTLET OR POSTED NEARBY.



5 100-YEAR FLOW RESTRICTOR B
 N.T.S.



6 SECTION CREST WALL DETAIL
 N.T.S.



10 OUTLET STRUCTURE DETAIL
 N.T.S.

4 ORIFICE PLATE AND TRASH RACK DETAIL
 N.T.S.
ORIFICE PLATE NOTES
 1. PROVIDE CONTINUOUS NEOPRENE GASKET MATERIAL BETWEEN THE ORIFICE PLATE AND CONCRETE.
 2. BOLT PLATE TO CONCRETE 12" MAX. ON CENTER. WITH A PLATE THICKNESS OF 0.25".

EURV AND WQCV TRASH RACKS

- WELL-SCREEN TRASH RACKS SHALL BE STAINLESS STEEL AND SHALL BE ATTACHED BY INTERMITTENT WELDS ALONG THE EDGE OF THE MOUNTING FRAME.
- BAR GATE TRASH RACKS SHALL BE ALUMINUM AND SHALL BE BOLTED USING STAINLESS STEEL HARDWARE.
- TRASH RACK OPEN AREAS ARE FOR SPECIFIED TRASH RACK MATERIALS. TOTAL TRASH RACK SIZE MAY NEED TO BE ADJUSTED FOR MATERIALS HAVING DIFFERENT OPEN AREA/GROSS AREA RATIO (R VALUE).
- STRUCTURAL DESIGN OF TRASH RACKS SHALL BE BASED ON FULL HYDROSTATIC HEAD WITH ZERO HEAD DOWNSTREAM OF THE RACK.

OVERFLOW SAFETY GRATES

- ALL SAFETY GRATES SHALL BE MOUNTED USING STAINLESS STEEL HARDWARE AND PROVIDED WITH HINGED AND LOCKABLE OR BOLTABLE ACCESS PANELS.
- SAFETY GRATES SHALL BE STAINLESS STEEL, ALUMINUM, OR STEEL. STEEL GRATES SHALL BE HOT DIP GALVANIZED AND MAY BE HOT POWDER COATED AFTER GALVANIZING.
- SAFETY GRATES SHALL BE DESIGNED SUCH THAT THE DIAGONAL DIMENSION OF EACH OPENING IS SMALLER THAN THE DIAMETER OF THE OUTLET PIPE.
- STRUCTURAL DESIGN OF SAFETY GRATES SHALL BE BASED ON FULL HYDROSTATIC HEAD WITH ZERO HEAD DOWNSTREAM OF THE RACK.

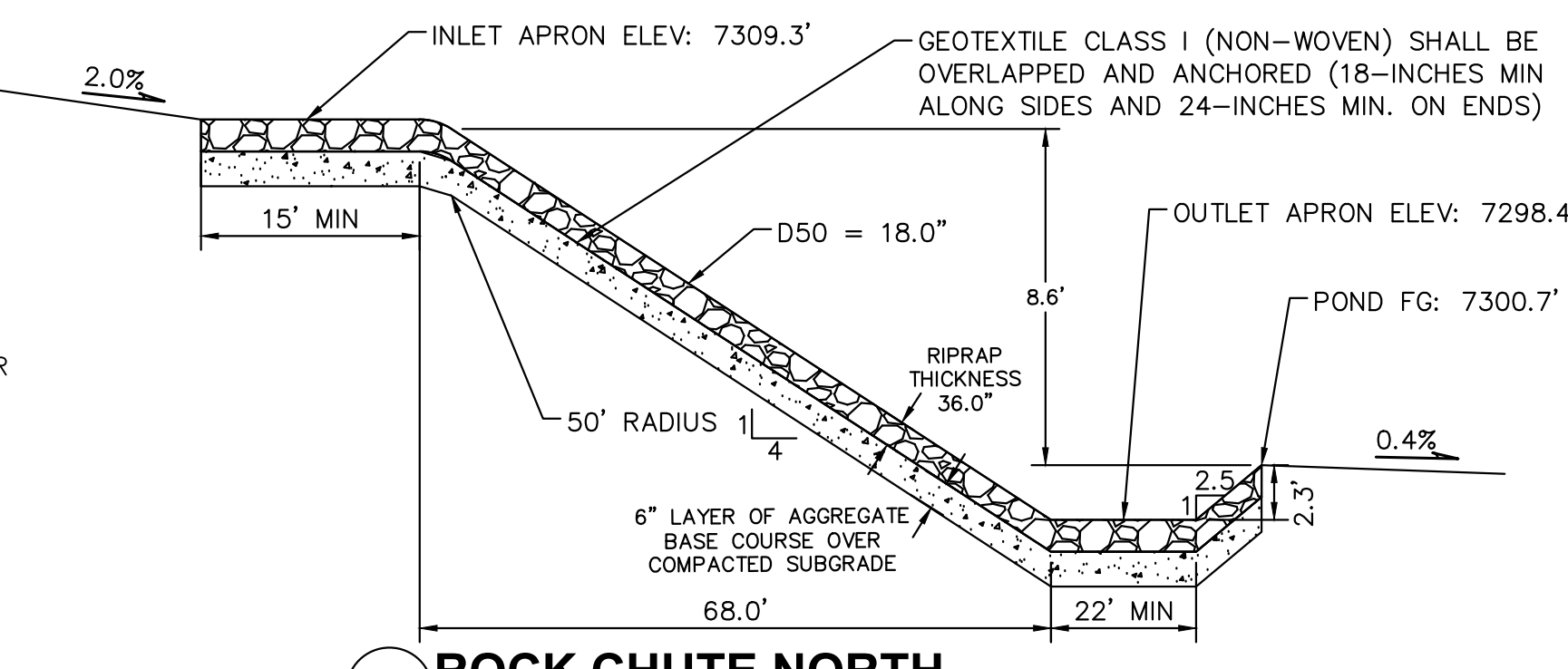
RIPRAP NOTES:

COLORADO DEPARTMENT OF TRANSPORTATION SECTION 506 REQUIREMENTS APPLY TO ALL RIPRAP.

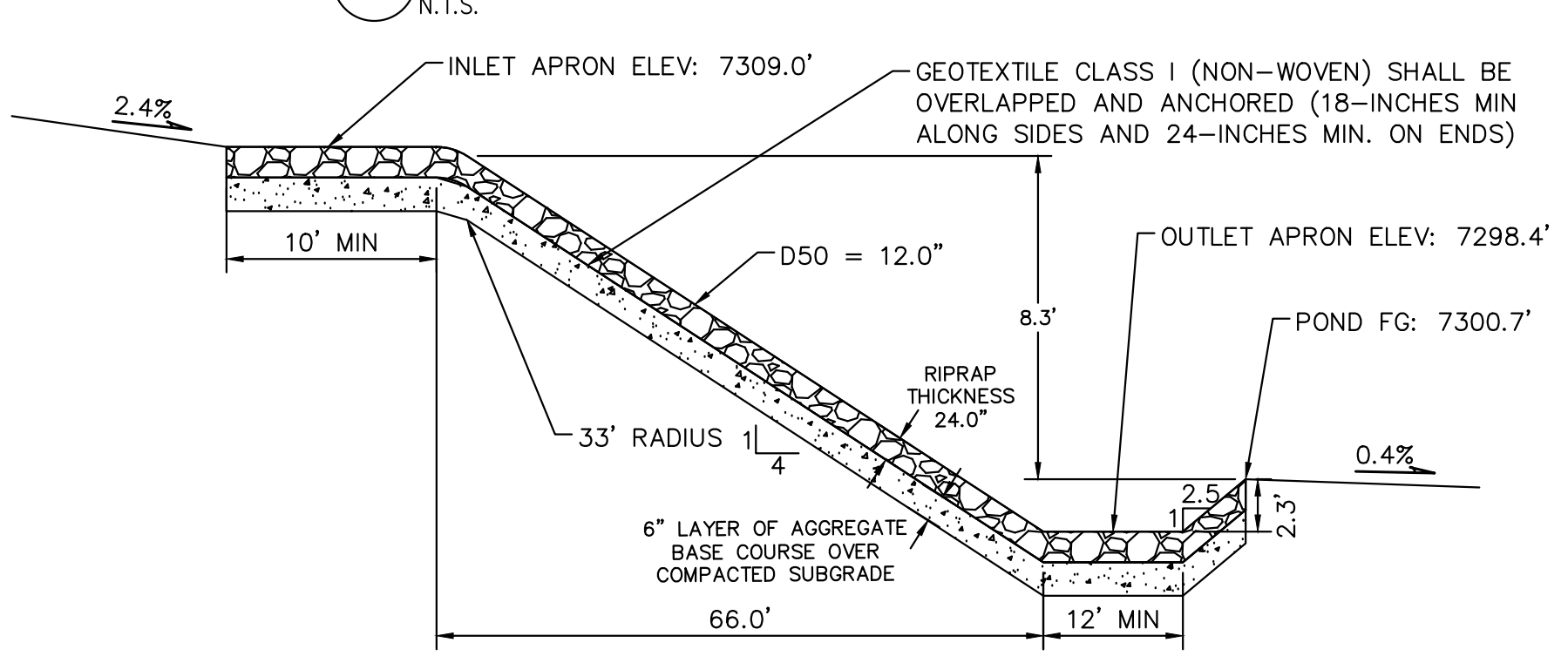
Table 506-2

Pay Item	Stone Size d50 ¹ (Inches)	Percent of Material Smaller Than Typical Stone ²	Typical Stone Dimensions ³ (Inches)	Typical Stone Weight ⁴ (Pounds)
Riprap	6	70-100	12	85
		50-70	9	35
		35-50	6	10
Riprap	9	70-100	15	160
		50-70	12	85
		35-50	9	35
Riprap	12	70-100	21	440
		50-70	18	275
		35-50	12	85
Riprap	18	70-100	30	1280
		50-70	24	650
		35-50	18	275
Riprap	24	70-100	42	3500
		50-70	33	1700
		35-50	24	650
		2-10	9	35

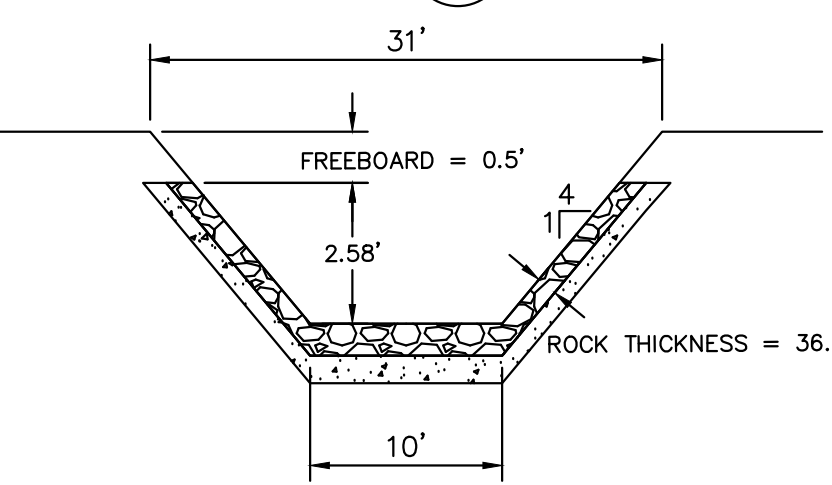
¹d50 = nominal stone size
²based on typical rock mass
³equivalent spherical diameter
⁴based on a specific gravity = 2.5



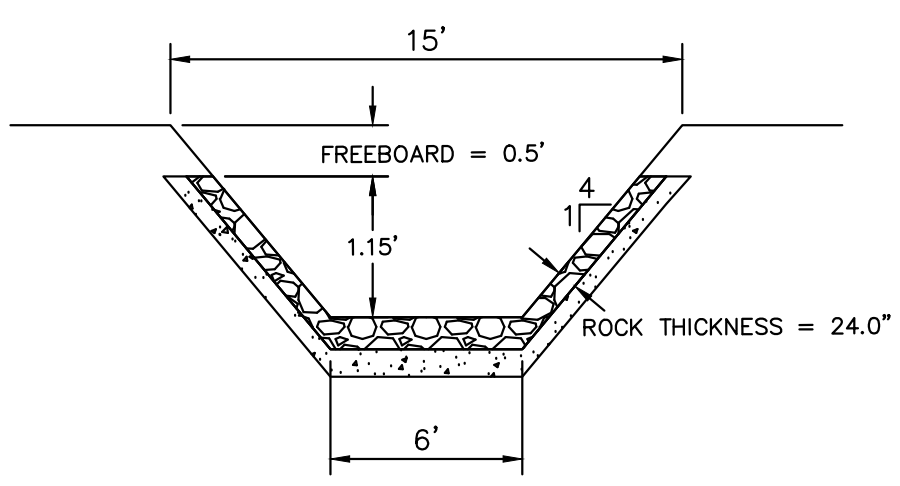
8 ROCK CHUTE NORTH
 N.T.S.



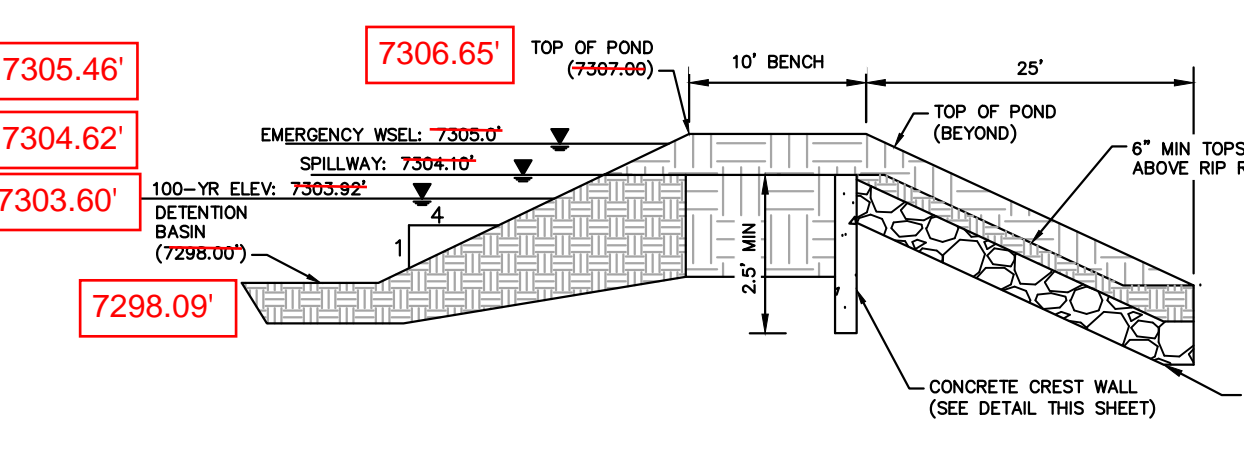
9 ROCK CHUTE SOUTH
 N.T.S.



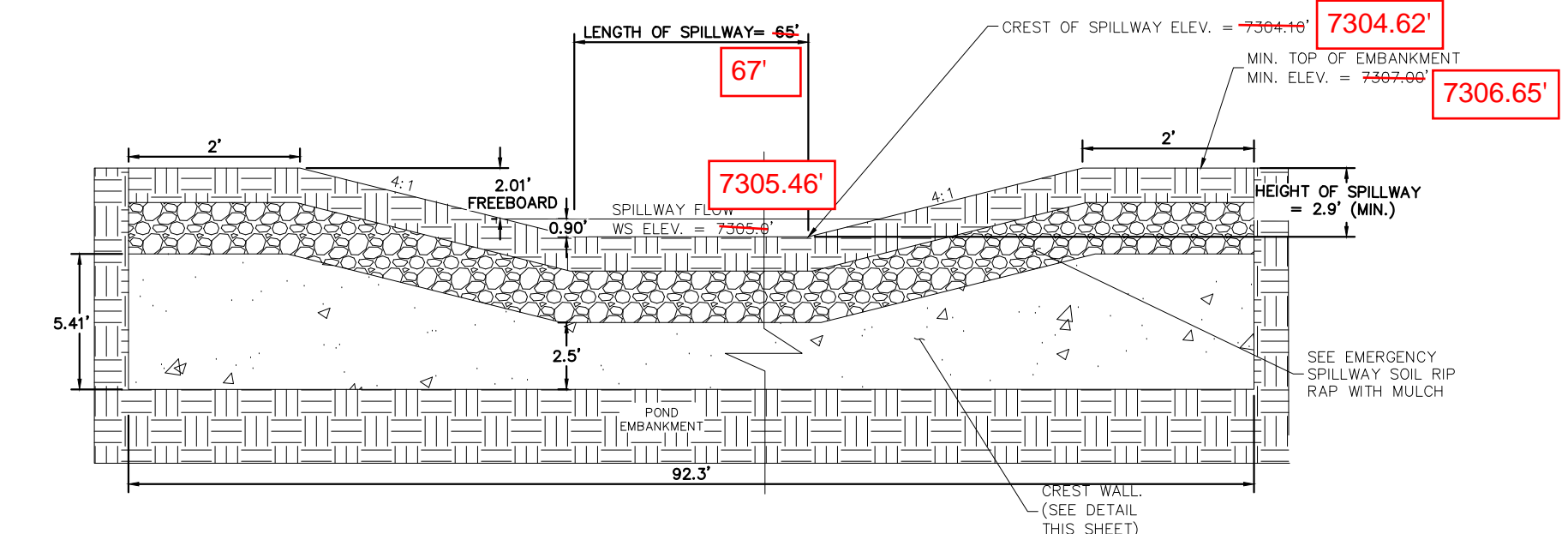
13 SECTION ROCK CHUTE NORTH DETAIL
 N.T.S.



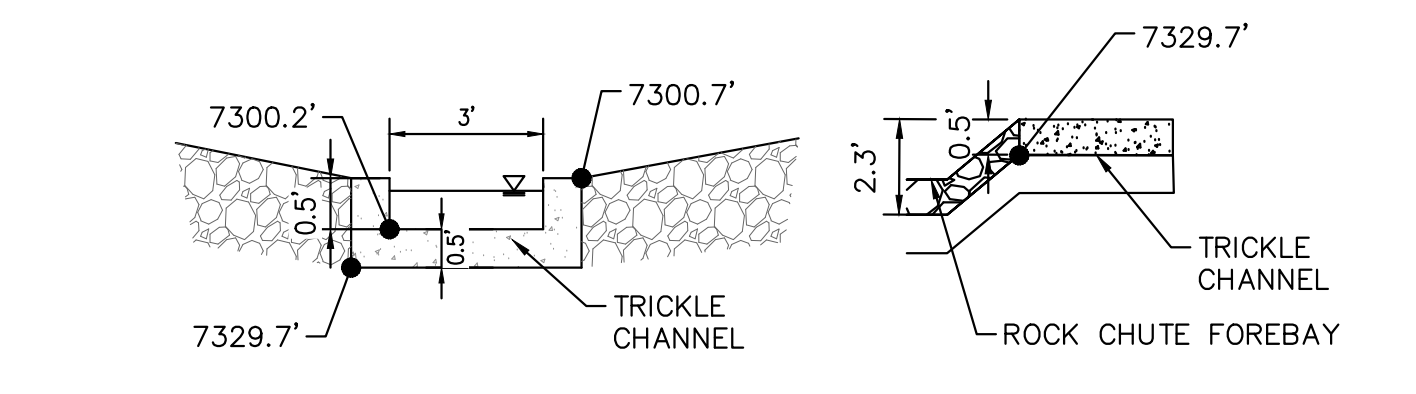
14 SECTION ROCK CHUTE SOUTH DETAIL
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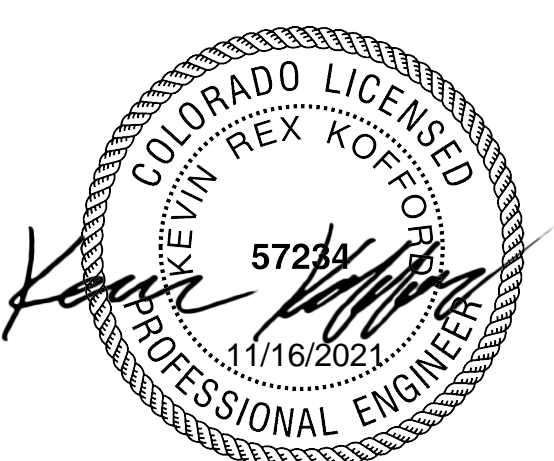
11 EMERGENCY SPILLWAY DETAILS
 N.T.S.



12 EMERGENCY SPILLWAY
 N.T.S.



15 ROCK CHUTE TO TRICKLE CHANNEL TRANSITION
 N.T.S.



Kimley >>> Horn

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LICENSED PROFESSIONAL ENGINEER
 KEVIN KOFFORD
 CO LICENSE NUMBER 57234
 CHECKED BY: KHTAM
 DATE: 08/25/2021

REVISIONS
 NO. DATE BY

4	10/29/21	KRK
3	9/15/21	KRK
2	8/25/21	KRK

POND 5 DETAILS

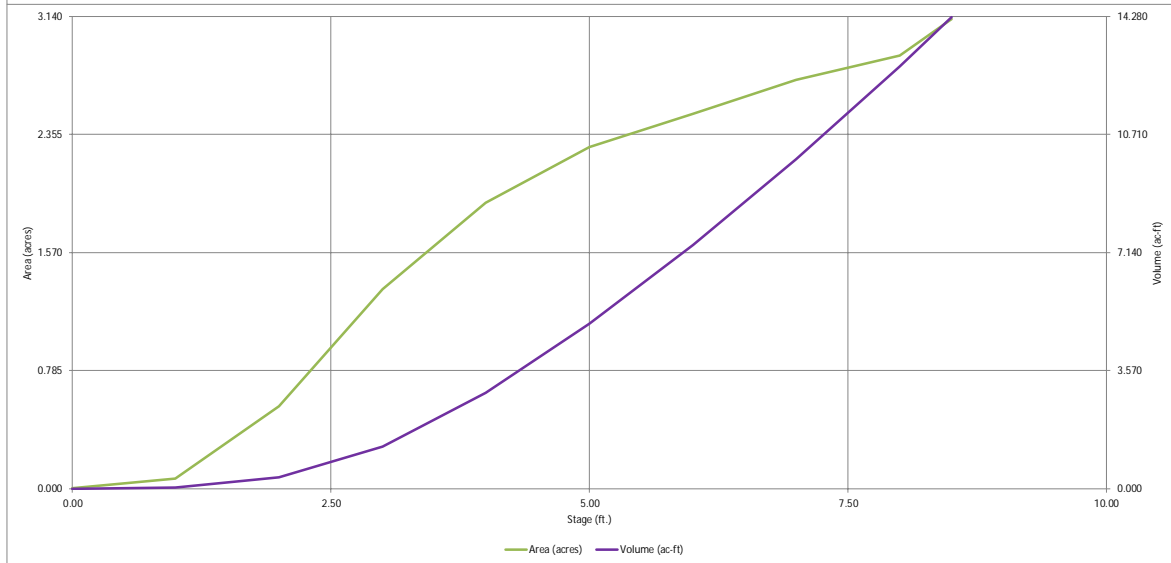
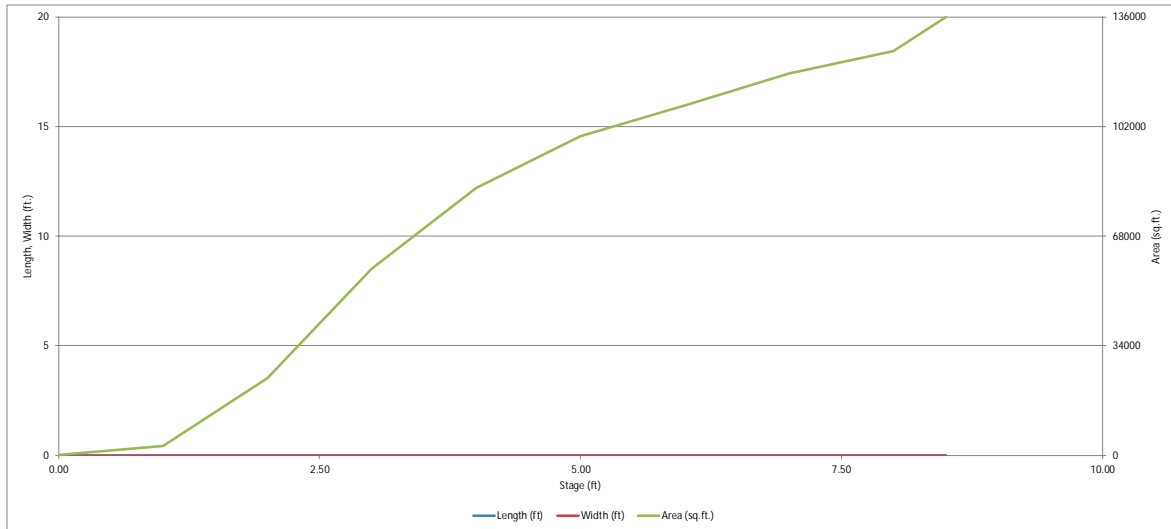
WINSOME FILING NO. 2
 PREPARED FOR
WINSOME LLC

EL PASO COUNTY

SHEET NUMBER
31

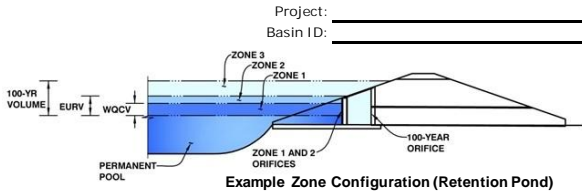
DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)



DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.81	1.033	Orifice Plate
Zone 2 (EURV)	3.39	0.792	Orifice Plate
Zone 3 (100-year)	5.51	4.339	Weir&Pipe (Restrict)
Total (all zones)		6.164	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft ²
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	N/A	ft ²
Depth at top of Zone using Orifice Plate =	3.28	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	N/A	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	N/A	sq. inches	Elliptical Slot Area =	N/A	ft ²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.11	2.19					
Orifice Area (sq. inches)	3.14	3.14	10.00					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice =	Not Selected	Not Selected	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	Not Selected	Not Selected	ft ²
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	N/A	N/A	feet
Vertical Orifice Diameter =	N/A	N/A	inches				

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

Overflow Weir Front Edge Height, Ho =	Zone 3 Weir: 3.28	Not Selected: N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Gate Upper Edge, H ₁ =	Zone 3 Weir: 4.78	Not Selected: N/A	feet
Overflow Weir Front Edge Length =	10.00	N/A	feet	Overflow Weir Slope Length =	6.18	N/A	feet
Overflow Weir Gate Slope =	4.00	N/A	H:V	Gate Open Area / 100-yr Orifice Area =	5.31	N/A	
Horiz. Length of Weir Sides =	6.00	N/A	feet	Overflow Gate Open Area w/o Debris =	43.05	N/A	ft ²
Overflow Gate Type =	Type C Gate	N/A		Overflow Gate Open Area w/ Debris =	21.52	N/A	ft ²
Debris Clogging % =	50%	N/A	%				

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe =	Zone 3 Restrictor: 1.64	Not Selected: N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	Zone 3 Restrictor: 8.11	Not Selected: N/A	ft ²
Outlet Pipe Diameter =	42.00	N/A	inches	Outlet Orifice Centroid =	1.51	N/A	feet
Restrictor Plate Height Above Pipe Invert =	33.00		inches	Half-Central Angle of Restrictor Plate on Pipe =	2.18	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	6.53	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth =	0.84	feet
Spillway Crest Length =	67.00	feet	Stage at Top of Freeboard =	8.37	feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	3.06	acres
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =	13.86	acre-ft

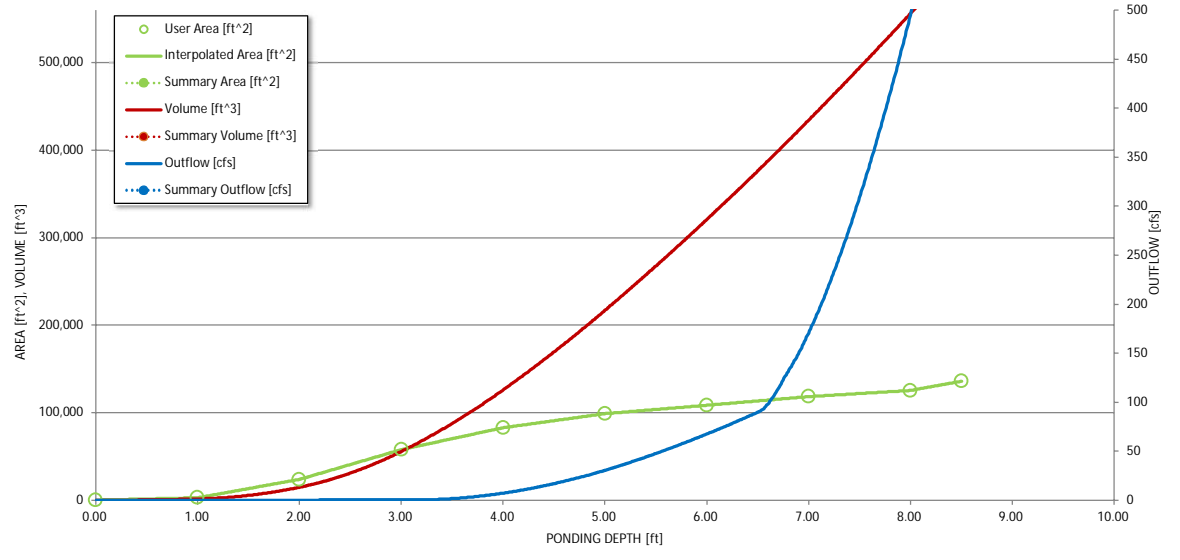
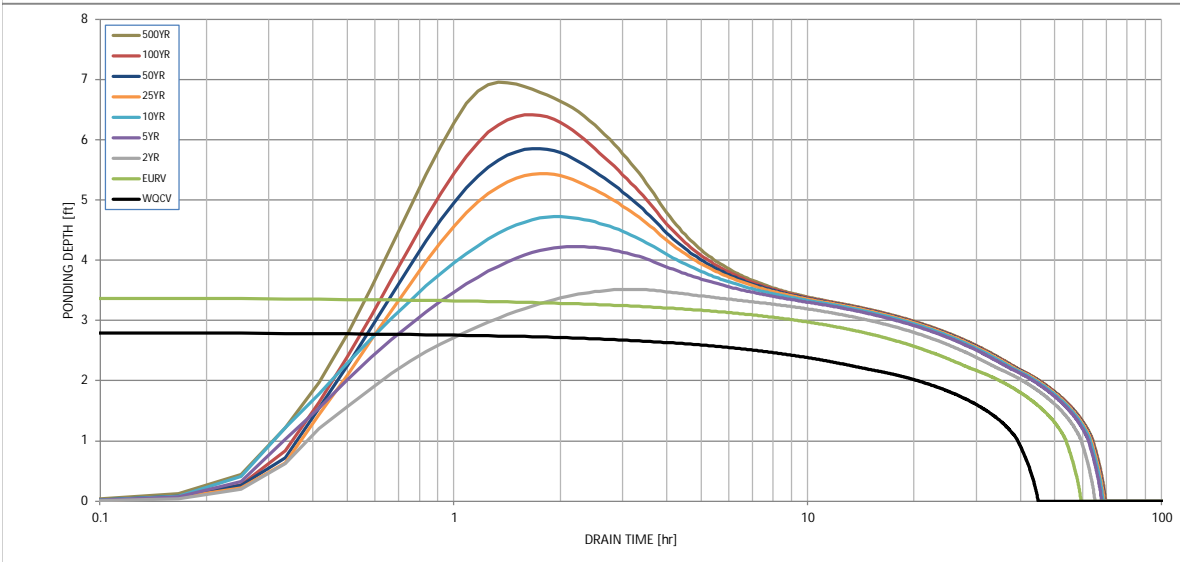
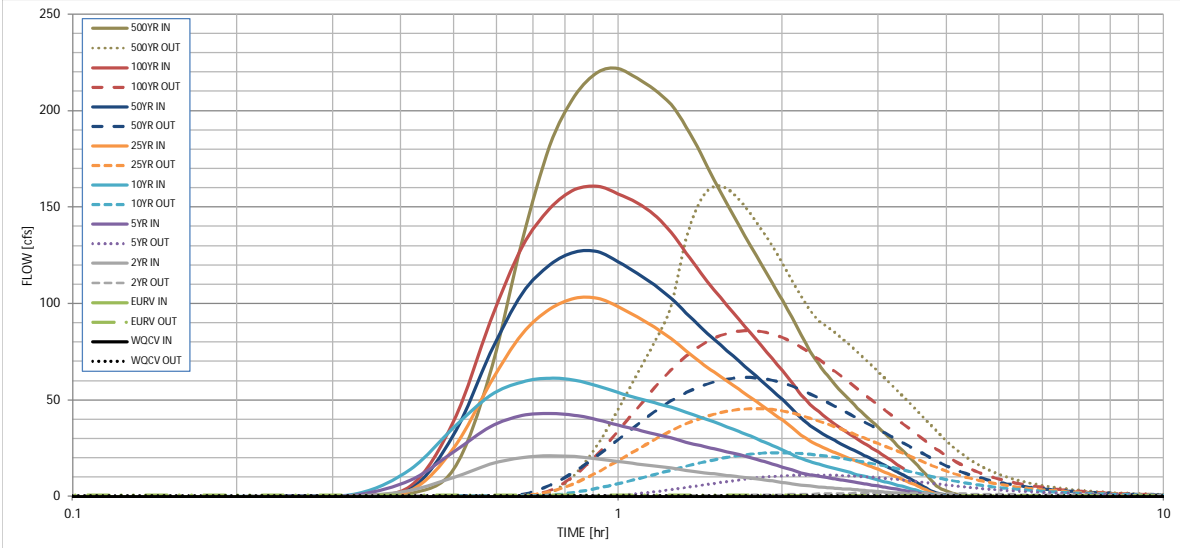
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.14
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.14
CUHP Runoff Volume (acre-ft) =	1.033	1.825	2.329	4.848	7.343	11.651	14.636	18.889	26.699
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	2.329	4.848	7.343	11.651	14.636	18.889	26.699
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	11.5	32.3	50.1	92.4	116.2	149.4	209.6
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.08	0.21	0.33	0.61	0.77	0.99	1.39
Peak Inflow Q (cfs) =	N/A	N/A	21.2	43.2	61.4	102.9	127.0	161.0	221.9
Peak Outflow Q (cfs) =	0.6	1.0	1.8	11.2	22.8	45.6	61.8	86.1	160.9
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.3	0.5	0.5	0.5	0.6	0.8
Structure Controlling Flow =	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Spillway
Max Velocity through Gate 1 (fps) =	N/A	0.01	0.03	0.2	0.5	1.0	1.4	2.0	2.4
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	40	52	56	54	50	44	40	36	30
Time to Drain 99% of Inflow Volume (hours) =	43	56	61	61	59	57	55	53	49
Maximum Ponding Depth (ft) =	2.81	3.39	3.52	4.23	4.73	5.44	5.86	6.42	6.96
Area at Maximum Ponding Depth (acres) =	1.18	1.55	1.63	1.99	2.17	2.37	2.46	2.59	2.71
Maximum Volume Stored (acre-ft) =	1.040	1.840	2.046	3.341	4.359	6.001	6.991	8.428	9.831

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.04
	0:15:00	0.00	0.00	0.10	0.16	0.20	0.14	0.19	0.17	0.29
	0:20:00	0.00	0.00	0.49	1.13	1.74	0.53	0.64	0.79	1.76
	0:25:00	0.00	0.00	3.30	8.51	14.40	3.22	4.09	5.67	14.18
	0:30:00	0.00	0.00	10.04	23.37	35.96	25.12	32.23	39.26	64.00
	0:35:00	0.00	0.00	16.78	36.26	52.47	58.75	74.25	90.66	132.23
	0:40:00	0.00	0.00	20.29	42.06	59.52	84.32	105.06	129.25	181.76
	0:45:00	0.00	0.00	21.16	43.19	61.45	97.12	120.12	149.38	207.63
	0:50:00	0.00	0.00	20.76	42.27	60.50	102.76	126.81	159.08	219.90
	0:55:00	0.00	0.00	19.63	39.92	57.34	102.91	127.02	160.98	221.91
	1:00:00	0.00	0.00	18.25	37.09	53.90	98.48	121.80	157.02	216.82
	1:05:00	0.00	0.00	17.06	34.66	51.15	93.16	115.74	152.34	211.03
	1:10:00	0.00	0.00	15.98	32.56	48.72	87.83	109.65	146.11	203.21
	1:15:00	0.00	0.00	14.83	30.44	46.35	81.93	102.77	137.02	191.74
	1:20:00	0.00	0.00	13.66	28.27	43.78	75.67	95.25	126.53	178.07
	1:25:00	0.00	0.00	12.63	26.39	41.26	69.62	87.81	116.13	164.17
	1:30:00	0.00	0.00	11.81	24.85	38.76	64.54	81.51	107.17	151.79
	1:35:00	0.00	0.00	11.05	23.37	36.26	59.89	75.69	99.10	140.50
	1:40:00	0.00	0.00	10.32	21.85	33.79	55.57	70.26	91.74	130.10
	1:45:00	0.00	0.00	9.61	20.25	31.37	51.44	65.06	84.82	120.28
	1:50:00	0.00	0.00	8.90	18.63	29.00	47.45	60.06	78.14	110.85
	1:55:00	0.00	0.00	8.17	17.00	26.65	43.53	55.15	71.64	101.68
	2:00:00	0.00	0.00	7.43	15.39	24.24	39.67	50.32	65.30	92.75
	2:05:00	0.00	0.00	6.68	13.79	21.79	35.81	45.48	59.04	83.91
	2:10:00	0.00	0.00	6.00	12.42	19.75	31.98	40.66	52.86	75.38
	2:15:00	0.00	0.00	5.48	11.42	18.20	28.92	36.84	47.88	68.50
	2:20:00	0.00	0.00	5.09	10.62	16.89	26.56	33.87	43.95	62.93
	2:25:00	0.00	0.00	4.75	9.89	15.69	24.58	31.33	40.57	58.07
	2:30:00	0.00	0.00	4.42	9.20	14.55	22.81	29.05	37.54	53.69
	2:35:00	0.00	0.00	4.11	8.55	13.47	21.20	26.97	34.77	49.67
	2:40:00	0.00	0.00	3.81	7.91	12.43	19.68	25.01	32.18	45.91
	2:45:00	0.00	0.00	3.51	7.29	11.43	18.22	23.13	29.76	42.40
	2:50:00	0.00	0.00	3.23	6.68	10.46	16.82	21.34	27.51	39.13
	2:55:00	0.00	0.00	2.95	6.09	9.54	15.45	19.59	25.31	35.96
	3:00:00	0.00	0.00	2.68	5.51	8.65	14.10	17.88	23.14	32.85
	3:05:00	0.00	0.00	2.41	4.94	7.77	12.76	16.19	20.98	29.76
	3:10:00	0.00	0.00	2.13	4.37	6.91	11.42	14.50	18.82	26.69
	3:15:00	0.00	0.00	1.87	3.82	6.06	10.10	12.82	16.66	23.62
	3:20:00	0.00	0.00	1.60	3.26	5.21	8.77	11.15	14.51	20.56
	3:25:00	0.00	0.00	1.33	2.72	4.36	7.45	9.48	12.37	17.51
	3:30:00	0.00	0.00	1.07	2.17	3.53	6.13	7.82	10.23	14.47
	3:35:00	0.00	0.00	0.81	1.63	2.70	4.82	6.17	8.10	11.44
	3:40:00	0.00	0.00	0.56	1.10	1.91	3.52	4.53	5.98	8.46
	3:45:00	0.00	0.00	0.35	0.71	1.36	2.28	2.97	3.97	5.76
	3:50:00	0.00	0.00	0.23	0.51	1.05	1.46	1.98	2.66	4.01
	3:55:00	0.00	0.00	0.18	0.40	0.84	0.98	1.38	1.83	2.86
	4:00:00	0.00	0.00	0.14	0.32	0.68	0.68	0.98	1.25	2.03
	4:05:00	0.00	0.00	0.12	0.26	0.55	0.46	0.69	0.84	1.41
	4:10:00	0.00	0.00	0.09	0.21	0.43	0.33	0.50	0.54	0.95
	4:15:00	0.00	0.00	0.07	0.16	0.33	0.23	0.36	0.33	0.62
	4:20:00	0.00	0.00	0.06	0.12	0.25	0.16	0.25	0.20	0.39
	4:25:00	0.00	0.00	0.05	0.09	0.18	0.12	0.19	0.15	0.29
	4:30:00	0.00	0.00	0.04	0.07	0.13	0.09	0.14	0.11	0.22
	4:35:00	0.00	0.00	0.03	0.05	0.10	0.07	0.11	0.09	0.17
	4:40:00	0.00	0.00	0.02	0.04	0.08	0.05	0.08	0.07	0.13
	4:45:00	0.00	0.00	0.02	0.02	0.05	0.04	0.06	0.05	0.10
	4:50:00	0.00	0.00	0.01	0.02	0.04	0.03	0.04	0.04	0.07
	4:55:00	0.00	0.00	0.01	0.01	0.02	0.02	0.03	0.03	0.05
	5:00:00	0.00	0.00	0.00	0.01	0.01	0.01	0.02	0.02	0.03
	5:05:00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.02
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00