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Preliminary and Final Drainage Report  
For  
**Mountain's Edge**  
Calhan, Colorado

June 23, 2010  
Approved March 9, 2011  
Revised and Updated February 5, 2021

**Prepared For:**

**OGC RE2, LLC  
P.O. Box 1385  
Colorado Springs, CO 80901  
Attn: Kelli O'Neil**

**Prepared By:**

**Land Development Consultants, Inc.  
Colorado Springs, Colorado 80909  
(719) 528-6133**

**Project # 08019**

**PCD Fil No. SF201**

# **TABLE OF CONTENTS**

## **I. CERTIFICATIONS**

Engineer's Statement  
Developer's Statement  
El Paso County  
Conditions

## **II. GENERAL INFORMATION**

Purpose  
Location of Property  
(City, County, surrounding development)  
Description of Property  
(area, topography, soils, drainageways, irrigation, utilities)  
Drainage Basins and Sub Basins  
(Major Basin Description, Sub-Basin Description)

## **III. DRAINAGE DESIGN CRITERIA**

Development Criteria references  
(Criteria References, Previous Studies)  
Hydrologic Criteria

## **IV. DRAINAGE FACILITY DESIGN**

General Drainage Concept  
Specific Drainage Details (Existing and Proposed)

## **V. DRAWING CONTENTS:**

Vicinity Map (See Appendix)  
Drainage Plans (Existing and Proposed) (See Appendix)

## **VI. IMPROVEMENT COSTS**

## **VII. DRAINAGE FEES**

## **VIII. REFERENCES**

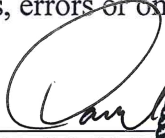
## **IX. APPENDICES**

Location (Vicinity Map)  
Floodplain (FEMA FIRM Map)  
Soils (S.C.S. Soils Classification)  
Hydrology (Criteria & Calculations)  
Overall Drainage Map  
Exhibits (Drainage Plans- Existing and Proposed)

## I. CERTIFICATIONS:

### **Engineer's Statement**

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

  
\_\_\_\_\_  
David R. Gorman, P.E. CO 3167  
For and on behalf of LDC, Inc.



\_\_\_\_\_  
2/10/2021

DATE:

### **Developer's Statement**

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

OGC RE2, LLC

BY: \_\_\_\_\_

  
Kelli O'Neil

TITLE: Manager

ADDRESS: P.O. Box 1385, Colorado Springs, CO 80901

### **El Paso County**

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

\_\_\_\_\_  
Jennifer Irvine, P.E.  
County Engineer / ECM Administrator

\_\_\_\_\_  
DATE

## **II. GENERAL INFORMATION:**

### **Purpose**

The purpose of this Preliminary and Final Drainage Report is to analyze the existing and proposed drainage facilities, determine runoff quantities from both on-site and off-site sources, ensure adequacy of existing facilities, size any new proposed facilities, and present solutions for proper conveyance of developed storm water runoff.

### **Location of Property (City/County, Major Drainageways and Existing Facilities, FEMA Floodplain, Surrounding Development)**

#### City/County

A tract of land being the North one-half of the North one-half of the Southwest One-Quarter of Section 13, Township 12 South, Range 63 West of the 6th P.M., situated in El Paso County, Colorado, encompassing approximately 30.613 acres.

#### Major drainageways and existing facilities

This tract of land is roughly rectangular in shape, with the long axis running east/west. A minor drainageway of approximately 68.89 acres passes through the parcel, draining north to south. There is no FEMA designated flood hazard area associated with this drainageway. A larger drainageway draining north to south of approximately 2,350 acres is located offsite to the east but does not pass through the subject parcel. This offsite drainageway is subject to a FEMA Zone A flood hazard designation.

#### FEMA Floodplain Statement

The current Flood Insurance Study of the region includes Flood Insurance Rate Maps (FIRMs), effective December 7, 2018. The project site is included in Community Panel Number 08041C0375 G of the FIRMs for El Paso County, Colorado. No portion of the site lies within FEMA designated Special Flood Hazard Areas (SFHAs). An excerpt of the current FEMA Flood Insurance Rate Maps with the site delineated is included in the Appendix.

#### Surrounding Development

The site is bordered to the north, east and south by single-family, multi-acre rural housing and to the west by McClelland Road, a gravel county road, beyond which to the west is more single-family, multi-acre rural housing.

### **Description of Property (Area, Topography, Soils, Drainageways, Irrigation, Utilities)**

#### Area

The project involves platting the 30.613-acre parcel into 5, single-family rural residential homesites of approximately 5 to 7 acres each.



### Topography

Currently the site is undeveloped and consists of two southeast trending drainages separated by a ridge. Surficial slopes range from 1% to 6% with the most southeasterly ridge sloping east from 5% to 11%. Vegetation on the property consists of native grasses with some weeds and cactus.

### Soils

A 'Geology and Soils Study' was prepared for this site by Kumar and Associates, dated November 14, 2008. Their soils analysis identified five soils unit types on site, predominately sandy loams with an SCS 'B' grouping and some loamy sand on the ridge tops with an SCS 'A' grouping. Geological recommendations were also made and consisted of the delineation of two "No-Build" areas within this site that is prone to flooding. See attached NRCS soils group map and proposed Drainage Plan in the Appendix.

### Drainageways

The westerly contributing watershed begins about one-half mile north and drains southerly, parallel to McClelland Road. Flows from the off-site basin enters this site from the north, continue unimpeded through the site and exit the south boundary, joining the larger offsite drainageway flows at a point just north of Scott Road. The easterly watershed drains easterly offsite and into the adjacent offsite drainage. Development of this property into rural residential home sites will have a minimal impact on both the calculated runoff quantity and the storm water quality characteristics of the existing conditions.

### Irrigation

No irrigation canals cross this property.

### Utilities

No public utilities are known to exist on this property. A domestic water steel well head was observed on the property.

## **Drainage Basins and Sub-Basins (Major Basin Description, Sub-Basin and Existing Flow Summary)**

### Major Basin Description

The site is located within the Upper Bracket Creek Major Drainage Basin (CHBR0600) located in the US Highway 24 area between Peyton and Calhan. This major drainage basin has not been studied and has no Drainage Basin Planning Study. The basin contains approximately 46 square miles and drains from northwest to southeast, extending from approximately 5 miles upstream of US Highway 24 and approximately 5 miles downstream of US Highway 24. The Upper Bracket Creek Drainage Basin does not have drainage fees associated with development within the basin.

### Sub-Basin Description

In order to analyze runoff in the existing conditions, three contributing drainage sub-basins were delineated and studied.

Basin A contains approximately 6.19 acres and drains the east end of the undeveloped site property sheet flows to the east into the Upper Bracket Creek drainageway. Calculated runoff rates for Basin A are Q5 of 1.6 cfs and Q100 of 11.8 cfs.

Basin B contains approximately 68.89 acres including the central portion of the undeveloped site property as well as an area north of the site. Flows passes through the site in a broad swale and exit to the south in an existing narrow swale. Calculated runoff rates for Basin B are Q5 of 9.7 cfs and Q100 of 63.0 cfs.

Basin C contains approximately 2.24 acres and drains the existing McClelland roadway as well as a portion of the existing undeveloped property. These flows continue in the existing roadside ditch flow south toward Scott Road. Calculated runoff rates for Basin C are Q5 = 1.6 cfs and Q100 = 6.0 cfs.

Runoff computations were prepared for the 5-year (minor) and the 100-year (major) storm frequency. See attached Existing Drainage Plan and calculations located in the Appendix of this report.

### **III. DRAINAGE DESIGN CRITERIA:**

#### **Development Criteria References (Criteria References, Previous Studies)**

##### Criteria References

The following were used for references for the calculations within this report:

1. **City of Colorado Springs and El Paso County Drainage Criteria Manual, Volume 1 and 2, May 2014.** The charts and graphs used from these manuals have been reproduced and are included within this report in the appropriate section.
2. **NRCS Web Soil Survey.** United States Department of Agriculture, Natural Resources Conservation Service.
3. **Geology and Soils Study, Mountains Edge Development,** prepared by Kumar and Associates on November 14, 2008.

##### Previous Studies

The “Preliminary and Final Drainage Report for Mountain’s Edge, Calhan Colorado” prepared by Land Development Consultants, Inc. and dated June 23, 2010 was approved by El Paso County on March 9, 2011. This report was for the same property and subdivision as the current report, except the east side end of the originally approved subdivision is no longer contained within the property and proposed lot lines have been adjusted from the previous layout.

#### **Hydrologic Criteria**

The Rational Method as described in the Drainage Criteria Manual has been used for all Storm Runoff calculations, as the development and all sub-basins are less than 130 acres in area.

“Colorado Springs Rainfall Intensity Duration Frequency” curves, Figure 6-5 in the DCM, was used to obtain the design rainfall values; a copy is included in the Appendix. The “Overland (Initial) Flow Equation” (Eq. 6-8) in the DCM, and Manning’s equation with estimated depths were used in time of concentration calculations. “Runoff Coefficients for Rational Method”, Table 6-6 in the DCM, was utilized as a guide in estimating runoff coefficient and Percent Impervious values; a copy is included in the Appendix.

## **IV. DRAINAGE FACILITY DESIGN:**

### **General Drainage Concept**

To access the home sites, a public gravel road will be built adjacent to the north site boundary to be named Farmhouse Court. Basin B main channel runoff will be passed directly under this road through a double 38” by 24” RCP culverts with flared end sections. Basin B upstream sheet flow from the north will be collected, concentrated, and conveyed via the roadside borrow ditches to this same culvert. This road is the only substantial diversion of the historic drainage flow pattern. The ditch flows along McClelland Road and will continue through Basin C, passing under the proposed roadway through a 14” by 23” elliptical RCP culvert.

The site is a 5-lot rural residential development with lot sizes more than 5 acres each and features a single gravel roadway. The increase in impervious area from existing conditions is minimal compared to the overall land area in the site. The increase in Basin A flow rates for the 5-year and 100-year storm events are  **$Q_5 = 0.5 \text{ cfs}$  and  $Q_{100} = 0.6 \text{ cfs}$**  (for 6.2 acres). The increase in Basin B flow rates for the 5-year and 100-year storm events are  **$Q_5 = 1.9 \text{ cfs}$  and  $Q_{100} = 2.8 \text{ cfs}$**  (for 68.9 acres). The increase in Basin C flow rates for the 5-year and 100-year storm events are  **$Q_5 = 0.0 \text{ cfs}$  and  $Q_{100} = 0.0 \text{ cfs}$**  (for 2.2 acres). In comparing the increase in flows from existing conditions to developed conditions, the calculated increase in the 5-year and 100-year rainfall events are insignificant and negligible for the 30.60-acre rural residential subdivision site. Therefore, no detention is required.

Since the site is a large lot single-family rural residential site with greater than 2.5 acres per dwelling; and the total lot imperviousness is less than 10% (2.14 impervious acres/30.6 total acres = 7%) single-family rural residential lot, the residential lot portions of the site are excluded from water quality requirements for water quality capture volume by ECM 1.7.1.B.5. Water Quality for the public roadway is accomplished by the vegetated roadside ditches serving as Green Infrastructure in accordance with the Runoff Reduction Standard of ECM 1.7.1.C.3. Also, permanent water quality BMP’s are included in the form of rock check dams at the roadside ditch exits on Farmhouse Court. These are discussed in a following sections.

### **Specific Drainage Details**

- **Existing Drainage Conditions**

Existing sub-basins and flows are described in Section II, above. The details of the existing sub-basin characteristics are listed below.

**Basin A**

**6.19-acres      undeveloped       $C = 0.08/0.35$        $Q_5 / Q_{100} = 1.6 / 11.8$  cfs**

**Basin B**

**68.89-acres      rural residential       $C = 0.09/0.36$        $Q_5 / Q_{100} = 9.7 / 63.0$  cfs**

**Basin C**

**2.25-acres      gravel road/rural residential       $C = 0.19/0.43$        $Q_5/Q_{100} = 1.6 / 6.0$  cfs**

**Specific Drainage Details**

- Proposed Drainage Conditions**

Contributing basins in the developed condition range in size from 0.57 to 40.2 acres. Details of the proposed condition sub-basin characteristics are listed below.

**Sub-Basin A1**

**6.19-acres      5 Ac rural residential       $C = 0.10/0.36$        $Q_5/Q_{100} = 2.1 / 12.4$  cfs**

Sub-Basin A1 drains east and then offsite to the adjacent offsite drainage way.

**Sub-Basin B1**

**40.20-acres      rural residential       $C = 0.10/0.36$        $Q_5/Q_{100} = 7.0 / 42.4$  cfs**

Sub-Basin B1 drains the area north of the site and flows directly into the existing broad swale. These flows will be intercepted by the proposed roadway culvert in Farmhouse Court.

**Sub-Basin B2**

**2.16-acres      rural residential       $C = 0.14/0.39$        $Q_5/Q_{100} = 0.8 / 3.6$  cfs**

Sub-Basin B2 drains the northern portion of Farmhouse Court from the roadway high point west to the proposed culvert. It also drains a portion of the undeveloped property north of the site. These flows will combine with the flows from Sub-Basin B1 at the proposed culvert. A permanent Rock Check for water quality will be placed upstream of the proposed culvert.

**Sub-Basin B3**

**3.75-acres      rural residential       $C = 0.11/0.37$        $Q_5/Q_{100} = 1.3 / 6.9$  cfs**

Sub-Basin B3 drains the northern portion of Farmhouse Court from the proposed culvert to the cul-de-sac. It also drains an area north of the site. These flows will combine with the flows from Sub-Basins B1 and B2 at the proposed culvert. A permanent Rock Check for water quality will be placed upstream of the proposed culvert.

**Design Point 1 DP1**

**46.11-acres       $C = 0.10/0.37$        $Q_5/Q_{100} = 8.3 / 49.0$  cfs**

Design Point DP1 includes flows from Sub-Basins B1, B2, and B3 and is located at the inlet of the proposed double 38" by 24" RCEP culverts. The resulting headwater elevation on the north side of the road is contained in the north roadside ditch and does not affect the upstream property on the north. These flows continue south in culvert to DP2 on the south side of Farmhouse Court.

#### **Sub-Basin B4**

**0.57-acres    gravel roadway     $C = 0.36/0.54$      $Q_5/Q_{100} = 0.8 / 2.0$  cfs**

Sub-Basin B4 drains the southern portion of the Farmhouse Court from the high point west to the culvert outlet. A permanent Rock Check for water quality will be placed upstream of the proposed culvert. These flows will combine with the flows from DP1 at DP2.

#### **Design Point 2 DP2**

**46.68-acres     $C = 0.11/0.37$      $Q_5/Q_{100} = 8.6 / 49.7$  cfs**

The ditch flows from Sub-Basin B4 combine with the culvert flows from DP1 and flow south through sub-basin B5 in the existing broad swale discussed below.

#### **Sub-Basin B5**

**22.24-acres    5 Ac rural residential     $C = 0.11/0.37$      $Q_5/Q_{100} = 5.0 / 27.5$  cfs**

Sub-Basin B5 drains the area south of the proposed roadway. A permanent Rock Check for water quality will be placed upstream of the proposed culvert. The construction of single-family residences on five-acre to seven-acre lots will contribute minimally to current flows and will be mitigated within the existing broad swale to which these flows will be tributary. This swale has a bottom width of approximately 3 feet at its narrowest point, with mild side slopes. The existing swale was analyzed for capacity and velocities. The calculated 100-yr flow depth in the swale range from 0.4 feet to 1.1 feet. The calculated velocities in this swale for the 100-year storm range from 1.7 fps to 3.7 fps. The grassed cover is adequate to convey the minor and major runoff flows without erosion or sedimentation. The swale exits the site and continues flowing in a southwesterly direction. A no-build area is established through sub-basin B5 to contain the swale and prevent disturbance of the flow area.

#### **Design Point 3 DP3**

**69.92-acres     $C = 0.11/0.37$      $Q_5/Q_{100} = 11.6 / 65.8$  cfs**

Design Point 3 is the combination of flows from DP2 and Sub-Basin B5. These combined flows exit the site through the existing broad swale along the south property line. The existing swale is the current outfall for the site. The swale is well vegetated and stable with no signs of erosion or sedimentation. Developed flows in the swale represent a negligible increase from existing conditions. The existing swale was analyzed for capacity and velocities. It was found that the swale has more than adequate capacity in the existing and developed conditions and the existing stable grass cover in the swale is adequate protection for the 100-year flow velocities of 3.7 fps.

#### **Sub-Basin C1**

**0.64-acre    gravel road/rural residential     $C = 0.32/0.51$      $Q_5/Q_{100} = 0.8 / 2.1$  cfs**

Sub-Basin C1 drains the western portion of McClelland Road north of the proposed development. The existing ditch flows will be transported under Farmhouse Drive through a 23" by 14" elliptical RCP culvert. These flows will then continue south through Sub-Basin C2.

#### **Sub-Basin C2**

**1.60-acres    gravel road/rural residential     $C = 0.15/0.40$      $Q_5/Q_{100} = 0.9 / 3.9$  cfs**

Sub-Basin C2 drains the western portion of McClelland Road from Farmhouse Court south to the property line and including a portion of the residential property.

**Design Point 4 DP4**

**2.23-acres      C = 0.20/0.43    Q<sub>5</sub>/Q<sub>100</sub> = 1.6 / 5.9 cfs**

Design Point 4 is the combination of flows from Sub-Basins C1 and C2. These flows will continue flowing south in the existing roadside ditch.

**V. DRAWING CONTENTS:**

**Vicinity Map**

**Overall Drainage Map**

**Drainage Plans (Existing and Proposed)**

**Drainage Calculations and Charts**

**VI. IMPROVEMENT COSTS (non-reimbursable):**

No offsite drainage improvements will be required. Costs for public non-reimbursable drainage improvements are listed below.

94 LF 38"x24" Elliptical RC Pipe	\$ 97/LF	=	\$ 9,118
47 LF 23"x14" Elliptical RC Pipe	\$ 65/LF	=	\$ 3,055
4 EA 38"x24" Elliptical FES	\$468/EA	=	\$ 1,872
2 EA 23"x24" Elliptical FES	\$390/EA	=	\$ 780
12 CY Rip Rap Culvert Outlet Pad	\$112/EA	=	\$ 1,344
4 EA Permanent Rock Check Dam	\$500/EA	=	<u>\$ 2,000</u>
Total			\$18,169

**VII. DRAINAGE FEES:**

The site is within the Upper Bracket Creek drainage basin, which is an unstudied basin. No drainage fees are due with the platting of the subdivision.

**VIII. FOUR STEP PROCESS:**

The El Paso County Engineering Criteria Manual (Appendix I, Section I.7.2) requires the consideration of a "Four Step Process for receiving water protection that focuses on reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainageways, and implementing long term source controls". The Four Step Process is incorporated in this project and the elements are discussed below.

The portion of the site that is contained within the 5-acre single family residential lots are excluded from Post Construction Stormwater Management requirements by ECM 1.7.1.B.5 due to the low development density as 5-acre lots. However, Farmhouse Court, a public roadway, is subject to Post Construction Stormwater Treatment requirements. This site will meet the requirements based the Runoff Reduction Standard indicated in ECM 1.7.1.C.3.

1) Runoff Reduction Practices are employed in this project. Impervious surfaces have been reduced as much as practically possible due to gravel roadways and reduced residential density. All impervious surfaces on the site will drain to the surrounding pervious areas allowing infiltration and water quality mitigation. Minimized Directly Connected Impervious Areas (MDCIA) is employed on the project because runoff passes through the western open space meadow area before leaving the site. Farmhouse Court, a public roadway, will have native vegetated roadside ditches that will capture and infiltrate runoff from the roadway surface. Runoff Reduction calculations are included in the appendix showing that the roadway runoff will infiltrate into the ground, evaporate, or evapotranspire a quantity of water equal to at least 60% of what the calculated WQCV would be if all impervious area for the applicable development site discharged without infiltration.

2) All drainage paths on the site are will remain stabilized with the natural native grass lining. Disturbed areas will be reseeded. All culverts will have rip-rap aprons at entrance and exits. The existing swale downstream of the new culvert crossing of proposed Farmhouse Court was analyzed for hydraulic depths velocities. The swale with the existing stable vegetative cover consisting of the natural native grasses on the site are adequate to convey the minor and major storm flows without erosion and sedimentation. No further stabilization is required.

3) The project contains no potentially hazardous uses. The site is exempted from the use of WQCV BMPs by ECM 1.7.1.B.5 by virtue of the large lot rural residential nature of the site having percent imperviousness of less than 10%. The site includes the use of permanent rock check dams at the roadside ditch exits to control potential sedimentation from the new roadside ditches. However, the roadside ditches of the public roadway will infiltrate into the ground, evaporate, or evapotranspire a quantity of water equal to at least 60% of what the calculated WQCV would be if all impervious area for the applicable development site discharged without infiltration. Runoff Reduction calculations are included in the appendix.

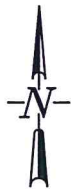
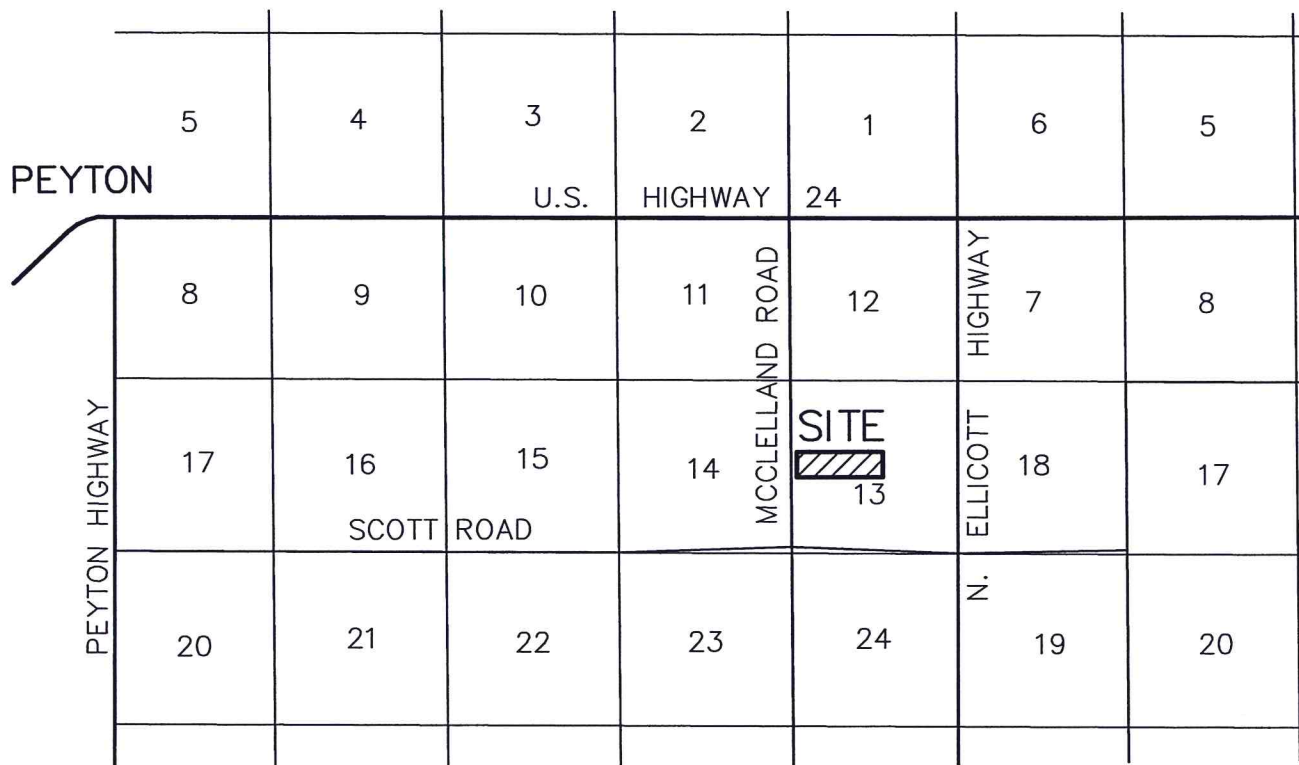
4) The rural residential site is not anticipated to contain storage of potentially harmful substances or use of potentially harmful substances. No Site Specific or Other Source Control BMP's are required.

## **IX. CONCLUSION:**

This Final Drainage Report presents existing and proposed drainage conditions for the proposed Mountains Edge subdivision project. Although storm detention is not provided, the large lot rural residential single-family development will have negligible and inconsequential increases in storm runoff flows with no effects on the existing site drainage and drainage conditions

downstream. The proposed project will not, with respect to stormwater runoff or water quality, negatively impact the adjacent properties and downstream properties.





## VICINITY MAP

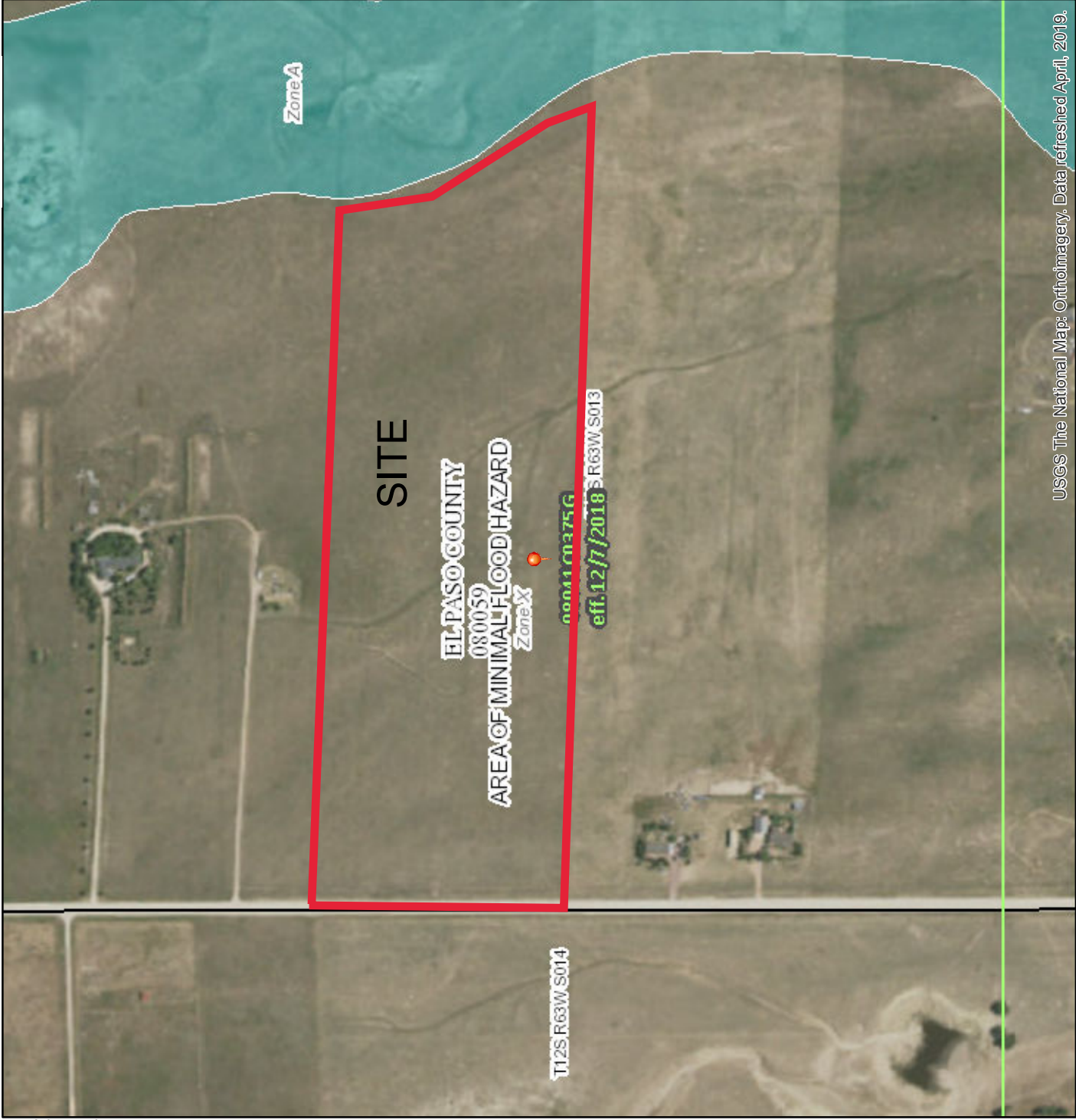
N.T.S.

# National Flood Hazard Layer FIRMette



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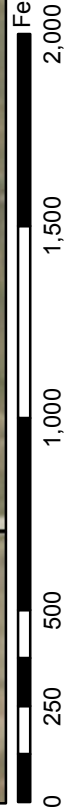
104°24'23.84"W



USGS The National Map: Orthoimagery, Data refreshed April, 2019.

38°59'58.07"N

Feet 1:6,000



## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

**SPECIAL FLOOD HAZARD AREAS**



**OTHER AREAS OF FLOOD HAZARD**



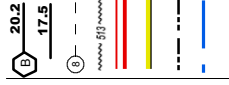
**OTHER AREAS**



**GENERAL STRUCTURES**



**OTHER FEATURES**



**MAP PANELS**



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

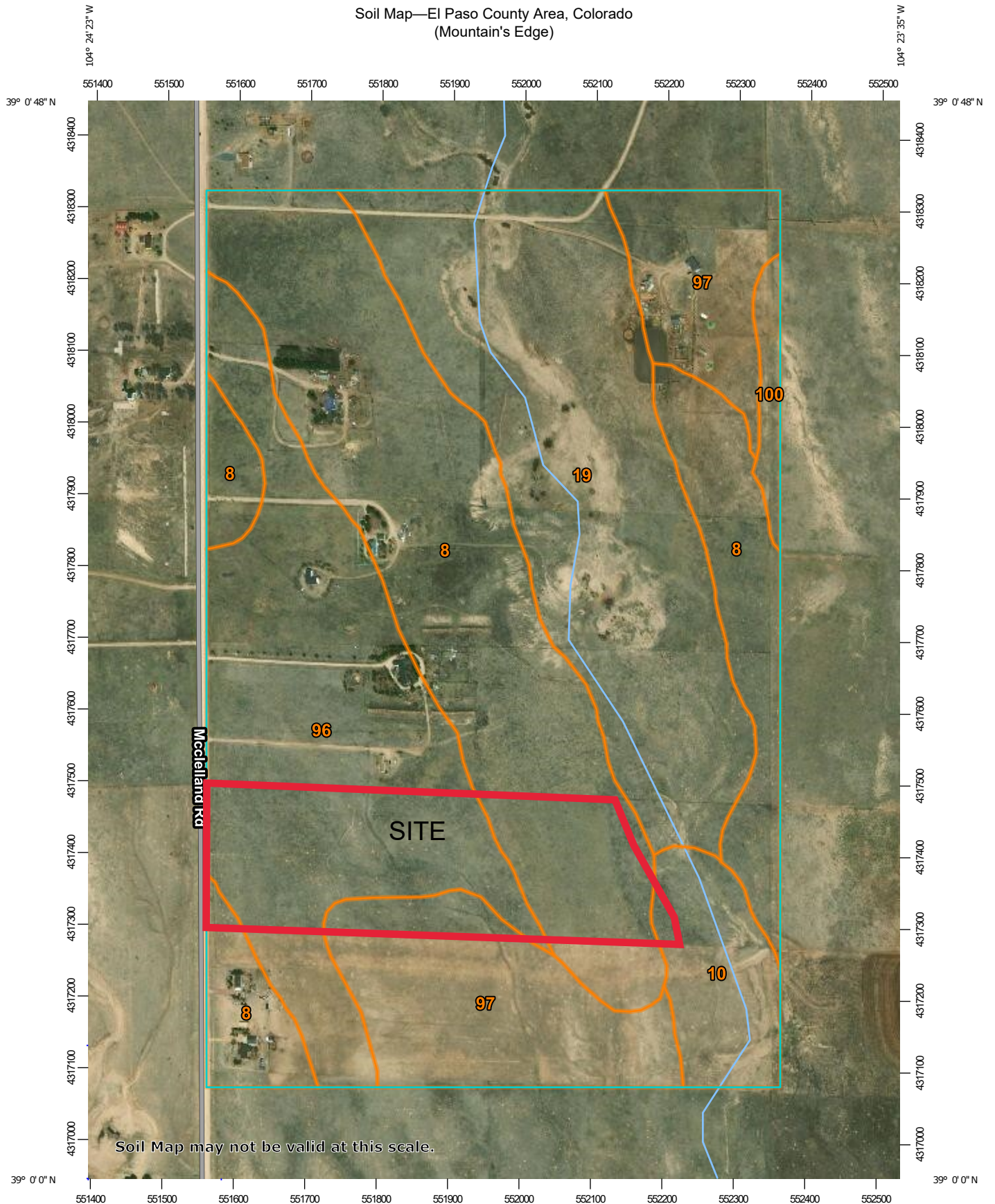
This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards.

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 6/20/2019 at 4:30:25 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

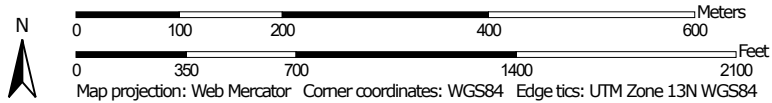


# Soil Map—El Paso County Area, Colorado (Mountain's Edge)



Soil Map may not be valid at this scale.

Map Scale: 1:7,330 if printed on A portrait (8.5" x 11") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84



























































**Natural Resources  
Conservation Service**

Web Soil Survey  
National Cooperative Soil Survey

6/20/2019  
Page 1 of 3

## MAP LEGEND

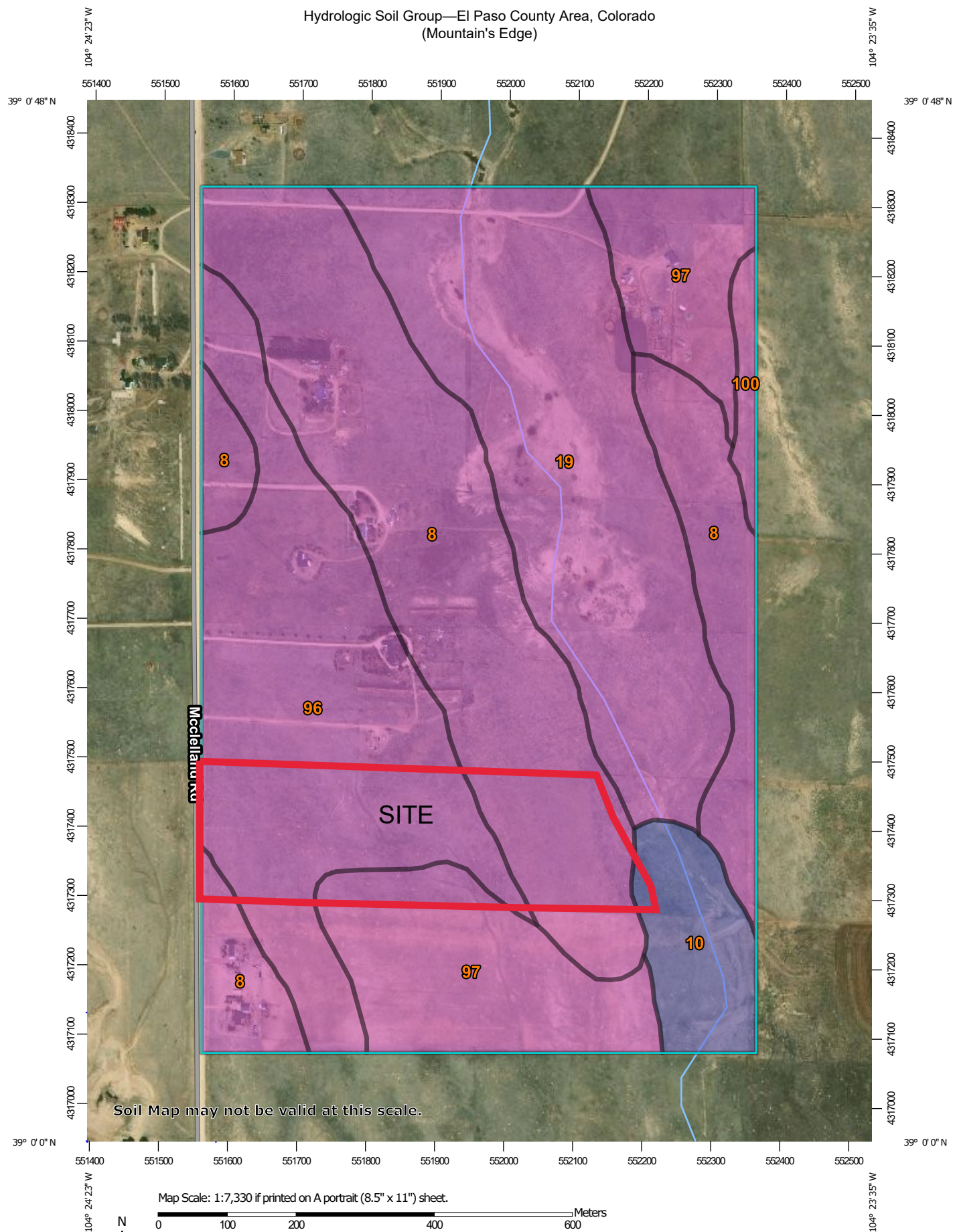
<b>Area of Interest (AOI)</b>		<b>Area of Interest (AOI)</b>	
<b>Soils</b>	  	<b>Soil Map Unit Polygons</b> <b>Soil Map Unit Lines</b> <b>Soil Map Unit Points</b>	    
<b>Special Point Features</b>	                   	<b>Special Point Features</b> Blowout Borrow Pit Clay Spot Closed Depression Gravel Pit Gravelly Spot Landfill Lava Flow Marsh or swamp Mine or Quarry Miscellaneous Water Perennial Water Rock Outcrop Saline Spot Sandy Spot Severely Eroded Spot Sinkhole Slide or Slip Sodic Spot	
<b>Water Features</b>	                         		

## Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	84.4	33.9%
10	Blendon sandy loam, 0 to 3 percent slopes	11.6	4.6%
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	58.6	23.5%
96	Truckton sandy loam, 0 to 3 percent slopes	55.7	22.4%
97	Truckton sandy loam, 3 to 9 percent slopes	35.9	14.4%
100	Truckton-Bresser complex, eroded	2.8	1.1%
<b>Totals for Area of Interest</b>		<b>249.1</b>	<b>100.0%</b>



# Hydrologic Soil Group—El Paso County Area, Colorado (Mountain's Edge)



Soil Map may not be valid at this scale.

Map Scale: 1:7,330 if printed on A portrait (8.5" x 11") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84




**Natural Resources  
Conservation Service**

Web Soil Survey  
National Cooperative Soil Survey

6/20/2019  
Page 1 of 4


MAP LEGEND


**Area of Interest (AOI)**


 Area of Interest (AOI)


**Soils**


**Soil Rating Polygons**


 A


 A/D


 B

 B/D


 C


 C/D


 D


 Not rated or not available


**Soil Rating Lines**


 A


 A/D


 B

 B/D


 C


 C/D


 D


 Not rated or not available

**Soil Rating Points**


 A

 A/D


 B


 B/D


**Water Features**


 Streams and Canals


**Transportation**

 Rails


 Interstate Highways

 US Routes


 Major Roads


 Local Roads


**Background**


 Aerial Photography

**C**

 C

 C/D

 D

 Not rated or not available

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

**Warning:** Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
Web Soil Survey URL:  
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado  
Survey Area Data: Version 16, Sep 10, 2018

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 7, 2016—Aug 17, 2017

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	A	84.4	33.9%
10	Blendon sandy loam, 0 to 3 percent slopes	B	11.6	4.6%
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A	58.6	23.5%
96	Truckton sandy loam, 0 to 3 percent slopes	A	55.7	22.4%
97	Truckton sandy loam, 3 to 9 percent slopes	A	35.9	14.4%
100	Truckton-Bresser complex, eroded	A	2.8	1.1%
<b>Totals for Area of Interest</b>			<b>249.1</b>	<b>100.0%</b>



## Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

## Rating Options

*Aggregation Method:* Dominant Condition

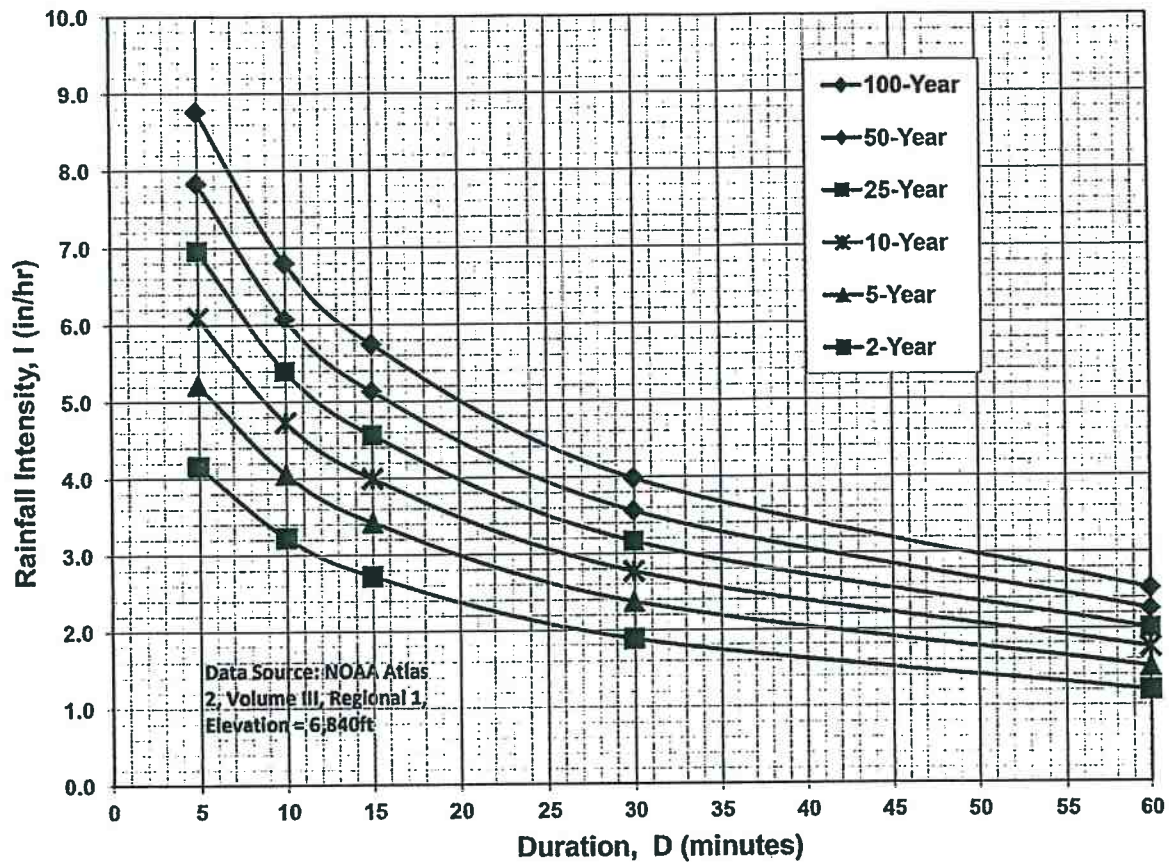
*Component Percent Cutoff:* None Specified

*Tie-break Rule:* Higher

**Table 6-6. Runoff Coefficients for Rational Method**  
(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients											
		2-year		5-year		10-year		25-year		50-year		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis--	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Greenbelts, Agriculture	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Exposed Rock	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Offsite Flow Analysis (when landuse is undefined)													
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

Figure 6-5. Colorado Springs Rainfall Intensity Duration Frequency



## IDF Equations

$$I_{100} = -2.52 \ln(D) + 12.735$$

$$I_{50} = -2.25 \ln(D) + 11.375$$

$$I_{25} = -2.00 \ln(D) + 10.111$$

$$I_{10} = -1.75 \ln(D) + 8.847$$

$$I_5 = -1.50 \ln(D) + 7.583$$

$$I_2 = -1.19 \ln(D) + 6.035$$

Note: Values calculated by equations may not precisely duplicate values read from figure.

**Time of Concentration** (Modified from Standard Form SF-1)

Sub-Basin	Sub-Basin Data				Overland			Shallow Channel				Channelized				t <sub>c</sub> Check		t <sub>c</sub> (min)
	Area (Acres)	C <sub>5</sub>	C <sub>100</sub> /CN	% Imp.	L <sub>0</sub> (ft)	S <sub>0</sub> (%)	t <sub>i</sub> (min)	L <sub>0t</sub> (ft)	S <sub>0t</sub> (ft/ft)	V <sub>osc</sub> (ft/s)	t <sub>t</sub> (min)	L <sub>0c</sub> (ft)	S <sub>0c</sub> (ft/ft)	V <sub>0c</sub> (ft/s)	t <sub>c</sub> (min)	L (min)	t <sub>c,alt</sub> (min)	
A (EX)	6.19	0.08	0.35	0%	300	7%	16.8	86	0.029	1.2	1.2	0	0.000	0.0	0.0	386	N/A	18.0
B (EX)	68.89	0.09	0.36	2%	300	2%	25.1	1400	0.025	1.1	21.1	1700	0.020	2.6	10.9	3400	N/A	57.0
C (EX)	2.24	0.19	0.43	17%	105	3%	11.3	0	0.000	0.0	0.0	269	0.019	2.7	1.7	374	N/A	13.0
A1	6.19	0.10	0.36	3%	300	7%	16.4	86	0.029	1.2	1.2	0	0.000	0.0	0.0	386	N/A	17.6
B1	40.20	0.10	0.36	3%	300	2%	23.6	1400	0.025	1.1	21.1	715	0.025	2.4	4.9	2415	N/A	49.6
B2	2.16	0.14	0.39	10%	300	2%	23.8	110	0.018	0.9	1.9	320	0.010	1.8	3.0	730	N/A	28.7
B3	3.75	0.11	0.37	5%	300	5%	18.1	235	0.047	1.5	2.6	200	0.010	2.0	1.7	735	N/A	22.4
B4	0.57	0.36	0.54	44%	32	2%	6.0	0	0.000	0.0	0.0	590	0.010	1.8	5.5	622	N/A	11.4
B5	22.24	0.11	0.37	5%	300	2%	26.1	730	0.019	1.0	12.6	410	0.022	2.1	3.3	1440	N/A	41.9
C1	0.64	0.32	0.51	37%	32	2%	6.3	0	0.000	0.0	0.0	740	0.018	2.2	5.6	772	N/A	11.9
C2	1.60	0.15	0.40	12%	105	3%	11.8	0	0.000	0.0	0.0	269	0.019	2.3	1.9	374	N/A	13.7



Sub-Basin and Combined Flows (Modified from Standard Form SF-2)

DP	Sub-Basin	Area (Acres)	C5	Direct Runoff				Combined Runoff				Streetflow		Pipe Flow				Travel Time		
				t <sub>c</sub> (min)	CA (Acres)	I5 (in/hr)	Q5 (cfs)	t <sub>c</sub> (min)	CA (Acres)	I5 (in/hr)	Q (cfs)	Slope (%)	Length (ft)	Q (cfs)	Slope (%)	Mnngs n	Length (ft)	D <sub>Pipe</sub> (in)	Length (ft)	V <sub>occ</sub> (ft/s)
	A (EX)	6.19	0.08	18.0	0.50	3.25	1.6													
	B (EX)	68.89	0.09	57.0	6.41	1.52	9.7													
	C (EX)	2.24	0.19	13.0	0.43	3.74	1.6													
	A1	6.19	0.10	17.6	0.63	3.28	2.1													
DP1	B1	40.20	0.10	49.6	4.06	1.73	7.0													
	B2	2.16	0.14	28.7	0.31	2.55	0.8													
	B3	3.75	0.11	22.4	0.43	2.92	1.3													
	B1,B2,B3	46.11	0.10					49.6	4.80	1.73	8.3									
DP2	B4	0.57	0.36	11.4	0.21	3.93	0.8													
	B1,B2,B3,B4	46.68	0.11					49.8	5.01	1.72	8.6									
DP3	B5	22.24	0.11	41.9	2.51	1.98	5.0													
	B1,B2,B3,B4,B5	68.92	0.11					56.3	7.52	1.54	11.6									
	C1	0.64	0.32	11.9	0.20	3.86	0.8													
DP4	C2	1.60	0.15	13.7	0.25	3.66	0.9													
	C1,C2	2.23	0.20					13.7	0.45	3.66	1.6									

DCM:  $I = C1 * \ln(t_c) + C2$   
 C1: 1.5  
 C2: 7.583

Job No.: 08019  
 Project: Mountain's Edge  
 Design Storm: 100-Year Storm (1% Probability)  
 Jurisdiction: DCM  
 Date: 6/2/2020 18:53  
 Calcs By: drg  
 Checked By:

Sub-Basin and Combined Flows (Modified from Standard Form SF-2)

DP	Sub-Basin	Area (Acres)	C100	Direct Runoff				Combined Runoff				Streetflow		Pipe Flow				Travel Time	
				t <sub>c</sub> (min)	CA (Acres)	I100 (in/hr)	Q100 (cfs)	t <sub>c</sub> (min)	CA (Acres)	I100 (in/hr)	Q100 (cfs)	Slope (%)	Length (ft)	Q (cfs)	Slope (%)	Mnngs n	Length (ft)	V <sub>occ</sub> (ft/s)	t <sub>t</sub> (min)
	A (EX)	6.19	0.35	18.0	2.17	5.45	11.8												
	B (EX)	68.89	0.36	57.0	24.73	2.55	63.0												
	C (EX)	2.24	0.43	13.0	0.95	6.28	6.0												
DP1	A1	6.19	0.36	17.6	2.26	5.50	12.4												
	B1	40.20	0.36	49.6	14.65	2.90	42.4												
	B2	2.16	0.39	28.7	0.85	4.27	3.6												
	B3	3.75	0.37	22.4	1.40	4.91	6.9												
DP2	B1,B2,B3	46.11	0.37					49.6	16.91	2.90	49.0								
	B4	0.57	0.54	11.4	0.31	6.60	2.0												
	B1,B2,B3,B4	46.68	0.37					49.8	17.22	2.89	49.7								
	B5	22.24	0.37	41.9	8.29	3.32	27.5												
DP3	B1,B2,B3,B4,B5	68.92	0.37					56.3	25.51	2.58	65.8								
	C1	0.64	0.51	11.9	0.33	6.49	2.1												
	C2	1.60	0.40	13.7	0.64	6.14	3.9												
	C1,C2	2.23	0.43					13.7	0.97	6.14	5.9								
DP4																			

DCM:  $I = C1 * \ln(t_c) + C2$   
 C1: 2.52  
 C2: 12.735

## Sub-Basin A (Ex) Runoff Calculations (Historic)

Job No.: 08019

Date: 6/2/2020 18:53

Project: Mountain's Edge

Calcs by: drg

Jurisdiction DCM

Checked by:

Runoff Coefficient Surface Type

Soil Type B  
Urbanization Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	269,660	6.19	0.02	0.08	0.15	0.25	0.3	0.35	0%
Combined	269,660	6.19	0.02	0.08	0.15	0.25	0.30	0.35	0.0%

269660

### Basin Travel Time

Shallow Channel Ground Cover Short Pasture/Lawns							
	$L_{max, Overland}$	300 ft			$C_v$	7	
	L (ft)	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	v (ft/s)	t (min)	$t_{Alt}$ (min)	
Total	386	24	-	-	-	-	
Initial Time	300	21	0.070	-	16.8	N/A	DCM Eq. 6-8
Shallow Channel	86	3	0.029	1.2	1.2	-	DCM Eq. 6-9
Channelized			0.000	0.0	0.0	-	V-Ditch
				$t_c$	18.0 min.		

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.60	3.25	3.79	4.33	4.87	5.45
Runoff (cfs)	0.3	1.6	3.5	6.7	9.1	11.8
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.3	1.6	3.5	6.7	9.1	11.8

DCM:  $I = C1 * \ln(tc) + C2$

C1 1.19 1.5 1.75 2 2.25 2.52

C2 6.035 7.583 8.847 10.111 11.375 12.735

### Notes

## Sub-Basin B (EX) Runoff Calculations (Historic)

Job No.: 08019

Date: 6/2/2020 18:53

Project: Mountain's Edge

Calcs by: drg

Jurisdiction DCM

Checked by:

Runoff Coefficient Surface Type

Soil Type B  
Urbanization Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	2,926,529	67.18	0.02	0.08	0.15	0.25	0.3	0.35	0%
Gravel	66,945	1.54	0.57	0.59	0.63	0.66	0.68	0.7	80%
Roofs	7,500	0.17	0.71	0.73	0.75	0.78	0.8	0.81	90%
Combined	3,000,974	68.89	0.03	0.09	0.16	0.26	0.31	0.36	2.0%

3000974

### Basin Travel Time

Shallow Channel Ground Cover		Short Pasture/Lawns					
$L_{max, Overland}$		300 ft	$C_v$		7		
	$L$ (ft)	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	$v$ (ft/s)	$t$ (min)	$t_{Alt}$ (min)	
Total	3,400	75	-	-	-	-	
Initial Time	300	6	0.020	-	25.1	N/A	DCM Eq. 6-8
Shallow Channel	1,400	35	0.025	1.1	21.1	-	DCM Eq. 6-9
Channelized	1,700	34	0.020	2.6	10.9	-	Trap Ditch
				$t_c$	57.0 min.		

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	1.22	1.52	1.77	2.03	2.28	2.55
Runoff (cfs)	2.9	9.7	19.8	36.3	48.6	63.0
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	2.9	9.7	19.8	36.3	48.6	63.0

DCM:  $I = C1 * \ln(t_c) + C2$

C1 1.19 1.5 1.75 2 2.25 2.52

C2 6.035 7.583 8.847 10.111 11.375 12.735

### Notes



## Sub-Basin C (EX) Runoff Calculations (Historic)

Job No.: 08019  
 Project: Mountain's Edge

Date: 6/2/2020 18:53

Calcs by: drg

Checked by: \_\_\_\_\_

Jurisdiction DCM  
 Runoff Coefficient Surface Type

Soil Type B  
 Urbanization Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	76,331	1.75	0.02	0.08	0.15	0.25	0.3	0.35	0%
Gravel	21,225	0.49	0.57	0.59	0.63	0.66	0.68	0.7	80%
<b>Combined</b>	<b>97,556</b>	<b>2.24</b>	<b>0.14</b>	<b>0.19</b>	<b>0.25</b>	<b>0.34</b>	<b>0.38</b>	<b>0.43</b>	<b>17.4%</b>

97556

### Basin Travel Time

Shallow Channel Ground Cover			Short Pasture/Lawns			
	L <sub>max,Overland</sub>	300 ft			C <sub>v</sub>	7
	L (ft)	ΔZ <sub>0</sub> (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)
Total	374	9	-	-	-	-
Initial Time	105	4	0.033	-	11.3	N/A DCM Eq. 6-8
Shallow Channel			0.000	0.0	0.0	- DCM Eq. 6-9
Channelized	269	5	0.019	2.7	1.7	- V-Ditch
				t <sub>c</sub>	13.0 min.	

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.98	3.74	4.36	4.98	5.61	6.28
Runoff (cfs)	0.9	1.6	2.5	3.8	4.8	6.0
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.9	1.6	2.5	3.8	4.8	6.0

$$DCM: I = C1 * \ln(tc) + C2$$

C1      1.19      1.5      1.75      2      2.25      2.52

C2      6.035      7.583      8.847      10.111      11.375      12.735

### Notes

## Sub-Basin A1 Runoff Calculations (Developed)

Job No.: 08019

Date: 6/2/2020 18:53

Project: Mountain's Edge

Calcs by: drg

Jurisdiction DCM  
Runoff Coefficient Surface Type

Checked by:

Soil Type B  
Urbanization Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	259,860	5.97	0.02	0.08	0.15	0.25	0.3	0.35	0%
Gravel	4,800	0.11	0.57	0.59	0.63	0.66	0.68	0.7	80%
Roofs	5,000	0.11	0.71	0.73	0.75	0.78	0.8	0.81	90%
Combined	269,660	6.19	0.04	0.10	0.17	0.27	0.32	0.36	3.1%

269660

### Basin Travel Time

Shallow Channel Ground Cover Short Pasture/Lawns							
	$L_{max, Overland}$	300 ft			$C_v$	7	
	L (ft)	$\Delta Z_o$ (ft)	$S_o$ (ft/ft)	$v$ (ft/s)	$t$ (min)	$t_{Alt}$ (min)	
Total	386	24	-	-	-	-	
Initial Time	300	21	0.070	-	16.4	N/A	DCM Eq. 6-8
Shallow Channel	86	3	0.029	1.2	1.2	-	DCM Eq. 6-9
Channelized			0.000	0.0	0.0	-	V-Ditch
				$t_c$	17.6 min.		

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.62	3.28	3.82	4.37	4.92	5.50
Runoff (cfs)	0.7	2.1	4.0	7.2	9.6	12.4
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.7	2.1	4.0	7.2	9.6	12.4

DCM:  $I = C1 * \ln(tc) + C2$

C1 1.19 1.5 1.75 2 2.25 2.52

C2 6.035 7.583 8.847 10.111 11.375 12.735

### Notes

## Sub-Basin B1 Runoff Calculations (Developed)

Job No.: 08019  
 Project: Mountain's Edge

Date: 6/2/2020 18:53

Calcs by: drg

Checked by: \_\_\_\_\_

Jurisdiction **DCM**  
 Runoff Coefficient **Surface Type**

Soil Type **B**  
 Urbanization **Non-Urban**

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	1,680,445	38.58	0.02	0.08	0.15	0.25	0.3	0.35	0%
Gravel	62,955	1.45	0.57	0.59	0.63	0.66	0.68	0.7	80%
Roofs	7,500	0.17	0.71	0.73	0.75	0.78	0.8	0.81	90%
<b>Combined</b>	<b>1,750,900</b>	<b>40.20</b>	<b>0.04</b>	<b>0.10</b>	<b>0.17</b>	<b>0.27</b>	<b>0.32</b>	<b>0.36</b>	<b>3.3%</b>

1750900

### Basin Travel Time

Shallow Channel Ground Cover		Short Pasture/Lawns				
	L <sub>max,Overland</sub>	300 ft			C <sub>v</sub>	7
	L (ft)	ΔZ <sub>0</sub> (ft)	S <sub>0</sub> (ft/ft)	v (ft/s)	t (min)	t <sub>Alt</sub> (min)
Total	2,415	60	-	-	-	-
Initial Time	300	7	0.023	-	23.6	N/A DCM Eq. 6-8
Shallow Channel	1,400	35	0.025	1.1	21.1	- DCM Eq. 6-9
Channelized	715	18	0.025	2.4	4.9	- Trap Ditch
				t <sub>c</sub>	49.6 min.	

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	1.39	1.73	2.01	2.30	2.59	2.90
Runoff (cfs)	2.4	7.0	13.8	24.7	32.9	42.4
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	2.4	7.0	13.8	24.7	32.9	42.4

DCM:  $I = C1 * \ln(tc) + C2$

C1 1.19 1.5 1.75 2 2.25 2.52

C2 6.035 7.583 8.847 10.111 11.375 12.735

### Notes

## Sub-Basin B2 Runoff Calculations (Developed)

Job No.: **08019**  
 Project: **Mountain's Edge**

Date: **6/2/2020 18:53**

Calcs by: **drg**

Checked by:

Jurisdiction: **DCM**  
 Runoff Coefficient: **Surface Type**

Soil Type: **B**  
 Urbanization: **Non-Urban**

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	82,599	1.90	0.02	0.08	0.15	0.25	0.3	0.35	0%
Gravel	11,588	0.27	0.57	0.59	0.63	0.66	0.68	0.7	80%
Combined	94,187	2.16	0.09	0.14	0.21	0.30	0.35	0.39	9.8%

94187

### Basin Travel Time

Shallow Channel Ground Cover Short Pasture/Lawns							
	$L_{max, Overland}$	300 ft			$C_v$	7	
	$L$ (ft)	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	$v$ (ft/s)	$t$ (min)	$t_{Alt}$ (min)	
Total	730	11	-	-	-	-	
Initial Time	300	6	0.020	-	23.8	N/A	DCM Eq. 6-8
Shallow Channel	110	2	0.018	0.9	1.9	-	DCM Eq. 6-9
Channelized	320	3	0.010	1.8	3.0	-	V-Ditch
				$t_c$	28.7 min.		

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.04	2.55	2.97	3.39	3.82	4.27
Runoff (cfs)	0.4	0.8	1.3	2.2	2.9	3.6
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.4	0.8	1.3	2.2	2.9	3.6

DCM:  $I = C1 * \ln(tc) + C2$

C1: 1.19    1.5    1.75    2    2.25    2.52

C2: 6.035    7.583    8.847    10.111    11.375    12.735

Notes

## Sub-Basin B3 Runoff Calculations (Developed)

Job No.: 08019

Date: 6/2/2020 18:53

Project: Mountain's Edge

Calcs by: drg

Jurisdiction DCM  
Runoff Coefficient Surface Type

Checked by: \_\_\_\_\_

Soil Type B  
Urbanization Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	152,352	3.50	0.02	0.08	0.15	0.25	0.3	0.35	0%
Gravel	11,136	0.26	0.57	0.59	0.63	0.66	0.68	0.7	80%
Combined	163,488	3.75	0.06	0.11	0.18	0.28	0.33	0.37	5.4%

163488

### Basin Travel Time

	Shallow Channel Ground Cover		Short Pasture/Lawns			
	$L_{max, Overland}$	300 ft			$C_v$	7
	L (ft)	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	v (ft/s)	t (min)	$t_{Alt}$ (min)
Total	735	28	-	-	-	-
Initial Time	300	15	0.050	-	18.1	N/A DCM Eq. 6-8
Shallow Channel	235	11	0.047	1.5	2.6	- DCM Eq. 6-9
Channelized	200	2	0.010	2.0	1.7	- V-Ditch
				$t_c$	22.4 min.	

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.34	2.92	3.41	3.90	4.38	4.91
Runoff (cfs)	0.5	1.3	2.3	4.1	5.4	6.9
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.5	1.3	2.3	4.1	5.4	6.9

DCM:  $I = C1 * \ln(t_c) + C2$

C1 1.19 1.5 1.75 2 2.25 2.52

C2 6.035 7.583 8.847 10.111 11.375 12.735

### Notes



## Combined Sub-Basin Runoff Calculations (DP1)

Includes Basins B1 B2 B3

Job No.: **08019**

Date: **6/2/2020 18:53**

Project: **Mountain's Edge**

Calcs by: **drg**

Jurisdiction: **DCM**

Checked by: \_\_\_\_\_

Runoff Coefficient: **Surface Type**

Soil Type: **B**

Urbanization: **Non-Urban**

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	1,915,396	43.97	0.02	0.08	0.15	0.25	0.3	0.35	0%
Roofs	7,500	0.17	0.71	0.73	0.75	0.78	0.8	0.81	90%
Gravel	85,679	1.97	0.57	0.59	0.63	0.66	0.68	0.7	80%
<b>Combined</b>	<b>2,008,575</b>	<b>46.11</b>	<b>0.05</b>	<b>0.10</b>	<b>0.17</b>	<b>0.27</b>	<b>0.32</b>	<b>0.37</b>	<b>3.7%</b>

### Basin Travel Time

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. $\Delta Z_0$ (ft)	$Q_i$ (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	B1	-	2,415	60	-	-	-	-	49.6
Channelized-1									
Channelized-2									
Channelized-3									
<b>Total</b>			<b>2,415</b>	<b>60</b>					
								<b><math>t_c</math> (min)</b>	<b>49.6</b>

### Contributing Offsite Flows (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas

$Q_{Minor}$  (cfs) - 5-year Storm  
 $Q_{Major}$  (cfs) - 100-year Storm

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
<b>Intensity (in/hr)</b>	1.39	1.73	2.01	2.30	2.59	2.90
<b>Site Runoff (cfs)</b>	2.95	<b>8.29</b>	16.04	28.61	37.99	<b>48.96</b>
<b>OffSite Runoff (cfs)</b>	-	<b>0.00</b>	-	-	-	<b>0.00</b>
<b>Release Rates (cfs/ac)</b>	-	-	-	-	-	-
<b>Allowed Release (cfs)</b>	-	<b>8.3</b>	-	-	-	<b>49.0</b>

DCM:  $I = C1 * \ln(tc) + C2$

C1 1.19 1.5 1.75 2 2.25 2.52  
C2 6.035 7.583 8.847 10.111 11.375 12.735

### Notes

Runoff from Offsite basins have been assumed constant, despite additional times of concentration.

## Sub-Basin B4 Runoff Calculations (Developed)

Job No.: 08019  
Project: Mountain's Edge

Date: 6/2/2020 18:53

Calcs by: drg

Checked by: \_\_\_\_\_

Jurisdiction **DCM**  
Runoff Coefficient **Surface Type**

Soil Type **B**  
Urbanization **Non-Urban**

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	11,053	0.25	0.02	0.08	0.15	0.25	0.3	0.35	0%
Gravel	13,760	0.32	0.57	0.59	0.63	0.66	0.68	0.7	80%
<b>Combined</b>	<b>24,813</b>	<b>0.57</b>	<b>0.33</b>	<b>0.36</b>	<b>0.42</b>	<b>0.48</b>	<b>0.51</b>	<b>0.54</b>	<b>44.4%</b>

24813

### Basin Travel Time

Shallow Channel Ground Cover Short Pasture/Lawns							
	$L_{max, Overland}$	300 ft			$C_v$	7	
	L (ft)	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	$v$ (ft/s)	t (min)	$t_{Alt}$ (min)	
Total	622	7	-	-	-	-	
Initial Time	32	1	0.020	-	6.0	N/A	DCM Eq. 6-8
Shallow Channel			0.000	0.0	0.0	-	DCM Eq. 6-9
Channelized	590	6	0.010	1.8	5.5	-	V-Ditch
				$t_c$	11.4 min.		

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	3.14	3.93	4.58	5.24	5.89	6.60
Runoff (cfs)	0.6	0.8	1.1	1.4	1.7	2.0
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.6	0.8	1.1	1.4	1.7	2.0

DCM:  $I = C1 * \ln(tc) + C2$

C1 1.19 1.5 1.75 2 2.25 2.52

C2 6.035 7.583 8.847 10.111 11.375 12.735

### Notes

## Combined Sub-Basin Runoff Calculations (DP2)

Includes Basins B1 B2 B3 B4

Job No.: **08019**

Date: **6/2/2020 18:53**

Project: **Mountain's Edge**

Calcs by: **drg**

Jurisdiction: **DCM**

Checked by: \_\_\_\_\_

Runoff Coefficient: **Surface Type**

Soil Type: **B**

Urbanization: **Non-Urban**

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	1,926,449	44.23	0.02	0.08	0.15	0.25	0.3	0.35	0%
Roofs	7,500	0.17	0.71	0.73	0.75	0.78	0.8	0.81	90%
Gravel	99,439	2.28	0.57	0.59	0.63	0.66	0.68	0.7	80%
<b>Combined</b>	<b>2,033,388</b>	<b>46.68</b>	<b>0.05</b>	<b>0.11</b>	<b>0.18</b>	<b>0.27</b>	<b>0.32</b>	<b>0.37</b>	<b>4.2%</b>

### Basin Travel Time

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. $\Delta Z_0$ (ft)	$Q_i$ (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	B1	-	2,415	60	-	-	-	-	49.6
Channelized-1	Pipe	RCP	75	1	9	3	0	8.2	0.2
Channelized-2									
Channelized-3									
<b>Total</b>			<b>2,490</b>	<b>61</b>					
								<b><math>t_c</math> (min)</b>	<b>49.8</b>

### Contributing Offsite Flows (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas

$Q_{Minor}$  (cfs) - 5-year Storm  
 $Q_{Major}$  (cfs) - 100-year Storm

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
<b>Intensity (in/hr)</b>	1.39	1.72	2.01	2.30	2.58	2.89
<b>Site Runoff (cfs)</b>	3.20	<b>8.63</b>	16.48	29.15	38.64	<b>49.72</b>
<b>OffSite Runoff (cfs)</b>	-	<b>0.00</b>	-	-	-	<b>0.00</b>
<b>Release Rates (cfs/ac)</b>	-	-	-	-	-	-
<b>Allowed Release (cfs)</b>	-	<b>8.6</b>	-	-	-	<b>49.7</b>

DCM:  $I = C1 * \ln(tc) + C2$

C1 1.19 1.5 1.75 2 2.25 2.52  
C2 6.035 7.583 8.847 10.111 11.375 12.735

### Notes

Runoff from Offsite basins have been assumed constant, despite additional times of concentration.



## Sub-Basin B5 Runoff Calculations (Developed)

Job No.: **08019**  
 Project: **Mountain's Edge**

Date: **6/2/2020 18:53**

Calcs by: **drg**

Checked by:

Jurisdiction: **DCM**  
 Runoff Coefficient: **Surface Type**

Soil Type: **B**  
 Urbanization: **Non-Urban**

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	911,859	20.93	0.02	0.08	0.15	0.25	0.3	0.35	0%
Gravel	36,890	0.85	0.57	0.59	0.63	0.66	0.68	0.7	80%
Roofs	20,000	0.46	0.71	0.73	0.75	0.78	0.8	0.81	90%
Combined	968,749	22.24	0.06	0.11	0.18	0.28	0.32	0.37	4.9%

968749

### Basin Travel Time

Shallow Channel Ground Cover Short Pasture/Lawns							
	$L_{max, Overland}$	300 ft			$C_v$	7	
	$L$ (ft)	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	$v$ (ft/s)	$t$ (min)	$t_{Alt}$ (min)	
Total	1,440	28	-	-	-	-	
Initial Time	300	5	0.017	-	26.1	N/A	DCM Eq. 6-8
Shallow Channel	730	14	0.019	1.0	12.6	-	DCM Eq. 6-9
Channelized	410	9	0.022	2.1	3.3	-	Trap Ditch
				$t_c$	41.9 min.		

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	1.59	1.98	2.31	2.64	2.97	3.32
Runoff (cfs)	2.0	5.0	9.3	16.2	21.5	27.5
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	2.0	5.0	9.3	16.2	21.5	27.5

DCM:  $I = C1 * \ln(tc) + C2$

C1: 1.19    1.5    1.75    2    2.25    2.52

C2: 6.035    7.583    8.847    10.111    11.375    12.735

Notes

## Combined Sub-Basin Runoff Calculations (DP3)

Includes Basins B1 B2 B3 B4 B5

Job No.:	08019	Date:	6/2/2020 18:53
Project:	Mountain's Edge	Calcs by:	drg
Jurisdiction	DCM	Checked by:	
Runoff Coefficient	Surface Type	Soil Type	B
		Urbanization	Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	2,838,308	65.16	0.02	0.08	0.15	0.25	0.3	0.35	0%
Roofs	27,500	0.63	0.71	0.73	0.75	0.78	0.8	0.81	90%
Gravel	136,329	3.13	0.57	0.59	0.63	0.66	0.68	0.7	80%
<b>Combined</b>	<b>3,002,137</b>	<b>68.92</b>	<b>0.05</b>	<b>0.11</b>	<b>0.18</b>	<b>0.27</b>	<b>0.32</b>	<b>0.37</b>	<b>4.5%</b>

### Basin Travel Time

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. $\Delta Z_0$ (ft)	$Q_i$ (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	B1	-	2,415	60	-	-	-	-	49.6
Channelized-1	Pipe	RCP	75	1	9	3	0	8.2	0.2
Channelized-2	Trap Ditch	2	925	16	12	3	4	2.4	6.5
Channelized-3									
Total			3,415	77					
$t_c$ (min)									56.3

2 = Natural, Winding, minimal vegetation/shallow grass

### Contributing Offsite Flows (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas

$Q_{Minor}$  (cfs) - 5-year Storm  
 $Q_{Major}$  (cfs) - 100-year Storm

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	1.24	1.54	1.79	2.05	2.31	2.58
Site Runoff (cfs)	4.38	11.56	21.91	38.63	51.15	65.76
OffSite Runoff (cfs)	-	0.00	-	-	-	0.00
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	-	11.6	-	-	-	65.8

DCM:  $I = C1 * \ln(tc) + C2$

C1	1.19	1.5	1.75	2	2.25	2.52
C2	6.035	7.583	8.847	10.111	11.375	12.735

### Notes

Runoff from Offsite basins have been assumed constant, despite additional times of concentration.

## Sub-Basin C1 Runoff Calculations (Developed)

Job No.: 08019

Date: 6/2/2020 18:53

Project: Mountain's Edge

Calcs by: drg

Jurisdiction **DCM**  
Runoff Coefficient **Surface Type**

Checked by: \_\_\_\_\_

Soil Type **B**  
Urbanization **Non-Urban**

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	14,901	0.34	0.02	0.08	0.15	0.25	0.3	0.35	0%
Gravel	12,825	0.29	0.57	0.59	0.63	0.66	0.68	0.7	80%
<b>Combined</b>	<b>27,726</b>	<b>0.64</b>	<b>0.27</b>	<b>0.32</b>	<b>0.37</b>	<b>0.44</b>	<b>0.48</b>	<b>0.51</b>	<b>37.0%</b>

27726

### Basin Travel Time

Shallow Channel Ground Cover Short Pasture/Lawns							
	$L_{max, Overland}$	300 ft			$C_v$	7	
	L (ft)	$\Delta Z_0$ (ft)	$S_0$ (ft/ft)	v (ft/s)	t (min)	$t_{Alt}$ (min)	
Total	772	14	-	-	-	-	
Initial Time	32	1	0.020	-	6.3	N/A	DCM Eq. 6-8
Shallow Channel			0.000	0.0	0.0	-	DCM Eq. 6-9
Channelized	740	13	0.018	2.2	5.6	-	V-Ditch
				$t_c$	11.9 min.		

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	3.08	3.86	4.51	5.15	5.80	6.49
Runoff (cfs)	0.5	0.8	1.1	1.4	1.8	2.1
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.5	0.8	1.1	1.4	1.8	2.1

DCM:  $I = C1 * \ln(tc) + C2$

C1 1.19 1.5 1.75 2 2.25 2.52

C2 6.035 7.583 8.847 10.111 11.375 12.735

### Notes

## Sub-Basin C2 Runoff Calculations (Developed)

Job No.: 08019

Date: 6/2/2020 18:53

Project: Mountain's Edge

Calcs by: drg

Jurisdiction DCM

Checked by:

Runoff Coefficient Surface Type

Soil Type B  
Urbanization Non-Urban

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	59,408	1.36	0.02	0.08	0.15	0.25	0.3	0.35	0%
Gravel	10,205	0.23	0.57	0.59	0.63	0.66	0.68	0.7	80%
Combined	69,613	1.60	0.10	0.15	0.22	0.31	0.36	0.40	11.7%

69613

### Basin Travel Time

Shallow Channel Ground Cover Short Pasture/Lawns							
	$L_{max, Overland}$	300 ft			$C_v$	7	
	L (ft)	$\Delta Z_o$ (ft)	$S_o$ (ft/ft)	$v$ (ft/s)	t (min)	$t_{Alt}$ (min)	
Total	374	9	-	-	-	-	
Initial Time	105	4	0.033	-	11.8	N/A	DCM Eq. 6-8
Shallow Channel			0.000	0.0	0.0	-	DCM Eq. 6-9
Channelized	269	5	0.019	2.3	1.9	-	V-Ditch
$t_c$					13.7 min.		

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
Intensity (in/hr)	2.92	3.66	4.27	4.88	5.49	6.14
Runoff (cfs)	0.5	0.9	1.5	2.4	3.1	3.9
Release Rates (cfs/ac)	-	-	-	-	-	-
Allowed Release (cfs)	0.5	0.9	1.5	2.4	3.1	3.9

DCM:  $I = C1 * \ln(t_c) + C2$

C1 1.19 1.5 1.75 2 2.25 2.52

C2 6.035 7.583 8.847 10.111 11.375 12.735

### Notes



## Combined Sub-Basin Runoff Calculations (DP 4)

Includes Basins C1 C2

Job No.: **08019**  
 Project: **Mountain's Edge**

Date: **6/2/2020 19:41**

Calcs by: **drg**

Checked by:

Jurisdiction **DCM**  
 Runoff Coefficient **Surface Type**

Soil Type **B**  
 Urbanization **Non-Urban**

### Basin Land Use Characteristics

Surface	Area		Runoff Coefficient						% Imperv.
	(SF)	(Acres)	C2	C5	C10	C25	C50	C100	
Pasture/Meadow	74,309	1.71	0.02	0.08	0.15	0.25	0.3	0.35	0%
Gravel	23,030	0.53	0.57	0.59	0.63	0.66	0.68	0.7	80%
<b>Combined</b>	<b>97,339</b>	<b>2.23</b>	<b>0.15</b>	<b>0.20</b>	<b>0.26</b>	<b>0.35</b>	<b>0.39</b>	<b>0.43</b>	<b>18.9%</b>

### Basin Travel Time

	Sub-basin or Channel Type	Material Type	L (ft)	Elev. $\Delta Z_0$ (ft)	$Q_i$ (cfs)	Base or Dia (ft)	Sides z:1 (ft/ft)	v (ft/s)	t (min)
Furthest Reach	C2	-	374	9	-	-	-	-	13.7
Channelized-1									
Channelized-2									
Channelized-3									
<b>Total</b>			<b>374</b>	<b>9</b>					
								<b>t<sub>c</sub> (min)</b>	<b>13.7</b>

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### Contributing Offsite Flows (Added to Runoff and Allowed Release, below.)

Contributing Basins/Areas

$Q_{Minor}$  (cfs) - 5-year Storm  
 $Q_{Major}$  (cfs) - 100-year Storm

### Rainfall Intensity & Runoff

	2-Yr	5-Yr	10-Yr	25-Yr	50-Yr	100-Yr
<b>Intensity (in/hr)</b>	2.92	3.66	4.27	4.88	5.49	6.14
<b>Site Runoff (cfs)</b>	0.98	1.64	2.51	3.78	4.78	5.94
<b>OffSite Runoff (cfs)</b>	-	0.00	-	-	-	0.00
<b>Release Rates (cfs/ac)</b>	-	-	-	-	-	-
<b>Allowed Release (cfs)</b>	-	1.6	-	-	-	5.9

DCM:  $I = C1 * \ln(tc) + C2$

C1 1.19 1.5 1.75 2 2.25 2.52  
 C2 6.035 7.583 8.847 10.111 11.375 12.735

### Notes

Runoff from Offsite basins have been assumed constant, despite additional times of concentration.

# Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Jun 2 2020

## Mountain's Edge - Station 0+84 14x23 elliptical RCP at McClelland & Farmhouse

Invert Elev Dn (ft) = 69.40  
 Pipe Length (ft) = 56.00  
 Slope (%) = 1.00  
 Invert Elev Up (ft) = 69.96  
 Rise (in) = 14.0  
 Shape = Elliptical  
 Span (in) = 23.0  
 No. Barrels = 1  
 n-Value = 0.013  
 Culvert Type = Horizontal Ellipse Concrete  
 Culvert Entrance = Square edge w/headwall (H)  
 Coeff. K,M,c,Y,k = 0.01, 2, 0.0398, 0.67, 0.5

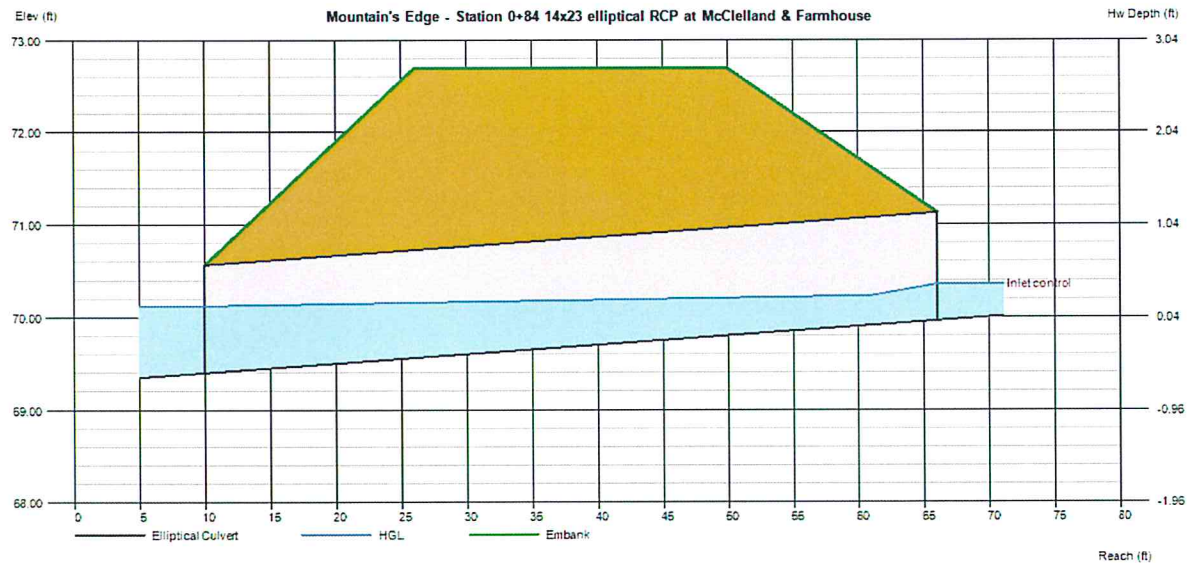
**Embankment**  
 Top Elevation (ft) = 72.69  
 Top Width (ft) = 24.00  
 Crest Width (ft) = 100.00

### Calculations

Qmin (cfs) = 0.80  
 Qmax (cfs) = 2.10  
 Tailwater Elev (ft) = (dc+D)/2

### Highlighted

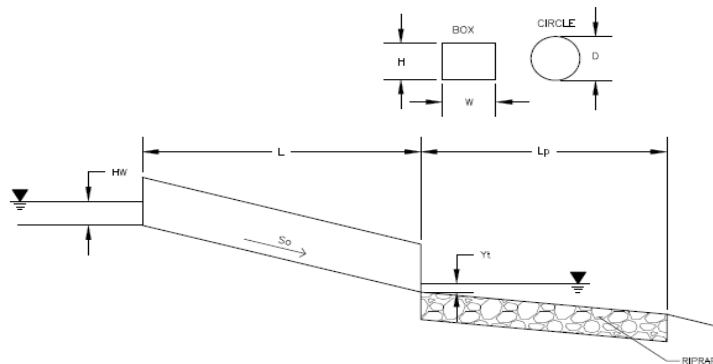
Qtotal (cfs) = 0.80  
 Qpipe (cfs) = 0.80  
 Qovertop (cfs) = 0.00  
 Veloc Dn (ft/s) = 0.65  
 Veloc Up (ft/s) = 2.76  
 HGL Dn (ft) = 70.12  
 HGL Up (ft) = 70.24  
 Hw Elev (ft) = 70.35  
 Hw/D (ft) = 0.34  
 Flow Regime = Inlet Control



## Determination of Culvert Headwater and Outlet Protection

Project: **Mountain's Edge**

Basin ID: **Farmhous Ct culvert sta 0+84 - rip rap apron sizing**



this sheet for riprap  
apron sizing using  
equivalent round pipe  
- see culvert  
calculations for  
elliptical culvert flow  
and headwater  
calculations

Soil Type:

Choose One:

☒ Sandy

☐ Non-Sandy

Supercritical Flow! Using Da to calculate protection type.

### Design Information (Input):

Design Discharge

Q = 2.1 cfs

Circular Culvert:

Barrel Diameter in Inches

D = 24 inches

Inlet Edge Type (Choose from pull-down list)

Square End with Headwall

Box Culvert:

Barrel Height (Rise) in Feet

Height (Rise) =

Barrel Width (Span) in Feet

Width (Span) =

Inlet Edge Type (Choose from pull-down list)

Number of Barrels

No = 1

Inlet Elevation

Elev IN = 69.97 ft

Outlet Elevation **OR** Slope

Elev OUT = 69.4 ft

Culvert Length

L = 56 ft

Manning's Roughness

n = 0.013

Bend Loss Coefficient

k<sub>b</sub> = 0

Exit Loss Coefficient

k<sub>x</sub> = 1

Tailwater Surface Elevation

Elev Y<sub>t</sub> = 70.2 ft

Max Allowable Channel Velocity

V = 5 ft/s

### Required Protection (Output):

Tailwater Surface Height

Y<sub>t</sub> = 0.80 ft

Flow Area at Max Channel Velocity

A<sub>t</sub> = 0.42 ft<sup>2</sup>

Culvert Cross Sectional Area Available

A = 3.14 ft<sup>2</sup>

Entrance Loss Coefficient

k<sub>e</sub> = 0.50

Friction Loss Coefficient

k<sub>f</sub> = 0.69

Sum of All Losses Coefficients

k<sub>s</sub> = 2.19

Culvert Normal Depth

Y<sub>n</sub> = 0.41 ft

Culvert Critical Depth

Y<sub>c</sub> = 0.50 ft

Tailwater Depth for Design

d = 1.25 ft

Adjusted Diameter **OR** Adjusted Rise

D<sub>a</sub> = 1.20 ft

Expansion Factor

1/(2\*tan(θ)) = 6.70

Flow/Diameter<sup>2.5</sup> **OR** Flow/(Span \* Rise<sup>1.5</sup>)

Q/D<sup>2.5</sup> = 0.37 ft<sup>0.5</sup>/s

Froude Number

Fr = 1.50 **Supercritical!**

Tailwater/Adjusted Diameter **OR** Tailwater/Adjusted Rise

Y<sub>t</sub>/D = 0.66

Inlet Control Headwater

HW<sub>i</sub> = 0.68 ft

Outlet Control Headwater

HW<sub>o</sub> = 0.70 ft

Design Headwater Elevation

HW = 70.67 ft

Headwater/Diameter **OR** Headwater/Rise Ratio

HW/D = 0.35

Minimum Theoretical Riprap Size

d<sub>50</sub> = 1 in

Nominal Riprap Size

d<sub>50</sub> = 6 in

UDFCD Riprap Type

Type = VL

Length of Protection

L<sub>p</sub> = 6 ft

Width of Protection

T = 3 ft

# Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Jun 2 2020

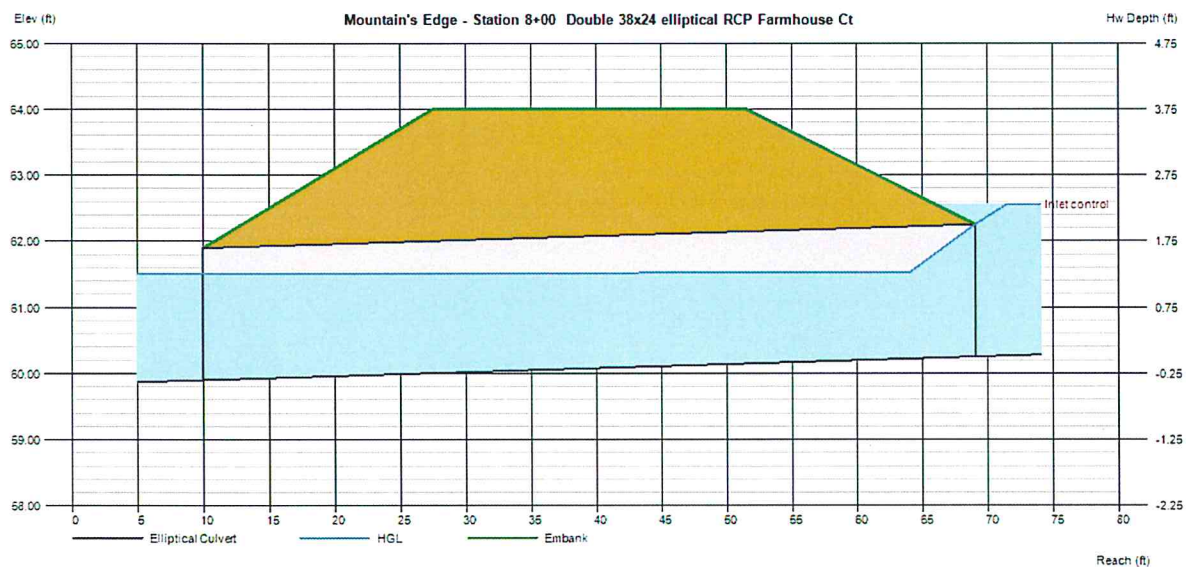
## Mountain's Edge - Station 8+00 Double 38x24 elliptical RCP Farmhouse Ct

Invert Elev Dn (ft) = 59.90  
 Pipe Length (ft) = 59.00  
 Slope (%) = 0.59  
 Invert Elev Up (ft) = 60.25  
 Rise (in) = 24.0  
 Shape = Elliptical  
 Span (in) = 38.0  
 No. Barrels = 2  
 n-Value = 0.013  
 Culvert Type = Horizontal Ellipse Concrete  
 Culvert Entrance = Square edge w/headwall (H)  
 Coeff. K,M,c,Y,k = 0.01, 2, 0.0398, 0.67, 0.5

**Embankment**  
 Top Elevation (ft) = 64.00  
 Top Width (ft) = 24.00  
 Crest Width (ft) = 200.00

**Calculations**  
 Qmin (cfs) = 8.30  
 Qmax (cfs) = 49.00  
 Tailwater Elev (ft) = (dc+D)/2

**Highlighted**  
 Qtotal (cfs) = 48.30  
 Qpipe (cfs) = 48.30  
 Qovertop (cfs) = 0.00  
 Veloc Dn (ft/s) = 5.58  
 Veloc Up (ft/s) = 6.94  
 HGL Dn (ft) = 61.50  
 HGL Up (ft) = 61.53  
 Hw Elev (ft) = 62.56  
 Hw/D (ft) = 1.16  
 Flow Regime = Inlet Control

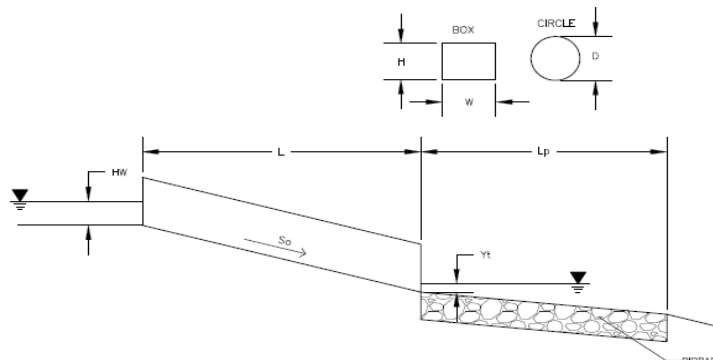




## Determination of Culvert Headwater and Outlet Protection

Project: **Mountain's Edge**

Basin ID: **Farmhouse Ct Culvert sta 8+00 - rip rap apron sizing**



this sheet for riprap apron sizing using equivalent round pipe - see culvert calculations for elliptical culvert flow and headwater calculations

Soil Type:

Choose One:

☒ Sandy

☐ Non-Sandy

### Design Information (Input):

Design Discharge

Q = 49 cfs

Circular Culvert:

Barrel Diameter in Inches

D = 30 inches

Inlet Edge Type (Choose from pull-down list)

Square End with Headwall

Box Culvert:

Barrel Height (Rise) in Feet

Height (Rise) =

Barrel Width (Span) in Feet

Width (Span) =

Inlet Edge Type (Choose from pull-down list)

Number of Barrels

No = 2

Inlet Elevation

Elev IN = 60.25 ft

Outlet Elevation **OR** Slope

Elev OUT = 59.9 ft

Culvert Length

L = 59 ft

Manning's Roughness

n = 0.013

Bend Loss Coefficient

k<sub>b</sub> = 0

Exit Loss Coefficient

k<sub>x</sub> = 1

Tailwater Surface Elevation

Elev Y<sub>t</sub> = 61.5 ft

Max Allowable Channel Velocity

V = 7 ft/s

### Required Protection (Output):

Tailwater Surface Height

Y<sub>t</sub> = 1.60 ft

Flow Area at Max Channel Velocity

A<sub>t</sub> = 3.50 ft<sup>2</sup>

Culvert Cross Sectional Area Available

A = 3.41 ft<sup>2</sup>

Entrance Loss Coefficient

k<sub>e</sub> = 0.50

Friction Loss Coefficient

k<sub>f</sub> = 0.69

Sum of All Losses Coefficients

k<sub>s</sub> = 2.19

Culvert Normal Depth

Y<sub>n</sub> = 1.35 ft

Culvert Critical Depth

Y<sub>c</sub> = 1.75 ft

Tailwater Depth for Design

d = 1.92 ft

Adjusted Diameter **OR** Adjusted Rise

D<sub>a</sub> = -

Expansion Factor

1/(2\*tan(θ)) = 6.70

Flow/Diameter<sup>2.5</sup> **OR** Flow/(Span \* Rise<sup>1.5</sup>)

Q/D<sup>2.5</sup> = 3.91 ft<sup>0.5</sup>/s

Froude Number

Fr = - **Pressure flow!**

Tailwater/Adjusted Diameter **OR** Tailwater/Adjusted Rise

Y<sub>t</sub>/D = 0.77

Inlet Control Headwater

HW<sub>i</sub> = 3.43 ft

Outlet Control Headwater

HW<sub>o</sub> = 3.32 ft

**Design Headwater Elevation**

**HW = 63.68 ft**

**Headwater/Diameter **OR** Headwater/Rise Ratio**

**HW/D = 1.64 **HW/D > 1.5!****

Minimum Theoretical Riprap Size

d<sub>50</sub> = 3 in

Nominal Riprap Size

d<sub>50</sub> = 6 in

**UDFCD Riprap Type**

**Type = VL**

**Length of Protection**

**L<sub>p</sub> = 7 ft**

**Width of Protection**

**T = 4 ft**

# Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Jun 2 2020

## Cross section 1 downsream of Farmhouse Ct. - 5 YR

### User-defined

Invert Elev (ft) = 6559.00  
Slope (%) = 1.62  
N-Value = 0.035

### Calculations

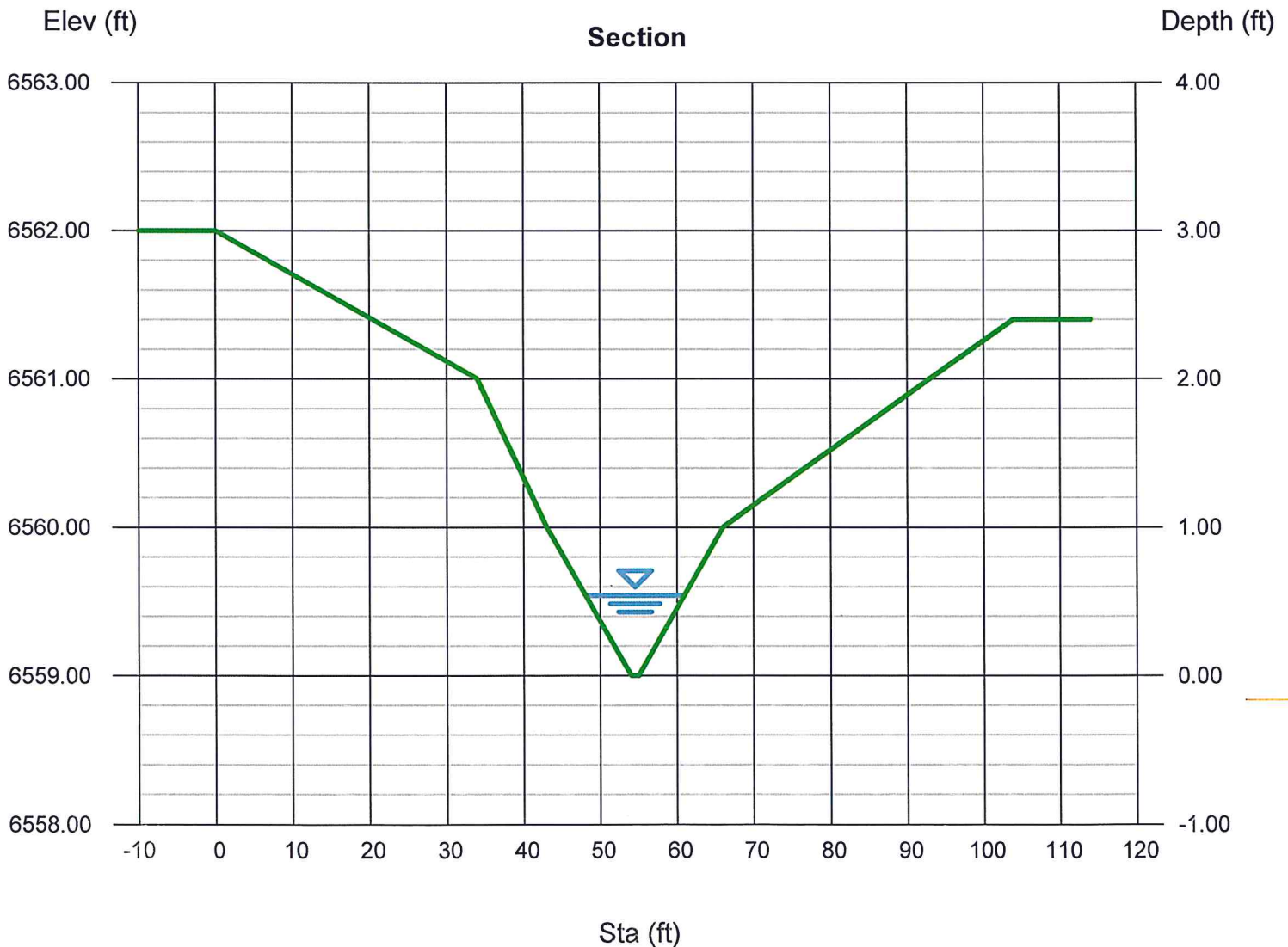
Compute by: Known Q  
Known Q (cfs) = 8.60

### Highlighted

Depth (ft) = 0.54  
Q (cfs) = 8.600  
Area (sqft) = 3.75  
Velocity (ft/s) = 2.29  
Wetted Perim (ft) = 12.93  
Crit Depth, Yc (ft) = 0.48  
Top Width (ft) = 12.88  
EGL (ft) = 0.62

### (Sta, El, n)-(Sta, El, n)...

(0.00, 6562.00)-(34.00, 6561.00, 0.035)-(43.00, 6560.00, 0.035)-(54.00, 6559.00, 0.035)-(55.00, 6559.00, 0.035)-(66.00, 6560.00, 0.035)-(93.00, 6561.00, 0.035)-(104.00, 6561.40, 0.035)



# Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Jun 2 2020

## cross section 1 downstream of Farmhouse Ct. 100-YR

### User-defined

Invert Elev (ft) = 6559.00  
Slope (%) = 1.62  
N-Value = 0.035

### Calculations

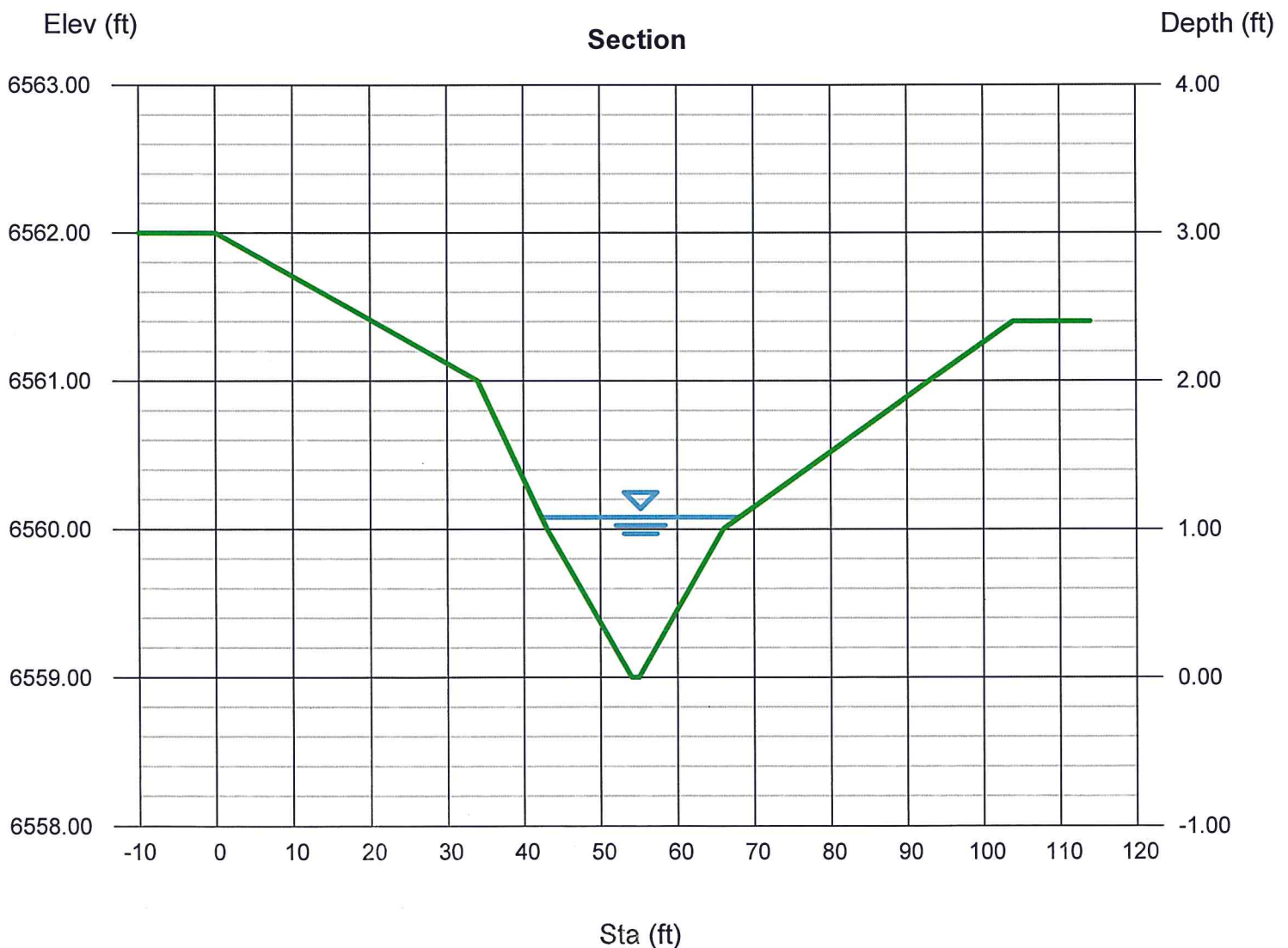
Compute by: Known Q  
Known Q (cfs) = 49.70

### Highlighted

Depth (ft) = 1.08  
Q (cfs) = 49.70  
Area (sqft) = 13.96  
Velocity (ft/s) = 3.56  
Wetted Perim (ft) = 25.98  
Crit Depth, Yc (ft) = 1.01  
Top Width (ft) = 25.88  
EGL (ft) = 1.28

### (Sta, El, n)-(Sta, El, n)...

(0.00, 6562.00)-(34.00, 6561.00, 0.035)-(43.00, 6560.00, 0.035)-(54.00, 6559.00, 0.035)-(55.00, 6559.00, 0.035)-(66.00, 6560.00, 0.035)-(93.00, 6561.00, 0.035)-(104.00, 6561.40, 0.035)



# Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Jun 2 2020

## Cross section 2 downsream of Farmhouse Ct. - 5 YR

### User-defined

Invert Elev (ft) = 6554.40  
Slope (%) = 1.62  
N-Value = 0.035

### Calculations

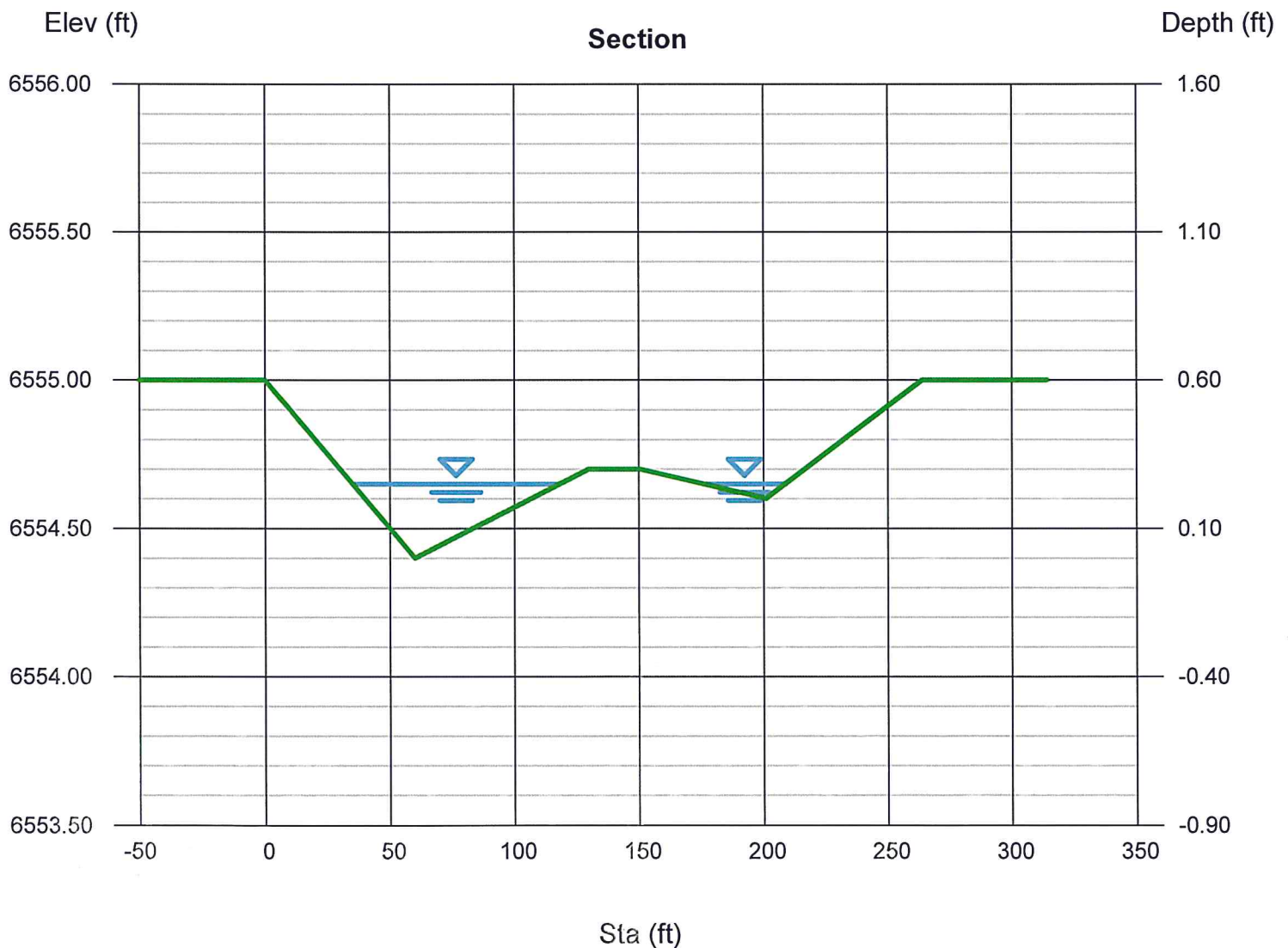
Compute by: Known Q  
Known Q (cfs) = 11.60

### Highlighted

Depth (ft) = 0.25  
Q (cfs) = 11.60  
Area (sqft) = 11.24  
Velocity (ft/s) = 1.03  
Wetted Perim (ft) = 116.50  
Crit Depth, Yc (ft) = 0.20  
Top Width (ft) = 116.49  
EGL (ft) = 0.27

### (Sta, El, n)-(Sta, El, n)...

(0.00, 6555.00)-(60.00, 6554.40, 0.035)-(130.00, 6554.70, 0.035)-(150.00, 6554.70, 0.035)-(201.00, 6554.60, 0.035)-(264.00, 6555.00, 0.035)





# Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Jun 2 2020

## cross section 2 downsream of Farmhouse Ct. 100-YR

### User-defined

Invert Elev (ft) = 6554.40  
Slope (%) = 1.62  
N-Value = 0.035

### Calculations

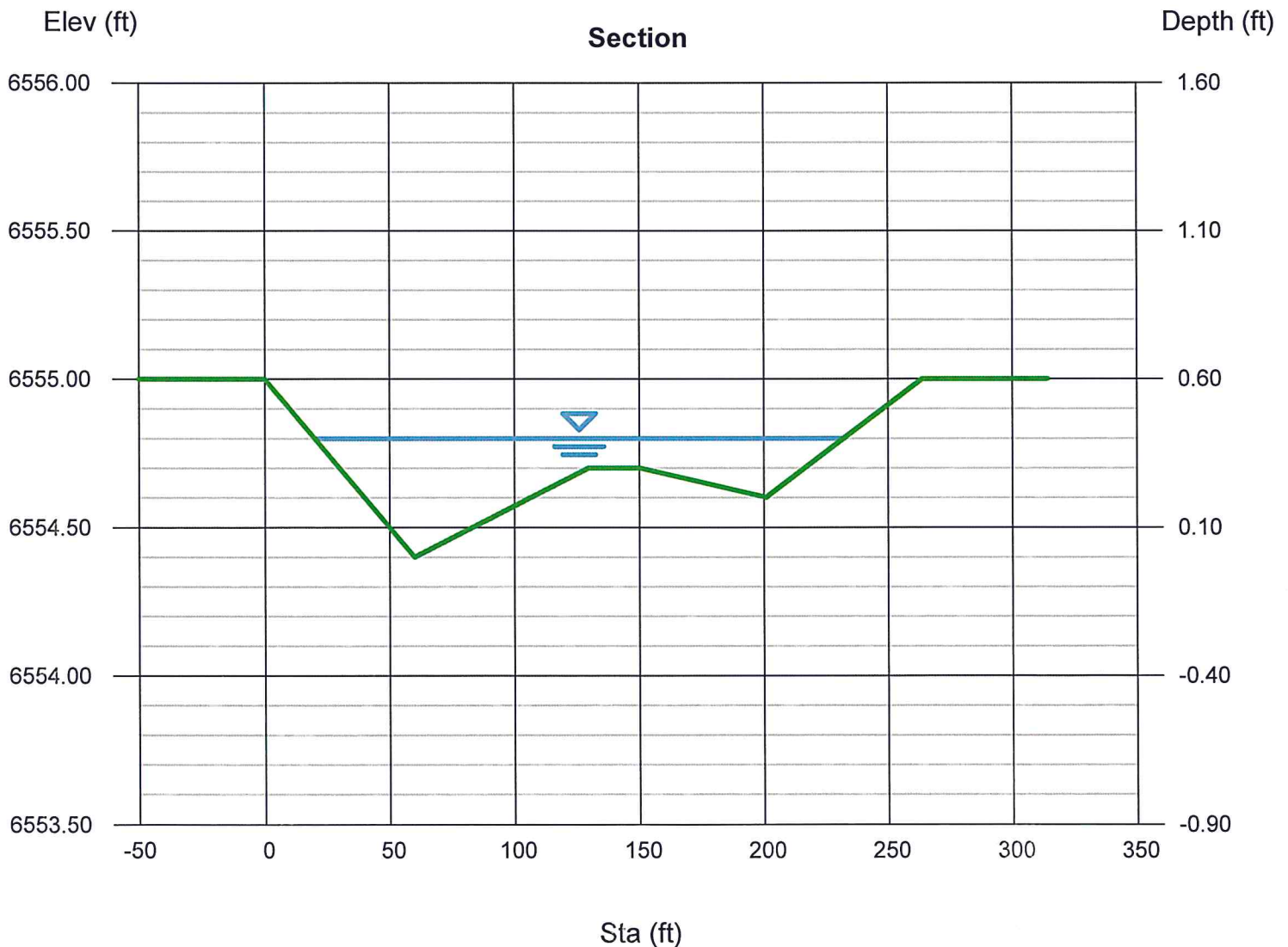
Compute by: Known Q  
Known Q (cfs) = 65.80

### Highlighted

Depth (ft) = 0.40  
Q (cfs) = 65.80  
Area (sqft) = 38.24  
Velocity (ft/s) = 1.72  
Wetted Perim (ft) = 212.45  
Crit Depth, Yc (ft) = 0.37  
Top Width (ft) = 212.45  
EGL (ft) = 0.45

### (Sta, El, n)-(Sta, El, n)...

(0.00, 6555.00)-(60.00, 6554.40, 0.035)-(130.00, 6554.70, 0.035)-(150.00, 6554.70, 0.035)-(201.00, 6554.60, 0.035)-(264.00, 6555.00, 0.035)



# Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Jun 2 2020

## Cross section 3 downsream of Farmhouse Ct. - 5 YR

### User-defined

Invert Elev (ft) = 6547.80  
Slope (%) = 1.62  
N-Value = 0.035

### Calculations

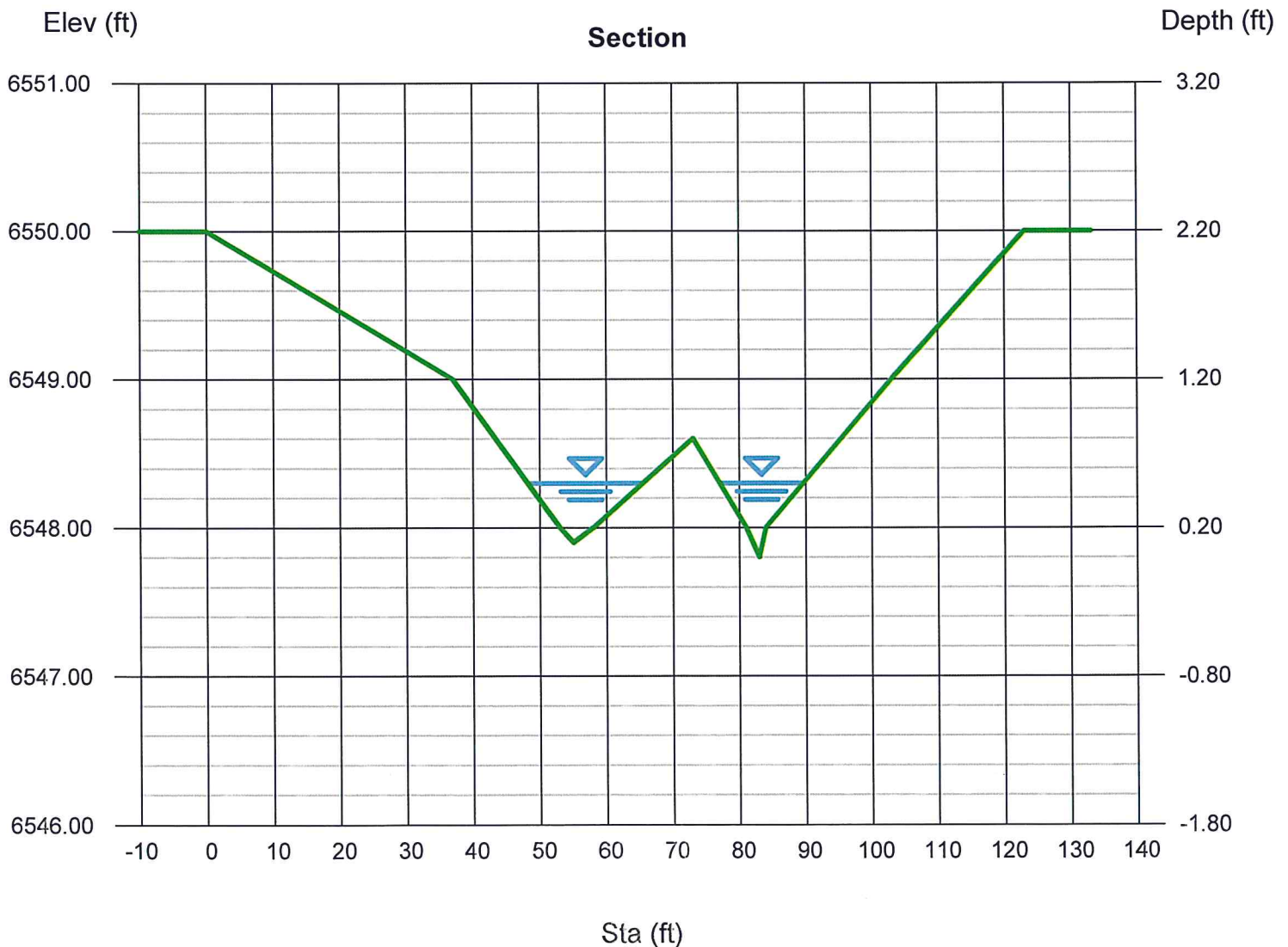
Compute by: Known Q  
Known Q (cfs) = 11.60

### Highlighted

Depth (ft) = 0.50  
Q (cfs) = 11.60  
Area (sqft) = 6.24  
Velocity (ft/s) = 1.86  
Wetted Perim (ft) = 30.05  
Crit Depth, Yc (ft) = 0.45  
Top Width (ft) = 29.98  
EGL (ft) = 0.55

### (Sta, El, n)-(Sta, El, n)...

(0.00, 6550.00)-(37.00, 6549.00, 0.035)-(53.00, 6548.00, 0.035)-(55.00, 6547.90, 0.035)-(58.00, 6548.00, 0.035)-(73.00, 6548.60, 0.035)-(81.00, 6548.00, 0.035)-(83.00, 6547.80, 0.035)-(84.00, 6548.00, 0.035)-(103.00, 6549.00, 0.035)-(123.00, 6550.00, 0.035)





# Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Jun 2 2020

## cross section 3 downsream of Farmhouse Ct. 100-YR

### User-defined

Invert Elev (ft) = 6547.80  
Slope (%) = 1.62  
N-Value = 0.035

### Calculations

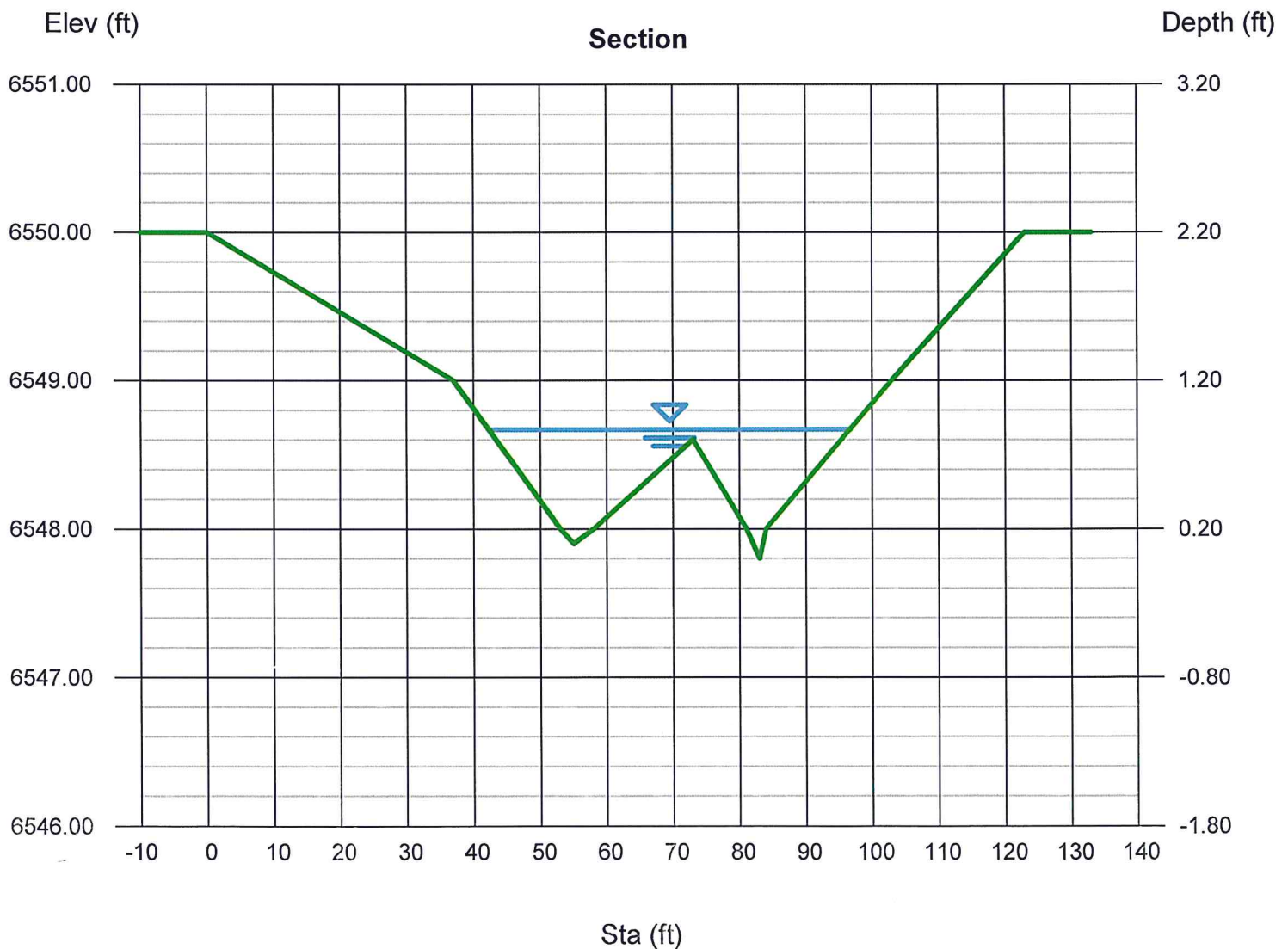
Compute by: Known Q  
Known Q (cfs) = 65.80

### Highlighted

Depth (ft) = 0.87  
Q (cfs) = 65.80  
Area (sqft) = 22.27  
Velocity (ft/s) = 2.95  
Wetted Perim (ft) = 54.55  
Crit Depth, Yc (ft) = 0.82  
Top Width (ft) = 54.45  
EGL (ft) = 1.01

### (Sta, El, n)-(Sta, El, n)...

( 0.00, 6550.00)-(37.00, 6549.00, 0.035)-(53.00, 6548.00, 0.035)-(55.00, 6547.90, 0.035)-(58.00, 6548.00, 0.035)-(73.00, 6548.60, 0.035)-(81.00, 6548.00, 0.035)-(83.00, 6547.80, 0.035)-(84.00, 6548.00, 0.035)-(103.00, 6549.00, 0.035)-(123.00, 6550.00, 0.035)



# Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Jun 2 2020

## cross section 4 downsream outfall 5-YR

### User-defined

Invert Elev (ft) = 6545.50  
Slope (%) = 1.62  
N-Value = 0.035

### Calculations

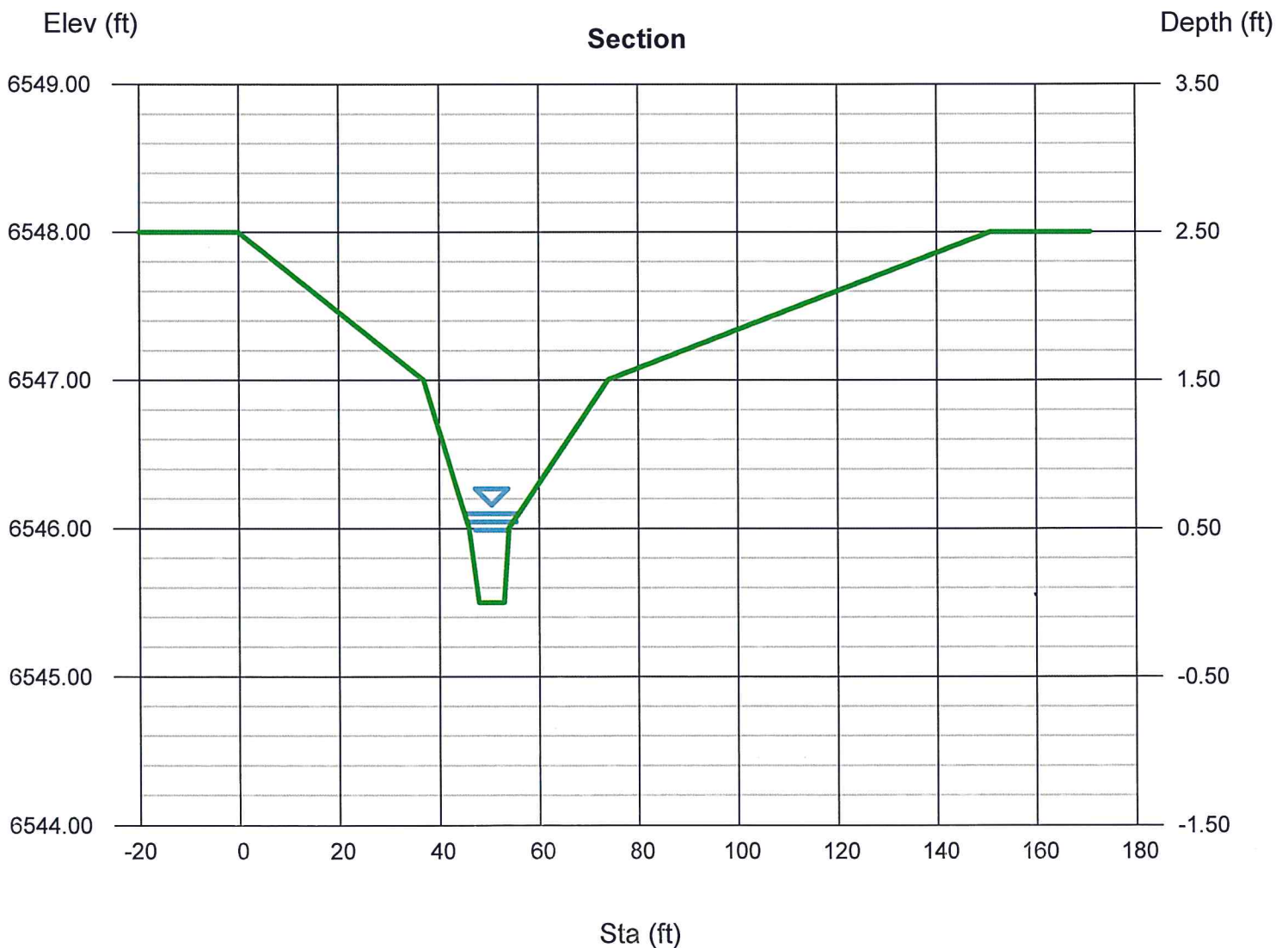
Compute by: Known Q  
Known Q (cfs) = 11.60

### Highlighted

Depth (ft) = 0.60  
Q (cfs) = 11.60  
Area (sqft) = 4.20  
Velocity (ft/s) = 2.76  
Wetted Perim (ft) = 11.09  
Crit Depth, Yc (ft) = 0.50  
Top Width (ft) = 10.90  
EGL (ft) = 0.72

### (Sta, El, n)-(Sta, El, n)...

(0.00, 6548.00)-(37.00, 6547.00, 0.035)-(46.00, 6546.00, 0.035)-(48.00, 6545.50, 0.035)-(53.00, 6545.50, 0.035)-(54.00, 6546.00, 0.035)-(74.00, 6547.00, 0.035)-(151.00, 6548.00, 0.035)



# Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Jun 2 2020

## cross section 4 downsream outfall 100-YR

### User-defined

Invert Elev (ft) = 6545.50  
Slope (%) = 1.62  
N-Value = 0.035

### Calculations

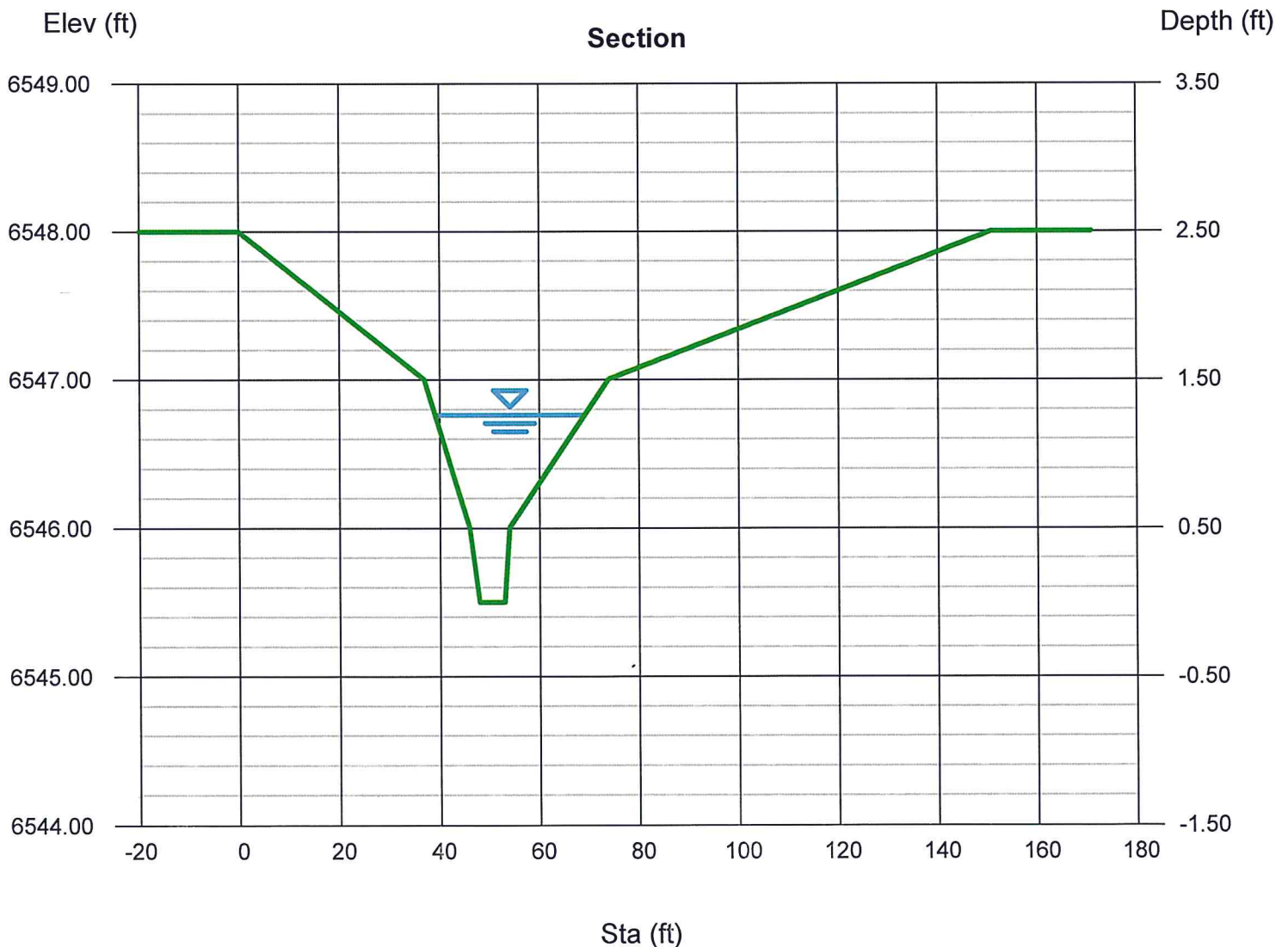
Compute by: Known Q  
Known Q (cfs) = 65.80

### Highlighted

Depth (ft) = 1.26  
Q (cfs) = 65.80  
Area (sqft) = 17.70  
Velocity (ft/s) = 3.72  
Wetted Perim (ft) = 30.27  
Crit Depth, Yc (ft) = 1.19  
Top Width (ft) = 30.03  
EGL (ft) = 1.47

### (Sta, El, n)-(Sta, El, n)...

(0.00, 6548.00)-(37.00, 6547.00, 0.035)-(46.00, 6546.00, 0.035)-(48.00, 6545.50, 0.035)-(53.00, 6545.50, 0.035)-(54.00, 6546.00, 0.035)-(74.00, 6547.00, 0.035)-(151.00, 6548.00, 0.035)



# Farmhouse Court Permanent BMP - Runoff Reduction from Roadside Ditch

## Design Procedure Form: Runoff Reduction

UD-BMP (Version 3.07, March 2018)

Sheet 1 of 1

**Designer:** D. Gorman  
**Company:** M.V.E., Inc.  
**Date:** February 9, 2021  
**Project:** Mountains Edge  
**Location:** Farmhouse Court - Runoff Reduction Calculations for Gravel Roadway Gravel ImperVIOUS Area Draining to Roadside Ditch

### SITE INFORMATION (User Input in Blue Cells)

WQCV Rainfall Depth = 0.60 inches  
 Depth of Average Runoff Producing Storm,  $d_6$  = 0.43 inches (for Watersheds Outside of the Denver Region, Figure 3-1 in USDCM Vol. 3)

Area Type	UIA:RPA	SPA	UIA:RPA	SPA		UIA:RPA	SPA	DCIA	UIA:RPA	SPA	DCIA
Area ID	B2-1	B2-2	B3-1	B3-2		B4a	B4b	B4c	B5a	B5b	B5c
Downstream Design Point ID	DP1	DP1	DP1	DP1		DP2	DP2	DP2	DP2	DP2	DP2
Downstream BMP Type	None	None	None	None		None	None	None	None	None	None
DCIA (ft <sup>2</sup> )	--	--	--	--		--	--	3,044	--	--	7,022
UIA (ft <sup>2</sup> )	11,588	--	7,146	--		10,870	--	--	4,863	--	--
RPA (ft <sup>2</sup> )	9,289	--	5,376	--		7,231	--	--	4,044	--	--
SPA (ft <sup>2</sup> )	--	73,677	--	151,021		--	2,794	--	--	905	--
HSG A (%)	100%	100%	100%	100%		100%	100%	--	100%	100%	--
HSG B (%)	0%	0%	0%	0%		0%	0%	--	0%	0%	--
HSG C/D (%)	0%	0%	0%	0%		0%	0%	--	0%	0%	--
Average Slope of RPA (ft/ft)	0.014	--	0.015	--		0.014	--	--	0.015	--	--
UIA:RPA Interface Width (ft)	575.00	--	390.00	--		530.00	--	--	280.00	--	--

### CALCULATED RUNOFF RESULTS

Area ID	B2-1	B2-2	B3-1	B3-2		B4a	B4b	B4c	B5a	B5b	B5c
UIA:RPA Area (ft <sup>2</sup> )	20,877	--	12,522	--		18,101	--	--	8,907	--	--
L / W Ratio	0.06	--	0.08	--		0.06	--	--	0.11	--	--
UIA / Area	0.5551	--	0.5707	--		0.6005	--	--	0.5460	--	--
Runoff (in)	0.00	0.00	0.00	0.00		0.00	0.00	0.50	0.00	0.00	0.50
Runoff (ft <sup>3</sup> )	0	0	0	0		0	0	127	0	0	293
Runoff Reduction (ft <sup>3</sup> )	483	3684	298	7551		453	140	0	203	45	0

### CALCULATED WQCV RESULTS

Area ID	B2-1	B2-2	B3-1	B3-2		B4a	B4b	B4c	B5a	B5b	B5c
WQCV (ft <sup>3</sup> )	483	0	298	0		453	0	127	203	0	293
WQCV Reduction (ft <sup>3</sup> )	483	0	298	0		453	0	0	203	0	0
WQCV Reduction (%)	100%	0%	100%	0%		100%	0%	0%	100%	0%	0%
Untreated WQCV (ft <sup>3</sup> )	0	0	0	0		0	0	127	0	0	293

### CALCULATED DESIGN POINT RESULTS (sums results from all columns with the same Downstream Design Point ID)

Downstream Design Point ID	DP1	DP2									
DCIA (ft <sup>2</sup> )	0	10,066									
UIA (ft <sup>2</sup> )	18,734	15,733									
RPA (ft <sup>2</sup> )	14,665	11,275									
SPA (ft <sup>2</sup> )	224,698	3,699									
Total Area (ft <sup>2</sup> )	258,097	40,773									
Total Impervious Area (ft <sup>2</sup> )	18,734	25,799									
WQCV (ft <sup>3</sup> )	781	1,075									
WQCV Reduction (ft <sup>3</sup> )	781	656									
WQCV Reduction (%)	100%	61%									
Untreated WQCV (ft <sup>3</sup> )	0	419									

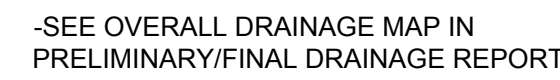
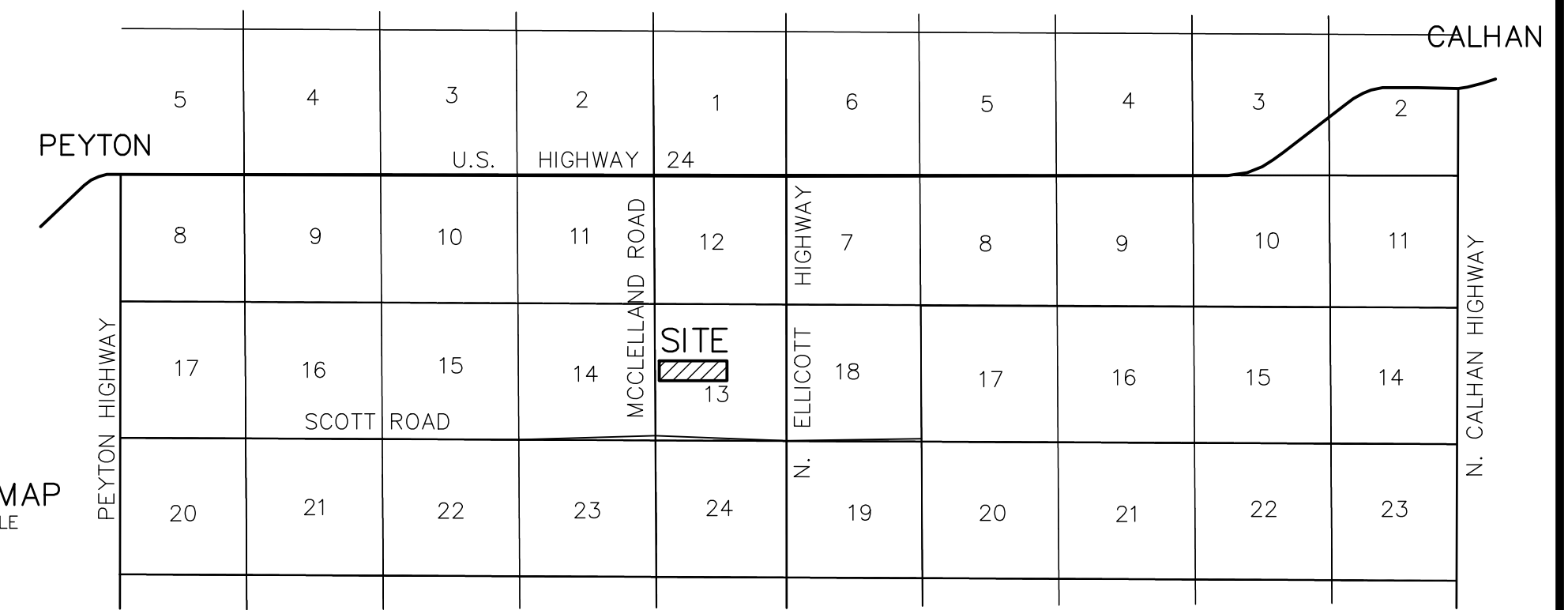
### CALCULATED SITE RESULTS (sums results from all columns in worksheet)








Total Area (ft <sup>2</sup> )	298,870
Total Impervious Area (ft <sup>2</sup> )	44,533
WQCV (ft <sup>3</sup> )	1,856
WQCV Reduction (ft <sup>3</sup> )	1,436
WQCV Reduction (%)	77%
Untreated WQCV (ft <sup>3</sup> )	419

Okay - Meets 60% WCCV Requirement for Runoff Reduction

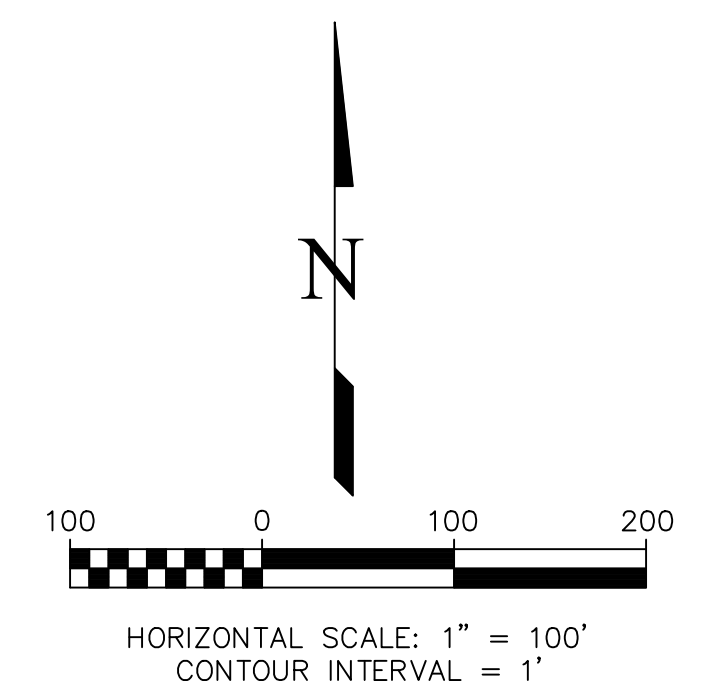


PRELIMINARY / FINAL DRAINAGE REPORT  
EXISTING DRAINAGE PLAN



DRAINAGE PLAN LEGEND	
	BASIN BOUNDARY
	CONCENTRATED FLOW PATH
	FLOW DIRECTION
	BASIN NAME ACREAGE
	DRAINAGE BASIN TAG
	DESIGN POINT DESIGNATION
	DESIGN POINT TAG

DRAINAGE BASIN SUMMARY EXISTING BASINS				
BASIN	DESIGN POINT	AREA (ACRES)	MINOR FLOW (cfs): (5 YR)	MAJOR FLOW (cfs): (100 YR)
A		6.19	1.6	11.8
B		68.89	9.7	63.0
C		2.24	1.6	6.0



FOR INFORMATION ONLY

# MOUNTAINS EDGE

## EXISTING DRAINAGE PLAN

NOT FOR CONSTRUCTION:

THESE PLANS ARE INTENDED  
FOR SUBMITTAL, REVIEW AND  
APPROVAL BY CITY/CITY  
PLANNING DEPARTMENTS AND  
SHOULD NOT BE USED ON SITE  
FOR CONSTRUCTION OR LAYOUT.

[illegible]

H Scale:	XX
V Scale:	XX
Designed By:	DCE
Drawn By:	JAR
Checked By:	DRG
Date:	6/25/19



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Project No.: 08019

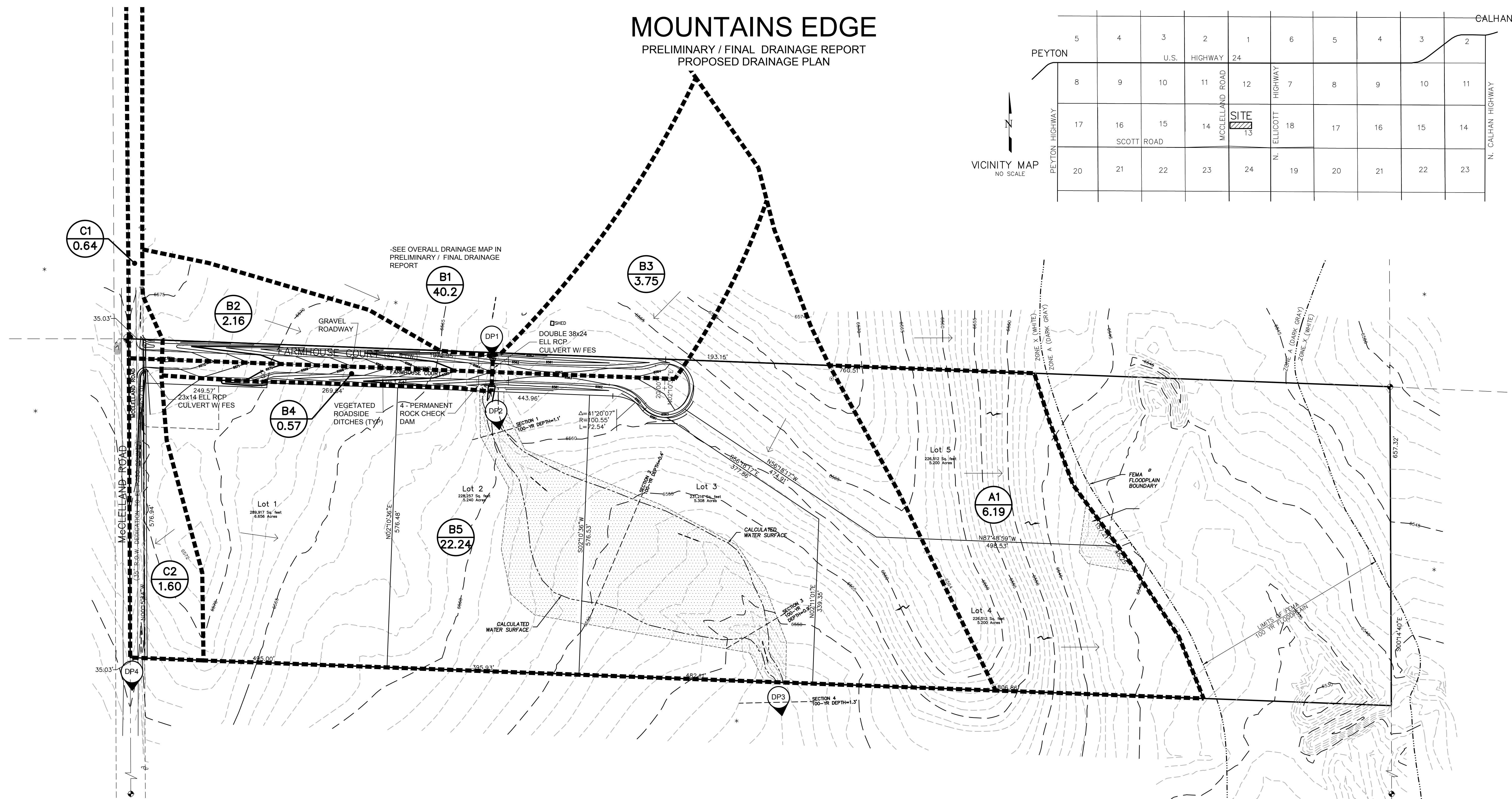
heet:  
1 of 2



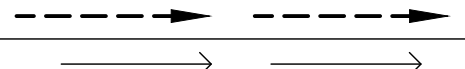


# PRELIMINARY / FINAL DRAINAGE REPORT PROPOSED DRAINAGE PLAN



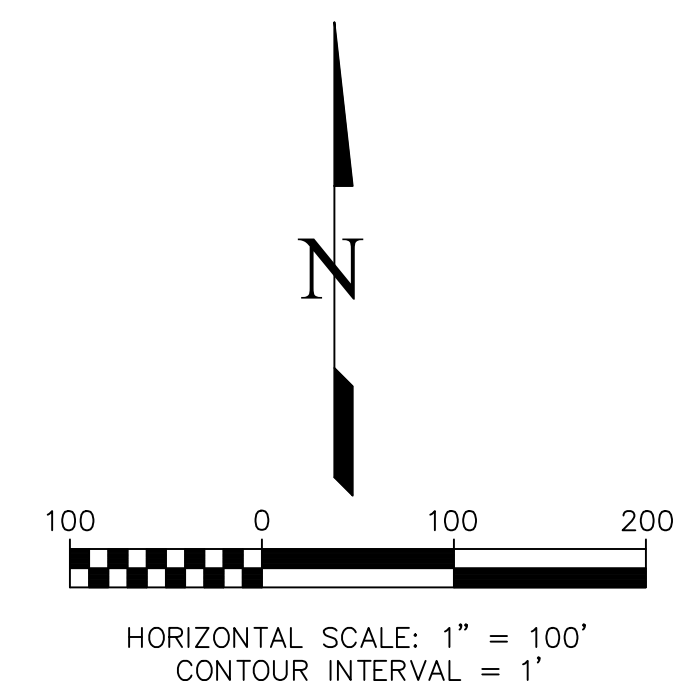
The map shows a grid of 24 lots, numbered 1 through 24. The lots are arranged in a 4x6 grid. The roads shown are Peyton Highway to the west, McClelland Road to the east, Scott Road to the south, N. Ellicott Highway to the south, and N. Calhan Highway to the east. A specific lot, Lot 13, is highlighted with a hatched pattern and labeled 'SITE 13'. The map also shows the locations of Highway 24 and Highway 7, and the names of the adjacent communities: Peyton to the west and Calhan to the east.



CONTOUR LEGEND		
EXISTING	----- (6540)	MAJOR CONTOUR
	----- (41)	MINOR CONTOUR
PROPOSED	===== 6540	MAJOR CONTOUR
	===== 41	MINOR CONTOUR

DRAINAGE PLAN LEGEND	
	BASIN BOUNDARY CONCENTRATED FLOW PATH FLOW DIRECTION
 <div style="display: inline-block; vertical-align: middle; margin-left: 10px;">             BASIN NAME ACREAGE           </div>	DRAINAGE BASIN TAG
	DESIGN POINT TAG DESIGN POINT DESIGNATION

DRAINAGE BASIN SUMMARY				
PROPOSED BASINS				
BASIN	DESIGN POINT	AREA (ACRES)	MINOR FLOW (cfs): (5_YR)	MAJOR FLOW (cfs): (100_YR)
A1		6.19	2.1	12.4
B1		40.2	7.0	42.4
B2		2.16	0.8	3.6
B3		3.75	1.3	6.9
B4		0.57	0.8	2.0
B5		2.24	5.0	27.5
C1		0.64	0.8	2.1
C2		1.60	0.9	3.9
	DP1	46.11	8.3	49.0
	DP2	46.68	8.6	49.7
	DP3	68.92	11.6	65.8
	DP4	2.24	1.6	5.9



# MOUNTAINS EDGE

## PROPOSED DRAINAGE PLAN

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PLANNING DEPARTMENTS AND  
SHOULD NOT BE USED ON SITE  
FOR CONSTRUCTION OR LAYOUT.

[illegible]

H Scale:	XX
V Scale:	XX
Designed By:	DCE
Drawn By:	JAR
Checked By:	DRG
Date:	2/5/21



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Sheet: