# ARACO Concrete Transportation Memorandum (LSC \#194560) <br> PCD File No.: PPR1950 <br> September 17, 2021 

## Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.


## Developer's Statement

$I$, the Developer, have read and will comply with all commitments made on my behalf within this report.


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Please update the site plan on page 74 of 76 to the latest site plan submitted.

Arturo Acosta
ARACO Enterprises
7470 Southmoor Drive
Fountain, CO 80817

RE: ARACO Concrete
El Paso County, CO
Transportation Memorandum
LSC \# 194560
PCD File No. PPR1950

Dear Mr. Acosta,
LSC Transportation Consultants, Inc. has prepared this Transportation Memorandum for the ARACO Concrete site. The site is located at 7470 Southmoor Drive in unincorporated El Paso County, Colorado. Access is proposed to Southmoor Drive. This report has been prepared as part of a Site Development Plan submittal to the El Paso County Planning and Community Development Department.

This memorandum has been prepared primarily to address the existing roadway and traffic conditions, the existing trip generation, and the projected trip generation following the completion of the proposed site improvements, as well as to evaluate the access plan with respect to the criteria in the Engineering Criteria Manual.

## REPORT CONTENTS

The preparation of this report included the following:

- An inventory of existing roadway and traffic conditions on the adjacent and nearby roadway system, including surface conditions, functional classification, jurisdictional control, widths, pavement markings, traffic control signs, posted speed limits, intersection and access spacing, roadway and intersection alignments, roadway grades, and auxiliary turn lanes;
- The proposed site improvement plan and access plan;
- Estimated peak-hour and average daily traffic (ADT) volumes adjacent to the proposed ARACO Concrete site on Southmoor Drive;
- Evaluation of access sight distances and comparison to El Paso County Engineering Criteria Manual (ECM) criteria for stopping sight distance and entering/intersection sight distance;
- Evaluation of the access points with respect to the ECM Criteria contained in Section 2.4.1.; and
- Pedestrian \& bicycle facilities.


## LAND USE AND ACCESS

The 4.2-acre ARACO Concrete site is located at 7470 Southmoor Drive in unincorporated El Paso County, Colorado. Access is to the adjacent Southmoor Drive. The proposed site improvements plan showing the proposed building, on-site circulation, and proposed access points is attached.

The current concrete services business operates out of the 2,000-square-foot building on the site. The company employs eight office staff members and approximately 10-16 field employees who will travel to/from the site for work via private vehicle. Crews then travel to job sites in company vehicles. This site is not a concrete batch plant and no concrete mixer trucks are dispatched from this site.

The proposed site plan shows the addition of a 6,000 -square-foot building. Although this will increase the building square footage on-site from 2,000 to 8,000 square feet, the new building is planned to be used for storage and as a shop. It will not contain business offices. There is also outside storage for contractor equipment, which will remain. The parking area south of the building will be formalized and access to this lot will be better defined. The three spaces in front of the existing building are proposed to remain. The site plan shows three ninety-degree parking spaces. A proposed 40 -foot-wide driveway (curb cut) is proposed for access to these parking spaces. The remainder of the site frontage to Southmoor Drive near these parking spaces will be closed off with new curb so that only the 40 -foot-wide access opening can be used. A deviation has been prepared (and is included with this submittal). This is depicted in Figure 2 and on the attached site plan exhibit.

A 114-space RV \& vehicle storage lot is also proposed on the north side of the site.
ROAD AND TRAFFIC CONDITIONPlease submit the revised deviation request.

The attached site plan shows the streets adjacent to and in the vicinity of the site. Adjacent streets serving the site are identified below, followed by a brief description of each:

US Highway 85/87 (US Hwy 85/87) is classified as NR-A (Non-Rural Principal Highway) extending north from I-25 in Fountain to the City of Colorado Springs. In the vicinity of the site, US Hwy 85/87 has a posted speed limit of 50 miles per hour ( mph ) and is a four-lane urban section with curb and gutter. The T-intersection of US Hwy 85-87/Southmoor Drive is stop-sign-controlled with auxiliary turn lanes.

Southmoor Drive is classified as a two-lane Collector roadway adjacent to the site by the El Paso County road inventory. The County section only extends between 425 feet south of River Drive to the point where the street turns to the east (from which point it extends east to US Highway 85). The posted speed limit on Southmoor Drive is 25 mph , and the paved roadway width is about 22 feet. The section north of the County portion is in the City of Fountain. The City of Fountain Traffic Master Plan shows the "Collector" portion of Southmoor Drive beginning at Carson Boulevard and extending north. There is another County-owned/maintained section north of Lovitt Lane.

## Existing Traffic Volumes

Vehicular turning-movement counts were conducted at the intersection of Southmoor Drive/Araco Concrete parking access/Southmoor Lane on Wednesday, August 14, 2019, from 6:30-8:30 a.m. and from 4:00-6:00 p.m. Count data is attached in Appendix Table 2 and is shown in Figure 2. The current volumes on the adjacent section of Southmoor Drive are light.

## TRIP GENERATION

Estimates of the existing vehicle-trip generation and trip generation following the implementation of the site plan have been made using the nationally published trip generation rates from Trip Generation, $10^{\text {th }}$ Edition, 2017 by the Institute of Transportation Engineers (ITE). Corresponding trip-generation rates from ITE Land Use Category "180 - Specialty Trade Contractor" have been used to develop the trip-generation estimates for the exiting business and are based on the Site Development Plan. Table 2 shows estimates of the trip generation. The estimates are based on ITE rates with "building square footage" as the predictor variable.

Regarding the proposed RV Storage lot, ITE's Trip Generation does not include trip-generation rates specifically for RV/boat storage businesses. For this report "RV/Vehicle Storage" rates (shown in the attached Table 2) are based on the results of a trip-generation study consisting of trip-generation data collection by LSC at several RV storage facilities in El Paso County (2018). Please refer to Appendix A for details.

Table 2 and Figure 3 (attached) present estimates of projected site trip generation. Existing count data have also been used in the trip estimate. The table shows estimates of the existing trip generation of the business, based on traffic count data and ITE rates with "building square footage" as the predictor variable.

The entire site is expected to generate about 105 vehicle trips on the average weekday (one half entering and one half exiting in a 24 -hour period) following expansion. During the morning peak hour, 12 vehicles are projected to enter the site while 5 are projected to exit. Approximately 7 vehicles would enter and 14 vehicles would exit the site during the evening peak hour.

## TRIP DISTRIBUTION AND ASSIGNMENT

## Directional Distribution

An estimate of the proportion of site-generated vehicle trips to/from the north and south on Southmoor Drive and other key study-area streets that will provide access to the site is a necessary component in determining the site's traffic impacts on these study-area streets. Figure 3 shows the estimated distribution/proportion of site-generated trips on the area roadway network.

Estimates were based on the following factors: existing traffic-count data, the proposed land use, the site-access plan, existing and planned area street and roadway system that will provide access to the site, the site's geographic location, and adjacent existing land uses.

It is our understanding that the intersection of US Hwy 85/87 with Carson Avenue has been identified as a future signalized intersection (Destination 2025 Priority Project \#186) on the City of Fountain's Major Thoroughfare Plan. As such, northbound vehicles exiting the site may decide to travel north via Southmoor Drive to access US Hwy 85/87 via the future signal at Carson Avenue rather than turning from the stop-sign-controlled Southmoor Drive intersection with US Hwy 85/87.

## Existing Plus Site-Generated Traffic Volumes

Figure 4 shows the sum of existing traffic volumes (from Figure 2 ) and site-generated peak-hour traffic volumes (shown in Figure 3). These volumes represent the projected short-term total traffic.

## Estimated Future 2041 Background Traffic Volumes

Figure 5 shows the projected 20-year background traffic volumes for the year 2041. Estimated 2041 background traffic volumes on adjacent roadways and at the study-area intersections are based on projected additional development (background traffic) in the vicinity of the site and minor northbound-/southbound-through volume increases on Southmoor Drive. Estimated 2040 background northbound and southbound through traffic volumes on SH 115 are based on the CDOT 20 -year growth factor of 1.25. The 2041 background volumes assume that Southmoor Drive/US 85/87 would remain a stop-sign-controlled intersection. Traffic from the site is not included in the background traffic volumes.

## Future 2041 Total Traffic Volumes

Figure 6 shows the projected 2041 total traffic volumes, which are the sum of 2041 background traffic volumes (from Figure 5) plus long-term site-generated traffic volumes (from Figure 3).

## LEVEL OF SERVICE ANALYSIS

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection and is indicated on a scale from "A" to "F." LOS A is indicative of little congestion or delay. LOS F indicates a high level of congestion or delay. Table 1 shows the level of service delay ranges for signalized and unsignalized intersections.

Table 1: Intersection Levels of Service Delay Ranges

| Level of Service | Signalized Intersections <br> Average Control Delay <br> (seconds per vehicle) | Unsignalized Intersections <br> (seconds per vehicle) ${ }^{(\mathbf{1})}$ |
| :---: | :---: | :---: |
|  | 10.0 sec or less | 10.0 sec or less |
|  | $10.1-20.0 \mathrm{sec}$ | $10.1-15.0 \mathrm{sec}$ |
| C | $20.1-35.0 \mathrm{sec}$ | $15.1-25.0 \mathrm{sec}$ |
| D | $35.1-55.0 \mathrm{sec}$ | $25.1-35.0 \mathrm{sec}$ |
| E | $55.1-80.0 \mathrm{sec}$ | $35.1-50.0 \mathrm{sec}$ |
| F | 80.1 sec or more | 50.1 sec or more |

(1) For unsignalized intersections, if $\mathrm{V} / \mathrm{C}$ ratio is greater than 1.0 the level of service is LOS F, regardless of the projected average control delay per vehicle.

Detailed Synchro reports are attached. A summary of LOS during the weekday morning and evening peak hours for the following unsignalized intersections is shown in the following figures:

- Figure 2: Existing Traffic, Lane Geometry, Traffic Control, and LOS
- Figure 4: Existing + Site Traffic, Lane Geometry, Traffic Control, and LOS
- Figure 5: 2041 Background Traffic, Lane Geometry, Traffic Control, and LOS
- Figure 6: 2041 Background + Site Traffic, Lane Geometry, Traffic Control, and LOS


## Site Access Intersections

All approaches at both the north and south site-access points currently operate at and are projected to remain at LOS A or better following site buildout. Please refer to the detailed Synchro reports (attached) for additional details.

## Southmoor Drive/US 85-87

The northeast-bound left-turn at the intersection of US Hwy 85-87/Southmoor Drive currently operates at LOS F and is projected to remain LOS F during the afternoon peak hour through the 20-year horizon. However, analysis results show a volume-to-capacity ( $\mathrm{v} / \mathrm{c}$ ) ratio to be well below 1.00 for the eastbound-through turning movement during all short-term traffic scenarios. This is not uncommon for minor street approaches on arterial streets to operate at levels of service E or even F during peak periods, as signal timings would be adjusted to favor heavier northbound and southbound through volumes on US Hwy 85-87. Despite Synchro's reported LOS F (HCM methodology) for the northeast-
bound left-turning movement during the afternoon peak hour, gaps created from the nearby signal at US Hwy 85-87/Mesa Ridge Parkway would allow side street vehicles to turn left onto US Hwy 85-87.

## SITE-ACCESS PLAN

## ECM Criteria for Access Design

The north and south site-access points will be allowed from the adjacent Southmoor Drive (Collector roadway). County staff has also indicated that a defined, single, 40-foot-wide driveway on Southmoor Drive will also be allowed for vehicular access to the three parking spaces in front of the building. Conditions of approval apply. Note: the head-in parking spaces on Southmoor Drive are addressed separately in the deviation request. The following summarizes Engineering Criteria Manual Section 2.4.1 access criteria, which states the following five access design guidelines:

- Adequate spacing
- Proper alignments
- Clear sight distances
- Coordinated widths with its intended use
- Clearances from intersections

The following sections address each of these criteria for access-point design throughout the site.

## Adequate Spacing

Southmoor Drive is a Collector roadway. The ECM indicates that accesses shall be separated by a distance equal to the entering sight-distance values in Table 2-35. Based on a posted speed limit of 25 mph , the prescribed spacing would be 425 feet. The distance between the two site-access points is 480 feet, which meets ECM criteria.

## Access Alignment

All proposed site-access points should be aligned at 90 degrees to the adjacent roadway centerline. The adjacent roadway grades are essentially level. The vertical alignment criteria in ECM Section 2.4.1.C. 2 shall be met for the driveways. The access points are shown to intersect Southmoor Drive at 90 degrees.

## Access Sight Distances

The access sight distance criteria in section 2.4.1.D would apply:
"Any potentially obstructing objects, such as but not limited to advertising signs, structures, trees, and bushes, shall be designed, placed, and maintained at a height not to interfere with the sight distance needed by any vehicle using the access."

Southmoor Drive has a straight horizontal alignment with no significant vertical curvature that would limit access sight distance. Site improvements, such as signs, on-street angled parking, and landscaping, should not impede the required sight-distance lines of sight. The sight distance from the south access to the 90-degree corner to the southeast would be acceptable, given the design speed of that corner and the distance from the driver's eye at the access.

Based on a $25-\mathrm{mph}$ posted speed limit, sight distances for both approaches from both proposed site-access locations exceed the required 425-foot requirement for multi-unit trucks, per ECM Table 2-35, with one exception - the sight distance to the south from the south access point. The following analysis corresponds to sight distances for the proposed site-access intersections with Southmoor Drive.

## Proposed Southmoor Drive/North Site-Access Intersection

Sight distances are as follows:

- To the northwest: greater than $1 / 4$-mile
- To the southeast: 730 feet (unobstructed to L-corner turn on Southmoor Drive)


## Proposed Southmoor Drive/South Site-Access Intersection

Sight distances are as follows:

- To the northwest: greater than 1/4-mile
- To the southeast: 290 feet (unobstructed to L-corner turn on Southmoor Drive). Although this is short of the 425 -foot $E C M$ minimum criteria, the design speed for traffic arriving from the south around the tight horizontal curve in Southmoor Drive is about 13 to 14 mph (the curve-warning sign indicates an advisory speed of 10 mph ). Based on the speed of the approaching vehicle as it turns the corner and is seen by possible, but infrequent, multi-unit trucks, the intersection sight distance, based on the AASHTO formula, is 236 feet. As 290 feet is available, the sight distance is acceptable.


## Access Width

The ECM requires a minimum of 25 -foot width (and maximum of 40 feet) for a commercial access point on a Non-Residential Collector roadway. The south access drive ( 30 feet wide) would meet this criterion, while the north (gated) access drive ( 24 feet wide) would be just short of ECM criterion. The middle access, which will provide access to the three parking spaces in front of the existing building, will be 40 feet wide.

For the north access, LSC recommends a 65 -foot stacking distance between the entry gate and the west edge of Southmoor Drive. This would allow for a Class A RV, 30-foot-long single-unit truck or a 35 -foot-long U-Haul truck (largest size) plus an additional 30 feet to allow for a towed utility trailer, moving trailer, or following passenger vehicle.

## Clearances from Intersections

Regarding access clearance from intersection criteria outlined in Section 2.4.1.F of the ECM:
Access to commercial or industrial properties fronting collector or local roads shall be located a minimum of 50 feet from the point of curvature or point of tangency of the curb line at the intersection. Access to commercial or industrial parcels fronting Nonresidential Collector roadways shall be located a minimum of 115 480 feet from the point of curvature or point of tangency of the curb line at the intersection, depending on the sight distance and location with respect to the intersection, intersection control, and posted speed.

In all cases, a minimum corner clearance of 50 feet shall be provided. If the minimum corner clearance cannot be attained, the ECM Administrator may require investigation to determine if left turns should be prohibited into or out of the access point. For proposed access points near stop or signalized intersections, the ECM Administrator will require studies to determine if stopping queues will block the access point and if left turns should be prohibited into or out of the access point.

Based on the proposed driveway locations shown in the site plan, the south access point would have a centerline offset of about 100 feet from the nearest intersection (Southmoor Drive/Southmoor Lane), which meets the aforementioned ECM criteria. This is a minor intersection with low through volumes and low turning volumes to/from Southmoor Lane.

## PEDESTRIAN \& BICYCLE FACILITIES

Sidewalks exist within the City of Fountain just to the north of the site (at the River Drive/Southmoor intersection). The existing drainage structure just to the north appears to have limited width and future sidewalk installation may not be feasible. There is an existing trail located just north of the site - the Crews Gulch Trail. The section of Southmoor connecting to US Hwy 85/87 has curb \& gutter, but no sidewalk. Sidewalk exists along the west side of US Hwy 85/87. Southmoor Lane, which exists directly across from this site, extends east to US Hwy 85. Although Southmoor adjacent to the site frontage does not have sidewalks on either side, the traffic volumes are light and there are paved areas and gravel shoulder areas along the site frontage which can be utilized by the few pedestrians who may walk through this generally industrial area at the south end of Southmoor Drive. Pedestrians would have the option to utilize Southmoor Lane, which is a slower speed, narrow street, to connect to sidewalks along Highway 85/87.

## ACCESS AUTOTURN VEHICLE-TURNING ANALYSIS

AutoTurn analysis was run at the request of staff and to assist with the planning and design of the proposed north site access. Detailed AutoTurn analysis exhibits depicting entering and exiting B-40 (simulating a Class A RV) vehicle-movement wheel paths are attached as "AutoTurn Exhibits 1-4."

## CONCLUSIONS/RECOMMENDATIONS

## Access Evaluation

- The site-access points meet ECM Criteria, or the intent of the ECM Criteria (in the case of the sight distance to the south at the south access point). Please refer to this section of the report for details.
- No improvements would be required to the existing auxiliary turn lanes at the intersection of US 85-87/Southmoor Drive due to this development's impact.


## Level of Service Analysis

- The site-access points would operate at an acceptable LOS. The intersection of Southmoor Drive/US Hwy 85 has been analyzed and results indicate a LOS F for side street left-turn movements during peak hours. This is not likely to be signalized or converted to a right-in/right-out intersection. Alternatives to the eastbound left-turn movement at this intersection are available. Please refer to this section of the report for details.


## County Road Impact Fee Program

## South Portion of the Site

This project will be required to participate in the El Paso County Road Improvement Fee Program. The preliminary indication from the applicant is to opt out of the PID option. The applicable fee program land use is "Industrial" and the corresponding building permit "Full Fee" is $\$ 3,651$ per thousand square feet. Based on 6,000 additional square feet, the fee amount would be $\$ 21,906$, plus the amount for the RV storage.

North Portion of the site (RV Storage)

Per our understanding of recent correspondence received from the County Principal Transportation Planner on another proposed RV Storage use, the roadway impact fee shall be calculated based on:

- The total square footage of RV storage parking spaces (not including drive aisles, landscaping islands, etc.) and
- The mini warehouse fee rate of $\$ 725$ per 1,000 square feet.
- The latest site plan indicates that the RV storage parking spaces would cover about 26,250 square feet. (26.25 KSF)
- Therefore, the calculated County Roadway Impact Fee for the RV storage use is \$19,031.
- This amount paid should be taken into account in the future upon any redevelopment of the RV storage area, so fees are not paid twice for the same lot.

Please contact me if you have any questions regarding this report.
Respectfully Submitted,

LSC TRANSPORTATION CONSULTANTS, INC.
By: Jeffrey C. Hodsdon, P.E.
Principal
JCH/JAB:jas

## Enclosures: Table 2

Figures 1-6
Appendix Table: Traffic Count Data
Traffic Counts
Levels of Service
AutoTurn Exhibits 1-4
Site Plan Exhibit
Appendix A - Trip Generation Study for RV Storage

Trip Generation Table

Table 2: Trip Generation Estimate

| ITE |  | Value | Units ${ }^{1}$ | Trip Generation Rates ${ }^{2}$ |  |  |  |  | Total Trips Generated |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Average <br> Weekday |  | A.M. |  | P.M. |  | Average Weekday | A.M. |  | P.M. |  |
| Code | Description |  |  |  | In | Out | In |  | Out | In | Out | In | Out |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Existing Trip Generation "Snapshot" (from an Actual Count) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 180 | Specialty Trade Contractor | 2.062 | KSF | - | - | - | - | - | N/A | 5 | 3 | 5 | 7 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Estimate Based on Building Square Footage (Based on ITE Rates) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 180 | Specialty Trade Contractor | 2.062 | KSF | 10.22 | 1.21 | 0.45 | 0.63 | 1.34 | 21 | 2 | 1 | 1 | 3 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Difference: Existing (Based on Counts) Minus Existing (Based on ITE Rates) |  |  |  |  |  |  |  |  | - | 3 | 2 | 4 | 4 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Estimate of Trips Following Site Improvements (Based on ITE Fitted Rates) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 180 | Specialty Trade Contractor | 8.062 | KSF | 10.22 | 1.21 | 0.45 | 0.63 | 1.34 | 82 | 10 | 4 | 5 | 11 |
| - | RV/Vehicle Storage | 1.14 | HOC | 20.00 | 2.28 | 1.37 | 1.98 | 2.81 | 23 | 2 | 1 | 2 | 3 |
|  |  |  |  |  |  |  |  | Total | 105 | 12 | 5 | 7 | 14 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ${ }^{1}$ KSF $=1,000$ square feet of gross floor area, $\mathrm{HOC}=$ hundred occupied spaces |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ${ }^{2}$ Source: Trip Generation, 10th Edition, 2017, by the Institute of Transportation Engineers (ITE) |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Note: "RV/Vehicle Storage" rates based on RV storage facility turning movement counts conducted by LSC in El Paso County (2018) |  |  |  |  |  |  |  |  |  |  |  |  |  |

Figures









## Appendix Tables

Appendix Table: Traffic Count Data
Southmoor Drive/Southmoor Lane/Existing Araco Access


## Traffic Counts

## LSC Transportation Consultants, Inc.

545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Hwy 85-87-Southmoor Dr AM
Site Code : 194560
Start Date : 7/16/2020
Page No : 1

Groups Printed- Unshifted

|  | Hwy 85/87 Southbound |  |  |  |  | Westbound |  |  |  |  | Hwy 85/87 Northbound |  |  |  |  | Southmoor Dr Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | Int. Total |
| 07:00 AM | 0 | 88 | 0 | 0 | 88 | 0 | 0 | 0 | 0 | 0 | 6 | 131 | 0 | 0 | 137 | 0 | 0 | 9 | 0 | 9 | 234 |
| 07:15 AM | 0 | 119 | 0 | 0 | 119 | 0 | 0 | 0 | 0 | 0 | 7 | 124 | 0 | 0 | 131 | 2 | 0 | 7 | 0 | 9 | 259 |
| 07:30 AM | 0 | 144 | 4 | 0 | 148 | 0 | 0 | 0 | 0 | 0 | 8 | 186 | 0 | 1 | 195 | 3 | 0 | 8 | 0 | 11 | 354 |
| 07:45 AM | 0 | 138 | 1 | 0 | 139 | 0 | 0 | 0 | 0 | 0 | 6 | 154 | 0 | 2 | 162 | 3 | 0 | 5 | 0 | 8 | 309 |
| Total | 0 | 489 | 5 | 0 | 494 | 0 | 0 | 0 | 0 | 0 | 27 | 595 | 0 | 3 | 625 | 8 | 0 | 29 | 0 | 37 | 1156 |
| 08:00 AM | 0 | 136 | 4 | 0 | 140 | 0 | 0 | 0 | 0 | 0 | 6 | 111 | 0 | 1 | 118 | 3 | 0 | 8 | 0 | 11 | 269 |
| 08:15 AM | 0 | 142 | 2 | 0 | 144 | 0 | 0 | 0 | 0 | 0 | 7 | 120 | 0 | 0 | 127 | 5 | 0 | 19 | 0 | 24 | 295 |
| 08:30 AM | 0 | 148 | 2 | 0 | 150 | 0 | 0 | 0 | 0 | 0 | 6 | 140 | 0 | 0 | 146 | 2 | 0 | 17 | 0 | 19 | 315 |
| 08:45 AM | 0 | 121 | 4 | 0 | 125 | 0 | 0 | 0 | 0 | 0 | 7 | 120 | 0 | 0 | 127 | 3 | 0 | 7 | 0 | 10 | 262 |
| Total | 0 | 547 | 12 | 0 | 559 | 0 | 0 | 0 | 0 | 0 | 26 | 491 | 0 | 1 | 518 | 13 | 0 | 51 | 0 | 64 | 1141 |
| Grand Total | 0 | 1036 | 17 | 0 | 1053 | 0 | 0 | 0 | 0 | 0 | 53 | 1086 | 0 | 4 | 1143 | 21 | 0 | 80 | 0 | 101 | 2297 |
| Apprch \% | 0 | 98.4 | 1.6 | 0 |  | 0 | 0 | 0 | 0 |  | 4.6 | 95 | 0 | 0.3 |  | 20.8 | 0 | 79.2 | 0 |  |  |
| Total \% | 0 | 45.1 | 0.7 | 0 | 45.8 | 0 | 0 | 0 | 0 | 0 | 2.3 | 47.3 | 0 | 0.2 | 49.8 | 0.9 | 0 | 3.5 | 0 | 4.4 |  |

## LSC Transportation Consultants, Inc.

## 545 E Pikes Peak Ave, Suite 210

Colorado Springs, CO 80905
719-633-2868
File Name : Hwy 85-87-Southmoor Dr AM
Site Code : 194560
Start Date : 7/16/2020
Page No : 2

|  | Hwy 85/87 Southbound |  |  |  |  | Westbound |  |  |  |  | Hwy 85/87 Northbound |  |  |  |  | Southmoor Dr Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | Int. Total |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 7:30:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 7:30:00 AM | 0 | 144 | 4 | 0 | 148 | 0 | 0 | 0 | 0 | 0 | 8 | 186 | 0 | 1 | 195 | 3 | 0 | 8 | 0 | 11 | 354 |
| 7:45:00 AM | 0 | 138 | 1 | 0 | 139 | 0 | 0 | 0 | 0 | 0 | 6 | 154 | 0 | 2 | 162 | 3 | 0 | 5 | 0 | 8 | 309 |
| 8:00:00 AM | 0 | 136 | 4 | 0 | 140 | 0 | 0 | 0 | 0 | 0 | 6 | 111 | 0 | 1 | 118 | 3 | 0 | 8 | 0 | 11 | 269 |
| 8:15:00 AM | 0 | 142 | 2 | 0 | 144 | 0 | 0 | 0 | 0 | 0 | 7 | 120 | 0 | 0 | 127 | 5 | 0 | 19 | 0 | 24 | 295 |
| Total Volume | 0 | 560 | 11 | 0 | 571 | 0 | 0 | 0 | 0 | 0 | 27 | 571 | 0 | 4 | 602 | 14 | 0 | 40 | 0 | 54 | 1227 |
| \% App. Total | 0 | 98.1 | 1.9 | 0 |  | 0 | 0 | 0 | 0 |  | 4.5 | 94.9 | 0 | 0.7 |  | 25.9 | 0 | 74.1 | 0 |  |  |
| PHF | . 000 | . 972 | . 688 | . 000 | . 965 | . 000 | . 000 | . 000 | . 000 | . 000 | . 844 | . 767 | . 000 | . 500 | . 772 | . 700 | . 000 | . 526 | . 000 | . 563 | . 867 |

## LSC Transportation Consultants, Inc.

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Colorado Springs, CO 80905
719-633-2868
File Name : Hwy 85-87-Southmoor Dr AM
Site Code : 194560
Start Date : 7/16/2020
Page No
: 3


## LSC Transportation Consultants, Inc.

## 545 E Pikes Peak Ave, Suite 210

Colorado Springs, CO 80905
719-633-2868
File Name : Hwy 85-87-Southmoor Dr AM
Site Code : 194560
Start Date : 7/16/2020
Page No : 4

|  | Hwy 85/87 Southbound |  |  |  |  | Westbound |  |  |  |  | Hwy 85/87 Northbound |  |  |  |  | Southmoor Dr Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total |  |
| Peak Hour Analysis From 7:00:00 AM to 8:45:00 AM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | 7:45:00 AM |  |  |  |  | 7:00:00 AM |  |  |  |  | 7:00:00 AM |  |  |  |  | 8:00:00 AM |  |  |  |  |  |
| +0 mins. | 0 | 138 | 1 | 0 | 139 | 0 | 0 | 0 | 0 | 0 | 6 | 131 | 0 | 0 | 137 | 3 | 0 | 8 | 0 | 11 |  |
| +5 mins. | 0 | 136 | 4 | 0 | 140 | 0 | 0 | 0 | 0 | 0 | 7 | 124 | 0 | 0 | 131 | 5 | 0 | 19 | 0 | 24 |  |
| +10 mins. | 0 | 142 | 2 | 0 | 144 | 0 | 0 | 0 | 0 | 0 | 8 | 186 | 0 | 1 | 195 | 2 | 0 | 17 | 0 | 19 |  |
| +15 mins. | 0 | 148 | 2 | 0 | 150 | 0 | 0 | 0 | 0 | 0 | 6 | 154 | 0 | 2 | 162 | 3 | 0 | 7 | 0 | 10 |  |
| Total Volume | 0 | 564 | 9 | 0 | 573 | 0 | 0 | 0 | 0 | 0 | 27 | 595 | 0 | 3 | 625 | 13 | 0 | 51 | 0 | 64 |  |
| \% App. Total | 0 | 98.4 | 1.6 | 0 |  | 0 | 0 | 0 | 0 |  | 4.3 | 95.2 | 0 | 0.5 |  | 20.3 | 0 | 79.7 | 0 |  |  |
| PHF | . 000 | . 953 | . 563 | . 000 | . 955 | . 000 | . 000 | . 000 | . 000 | . 000 | . 844 | . 800 | . 000 | . 375 | . 801 | . 650 | . 000 | . 671 | . 000 | . 667 |  |

## LSC Transportation Consultants, Inc.

545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Hwy 85-87-Southmoor Dr AM
Site Code : 194560
Start Date : 7/16/2020
Page No : 5


## LSC Transportation Consultants, Inc.

545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Hwy 85-87-Southmoor Dr PM
Site Code : 194560
Start Date : 7/16/2020
Page No : 1

Groups Printed- Unshifted

|  | Hwy 85-87 Southbound |  |  |  |  | Westbound |  |  |  |  | Hwy 85-87 Northbound |  |  |  |  | Southmoor Dr Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start <br> Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | Int. Total |
| 04:00 PM | 0 | 223 | 5 | 0 | 228 | 0 | 0 | 0 | 0 | 0 | 6 | 215 | 0 | 0 | 221 | 2 | 0 | 5 | 0 | 7 | 456 |
| 04:15 PM | 0 | 251 | 1 | 0 | 252 | 0 | 0 | 0 | 0 | 0 | 10 | 203 | 0 | 1 | 214 | 4 | 0 | 14 | 0 | 18 | 484 |
| 04:30 PM | 0 | 240 | 4 | 0 | 244 | 0 | 0 | 0 | 0 | 0 | 6 | 207 | 0 | 0 | 213 | 3 | 0 | 10 | 0 | 13 | 470 |
| 04:45 PM | 0 | 286 | 2 | 0 | 288 | 0 | 0 | 0 | 0 | 0 | 6 | 231 | 0 | 0 | 237 | 1 | 0 | 2 | 0 | 3 | 528 |
| Total | 0 | 1000 | 12 | 0 | 1012 | 0 | 0 | 0 | 0 | 0 | 28 | 856 | 0 | 1 | 885 | 10 | 0 | 31 | 0 | 41 | 1938 |
| 05:00 PM | 0 | 247 | 3 | 0 | 250 | 0 | 0 | 0 | 0 | 0 | 8 | 214 | 0 | 0 | 222 | 1 | 0 | 4 | 0 | 5 | 477 |
| 05:15 PM | 0 | 298 | 7 | 0 | 305 | 0 | 0 | 0 | 0 | 0 | 7 | 217 | 0 | 0 | 224 | 4 | 0 | 16 | 0 | 20 | 549 |
| 05:30 PM | 0 | 260 | 3 | 0 | 263 | 0 | 0 | 0 | 0 | 0 | 7 | 197 | 0 | 1 | 205 | 0 | 0 | 8 | 0 | 8 | 476 |
| 05:45 PM | 0 | 247 | 5 | 0 | 252 | 0 | 0 | 0 | 0 | 0 | 6 | 198 | 0 | 0 | 204 | 4 | 0 | 12 | 0 | 16 | 472 |
| Total | 0 | 1052 | 18 | 0 | 1070 | 0 | 0 | 0 | 0 | 0 | 28 | 826 | 0 | 1 | 855 | 9 | 0 | 40 | 0 | 49 | 1974 |
| Grand Total | 0 | 2052 | 30 | 0 | 2082 | 0 | 0 | 0 | 0 | 0 | 56 | 1682 | 0 | 2 | 1740 | 19 | 0 | 71 | 0 | 90 | 3912 |
| Apprch \% | 0 | 98.6 | 1.4 | 0 |  | 0 | 0 | 0 | 0 |  | 3.2 | 96.7 | 0 | 0.1 |  | 21.1 | 0 | 78.9 | 0 |  |  |
| Total \% | 0 | 52.5 | 0.8 | 0 | 53.2 | 0 | 0 | 0 | 0 | 0 | 1.4 | 43 | 0 | 0.1 | 44.5 | 0.5 | 0 | 1.8 | 0 | 2.3 |  |

## LSC Transportation Consultants, Inc.

## 545 E Pikes Peak Ave, Suite 210

Colorado Springs, CO 80905
719-633-2868
File Name : Hwy 85-87-Southmoor Dr PM
Site Code : 194560
Start Date : 7/16/2020
Page No :2

|  | Hwy 85-87 Southbound |  |  |  |  | Westbound |  |  |  |  | Hwy 85-87 Northbound |  |  |  |  | Southmoor Dr Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | Int. Total |
| Peak Hour Analysis From 4:00:00 PM to 5:45:00 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 4:45:00 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 4:45:00 PM | 0 | 286 | 2 | 0 | 288 | 0 | 0 | 0 | 0 | 0 | 6 | 231 | 0 | 0 | 237 | 1 | 0 | 2 | 0 | 3 | 528 |
| 5:00:00 PM | 0 | 247 | 3 | 0 | 250 | 0 | 0 | 0 | 0 | 0 | 8 | 214 | 0 | 0 | 222 | 1 | 0 | 4 | 0 | 5 | 477 |
| 5:15:00 PM | 0 | 298 | 7 | 0 | 305 | 0 | 0 | 0 | 0 | 0 | 7 | 217 | 0 | 0 | 224 | 4 | 0 | 16 | 0 | 20 | 549 |
| 5:30:00 PM | 0 | 260 | 3 | 0 | 263 | 0 | 0 | 0 | 0 | 0 | 7 | 197 | 0 | 1 | 205 | 0 | 0 | 8 | 0 | 8 | 476 |
| Total Volume | 0 | 1091 | 15 | 0 | 1106 | 0 | 0 | 0 | 0 | 0 | 28 | 859 | 0 | 1 | 888 | 6 | 0 | 30 | 0 | 36 | 2030 |
| \% App. Total | 0 | 98.6 | 1.4 | 0 |  | 0 | 0 | 0 | 0 |  | 3.2 | 96.7 | 0 | 0.1 |  | 16.7 | 0 | 83.3 | 0 |  |  |
| PHF | . 000 | . 915 | . 536 | . 000 | . 907 | . 000 | . 000 | . 000 | . 000 | . 000 | . 875 | . 930 | . 000 | . 250 | . 937 | . 375 | . 000 | . 469 | . 000 | . 450 | . 924 |

## LSC Transportation Consultants, Inc.

545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Hwy 85-87 - Southmoor Dr PM
Site Code : 194560
Start Date : 7/16/2020
Page No
: 3


## LSC Transportation Consultants, Inc.

## 545 E Pikes Peak Ave, Suite 210

Colorado Springs, CO 80905
719-633-2868
File Name : Hwy 85-87-Southmoor Dr PM
Site Code : 194560
Start Date : 7/16/2020
Page No : 4

|  | Hwy 85-87 Southbound |  |  |  |  | Westbound |  |  |  |  | Hwy 85-87 Northbound |  |  |  |  | Southmoor Dr Eastbound |  |  |  |  | Int. Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total | L | T | R | U | App. Total |  |
| Peak Hour Analysis From 4:00:00 PM to 5:45:00 PM - Peak 1 of 1 Peak Hour for Each Approach Begins at: |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | 4:45:00 PM |  |  |  |  | 4:00:00 PM |  |  |  |  | 4:30:00 PM |  |  |  |  | 5:00:00 PM |  |  |  |  |  |
| +0 mins. | 0 | 286 | 2 | 0 | 288 | 0 | 0 | 0 | 0 | 0 | 6 | 207 | 0 | 0 | 213 | 1 | 0 | 4 | 0 | 5 |  |
| +5 mins. | 0 | 247 | 3 | 0 | 250 | 0 | 0 | 0 | 0 | 0 | 6 | 231 | 0 | 0 | 237 | 4 | 0 | 16 | 0 | 20 |  |
| +10 mins. | 0 | 298 | 7 | 0 | 305 | 0 | 0 | 0 | 0 | 0 | 8 | 214 | 0 | 0 | 222 | 0 | 0 | 8 | 0 | 8 |  |
| +15 mins. | 0 | 260 | 3 | 0 | 263 | 0 | 0 | 0 | 0 | 0 | 7 | 217 | 0 | 0 | 224 | 4 | 0 | 12 | 0 | 16 |  |
| Total Volume | 0 | 1091 | 15 | 0 | 1106 | 0 | 0 | 0 | 0 | 0 | 27 | 869 | 0 | 0 | 896 | 9 | 0 | 40 | 0 | 49 |  |
| \% App. Total | 0 | 98.6 | 1.4 | 0 |  | 0 | 0 | 0 | 0 |  | 3 | 97 | 0 | 0 |  | 18.4 | 0 | 81.6 | 0 |  |  |
| PHF | . 000 | . 915 | . 536 | . 000 | . 907 | . 000 | . 000 | . 000 | . 000 | . 000 | . 844 | . 940 | . 000 | . 000 | . 945 | . 563 | . 000 | . 625 | . 000 | . 613 |  |

## LSC Transportation Consultants, Inc.

545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Hwy 85-87 - Southmoor Dr PM
Site Code : 194560
Start Date : 7/16/2020
Page No : 5


LSC Transportation Consultants, Inc.
545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Southmoor Dr - Araco Concrete Access AM
Site Code : 00000000
Start Date : 8/14/2019
Page No : 1

|  | Southmoor Dr Southbound |  |  |  |  | Araco Access Westbound |  |  |  |  | Southmoor Dr Northbound |  |  |  |  | Araco Annex Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Through | Right | Peds | App. Total | Left | Through | Right | Peds | App. Total | Left | Through | Right | Peds | App. Total | Left | Through | Right | Peds | App. Total | Int. Total |
| 06:30 AM | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 6 |
| 06:45 AM | 1 | 6 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 7 |
| Total | 1 | 10 | 0 | 0 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 13 |
| 07:00 AM | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 2 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 8 |
| 07:15 AM | 1 | 1 | 0 | 0 | 2 | 1 | 0 | 1 | 0 | 2 | 2 | 1 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 7 |
| 07:30 AM | 0 | 5 | 0 | 0 | 5 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 2 | 0 | 2 | 9 |
| 07:45 AM | 0 | 2 | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 2 | 0 | 4 | 1 | 0 | 5 | 0 | 0 | 1 | 0 | 1 | 10 |
| Total | 1 | 11 | 0 | 0 | 12 | 4 | 0 | 1 | 0 | 5 | 5 | 5 | 4 | 0 | 14 | 0 | 0 | 3 | 0 | 3 | 34 |
| 08:00 AM | 0 | 4 | 0 | 0 | 4 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 |
| 08:15 AM | 0 | 3 | 0 | 0 | 3 | 1 | 0 | 1 | 0 | 2 | 1 | 3 | 0 | 0 | 4 | 0 | 0 | 0 | 0 | 0 | 9 |
| Grand Total | 2 | 28 | 0 | 0 | 30 | 6 | 0 | 2 | 0 | 8 | 6 | 9 | 4 | 0 | 19 | 0 | 0 | 4 | 0 | 4 | 61 |
| Apprch \% | 6.7 | 93.3 | 0 | 0 |  | 75 | 0 | 25 | 0 |  | 31.6 | 47.4 | 21.1 | 0 |  | 0 | 0 | 100 | 0 |  |  |
| Total \% | 3.3 | 45.9 | 0 | 0 | 49.2 | 9.8 | 0 | 3.3 | 0 | 13.1 | 9.8 | 14.8 | 6.6 | 0 | 31.1 | 0 | 0 | 6.6 | 0 | 6.6 |  |

LSC Transportation Consultants, Inc.
545 E Pikes Peak Ave, Suite 210 Colorado Springs, CO 80905

719-633-2868
File Name : Southmoor Dr - Araco Concrete Access AM
Site Code : 00000000
Start Date : 8/14/2019
Page No :2

|  | Southmoor Dr Southbound |  |  |  |  | Araco Access Westbound |  |  |  |  | Southmoor Dr Northbound |  |  |  |  | Araco Annex Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Through | Right | Peds | App. Total | Left | Through | Right | Peds | App. Total | Left | Through | Right | Peds | App. Total | Left | Through | Right | Peds | App. Total | Int. Total |
| Peak Hour Analysis From 06:30 AM to 08:15 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 07:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 07:00 AM | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 2 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 8 |
| 07:15 AM | 1 | 1 | 0 | 0 | 2 | 1 | 0 | 1 | 0 | 2 | 2 | 1 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 7 |
| 07:30 AM | 0 | 5 | 0 | 0 | 5 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 2 | 0 | 2 | 9 |
| 07:45 AM | 0 | 2 | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 2 | 0 | 4 | 1 | 0 | 5 | 0 | 0 | 1 | 0 | 1 | 10 |
| Total Volume | 1 | 11 | 0 | 0 | 12 | 4 | 0 | 1 | 0 | 5 | 5 | 5 | 4 | 0 | 14 | 0 | 0 | 3 | 0 | 3 | 34 |
| \% App. Total | 8.3 | 91.7 | 0 | 0 |  | 80 | 0 | 20 | 0 |  | 35.7 | 35.7 | 28.6 | 0 |  | 0 | 0 | 100 | 0 |  |  |
| PHF | . 250 | . 550 | . 000 | . 000 | . 600 | . 500 | . 000 | . 250 | . 000 | . 625 | . 417 | . 313 | . 500 | . 000 | . 700 | . 000 | . 000 | . 375 | . 000 | . 375 | . 850 |



LSC Transportation Consultants, Inc.
545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Southmoor Dr - Araco Concrete Access AM
Site Code : 00000000
Start Date : 8/14/2019
Page No : 3

|  | Southmoor Dr Southbound |  |  |  |  | Araco Access Westbound |  |  |  |  | Southmoor Dr Northbound |  |  |  |  | Araco Annex Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | rougn | Right | Peds | App. Total | Left | Throug | Right | Peds | Tota | Left | Trough | Right | Peds | App. Top | Left | Throug | Right | Peds | mp. Tot | int Tol |

Peak Hour Analysis From 06:30 AM to 08:15 AM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 06:45 AM |  |  |  |  | 07:15 AM |  |  |  |  | 07:00 AM |  |  |  |  | 07:00 AM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 1 | 6 | 0 | 0 | 7 | 1 | 0 | 1 | 0 | 2 | 3 | 0 | 2 | 0 | 5 | 0 | 0 | 0 | 0 | 0 |
| +15 mins. | 0 | 3 | 0 | 0 | 3 | 1 | 0 | 0 | 0 | 1 | 2 | 1 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 |
| +30 mins. | 1 | 1 | 0 | 0 | 2 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 1 | 0 | 1 | 0 | 0 | 2 | 0 | 2 |
| +45 mins. | 0 | 5 | 0 | 0 | 5 | 1 | 0 | 0 | 0 | 1 | 0 | 4 | 1 | 0 | 5 | 0 | 0 | 1 | 0 | 1 |
| Total Volume | 2 | 15 | 0 | 0 | 17 | 5 | 0 | 1 | 0 | 6 | 5 | 5 | 4 | 0 | 14 | 0 | 0 | 3 | 0 | 3 |
| \% App. Total | 11.8 | 88.2 | 0 | 0 |  | 83.3 | 0 | 16.7 | 0 |  | 35.7 | 35.7 | 28.6 | 0 |  | 0 | 0 | 100 | 0 |  |
| PHF | . 500 | . 625 | . 000 | . 000 | . 607 | . 625 | . 000 | . 250 | . 000 | . 750 | . 417 | . 313 | . 500 | . 000 | . 700 | . 000 | . 000 | . 375 | . 000 | . 375 |



LSC Transportation Consultants, Inc.
545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Southmoor Dr - Araco Concrete Access PM
Site Code : 00194560
Start Date : 8/15/2019
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LSC Transportation Consultants, Inc.
545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Southmoor Dr - Araco Concrete Access PM
Site Code : 00194560
Start Date : 8/15/2019
Page No : 2

|  | Southmoor Dr Southbound |  |  |  |  | Araco Access Westbound |  |  |  |  | Southmoor Dr Northbound |  |  |  |  | Araco Annex Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | ough | Right | Peds | App. Total | Left | Throug | Right | Peds | App. Total | Left | Throug | Right | Peds | App. Toal | Left | Troo | Right | Peds | App. Toal | Int. Total |
| Peak Hour Analysis From 04:00 PM to 05:30 PM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Peak Hour for Entire Intersection Begins at 04:45 PM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 04:45 PM | 0 | 5 | 2 | 0 | 7 | 0 | 0 | 1 | 0 | 1 | 0 | 3 | 0 | 0 | 3 | 1 | 1 | 0 | 0 | 2 | 13 |
| 05:00 PM | 1 | 2 | 0 | 0 | 3 | 0 | 0 | 2 | 0 | 2 | 0 | 4 | 0 | 0 | 4 | 1 | 0 | 0 | 0 | 1 | 10 |
| 05:15 PM | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 1 | 0 | 1 | 2 | 2 | 1 | 0 | 5 | 0 | 1 | 2 | 0 | 3 | 15 |
| 05:30 PM | 1 | 3 | 0 | 0 | 4 | 3 | 0 | 0 | 0 | 3 | 1 | 6 | 3 | 0 | 10 | 1 | 0 | 0 | 0 | 1 | 18 |
| Total Volume | 2 | 16 | 2 | 0 | 20 | 3 | 0 | 4 | 0 | 7 | 3 | 15 | 4 | 0 | 22 |  | 2 | 2 | 0 | 7 | 56 |
| \% App. Total | 10 | 80 | 10 | 0 |  | 42.9 | 0 | 57.1 | 0 |  | 13.6 | 68.2 | 18.2 | 0 |  | 42.9 | 28.6 | 28.6 | 0 |  |  |
| PHF | . 500 | . 667 | . 250 | . 000 | . 714 | . 250 | . 000 | . 500 | . 000 | . 583 | . 375 | . 625 | . 333 | . 000 | 550 | . 750 | . 500 | . 250 | . 000 | . 583 | 778 |



LSC Transportation Consultants, Inc.
545 E Pikes Peak Ave, Suite 210
Colorado Springs, CO 80905
719-633-2868
File Name : Southmoor Dr - Araco Concrete Access PM
Site Code : 00194560
Start Date : 8/15/2019
Page No : 3

|  | Southmoor Dr Southbound |  |  |  |  | Araco Access Westbound |  |  |  |  | Southmoor Dr Northbound |  |  |  |  | Araco Annex Eastbound |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Left | Throug | Right | Peds | App. Total | Left | mough | Right | Peds | App. Total | Left | Trough | Right | Peds | App. To | Left | Troug | Right | Peds | App. Toala | int. Tota |

Peak Hour Analysis From 04:00 PM to 05:30 PM - Peak 1 of 1
Peak Hour for Each Approach Begins at:

|  | 04:30 PM |  |  |  |  | 04:15 PM |  |  |  |  | 04:45 PM |  |  |  |  | 04:15 PM |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| +0 mins. | 0 | 4 | 0 | 0 | 4 | 1 | 0 | 3 | 0 | 4 | 0 | 3 | 0 | 0 | 3 | 1 | 0 | 2 | 0 | 3 |
| +15 mins. | 0 | 5 | 2 | 0 | 7 | 1 | 0 | 0 | 0 | 1 | 0 | 4 | 0 | 0 | 4 | 0 | 0 | 1 | 0 | 1 |
| +30 mins. | 1 | 2 | 0 | 0 | 3 | 0 | 0 | 1 | 0 | 1 | 2 | 2 | 1 | 0 | 5 | 1 | 1 | 0 | 0 | 2 |
| +45 mins. | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 2 | 0 | 2 | 1 | 6 | 3 | 0 | 10 | 1 | 0 | 0 | 0 | 1 |
| Total Volume | 1 | 17 | 2 | 0 | 20 | 2 | 0 | 6 | 0 | 8 | 3 | 15 | 4 | 0 | 22 | 3 | 1 | 3 | 0 | 7 |
| \% App. Total | 5 | 85 | 10 | 0 |  | 25 | 0 | 75 | 0 |  | 13.6 | 68.2 | 18.2 | 0 |  | 42.9 | 14.3 | 42.9 | 0 |  |
| PHF | . 250 | . 708 | . 250 | . 000 | . 714 | . 500 | . 000 | . 500 | . 000 | . 500 | . 375 | . 625 | . 333 | . 000 | . 550 | . 750 | . 250 | . 375 | . 000 | . 583 |



| Intersection |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.9 |  |  |  |  |  |
| Movement | SET | SER | NWL | NWT | NEL | NER |
| Lane Configurations | $\uparrow$ |  |  | - | rin |  |
| Traffic Vol, veh/h | 15 | 0 | 5 | 9 | 0 | 3 |
| Future Vol, veh/h | 15 | 0 | 5 | 9 | 0 | 3 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 19 | 0 | 6 | 12 | 0 | 4 |


| Major/Minor M | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 19 | 0 | 43 | 19 |
| Stage 1 | - | - | - | - | 19 | - |
| Stage 2 | - | - | - | - | 24 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1597 | - | 968 | 1059 |
| Stage 1 | - | - | - | - | 1004 | - |
| Stage 2 | - | - | - | - | 999 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1597 | - | 964 | 1059 |
| Mov Cap-2 Maneuver | - | - | - | - | 964 | - |
| Stage 1 | - | - | - | - | 1004 | - |
| Stage 2 | - | - | - | - | 995 | - |
|  |  |  |  |  |  |  |
| Approach | SE |  | NW |  | NE |  |
| HCM Control Delay, s | 0 |  | 2.6 |  | 8.4 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NELn1 | NWL | NWT | SET | SER |
| Capacity (veh/h) |  | 1059 | 1597 | - | - | - |
| HCM Lane V/C Ratio |  | 0.004 | 0.004 | - | - | - |
| HCM Control Delay (s) |  | 8.4 | 7.3 | 0 | - | - |
| HCM Lane LOS |  | A | A | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0 | 0 | - | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.4 |  |  |  |  |  |
| Movement | SET | SER | NWL | NWT | NEL | NER |
| Lane Configurations | $\uparrow$ |  |  | - | rin |  |
| Traffic Vol, veh/h | 19 | 2 | 3 | 19 | 3 | 2 |
| Future Vol, veh/h | 19 | 2 | 3 | 19 | 3 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 24 | 3 | 4 | 24 | 4 | 3 |


| Major/Minor M | Major1 |  | Major2 |  | inor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 27 | 0 | 58 | 26 |
| Stage 1 | - |  | - | - | 26 | - |
| Stage 2 | - | - | - | - | 32 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1587 | - | 949 | 1050 |
| Stage 1 | - | - | - | - | 997 | - |
| Stage 2 | - | - | - | - | 991 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1587 | - | 946 | 1050 |
| Mov Cap-2 Maneuver | - | - | - | - | 946 | - |
| Stage 1 | - | - | - | - | 997 | - |
| Stage 2 | - | - | - | - | 988 | - |
|  |  |  |  |  |  |  |
| Approach | SE |  | NW |  | NE |  |
| HCM Control Delay, s | 0 |  | 1 |  | 8.7 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NELn1 | NWL | NWT | SET | SER |
| Capacity (veh/h) |  | 985 | 1587 | - | - | - |
| HCM Lane V/C Ratio |  | 0.007 | 0.002 | - | - | - |
| HCM Control Delay (s) |  | 8.7 | 7.3 | 0 | - | - |
| HCM Lane LOS |  | A | A | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0 | 0 | - | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.5 |  |  |  |  |  |
| Movement | SEL | SET | NWT | NWR | SWL | SWR |
| Lane Configurations |  | -1 | F |  | Mr |  |
| Traffic Vol, veh/h | 2 | 18 | 18 | 4 | 3 | 4 |
| Future Vol, veh/h | 2 | 18 | 18 | 4 | 3 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 3 | 23 | 23 | 5 | 4 | 5 |


| Major/Minor M | Major1 | Major2 |  | Minor2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 28 | 0 |  | 0 | 55 | 26 |
| Stage 1 | - | - |  | - | 26 | - |
| Stage 2 | - | - |  | - | 29 | - |
| Critical Hdwy | 4.12 | - |  | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - |  | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - |  | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - |  | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1585 | - |  | - | 953 | 1050 |
| Stage 1 | - | - |  | - | 997 | - |
| Stage 2 | - | - |  | - | 994 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1585 | - |  | - | 951 | 1050 |
| Mov Cap-2 Maneuver | - | - |  | - | 951 | - |
| Stage 1 | - | - | - | - | 995 | - |
| Stage 2 | - | - | - | - | 994 | - |
|  |  |  |  |  |  |  |
| Approach | SE |  |  |  | SW |  |
| HCM Control Delay, s | 0.7 |  | O |  | 8.6 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  |  |  | SEL | SETS | WLn1 |
| Capacity (veh/h) |  | - |  | 1585 |  | 1005 |
| HCM Lane V/C Ratio |  | - |  | 0.002 | - | 0.009 |
| HCM Control Delay (s) |  | - | - | 7.3 | 0 | 8.6 |
| HCM Lane LOS |  | - | - | A | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0 |  | 0 |


| Intersection |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.1 |  |  |  |  |  |  |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.3 |  |  |  |  |  |
| Movement | SET | SER | NWL | NWT | NEL | NER |
| Lane Configurations | F |  |  | $\uparrow$ | M |  |
| Traffic Vol, veh/h | 17 | 1 | 14 | 11 | 1 | 7 |
| Future Vol, veh/h | 17 | 1 | 14 | 11 | 1 | 7 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 22 | 1 | 18 | 14 | 1 | 9 |


| Major/Minor M | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 23 | 0 | 73 | 23 |
| Stage 1 | - |  | - | - | 23 | - |
| Stage 2 | - | - | - | - | 50 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1592 | - | 931 | 1054 |
| Stage 1 | - | - | - | - | 1000 | - |
| Stage 2 | - | - | - | - | 972 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1592 | - | 921 | 1054 |
| Mov Cap-2 Maneuver | - | - | - | - | 921 | - |
| Stage 1 | - | - | - | - | 1000 | - |
| Stage 2 | - | - | - | - | 961 | - |
|  |  |  |  |  |  |  |
| Approach | SE |  | NW |  | NE |  |
| HCM Control Delay, s | 0 |  | 4.1 |  | 8.5 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NELn1 | NWL | NWT | SET | SER |
| Capacity (veh/h) |  | 1035 | 1592 | - | - | - |
| HCM Lane V/C Ratio |  | 0.01 | 0.011 | - | - | - |
| HCM Control Delay (s) |  | 8.5 | 7.3 | 0 | - | - |
| HCM Lane LOS |  | A | A | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0 | 0 | - | - | - |





| Major/Minor M | Major1 |  | Major2 |  |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 1148 | 1165 | 0 | 1686 | 574 |
| Stage 1 | - | - | - | - | - | 1148 | - |
| Stage 2 | - | - | - | - | - | 538 | - |
| Critical Hdwy | - | - | 6.44 | 4.14 | - | 6.84 | 6.94 |
| Critical Hdwy Stg 1 | - | - | - | - | - | 5.84 | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | 5.84 | - |
| Follow-up Hdwy | - | - | 2.52 | 2.22 | - | 3.52 | 3.32 |
| Pot Cap-1 Maneuver | - | - | 267 | 595 | - | 85 | 462 |
| Stage 1 | - | - | - | - | - | 264 | - |
| Stage 2 | - | - | - | - | - | 549 | - |
| Platoon blocked, \% | - | - |  |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 571 | 571 | - | 79 | 462 |
| Mov Cap-2 Maneuver | - | - | - | - | - | 79 | - |
| Stage 1 | - | - | - | - | - | 264 | - |
| Stage 2 | - | - | - | - | - | 512 | - |
|  |  |  |  |  |  |  |  |
| Approach | SE |  | NW |  |  | NE |  |
| HCM Control Delay, s | 0 |  | 0.5 |  |  | 20.7 |  |
| HCM LOS |  |  |  |  |  | C |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NELn1 NELn2 |  | NWL | NWT | SET | SER |
| Capacity (veh/h) |  | 79 | 462 | 571 | - | - | - |
| HCM Lane V/C Ratio |  | 0.122 | 0.107 | 0.066 | - | - | - |
| HCM Control Delay (s) |  | 56.8 | 13.7 | 11.7 | - | - | - |
| HCM Lane LOS |  | F | B | B | - | - | - |
| HCM 95th \%tile Q(veh) |  | 0.4 | 0.4 | 0.2 | - | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Major1 | Major2 Minor1 |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 31 | 0 | 76 | 30 |  |
| Stage 1 | - | - | - | - | 30 | - |  |
| Stage 2 | - | - | - | - | 46 | - |  |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | - | - | 1582 | - | 927 | 1044 |  |
| Stage 1 | - | - | - |  | 993 | - |  |
| Stage 2 | - | - | - |  | 976 | - |  |
| Platoon blocked, \% | - | - |  | - |  |  |  |
| Mov Cap-1 Maneuver | - | - | 1582 | - | 921 | 1044 |  |
| Mov Cap-2 Maneuver | - | - | - |  | 921 | - |  |
| Stage 1 | - | - | - | - | 993 | - |  |
| Stage 2 | - | - | - | - | 970 | - |  |
|  |  |  |  |  |  |  |  |
| Approach | SE |  | NW |  | NE |  |  |
| HCM Control Delay, s | 0 |  | 2.1 |  | 8.6 |  |  |
| HCM LOS |  |  |  |  | A |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NELn1 | NWL | NWT | SET | SER |  |
| Capacity (veh/h) |  | 1010 | 1582 | - | - | - |  |
| HCM Lane V/C Ratio |  | 0.02 | 0.006 | - | - | - |  |
| HCM Control Delay (s) |  | 8.6 | 7.3 | 0 | - | - |  |
| HCM Lane LOS |  | A | A | A | - | - |  |
| HCM 95th \%tile Q(veh) |  | 0.1 | 0 | - | - | - |  |






| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.2 |  |  |  |  |  |
| Movement | SEL | SET | NWT | NWR | SWL | SWR |
| Lane Configurations |  | $\uparrow$ | A |  |  |  |
| Traffic Vol, veh/h | 1 | 25 | 25 | 10 | 4 | 5 |
| Future Vol, veh/h | 1 | 25 | 25 | 10 | 4 | 5 |
| Conflicting Peds, \#hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RTChannelized | - | None | - None | - | None |  |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 32 | 32 | 13 | 5 | 6 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.3 |  |  |  |  |  |
| Movement | SET | SER | NWL | NWT | NEL | NER |
| Lane Configurations | 个4 | $\mathbf{7}$ | 2 | 个4 | T | $\mathbf{7}$ |
| Traffic Vol, veh/h | 1350 | 50 | 50 | 1075 | 25 | 50 |
| Future Vol, veh/h | 1350 | 50 | 50 | 1075 | 25 | 50 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - Signal |  |
| Storage Length | - | 0 | 250 | - | 100 | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 93 | 93 | 93 | 93 | 83 | 83 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1452 | 54 | 54 | 1156 | 30 | 60 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.8 |  |  |  |  |  |
| Movement | SET | SER | NWL | NWT | NEL | NER |
| Lane Configurations | $\uparrow$ |  |  | -1 | M |  |
| Traffic Vol, veh/h | 30 | 2 | 8 | 55 | 4 | 12 |
| Future Vol, veh/h | 30 | 2 | 8 | 55 | 4 | 12 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 78 | 78 | 83 | 83 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 38 | 3 | 10 | 66 | 5 | 15 |


| Major/Minor M | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 41 | 0 | 126 | 40 |
| Stage 1 | - |  | - | - | 40 | - |
| Stage 2 | - | - | - | - | 86 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 |  | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1568 | - | 869 | 1031 |
| Stage 1 | - | - | - | - | 982 | - |
| Stage 2 | - | - | - | - | 937 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1568 | - | 863 | 1031 |
| Mov Cap-2 Maneuver | - | - | - | - | 863 | - |
| Stage 1 | - | - | - | - | 982 | - |
| Stage 2 | - | - | - | - | 930 | - |
|  |  |  |  |  |  |  |
| Approach | SE |  | NW |  | NE |  |
| HCM Control Delay, s | 0 |  | 0.9 |  | 8.7 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NELn1 | NWL | NWT | SET | SER |
| Capacity (veh/h) |  | 983 | 1568 | - | - | - |
| HCM Lane V/C Ratio |  | 0.021 | 0.006 | - | - | - |
| HCM Control Delay (s) |  | 8.7 | 7.3 | 0 | - | - |
| HCM Lane LOS |  | A | A | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0.1 | 0 | - | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.3 |  |  |  |  |  |
| Movement | SEL | SET | NWT | NWR | SWL | SWR |
| Lane Configurations |  | -1 | 1 |  | Y |  |
| Traffic Vol, veh/h | 2 | 25 | 50 | 10 | 3 | 10 |
| Future Vol, veh/h | 2 | 25 | 50 | 10 | 3 | 10 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 3 | 32 | 64 | 13 | 4 | 13 |


| Major/Minor M | Major1 |  |  |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 77 | 0 |  | 0 | 109 | 71 |
| Stage 1 | - | - |  | - | 71 | - |
| Stage 2 | - | - |  | - | 38 | - |
| Critical Hdwy | 4.12 | - |  | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - |  | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - |  | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - |  | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1522 | - |  | - | 888 | 991 |
| Stage 1 | - | - |  | - | 952 | - |
| Stage 2 | - | - |  | - | 984 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1522 | - |  | - | 886 | 991 |
| Mov Cap-2 Maneuver | - | - |  | - | 886 | - |
| Stage 1 | - | - | - | - | 950 | - |
| Stage 2 | - | - | - | - | 984 | - |
|  |  |  |  |  |  |  |
| Approach | SE |  |  |  | SW |  |
| HCM Control Delay, s | 0.5 |  | O |  | 8.8 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NWT NWR |  | SEL | SETSWLn1 |  |
| Capacity (veh/h) |  | - |  | 1522 | - | 965 |
| HCM Lane V/C Ratio |  | - |  | 0.002 | - | 0.017 |
| HCM Control Delay (s) |  | - | - | 7.4 | 0 | 8.8 |
| HCM Lane LOS |  | - | - | A | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0 | - | 0.1 |


| Intersection |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | :--- |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 4 |  |  |  |  |  |
| Movement | SET | SER | NWL | NWT | NEL | NER |
| Lane Configurations | 个 | $\mathbf{T}$ | 2 | 个 | T | $\mathbf{7}$ |
| Traffic Vol, veh/h | 1350 | 51 | 55 | 1075 | 27 | 61 |
| Future Vol, veh/h | 1350 | 51 | 55 | 1075 | 27 | 61 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - Signal |  |
| Storage Length | - | 0 | 250 | - | 100 | 0 |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 93 | 93 | 93 | 93 | 83 | 83 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1452 | 55 | 59 | 1156 | 33 | 73 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.8 |  |  |  |  |  |
| Movement | SET | SER | NWL | NWT | NEL | NER |
| Lane Configurations | $\uparrow$ |  |  | -1 | M |  |
| Traffic Vol, veh/h | 33 | 2 | 8 | 56 | 4 | 12 |
| Future Vol, veh/h | 33 | 2 | 8 | 56 | 4 | 12 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 78 | 78 | 83 | 83 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 42 | 3 | 10 | 67 | 5 | 15 |


| Major/Minor M | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 45 | 0 | 131 | 44 |
| Stage 1 | - | - | - | - | 44 | - |
| Stage 2 | - | - | - | - | 87 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1563 | - | 863 | 1026 |
| Stage 1 | - | - | - | - | 978 | - |
| Stage 2 | - | - | - | - | 936 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1563 | - | 857 | 1026 |
| Mov Cap-2 Maneuver | - | - | - | - | 857 | - |
| Stage 1 | - | - | - | - | 978 | - |
| Stage 2 | - | - | - | - | 929 | - |
|  |  |  |  |  |  |  |
| Approach | SE |  | NW |  | NE |  |
| HCM Control Delay, s | 0 |  | 0.9 |  | 8.8 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NELn1 | NWL | NWT | SET | SER |
| Capacity (veh/h) |  | 978 | 1563 | - | - | - |
| HCM Lane V/C Ratio |  | 0.021 | 0.006 | - | - | - |
| HCM Control Delay (s) |  | 8.8 | 7.3 | 0 | - | - |
| HCM Lane LOS |  | A | A | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0.1 | 0 | - | - | - |




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.2 |  |  |  |  |  |
| Movement | SEL | SET | NWT | NWR | SWL | SWR |
| Lane Configurations |  | -1 | 1 |  | Y |  |
| Traffic Vol, veh/h | 2 | 28 | 52 | 10 | 3 | 10 |
| Future Vol, veh/h | 2 | 28 | 52 | 10 | 3 | 10 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 78 | 78 | 78 | 78 | 78 | 78 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 3 | 36 | 67 | 13 | 4 | 13 |



## AutoTurn Exhibits






Site Plan


## Appendix A - Trip Generation Study for RV Storage

## APPENDIX A - TRIP GENERATION STUDY FOR RV STORAGE

For this report "RV/Vehicle Storage" rates (shown in Table 2) are based on the results of a trip generation study consisting of trip generation data collection by LSC at several RV storage facilities in El Paso County (2018). These counts were conducted specifically to estimate a tripgeneration rate for this land use, as ITE's Trip Generation does not include trip-generation rates specifically for RV/boat storage businesses. These rates have been used within TIS reports for other RV storage projects in El Paso County within the past couple of years. The following list contains dates and location data for these sample RV storage facility counts in El Paso County. Raw count data is attached:

- Dalby Drive, LLC RV Storage - July 20, 2018
- 6850 Dalby Drive, Colorado Springs, CO 80923
- All About Outdoor Storage - July 24-25, 2018
- 16140 Old Denver Road, Monument, CO 80312
- All Outside Storage - July 23, 2018
- 835 N Washington Street, Monument, CO 80132
- Falcon Meadow Campground (2 site accesses) - July 2018
- 11150 US 24, Peyton, CO 80831

