BUILDER/CONTRACTOR RESPONSIBILITIES

It is the responsibility of the BUILDER/CONTRACTOR to ensure that all project plans and specifications comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that the building nanufacturer or its design engineer is acting as the engineer of record or design professional for a construction project.

The contractor must secure all required approvals and permits from the appropriate agency as required.

Approval of drawings and calculations indicate that the building manufacturer correctly interpreted and applied the requirements of the contract drawings and specifications (Sect. 4.2.1 AISC code of standard practices, 9th. edition.)

Where discrepancies exist between the manufacturer's structural steel plans and the plans for other trades, the structural steel plans shall govern (Sect. 3.3 AISC code of standard practices 9th, ed).

Design considerations of any materials in the structure which are not furnished by the building manufacturer/supplier are the responsibility of the contractors and engineers other than the building manufacturer/supplier's engineer unless specifically indicated.

The contractor is responsible for all erection of steel and associated work in compliance with the building manufacturer's 'for construction' drawings.

Products shipped to builder or his customer shall be inspected immediately upon arrival. Claims for shortages or defective material if not packaged must be mailed to Steel Building MFG in writing within five (3) days after the builder learns of the defect. The manufacturer/ Steel Building MFG shall not be liable for any defect unless claim is made within one (1) year after date of the original shipment by the manufacturer to the builder or his customer. Steel Building MFG will be given a reasonable opportunity to inspect defective materials upon receipt of claim by builder.

If a defect is of such nature that it can be remedied by a field operation at the job site without the necessity of returning the material to the manufacturer, then upon written authorization of the manufacturer the builder may repair or cause the material to be repaired and the manufacturer will reimburse the builder for the cost of the repair in accordance with the written authorization.

All bracing as shown and provided by the manufacturer for this building is required and shall be installed by the erector as a permanent part of the structure.

Temporary supports, such as temporary guys, braces, false work, cribbing or other elements required for the erection operation will be determined and furnished and installed by the erector. These temporary supports will secure the steel framing or any partly assembled steel framing against loads comparable in intensity to those for which the structure was designed, resulting from wind, seismic forces and erection operations, but not the loads resulting from the performance of work by or the acts of others, nor such unpredictable loads as those due to tornado, explosion or collision (Sect. 7.9.1 AISC code of standard practices, 9th. edition.)

Design of gutter and downspout is a function of the rainfall intensity and area to be drained. Design parameters utilized are in accordance with the 1986 low rise building system manual and/or the 9th edition of the architectural graphic standards, as applicable. Proper owner maintenance dictates that the drainage system be kept free and clear of debris and/or ice at all times to ensure proper function of the gutter and downspout. In those cases where the owner/tenant of a property is unwilling or unable to provide proper maintenance, elimination of gutter should be considered as an alternative.

APPROVAL NOTES

The following conditions may apply in the event that these drawings are used as approval drawings:

A) It is imperative that any changes to these drawings:

- 1) Be made in contrasting ink and surrounded by 'clouding'
- 2) Have all instances of change clearly indicated.
 3) Be legible and unambiguous.
- B). Dated signature is required on all pages.
- C). Steel Building MFG reserves the right to resubmit drawings with extensive or complex changes required to avoid misfabrication. This may impact the delivery schedule.

D). Any changes noted on the drawings not in conformance with the terms and requirements of the contract between Steel Building MFG and its customer are not binding on Steel Building MFG unless subsequently specifically acknowledged and agreed to in writing by change order or separate documentation. Steel Building MFG recognizes that rubber stamps are routinely used for indicating approval, disapproval, rejection or mere review of the drawings submitted. However, Steel Building MFG does not accept changes or additions to contractual terms and conditions that may appear with use of a stamp or similar indication of approval, disapproval, etc.. Such language applied to the drawings of Steel Building MFG by the customer, architect, engineer, or any other party will be considered as unacceptable alterations to these drawing notes, and will not alter the contractual rights and obligations existing between Steel Building MFG and its customer.

Drawing Status:

| - | For Approval these drawings, being for approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued 'for construction' can be considered complete. | |
|---|---|--|
| | For Perniti These drawings, being for permit, are by definition not final in that, as a minimum, piece markings are not identified. Only drawings issued 'for construction' can be considered as complete. For Construction Final Drawings. | |

GENERAL NOTES <> Steel Building MFG

The structure under this contract has been designed and detailed for the loads and conditions stipulated in the contract and shown on these drawings. Any alterations to the structural system or removal of any component parts, or the addition of other construction naterials or loads must be done under the advice and direction of a registered architect, civil or structural engineer. The building manufacturer will assume no responsibility for any loads not indicated.

This metal building is designed with the building manufacturer's standard practices which are based on pertinent procedures and recommendations of the following organizations and codes.

- 1. American Institute of Steel Construction 'Specification for the Design, Fabrication and Erection of Structural Steel for Buildings,
- 2. American Iron and Steel Institute: 'Specification for the Design of Cold Formed Steel Structural Members'.
- 3. American Welding Society: 'Structural Welding Code' AWS DI.1.
- 4. Metal Building Manufacturer's Association 'Low Rise Building Systems Manual'
- 5. International Conference of Building Officials: 'Uniforn Building Code
- 6. Southern Building Code Congress International 'Standard Building Code'
- 7. Building Official and Code Administrators International: 'BOCA National Building Code'
- 8. National Building Code of Canada.
- 9. International Building Code

Material properties of steel plate used in the fabrication of primary rigid frames, and other primary structural exclusive of cold formed sections, conform to ASTM-AS29 or A-572. Flanges with thickness of 1' or less and width of 12' or less conform to A-529 with a minimum yield point of 55,000 PSI. Web material conforms to ASTM-A36 modified with a minimum yield point of 46,000 PSI.

Material properties of pipe sections conform to ASTM-ASS type E, grade B with a minimum yield point of 35,000 PSI.

Material properties of hot rolled steel members conform to the requirements of ASTM-A36 or A572 with a minimum yield point of 50,000 PSI.

Material properties of cold formed light gage steel members conform to ASTM-AS70 or A607 grade 55 modified with a minimum yield point of 57,000 PSI.

Material properties of roof/wall sheeting, base metal conform to ASTM-A792 grades D or E with minimum yield points of 50,000 PSI and 80,000 PSI respectively, as required by design. Coating of base material is 55% aluminum-zinc alloy in accordance with AZ55 specifications.

Cable utilized for bracing conforms to ASTM A475. Cable bracing is to be installed to a taut condition with all slack removed.

Rod and angle utilized for bracing members conform to ASTM A36.

Structural joints with A.S.T.M. A-325 high strength bolts, where indicated on the drawings, shall be assembled and the fasteners tightened in accordance with 'turn of nut' method as described in the specification for structural joints using A.S.T.M. A-325 or A-490 bolts (11-18-35), unless otherwise noted. All joints will be assembled without washers unless otherwise noted.

All steel members except bolts, fasteners and cable shall receive one shop coat of iron oxide corrosion inhibitive primer, meeting the performance requirements of TIP-636.

Shop and field inspections and associated fees are the responsibility of the contractor, unless stipulated otherwise in the contract.

The metal building manufacturers will identify primary structural steel with a minimum yield point greater than 36,000 PSI by means of a sticker near the erection mark on each shipped piece. Secondary members with a yield point equal to or greater than 33,000 PSI shall be identified by means of a sticker near the erection mark on each shipped piece. (This is in accordance with the 1997 UBC section 2203, subsection 22032 and 2203.3.)

SAFETY⇔Steel Building MFG

The building manufacturer/supplier has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of the building manufacturer.

The supplier strongly recommends that safe working conditions and accident prevention practices be the top priority of any job site.

Local, state and federal safety and health standards (www.osha.gov) should always be followed to help insure worker safety.

Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees.

Daily meetings highlighting safety procedures are also recommended.

The use of hard hats, rubber sole shoes for roof work, proper equipment for had and safety nets where applicable are recommended.



PRIMER

Shop primer paint is a rust inhibitive primer which meets or exceeds the end performance of federal specifications TT-P-636c and TT-P-644 and is a red oxide primer. This primer is not intended for long term exposure to the elements. Steel Building MFG is not responsible for any deterioration of the shop primer as a result of improper handling and/or storage. Steel Building MFG shall not be responsible for any field-applied paint and/or coatings. (Section 6.5 AISC Code of Standard Practice, 9th ED.)

A325 BOLT TIGHTENING REQUIREMENTS

All high strength bolts are A325-N unless specifically noted otherwise. Structural bolts shall be tightened by the turn of nut method in accordance with the 9th edition of the 'AISC Steel Construction Manual'. A325 bolts are supplied without washers unless noted otherwise. All bolted connections are designed as bearing type connections with the bolt threads included in the shear plane.

ERECTION NOTE: (ERECTION AND UNLOADING NOT BY Steel Building MFG)

All bracing shown and provided by Steel Building MFG for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.

SHORTAGES (SEE MBMA 5.2.1)

The quantity of crates and structural items shipped shall be checked and any shortages or other discrepancies with respect thereto, shall be reported to Steel Building MFG on the day of delivery and such discrepancy confirmed in writing within seven (7) days. With respect to items or quantities within unopened crates and any latent defects, it shall be the duty of the purchaser to notify Steel Building MFG on the date such defect or shortage is discovered and confirm such notice in writing to Steel Building MFG within (7) days thereof.

CORRECTIONS OF ERRORS AND REPAIRS (SEE MBMA 6.10)

Claims for correction of alleged misfits will be disallowed unless
Steel Building MFG shall have received prior notice thereof and allowed
reasonable inspection of such misfits. The correction of minor misfits by use
of drift pins to draw the components into line, moderate amounts of reaming,
chipping and cutting, and the replacement of minor shortages of material
are a normal part of erection and are not subject to claim, no part of the
building may be returned for alleged misfits without the prior approval of
Steel Building MFG.

FIELD WORK SUMMARY

 Valk doors and windows will need to be 'cut in.' Check for bracing on the plan before locations are determined.

2. For a pitched roof, the top of the panels on the rake side will need to be clipped in order for the rake trim to fit.

Slots will need to be cut in the appropriate girts to accommodate the cables or rods
where such bracing will be used.

PRILIFCT

| ÷ | | REAL PARTIES AND A CONTROL OF THE PROPERTY OF | | $\overline{\Box}$ |
|---|----------|---|-----|-------------------|
| | CUSTOMER | Don Soils REPORT AND FOUNDATION DES | 310 | GÑ |
| | PROJECT | Don S BY LICENSED COLORADO ENGINEER ARCHITECT SHALL BE ON HAND | C | R |
| | JOB ID | DBS073366 AT TIME OF FIRST INSPECTION | | \leq |
| | | 203 Beacons Light Monument, CO 80132 | | ENGINE |
| | SIZE | 50'-0" × 100'-0" × 16'-0" | | |

Steel Building MFG

303-358-5808

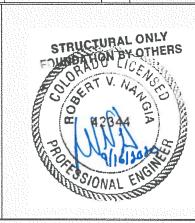
DRAWINGS COVER SHEET (C1) NBS

SCALE: NONE DATE: 9/14/20 REV NO.

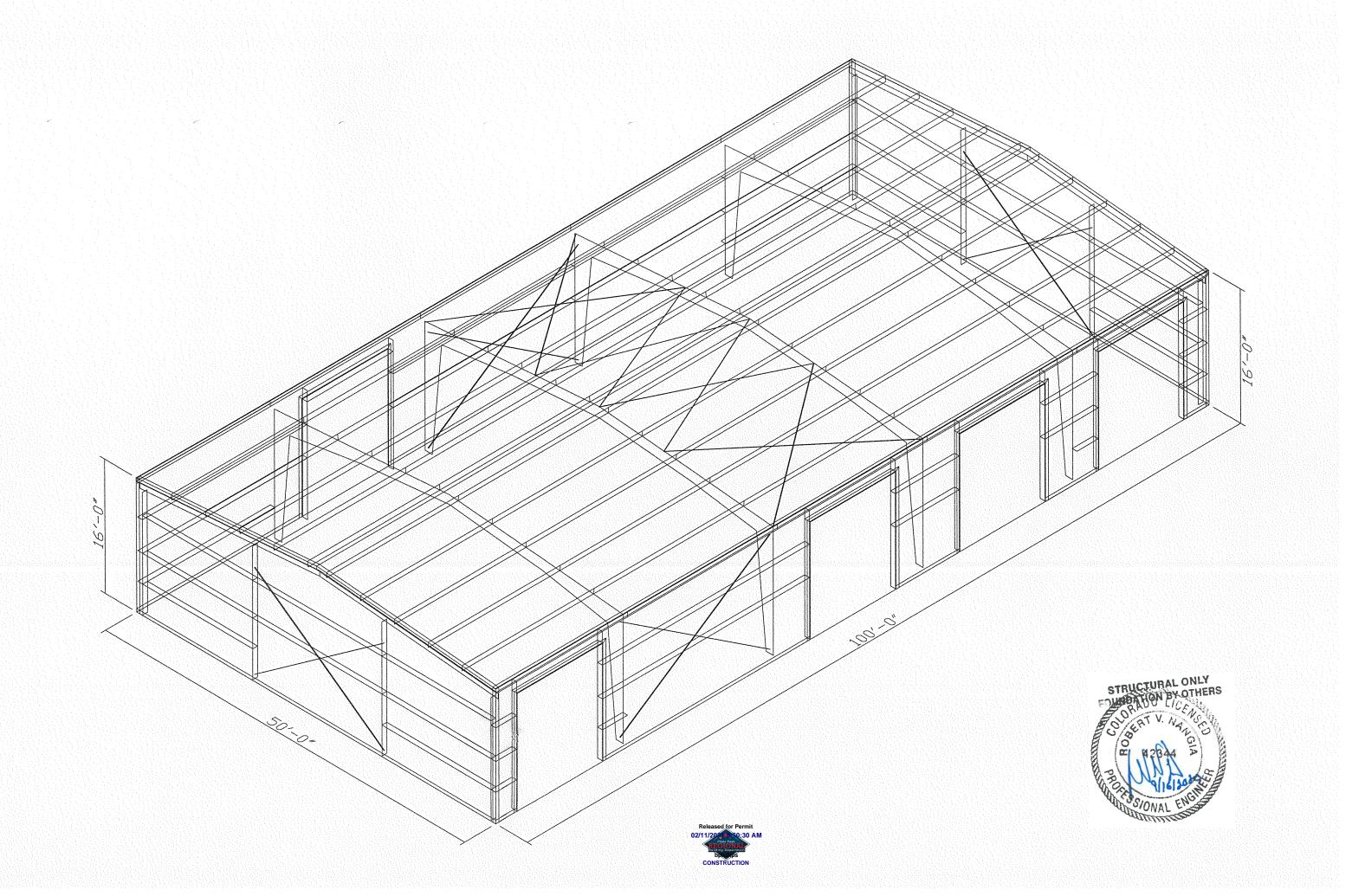
| WIDTH (ft) | 50 |
|----------------------------|------------------------|
| LENGTH (Ft) | 100 |
| EAVE HEIGHT (ft) | 16 |
| ROOF SLOPE (rise/12°) | 1.012 |
| SIDEWALL BAY SPACING | 1 at 18.5 |
| LEFT ENDWALL BAY SP | 1 at 16 |
| RIGHT ENDWALL BAY SP | 1 at 16 |
| ROOF FRAMING 5 at | 4.7507 along surface 2 |
| ROOF PANEL TYPE | Classic Roof (PBR) |
| ROOF PANEL GAUGE | 26 GA |
| INTERIOR FRAMING 1 rig | id frame, clear span |
| WALL PANEL TYPE | Classic Wall (PBR) |
| WALL PANEL GAUGE | 26 GA |
| DEAD LOAD (psf) | 2.000 |
| LIVE LOAD (psf) | 20.00 |
| REDUCTION ALLOWED | NO |
| ROOF SNOW LOAD(psf) | 40 |
| COLLATERAL LOAD (psf) | 0.5 |
| GOVERNING CODE | IBC 15 |
| WIND SPEED (mph) | 130 |
| CLOSURE | Closed |
| VIND EXPOSURE | |
| VIND IMPORTANCE | 1,00 |
| WALL GIRT, MAX. DEFL. L/s | |
| | span 180 |
| | l/span 60 |
| SEISMIC ZONE/DESIGN CATEGO | |
| SEISMIC COEFF. | 0.298 |
| SEISMIC IMPORTANCE | 1.00 |
| SEISMIC USE GROUP | |
| SEISMIC Sds | CO CONTRACTOR |
| SEISMIC 2dl | Monument |
| ANCHOR BOLTS | BY OTHERS |

RITE DING DATA

| PAGE | DESCRIPTION | PAGE | DESCRIPTION |
|------|----------------------------|----------|--|
| C1 | COVER SHEET | 13 | Roof Sheeting |
| C5 | 3D | 14 | Rigid Frame 1 |
| 1 | Anchor Bolt Plan | 15 | Rigid Frame 2 |
| 2 | Anchor Bolt Connections | 16 | Rigid Frame 3 |
| 3 | Anchor Bolt Reactions | 17 | Assembly Details 1 |
| 4 | Front Sidewall Framing | 18 | Assembly Details 2 |
| 5 | Front Sidewall Sheeting | 19 | Assembly Details 3 |
| 6 | Back Sidewall Framing | 20 | Assembly Details 4 |
| 7 | Back Sidewall Sheeting | 21 | Assembly Details 5 |
| 8 | Left Endwall Framing | 1 355 | |
| 9 | Left Endwall Sheeting | 9 9650 | 计划系统数量和数据 |
| 10 | Right Endwall Framing | N COUR | |
| 11 | Right Endwall Sheeting | | |
| 12 | Roof Framing | | |
| | I dalam di baga aya da kat | V 1801AG | U dan da |



ROBERT V. NANGIA P.E. 7423 HOLLOW RIDGE DR. HOUSTON, TX 77095



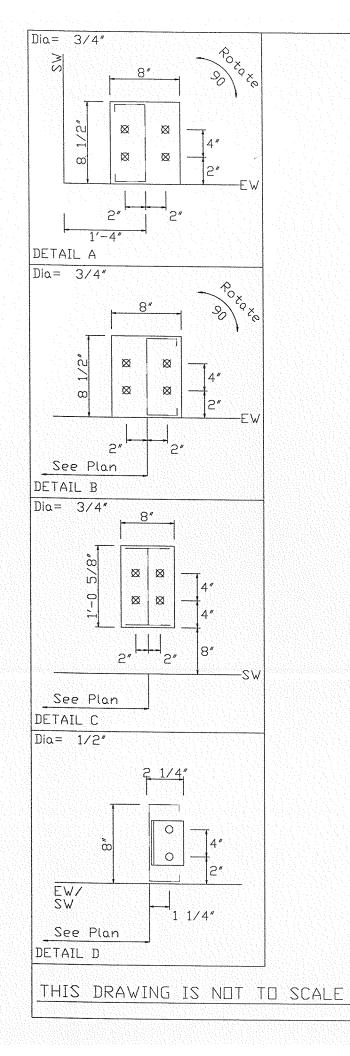
ANCHOR BOLT SUMMARY 100'-0" DUT-TD-DUT DF STEE 18'-6" 21'-0" 18'-6" C C C E S A A 50'-0' 0UT-TO-0UT OF STEEL (C) (B B Bes -(B) a s B Bles 2 E A D c A E M D 🔓 (I) STRUCTURAL ONLY 4'-6" 4'-6" (2) (3) ANCHOR BOLT PLAN NOTE: All Base Plates @ 100'-0" (U.N.) Don S Steel Building MFG o Dia= 1/2" PHDNE: 303-358-5808 PROJECT Don S THIS DRAWING IS NOT TO SCALE ⊠ Dia= 3/4" DBS073366 ANCHOR BOLT PLAN & REACTIONS PROJECT 203 Beacons L DESIGN: Designer NBS

ADDRESS Monument, CO ONTINUETION

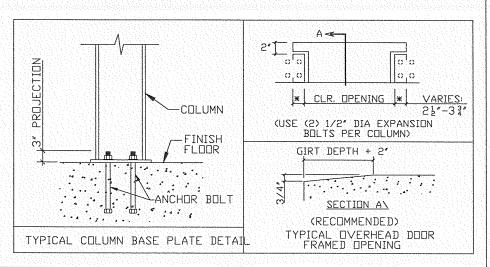
DATE: 9/14/20

SHEET 1

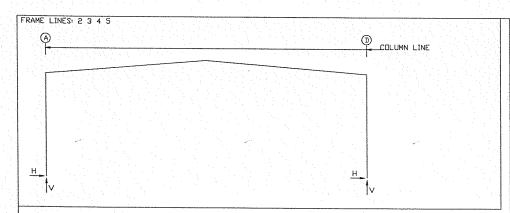
OF 21







| Don S | Steel Building MFG | |
|-----------------------------------|-----------------------------|--|
| | PHDNE: 303-358-5808 | |
| ID DBS073366 02/11/20 00:30 AM | ANCHOR BOLT DETAILS | |
| PROJECT 203 Beacons L | DESIGN: Designer NBS | |
| ADDRESS Monument, CO CONSTRUCTION | DATE: 9/14/20 SHEET 2 DF 21 | |



RIGID FRAME: MAXIMUM REACTIONS

| Frm Line | Col Line | Load | Jo3 X Hmax H | umn_Red V Vmax | | ns(k) - I Hmin H | V Vmin |
|-------------|-------------|------|--------------------|----------------------|--------|-------------------------|--------------|
| 2 | Α | 1 | 11.9 11.9 | 22.8 | 2 | -5.1 | -7.7 |
| 2 | D | 3 | 5.1 -11.9 | -7.7 22.8 | 1 5 | -11.9 1.2 | 22.8 -9.0 |

RIGID FRAME: MAXIMUM REACTIONS

| Frm | Col Line | Load | Hmax H | lumn_Rei V Vmax | | Hmin H | V Vmin |
|-----|-------------|--------|--------------|-----------------------|-----|--------------|--------------|
| 3* | Α | 1 | 12.3 | 24.2 | 2 | -5.1 -1.1 | -7.9 -9.2 |
| 3* | D | 3 1 | 5.1 -12.3 | -7.9 24.2 | 1 5 | -12.3 1.1 | 24.2 -9.2 |

RIGID FRAME: MAXIMUM REACTIONS

| Frm Col Line Line | Column_Reactions(k) Load Hmax V Load Hmin V Id H Vmax Id H Vmin |
|----------------------|--|
| 5 A 5 B | 1 11.9 22.8 2 -5.1 -7.7 1 11.9 22.8 |
| J | 3 5.1 -7.7 1 -11.9 22.8 1 -11.9 22.8 3 5.1 -7.7 |

NOTES FOR REACTIONS

- All loading conditions are examined and only maximum/minimum H or V and the corresponding H or V are reported.
 Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.

c. Positive reactions are as shown in the sketch. Foundation loads are in opposite directions.

3. Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.

4. Building reactions are based on the following building data:

Width (ft) = 50.0

Length (ft) = 100.0

Eave Height (ft) = 16.0 / 16.0

Roof Slope (rise/12) > 1.0 / 1.0

Dead Load (psf) = 2.0

Collateral Load (psf) = 0.5

Live Load (psf) = 20.0

Snow Load (psf) = 20.0

Snow Load (psf) = 130.0

Wind Speed (mph) = 130.0

Wind Code = 1BC 15

Exposure = C

Closed/Ipen = C

Importance Vind = 1.00

Importance Seismic = 1.00

Seismic Coeff (Fa*Ss) = 0.30

5. Loading conditions are

| RIGID FRA | ME: BASIC COL | UMN REACTIONS | (k) | | | |
|---|---|--|--|--|--|---|
| Line Line | Dead Horlz Vert 0.8 1.8 -0.8 1.8 | Horlz Vert 0.1 0.3 | Live Horiz Vert 5.5 10.4 -5.5 10.4 | Snow Horiz Vert 11.0 20.7 -11.0 20.7 | Wind_Left1- Horlz Vert -9.2 -14.5 2.1 -9.4 | -Wind_Right1- Horlz Vert -2.1 -9.4 9.2 -14.5 |
| Line Line | -7.9 -8.6 | Horiz Vert | Wind_Long1- Horlz Vert -2.8 -13.3 3.3 -14.2 | Wind_Long2- Horiz Vert -3.3 -10.7 2.8 -16.8 | -Seismic_Left Horiz Vert -0.5 -0.3 -0.5 0.3 | Seismic_Right Horlz Vert 0.5 0.3 0.5 -0.3 |
| Line Line 2 A 2 D | Horiz Vert 0.0 0.0 0.0 -1.5 | -MIN_SNOW Horiz Vert 5.5 10.4 -5.5 10.4 | F1UNB_SL_L- Horiz Vert 8.4 19.8 -8.4 10.9 | F1UNB_SL_R- Horiz Vert 8.4 10.9 -8.4 19.8 | | |
| 3* A | Bead Horiz Vert 0.8 1.9 -0.8 1.9 | Collateral- Horiz Vert 0.1 0.3 -0.1 0.3 | Horiz Vert 5.7 11.0 -5.7 11.0 | Horlz Vert | Wind_Left1- Horiz Vert -9.3 -15.0 2.0 -9.8 | -Wind_Right1- Horiz Vert -2.0 -9.8 9.3 -15.0 |
| Line Line 3* A | Horiz Vert | -Wind_Right2- Horiz Vert -0.8 -3.5 8.0 -8.7 | Wind_Long1- Horiz Vert -2.7 -17.2 3.2 -14.6 | Horiz Vert | Horiz Vert | Seismic_Right Horiz Vert 0.5 0.3 0.5 -0.3 |
| Line Line 3* A | -Seismic_Long Horiz Vert 0.0 -1.5 0.0 -1.5 | -MIN_SNOW Horiz Vert 5.7 11.0 -5.7 11.0 | F2UNB_SL_L- Horlz Vert 8.7 21.0 -8.7 11.6 | F2UNB_SL_R- Horiz Vert 8.7 11.6 -8.7 21.0 | | |
| Line Line | Dead Horiz Vert 0.8 1.8 -0.8 1.8 | Collateral- Horiz Vert 0.1 0.3 -0.1 0.3 | Horiz Vert 5.5 10.4 -5.5 10.4 | Snow Horiz Vert 11.0 20.7 -11.0 20.7 | Wind_Left1- Horiz Vert -9.2 -14.5 2.1 -9.4 | -Wind_Right1- Horiz Vert -2.1 -9.4 9.2 -14.5 |
| Line Line | Wind_Left2- Horiz Vert -7.9 -8.6 0.9 -3.5 | Horlz Vert | Wind_Long1- Horiz Vert -2.8 -13.3 3.3 -10.7 | Wind_Long2- Horiz Vert -3.3 -10.7 2.8 -13.3 | Horlz Vert | Seismic_Right Horiz Vert 0.5 0.3 0.5 -0.3 |
| Frame Column Line Line 5 A 5 D | Horiz Vert | F3UNB_SL_L- Horiz Vert 8.4 19.8 -8.4 10.9 | F3UNB_SL_R- Horiz Vert 8.4 10.9 -8.4 19.8 | | | |
| 3× Frame lin | nesi 34 | | | | | |

GENERAL NOTES

- 1. ALL LDADING CONDITIONS ARE EXAMINED AND ONLY MAXIMUM/MINIMUM H \square R \vee AND THE CORRESPONDING H \square R \vee ARE REPORTED.
- 2. POSITIVE REACTIONS ARE AS SHOWN IN THE SKETCH, FOUNDATION LOADS ARE IN OPPOSITE DIRECTIONS.
- 3. BRACING REACTIONS ARE IN THE PLANE OF THE BRACE WITH THE H POINTING AWAY FROM THE BRACED BAY. THE VERTICAL REACTION IS DOWNWARD.

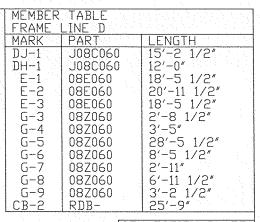
| 194 5 5 6 | | | | | | | | |
|----------------------|------------------|--------------------------|--------------------------|--------------------------|--------------------------|--------------------------|-----------------------|------|
| BUIL | DIN | G BR | ACIN | IG RI | EAC [*] | ΓΙΟΝ | S | |
| | all — Line | | Wi | nd — | -Sei | snic - | Panel (lb) Wind | (ft) |
| 8_SW 8_EW B_SW | 1 D 6 A | B,C 2,3 C,B 4,3 | 2.1 5.4 2.1 5.4 | 1.8 3.5 1.8 3.5 | 0.4 2.2 0.4 2.2 | 0.4 1.5 0.4 1.5 | | |

| | Col Line A B C D | Dead Vert 0.2 0.7 0.7 | Collat Vert 0.0 0.1 0.1 | Live Vert 1.1 3.8 3.8 1.1 | Sr Ve | now ert | -2.1 | eft1 | Wind_R Horz 0.0 0.0 2.1 0.0 | | Wind_L Horz 0.0 -2.1 0.0 0.0 | | Wind_R Horz 0.0 0.0 2.1 0.0 | light2 Vert -0.5 -0.4 -6.7 -1.3 | Wind Press Horz -1.7 -3.8 -3.8 -1.7 |
|----------------------------|---------------------------------|--|--|--|--|---|---|--|--|--|---|--|--|--|---|
| Frm Line 1 1 | | Wind Suct Horz 2.0 4,2 4,2 2.0 | Wind_L Horz 0.0 0.0 0.3 0.0 | ong1 Vert -2.2 -6.5 -4.6 -1.1 | Wind_ Horz 0.0 -0.3 0.0 0.0 | Long2 Vert -1.1 -4.6 -6.5 -2.2 | Seis Horz 0.0 -0.4 0.0 0.0 | _Left Vert 0.0 -0.4 0.4 0.0 | | Right Vert 0.0 0.4 -0.4 0.0 | | SNOW- Vert 1.1 3.8 3.8 1.1 | | | |
| Frm Line 1 1 1 | Col Line A B C D | E1UNB_: Horz 0.0 0.0 0.0 0.0 | SL_R- Vert 0.4 3.0 8.8 2.4 | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | V flooral |
| Frm Line 6 6 6 | | Dead Vert 0.2 0.7 0.7 0.2 | Collat Vert 0.0 0.1 0.1 0.0 | Live Ver 1.1 3.8 3.8 1.1 | t V | | | | Wind_R Horz 0.0 0.0 2.1 0.0 | Right1 Vert -1.2 -2.4 -8.7 -2.1 | Vind_L Horz 0.0 -2.1 0.0 | eft2 Vert -1.3 -6.7 -0.4 -0.5 | Wind_R Horz 0.0 0.0 2.1 0.0 | Right2 Vert -0.5 -0.4 -6.7 -1.3 | Vind Press Horz -1.7 -3.8 -3.8 -1.7 |
| Line 6 6 6 6 | Line D C B A | Vert 0.2 0.7 0.7 | Vert 0.0 0.1 0.1 | Ver 1.1 3.8 3.8 1.1 | t V | ert 2.2 7.7 7.7 | Horz 0.0 -2.1 0.0 0.0 Sels | Vert -2.1 -8.7 -2.4 -1.2 | Horz 0.0 0.0 2.1 0.0 Sels | Vert -1.2 -2.4 -8.7 -2.1 -Right | Horz 0.0 -2.1 0.0 0.0 | Vert -1.3 -6.7 -0.4 -0.5 | Horz 0.0 0.0 2.1 0.0 | Vert -0.5 -0.4 -6.7 -1.3 | Press Horz -1.7 -3.8 -3.8 -1.7 |

| | | | Col | umn Reo | ction | -(k) - | 5 5, 5 5 |
|-----------|-------------|------------|------------|-------------|----------|-------------|--------------|
| rm Ine | Col Line | Load Id | | | Load | | V Vmin |
| | Α | 6 8 | 1.2 | -1.2 2.7 | 7 6 | -1.0 1.2 | -1.2 -1.2 |
| | В | 9 | 2.5 0.0 | -4.9 9.5 | 7 9 | -2.3 2.5 | -3.5 -4.9 |
| | С | 10 12 | 2.5 0.0 | -4.9 9.5 | 11 10 | -2.3 2.5 | -3.5 -4.9 |
| | D | 13 12 | 1,2 | -1.2 2.7 | 11 13 | -1.0 1.2 | -1.2 -1.2 |
| • | D | 6 14 | 1.2 0.0 | -1.2 2.7 | 7 6 | -1.0 1.2 | -1.2 -1.2 |
| | С | 9 14 | 2.5 0.0 | -4.9 9.5 | 7 9 | -2.3 2.5 | -3.5 -4.9 |
| , | В | 10 15 | 2.5 0.0 | -4.9 9.5 | 11 10 | -2.3 2.5 | -3.5 -4.9 |
| , | Α | 13 15 | 1.2 0.0 | -1.2 2.7 | 11 13 | -1.0 1.2 | -1.2 -1.2 |



| | | 그렇게 하는 이 경우는 그리지 않는데 그런 그는 그는 그는 것은 경우 가장 작년 |
|-----------------------------------|---------------------------------|--|
| Don S | Steel Building MFG | |
| PROJECT Don S Released for Permit | PHDNE: 303-358-5808 | |
| ID DBS073366 02/11/28 09:30 AM | ANCHOR BOLT REACTIONS | |
| PROJECT 203 Beacons L | DESIGN: Designer NBS | |
| ADDRESS Monument, CO CONSTRUCTION | DATE: 9/14/20 SHEET 3 DE 21 | |



| CON | NECTION PLATE |
|-----|---------------|
| | ME LINE D |
| | MARK/PART |
| 1 | JCE01 |
| 5 | JCA&P02 |

| | 1) | | 100'-0" OUT-TO-OUT (| JF STEEL | | | 6 |
|----------------------------|---|---------|----------------------|---|--------------|----------------|------------|
| | 18'-6" | 21′-0″ | 21'-0" | 4 | 21′-0″ | 5) 18'-6" | |
| | (16) (14) E-1 (13) | (₽) E-2 | E-2 | | E-2 | E-3 | (TE) |
| 4'-0" 4'-8" | DH-1 | G-5 | DH-1 A | G-6 2 2 | DH-1 G-8 | DH-1 | G-9 2 |
| 16'-0" 3'-8" 3'-8" 4'-C | G-3 C-3 C-3 C-4 C-3 C-4 C-3 C-4 C-5 C-5 C-5 C-5 C-6 C-7 C-7 C-7 C-7 C-7 C-7 C-7 C-7 | | DO 14′-0″ | - LU | 14'-0' DU-1 | ©-7 DU-1-0″ | G-9 B |
| | | | [] [] 2-1 | C I J RF2-1 | E R | _ | _ C |
| | 3'-0" 12'-0" 3'-6 | | 4'-6" 12'-0" | 4'-6" 4'-6" | 12'-0" 4'-6 | " B'-0" 12'-0" | 3'-6" |

SIDEWALL FRAMING: FRAME LINE D



SIDEWALL FRAMING PLAN

GENERAL NOTES

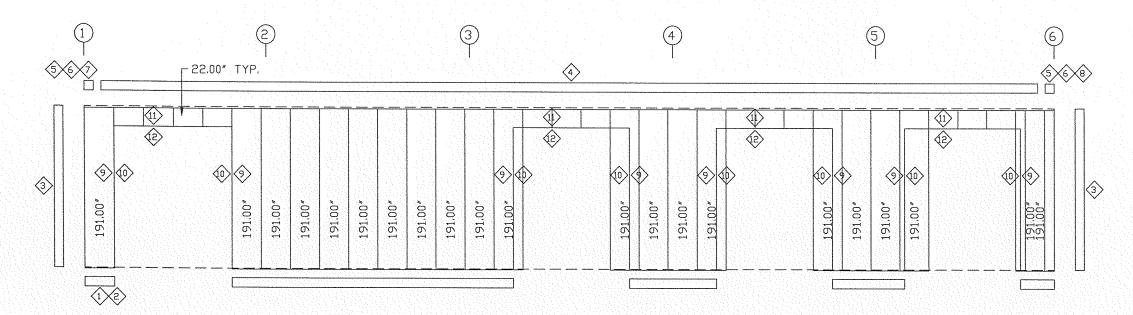
1. STD. ROD/CABLE SIZES PER PART PREFIX AF

| 1. | 31D KUDI CHDEE 312C3 | 1 CK I DIST I IN | CLI IN DIVE |
|----|----------------------|------------------|--------------|
| | ROD | CABLE | |
| | RDB- = 5/8' RDD | CAA = | = 1/4" CABLE |
| | RDC- = 3/4' RDD | CAB- = | = 3/8" CABLE |
| | RDD- = 7/8' RDD | CAC~ = | = 1/2' CABLE |
| | RDE- = 1' ROD | | |
| | RDF- = 1 1/8' RDD | | |
| | RDG- = 1 1/4" RDD | | |

- 2. ROD/CABLE BRACING THAT OCCURS IN FLUSH OR INSET GIRT CONDITIONS WILL REQUIRE FIELD SLOTTING OF GIRT WEBS TO ALLOW FOR BRACING.
- 3. FRAMED OPENINGS WHICH ARE FIELD LOCATED WILL REQUIRE FIELD CUTTING OF GIRTS AND SHEETING.
- 4. THIS DRAWING IS NOT TO SCALE.

| Don S | | Steel Building MFG |
|---|-----------------------------|-----------------------------|
| PROJECT | Don S Released for Permit | PHDNE: 303-358-5808 |
| 4LT-70-6-5-5-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6- | DRS073366 02/11/20 50:30 AM | SIDEWALL FRAMING |
| | 203 Beacons L | DESIGN: Designer NBS |
| ANNKEZZ | Monument, СП 🤫 МЯТВ ЙСТІОН | DATE: 9/14/20 SHEET 4 DF 21 |

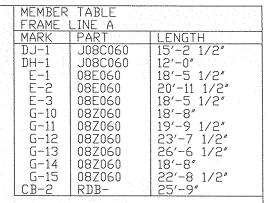
| 1 | | 1 TABLE | | |
|---|-----|-----------|----------|----------|
| | FRA | ME LINE D | | |
| | ΦID | PART | LENGTH | DETAIL |
| | 1 | BSD01 | 10'-2" | TRIM_200 |
| | 2 | BSD01 | Use Drop | TRIM 200 |
| | 3 | □CA01 | 20'-2" | TRIM 79 |
| | 4 | LEB01 | 10'-2" | TRIM_5 |
| | 5 | H4000 | 5" | |
| | 6 | ERA01 | 8 1/16" | |
| | 7 | RCA01 | 9 1/4" | 4 |
| | 8 | RCA02 | 9 1/4" | |
| | 9 | CCA169 | 14'-1" | TRIM_19 |
| | 10 | JTA169 | 14'-1" | TRIM 98 |
| | 11 | CCA145 | 12'-1" | TRIM 19 |
| | 12 | HTA148 | 12'-4" | TRIM_98 |



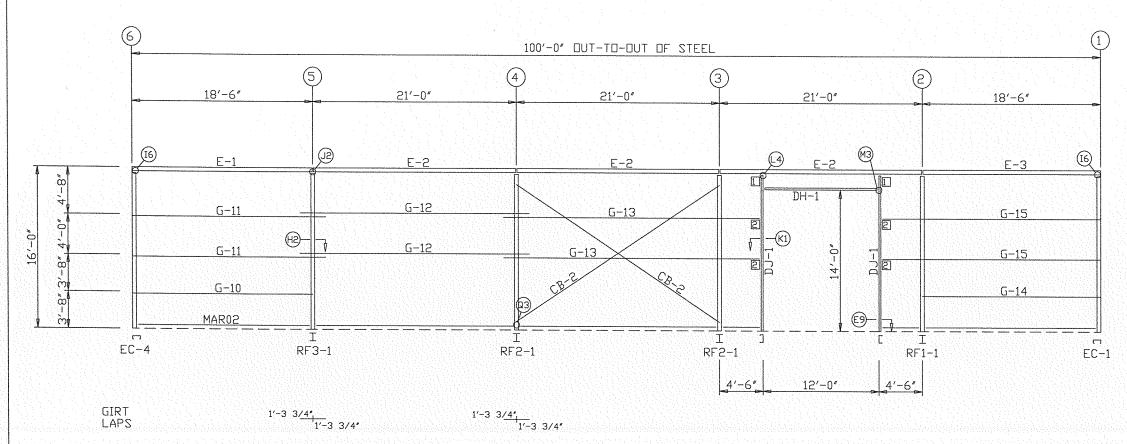
SIDEWALL SHEETING & TRIM: FRAME LINE D PANELS: 26 Ga. CW - Fox Gray SP



| 4.743 | | |
|----------------------|---------------------------|-----------------------------|
| Don S | | Steel Building MFG |
| PROJECT | Don S Released for Permit | PHDNE: 303-358-5808 |
| | Protontal | SIDEWALL FRAMING |
| decomposition in the | | DESIGN: Designer NBS |
| ADDRESS | Monument, CO SONSTRUCTION | DATE: 9/14/20 SHEET 5 OF 21 |



CONNECTION PLATES
FRAME LINE A
DID MARK/PART
1 JCE01
2 JCA&P02



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL FRAMING PLAN

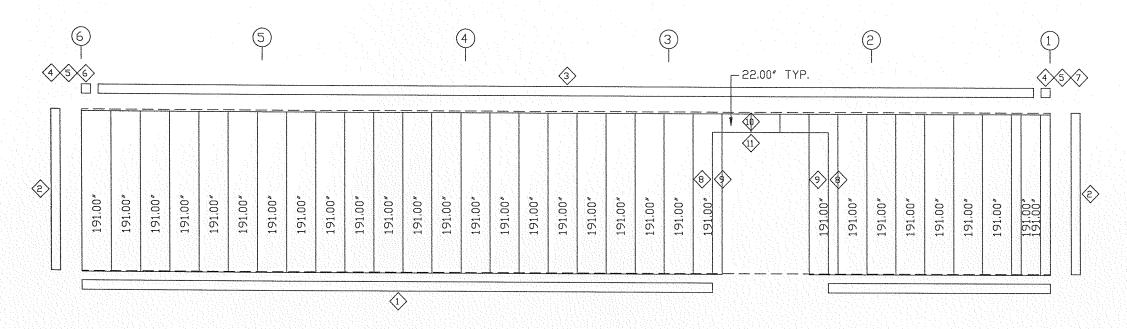
GENERAL NOTES

| STD. ROD/CABLE SIZES PER | PART PREFIX ARE |
|--------------------------|--|
| ROD | CABLE |
| RDB- = 5/8' ROD | CAA- = 1/4" CABLE |
| RDC- = 3/4' ROD | CAB- = 3/8' CABLE |
| RDD- = 7/8' RDD | CAC- = 1/2' CABLE |
| RDE- = 1' ROD | |
| RDF- = 1 1/8' RDD | |
| RDG- = 1 1/4' RDD | |
| | ROD RDB- = 5/8' ROD RDC- = 3/4' ROD RDD- = 7/8' ROD RDE- = 1' ROD RDF- = 1 1/8' ROD |

- 2. ROD/CABLE BRACING THAT OCCURS IN FLUSH OR INSET GIRT CONDITIONS WILL REQUIRE FIELD SLOTTING OF GIRT WEBS TO ALLOW FOR BRACING.
- 3. FRAMED OPENINGS WHICH ARE FIELD LOCATED WILL REQUIRE FIELD CUTTING OF GIRTS AND SHEETING.
- 4. THIS DRAWING IS NOT TO SCALE.

| Don S | | Steel Building MFG |
|---------|-------------------------------|-----------------------|
| PROJECT | | PHINE: 303-358-5808 |
| | DBS073366 02/11/20 1 50:30 AM | SIDEWALL FRAMING |
| PROJECT | 203 Beacons Li | DESIGN: Designer NBS |
| ADDKF22 | Monument, CD сомытистом | DATE: 9/14/20 SHEET 6 |

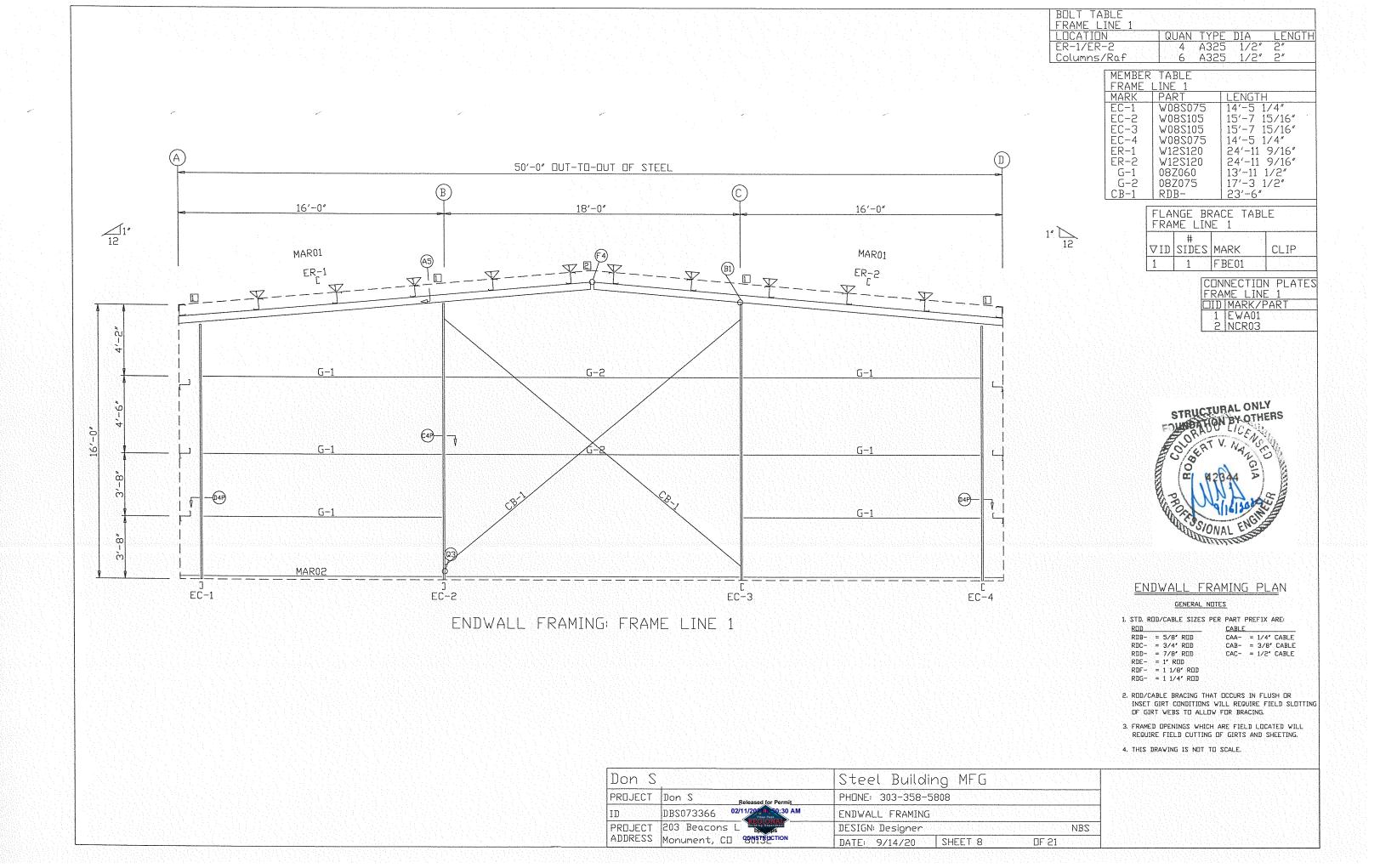
| | | 1 TABLE ME LINE | ^ | | | 1.7 |
|-----|----------------|--------------------|----------|---------|-------------|-----|
| | FKH | | <u> </u> | | | |
| . 1 | \Diamond III | PART | | LENGIH | DETAIL | |
| | 1 | BSD01 | | 10'-2" | TRIM 200 | |
| | 2 | DCA01 | | 20'-2" | TRIM 79 | |
| | 3 | LEB01 | | 10'-2" | TRIM_5 | |
| | 4 | H4000 | | 5″ | | |
| | 5 | ERA01 | | 8 1/16" | | |
| | 6 | RCA01 | | 9 1/4" | | |
| | 7 | RCA02 | | 9 1/4" | | |
| | 8 | CCA169 | | 14'-1" | TRIM_19 | |
| | 9 | JTA169 | | 14'-1" | TRIM_98 | |
| | 10 | CCA145 | | 12'-1" | TRIM_19 | |
| | 11 | HTA148 | | 12'-4" | TRIM_98 | |



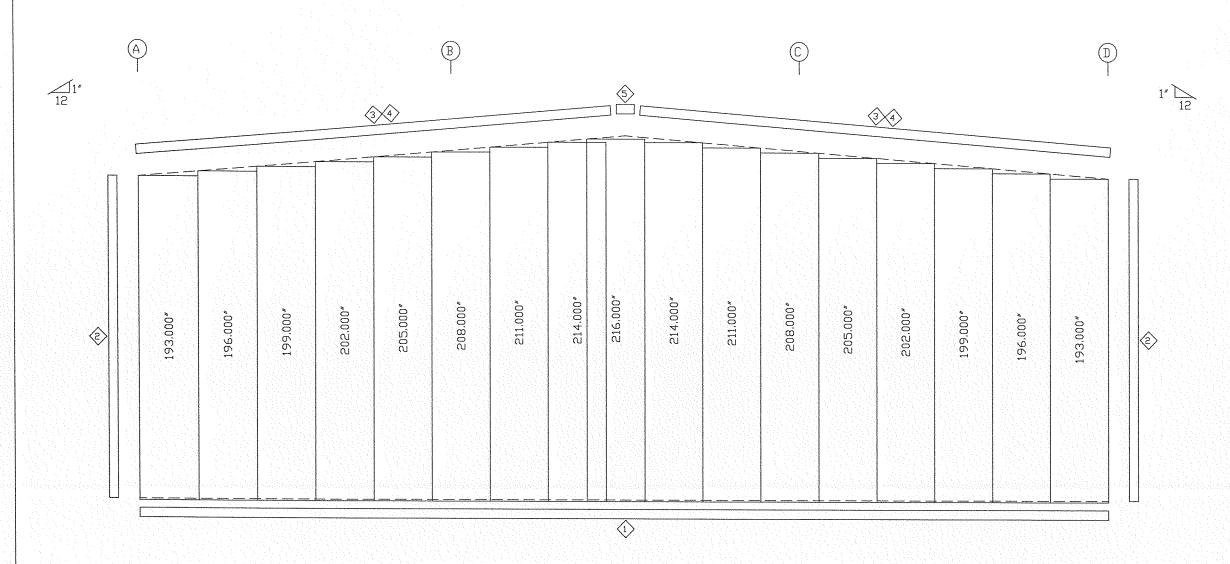
SIDEWALL SHEETING & TRIM: FRAME LINE A PANELS: 26 Ga. CW - Fox Gray SP



| Don S | | Steel Building MFG |
|---------|---|-----------------------------|
| PROJECT | Don S | PHDNE: 303-358-5808 |
| | DBS073366 02/11/20 09:30 AM | SIDEWALL FRAMING |
| PROJECT | 203 Beacons Light ps Monument, Co construction | DESIGN: Designer NBS |
| ADDKF22 | Monument, Co CONSTRUCTION | DATE: 9/14/20 SHEET 7 DF 21 |



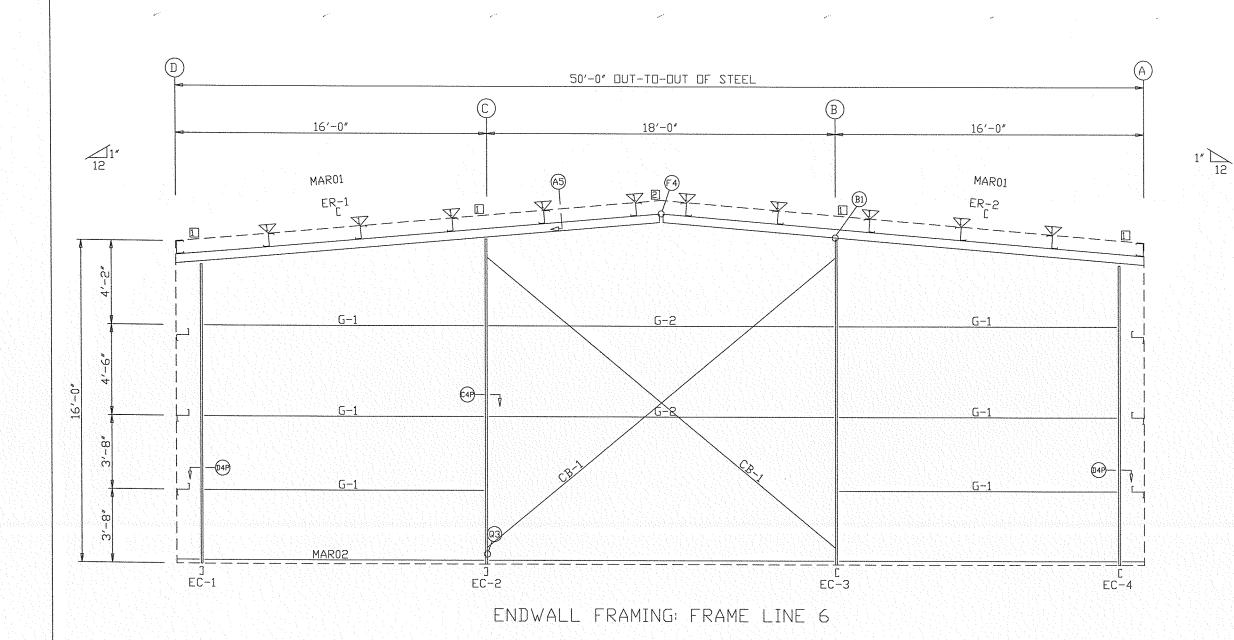
| TRIN | 1 TABLE | | |
|------|-----------|------------|----------|
| FRA | ME LINE 1 | | |
| ♦ID | PART | LENGTH | DETAIL |
| 1 | BSD01 | 10'-2" | TRIM_200 |
| 2 | □CA01 | 20'-2" | TRIM_79 |
| 3 | RTA01 | 10'-1" | TRIM_2 |
| 4 | RTA02 | 20'-2" | TRIM_2 |
| 5 | MPB01 | 2'-2 7/16" | |



ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 26 Ga. CW - Fox Gray SP



| | Don S | | Steel Building MFG | |
|---|---------|-------------------------------|-----------------------------|--|
| | PROJECT | | PHDNE: 303-358-5808 | |
| | | DBS073366 02/11/29 1 50:30 AM | ENDWALL FRAMING | |
| | | | DESIGN: Designer NBS | |
| À | ADDRESS | Monument, Co construction | DATE: 9/14/20 SHEET 9 DF 21 | |



BOLT TABLE
FRAME LINE 6
LOCATION QUAN TYPE DIA LENGTH
ER-1/ER-2 4 A325 1/2" 2"
Columns/Raf 6 A325 1/2" 2"

| | MEMBER | TABLE | |
|---|--------|---------|--------------|
| - | FRAME | LINE 6 | * |
| | MARK | PART | LENGTH |
| ĺ | EC-1 | W08S075 | 14'-5 1/4" |
| | EC-5 | W08S105 | 15'-7 15/16" |
| | EC-3 | W08S105 | 15'-7 15/16" |
| | EC-4 | W08S075 | 14'-5 1/4" |
| | ER-1 | W12S120 | 24'-11 9/16" |
| 1 | ER-2 | W12S120 | 24'-11 9/16" |
| | G-1 | 08Z060 | 13'-11 1/2" |
| | G-2 | 08Z075 | 17'-3 1/2" |
| | CB-1 | RDB- | 23'-6" |

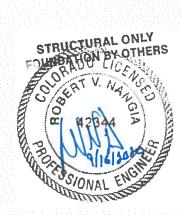
FLANGE BRACE TABLE
FRAME LINE 6

#

VID SIDES MARK CLIP

1 1 FBE01

CONNECTION PLATES
FRAME LINE 6
DID MARK/PART
1 EWA01
2 NCR03



ENDWALL FRAMING PLAN

GENERAL NOTES

1. STD. ROD/CABLE SIZES PER PART PREFIX ARE

ROD CABLE

ROD CABLE

ROD- = 5/8' ROD CAA- = 1/4' CABLE

ROD- = 3/4' ROD CAB- = 3/8' CABLE

ROD- = 7/8' ROD CAC- = 1/2' CABLE

ROD- = 1' ROD

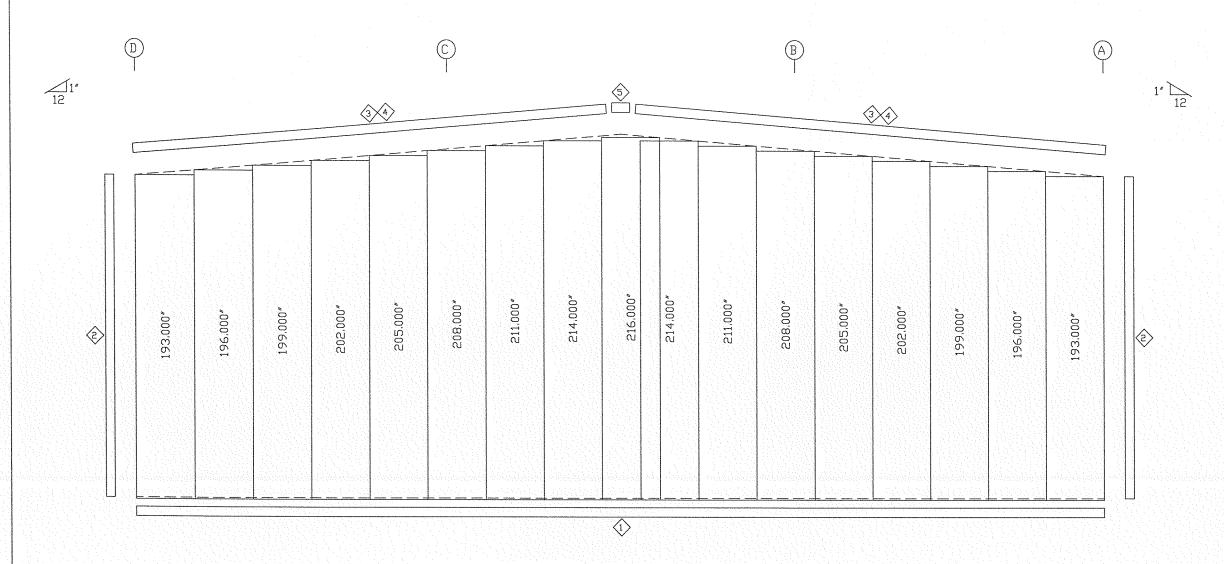
ROF- = 1 1/8' ROD

ROG- = 1 1/4' ROD

- 2. ROD/CABLE BRACING THAT OCCURS IN FLUSH OR INSET GIRT CONDITIONS WILL REQUIRE FIELD SLOTTING OF GIRT WEBS TO ALLOW FOR BRACING.
- 3. FRAMED OPENINGS WHICH ARE FIELD LOCATED WILL REQUIRE FIELD CUTTING OF GIRTS AND SHEETING.
- 4. THIS DRAWING IS NOT TO SCALE.

| Don S | | Steel Building MFG |
|---------|---------------------------|--------------------------|
| PROJECT | Don S | PHDNE: 303-358-5808 |
| ID | | ENDWALL FRAMING |
| | | DESIGN: Designer NBS |
| ADDRESS | Monument, CO CONSTITUTION | DATE: 9/14/20 SHEET 10 |

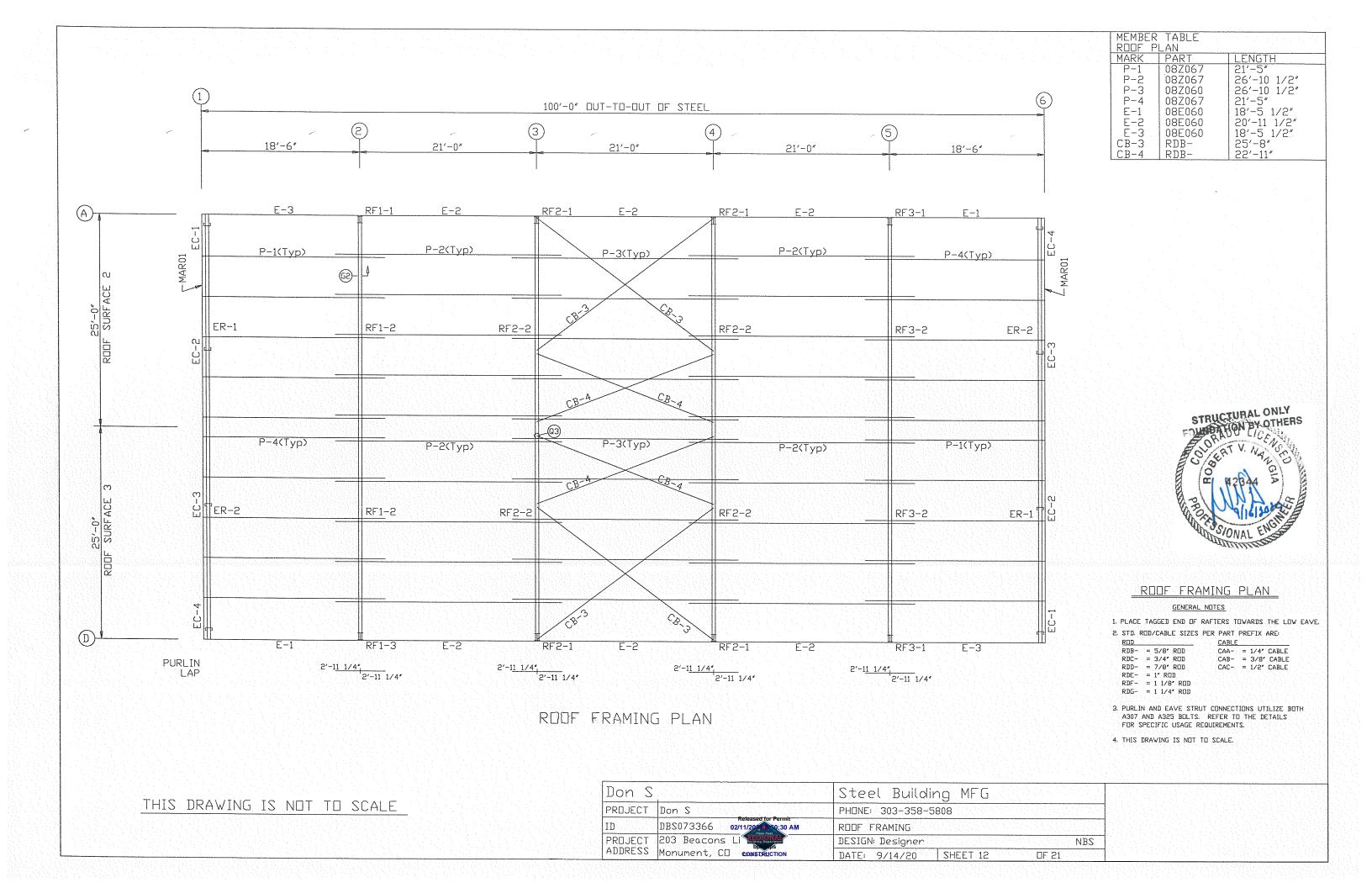
| TRIN | 1 TABLE | | |
|------|-----------|------------|----------|
| FRA | ME LINE 6 | | |
| ♦ID | PART | LENGTH | DETAIL |
| 1 | BSD01 | 10'-2" | TRIM_200 |
| 2 | □CA01 | 20'-2" | TRIM_79 |
| 3 | RTA01 | 10'-1" | TRIM_2 |
| 4 | RTA02 | 20'-2" | TRIM_2 |
| 5 | MPB01 | 2'-2 7/16" | |



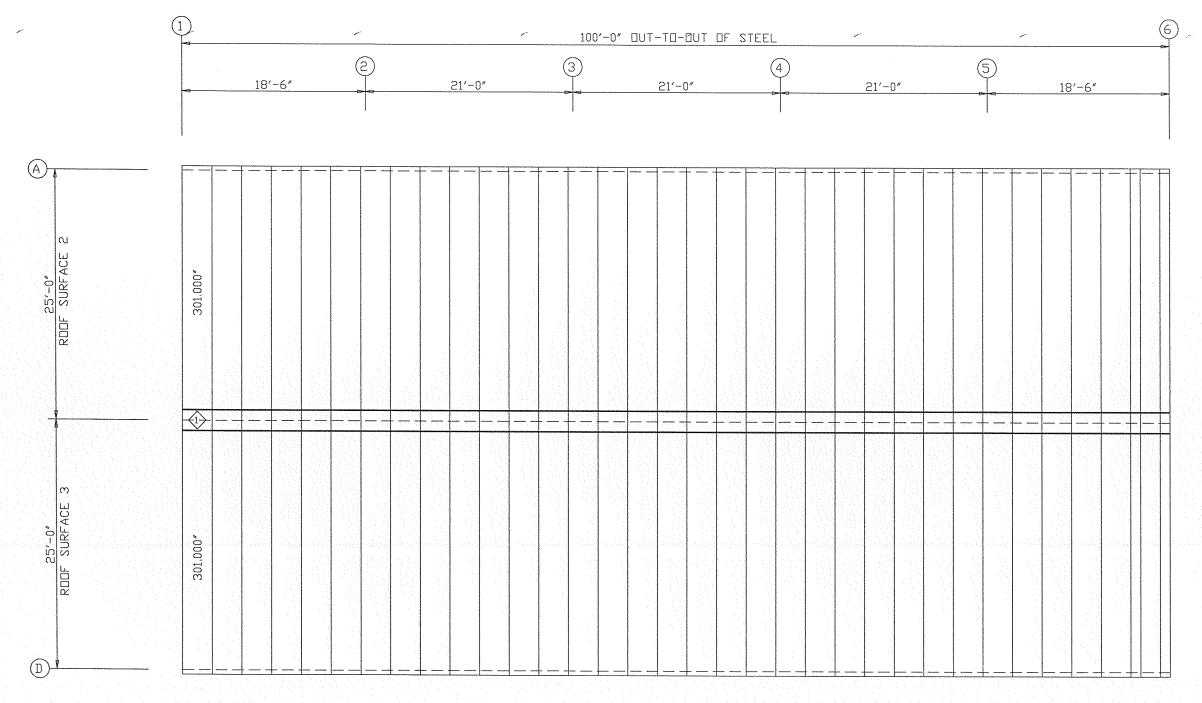
ENDWALL SHEETING & TRIM: FRAME LINE 6
PANELS: 26 Ga, CW - Fox Gray SP



| Don S | | Steel Building MFG | |
|---------|----------------------------|------------------------------|--|
| PROJECT | Don S Released for Permit | PHDNE: 303-358-5808 | |
| ID | DBS073366 02/11/20 0:30 AM | ENDWALL FRAMING | |
| | | DESIGN: Designer NBS | |
| ANDKEZZ | Monument, Co 801532 CTION | DATE: 9/14/20 SHEET 11 DF 21 | |



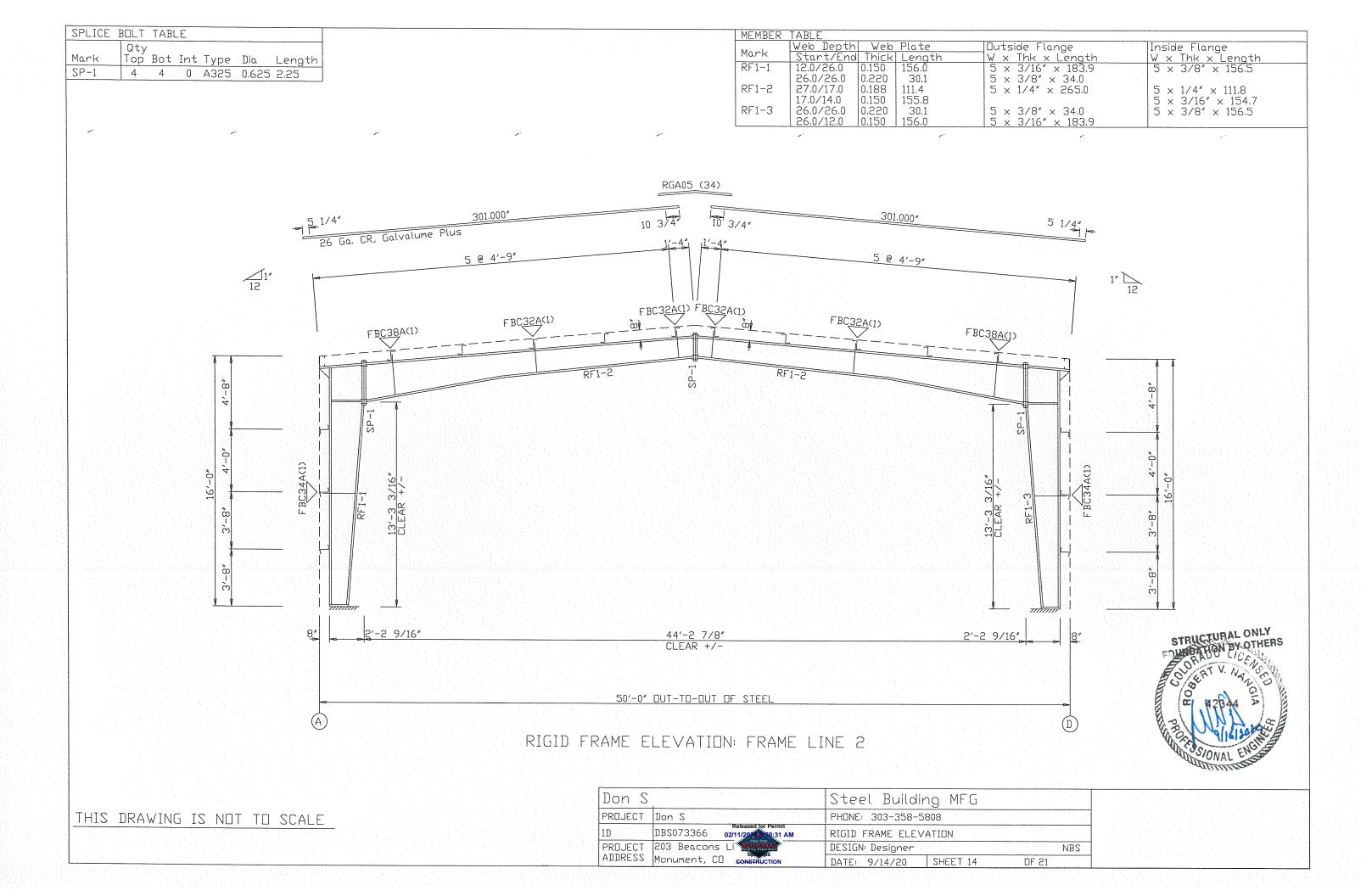
| TRIM TABLE | | |
|------------|--------|--------|
| ROOF PLAN | · · · | |
| ♦ID PART | LENGTH | DETAIL |
| 1 RGA05 | 3'-0" | TRIM_3 |

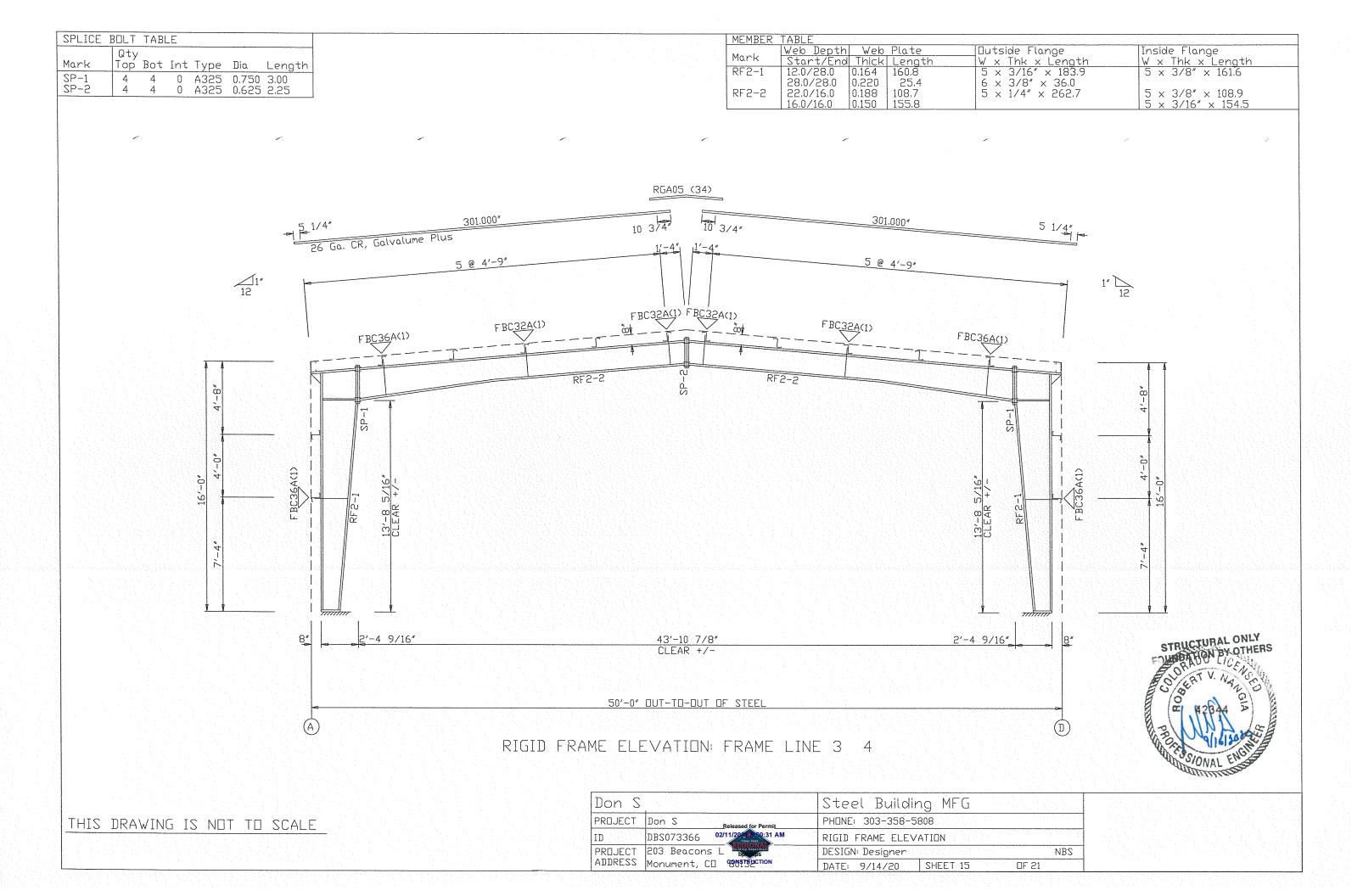


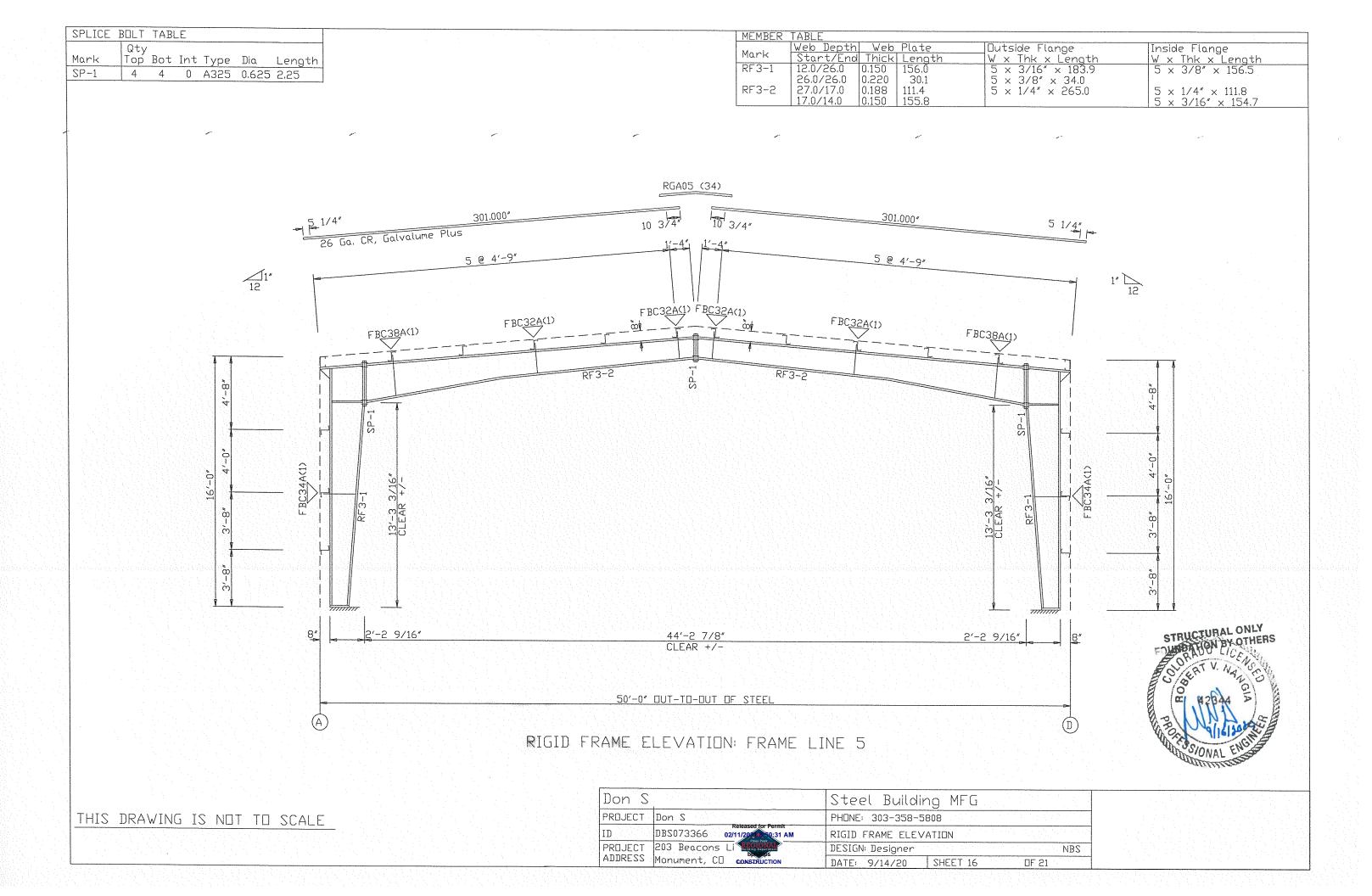


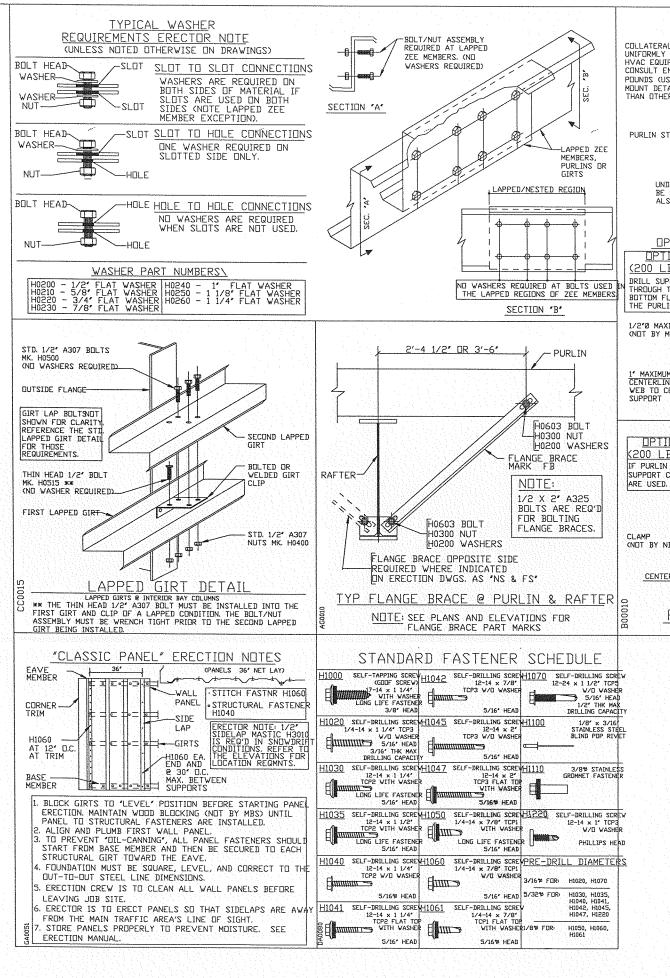
| Don S | | Steel Building MFG | |
|---------|------------------------------|------------------------|-------|
| PROJECT | 1 | PHONE: 303-358-5808 | |
| | DBS073366 02/11/20 150:31 AM | ROOF SHEETING | |
| PROJECT | 203 Beacons Li REGIONAL | DESIGN: Designer | NBS |
| ADDRESS | Monument, CD CONSTRUCTION | DATE: 9/14/20 SHEET 13 | OF 21 |











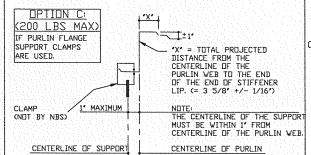
COLLATERAL DEAD LOADS, UNLESS DTHERVISE NOTED, ARE ASSUMED TO BE UNIFORMLY DISTRIBUTED. WHEN SUSPENDED SPRINKLER SYSTEMS, LIGHTING, HVAC EQUIPMENT, CEILINGS, ETC. ARE SUSPENDED FROM ROOF MEMBERS, CONSULT ENGINEER OF RECORD IF THESE CONCENTRATED LOADS EXCEED SO POUNDS (USING THE WEB MOUNT DETAIL) OR 200 POUNDS (USING THE FLANGE MOUNT DETAIL) OR IF INDIVIDUAL MEMBERS ARE LOADED SIGNIFICANTLY MORE THAN OTHERS.



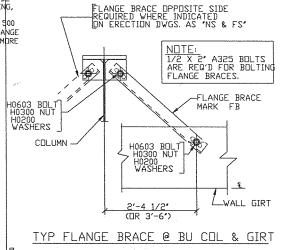
GENERAL RESTRICTION:

UNDER NO CIRCUMSTANCES CAN THE PURLIN STIFFENING LIP BE FIELD MODIFIED FROM THE FACTORY SUPPLIED CONDITION. ALSO DO NOT HANG ANYTHING FROM PURLIN STIFFENING LIP.

OPTIONS FOR SUPPORT ATTACHMENTS OPTION B (200 LBS MAX) (500 LBS MAX) SUPPORT ANGLE OR SOME OTHER TYPE OF BRACKET. (NOT BY NBS) TRITISH SUPPRIE THROUGH THE BOTTOM FLANGE OF THE PURLIN. SUPPORT THROUGH PURLIN WEB. 1/2'Ø MAXIMUM BOLT (NOT BY MBS) ANGLE SUPPORT (NOT BY MBS) 1' MAXIMUM FROM CENTERLINE OF PURLIN CENTERLINE OF PURLIN WEB TO CENTERLINE OF SUPPORT WEB TO CENTERLINE OF



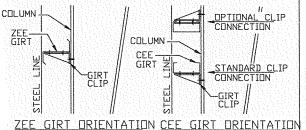
PURLIN SUPPORT METHODS



ERECTOR NOTE:UNLESS SPECIFICALLY NOTED OTHERWISE, STANDARD ZEE GIRT DRIENTATION IS TO HAVE THE GIRT TOED DOWN AT THE STEEL LINE AS SHOWN IN THE DETAIL BELOW.

NOTE: SEE PLANS AND ELEVATIONS FOR FLANGE BRACE PART MARKS

UNLESS SPECIFICALLY NOTED OTHERWISE, STANDARD CEE GIRT ORIENTATION IS TO HAVE THE GIRT TOED UP AS SHOWN IN THE DETAIL BELOW, STANDARD CLIP ATTACHMENT IS BELOW THE LIGHT, HOWEVER SOME DETAILS REQUIRE THAT THE CLIP BE ABOVE THE GIRT. (REFER TO THE GIRT DETAILS ON THE ERECTION DRAWINGS FOR REQUIREMENTS) BOTH CLIP ATTACHMENTS ARE SHOWN IN THE DETAIL BELOW.



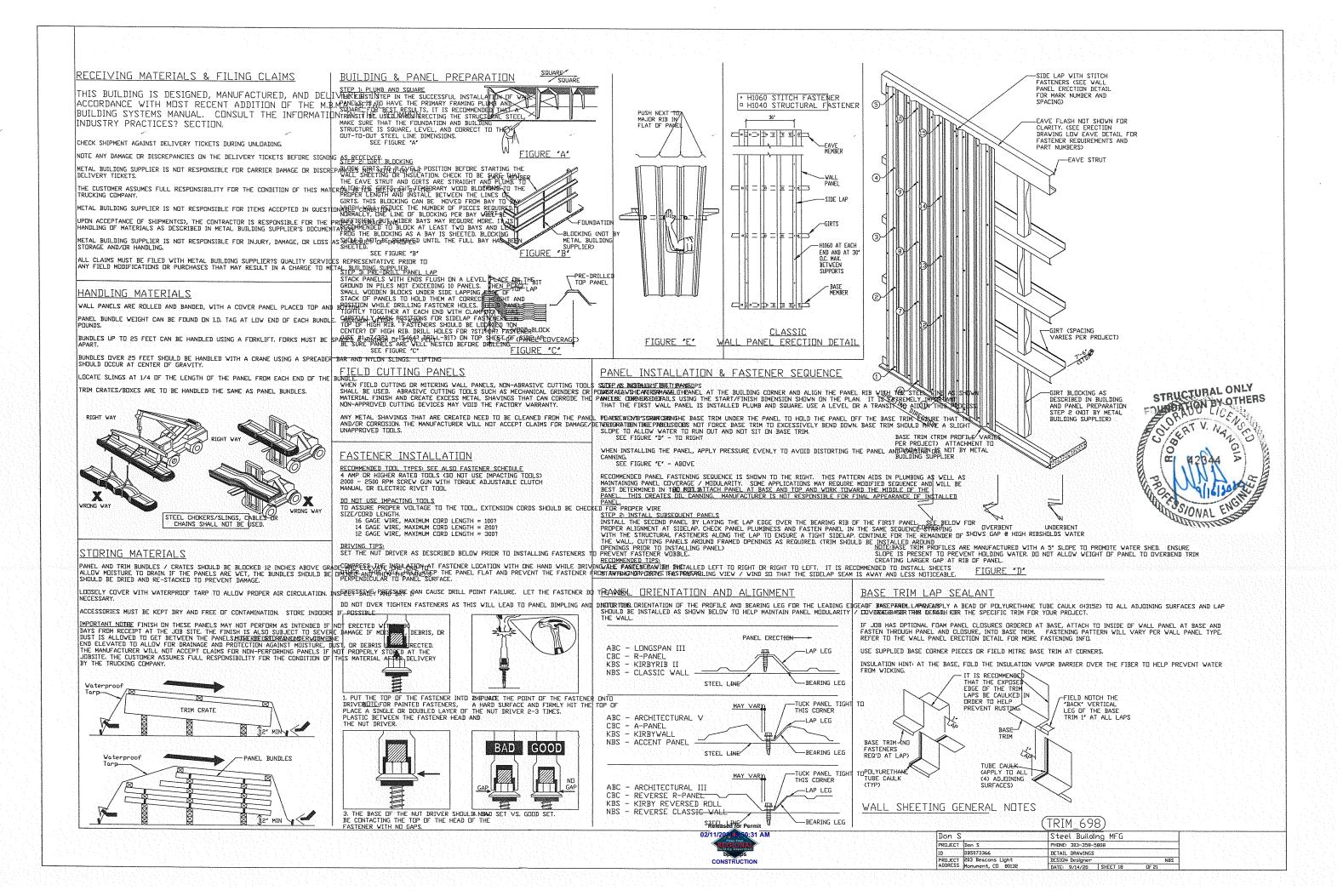
STANDARD GIRT DRIENTATION DETAIL

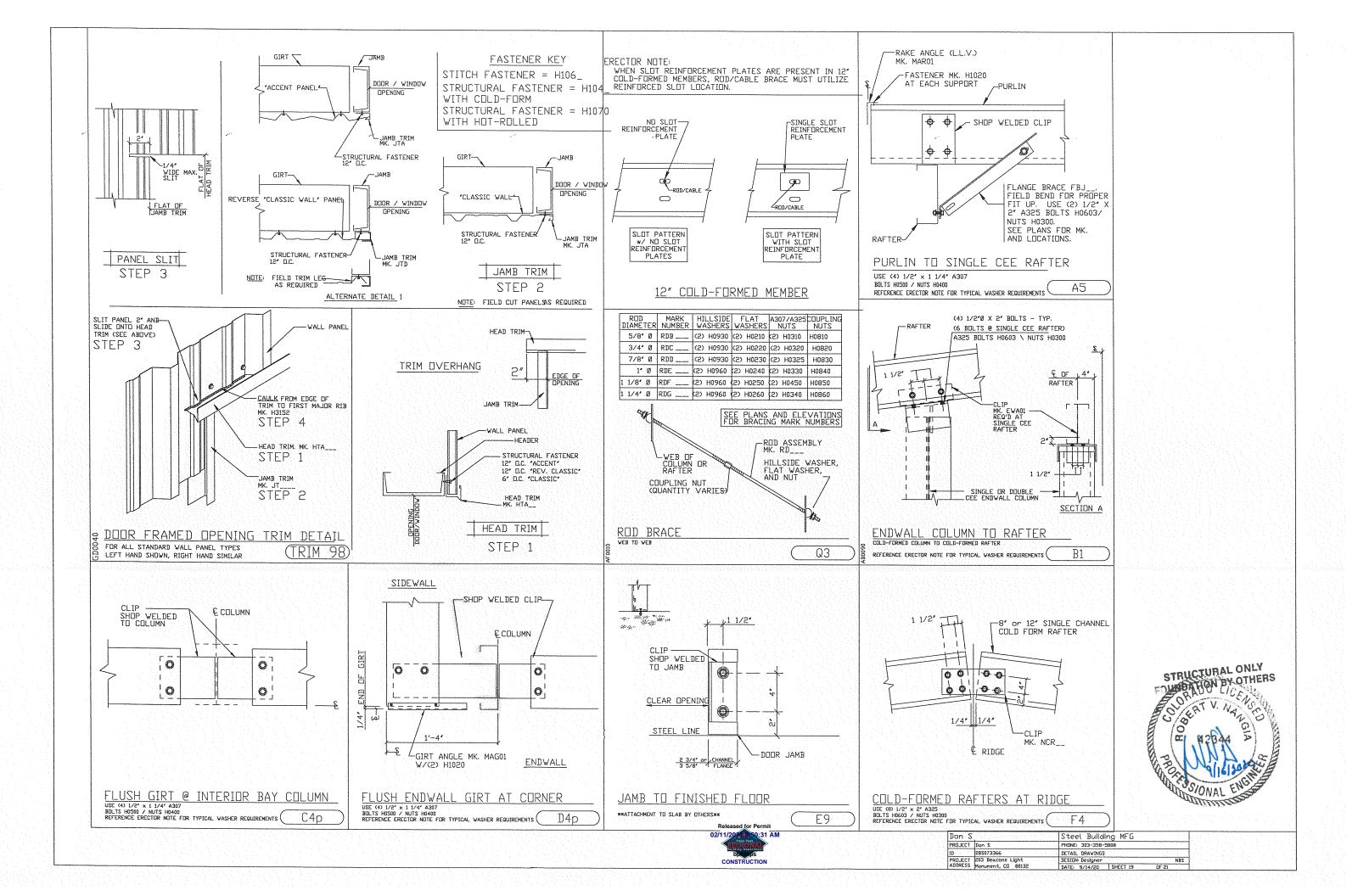
NOTE: BYPASS GIRT CONDITION IS SHOWN FOR REFERENCE ONLY. YOUR PROJECT MAY HAVE FLUSH OR INSET GIRTS.

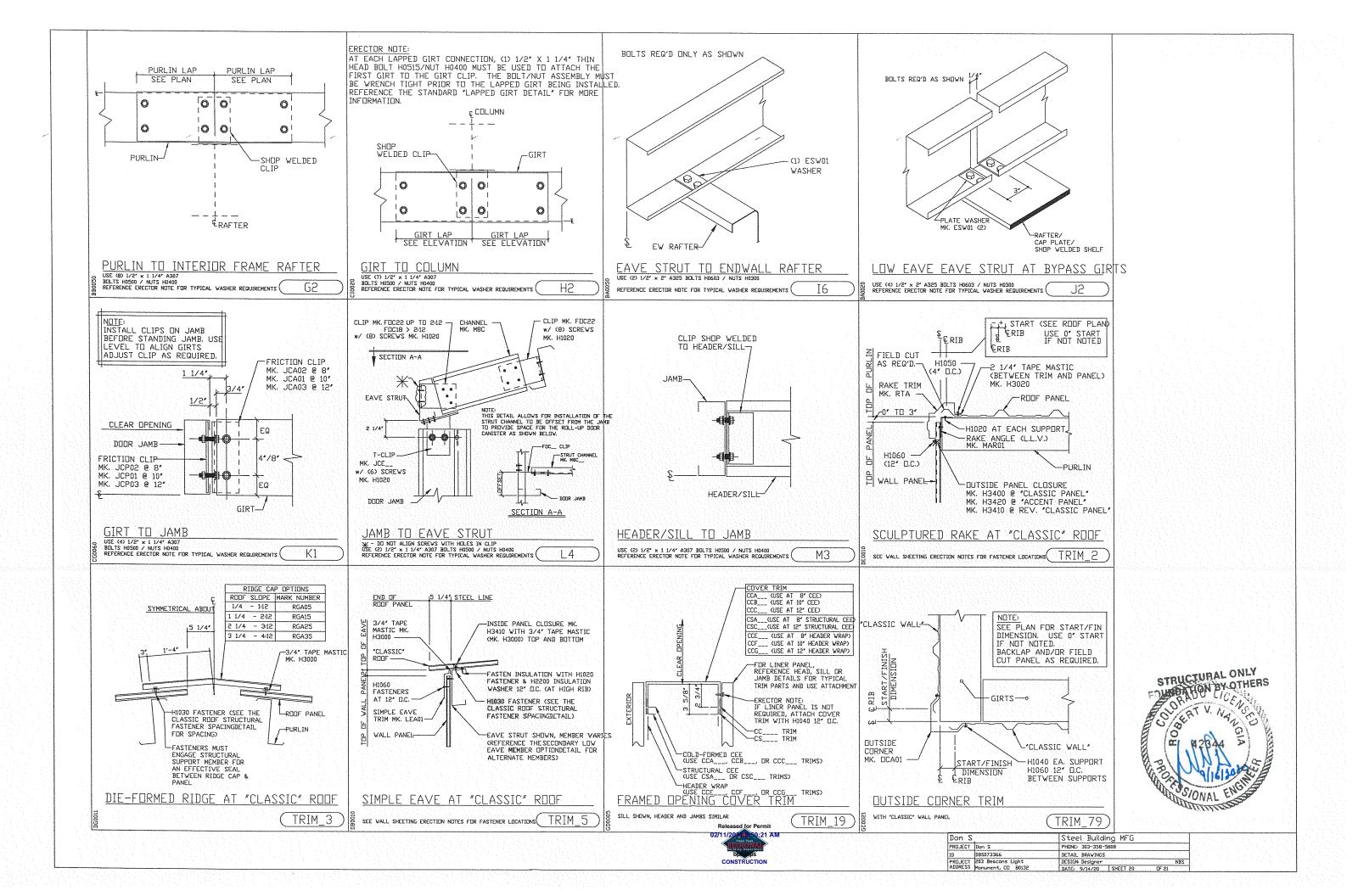


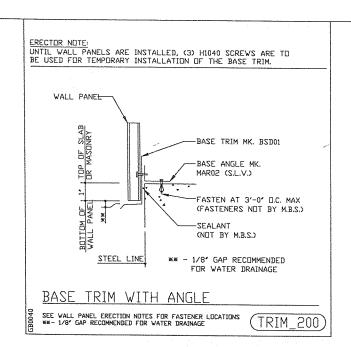


| on S | | Steel Building MFG | |
|---------|--------------------|------------------------------|-----|
| ROJECT | Don S | PHDNE: 303-358-5808 | |
| D | DBS073366 | DETAIL DRAWINGS | |
| | 203 Beacons Light | DESIGN: Designer NBS | |
| ADDRESS | Monument, CO 80132 | DATE: 9/14/20 SHEET 17 OF 21 | 4.1 |













| | Don S | · · | Steel Building MFG | |
|-----|---------|--------------------|------------------------------|--|
| | PROJECT | Don S | PHDNE: 303-358-5808 | |
| | ID | DBS073366 | DETAIL DRAWINGS | |
| | | 203 Beacons Light | DESIGN: Besigner NBS | |
| 100 | ADDRESS | Monument, CO 80132 | DATE: 9/14/20 SHEET 21 DF 21 | |