

**DRAINAGE LETTER REPORT**  
For  
**BENT GRASS COMMERCIAL FILING NO. 4**

Bent Grass Market View  
El Paso County, Colorado

**September, 2026**

**PCD File No: PPR-2625**

Prepared for:

**Copestone General Contractors**  
555 Amelia Street, Suite 100  
Colorado Springs, CO 80915  
Contact: Nathan Derwick  
(719) 578-6155

Prepared by:

**Drexel, Barrell & Co.**  
101 Sawatch St, Suite 100  
Colorado Springs, CO 80903  
Contact: Tim McConnell, P.E.  
(719) 260-0887

## **TABLE OF CONTENTS**

|             |                                         |          |
|-------------|-----------------------------------------|----------|
| <b>1.0</b>  | <b>CERTIFICATION STATEMENTS .....</b>   | <b>x</b> |
| <b>2.0</b>  | <b>PURPOSE .....</b>                    | <b>1</b> |
| <b>3.0</b>  | <b>GENERAL SITE DESCRIPTION .....</b>   | <b>1</b> |
| <b>4.0</b>  | <b>DRAINAGE CRITERIA .....</b>          | <b>2</b> |
| <b>5.0</b>  | <b>EXISTING CONDITION .....</b>         | <b>2</b> |
| <b>6.0</b>  | <b>DEVELOPED CONDITION .....</b>        | <b>3</b> |
| <b>7.0</b>  | <b>FOUR STEP PROCESS .....</b>          | <b>5</b> |
| <b>8.0</b>  | <b>DRAINAGE &amp; BRIDGE FEES .....</b> | <b>6</b> |
| <b>9.0</b>  | <b>SUMMARY .....</b>                    | <b>6</b> |
| <b>10.0</b> | <b>REFERENCES .....</b>                 | <b>6</b> |

### **APPENDICES**

VICINITY MAP  
SOILS MAP  
FLOODPLAIN MAP  
HYDROLOGY CALCULATIONS  
REPORT EXCERPTS  
DRAINAGE MAP

**DRAINAGE LETTER REPORT**  
for  
**BENT GRASS COMMERCIAL FILING NO. 4**

**1.0 CERTIFICATION STATEMENTS**

**Engineer's Statement**

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the city/county for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

SIGNATURE (Affix Seal): \_\_\_\_\_  
For and on behalf of Drexel, Barrell & Co. Date  
Katherine Varnum, P.E. #53459

**Developer's Statement**

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

\_\_\_\_\_  
Nathan Derwick, President Date  
Copestone General Contractors  
555 Amelia Street, Suite 100  
Colorado Springs, CO 80915

**El Paso County**

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

\_\_\_\_\_  
Joshua J. Palmer, P.E. Date  
County Engineer / ECM Administrator

Conditions:

**DRAINAGE LETTER REPORT**  
for  
**BENT GRASS COMMERCIAL FILING NO. 4**

## **2.0 PURPOSE**

The purpose of this letter is to supplement the Final Drainage Report for Bent Grass East Commercial Filing No. 3 (approved November 2, 2021) with regards to the development Bent Grass Commercial Filing No. 4, a replat of Lots 4, 5, & 6 Bent Grass East Commercial Filing No. 3, in order to establish that the development is in conformance with the approved drainage design.

Runoff patterns, drainage facilities and the ability to safely pass developed runoff to historic downstream facilities shall be presented.

## **3.0 GENERAL SITE DESCRIPTION**

### Location

Bent Grass Commercial Filing No. 4 is located in Peyton, El Paso County, Colorado, within the Southeast Quarter of Section 1, Township 13 South, Range 65 West of the 6<sup>th</sup> P.M. El Paso County, Colorado. The property is bounded to the north by Bent Grass Market View, to the east is Meridian Park Drive, Tract A of Bent Grass East Commercial to the south, and Bent Grass Residential Filing No. 1 lots to the west.

### Proposed Development

The proposed development is the construction of a child care facility on Lot 2, with associated parking and landscaping. The proposed disturbed area consists of 1.36 acres.

### Soils

According to the Soil Survey of El Paso County Area, Colorado, prepared by the U.S. Department of Agriculture Soil Conservation Service, the site is underlain by the Columbine gravelly sandy loam (Soil No. 19), a hydrologic type A soils. See appendix for Soils map.

### Climate

This area of El Paso County can be described as the foothills, with total precipitation amounts typical of a semi-arid region. Winters are generally cold and dry, and summers relatively warm and dry. Precipitation ranges from 12 to 14 inches per year, with the majority of this moisture occurring in the spring and summer in the form of rainfall. Thunderstorms are common during the summer months.

### Floodplain Statement

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate

Map (FIRM) Panel 08041C0553G (December 7, 2018), no portion of the site lies within a designated floodplain.

#### **4.0 DRAINAGE CRITERIA**

The drainage analysis has been prepared in accordance with the current El Paso County Drainage Criteria Manual. Calculations were performed to determine runoff quantities during the 5-year and 100-year frequency storms for existing and developed conditions using the Rational Method as required for basins containing less than 100 acres.

#### **5.0 EXISTING CONDITION**

The existing condition were initially described in the aforementioned approved Final Drainage Report for the Bent Grass East Commercial Filing No. 3 development and have been copied below. These existing conditions considered each lot to be already developed, so impervious and runoff calculations reflect these developed conditions. The proposed development will affect part of Basins B and C (see appendix for drainage report excerpts). Overlot grading has been completed, and an access roadway, detention facility and utility infrastructure have been installed. The site generally follows a 3%-5% grade from north to south and currently drains directly to the south towards the existing water quality detention facility.

**Design Point 1 ( $Q_5=4$  cfs and  $Q_{100}=8$  cfs)** represents developed flows from Basin A (lots 2 and 3 and north half of the private road). These flows will be routed towards Design Point 1 where a private 5' Type R Sump Inlet will completely collect both the 5 and 100 yr. developed flows.

**Design Point 2 ( $Q_5=1$  cfs and  $Q_{100}=3$  cfs)** represents developed flows from Basin B (portion of lots 4 and 5 and south half of the private road). These flows will be routed towards Design Point 2 where a private 5' Type R Sump Inlet will completely collect both the 5 and 100 yr. developed flows.

**Design Point 3 ( $Q_5=7$  cfs and  $Q_{100}=13$  cfs)** represents developed flows from Basin C (lots 4 and 6) and a portion of offsite Basin K (existing residential development to the west). These flows will be routed towards Design Point 3 where a private 24" RCP storm stub will collect both the 5 and 100 yr. developed flows. The individual site plans for each of these lots will show how curb and gutter will collect these developed flows and route them towards the provided 24" RCP private storm stub. These flows remain consistent with Basins L ( $Q_5=18$  cfs and  $Q_{100}=35$  cfs) and K ( $Q_5=2$  cfs and  $Q_{100}=4$  cfs) from the previous report.

**Design Point 4 ( $Q_5=11$  cfs and  $Q_{100}=22$  cfs)** represents the total developed flows that will enter the existing pond at this location (Basins A, B, C, and a portion of Basin K). A concrete forebay is proposed within the existing pond at this location with the following criteria: (See Appendix)

Per UD-BMP Spreadsheet – **Concrete Forebay sizing**

0.003 Ac.-ft. or 131 SF min. Forebay with 12" high walls OR **88 SF min. with 18" high walls**  
**4.5" wide notch at end of forebay**

**Basin E (Q<sub>5</sub>=0.3 cfs and Q<sub>100</sub>=1.0 cfs)** represents developed flows from Basin E (landscape/setback area within lots 1, 2, and 3) that will continue to sheet flow in a northeasterly direction and directly into Bent Grass Meadows Dr. This minor developed flow was accounted for and remains consistent with the previously approved report. Also, per ECM I.7.1.C.1.a this basin of 0.3 ac. is not practical to be captured and will not drain towards the downstream control measures.

**Basin D (Q<sub>5</sub>=3 cfs and Q<sub>100</sub>=6 cfs)** represents developed flows from Basin D (lots 1 and a portions of lot 5 and a portions of the private road). These flows will continue to sheet flow directly into Meridian Park Dr. They then travel as curb and gutter flows to the existing sump inlet within the cul-de-sac and then directly into the existing pond. These flows were accounted for in the previously approved drainage report and remain consistent with Basin M1 (Q<sub>5</sub>=6 cfs and Q<sub>100</sub>=11 cfs) from the previous report.

## 6.0 DEVELOPED CONDITION

The proposed development consists of a childcare facility and associated parking and landscaping. The proposed grading and storm system will route flows to the south where they will enter the existing private 24" RCP storm sewer and be directed towards the existing water quality detention facility, Pond 2. Which was approved as part of Bent Grass East Commercial Filing No. 2 (SF1412). The rest of the subdivision will remain undeveloped at this time.

The table below lists the developed basins and design points along with their developed flow rates. See further discussion at the end of this section for a comparison with existing flows.

| DEVELOPED   |     |            |                      |                        |
|-------------|-----|------------|----------------------|------------------------|
| BASIN       | DP  | Area (Ac.) | Q <sub>5</sub> (CFS) | Q <sub>100</sub> (CFS) |
| OS1         | OS1 | 0.42       | 0.1                  | 0.9                    |
| A           | 1   | 0.28       | 1.0                  | 1.9                    |
| B           | 2   | 0.23       | 0.8                  | 1.4                    |
| C           | 3   | 0.44       | 1.7                  | 3.2                    |
| DP1+DP2+DP3 | 3A  | 0.95       | 3.1                  | 5.9                    |
| D           | 4   | 0.15       | 0.1                  | 0.4                    |
| E           | 5   | 0.07       | 0.1                  | 0.3                    |
| F           | 6   | 0.02       | 0.07                 | 0.1                    |
| G           | 7   | 0.57       | 2.1                  | 3.9                    |
| H           |     | 0.84       | 2.7                  | 5.3                    |
| DPOS1+H     | 8   | 1.27       | 2.2                  | 4.9                    |

**Basin A** is 0.28-acres of primarily parking area with some surrounding landscaping, located at the northern side of the site. The runoff generated within this basin is directed via curb and gutter as well as a concrete drainage chase towards a proposed, private, Type C area inlet located on the east side of the basin. The runoff entering the inlet, **Design Point 1**, has rates of Q<sub>5</sub>=1.0 cfs and Q<sub>100</sub>=1.9 cfs, and flows directly into the existing, private, 24" RCP storm sewer that bisects the site.

**Basin B** represents the 10,000 s.f. proposed single-story building, located in the middle of

the site. The runoff captured on the buildings roof is directed towards the roof drains at rates of  $Q_5=0.8$  cfs and  $Q_{100}=1.4$  cfs before being discharged directly into the same existing, private, 24" RCP storm sewer as previously mentioned for **Basin A**.

**Basin C** is 0.44-acres and consists of the western parking area for the site, as well as some surrounding sidewalks and landscaping. The runoff generated within this basin has rates of  $Q_5=1.7$  cfs and  $Q_{100}=3.2$  cfs and is directed south via a combination of curb and gutters as well as a concrete drainage chase. Eventually the runoff is captured by a proposed, private, 5' Type R curb inlet located at the southern end of the site, **Design Point 3**. After being captured at **DP3** the runoff is directed east via a proposed, private, 18" RCP storm sewer before joining the same existing, private, 24" RCP storm sewer mentioned for the previous basins.

**Design Point 3A** represents all the runoff from the site that will be entering the existing storm sewer system that runs north to south through the site. The runoff from the site that combines at **DP3A** has rates of  $Q_5=3.1$  cfs and  $Q_{100}=5.9$  cfs and continues south within the private 30" RCP storm sewer before being discharged into Pond 2 (SP-20-010).

**Basin D** is 0.15-acres located along the southern side of the site and is primarily made up of playground area for the childcare facility. This basin creates runoff rates of  $Q_5=0.1$  cfs and  $Q_{100}=0.4$  cfs, this runoff sheet flows to the south before being directly discharged into the existing, private, detention pond directly south of the site.

**Basin E** is a relatively small, 0.07-acre, basin that runs down the east side of the proposed building and playground, **Basin B** and **Basin D**. This basin creates runoff with rates of  $Q_5=0.1$  cfs and  $Q_{100}=0.3$  cfs, that will be directed east and south. Eventually this runoff will be directly discharged into the existing, private, detention pond.

**Basin F** consists of a proposed 5' sidewalk that will run along the southern side of Bent Grass Market View. This 0.02-acre basin directs runoff with rates of  $Q_5=0.07$  cfs and  $Q_{100}=0.1$  cfs, directly into Bent Grass Market View, where it will travel east via curb and gutter towards an existing, private, 5' Type R sump inlet. This runoff from the site was already accounted for in the original Bent Grass East Commercial Filing No. 3 Drainage Report. See Appendix for inlet calculations.

**Basin G** represents the eastern portion of the site which will remain undisturbed for this development. This 0.57-acre basin produces runoff with rates of  $Q_5=2.1$  cfs and  $Q_{100}=3.9$  cfs, whose drainage pattern will remain unchanged from the previous report. Runoff sheet flows into Lot 3B of Bent Grass East Commercial Filing No. 2B.

**Basin OS1** is a 0.42-acre offsite basin that is located along the western boundary of the site, containing the backyards of a number of residential lots from the Bent Grass Residential Filing No. 1 development. This basin creates runoff with rates of  $Q_5=0.1$  cfs and  $Q_{100}=0.9$  cfs that are directed southeast, sheet flowing into **Basin H** through **Design Point OS1 (DPOS1)**.

**Basin H** represents the western portion of the site which will remain largely undisturbed for the development, aside from a concrete pad being added to the southeast corner of the basin to act as a vehicle turnaround. This 0.84-acre basin creates runoff with rates of

Q<sub>5</sub>=2.7 cfs and Q<sub>100</sub>=5.3 cfs that follows the same drainage pattern as described in the previous drainage report, flowing south and discharging directly into the detention pond south of the site.

### **Flow Comparison**

| Comparison Flows |     |      |                |     |      |
|------------------|-----|------|----------------|-----|------|
| Existing         | Q5  | Q100 | Developed      | Q5  | Q100 |
| DP3              | 7.0 | 13.0 | DP3A+DP4-5+DP8 | 5.5 | 11.5 |
| Basin D          | 3.0 | 6.0  | DP7            | 2.1 | 3.9  |

The above table shows the comparison of existing vs. proposed flows leaving the site. As indicated, developed flows are less than those calculated in the existing conditions.

## **7.0 FOUR STEP PROCESS**

This project conforms to the El Paso County Four Step Process. The process for this site focuses on reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainage ways, and implementing long-term source controls.

1. **Employ Runoff Reduction Practices:** Proposed impervious areas on this site (roofs, asphalt/sidewalk) will sheet flow across landscaped ground as much as possible to slow runoff and increase time of concentration prior to being conveyed to the proposed public streets and storm sewer system. This will minimize directly connected impervious areas within the project site.
2. **Implement BMP's that provide a Water Quality Capture Volume with slow release:** Runoff from this project will be routed through onsite storm sewer to an existing detention basin, Pond 2 (SP-20-010), to the south. This will allow for the runoff to be treated for water quality before discharging into the offsite storm system.
3. **Stabilize Drainage Ways:** No drainage ways exist within the project boundaries. Runoff will enter the storm sewer system, and be directed towards the existing detention basin, Pond 2 (SP-20-010), to the south, this will allow for flow rate reduction and protection of downstream facilities.
4. **Implement Site Specific and Other Source Control BMP's:** Standard commercial source control will be utilized in order to minimize potential pollutants entering the storm system. Example source control measures consist of: indoor storage of chemicals; and trash receptacles in common areas.

## **8.0 DRAINAGE & BRIDGE FEES**

All Basin and Bridge fees were paid with the Final Plat for Bent Grass East Commercial Filing No. 3 "R15062". No additional Basin nor Bridge fees are required.

Based on the developments associated with this drainage report, the total runoff reaching the existing detention basin, Pond 2 (SP-20-010), as well as the overall imperviousness of the site will be slightly reduced. Due to these reductions, there are no updates nor upgrades that are necessary for the existing Pond 2. See appendix for Pond and Forebay calculations.

## **9.0 SUMMARY**

Development of Bent Grass Commercial Filing No. 4 will not adversely affect surrounding or downstream developments. All the of the developed flows being generated on, and subsequently leaving the site are less than those anticipated in the initial Final Drainage Report and analysis of the existing storm system that conveys the flows to the south has determined that the storm system adequately sized to accommodate the flows from the site. Therefore, it is acceptable to state that the drainage design for Bent Grass Commercial Filing No. 4 is in conformance with the Final Drainage Report from the original Bent Grass East Commercial Filing No. 3.

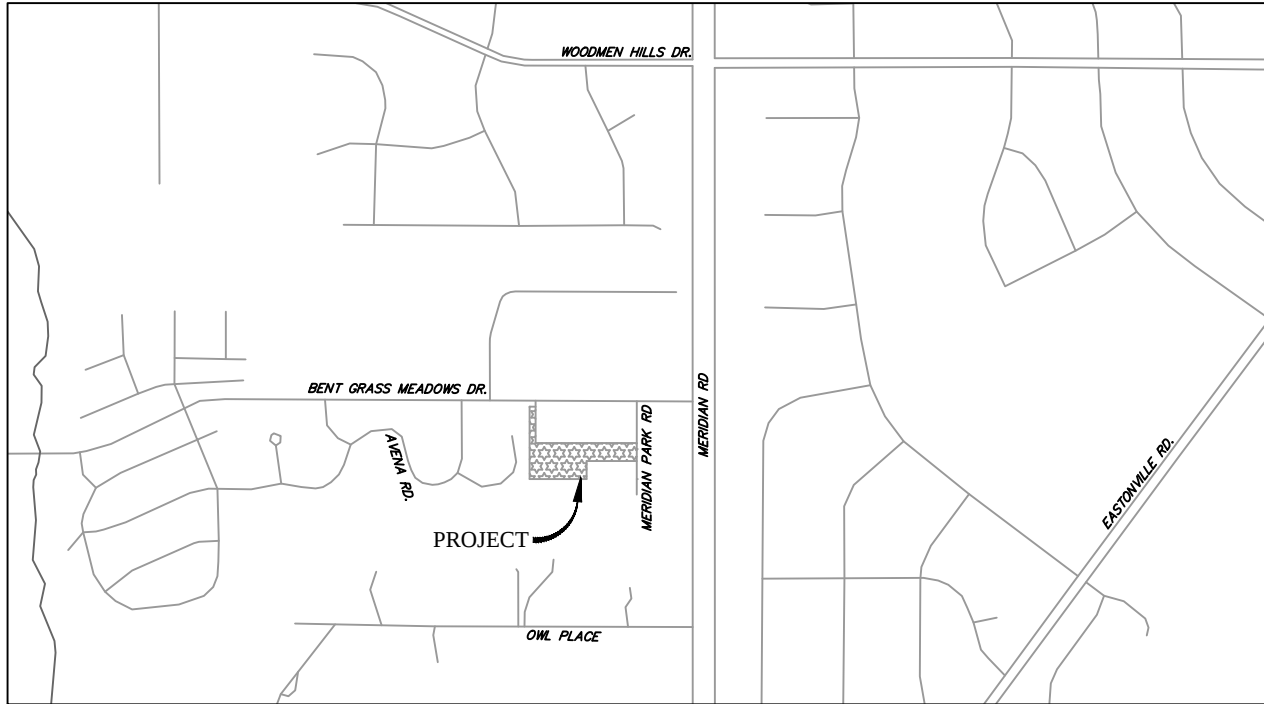
The downstream existing detention facility is functioning as intended and was designed to treat flows generated by this property.

## **10.0 REFERENCES**

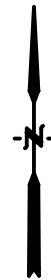
The sources of information used in the development of this study are listed below:

1. El Paso County Drainage Criteria Manual, 10-31-2018.
2. El Paso County Land Development Code, 5-25-2023.
3. Final Drainage Report for Bent Grass East Commercial Filing No. 3 (Classic Consulting) 11-02-2021. Amended 4-21-2022
4. United States Department of Agriculture – National Resources Conservation Service. Web Soil Survey.
5. Federal Emergency Management Agency (FEMA) Flood Map Service Center.

## Appendix



*Vicinity Map*  
Not to scale



BENT GRASS COMMERCIAL  
EL PASO COUNTY, CO  
VICINITY MAP

**Drexel, Barrell & Co.**  
Engineers • Surveyors

DATE:

DWG. NO.

JOB NO:

**21814-01CSCV**

**VMAP**

SHEET 1 OF 1



United States  
Department of  
Agriculture

**NRCS**

Natural  
Resources  
Conservation  
Service

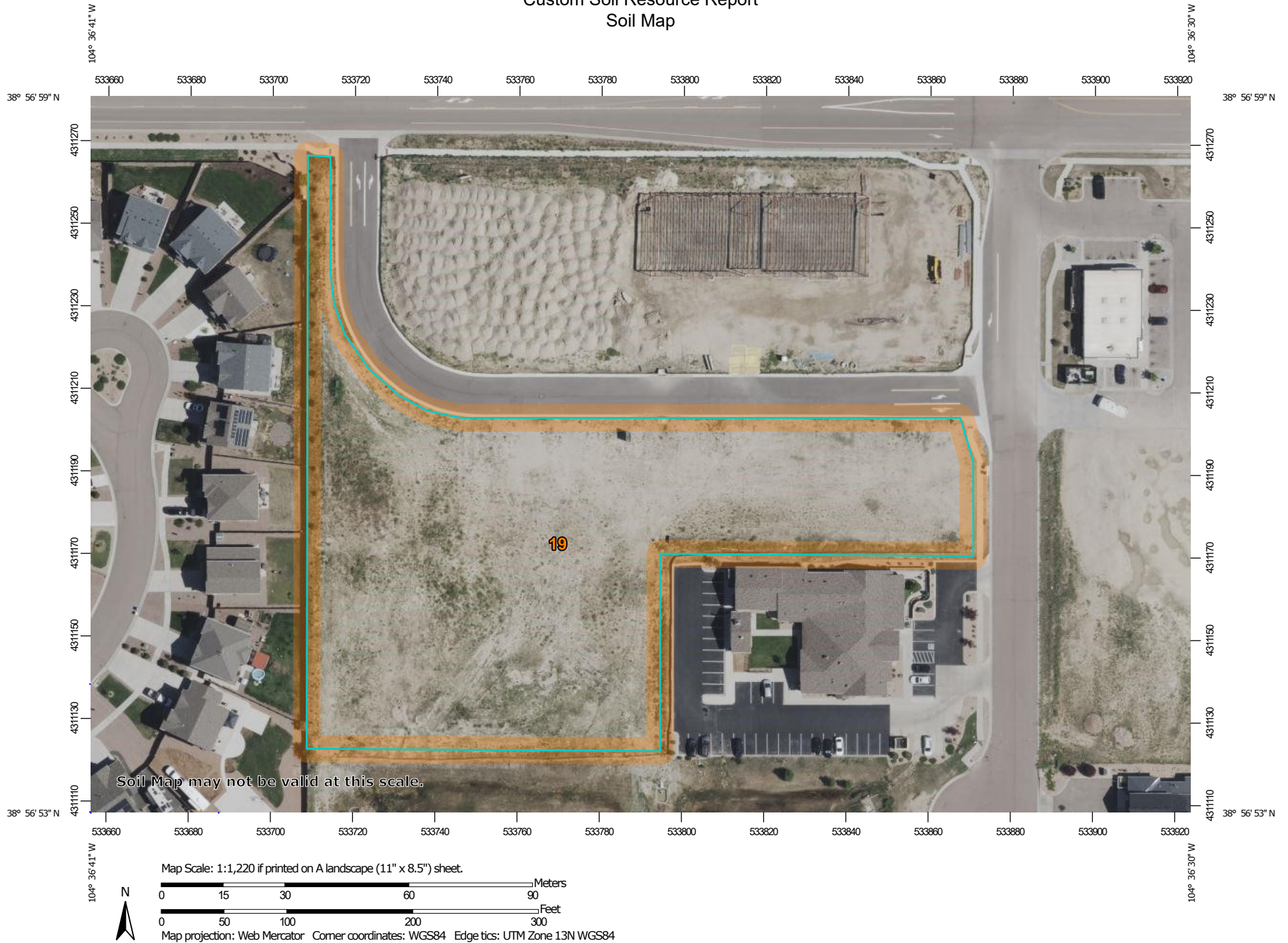
A product of the National  
Cooperative Soil Survey,  
a joint effort of the United  
States Department of  
Agriculture and other  
Federal agencies, State  
agencies including the  
Agricultural Experiment  
Stations, and local  
participants

# Custom Soil Resource Report for **El Paso County Area, Colorado**



August 12, 2026

# Custom Soil Resource Report Soil Map



# Custom Soil Resource Report

## MAP LEGEND

### Area of Interest (AOI)

 Area of Interest (AOI)

### Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines

 Soil Map Unit Points

### Special Point Features

 Blowout

 Borrow Pit

 Clay Spot

 Closed Depression

 Gravel Pit

 Gravelly Spot

 Landfill

 Lava Flow

 Marsh or swamp

 Mine or Quarry

 Miscellaneous Water

 Perennial Water

 Rock Outcrop

 Saline Spot

 Sandy Spot

 Severely Eroded Spot

 Sinkhole

 Slide or Slip

 Sodic Spot

 Spoil Area

 Stony Spot

 Very Stony Spot

 Wet Spot

 Other

 Special Line Features

### Water Features

 Streams and Canals

### Transportation

 Rails

 Interstate Highways

 US Routes

 Major Roads

 Local Roads

### Background

 Aerial Photography

## MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service  
Web Soil Survey URL:  
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado  
Survey Area Data: Version 23, Aug 29, 2025

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jul 23, 2024—Aug 4, 2024

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

## Map Unit Legend

| Map Unit Symbol                    | Map Unit Name                                           | Acres in AOI | Percent of AOI |
|------------------------------------|---------------------------------------------------------|--------------|----------------|
| 19                                 | Columbine gravelly sandy loam,<br>0 to 3 percent slopes | 2.5          | 100.0%         |
| <b>Totals for Area of Interest</b> |                                                         | <b>2.5</b>   | <b>100.0%</b>  |

## Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

## Custom Soil Resource Report

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## El Paso County Area, Colorado

### 19—Columbine gravelly sandy loam, 0 to 3 percent slopes

#### Map Unit Setting

*National map unit symbol:* 367p  
*Elevation:* 6,500 to 7,300 feet  
*Mean annual precipitation:* 14 to 16 inches  
*Mean annual air temperature:* 46 to 50 degrees F  
*Frost-free period:* 125 to 145 days  
*Farmland classification:* Not prime farmland

#### Map Unit Composition

*Columbine and similar soils:* 97 percent  
*Minor components:* 3 percent  
*Estimates are based on observations, descriptions, and transects of the mapunit.*

#### Description of Columbine

##### Setting

*Landform:* Fans, Fan terraces, Flood plains  
*Down-slope shape:* Linear  
*Across-slope shape:* Linear  
*Parent material:* Alluvium

##### Typical profile

*A - 0 to 14 inches:* gravelly sandy loam  
*C - 14 to 60 inches:* very gravelly loamy sand

##### Properties and qualities

*Slope:* 0 to 3 percent  
*Depth to restrictive feature:* More than 80 inches  
*Drainage class:* Well drained  
*Runoff class:* Very low  
*Capacity of the most limiting layer to transmit water (Ksat):* High to very high (5.95 to 19.98 in/hr)  
*Depth to water table:* More than 80 inches  
*Frequency of flooding:* None  
*Frequency of ponding:* None  
*Available water supply, 0 to 60 inches:* Very low (about 2.5 inches)

##### Interpretive groups

*Land capability classification (irrigated):* 4e  
*Land capability classification (nonirrigated):* 6e  
*Hydrologic Soil Group:* A  
*Ecological site:* R049XY214CO - Gravelly Foothill  
*Hydric soil rating:* No

#### Minor Components

##### Fluvaquentic haplaquolls

*Percent of map unit:* 1 percent  
*Landform:* Swales  
*Hydric soil rating:* Yes

## Custom Soil Resource Report

### **Other soils**

*Percent of map unit:* 1 percent

*Hydric soil rating:* No

### **Pleasant**

*Percent of map unit:* 1 percent

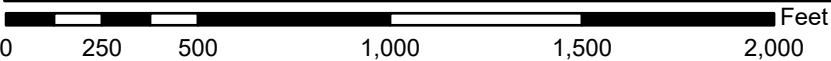
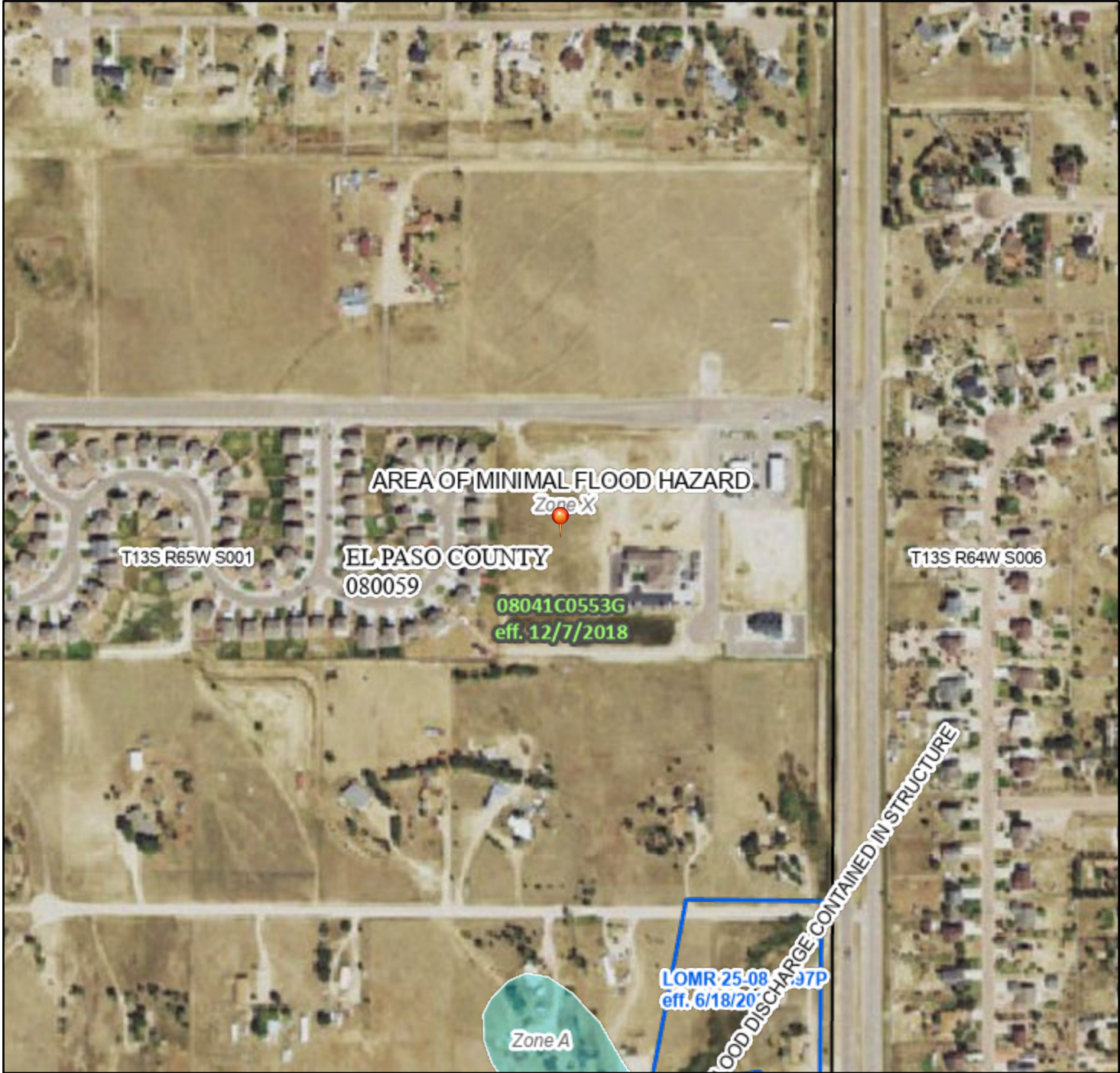
*Landform:* Depressions

*Hydric soil rating:* Yes

# National Flood Hazard Layer FIRMette



104°36'56"W 38°57'10"N



1:6,000

104°36'19"W 38°56'42"N

Basemap Imagery Source: USGS National Map 2023

## Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

|                             |  |                                                                                                                                                                   |
|-----------------------------|--|-------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| SPECIAL FLOOD HAZARD AREAS  |  | Without Base Flood Elevation (BFE)<br>Zone A, V, A99                                                                                                              |
|                             |  | With BFE or Depth Zone AE, AO, AH, VE, AR                                                                                                                         |
|                             |  | Regulatory Floodway                                                                                                                                               |
| OTHER AREAS OF FLOOD HAZARD |  | 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X |
|                             |  | Future Conditions 1% Annual Chance Flood Hazard Zone X                                                                                                            |
|                             |  | Area with Reduced Flood Risk due to Levee. See Notes. Zone X                                                                                                      |
|                             |  | Area with Flood Risk due to Levee Zone D                                                                                                                          |
| OTHER AREAS                 |  | NO SCREEN Area of Minimal Flood Hazard Zone X                                                                                                                     |
|                             |  | Effective LOMRs                                                                                                                                                   |
|                             |  | Area of Undetermined Flood Hazard Zone D                                                                                                                          |
| GENERAL STRUCTURES          |  | Channel, Culvert, or Storm Sewer                                                                                                                                  |
|                             |  | Levee, Dike, or Floodwall                                                                                                                                         |
| OTHER FEATURES              |  | 20.2 Cross Sections with 1% Annual Chance Water Surface Elevation                                                                                                 |
|                             |  | 17.5 Cross Sections with 1% Annual Chance Water Surface Elevation                                                                                                 |
|                             |  | Coastal Transect                                                                                                                                                  |
|                             |  | Base Flood Elevation Line (BFE)                                                                                                                                   |
|                             |  | Limit of Study                                                                                                                                                    |
|                             |  | Jurisdiction Boundary                                                                                                                                             |
|                             |  | Coastal Transect Baseline                                                                                                                                         |
| MAP PANELS                  |  | Digital Data Available                                                                                                                                            |
|                             |  | No Digital Data Available                                                                                                                                         |
|                             |  | Unmapped                                                                                                                                                          |



The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on **8/12/2026 at 8:55 PM** and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

**EXISTING RUNOFF  
CALCULATIONS FROM BENT  
GRASS EAST COMMERCIAL  
FILING NO. 3 (SF2147)**

JOB NAME: BENT GRASS EAST COMMERCIAL FILING NO. 3  
 JOB NUMBER: 2177.64  
 DATE: 11/04/20  
 CALCULATED BY: MAW

**FINAL DRAINAGE REPORT ~ BASIN RUNOFF COEFFICIENT SUMMARY**

| BASIN | TOTAL<br>AREA (AC) | IMPERVIOUS AREA / STREETS |      |      |        | LANDSCAPE/UNDEVELOPED AREAS |      |      |        | WEIGHTED |      |        | WEIGHTED CA |       |         |
|-------|--------------------|---------------------------|------|------|--------|-----------------------------|------|------|--------|----------|------|--------|-------------|-------|---------|
|       |                    | AREA (AC)                 | C(2) | C(5) | C(100) | AREA (AC)                   | C(2) | C(5) | C(100) | C(2)     | C(5) | C(100) | CA(2)       | CA(5) | CA(100) |
| A     | 1.4                | 1.10                      | 0.89 | 0.90 | 0.96   | 0.30                        | 0.02 | 0.08 | 0.35   | 0.70     | 0.72 | 0.83   | 0.99        | 1.01  | 1.16    |
| B     | 0.4                | 0.30                      | 0.89 | 0.90 | 0.96   | 0.10                        | 0.02 | 0.08 | 0.35   | 0.67     | 0.70 | 0.81   | 0.27        | 0.28  | 0.32    |
| C     | 2.0                | 1.60                      | 0.89 | 0.90 | 0.96   | 0.40                        | 0.02 | 0.08 | 0.35   | 0.72     | 0.74 | 0.84   | 1.43        | 1.47  | 1.68    |
| D     | 0.9                | 0.75                      | 0.89 | 0.90 | 0.96   | 0.15                        | 0.02 | 0.08 | 0.35   | 0.75     | 0.76 | 0.86   | 0.67        | 0.69  | 0.77    |
| E     | 0.3                | 0.05                      | 0.89 | 0.90 | 0.96   | 0.25                        | 0.02 | 0.08 | 0.35   | 0.17     | 0.22 | 0.45   | 0.05        | 0.07  | 0.14    |
| K     | 1.0                | 0.35                      | 0.89 | 0.90 | 0.96   | 0.65                        | 0.02 | 0.08 | 0.35   | 0.32     | 0.37 | 0.56   | 0.32        | 0.37  | 0.56    |
|       |                    |                           |      |      |        |                             |      |      |        |          |      |        |             |       |         |

JOB NAME: BENT GRASS EAST COMMERCIAL FILING NO. 3  
 JOB NUMBER: 2177.64  
 DATE: 11/04/20  
 CALC'D BY: MAW

Table 6-7. Conveyance Coefficient,  $C_v$

| Type of Land Surface                            | $C_v$ |
|-------------------------------------------------|-------|
| Heavy meadow                                    | 2.5   |
| Tillage/field                                   | 5     |
| Riprap (not buried)* $t_c = \frac{L}{180} + 10$ | 6.5   |
| Short pasture and lawns                         | 7     |
| Nearly bare ground                              | 10    |
| Grassed waterway                                | 15    |
| Paved areas and shallow paved swales            | 20    |

\*For buried riprap, select  $C_v$  value based on type of vegetative cover.

EXISTING RUNOFF  
 CALCULATIONS FROM BENT  
 GRASS EAST COMMERCIAL  
 FILING NO. 3 (SF2147)

$$t_i = \frac{0.395(1.1 - C_s)\sqrt{L}}{S^{0.33}}$$

$$V = C_v S_w^{0.5} \quad T_c = L/V$$

### FINAL DRAINAGE REPORT ~ BASIN RUNOFF SUMMARY

| BASIN | TOTAL<br>AREA (AC) | WEIGHTED |       |         | OVERLAND |                |                |             | STREET / CHANNEL FLOW |              |                   |             | Tc<br>TOTAL<br>(min) | INTENSITY       |                 |                   | TOTAL FLOWS   |               |                 |
|-------|--------------------|----------|-------|---------|----------|----------------|----------------|-------------|-----------------------|--------------|-------------------|-------------|----------------------|-----------------|-----------------|-------------------|---------------|---------------|-----------------|
|       |                    | CA(2)    | CA(5) | CA(100) | C(5)     | Length<br>(ft) | Height<br>(ft) | Tc<br>(min) | Length<br>(ft)        | Slope<br>(%) | Velocity<br>(fps) | Tc<br>(min) |                      | I(2)<br>(in/hr) | I(5)<br>(in/hr) | I(100)<br>(in/hr) | Q(2)<br>(cfs) | Q(5)<br>(cfs) | Q(100)<br>(cfs) |
| A     | 1.4                | 0.99     | 1.01  | 1.16    | 0.08     | 30             | 2              | 5.4         | 250                   | 2.0%         | 1.0               | 4.2         | 9.6                  | 3.34            | 4.19            | 7.03              | 3             | 4             | 8               |
| B     | 0.4                | 0.27     | 0.28  | 0.32    | 0.08     | 10             | 0.2            | 4.6         | 100                   | 1.5%         | 0.9               | 1.9         | 6.6                  | 3.79            | 4.76            | 7.99              | 1             | 1             | 3               |
| C     | 2.0                | 1.43     | 1.47  | 1.68    | 0.08     | 30             | 1.5            | 5.9         | 400                   | 2.0%         | 1.0               | 6.7         | 12.7                 | 3.01            | 3.77            | 6.34              | 4             | 6             | 11              |
| D     | 0.9                | 0.67     | 0.69  | 0.77    | 0.08     | 10             | 0.2            | 4.6         | 200                   | 2.0%         | 1.4               | 2.4         | 7.0                  | 3.72            | 4.67            | 7.83              | 2             | 3             | 6               |
| E     | 0.3                | 0.05     | 0.07  | 0.14    | 0.08     | 25             | 0.5            | 7.3         |                       |              |                   |             | 7.3                  | 3.67            | 4.60            | 7.72              | 0.2           | 0.3           | 1.0             |
| K     | 1.0                | 0.32     | 0.37  | 0.56    | 0.08     | 65             | 3              | 9.0         |                       |              |                   |             | 9.0                  | 3.43            | 4.29            | 7.21              | 1             | 2             | 4               |
|       |                    |          |       |         |          |                |                |             |                       |              |                   |             |                      |                 |                 |                   |               |               |                 |

**EXISTING RUNOFF  
CALCULATIONS FROM BENT  
GRASS EAST COMMERCIAL  
FILING NO. 3 (SF2147)**

JOB NAME: **BENT GRASS EAST COMMERCIAL FILING NO. 3**  
 JOB NUMBER: **2177.64**  
 DATE: **11/04/20**  
 CALCULATED BY: **MAW**

**FINAL DRAINAGE REPORT ~ SURFACE ROUTING SUMMARY**

| Design Point(s) | Contributing Basins | Equivalent CA(5) | Equivalent CA(100) | Maximum Tc | Intensity |        | Flow |        | Outfall / Inlet Size |
|-----------------|---------------------|------------------|--------------------|------------|-----------|--------|------|--------|----------------------|
|                 |                     |                  |                    |            | I(5)      | I(100) | Q(5) | Q(100) |                      |
| 1               | A                   | 1.01             | 1.16               | 9.6        | 4.19      | 7.03   | 4    | 8      | 5' Type R Sump Inlet |
| 2               | B                   | 0.28             | 0.32               | 6.6        | 4.76      | 7.99   | 1    | 3      | 5' Type R Sump Inlet |
| 3               | C, 70% K            | 1.73             | 2.07               | 12.7       | 3.77      | 6.34   | 7    | 13     | 24" RCP Stub         |
| 4               | A, B, C, 70% K      | 3.02             | 3.55               | 12.8       | 3.76      | 6.32   | 11   | 22     | Concrete Forebay     |

## PROJECT INFORMATION

PROJECT: Bent Grass Commercial  
 PROJECT NO: 21814-01CSCV  
 DESIGN BY: CGH  
 REV. BY: KGV  
 AGENCY: El Paso County  
 REPORT TYPE: Final  
 DATE: 9/18/2026



Drexel, Barrell & Co.

|                           | C2* | C5*  | C10* | C100* | % IMPERV |
|---------------------------|-----|------|------|-------|----------|
| Roofs                     |     | 0.73 |      | 0.81  | 90       |
| Lawns/Landscaping         |     | 0.08 |      | 0.35  | 0        |
| Business-Commercial Areas |     | 0.81 |      | 0.88  | 95       |
| Drive and Walks           |     | 0.90 |      | 0.96  | 100      |

\*C-Values and Basin Imperviousness based on Table 6-6, City of Colorado Springs Drainage Criteria Manual

| SUB-BASIN | SURFACE DESIGNATION       |  | AREA<br>ACRE | COMPOSITE RUNOFF COEFFICIENTS |      |     |      | % IMPERV |
|-----------|---------------------------|--|--------------|-------------------------------|------|-----|------|----------|
|           |                           |  |              | C2                            | C5   | C10 | C100 |          |
| DEVELOPED |                           |  |              |                               |      |     |      |          |
| OS1       | Roofs                     |  | 0.00         |                               | 0.73 |     | 0.81 | 90       |
|           | Lawns/Landscaping         |  | 0.42         |                               | 0.08 |     | 0.35 | 0        |
|           | Business-Commercial Areas |  | 0.00         |                               | 0.81 |     | 0.88 | 95       |
|           | Drive and Walks           |  | 0.00         |                               | 0.90 |     | 0.96 | 100      |
| OS1 TOTAL | WEIGHTED AVERAGE          |  | 0.42         |                               | 0.08 |     | 0.35 | 0        |
| A         | Roofs                     |  | 0.00         |                               | 0.73 |     | 0.81 | 90       |
|           | Lawns/Landscaping         |  | 0.07         |                               | 0.08 |     | 0.35 | 0        |
|           | Business-Commercial Areas |  | 0.00         |                               | 0.81 |     | 0.88 | 95       |
|           | Drive and Walks           |  | 0.21         |                               | 0.90 |     | 0.96 | 100      |
| A TOTAL   | WEIGHTED AVERAGE          |  | 0.28         |                               | 0.70 |     | 0.81 | 76       |
| B         | Roofs                     |  | 0.23         |                               | 0.73 |     | 0.81 | 90       |
|           | Lawns/Landscaping         |  | 0.00         |                               | 0.08 |     | 0.35 | 0        |
|           | Business-Commercial Areas |  | 0.00         |                               | 0.81 |     | 0.88 | 95       |
|           | Drive and Walks           |  | 0.00         |                               | 0.90 |     | 0.96 | 100      |
| B TOTAL   | WEIGHTED AVERAGE          |  | 0.23         |                               | 0.73 |     | 0.81 | 90       |
| C         | Roofs                     |  | 0.00         |                               | 0.73 |     | 0.81 | 90       |
|           | Lawns/Landscaping         |  | 0.08         |                               | 0.08 |     | 0.35 | 0        |
|           | Business-Commercial Areas |  | 0.00         |                               | 0.81 |     | 0.88 | 95       |
|           | Drive and Walks           |  | 0.36         |                               | 0.90 |     | 0.96 | 100      |
| C TOTAL   | WEIGHTED AVERAGE          |  | 0.44         |                               | 0.76 |     | 0.86 | 83       |
| D         | Roofs                     |  | 0.00         |                               | 0.73 |     | 0.81 | 90       |
|           | Lawns/Landscaping         |  | 0.14         |                               | 0.08 |     | 0.35 | 0        |
|           | Business-Commercial Areas |  | 0.00         |                               | 0.81 |     | 0.88 | 95       |
|           | Drive and Walks           |  | 0.02         |                               | 0.90 |     | 0.96 | 100      |
| D TOTAL   | WEIGHTED AVERAGE          |  | 0.15         |                               | 0.17 |     | 0.42 | 11       |
| E         | Roofs                     |  | 0.00         |                               | 0.73 |     | 0.81 | 90       |
|           | Lawns/Landscaping         |  | 0.05         |                               | 0.08 |     | 0.35 | 0        |

## PROJECT INFORMATION

PROJECT: Bent Grass Commercial  
 PROJECT NO: 21814-01CSCV  
 DESIGN BY: CGH  
 REV. BY: KGV  
 AGENCY: El Paso County  
 REPORT TYPE: Final  
 DATE: 9/18/2026



Drexel, Barrell & Co.

|                           | C2* | C5*  | C10* | C100* | % IMPERV |
|---------------------------|-----|------|------|-------|----------|
| Roofs                     |     | 0.73 |      | 0.81  | 90       |
| Lawns/Landscaping         |     | 0.08 |      | 0.35  | 0        |
| Business-Commercial Areas |     | 0.81 |      | 0.88  | 95       |
| Drive and Walks           |     | 0.90 |      | 0.96  | 100      |

\*C-Values and Basin Imperviousness based on Table 6-6, City of Colorado Springs Drainage Criteria Manual

| SUB-BASIN    | SURFACE DESIGNATION       |  | AREA<br>ACRE | COMPOSITE RUNOFF COEFFICIENTS |      |     |      | % IMPERV |
|--------------|---------------------------|--|--------------|-------------------------------|------|-----|------|----------|
|              |                           |  |              | C2                            | C5   | C10 | C100 |          |
| DEVELOPED    |                           |  |              |                               |      |     |      |          |
|              | Business-Commercial Areas |  | 0.00         |                               | 0.81 |     | 0.88 | 95       |
|              | Drive and Walks           |  | 0.02         |                               | 0.90 |     | 0.96 | 100      |
| E TOTAL      | WEIGHTED AVERAGE          |  | 0.07         |                               | 0.29 |     | 0.51 | 26       |
| F            | Roofs                     |  | 0.00         |                               | 0.73 |     | 0.81 | 90       |
|              | Lawns/Landscaping         |  | 0.00         |                               | 0.08 |     | 0.35 | 0        |
|              | Business-Commercial Areas |  | 0.00         |                               | 0.81 |     | 0.88 | 95       |
|              | Drive and Walks           |  | 0.02         |                               | 0.90 |     | 0.96 | 100      |
| F TOTAL      | WEIGHTED AVERAGE          |  | 0.02         |                               | 0.90 |     | 0.96 | 100      |
| G            | Roofs                     |  | 0.00         |                               | 0.73 |     | 0.81 | 90       |
|              | Lawns/Landscaping         |  | 0.07         |                               | 0.08 |     | 0.35 | 0        |
|              | Business-Commercial Areas |  | 0.49         |                               | 0.81 |     | 0.88 | 95       |
|              | Drive and Walks           |  | 0.00         |                               | 0.90 |     | 0.96 | 100      |
| G TOTAL      | WEIGHTED AVERAGE          |  | 0.57         |                               | 0.71 |     | 0.81 | 83       |
| H            | Roofs                     |  | 0.00         |                               | 0.73 |     | 0.81 | 90       |
|              | Lawns/Landscaping         |  | 0.17         |                               | 0.08 |     | 0.35 | 0        |
|              | Business-Commercial Areas |  | 0.67         |                               | 0.81 |     | 0.88 | 95       |
|              | Drive and Walks           |  | 0.01         |                               | 0.90 |     | 0.96 | 100      |
| H TOTAL      | WEIGHTED AVERAGE          |  | 0.84         |                               | 0.67 |     | 0.78 | 76       |
|              |                           |  |              |                               |      |     |      |          |
| TOTALS (A-H) |                           |  | 2.60         |                               | 0.66 |     | 0.78 | 74.9     |

## PROJECT INFORMATION

PROJECT: Bent Grass Commercial  
 PROJECT NO: 21814-01CSCV  
 DESIGN BY: CGH  
 REV. BY: KGV  
 AGENCY: El Paso County  
 REPORT TYPE: Final  
 DATE: 9/18/2026



## RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

DEVELOPED TIME OF CONCENTRATION STANDARD FORM SF-2

| SUB-BASIN DATA |            |                |                  |      | CA   |      | INITIAL/OVERLAND TIME (t <sub>i</sub> ) |       |                | TRAVEL TIME (t <sub>t</sub> ) |       |      |                | TIME OF CONC. t <sub>c</sub> |                | FINAL t <sub>c</sub> |
|----------------|------------|----------------|------------------|------|------|------|-----------------------------------------|-------|----------------|-------------------------------|-------|------|----------------|------------------------------|----------------|----------------------|
| BASIN          | DESIGN PT. | C <sub>s</sub> | C <sub>100</sub> | AREA | 5    | 100  | LENGTH                                  | SLOPE | t <sub>i</sub> | LENGTH                        | SLOPE | VEL. | t <sub>t</sub> | COMP.                        | MINIMUM        |                      |
|                |            |                |                  | Ac   |      |      | Ft                                      | %     | Min            | Ft                            | %     | FPS  | Min            | t <sub>c</sub>               | t <sub>c</sub> | Min                  |
| DEVELOPED      |            |                |                  |      |      |      |                                         |       |                |                               |       |      |                |                              |                |                      |
| OS1            | OS1        | 0.08           | 0.35             | 0.42 | 0.03 | 0.15 | 100                                     | 3.7   | 12.3           | 256                           | 2.7   | 8.5  | 0.5            | 12.8                         | 5.0            | 12.8                 |
| A              | 1          | 0.70           | 0.81             | 0.28 | 0.19 | 0.23 | 50                                      | 4.4   | 3.2            | 180                           | 2.4   | 9.0  | 0.3            | 3.5                          | 5.0            | 5.0                  |
| B              | 2          | 0.73           | 0.81             | 0.23 | 0.17 | 0.19 | 70                                      | 0.5   | 7.3            | 70                            | 0.5   | 5.5  | 0.2            | 7.5                          | 5.0            | 7.5                  |
| C              | 3          | 0.76           | 0.86             | 0.44 | 0.33 | 0.38 | 50                                      | 3.8   | 2.9            | 290                           | 2.2   | 8.6  | 0.6            | 3.5                          | 5.0            | 5.0                  |
| DP1+DP2+DP3    | 3A         | 0.74           | 0.83             | 0.95 | 0.70 | 0.79 | From DP-2                               |       |                | 65                            | 1.0   | 7.2  | 0.2            | 7.7                          | 5.0            | 7.7                  |
| D              | 4          | 0.17           | 0.42             | 0.15 | 0.03 | 0.06 | 65                                      | 1.6   | 12.0           | 65                            | 5.8   | 11.3 | 0.1            | 12.1                         | 5.0            | 12.1                 |
| E              | 5          | 0.29           | 0.51             | 0.07 | 0.02 | 0.04 | 25                                      | 2.0   | 6.0            | 75                            | 4.4   | 12.2 | 0.1            | 6.1                          | 5.0            | 6.1                  |
| F              | 6          | 0.90           | 0.96             | 0.02 | 0.01 | 0.02 | 20                                      | 2.0   | 1.3            | 20                            | 3.0   | 13.5 | 0.0            | 1.4                          | 5.0            | 5.0                  |
| G              | 7          | 0.71           | 0.81             | 0.57 | 0.40 | 0.46 | 100                                     | 7.6   | 3.7            | 105                           | 2.7   | 7.0  | 0.3            | 3.9                          | 5.0            | 5.0                  |
| H              |            | 0.67           | 0.78             | 0.84 | 0.56 | 0.66 | 100                                     | 5.0   | 4.7            | 508                           | 2.7   | 7.0  | 1.2            | 5.9                          | 5.0            | 5.9                  |
| DPOS1+H        | 8          | 0.47           | 0.63             | 1.27 | 0.60 | 0.80 | From Basin H                            |       |                | 45                            | 6.9   | 7.0  | 0.1            | 12.9                         | 5.0            | 12.9                 |

## PROJECT INFORMATION

PROJECT: Bent Grass Commercial  
 PROJECT NO: 21814-01CSCV  
 DESIGN BY: CGH  
 REV. BY: KGV  
 AGENCY: El Paso County  
 REPORT TYPE: Final  
 DATE: 9/18/2026



## RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

DEVELOPED RUNOFF 5 YR STORM P1= **1.50**

| BASIN (S)   | DIRECT RUNOFF |           |              |                      |       |           |         |
|-------------|---------------|-----------|--------------|----------------------|-------|-----------|---------|
|             | DESIGN POINT  | AREA (AC) | RUNOFF COEFF | t <sub>c</sub> (MIN) | C * A | I (IN/HR) | Q (CFS) |
| DEVELOPED   |               |           |              |                      |       |           |         |
| OS1         | OS1           | 0.42      | 0.08         | 12.8                 | 0.03  | 3.66      | 0.1     |
| A           | 1             | 0.28      | 0.70         | 5.0                  | 0.19  | 5.09      | 1.0     |
| B           | 2             | 0.23      | 0.73         | 7.5                  | 0.17  | 4.51      | 0.8     |
| C           | 3             | 0.44      | 0.76         | 5.0                  | 0.33  | 5.09      | 1.7     |
| DP1+DP2+DP3 | 3A            | 0.95      | 0.74         | 7.7                  | 0.70  | 4.48      | 3.1     |
| D           | 4             | 0.15      | 0.17         | 12.1                 | 0.03  | 3.75      | 0.1     |
| E           | 5             | 0.07      | 0.29         | 6.1                  | 0.02  | 4.81      | 0.1     |
| F           | 6             | 0.02      | 0.90         | 5.0                  | 0.01  | 5.09      | 0.07    |
| G           | 7             | 0.57      | 0.71         | 5.0                  | 0.40  | 5.09      | 2.1     |
| H           |               | 0.84      | 0.67         | 5.9                  | 0.56  | 4.85      | 2.7     |
| DPOS1+H     | 8             | 1.27      | 0.47         | 12.9                 | 0.60  | 3.65      | 2.2     |

## PROJECT INFORMATION

PROJECT: Bent Grass Commercial  
 PROJECT NO: 21814-01CSCV  
 DESIGN BY: CGH  
 REV. BY: KGV  
 AGENCY: El Paso County  
 REPORT TYPE: Final  
 DATE: 9/18/2026

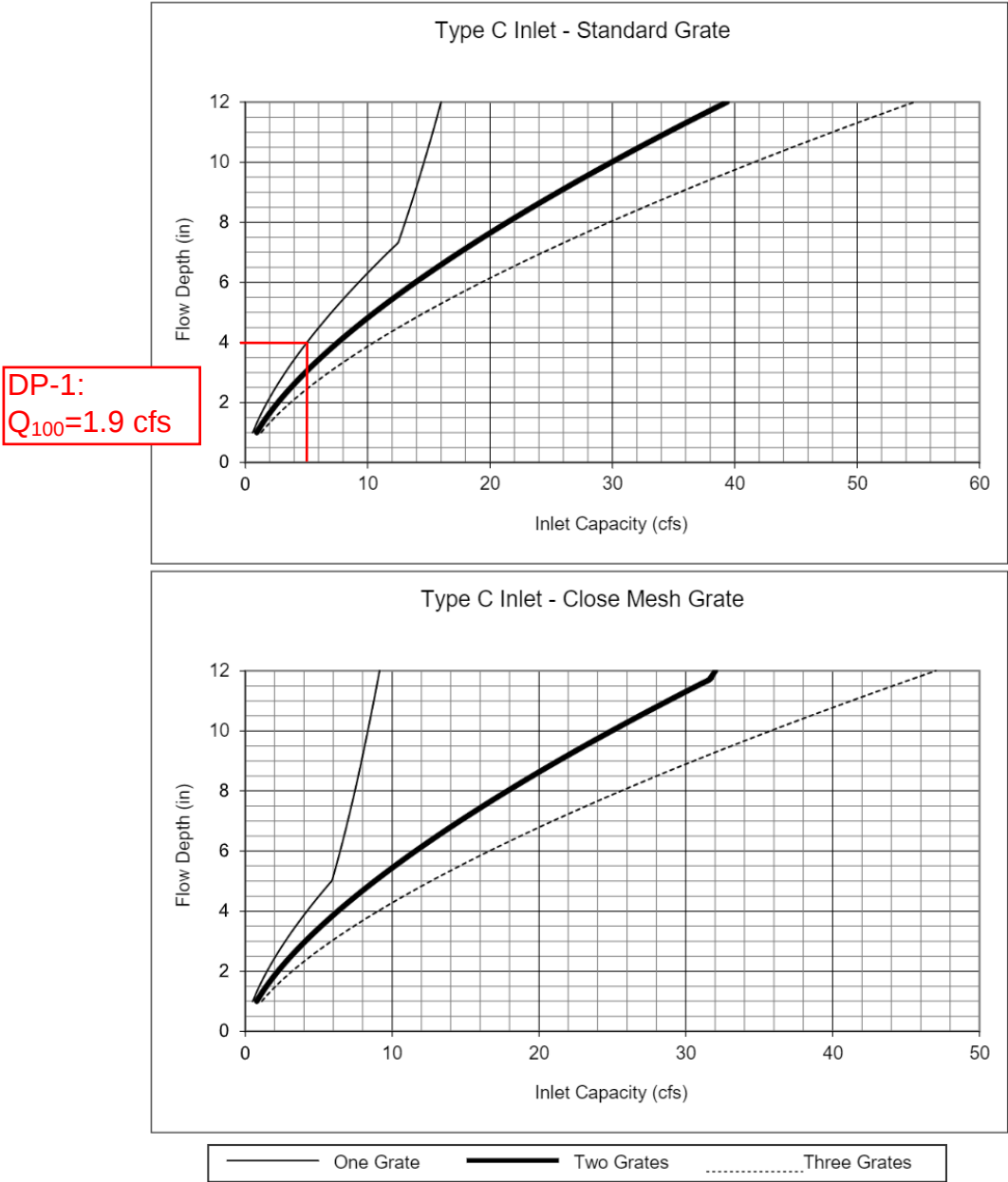


## RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

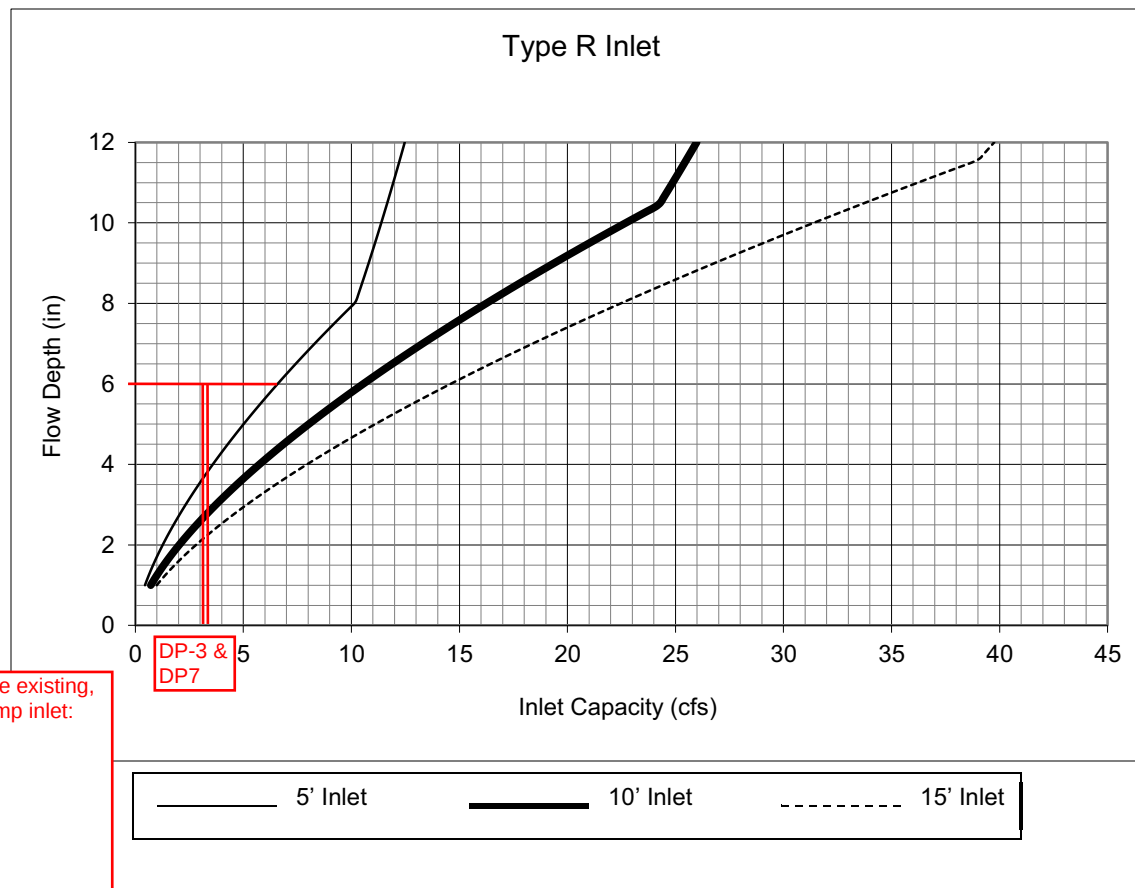
DEVELOPED RUNOFF 100 YR STORM P1= 2.52

| BASIN (S)   | DIRECT RUNOFF |           |              |                      |       |           |         |
|-------------|---------------|-----------|--------------|----------------------|-------|-----------|---------|
|             | DESIGN POINT  | AREA (AC) | RUNOFF COEFF | t <sub>c</sub> (MIN) | C * A | I (IN/HR) | Q (CFS) |
| DEVELOPED   |               |           |              |                      |       |           |         |
| OS1         | OS1           | 0.42      | 0.35         | 12.8                 | 0.15  | 6.15      | 0.9     |
| A           | 1             | 0.28      | 0.81         | 5.0                  | 0.23  | 8.55      | 1.9     |
| B           | 2             | 0.23      | 0.81         | 7.5                  | 0.19  | 7.57      | 1.4     |
| C           | 3             | 0.44      | 0.86         | 5.0                  | 0.38  | 8.55      | 3.2     |
| DP1+DP2+DP3 | 3A            | 0.95      | 0.83         | 7.7                  | 0.79  | 7.52      | 5.9     |
| D           | 4             | 0.15      | 0.42         | 12.1                 | 0.06  | 6.31      | 0.4     |
| E           | 5             | 0.07      | 0.51         | 6.1                  | 0.04  | 8.09      | 0.3     |
| F           | 6             | 0.02      | 0.96         | 5.0                  | 0.02  | 8.55      | 0.1     |
| G           | 7             | 0.57      | 0.81         | 5.0                  | 0.46  | 8.55      | 3.9     |
| H           |               | 0.84      | 0.78         | 5.9                  | 0.66  | 8.15      | 5.3     |
| DPOS1+H     | 8             | 1.27      | 0.63         | 12.9                 | 0.80  | 6.13      | 4.9     |

Figure 8-10. Inlet Capacity Chart Sump Conditions, Area (Type C) Inlet



Notes:  
1. The standard inlet parameters must apply to use these charts.

**Figure 8-11. Inlet Capacity Chart Sump Conditions , Curb Opening (Type R) Inlet****Notes:**

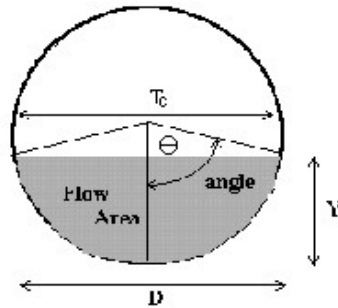
1. The standard inlet parameters must apply to use this chart.

# CIRCULAR CONDUIT FLOW (Normal & Critical Depth Computation)

MHFD-Culvert, Version 4.00 (May 2020)

Project: **BENT GRASS EAST COMMERCIAL FILING NO. 3**

Pipe ID: **24" RCP (DP-1 & DP-2)**



## Design Information (Input)

|                        |         |        |        |
|------------------------|---------|--------|--------|
| Pipe Invert Slope      | $S_o =$ | 0.0100 | ft/ft  |
| Pipe Manning's n-value | $n =$   | 0.0130 |        |
| Pipe Diameter          | $D =$   | 24.00  | inches |
| Design discharge       | $Q =$   | 10.00  | cfs    |

New Runoff Rates from site with this development will be:

$Q(5) = 5.07$  cfs

$Q(100) = 10.1$  cfs

## Full-Flow Capacity (Calculated)

|                            |            |       |         |
|----------------------------|------------|-------|---------|
| Full-flow area             | $A_f =$    | 3.14  | sq ft   |
| Full-flow wetted perimeter | $P_f =$    | 6.28  | ft      |
| Half Central Angle         | $\theta =$ | 3.14  | radians |
| Full-flow capacity         | $Q_f =$    | 22.68 | cfs     |

Existing Designed runoff rates are 10 cfs, with the modification of flows from the site (DP6), pipe capacity will still be well below the maximum capacity of 22.68 cfs

## Calculation of Normal Flow Condition

|                                            |                 |       |               |
|--------------------------------------------|-----------------|-------|---------------|
| Half Central Angle ( $0 < \theta < 3.14$ ) | $\theta =$      | 1.50  | radians       |
| Flow area                                  | $A_n =$         | 1.43  | sq ft         |
| Top width                                  | $T_n =$         | 2.00  | ft            |
| Wetted perimeter                           | $P_n =$         | 3.00  | ft            |
| Flow depth                                 | $Y_n =$         | 0.93  | ft            |
| Flow velocity                              | $V_n =$         | 6.99  | fps           |
| Discharge                                  | $Q_n =$         | 10.00 | cfs           |
| Percent of Full Flow                       | $\text{Flow} =$ | 44.1% | of full flow  |
| Normal Depth Froude Number                 | $Fr_n =$        | 1.46  | supercritical |

## Calculation of Critical Flow Condition

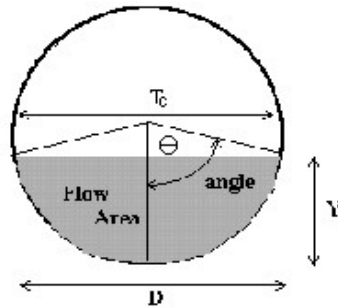
|                                              |              |      |         |
|----------------------------------------------|--------------|------|---------|
| Half Central Angle ( $0 < \theta_c < 3.14$ ) | $\theta_c =$ | 1.70 | radians |
| Critical flow area                           | $A_c =$      | 1.83 | sq ft   |
| Critical top width                           | $T_c =$      | 1.98 | ft      |
| Critical flow depth                          | $Y_c =$      | 1.13 | ft      |
| Critical flow velocity                       | $V_c =$      | 5.46 | fps     |
| Critical Depth Froude Number                 | $Fr_c =$     | 1.00 |         |

# CIRCULAR CONDUIT FLOW (Normal & Critical Depth Computation)

MHFD-Culvert, Version 4.00 (May 2020)

Project: **BENT GRASS EAST COMMERCIAL FILING NO. 3**

Pipe ID: **24" RCP (DP-3)**



## Design Information (Input)

|                        |         |        |        |
|------------------------|---------|--------|--------|
| Pipe Invert Slope      | $S_o =$ | 0.0100 | ft/ft  |
| Pipe Manning's n-value | $n =$   | 0.0130 |        |
| Pipe Diameter          | $D =$   | 24.00  | inches |
| Design discharge       | $Q =$   | 13.00  | cfs    |

New Runoff Rates from site with this development will be:

$Q(5) = 6.9$  cfs

$Q(100) = 11.4$  cfs

## Full-Flow Capacity (Calculated)

|                            |                  |       |         |
|----------------------------|------------------|-------|---------|
| Full-flow area             | $A_f =$          | 3.14  | sq ft   |
| Full-flow wetted perimeter | $P_f =$          | 6.28  | ft      |
| Half Central Angle         | $\text{Theta} =$ | 3.14  | radians |
| Full-flow capacity         | $Q_f =$          | 22.68 | cfs     |

Existing Designed runoff rates are 13 cfs, with the modification of flows from the site (DP1 & DP2), pipe capacity will still be well below the maximum capacity of 22.68 cfs

## Calculation of Normal Flow Condition

|                                                  |                  |       |               |
|--------------------------------------------------|------------------|-------|---------------|
| Half Central Angle ( $0 < \text{Theta} < 3.14$ ) | $\text{Theta} =$ | 1.66  | radians       |
| Flow area                                        | $A_n =$          | 1.74  | sq ft         |
| Top width                                        | $T_n =$          | 1.99  | ft            |
| Wetted perimeter                                 | $P_n =$          | 3.31  | ft            |
| Flow depth                                       | $Y_n =$          | 1.09  | ft            |
| Flow velocity                                    | $V_n =$          | 7.47  | fps           |
| Discharge                                        | $Q_n =$          | 13.00 | cfs           |
| Percent of Full Flow                             | $\text{Flow} =$  | 57.3% | of full flow  |
| Normal Depth Froude Number                       | $Fr_n =$         | 1.41  | supercritical |

## Calculation of Critical Flow Condition

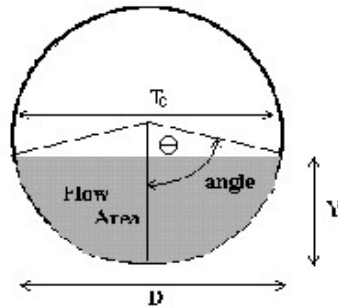
|                                                    |                    |      |         |
|----------------------------------------------------|--------------------|------|---------|
| Half Central Angle ( $0 < \text{Theta-c} < 3.14$ ) | $\text{Theta-c} =$ | 1.87 | radians |
| Critical flow area                                 | $A_c =$            | 2.16 | sq ft   |
| Critical top width                                 | $T_c =$            | 1.91 | ft      |
| Critical flow depth                                | $Y_c =$            | 1.30 | ft      |
| Critical flow velocity                             | $V_c =$            | 6.03 | fps     |
| Critical Depth Froude Number                       | $Fr_c =$           | 1.00 |         |

# CIRCULAR CONDUIT FLOW (Normal & Critical Depth Computation)

MHFD-Culvert, Version 4.00 (May 2020)

Project: **BENT GRASS EAST COMMERCIAL FILING NO. 3**

Pipe ID: **30" RCP (DP-4)**



## Design Information (Input)

|                        |         |        |        |
|------------------------|---------|--------|--------|
| Pipe Invert Slope      | $S_o =$ | 0.0100 | ft/ft  |
| Pipe Manning's n-value | $n =$   | 0.0130 |        |
| Pipe Diameter          | $D =$   | 30.00  | inches |
| Design discharge       | $Q =$   | 22.00  | cfs    |

New Runoff Rates from site with this development will be:

$Q(5) = 8.2$  cfs

$Q(100) = 16.0$  cfs

## Full-Flow Capacity (Calculated)

|                            |            |       |         |
|----------------------------|------------|-------|---------|
| Full-flow area             | $A_f =$    | 4.91  | sq ft   |
| Full-flow wetted perimeter | $P_f =$    | 7.85  | ft      |
| Half Central Angle         | $\theta =$ | 3.14  | radians |
| Full-flow capacity         | $Q_f =$    | 41.13 | cfs     |

Existing Designed runoff rates are 22 cfs, with the modification of flows from the site (DP3A), pipe capacity will still be well below the maximum capacity of 41.13 cfs

## Calculation of Normal Flow Condition

|                                            |                 |       |               |
|--------------------------------------------|-----------------|-------|---------------|
| Half Central Angle ( $0 < \theta < 3.14$ ) | $\theta =$      | 1.61  | radians       |
| Flow area                                  | $A_n =$         | 2.58  | sq ft         |
| Top width                                  | $T_n =$         | 2.50  | ft            |
| Wetted perimeter                           | $P_n =$         | 4.03  | ft            |
| Flow depth                                 | $Y_n =$         | 1.30  | ft            |
| Flow velocity                              | $V_n =$         | 8.52  | fps           |
| Discharge                                  | $Q_n =$         | 22.00 | cfs           |
| Percent of Full Flow                       | $\text{Flow} =$ | 53.5% | of full flow  |
| Normal Depth Froude Number                 | $Fr_n =$        | 1.48  | supercritical |

## Calculation of Critical Flow Condition

|                                              |              |      |         |
|----------------------------------------------|--------------|------|---------|
| Half Central Angle ( $0 < \theta_c < 3.14$ ) | $\theta_c =$ | 1.85 | radians |
| Critical flow area                           | $A_c =$      | 3.31 | sq ft   |
| Critical top width                           | $T_c =$      | 2.40 | ft      |
| Critical flow depth                          | $Y_c =$      | 1.59 | ft      |
| Critical flow velocity                       | $V_c =$      | 6.66 | fps     |
| Critical Depth Froude Number                 | $Fr_c =$     | 1.00 |         |

# Design Procedure Form: Extended Detention Basin (EDB)

UD-BMP (Version 3.07, March 2018)

Sheet 1 of 3

Designer: **Marc A. Whorton, P.E.**  
 Company: **Classic Consulting**  
 Date: **November 4, 2020**  
 Project: **Bent Grass East Commercial Filing No. 3**  
 Location: **Exist. Pond - Proposed Forebay for Commercial Development**

## 1. Basin Storage Volume

- A) Effective Imperviousness of Tributary Area,  $I_a$
- B) Tributary Area's Imperviousness Ratio ( $i = I_a / 100$ )
- C) Contributing Watershed Area
- D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm
- E) Design Concept  
(Select EURV when also designing for flood control)
- F) Design Volume (WQCV) Based on 40-hour Drain Time  
( $V_{DESIGN} = (1.0 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i) / 12 * Area)$ )
- G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume  
( $V_{WQCV\ OTHER} = (d_6 * (V_{DESIGN} / 0.43))$ )
- H) User Input of Water Quality Capture Volume (WQCV) Design Volume  
(Only if a different WQCV Design Volume is desired)
- I) NRCS Hydrologic Soil Groups of Tributary Watershed  
 i) Percentage of Watershed consisting of Type A Soils  
 ii) Percentage of Watershed consisting of Type B Soils  
 iii) Percentage of Watershed consisting of Type C/D Soils
- J) Excess Urban Runoff Volume (EURV) Design Volume  
 For HSG A:  $EURV_A = 1.68 * i^{1.28}$   
 For HSG B:  $EURV_B = 1.36 * i^{1.08}$   
 For HSG C/D:  $EURV_{C/D} = 1.20 * i^{1.08}$
- K) User Input of Excess Urban Runoff Volume (EURV) Design Volume  
(Only if a different EURV Design Volume is desired)

$I_a = 90.0$  %

$i = 0.900$

Area = 4.500 ac

$d_6 = 0.42$  in

Choose One

- ☐ Water Quality Capture Volume (WQCV)  
☒ Excess Urban Runoff Volume (EURV)

$V_{DESIGN} =$  ac-ft

$V_{DESIGN\ OTHER} = 0.147$  ac-ft

$V_{DESIGN\ USER} =$  ac-ft

HSG A = 0 %

HSG B = 100 %

HSG C/D = 0 %

$EURV_{DESIGN} = 0.455$  ac-ft

$EURV_{DESIGN\ USER} =$  ac-ft

## 2. Basin Shape: Length to Width Ratio

(A basin length to width ratio of at least 2:1 will improve TSS reduction.)

L : W = 2.0 : 1

## 3. Basin Side Slopes

- A) Basin Maximum Side Slopes  
(Horizontal distance per unit vertical, 4:1 or flatter preferred)

Z = 4.00 ft / ft

## 4. Inlet

- A) Describe means of providing energy dissipation at concentrated inflow locations:

## 5. Forebay

- A) Minimum Forebay Volume  
( $V_{FMIN} = 2\%$  of the WQCV)

- B) Actual Forebay Volume

- C) Forebay Depth  
( $D_F = 18$  inch maximum)

- D) Forebay Discharge

- i) Undetained 100-year Peak Discharge

- ii) Forebay Discharge Design Flow  
( $Q_F = 0.02 * Q_{100}$ )

- E) Forebay Discharge Design

- F) Discharge Pipe Size (minimum 8-inches)

- G) Rectangular Notch Width

$V_{FMIN} = 0.003$  ac-ft

$V_F = 0.003$  ac-ft

$D_F = 18.0$  in

$Q_{100} = 22.00$  cfs

$Q_F = 0.44$  cfs

Choose One

- ☐ Berm With Pipe  
☒ Wall with Rect. Notch  
☐ Wall with V-Notch Weir

Calculated  $D_P =$  in

Calculated  $W_N = 4.5$  in

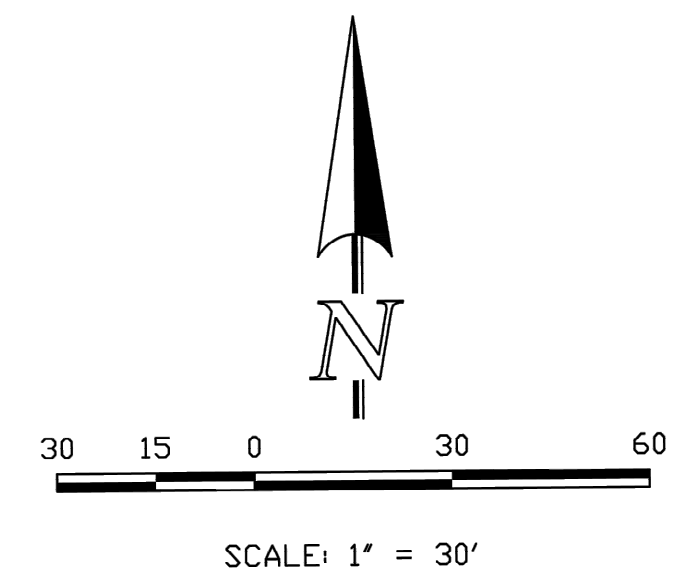
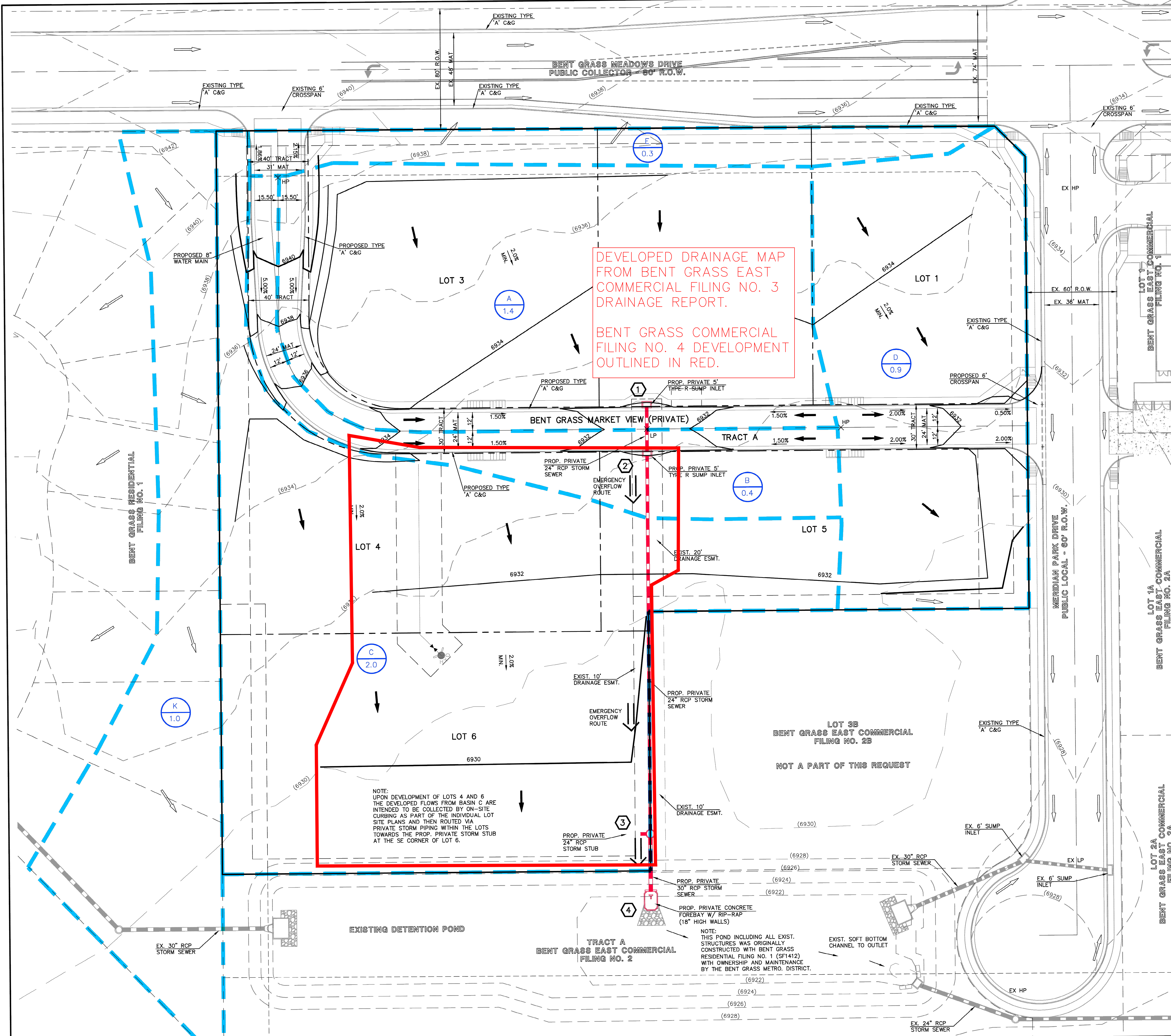
New Runoff Rates from site with this development will be:

Q(5) = 8.2 cfs

Q(100) = 16.0 cfs

Existing Designed runoff rates are 22 cfs, with the modification of flows from the site (DP3A), pond capacity will be well below the current rates of 22 cfs

Flow too small for berm w/ pipe



**LEGEND**

| DESCRIPTION                  | SYMBOL |
|------------------------------|--------|
| PROPOSED CONTOUR-10          |        |
| PROPOSED CONTOUR-2           |        |
| EXISTING CONTOUR-10          |        |
| EXISTING CONTOUR-2           |        |
| PROPERTY LINE                |        |
| BOUNDARY LINE                |        |
| PROPOSED PRIVATE STORM SEWER |        |
| PROPOSED PRIVATE STORM INLET |        |
| EXISTING STORM SEWER         |        |
| EXISTING STORM INLET         |        |
| EXISTING FLOW DIRECTION      |        |
| PROPOSED FLOW                |        |
| HIGH POINT                   | HP     |
| LOW POINT                    | LP     |
| OVERFLOW ROUTE               |        |
| BASIN IDENTIFIER             |        |
| AREA IN ACRES                |        |
| DESIGN POINT                 |        |
| DRAINAGE BASIN               |        |

| FINAL DRAINAGE REPORT ~ BASIN RUNOFF SUMMARY |                 |          |       |         |          |             |             |                       |             |           |                |          |           |              |              |                |            |            |              |
|----------------------------------------------|-----------------|----------|-------|---------|----------|-------------|-------------|-----------------------|-------------|-----------|----------------|----------|-----------|--------------|--------------|----------------|------------|------------|--------------|
| BASIN                                        | TOTAL AREA (AC) | WEIGHTED |       |         | OVERLAND |             |             | STREET / CHANNEL FLOW |             |           |                | Tc (min) | INTENSITY |              |              | TOTAL FLOWS    |            |            |              |
|                                              |                 | CA(2)    | CA(5) | CA(100) | C(5)     | Length (ft) | Height (ft) | Tc (min)              | Length (ft) | Slope (%) | Velocity (fps) |          | Tc (min)  | I(2) (in/hr) | I(5) (in/hr) | I(100) (in/hr) | Q(2) (cfs) | Q(5) (cfs) | Q(100) (cfs) |
| A                                            | 1.4             | 0.99     | 1.01  | 1.16    | 0.08     | 30          | 2           | 5.4                   | 250         | 2.0%      | 1.0            | 4.2      | 9.6       | 3.34         | 4.19         | 7.93           | 1          | 4          | 8            |
| B                                            | 0.4             | 0.27     | 0.28  | 0.32    | 0.08     | 10          | 0.2         | 4.6                   | 100         | 1.5%      | 0.9            | 1.9      | 6.6       | 1.75         | 4.76         | 7.99           | 1          | 1          | 3            |
| C                                            | 2.6             | 1.43     | 1.47  | 1.68    | 0.08     | 36          | 1.6         | 5.9                   | 400         | 2.0%      | 1.0            | 8.7      | 12.7      | 3.01         | 3.97         | 6.34           | 4          | 5          | 11           |
| D                                            | 0.9             | 0.67     | 0.69  | 0.77    | 0.08     | 10          | 0.2         | 4.6                   | 200         | 2.0%      | 1.4            | 2.4      | 7.0       | 1.72         | 4.07         | 7.83           | 3          | 3          | 6            |
| E                                            | 0.3             | 0.05     | 0.07  | 0.14    | 0.08     | 25          | 0.5         | 7.3                   |             |           |                |          | 7.3       | 13.67        | 4.60         | 7.72           | 0.2        | 0.3        | 1.0          |
| K                                            | 1.0             | 0.32     | 0.37  | 0.56    | 0.08     | 85          | 3           | 9.0                   |             |           |                |          | 9.0       | 3.43         | 4.29         | 7.21           | 1          | 2          | 4            |

| FINAL DRAINAGE REPORT - SURFACE ROUTING SUMMARY |                     |                  |                    |            |           |        |      |        |                      |
|-------------------------------------------------|---------------------|------------------|--------------------|------------|-----------|--------|------|--------|----------------------|
| Design Point(s)                                 | Contributing Basins | Equivalent CA(5) | Equivalent CA(100) | Maximum Tc | Intensity |        |      | Flow   |                      |
|                                                 |                     |                  |                    |            | I(5)      | I(100) | Q(5) | Q(100) | Outfall / Inlet Size |
| 1                                               | A                   | 1.01             | 1.16               | 9.6        | 4.19      | 7.03   | 4    | 8      | 8" Type R Sump Inlet |
| 2                                               | B                   | 0.28             | 0.32               | 6.6        | 4.76      | 7.99   | 1    | 3      | 8" Type R Sump Inlet |
| 3                                               | C, 70% K            | 1.73             | 2.07               | 12.7       | 3.77      | 6.34   | 7    | 13     | 24" RCP Sub          |
| 4                                               | A, B, C, 70% K      | 3.02             | 3.55               | 12.8       | 3.76      | 6.32   | 11   | 22     | Concrete Forebay     |

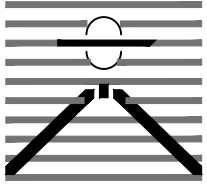
**BENT GRASS EAST COMMERCIAL FILING NO. 3**  
DEVELOPED DRAINAGE MAP

|             |     |              |                 |
|-------------|-----|--------------|-----------------|
| DESIGNED BY | MAW | SCALE        | DATE            |
| DRAWN BY    | MAW | (H) 1" = 30' | 10-27-20        |
| CHECKED BY  |     | (V) 1" = N/A | SHEET 1 OF 1    |
|             |     |              | JOB NO. 2177.64 |

619 N. Cascade Avenue, Suite 200  
Colorado Springs, Colorado 80903  
(719) 785-0790  
(719) 785-0799 (Fax)

CLASSIC CONSULTING

PREPARED BY:



DREXEL, BARRELL & CO.  
Engineers • Surveyors  
101 SAWATCH ST., SUITE 100  
COLORADO SPRGS, COLORADO 80903  
CONTACT: TIM D. MCCONNELL, P.E.  
(719)260-0887  
COLORADO SPRINGS • LAFAYETTE

CLIENT:

THE LEARNING  
EXPERIENCE  
210 HILLSBORO TECHNOLOGY DR.  
DEERFIELD BEACH, FL 33441

DRAINAGE PLANS FOR:

THE LEARNING  
EXPERIENCE  
11659 BENT GRASS MARKET VIEW  
FALCON, COLORADO

| ISSUE         | DATE            |
|---------------|-----------------|
| INITIAL ISSUE | 08/14/2026      |
| RESUBMITTAL   | 09/18/2026      |
|               |                 |
|               |                 |
|               |                 |
|               |                 |
| DESIGNED BY:  | KGV             |
| DRAWN BY:     | CGH             |
| CHECKED BY:   | KGV             |
| FILE NAME:    | 21814-01-DRN-PP |

PREPARED UNDER MY DIRECT  
SUPERVISION FOR AND ON BEHALF  
OF DREXEL, BARRELL & CO.

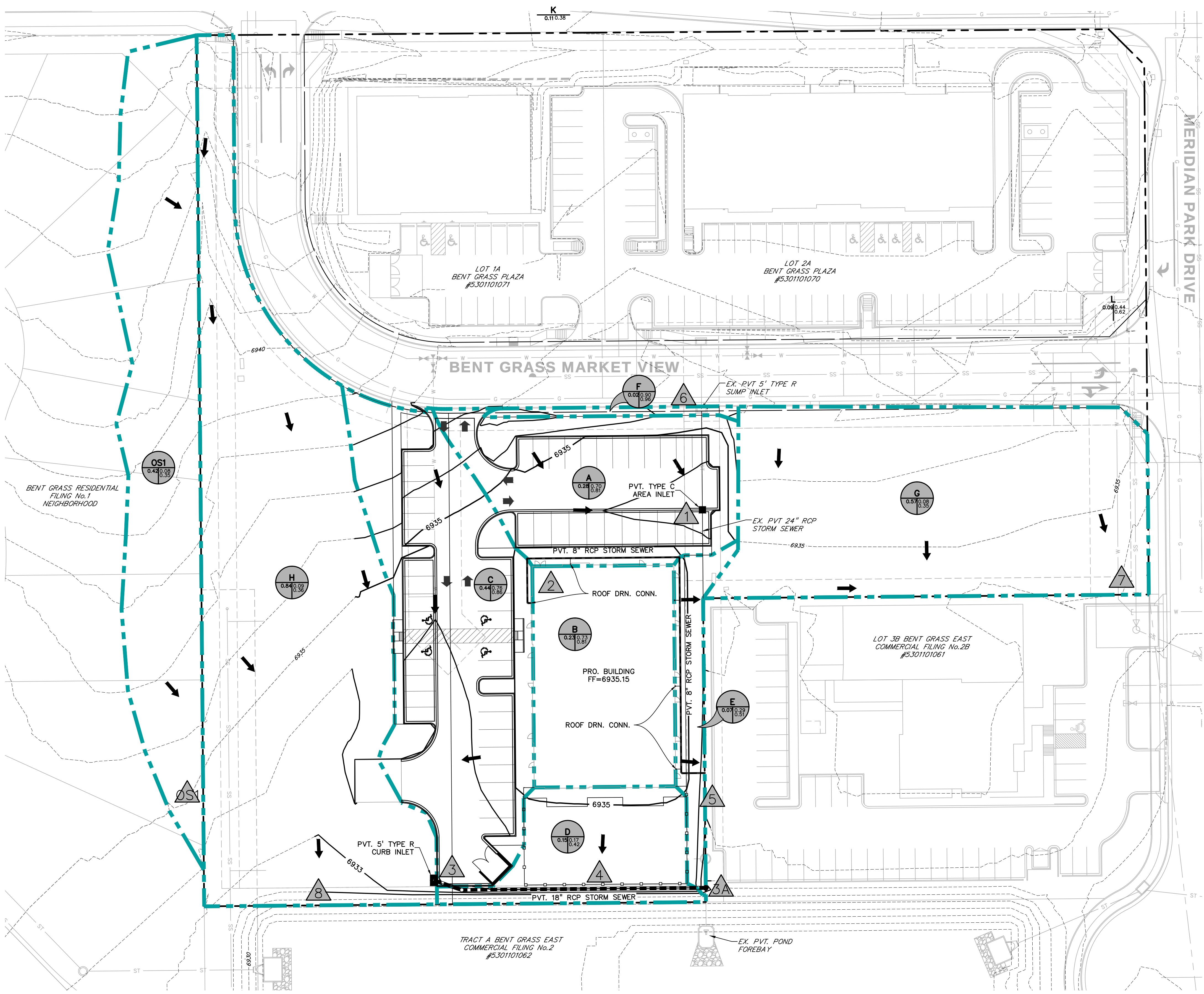
DRAWING SCALE:  
HORIZONTAL: 1" = 30"  
VERTICAL: N/A

PROPOSED  
DRAINAGE  
PLAN

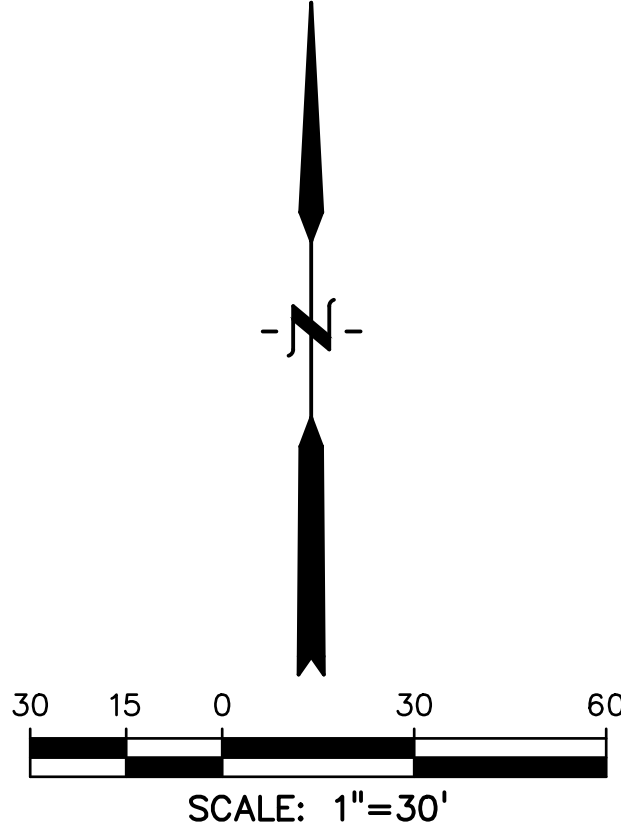
PROJECT NO. 21814-01CSCV  
DRAWING NO.

DRN

SHEET: 1 OF 1



SITE DRAINAGE PLAN



- LEGEND
- PROPOSED INTERMEDIATE CONTOUR..... 5522
  - PROPOSED INDEX CONTOUR..... 5520
  - EX. INTERMEDIATE CONTOUR..... 5364
  - EX. INDEX CONTOUR..... 5365
  - HIGH POINT..... HP
  - LOW POINT..... LP
  - PROPOSED INLET.....
  - PROPOSED MANHOLE.....

- BASIN BOUNDARY
- FLOW DIRECTION
- DESIGN POINT
- BASIN
- AREA (ACRE)
- AREA (C100)
- C5
- C100

| DEVELOPED   |     |            |                      |                        |  |
|-------------|-----|------------|----------------------|------------------------|--|
| BASIN       | DP  | Area (Ac.) | Q <sub>6</sub> (CFS) | Q <sub>100</sub> (CFS) |  |
| OS1         | OS1 | 0.42       | 0.1                  | 0.9                    |  |
| A           | 1   | 0.28       | 1.0                  | 1.9                    |  |
| B           | 2   | 0.23       | 0.8                  | 1.4                    |  |
| C           | 3   | 0.44       | 1.7                  | 3.2                    |  |
| DP1+DP2+DP3 | 3A  | 0.95       | 3.1                  | 5.9                    |  |
| D           | 4   | 0.15       | 0.1                  | 0.4                    |  |
| E           | 5   | 0.07       | 0.1                  | 0.3                    |  |
| F           | 6   | 0.02       | 0.07                 | 0.1                    |  |
| G           | 7   | 0.57       | 2.1                  | 3.9                    |  |
| H           |     | 0.84       | 2.7                  | 5.3                    |  |
| DPOS1+H     | 8   | 1.27       | 2.2                  | 4.9                    |  |