Please add PCD File No．MS227
Also please include the El Paso County standard signature block． See attached file．

PCD file number added．
El Paso County signature block added．

## RE：Space Village Filing No． 4 ／Traffic Memorandum El Paso County，Colorado

Dear David，
SM ROCHA，LLC is pleased to provide traffic generation information for the development entitled Space Village Filing No．4．This development is located near the southeast corner of Space Village Avenue and Peterson Boulevard in El Paso County，Colorado．

The intent of this analysis is to present traffic volumes likely generated by the proposed development and consider potential impacts to the adjacent roadway network．This analysis is also provided to address the capacity，geometric，and control requirements associated with the development in accordance with Section B．2．4．D of the County＇s Engineering Criteria Manual（ECM）${ }^{1}$ ．

The following is a summary of analysis results．

## Site Description and Access

Land for the development is currently vacant and surrounded by open space and a mix of military， commercial，industrial，and residential land uses．The proposed development is understood to entail the new construction of a contractor yard outdoor storage，on two lots totaling approximately 22.82 acres of land．

Proposed access to the development is provided via two full－movement accesses onto Space Village Avenue（referred to as Access A and Access B）．It is understood that Access A will provide sole access to Lot 1 （11．23 acres）and Access B will serve Lot 2 （11．59 acres）．

For purposed of this study，it is anticipated that development construction would be completed by end of Year 2024.

General site and access locations are shown on Figure 1.
A site plan，as prepared by Sterling Design Associates，is shown on Figure 2．This plan is provided for illustrative purposes only．
${ }^{1}$ El Paso County Engineering Criteria Manual，El Paso County，October 2020.

[^0]

## Existing and Background Traffic Volumes

Existing morning (AM) and afternoon (PM) peak hour traffic counts were collected at the Space Village Avenue intersections with Peterson Boulevard, the U.S. Highway 24 eastbound on/off ramps, Marksheffel Road, and the Storage Sense access drive. Average daily traffic (ADT) volumes were collected over a 24 -hour period on Space Village Avenue. Counts were collected on Wednesday, October 26, 2022, with AM peak hour counts being collected during the period of 7:00 a.m. to 9:00 a.m. and PM peak hour counts being collected during the period of 4:00 p.m. to 6:00 p.m. Traffic count data is included for reference in Attachment A.

It is important to note that at the time of count collection, the east leg of the Space Village Avenue and Marksheffel Road intersection was closed. Therefore, existing traffic volumes for Space Village Avenue, east of Marksheffel Road, were obtained from the Reagan Ranch Traffic Impact Study².

Background traffic is the traffic projected to be on area roadways without consideration of the proposed development. Background traffic includes traffic generated by the development of vacant parcels in the area.

To account for projected increases in background traffic by Year 2024, a compounded annual growth rate was determined using historical traffic data for the surrounding area provided by CDOT's Online Transportation Information System (OTIS), which anticipates a 20-year growth rate between one and two percent. Therefore, in order to provide for a conservative analysis, a growth rate of two percent was applied to existing traffic volumes. This annual growth rate provides for a conservative analysis and is assumed to account for regional growth projections and the level of in-fill development expected within the area.

[^1]
## Vehicle Trip Generation

Standard traffic generation characteristics compiled by the Institute of Transportation Engineers (ITE) in their report entitled Trip Generation Manual, 11 th Edition, were considered for the proposed land use in order to estimate the average daily traffic (ADT) and peak hour vehicle trips. A vehicle trip is defined as a one-way vehicle movement from point of origin to point of destination.

However, ITE's Trip Generation Manual does not provide traffic generation information for this particular land use. As such, trip generation data was gathered from previous studies for similar land use projects ${ }^{3,4,5}$ and used to estimate average daily and weekday peak hour trip information. Table 1 presents trip generation rates from the referenced studies.

Table 1 - Trip Generation Rates


Key: $\quad$ ACRE $=$ Acres.
Note: All data and calculations above are subject to being rounded to nearest value.

Table 2 applies average rates from Table 1 and summarizes the projected ADT and peak hour traffic volumes likely generated by the land use area proposed.

Table 2 - Trip Generation Summary

| $\begin{gathered} \text { ITE } \\ \text { CODE } \end{gathered}$ | LAND USE | SIZE | TOTAL TRIPS GENERATED |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{gathered} 24 \\ \text { HOUR } \end{gathered}$ | AM PEAK HOUR |  |  | PM PEAK HOUR |  |  |
|  |  |  |  | ENTER | EXIT | TOTAL | ENTER | EXIT | TOTAL |
| Site Development - Proposed |  |  |  |  |  |  |  |  |  |
|  | Hampton Partners Contractor Yard and Storage (El Paso County, CO) | 22.8 ACRE | 309 | 34 | 11 | 45 | 17 | 15 | 32 |
| Proposed Total: |  |  | 309 | 34 | 11 | 45 | 17 | 15 | 32 |

[^2]As Table 2 shows, the development area has the potential to generate approximately 309 daily trips with 45 of those occurring during the morning peak hour and 32 during the afternoon peak hour.

[^3]
## Adjustments to Trip Generation Rates

A development of this type is not likely to attract trips from within area land uses nor pass-by or diverted link trips from the adjacent roadway system, therefore no trip reduction was taken in this analysis.

## Trip Generation Distribution and Assignment

The overall directional distribution of site-generated traffic was determined based on the location of development within the County, proposed and existing area land uses, allowed turning movements, available road network, and in reference to historical traffic count data provided by CDOT's Traffic Count Database System (TCDS) ${ }^{6}$.

Traffic assignment is how the site-generated and distributed trips are expected to be loaded onto the available roadway network. Applying assumed trip distribution patterns to site-generated traffic provides the peak hour trip volume assignments for proposed access drives.

Overall site-generated trip distribution patterns and assignments are shown on Figure 3.

[^4]
## Total Traffic Volumes

Total traffic is the traffic projected to be on area roadways with consideration of the proposed development. Total traffic includes background traffic projections for Year 2024 with consideration of site-generated traffic. For analysis purposes, it was assumed that development construction would be completed by end of Year 2024.

Projected Year 2024 total traffic volumes and intersection geometry are shown in Figure 4.




## - Analysis updated to include discussion of existing and long-term conditions. Pe - Section added to include the development's impact to LOS.

The Signalized and Unsignalized Intersection Analysis techniques, as published in the Highway Capacity Manual (HCM), $6^{\text {th }}$ Edition, by the Transportation Research Board and as incorporated into the SYNCHRO computer program, were used to analyze the study intersections for total traffic conditions. These nationally accepted techniques allow for the determination of intersection level of service (LOS) based on the congestion and delay of each traffic movement.

Definitions of levels of service are given in Attachment B. Intersection capacity worksheets are provided in Attachment C.

Table 3 - Intersection Capacity Analysis Summary - Total Traffic - Year 2024

| INTERSECTION | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
|  | AM PEAK HOUR | PM PEAK HOUR |
| Peterson Boulevard / Space Village Avenue (Signalized) | $\mathrm{C}(29.4)$ | $\mathrm{C}(30.9)$ |
| EB US 24 On/Off Ramp / Space Village Avenue (Stop-Controlled) |  |  |
| Eastbound Left | A | A |
| Westbound Left | A | A |
| Northbound Left, Through and Right | C | E |
| Southbound Left | D | F |
| Storage Sense Access / Space Village Avenue (Stop-Controlled) |  |  |
| Eastbound Left and Through | A | A |
| Southbound Left and Right | B | B |
| Marksheffel Road / Space Village Avenue (Stop-Controlled) |  |  |
| Eastbound Left | F | F |
| Eastbound Through | F | F |
| Eastbound Right | B | B |
| Westbound Left | F | F |
| Westbound Through | F | F |
| Westbound Right | B | B |
| Northbound Left | C | A |
| Southbound Left | A | B |
| Access A / Space Village Avenue (Stop-Controlled) |  |  |
| Westbound Left and Through | A | A |
| Northbound Left and Right | B | B |
| Access B / Space Village Avenue (Stop-Controlled) |  |  |
| Westbound Leff and Through | A | A |
| Northbound Left and Right | B | B |

Key: Signalized Intersection: Level of Service (Control Delay in sec/veh)
Stop-Controlled Intersection: Level of Service
Table 3 illustrates how, by Year 2024 and upon development build-out, the signalized intersection of Space Village Avenue with Peterson Boulevard shows an overall LOS C operation during both the morning and afternoon peak traffic hours.

The stop-controlled intersection of Space Village Avenue with the eastbound U.S. 24 On/Off Ramp predicts turning movement operations of LOS D or better for the morning peak traffic hour and LOS A for the afternoon peak traffic hour. Exceptions would include the northbound and southbound turning movements which operate at LOS E and LOS F, respectively, during the PM peak traffic hour. The LOS E and F operations are attributed to through traffic volumes along Space Village Avenue and the stop-controlled nature of the intersection.

The stop-controlled intersection of Space Village Avenue with the Storage Sense Access drive expects turning movement operations of LOS B or better during both peak traffic hours.

The stop-controlled intersection of Marksheffel Road with Space Village Avenue anticipates turning movement operations of LOS C or better during the AM peak traffic hour and LOS B or better during the PM peak traffic hour. Exceptions include the eastbound and westbound left and through turning movements which predict operations of LOS F for both peak traffic hours. The LOS F operations are attributed to the through traffic volume along Marksheffel Road and the stop-controlled nature of the intersection. To mitigate poor operations along the minor approach, signalization is shown to provide acceptable levels of service, as described within the Reagan Ranch Traffic Impact Study.

The stop-controlled intersections of Space Village Avenue with Access A and Access B predict turning movement operations at or better than LOS B during both the AM and PM peak traffic hours.

It is to be noted that it is not uncommon for unsignalized movements to or from an arterial roadway, in urban areas, to operate with noticeable delays during peak traffic hours. It is, however, likely that turn movements will operate better than the results obtained with this HCM Two-Way Stop-Control (TWSC) level of service analysis would indicate, as the HCM analysis may not accurately account for the effect of vehicle platooning and gaps caused by upstream signals. Upstream signal controls along Marksheffel Road will tend to create additional gaps in the traffic stream for turning movements at Space Village Avenue and will most likely provide mitigation to the LOS F operation projected during both peak traffic hours.

## Auxiliary Lane Analysis

Auxiliary lanes for site development accesses were based on the County's ECM.
Considering development build-out, an evaluation of auxiliary lane requirements, pursuant to Section 2.3.7 of the County's ECM, reveals that this development does not warrant the need for right or left turn lane deceleration lanes along Space Village Avenue since ingress traffic volumes do not exceed design hourly volume thresholds.

## State the expected turn volumes and how they compare to the criteria

Turn lane analysis discussion expanded to include the mention of expected turn volumes and how they compare to the County's criteria.

## Conclusion

This analysis assessed traffic generation for the Space Village Filing No. 4 development and potential impacts to the adjacent roadway network.

It is our professional opinion that the proposed site-generated traffic resulting from the development is expected to create no negative impact to traffic operations for the surrounding roadway network and proposed site accesses, nor at the Space Village Avenue intersections with Peterson Boulevard and Marksheffel Road. Analysis of site-generated traffic concludes that proposed development traffic volumes are minor.

Analysis of future traffic conditions indicates that the addition of site-generated traffic is expected to create no negative impact to traffic operations for the existing and surrounding roadway system upon roadway and intersection control improvements assumed within the analysis. With all conservative assumptions defined in this analysis, the study intersections are projected to operate at LOS D or better during peak traffic periods and upon build-out. Exceptions include the northbound and southbound turning movements at the Eastbound U.S. Highway 24 On/Off Ramp intersection with Space Village Avenue as well as the eastbound and westbound left and through turning movements at the intersection of Space Village Avenue and Marksheffel Road, which predict LOS E and F operations for their respective peak traffic hours.

We trust that our findings will assist in the planning and approval of the Space Village Filing No. 4 development. Please contact us should further assistance be needed.

Sincerely,

## SM ROCHA, LLC

Traffic and Transportation Consultants


Megan Bock, EIT
Road Impact Fees Statement added.


Unresolved Review 1 Comment:
Per ECM Appendix B. 8 please state the current applicable Road Impact Fees and the developer's time of payment.
https://publicworks.elpasoco.com/road-impact-fees/

## Unresolved Review 1 Comment:

Space Village Avenue is anticipated to be improved to an Urban Minor Arterial road classification. Per ECM Table 2-6 access spacing shall meet spacing requirements shown in Table 2-35. Please discuss if the proposed access locations meet criteria. If access spacing criteria is not met please provide alternative locations or submit a deviation request for review. Refer to ECM Chapter 2.4 for criteria.

Discussion of proposed access spacing included, as shown on the site plan. Please refer to Site Civil for site plan and access spacing design elements.

## Unresolved Review 1 Comment:

- Provide recommendations for the curb return radius at the access points for the vehicles that will utilize the site. Per ECM table 2-36 the typical design vehicle for industrial lots is a multi-unit truck.
- Provide the classification of all adjacent roadways per the MTCP.
- Per ECM B.2.4D discuss the requirement for pedestrian and bicycle facilities. Sidewalk and curb/gutter is required along Space Village Avenue.
- State what the sight distance is for every affected access and whether it can be met. If it cannot be met, state the required modifications so that it can be met. -If an intersection does not meet LOS D or better, discuss what steps can be taken to bring the intersection to a satisfactory level.
-State whether the MTCP or other approved corridor study calls for the construction of improvements in the immediate area.
-State whether or not any improvements affected by the project are reimbursable under the current Major Transportation Corridors Plan (MTCP).
- Please refer to Site Civil for civil related concerns.
- Section on adjacent roadway classifications and conditions added.
- Discussion on pedestrian and bicycle facilities added.
- A general discussion of sight distance added. Please refer to Site Civil for confirmation of sight distance triangles and the potential for mitigation.
- Discussion added on mitigation for intersections with LOS E or F.
- Discussion on roadway improvements was added to the study. However, per the MTCP, no roadway improvements are expected to occur in the immediate area.
- The project is not expected to require any improvements which are reimbursable under the MTCP. Discussion added.

Location: 1 PETERSON BLVD \& SPACE VILLAGE AVE AM
Date: Wednesday, October 26, 2022
Peak Hour: 07:00 AM - 08:00 AM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 07:30 AM - 07:45 AM


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | SPACE VILLAGE AVE Eastbound |  |  |  | SPACE VILLAGE AVE <br> Westbound |  |  |  | PETERSON BLVD <br> Northbound |  |  |  | PETERSON BLVD <br> Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 7:00 AM | 0 | 9 | 33 | 114 | 0 | 20 | 0 | 62 | 0 | 0 | 26 | 10 | 0 | 28 | 165 | 0 | 467 | 2,102 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 18 | 43 | 111 | 0 | 23 | 0 | 85 | 0 | 0 | 31 | 16 | 0 | 34 | 180 | 0 | 541 | 2,020 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 14 | 45 | 109 | 0 | 25 | 0 | 101 | 0 | 0 | 32 | 19 | 0 | 52 | 189 | 0 | 586 | 1,760 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 15 | 24 | 109 | 0 | 26 | 0 | 110 | 0 | 0 | 32 | 11 | 0 | 34 | 147 | 0 | 508 | 1,443 | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 18 | 21 | 71 | 0 | 14 | 0 | 87 | 0 | 0 | 24 | 15 | 0 | 28 | 107 | 0 | 385 | 1,150 | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 18 | 14 | 52 | 0 | 10 | 0 | 70 | 0 | 0 | 15 | 8 | 0 | 28 | 66 | 0 | 281 |  | 0 | 0 | 0 | 0 |
| 8:30 AM | 0 | 19 | 16 | 37 | 0 | 5 | 0 | 60 | 0 | 0 | 18 | 11 | 0 | 26 | 77 | 0 | 269 |  | 0 | 0 | 0 | 0 |
| 8:45 AM | 0 | 18 | 12 | 29 | 0 | 6 | 0 | 50 | 0 | 0 | 19 | 12 | 0 | 34 | 35 | 0 | 215 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 129 | 208 | 632 | 0 | 129 | 0 | - 625 | 0 | 0 | 197 | 102 | 0 | 264 | 966 | 0 | 3,252 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 0 | 56 | 145 | 443 | 0 | 94 | 0 | - 358 | 0 | 0 | 121 | 56 | 0 | 148 | 681 |  | 0 2,102 |  | 0 | 0 | 0 | 0 |

Location: 1 PETERSON BLVD \& SPACE VILLAGE AVE PM
Date: Wednesday, October 26, 2022
Peak Hour: 04:00 PM - 05:00 PM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 04:30 PM - 04:45 PM
Peak Hour - All Vehicles


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | SPACE VILLAGE AVE Eastbound |  |  |  | SPACE VILLAGE AVE <br> Westbound |  |  |  | PETERSON BLVD <br> Northbound |  |  |  | PETERSON BLVD <br> Southbound |  |  |  | Total |  | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 4:00 PM | 0 | 33 | 28 | 0 | 0 | 0 | 0 | 70 | 0 | 0 | 241 | 99 | 0 | 32 | 0 | 0 | 0 | 503 |  | 1,874 | 0 | 0 | 0 | 0 |
| 4:15 PM | 0 | 47 | 25 | 0 | 0 | 0 | 0 | 67 | 0 | 0 | 204 | 79 | 0 | 54 | 1 | 0 | 0 | 477 | 1,752 | 0 | 0 | 0 | 0 |
| 4:30 PM | 0 | 48 | 20 | 0 | 0 | 0 | 0 | 75 | 0 | 0 | 225 | 93 | 0 | 45 | 1 | 10 | ) | 507 | 1,592 | 0 | 0 | 0 | 0 |
| 4:45 PM | 0 | 42 | 23 | 0 | 0 | 0 | 0 | 64 | 0 | 0 | 163 | 52 | 0 | 43 | 0 | 0 | 0 | 387 | 1,368 | 0 | 0 | 0 | 0 |
| 5:00 PM | 0 | 33 | 19 | 0 | 0 | 0 | 0 | 72 | 0 | 0 | 137 | 52 | 0 | 68 | 0 | 0 | 0 | 381 | 1,217 | 0 | 0 | 0 | 0 |
| 5:15 PM | 0 | 43 | 31 | 2 | 0 | 0 | 0 | 53 | 0 | 0 | 95 | 42 | 0 | 51 | 0 | 0 | 0 | 317 |  | 0 | 0 | 0 | 0 |
| 5:30 PM | 0 | 43 | 23 | 2 | 0 | 0 | 0 | 59 | 0 | 0 | 88 | 28 | 0 | 40 | 0 | 0 | 0 | 283 |  | 0 | 0 | 0 | 0 |
| 5:45 PM | 0 | 38 | 15 | 2 | 0 | 0 | 0 | 62 | 0 | 0 | 59 | 28 | 0 | 32 | 0 | 0 | 0 | 236 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 327 | 184 | 6 | 0 | 0 | 0 | 522 | 0 | 0 | 1,212 | 473 | 0 | 365 | 2 | 2 | 0 | 3,091 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 0 | 170 | 96 | 0 | 0 | 0 | 0 | 276 | 0 | 0 | 0833 | 323 | 0 | 174 |  | 2 | 0 | 1,87 |  | 0 | 0 | 0 | 0 |

Location: 2 US24 EB RAMPS \& SPACE VILLAGE AVE AM
Date: Wednesday, October 26, 2022
Peak Hour: 07:15 AM - 08:15 AM
(303) 216-2439 www.alltrafficdata.net

Peak 15-Minutes: 07:30 AM - 07:45 AM

Peak Hour - All Vehicles


Note: Total study counts contained in parentheses.

Traffic Counts

| Interval | SPACE VILLAGE AVE Eastbound |  |  |  | SPACE VILLAGE AVE <br> Westbound |  |  |  | US24 EB RAMPS <br> Northbound |  |  |  | US24 EB RAMPS <br> Southbound |  |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | eft | Thru R | Right | U-Turn | Left | Thru | Right | U-Turn | Left | Thru | Righ |  |  |  | West | East | South |  |
| 7:00 AM | 0 | 15 | 53 | 1 | 0 | 0 | 82 | 7 | 0 | 4 | 1 | 2 | 0 | 8 | 0 |  | 1 | 174 | 881 | 0 | 0 | 0 | 0 |
| 7:15 AM | 0 | 23 | 63 | 2 | 0 | 1 | 102 | 6 | 0 | 2 | 0 | 0 | 0 | 8 | 0 |  | 6 | 213 | 887 | 0 | 0 | 0 | 0 |
| 7:30 AM | 0 | 18 | 93 | 1 | 0 | 1 | 120 | 5 | 0 | 4 | 1 | 3 | 0 | 14 | 0 |  | 6 | 266 | 815 | 0 | 0 | 0 | 0 |
| 7:45 AM | 0 | 16 | 48 | 1 | 0 | 2 | 128 | 9 | 0 | 3 | 1 | 2 | 0 | 12 | 0 |  | 6 | 228 | 664 | 0 | 0 | 0 | 0 |
| 8:00 AM | 0 | 27 | 31 | 2 | 0 | 2 | 87 | 11 | 0 | 5 | 3 | 1 | 0 | 9 | 0 |  | 2 | 180 | 552 | 0 | 0 | 0 | 0 |
| 8:15 AM | 0 | 19 | 27 | 1 | 0 | 1 | 70 | 1 | 0 | 7 | 1 | 2 | 0 | 7 | 0 |  | 5 | 141 |  | 0 | 0 | 0 | 0 |
| 8:30 AM | 0 | 14 | 24 | 3 | 0 | 3 | 58 | 0 | 0 | 4 | 0 | 5 | 0 | 4 | 0 |  | 0 | 115 |  | 0 | 0 | 0 | 0 |
| 8:45 AM | 0 | 19 | 34 | 3 | 0 | 3 | 47 | 3 | 0 | 1 | 1 | 1 | 0 | 2 | 0 |  | 2 | 116 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 151 | 373 | 14 | 0 | 13 | 694 | 42 | 0 | 30 | 8 | 16 | 0 | 64 | 0 |  | 28 | 1,433 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 0 | 84 | 235 | 6 | 0 | 6 | 437 | 31 | 0 | 14 | 5 | 6 | 0 | 43 | 0 | 0 | 20 | 88 | 7 | 0 | 0 | 0 | 0 |

(303) 216-2439 www.alltrafficdata.net

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | SPACE VILLAGE AVE Eastbound |  |  |  | SPACE VILLAGE AVE Westbound |  |  |  | US24 EB RAMPS <br> Northbound |  |  |  | US24 EB RAMPS <br> Southbound |  |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn |  | Thru |  | U-Turn | Left | Thru | Right | U-Turn | Left | Thru |  | Right |  |  | West | East | South |  |
| 4:00 PM | 0 | 95 | 56 | 4 | 0 | 1 | 58 | 8 | 0 | 3 | 2 | 1 | 0 | 11 | 0 |  | 10 | 249 | 938 | 0 | 0 | 0 | 0 |
| 4:15 PM | 0 | 75 | 73 | 1 | 0 | 0 | 55 | 4 | 0 | 3 | 3 | 2 | 0 | 14 | 1 |  | 20 | 251 | 911 | 0 | 0 | 0 | 0 |
| 4:30 PM | 0 | 101 | 53 | 2 | 0 | 1 | 51 | 3 | 0 | 4 | 1 | 2 | 0 | 10 | 0 |  | 11 | 239 | 866 | 0 | 0 | 0 | 0 |
| 4:45 PM | 1 | 54 | 54 | 4 | 0 | 3 | 58 | 3 | 0 | 2 | 2 | 5 | 0 | 6 | 0 |  | 7 | 199 | 803 | 0 | 0 | 0 | 0 |
| 5:00 PM | 0 | 74 | 49 | 7 | 0 | 2 | 52 | 1 | 0 | 6 | 1 | 5 | 0 | 12 | 0 |  | 13 | 222 | 755 | 0 | 0 | 0 | 0 |
| 5:15 PM | 0 | 60 | 54 | 3 | 0 | 2 | 41 | 12 | 0 | 2 | 4 | 4 | 0 | 18 | 0 |  | 6 | 206 |  | 0 | 0 | 0 | 0 |
| 5:30 PM | 0 | 40 | 44 | 1 | 0 | 1 | 53 | 13 | 0 | 3 | 2 | 3 | 0 | 10 | 0 |  | 6 | 176 |  | 0 | 0 | 0 | 0 |
| 5:45 PM | 0 | 33 | 43 | 3 | 0 | 0 | 49 | 3 | 0 | 2 | 0 | 3 | 0 | 7 | 0 |  | 8 | 151 |  | 0 | 0 | 0 | 0 |
| Count Total | 1 | 532 | 426 | 25 | 0 | 10 | 417 | 47 | 0 | 25 | 15 | 25 | 0 | 88 | 1 |  | 81 | 1,693 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 1 | 325 | 236 | 11 | 0 | 5 | 222 | 18 | 0 | 12 | 8 | 10 | 0 | 41 |  | 1 | 48 | 93 | 38 | 0 | 0 | 0 | 0 |

(303) 216-2439 www.alltrafficdata.net

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

(303) 216-2439 www.alltrafficdata.net

Location: 3 STORAGE ACCESS \& SPACE VILLAGE AVE PM
Date: Wednesday, October 26, 2022
Peak Hour: 04:15 PM - 05:15 PM
Peak 15-Minutes: 05:00 PM - 05:15 PM

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

(303) 216-2439 www.alltrafficdata.net


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

(303) 216-2439 www.alltrafficdata.net

Peak Hour - All Vehicles


Peak Hour - Pedestrians/Bicycles on Crosswalk


Note: Total study counts contained in parentheses.
Traffic Counts

| Interval | SPACE VILLAGE AVE Eastbound |  |  |  | SPACE VILLAGE AVE <br> Westbound |  |  |  |  | MARKSHEFFEL RD Northbound |  |  |  |  | MARKSHEFFEL RD Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left |  | Thru Right |  | U-Turn | Left | Thru | Right |  | U-Turn | Left | Thru | Right |  |  | West | East | South |  |
| 4:00 PM | 0 | 7 | 0 | 48 | 0 | 0 |  | 0 | 0 | 0 | 43 | 194 | 0 | ) | 0 | 0 | 140 | 5 | 437 | 1,866 | 0 | 0 | 0 | 0 |
| 4:15 PM | 0 | 3 | 0 | 61 | 0 | 0 |  | 0 | 0 | 0 | 36 | 234 | 0 | 0 | 0 | 0 | 138 | 15 | 487 | 1,866 | 0 | 0 | 0 | 0 |
| 4:30 PM | 0 | 2 | 0 | 43 | 0 | 0 |  | 0 | 0 | 0 | 44 | 223 | 0 | 0 | 0 | 0 | 145 | 6 | 463 | 1,804 | 0 | 0 | 0 | 0 |
| 4:45 PM | 0 | 2 | 0 | 46 | 0 | 0 |  | 0 | 0 | 0 | 47 | 246 | 0 | 0 | 0 | 0 | 136 | 2 | 479 | 1,753 | 0 | 0 | 0 | 0 |
| 5:00 PM | 0 | 7 | 0 | 61 | 0 | 0 |  | 0 | 0 | 0 | 27 | 198 | 0 | 0 | 0 | 0 | 138 | 6 | 437 | 1,629 | 0 | 0 | 0 | 0 |
| 5:15 PM | 0 | 7 | 0 | 63 | 0 | 0 |  | 0 | 0 | 1 | 28 | 200 | 0 | 0 | 0 | 0 | 119 | 7 | 425 |  | 0 | 0 | 0 | 0 |
| 5:30 PM | 0 | 3 | 0 | 38 | 0 | 0 |  | 0 | 0 | 0 | 41 | 176 | 0 | 0 | 0 | 0 | 145 | 9 | 412 |  | 0 | 0 | 0 | 0 |
| 5:45 PM | 0 | 2 | 0 | 44 | 0 | 0 |  | 00 | 0 | 0 | 41 | 143 | 0 | 0 | 0 | 0 | 125 | 0 | 355 |  | 0 | 0 | 0 | 0 |
| Count Total | 0 | 33 | 0 | 404 | 0 | 0 |  | 00 | 0 | 1 | 307 | 1,614 | 0 | 0 | 0 | 0 | 1,086 | 50 | 3,495 |  | 0 | 0 | 0 | 0 |
| Peak Hour | 0 | 14 | 0 | 198 | 0 | 0 |  | 00 | 0 | 0 | 170 | 897 |  | 0 | 0 |  | $0 \quad 559$ | 28 | 1,866 |  | 0 | 0 | 0 | 0 |



Intersection 266 at Highway 24 and Peterson Rd S - Sequence table, page 1

| Page 1 | Ring 1 | Phases |  |  | Ring | 2 Ph | Phases |  |  |  | Ring | Pha |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1 | 2 | 3 | 4 | 5 | 6 |  | 7 | 8 |  | 9 | 10 | 11 | 12 |  |
| State 1 |  | Vehicle |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Barrier 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| State 2 | Vehicle |  |  |  |  |  | Vehicle\| |  |  |  |  |  |  |  |  |
| Barrier 2 | x $\times$ YXX | xxxxxx | xxxxxx | xxXXXX | XXX | xxX | XxXXXX | XXX | xxX | XXXXX |  |  |  |  |  |
| State 3 | - |  | Vehicle |  |  |  |  |  |  |  |  |  |  |  |  |
| Barrier 2 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| State 4 |  |  |  | V \& P |  |  |  |  |  |  |  |  |  |  |  |
| Barrier 4 | XXXXX | Xxxxxx | XXXXXX | XXXXXX | xXX | XXX | XXXXXX | XXX | XXXX | XXXXX |  |  |  |  |  |
| State 5 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Barrier 5 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| State 6 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Barrier 6 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| State 7 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Barrier 7 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| State 8 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Barrie 8 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| State 9 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Barrier 9 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| State 10 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Barrier 10 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| State 11 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Barrier 11 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| State 12 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Barrier 12 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |

Intersection 266 at Highway 24 and Peterson Rd S - Phases control table, page 1

Intersection 266 at Highway 24 and Peterson Rd S - Schedule table, events 1-25

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## ATTACHMENT B

## Level of Service Definitions

The following information can be found in the Highway Capacity Manual, Transportation Research Board, 2016: Chapter 19 - Signalized Intersections and Chapter 20 - Two-Way Stop Controlled Intersections.

## Automobile Level of Service (LOS) for Signalized Intersections

Levels of service are defined to represent reasonable ranges in control delay.

## LOS A

Describes operations with a control delay of $10 \mathrm{~s} / \mathrm{veh}$ or less and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

## LOS B

Describes operations with control delay between 10 and 20 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.

## LOS C

Describes operations with control delay between 20 and 35 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

## LOS D

Describes operations with control delay between 35 and $55 \mathrm{~s} / \mathrm{veh}$ and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

## LOS E

Describes operations with control delay between 55 and 80 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

LOS F
Describes operations with control delay exceeding $80 \mathrm{~s} / \mathrm{veh}$ or a volume-to-capacity ratio greater than 1.0 . This level is typically assigned when the volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

## Level of Service (LOS) for Unsignalized TWSC Intersections

| Level of Service (v/c $\leq 1.0)$ | Average Control Delay (s/veh) |
| :---: | :---: |
| A | $0-10$ |
| B | $>10-15$ |
| C | $>15-25$ |
| D | $>25-35$ |
| E | $>35-50$ |
| F | $>50$ |

## ATTACHMENT C

Capacity Worksheets

|  | 4 |  |  | 7 |  |  |  |  | $p$ |  | $\dagger$ | $\pm$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 | F | ${ }^{*}$ |  | 「 |  | 中4 | 「 | ${ }^{7}$ | 中4 |  |
| Traffic Volume（vph） | 58 | 156 | 461 | 100 | 0 | 375 | 0 | 126 | 63 | 159 | 708 | 0 |
| Future Volume（vph） | 58 | 156 | 461 | 100 | 0 | 375 | 0 | 126 | 63 | 159 | 708 | 0 |
| Satd．Flow（prot） | 0 | 1839 | 1583 | 1770 | 0 | 1583 | 0 | 3539 | 1583 | 1770 | 3539 | 0 |
| Flt Permitted |  | 0.987 |  | 0.613 |  |  |  |  |  | 0.558 |  |  |
| Satd．Flow（perm） | 0 | 1839 | 1583 | 1142 | 0 | 1583 | 0 | 3539 | 1583 | 1039 | 3539 | 0 |
| Satd．Flow（RTOR） |  |  | 255 |  |  | 408 |  |  | 164 |  |  |  |
| Lane Group Flow（vph） | 0 | 233 | 501 | 109 | 0 | 408 | 0 | 137 | 68 | 173 | 770 | 0 |
| Turn Type | Split | NA | Perm | Perm |  | Perm |  | NA | Perm | pm＋pt | NA |  |
| Protected Phases | 4 | 4 |  |  |  |  |  | 2 |  | 1 | 6 |  |
| Permitted Phases |  |  | 4 | 8 |  | 8 |  |  | 2 | 6 |  |  |
| Detector Phase | 4 | 4 | 4 | 8 |  | 8 |  | 2 | 2 | 1 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 4.0 | 4.0 | 4.0 | 4.0 |  | 4.0 |  | 4.0 | 4.0 | 4.0 | 4.0 |  |
| Minimum Split（s） | 10.0 | 10.0 | 10.0 | 10.0 |  | 10.0 |  | 10.0 | 10.0 | 10.0 | 10.0 |  |
| Total Split（s） | 26.0 | 26.0 | 26.0 | 26.0 |  | 26.0 |  | 30.0 | 30.0 | 18.0 | 48.0 |  |
| Total Split（\％） | 26．0\％ | 26．0\％ | 26．0\％ | 26．0\％ |  | 26．0\％ |  | 30．0\％ | 30．0\％ | 18．0\％ | 48．0\％ |  |
| Yellow Time（s） | 4.0 | 4.0 | 4.0 | 4.0 |  | 4.0 |  | 4.0 | 4.0 | 4.0 | 4.0 |  |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 |  | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust（s） |  | 0.0 | 0.0 | 0.0 |  | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time（s） |  | 6.0 | 6.0 | 6.0 |  | 6.0 |  | 6.0 | 6.0 | 6.0 | 6.0 |  |
| Lead／Lag |  |  |  |  |  |  |  | Lag | Lag | Lead |  |  |
| Lead－Lag Optimize？ |  |  |  |  |  |  |  | Yes | Yes | Yes |  |  |
| Recall Mode | None | None | None | None |  | None |  | C－Max | C－Max | None | C－Max |  |
| Act Effct Green（s） |  | 19.6 | 19.6 | 14.5 |  | 14.5 |  | 31.3 | 31.3 | 47.9 | 47.9 |  |
| Actuated g／C Ratio |  | 0.20 | 0.20 | 0.14 |  | 0.14 |  | 0.31 | 0.31 | 0.48 | 0.48 |  |
| v／c Ratio |  | 0.65 | 0.97 | 0.66 |  | 0.71 |  | 0.12 | 0.11 | 0.30 | 0.45 |  |
| Control Delay |  | 46.2 | 54.3 | 58.5 |  | 11.0 |  | 27.4 | 0.4 | 18.0 | 19.3 |  |
| Queue Delay |  | 0.0 | 0.0 | 0.0 |  | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Delay |  | 46.2 | 54.3 | 58.5 |  | 11.0 |  | 27.4 | 0.4 | 18.0 | 19.3 |  |
| LOS |  | D | D | E |  | B |  | C | A | B | B |  |
| Approach Delay |  | 51.7 |  |  | 21.0 |  |  | 18.4 |  |  | 19.1 |  |
| Approach LOS |  | D |  |  | C |  |  | B |  |  | B |  |
| Queue Length 50th（ft） |  | 137 | 170 | 66 |  | 0 |  | 33 | 0 | 62 | 168 |  |
| Queue Length 95th（ft） |  | 219 | \＃382 | 118 |  | 83 |  | 62 | 0 | 117 | 243 |  |
| Internal Link Dist（ft） |  | 419 |  |  | 647 |  |  | 421 |  |  | 121 |  |
| Turn Bay Length（ft） |  |  |  |  |  |  |  |  | 250 | 280 |  |  |
| Base Capacity（vph） |  | 367 | 520 | 228 |  | 643 |  | 1107 | 608 | 587 | 1695 |  |
| Starvation Cap Reductn |  | 0 | 0 | 0 |  | 0 |  | 0 | 0 | 0 | 0 |  |
| Spillback Cap Reductn |  | 0 | 0 | 0 |  | 0 |  | 0 | 0 | 0 | 0 |  |
| Storage Cap Reductn |  | 0 | 0 | 0 |  | 0 |  | 0 | 0 | 0 | 0 |  |
| Reduced v／c Ratio |  | 0.63 | 0.96 | 0.48 |  | 0.63 |  | 0.12 | 0.11 | 0.29 | 0.45 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length： 100 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length： 100 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset： $20(20 \%)$ ，Referenced to phase 2：NBT and 6：SBTL，Start of Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle： 45 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type：Actuated－Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |

Maximum v/c Ratio: 0.97
Intersection Signal Delay: 29.4 Intersection LOS: C

Intersection Capacity Utilization 67.0\%
ICU Level of Service C
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Peterson Boulevard \& EB Highway 24 Off Ramp/Space Village Avenue




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.1 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{-}$ | $\mathbf{4}$ | $\mathbf{r}$ | Tr |  |
| Traffic Vol, veh/h | 4 | 201 | 478 | 3 | 0 | 1 |
| Future Vol, veh/h | 4 | 201 | 478 | 3 | 0 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | 125 | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 4 | 218 | 520 | 3 | 0 | 1 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 523 | 0 | - | 0 | 746 | 520 |
| Stage 1 | - | - | - - | - | 520 | - |
| Stage 2 | - | - | - - | - | 226 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1043 | - | - | - | 381 | 556 |
| Stage 1 | - | - | - - | - | 597 | - |
| Stage 2 | - | - | - - | - | 812 | - |
| Platoon blocked, \% |  | - | - - | - |  |  |
| Mov Cap-1 Maneuver | 1043 | - | - - | - | 379 | 556 |
| Mov Cap-2 Maneuver | - | - | - - | - | 379 | - |
| Stage 1 | - | - | - - | - | 595 | - |
| Stage 2 | - | - | - - | - | 812 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 0.2 |  | 0 |  | 11.5 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1043 | - | - | - | 556 |
| HCM Lane V/C Ratio |  | 0.004 |  | - | - | 0.002 |
| HCM Control Delay (s) |  | 8.5 | 0 | - | - | 11.5 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0 | , | - | - | 0 |




HCM LOS

| Minor Lane/Major Mvmt | NBL | NBT | NBREBL | 11 EBLn2 | EBLn3V | 1 W | BLn2 | NBLn3 | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Capacity (veh/h) | 642 | - | - | 14 | 535 | - | 12 | 657 | 898 | - | - |
| HCM Lane V/C Ratio | 0.579 | - | - | - 3.649 | 0.284 |  | 1.178 | 0.003 | 0.01 | - | - |
| HCM Control Delay (s) | 18 | - | - | -\$ 1740 | 14.4 |  | 745.6 | 10.5 | 9 | - | - |
| HCM Lane LOS | C | - | - | F | B | - | F | B | A | - | - |
| HCM 95th \%tile Q(veh) | 3.7 | - | - | 7.3 | 1.2 | - | 2.4 | 0 | 0 | - | - |
| Notes |  |  |  |  |  |  |  |  |  |  |  |
| $\sim$ : Volume exceeds capacity | \$: Dela | ay exc | ceds 300s | +: Comp | putation | Def | fined | *: All | major vo | ume in | platoon |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.1 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | - | rin |  |
| Traffic Vol, veh/h | 313 | 8 | 8 | 470 | 3 | 2 |
| Future Vol, veh/h | 313 | 8 | 8 | 470 | 3 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 340 | 9 | 9 | 511 | 3 | 2 |


| Major/Minor | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 349 | 0 | 874 | 345 |
| Stage 1 | - | - | - | - | 345 | - |
| Stage 2 | - | - | - | - | 529 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1210 | - | 320 | 698 |
| Stage 1 | - | - | - | - | 717 | - |
| Stage 2 | - | - | - | - | 591 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1210 | - | 317 | 698 |
| Mov Cap-2 Maneuver | - | - | - | - | 317 | - |
| Stage 1 | - | - | - | - | 717 | - |
| Stage 2 | - | - | - | - | 585 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 0.1 |  | 14 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBT | EBR | WBL WBT |  |
| Capacity (veh/h) |  | 06 | - | - | 1210 | - |
| HCM Lane V/C Ratio |  | 13 | - | - | 0.007 | - |
| HCM Control Delay (s) |  | 14 | - | - | 8 | 0 |
| HCM Lane LOS |  | B | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\boldsymbol{F}$ |  |  | $\mathbf{A}$ | Mr |  |
| Traffic Vol, veh/h | 304 | 9 | 9 | 478 | 3 | 3 |
| Future Vol, veh/h | 304 | 9 | 9 | 478 | 3 | 3 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 330 | 10 | 10 | 520 | 3 | 3 |



|  | 4 | $\rightarrow$ |  |  |  |  |  |  | $p$ |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | $\uparrow$ | F | ${ }^{1}$ |  | 「 |  | 44 | 「 | 7 | 44 |  |
| Traffic Volume (vph) | 177 | 103 | 0 | 2 | 0 | 292 | 0 | 866 | 338 | 184 | 2 | 0 |
| Future Volume (vph) | 177 | 103 | 0 | 2 | 0 | 292 | 0 | 866 | 338 | 184 | 2 | 0 |
| Satd. Flow (prot) | 0 | 1805 | 1863 | 1770 | 0 | 1583 | 0 | 3539 | 1583 | 1770 | 3539 | 0 |
| Flt Permitted |  | 0.969 |  | 0.575 |  |  |  |  |  | 0.177 |  |  |
| Satd. Flow (perm) | 0 | 1805 | 1863 | 1071 | 0 | 1583 | 0 | 3539 | 1583 | 330 | 3539 | 0 |
| Satd. Flow (RTOR) |  |  |  |  |  | 317 |  |  | 350 |  |  |  |
| Lane Group Flow (vph) | 0 | 304 | 0 | 2 | 0 | 317 | 0 | 941 | 367 | 200 | 2 | 0 |
| Turn Type | Split | NA | Perm | Perm |  | Perm |  | NA | Perm | pm+pt | NA |  |
| Protected Phases | 4 | 4 |  |  |  |  |  | 2 |  | 1 | 6 |  |
| Permitted Phases |  |  | 4 | 8 |  | 8 |  |  | 2 | 6 |  |  |
| Detector Phase | 4 | 4 | 4 | 8 |  | 8 |  | 2 | 2 | 1 | 6 |  |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 4.0 | 4.0 | 4.0 | 4.0 |  | 4.0 |  | 4.0 | 4.0 | 4.0 | 4.0 |  |
| Minimum Split (s) | 10.0 | 10.0 | 10.0 | 10.0 |  | 10.0 |  | 10.0 | 10.0 | 10.0 | 10.0 |  |
| Total Split (s) | 29.0 | 29.0 | 29.0 | 25.0 |  | 25.0 |  | 47.0 | 47.0 | 24.0 | 71.0 |  |
| Total Split (\%) | 23.2\% | 23.2\% | 23.2\% | 20.0\% |  | 20.0\% |  | 37.6\% | 37.6\% | 19.2\% | 56.8\% |  |
| Yellow Time (s) | 4.0 | 4.0 | 4.0 | 4.0 |  | 4.0 |  | 4.0 | 4.0 | 4.0 | 4.0 |  |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 |  | 2.0 |  | 2.0 | 2.0 | 2.0 | 2.0 |  |
| Lost Time Adjust (s) |  | 0.0 | 0.0 | 0.0 |  | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Lost Time (s) |  | 6.0 | 6.0 | 6.0 |  | 6.0 |  | 6.0 | 6.0 | 6.0 | 6.0 |  |
| Lead/Lag |  |  |  |  |  |  |  | Lag | Lag | Lead |  |  |
| Lead-Lag Optimize? |  |  |  |  |  |  |  | Yes | Yes | Yes |  |  |
| Recall Mode | None | None | None | None |  | None |  | C-Max | C-Max | None | C-Max |  |
| Act Effct Green (s) |  | 22.6 |  | 8.8 |  | 8.8 |  | 56.3 | 56.3 | 75.6 | 75.6 |  |
| Actuated g/C Ratio |  | 0.18 |  | 0.07 |  | 0.07 |  | 0.45 | 0.45 | 0.60 | 0.60 |  |
| v/c Ratio |  | 0.93 |  | 0.03 |  | 0.78 |  | 0.59 | 0.41 | 0.57 | 0.00 |  |
| Control Delay |  | 86.1 |  | 50.5 |  | 19.5 |  | 29.5 | 5.0 | 18.6 | 12.0 |  |
| Queue Delay |  | 0.0 |  | 0.0 |  | 0.0 |  | 0.0 | 0.0 | 0.0 | 0.0 |  |
| Total Delay |  | 86.1 |  | 50.5 |  | 19.5 |  | 29.5 | 5.0 | 18.6 | 12.0 |  |
| LOS |  | F |  | D |  | B |  | C | A | B | B |  |
| Approach Delay |  | 86.1 |  |  | 19.7 |  |  | 22.6 |  |  | 18.5 |  |
| Approach LOS |  | F |  |  | B |  |  | C |  |  | B |  |
| Queue Length 50th (ft) |  | 244 |  | 2 |  | 0 |  | 277 | 7 | 64 | 0 |  |
| Queue Length 95th (ft) |  | \#414 |  | 9 |  | 88 |  | 449 | 82 | 127 | 2 |  |
| Internal Link Dist (ft) |  | 419 |  |  | 647 |  |  | 421 |  |  | 121 |  |
| Turn Bay Length (ft) |  |  |  |  |  |  |  |  | 250 | 280 |  |  |
| Base Capacity (vph) |  | 332 |  | 162 |  | 509 |  | 1593 | 904 | 411 | 2139 |  |
| Starvation Cap Reductn |  | 0 |  | 0 |  | 0 |  | 0 | 0 | 0 | 0 |  |
| Spillback Cap Reductn |  | 0 |  | 0 |  | 0 |  | 0 | 0 | 0 | 0 |  |
| Storage Cap Reductn |  | 0 |  | 0 |  | 0 |  | 0 | 0 | 0 | 0 |  |
| Reduced v/c Ratio |  | 0.92 |  | 0.01 |  | 0.62 |  | 0.59 | 0.41 | 0.49 | 0.00 |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 125 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 125 |  |  |  |  |  |  |  |  |  |  |  |  |
| Offset: 1 (1\%), Referenced to phase 2:NBT and 6:SBTL, Start of Green |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 80 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Coordinated |  |  |  |  |  |  |  |  |  |  |  |  |

Maximum v/c Ratio: 0.93
Intersection Signal Delay: 30.9 Intersection LOS: C
Intersection Capacity Utilization 72.2\% ICU Level of Service C
Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 1: Peterson Boulevard \& EB Highway 24 Off Ramp/Space Village Avenue




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.1 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{-}$ | 4 | $\mathbf{r}$ | Tr |  |
| Traffic Vol, veh/h | 4 | 266 | 259 | 0 | 1 | 4 |
| Future Vol, veh/h | 4 | 266 | 259 | 0 | 1 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | 125 | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 4 | 289 | 282 | 0 | 1 | 4 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay，s／veh | 45.6 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 4 | 「 | ${ }^{7}$ | 4 | 「 | ${ }^{7}$ | 44 | 「 | ${ }^{*}$ | 44 | 「 |
| Traffic Vol，veh／h | 18 | 34 | 209 | 32 | 11 | 1 | 181 | 933 | 19 | 1 | 581 | 32 |
| Future Vol，veh／h | 18 | 34 | 209 | 32 | 11 | 1 | 181 | 933 | 19 | 1 | 581 | 32 |
| Conflicting Peds，\＃／hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | － | － | None | － | － | None | － | － | None | － | － | None |
| Storage Length | 215 | － | 250 | 290 | － | 220 | 390 | － | 430 | 390 | － | 415 |
| Veh in Median Storage，\＃ | \＃ | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Grade，\％ | － | 0 | － | － | 0 | － | － | 0 | － | － | 0 | － |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles，\％ | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 20 | 37 | 227 | 35 | 12 | 1 | 197 | 1014 | 21 | 1 | 632 | 35 |



| Approach | EB | WB | NB | SB |
| :--- | ---: | ---: | ---: | ---: |
| HCM Control Delay，s | 50.5 | $\$ 1784.5$ | 1.6 | 0 |


| Minor Lane／Major Mvmt | NBL | NBT | NBR EBLn1 EBLn2 EBLn3WBLn1WBLn2WBLn3 | SBL | SBT | SBR |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Capacity（veh／h） | 919 | - | - | 52 | 42 | 680 | 8 | 42 | 511 | 667 | - | - |
| HCM Lane V／C Ratio | 0.214 | - | - | 0.376 | 0.88 | 0.334 | 4.348 | 0.285 | 0.002 | 0.002 | - | - |
| HCM Control Delay（s） | 10 | - | -111.1 | 249.9 | 12.92411 .5 | 121.8 | 12.1 | 10.4 | - | - |  |  |
| HCM Lane LOS | A | - | - | F | F | B | F | F | B | B | - | - |
| HCM 95th \％tile Q（veh） | 0.8 | - | - | 1.4 | 3.4 | 1.5 | 5.7 | 1 | 0 | 0 | - | - |

## Notes

$\sim$ ：Volume exceeds capacity $\quad \$$ ：Delay exceeds 300s $\quad$ ：Computation Not Defined $\quad$ ：All major volume in platoon

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  |  | $\mathbf{T}$ | Mr |  |
| Traffic Vol, veh/h | 308 | 4 | 4 | 259 | 4 | 3 |
| Future Vol, veh/h | 308 | 4 | 4 | 259 | 4 | 3 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 335 | 4 | 4 | 282 | 4 | 3 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  |  | $\uparrow$ | Mr |  |
| Traffic Vol, veh/h | 303 | 5 | 4 | 263 | 4 | 4 |
| Future Vol, veh/h | 303 | 5 | 4 | 263 | 4 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 329 | 5 | 4 | 286 | 4 | 4 |



## V2_Traffic Memo Redlines.pdf Markup Summary

| Callout (1) |  |  |
| :---: | :---: | :---: |
|  | Subject: Callout <br> Page Label: 24 <br> Author: Daniel Torres <br> Date: 1/30/2023 3:33:29 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | ? |
| File Attachment (1) |  |  |
| $W \equiv$ | Subject: File Attachment <br> Page Label: 1 <br> Author: Carlos <br> Date: 1/19/2023 1:25:20 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 |  |
| Text Box (7) |  |  |
| $\pm$ $\qquad$ <br>  | Subject: Text Box <br> Page Label: 1 <br> Author: Carlos <br> Date: 1/19/2023 1:25:13 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | Please add PCD File No. MS227 <br> Also please include the El Paso County standard signature block. See attached file. |
| $=$ | Subject: Text Box <br> Page Label: 12 <br> Author: Carlos <br> Date: 1/19/2023 2:07:30 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | Unresolved Review 1 Comment: <br> Per ECM Appendix B. 8 please state the current applicable Road Impact Fees and the developer's time of payment. <br> https://publicworks.elpasoco.com/road-impact-fees / |
|  | Subject: Text Box <br> Page Label: 12 <br> Author: Carlos <br> Date: 1/30/2023 4:16:00 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | Unresolved Review 1 Comment: <br> Space Village Avenue is anticipated to be improved to an Urban Minor Arterial road classification. Per ECM Table 2-6 access spacing shall meet spacing requirements shown in Table 2-35. Please discuss if the proposed access locations meet criteria. If access spacing criteria is not met please provide alternative locations or submit a deviation request for review. Refer to ECM Chapter 2.4 for criteria. |


| + | Subject: Text Box <br> Page Label: 13 <br> Author: Carlos <br> Date: 2/7/2023 3:00:39 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | Unresolved Review 1 Comment: <br> - Provide recommendations for the curb return radius at the access points for the vehicles that will utilize the site. Per ECM table 2-36 the typical design vehicle for industrial lots is a multi-unit truck. <br> - Provide the classification of all adjacent roadways per the MTCP. <br> - Per ECM B.2.4D discuss the requirement for pedestrian and bicycle facilities. Sidewalk and curb/gutter is required along Space Village Avenue. <br> - State what the sight distance is for every affected access and whether it can be met. If it cannot be met, state the required modifications so that it can be met. <br> -If an intersection does not meet LOS D or better, discuss what steps can be taken to bring the intersection to a satisfactory level. <br> -State whether the MTCP or other approved corridor study calls for the construction of improvements in the immediate area. <br> -State whether or not any improvements affected by the project are reimbursable under the current Major Transportation Corridors Plan (MTCP). |
| :---: | :---: | :---: |
| $=\square$ | Subject: Text Box <br> Page Label: 11 <br> Author: Carlos <br> Date: 1/30/2023 4:09:42 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | State the expected turn volumes and how they compare to the criteria |
|  | Subject: Text Box <br> Page Label: 7 <br> Author: Carlos <br> Date: 2/9/2023 2:38:16 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | Please provide analysis and figure for existing conditions and long-term. |
| $\square=\square$ | Subject: Text Box <br> Page Label: 10 <br> Author: Carlos <br> Date: 2/9/2023 2:41:06 PM <br> Length: 0 <br> Area: 0 <br> Volume: 0 | - Provide discussion on LOS for existing and long-term conditions. <br> - Discuss any changes in LOS due to the development's traffic. |


[^0]:    87ロ3 Yates Drive，Suite 210 Westminster，Calarada 8ロロ31（3ロ3）458－9798 6 SaUth TEJan Street，Suite 515 CaLarada Springs，Calarada ba9a3（719）203－6639 www．smRacha．cam

[^1]:    ${ }^{2}$ Reagan Ranch Traffic Impact Study, Kimley-Horn and Associates, Inc., November 2020.

[^2]:    $\begin{array}{ll}\text { Key: } & \text { ACRE }=\text { Acres. } \\ \text { Note: } & \text { All data }\end{array}$
    Note: All data and calculations above are subject to being rounded to nearest value.

[^3]:    ${ }^{3}$ SH 141 \& Springfield Road Transportation Impact Study, McDowell Engineering, LLC, January 2019.
    ${ }^{4}$ Small Contractors Yard Land Use Application, US Hwy 6 and County Road 240 (Bruce Road), Timberline Energy Inc., November 2013.
    ${ }^{5}$ Ute Creek Industrial Storage Facility Expansion Traffic Impact Assessment, TDA Colorado, Inc., November 2010.

[^4]:    ${ }^{6}$ Transportation Data Management System, MS2, 2022.

