



05/12/2022



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Site Information:	Page 1:
Customer: Rigid Component Systems	Job Number: LBC22268
Job Description: ,Larkspur Outlets LLC ,Britt Ely	
Address: 8510 Kirk Dr, Colorado Spgs, CO 80908	

Job Engineering Criteria:	
Design Code: IBC 2018	IntelliVIEW Version: 20.02.00A JRef #: 1XH72750004
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 50.00
Building Type: Closed	

This package contains general notes pages, 2 truss drawing(s) and 0 detail(s).

Item	Drawing Number	Truss
1	132.22.1357.33463	A01 36' Common

Item	Drawing Number	Truss
2	132.22.1357.41403	AG1 36' Gable

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

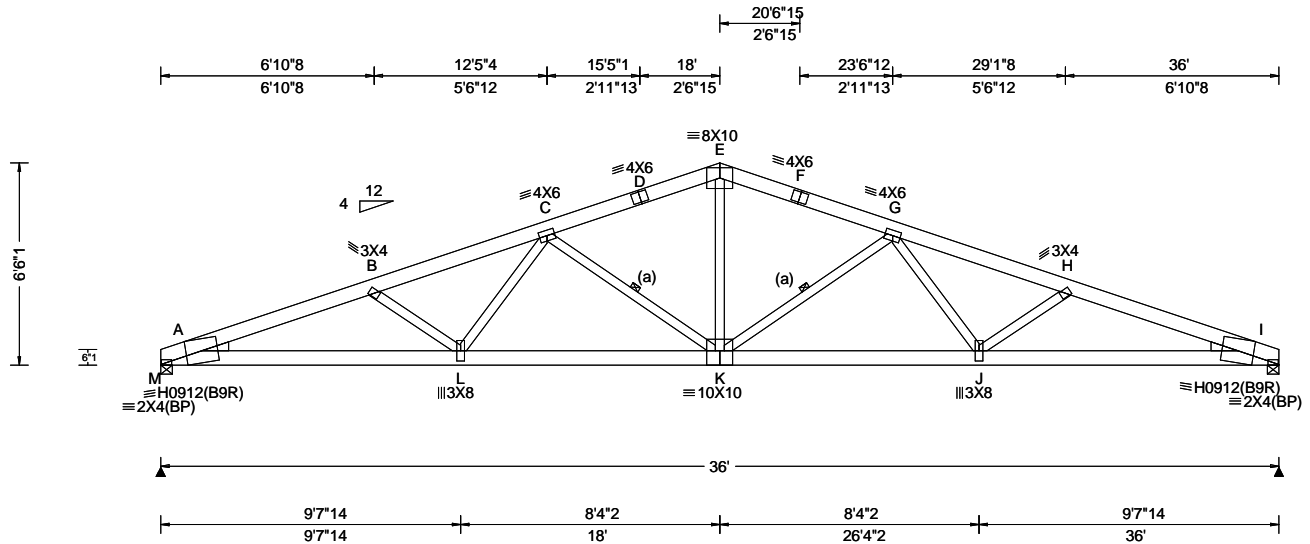
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catocin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.



Loading Criteria (psf) TCLL: 40.00 TCDL: 5.00 BCLL: 0.00 BCDL: 5.00 Des Ld: 50.00 NCBCLL: 20.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 48.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCDL: 3.0 psf BCDL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.60 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.33	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IBC 2018 TPI Std: 2014 Rep Fac: No FT/RT/PT:10(5)/2(2)/2(0) Plate Type(s): HS, WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.621 K 690 360 VERT(CL): 0.780 K 549 240 HORZ(LL): 0.175 I - - HORZ(TL): 0.220 I - - Creep Factor: 2.0 Max TC CSI: 0.967 Max BC CSI: 0.980 Max Web CSI: 0.928 VIEW Ver: 20.02.00A.1020.21	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL M 3619 /- /- /925 /615 /242 I 3619 /- /- /925 /615 /- Wind reactions based on MWFRS M Brg Wid = 4.5 Min Req = 3.9 I Brg Wid = 4.5 Min Req = 3.9 Bearings M & I Fcperp = 625psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 1592 -9072 E - F 1112 -5690 B - C 1435 -8137 F - G 1102 -5865 C - D 1102 -5865 G - H 1435 -8137 D - E 1112 -5690 H - I 1592 -9072 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - L 8462 -1424 K - J 7005 -1100 L - K 7005 -1100 J - I 8462 -1424 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - L 333 -1141 K - G 438 -1975 L - C 1200 -119 G - J 1200 -119 C - K 438 -1975 J - H 333 -1141 E - K 2559 -432
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Lumber
Top chord: 2x6 SPF 1650f-1.5E;
Bot chord: 2x6 SPF 2100f-1.8E;
Webs: 2x4 HF #2;
Lt Wedge: 2x4 HF #2; Rt Wedge: 2x4 HF #2;

Bracing
(a) Continuous lateral restraint equally spaced on member. Or 2x4 #3 or better "T" reinforcement. 80% length of web member. Attached with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes
Handling stresses not considered for plates. Handling of this truss requires special care by truss manufacturer and installation contractor to prevent plate damage.

Purlins
In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:
Chord Spacing(in oc) Start(ft) End(ft)
TC 24 0.00 18.00
TC 24 18.00 36.00
BC 95 0.15 35.85
Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

Loading
Bottom chord checked for 20.00 psf non-concurrent live load.

Wind
Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.
Uplifts based on an elevation at or above 5000 ft.
DESIGN TO BE USED WITH LIGHT GAGE METAL ROOFING AND PURLINS. SUSPENDED TYPE CEILING AND/OR INSULATION ONLY ON THE BOTTOM CHORD.

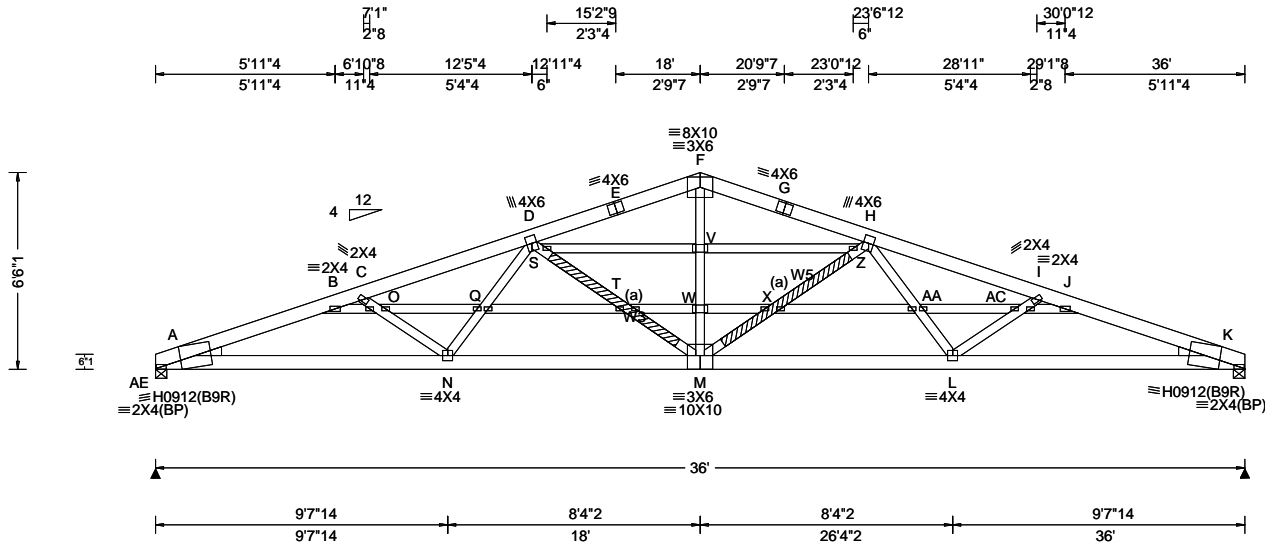


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****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 85157 FROM: MM	GABL Ply: 1 Qty: 2	Job Number: LBC22268 Larkspur Outlets LLC, Britt Ely Truss Label: AG1 36' Gable	Cust: R 7275 JRef: 1XfH7250004 T2 DrwNo: 132.22.1357.41403 CAT / NDW 05/12/2022
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Loading Criteria (psf) TCCL: 40.00 TCCL: 5.00 BCLL: 0.00 BCLL: 5.00 Des Ld: 50.00 NCBCLL: 20.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 130 mph Enclosure: Closed Risk Category: II EXP: C Kzt: NA Mean Height: 15.00 ft TCCL: 3.0 psf BCLL: 3.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.60 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.33	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: IBC 2018 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT/PT: 10(5)/2(2)/2(0) Plate Type(s): HS, WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.618 M 693 360 VERT(CL): 0.770 M 556 240 HORZ(LL): 0.182 K - - HORZ(TL): 0.227 K - - Creep Factor: 2.0 Max TC CSI: 0.901 Max BC CSI: 0.971 Max Web CSI: 0.567 VIEW Ver: 20.02.00A.1020.21	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>AE</td> <td>3589</td> <td>-</td> <td>-</td> <td>1745</td> <td>1723</td> <td>1242</td> </tr> <tr> <td>K</td> <td>3589</td> <td>-</td> <td>-</td> <td>1745</td> <td>1723</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS AE Brg Wid = 4.5 Min Req = 3.9 K Brg Wid = 4.5 Min Req = 3.9 Bearings AE & K Fcperp = 625psi. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>1866 -9161</td> <td>F - G</td> <td>1148 -5152</td> </tr> <tr> <td>B - C</td> <td>1683 -8242</td> <td>G - H</td> <td>1137 -5329</td> </tr> <tr> <td>C - D</td> <td>1576 -7705</td> <td>H - I</td> <td>1576 -7705</td> </tr> <tr> <td>D - E</td> <td>1137 -5329</td> <td>I - J</td> <td>1683 -8242</td> </tr> <tr> <td>E - F</td> <td>1148 -5152</td> <td>J - K</td> <td>1866 -9161</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	AE	3589	-	-	1745	1723	1242	K	3589	-	-	1745	1723	-	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	1866 -9161	F - G	1148 -5152	B - C	1683 -8242	G - H	1137 -5329	C - D	1576 -7705	H - I	1576 -7705	D - E	1137 -5329	I - J	1683 -8242	E - F	1148 -5152	J - K	1866 -9161
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Lumber
 Top chord: 2x6 SPF 1650f-1.5E;
 Bot chord: 2x6 SPF 2100f-1.8E;
 Webs: 2x4 HF #2; W3,W5 2x4 SPF 1650f-1.5E;
 Lt Wedge: 2x4 HF #2; Rt Wedge: 2x4 HF #2;

Bracing
 (a) #3 or better scab reinforcement. Same size & 80% length of web member. Attach with 10d Box or Gun (0.128"x3",min.)nails @ 6" oc.

Plating Notes
 All plates are 1.5X3 except as noted.
 Handling stresses not considered for plates. Handling of this truss requires special care by truss manufacturer and installation contractor to prevent plate damage.

Purlins
 In lieu of structural panels or rigid ceiling use purlins to laterally brace chords as follows:

Chord	Spacing(in oc)	Start(ft)	End(ft)
TC	24	0.00	18.00
TC	24	18.00	36.00
BC	92	0.15	35.85

Apply purlins to any chords above or below fillers at 24" OC unless shown otherwise above.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.
 Uplifts based on an elevation at or above 5000 ft.

Loading
 Truss designed to support 2-0-0 top chord outlookers and cladding load not to exceed 2.00 PSF one face and 24.0" span opposite face. Top chord must not be cut or notched, unless specified otherwise.

Bottom chord checked for 20.00 psf non-concurrent live load.

Additional Notes
 Truss has been designed for vertical in-plane loads only. Any lateral/horizontal wind loads shall be transferred into the roof and ceiling diaphragms. Connection and design of these systems is the responsibility of the Building Designer in accordance with ANSI/TPI 1.

DESIGN TO BE USED WITH LIGHT GAGE METAL ROOFING AND PURLINS. SUSPENDED TYPE CEILING AND/OR INSULATION ONLY ON THE BOTTOM CHORD.



Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - N	8548 -1691	M - L	7372 -1405
N - M	7372 -1405	L - K	8548 -1691

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
C - N	269 -962	M - H	511 -2124
N - D	756 -136	H - L	756 -136
D - M	511 -2124	L - I	269 -962
F - M	2140 -461		

Maximum Gable Forces Per Ply (lbs)

Gables	Tens.Comp.	Gables	Tens. Comp.
B - O	209 -713	V - Z	144 -635
O - Q	182 -640	W - X	137 -438
Q - T	172 -593	X -AA	172 -593
S - V	144 -635	AA-AC	182 -640
T - W	137 -438	AC - J	209 -713

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

