Hydraulics and Hydrology Memorandum

Subject:	Hydrologic Analysis and Hydraulic Assessment
From:	Elizabeth Staten, PE, HDR
Cc:	Alissa Werre, PE, El Paso County; Cory Beasley, PE, HDR
To:	Monument Academy
Project:	Monument Academy Recirculation Project
Date:	June 2, 2023

Design Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or WILLIAM . omissions on my part in preparing this report.

		ORADO LICAL
Elizabeth Staten, P.E. #38974	Date	38974
		3.06/01/2023
Owner/Developer's Statement:		SONAL ENG

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

Name, Title		Date	
Business Name:	K		'
Address:			
		Have owne	r sign and date
El Doco Country			

El Paso County:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

County Engineer / ECM Administrator Date

Conditions:

1

1 Introduction

The purpose of this technical memorandum is to present a hydrologic analysis and hydraulic assessment for the Monument Academy Recirculation Project (Project) in El Paso County, Colorado. The Project aims to redirect or capture stormwater runoff flows from new impervious surfaces with minimal changes in hydraulic conditions.

The Project will add two, paved, private, internal roads to improve circulation of traffic on Monument Academy's property in El Paso County, CO. It also includes modular block walls, private storm system additions, and landscaping modifications.



Figure 1: Vicinity Map

1.1 Location

The Project is at the Monument Academy West Campus located just outside the limits of the Town of Monument, El Paso County CO. The Project is in the NW ¼, Section 13, Township 11S, Range 67W. The project site is located north of Highway 105, and east of Knollwood Drive, and is accessed from Knollwood Drive by turning east on Village Ridge Point. The subject property is in Knollwood Village Filing No. 2, and neighboring properties are either in the same development, or in Harmon Hills Filing No. 1 to the north.

1.2 Description of Property

The project site on Monument Academy private property (MA Property) is in a mixed-use suburban area northeast of Monument, Colorado. The MA Property is bounded by Highway 105

on the south and encompasses a segment of Dirty Woman Creek on the north. The property includes 15.4 acres, though approximately seven acres is considered a "No-Build" Area for habitat conservation and floodplain preservation.

The developed area of the site includes the school building, private internal roads, parking areas, a playground, and an athletic field. Undeveloped areas include upland hills forested with deciduous and evergreen trees, and lowland plains near the creek with grassland and wetland vegetation. An NRCS Soil Report shows two types of soils in the vicinity. Loams and clay loams (with 1-3% slopes) are found along Dirty Woman Creek and its floodplain. Upland areas are expected to be loamy sands or coarse sands (with 3-8% slopes). See Appendix C.

The MA Property generally drains from the southeast to northwest with stormwater runoff flowing overland to Dirty Woman Creek. Dirty Woman Creek runs through the northern part of the property and remains north of the project site area. No other major drainageways cross through the MA Property.

1.3 Basin Descriptions

The MA Property is in the Dirty Woman Creek Drainage Basin. The 1993 Drainage Basin Planning Study for Dirty Woman Creek provides recommendations for the basin including upsizing culverts both upstream and downstream of the MA Property. The recommendations do not affect the project site, as it does not contain the creek.

Dirty Woman Creek includes FEMA mapped special flood hazard areas. The 100-year floodplain is outside the project perimeter as shown in Flood Insurance Rate Map (FIRM) Map Number 08041C0276G, December 7, 2018, and Map Number 08041C0278G, December 7, 2018. This project does not cause impacts to the neighboring floodplain.

Tributary basin areas consist primarily of the project site itself and the undeveloped hillside to the east. A one-third acre portion of the neighboring property to the east represents the only offsite flow. Highway 105 located along the southern edge of the project intercepts stormwater runoff flow from the south before it reaches the project. Stormwater runoff from the developed areas is collected along the existing internal road curb and gutter, and at several low point inlets around the property. The runoff is then routed through the Monument Academy private stormwater network prior to outfalling on the northwest side of the developed area and flowing to Dirty Woman Creek. Runoff from the northern and western portions of the property sheet flows directly to Dirty Woman Creek.

2 Design Criteria

The Monument Academy project has elected to obtain construction permitting through the Colorado Department of Public Safety. Design standards for the project are not specific with respect to hydrology and hydraulics criteria since El Paso County is not the permitting body. El Paso County's Engineering Criteria and Drainage Criteria Manuals were referenced for guidelines as well as design criteria published by Colorado Department of Transportation (CDOT), and Mile High Flood District (MHFD).

2.1 Four Step Process

Considerations were taken to reduce the impacts of the additional impervious area associated with the project.

- Runoff reduction practices were incorporated by limiting the additions to the existing
 private stormwater network. The proposed internal road at the back of school drains to
 swales and sheet flows to the existing open field rather than to a network of inlets and
 pipes.
- 2. Stabilization of drainageways is achieved through soil riprap outlet protection near the wetlands area, and riprap lined channels on steeper slopes.
- Permanent Water Quality requirements are achieved through runoff reduction.
 Calculations are evidenced in Appendix F. Receiving pervious areas for infiltration are in the existing No-Build easement on the MA Property.
- 4. Industrial and commercial source control BMPs are not required for this project.

2.2 Hydrology

A hydrologic analysis using the rational method was performed to determine the 10-year and 100-year peak flow rates for sizing the stormwater system for the Project.

Precipitation data is developed in accordance with Colorado Dept. of Transportation (CDOT) and Mile High Flood District (MHFD) criteria and uses NOAA Atlas 14 guidance. An Intensity Duration Frequency (IDF) curve (Figure 2) was developed for Monument to define the rainfall intensity.

Sub-basins are typically less than one acre in size. The minimum time of concentration of five minutes was used for all basins for a conservative approach. The project area comprises multiple land uses with impervious surface areas, constructed outdoor spaces, undeveloped areas, and combinations thereof.

Drainage Basin maps, NRCS soil maps, and rational method calculations are located in the Appendices. A summary is provided in the table below.



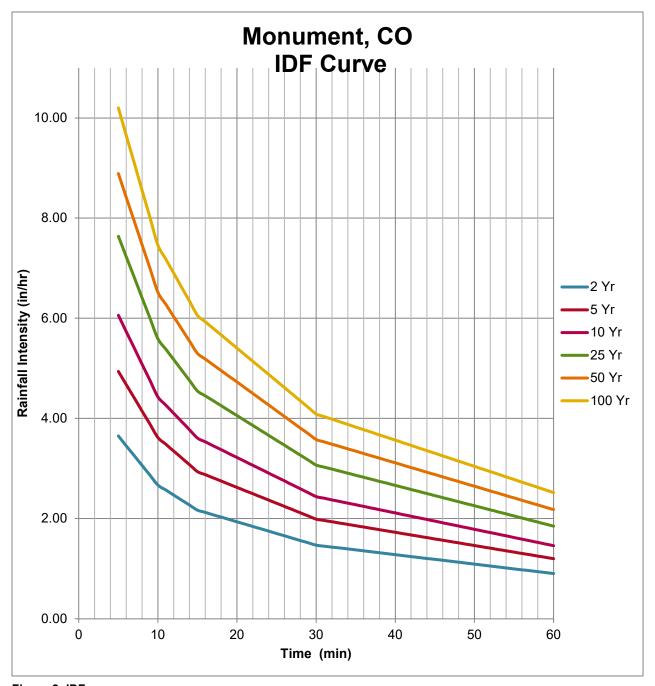


Figure 2: IDF curve

Basin ID Area (ac) C10 Q10 (cfs) C100 Q100 (cfs) 0.62 1.2 2.4 0.31 0.75 A-0 0.81 3.7 0.88 6.8 A-1 0.75 0.74 1.6 0.81 2.9 A-EX-2 0.35 0.92 1.8 A- EX-3 0.96 3.1 0.32 0.58 0.70 0.52 1.9 3.7 A-4 0.61 5.9 0.74 12.0 A-5 1.59 0.86 2.2 0.90 3.9 A-6 0.42 0.92 3.2 0.96 5.6 A- EX-7 0.57 0.38 0.6 0.58 1.5 0.25 A-8 A-9 1.05 0.41 2.6 0.60 6.4 0.39 8.0 0.58 2.0 A-10 0.34 0.72 4.8 0.81 9.0 A-11 1.09

Table 1. Proposed Basins Hydrology

2.3 Related Projects

Improvement projects continue to evolve localized drainage patterns in and around the project area. Recent changes include a project to widen Highway 105 south of the MA Property. The Highway 105 project will incorporate a new retaining wall between the MA Property and Highway 105 reducing the total tributary area draining north to the school's stormwater network. The changed area instead sends the runoff flow to the Highway 105 stormwater network, which outfalls to Dirty Woman Creek approximately 2000 feet downstream of the MA Property.

2.4 Environmental Features

Most of the project site is located near the banks of Dirty Woman Creek, which flows through the northern portion of this 15-acre property. The project site limits are approximately 100-feet from Dirty Woman Creek, and approximately 50-feet outside the limits of the FEMA 100-year floodplain boundary. Riparian vegetation blankets the land between the developed areas of the property and the creek. The area along this portion of Dirty Woman Creek has been designated as non-critical habitat area for the Preble's Meadow Jumping Mouse. Wetland areas are also located in the vicinity of the project. These features are notated on the project's plan sheets.

Drainage Design

The existing stormwater system was analyzed to verify that changes in flow to this system do not result in an adverse impact. It is expected that the existing stormwater network will remain in place unless shown otherwise in the plan documents.

3.1 Existing Conditions

The Monument Academy existing drainage network captures stormwater runoff in the existing internal roads' curb inlets and in local grate inlets along the front of the school. Flow is then conveyed north and west to the 24-inch pipe outfall location behind the school. The stormwater system is a private network and is not connected to other stormwater systems. Analysis of the

existing system includes 0.3 acres of tributary area that will be allotted to the Highway 105 storm system when the neighboring project constructs a new retaining wall.

Stormwater runoff from the school building is collected in a series of downspouts from the roof and piped to the open field areas northwest of the building. Stormwater runoff from the hills, offsite area, and play areas north of the building flows overland or collects in shallow ditches then discharges to the same open field which is tributary to Dirty Woman Creek.

An outfall for an 18-inch pipe is located onsite west of the playground area. The source of flow for the pipe was unable to be confirmed with the site survey or a field visit. Conversations with Monument Academy were also unable to confirm the source. It is assumed the outfall pipe drains groundwater from underneath the playground and should be protected or extended.

3.2 Proposed Conditions

Replacement or protection of existing storm systems is as directed by Monument Academy. The proposed system additions serve to collect stormwater flows from the proposed internal roadways, extend the existing pipe outfalls due to proposed internal roadway reconfiguration, and to convey concentrated runoff to inlets or ditches.

Basin A-0 to LINE 100 – A new inlet upstream of the pedestrian crossing will collect stormwater runoff from the proposed internal road in front of the school, then pipe flows to the existing private stormwater network.

Basins A-1, A-5, and A-6 to Existing – The basin areas are reduced from existing condition as some of the stormwater runoff is now intercepted upstream in Basin A-0 with the new proposed internal road in front of the school. Inlets along the existing internal road will be protected in place.

Basin A-EX-2 to LINE 102 – Inlets in front of the school will be protected in place. The 24-inch outfall pipe for the school's private storm network will be extended to accommodate for the proposed internal road behind the school. Total flows through the existing storm network are decreased in proposed condition due to the reduction in total area from the adjacent Highway 105 project. An outfall stilling basin is proposed to dissipate energy, reduce flow velocities, and reduce erosion potential.



Figure 3: Looking West

Basin A-4 to Underdrain -

In existing condition, the playground and sand play area north of the building drain from east to west overland or through the sandy subgrade below the sidewalk. In proposed condition, the flow path is impeded by the proposed internal road behind the school, which is several inches higher than the sand area. An underdrain is proposed along the western edge of the basin to promote infiltration of stormwater. The underdrain will outfall west of the proposed internal road behind the school.

LINE 104 – The 18-inch outfall pipe near the playground area will be extended to route flows under the proposed internal road behind the school.

Basin A-EX-3 and A-EX-7 to Existing – Downspouts and existing outfalls from the building roofs to the front and back of school will be protected in place.

Basin A-8 to LINE 108 – A new crossing culvert is proposed to drain the infield area between the playground and athletic field. The culvert will outfall west of the proposed internal road behind the school.

Basin A-9 to V-DITCH 2 – Stormwater runoff from the upland hill areas east of the athletic field will be intercepted by the proposed internal road behind the school and routed north along the curb and gutter flow line. This flow will outfall to a riprap-lined channel which routes the stormwater away from the roadway and west toward the existing open field between the school and Dirty Woman Creek. Riprap calculations are included with Appendix F.

Basin A-10 to V-DITCH 1 – A new grass-lined channel is proposed between the athletic field and modular block retaining wall to route stormwater runoff west. Flows then cross over the proposed internal road behind the school in a gutter pan before continuing west. Runoff flows to an existing grass buffer prior to infiltrating or flowing toward the creek.

Basin A-11 to Existing – Runoff flows from the back of school internal road sheet flow to an existing grass buffer within the no-build area.

A comparison of the existing and proposed tributary flow rates is reflected in Table 2. Increased imperviousness from the internal roads contributes to marginally higher runoff rates, but the effects are mitigated on-site.

- Subtotal A for the stormwater network reflects flows are slightly increased in proposed condition. This is attributable to both the reduction in total area from the adjacent Highway 105 project, and the increased impervious area.
- Subtotal B reflects a reduction in flows consistent with a slight reduction in tributary area in basin A-4.
- Subtotal C reflects an increase in tributary flow in part due to additional tributary area. For the back of school basins, A-8 to A-11, stormwater runoff flows are redistributed within the MA Property between existing and proposed conditions. The remaining increase in flow is associated with imperviousness from the new back of school internal road. Higher flow rates are attenuated by existing grass buffer Receiving Pervious Area (RPA) between the school and the Dirty Woman Creek floodplain. Calculations support the buffer RPA provides sufficient space and time to infiltrate the flow.

Table 2. Tributary Flow Comparison

Outfall	Tributary Existing Basins	Tributary Existing Area (ac)	Tributary Existing Q10 (cfs)	Outfall	Tributary Proposed Basins	Tributary Proposed Area (ac)	Tributary Proposed Q10 (cfs)
		0.00	0.0		A-0	0.31	1.2
Existing	A-EX-1	1.08	4.2		A-1	0.75	3.7
Storm	A-EX-2	0.35	1.6	Line 102	A-EX-2	0.35	1.6
Network	A-EX-5	1.79	6.4		A-5	1.59	5.9
	A-EX-6	0.43	2.2		A-6	0.42	2.2
	Subtotal A	3.65	14.34		Subtotal A	3.43	14.48
Ex. Roof Drains	A-EX-3	0.32	1.8	Ex. Roof Drains	A-EX-3	0.32	1.8
Wetlands	A-EX-4	0.60	2.1	Wetlands	A-4	0.52	1.9
Ex. Roof Drains	A-EX-7	0.57	3.2	Ex. Roof Drains	A-EX-7	0.57	3.2
	Subtotal B	1.50	7.04		Subtotal B	1.42	6.83
Existing Channel	A-EX-8	0.81	1.5	Line 108 to RPA	A-8	0.25	0.6
Ex. Grass buffer	A-EX-10	0.98	3.9	V-Ditch 1 to RPA	A-10	0.34	0.8
				Ex. Grass buffer (RPA)	A-11	1.09	4.8
Ex. Grass buffer	A-EX-9	0.77	1.6	V-Ditch 2 to RPA	A-9	1.05	2.6
	Subtotal C	2.56	6.95		Subtotal C	2.74	8.75
Existing	total	7.70	28.33	Proposed	total	7.58	30.06

Calculations for flow routing through the proposed stormwater network were performed in Bentley InRoads and are presented in summary form in the tables below. Four pipes are shown with zero flow reported. The source of flow for Line 104, which includes P-EX-19 and P-104 was undetermined as previously discussed. Pipes P-EX-11 and P-EX-17 are within basin A-EX-2 which includes four inlet points. Flow for the entire basin was routed through other points in the storm network. Additional information is shown in a detailed report in Appendix F. Calculations support that the private stormwater system is not adversely affected by the changes associated with this project.

Table 3. InRoads Pipe Data

						10 – year		
Pipe ID	Material	Dia. (in)	Length (ft)	Slope (%)	Capacity (cfs)	Total Flow (cfs)	Velocity (ft/s)	Flow Depth (ft)
P-101	RCP	18	39.4	4.5	22.3	0.9	6.2	0.21
P-102	RCP	24	20.5	1.0	22.6	13.1	7.5	1.09
P-104	RCP	18	20.8	5.7	25.1	0.0	0.0	0.0
P-108	RCP	18	61.5	4.8	23.1	0.6	5.5	0.16

(719) 272-8800



							10 – year	
Pipe ID	Material	Dia. (in)	Length (ft)	Slope (%)	Capacity (cfs)	Total Flow (cfs)	Velocity (ft/s)	Flow Depth (ft)
P-EX-1	RCP	24	45.9	0.5	16.0	4.8	4.5	0.75
P-EX-2	RCP	12	18.7	3.0	6.2	0.7	5.2	0.23
P-EX-3	RCP	24	113.1	0.5	16.0	5.4	4.6	0.80
P-EX-4	RCP	24	14.5	0.5	16.0	5.3	4.6	0.79
P-EX-5	RCP	24	20.4	0.5	16.0	5.3	4.6	0.79
P-EX-6	RCP	24	34.8	0.5	16.0	5.3	4.6	0.79
P-EX-7	RCP	12	37.5	16.3	14.4	1.0	10.6	0.18
P-EX-8	RCP	24	45.4	0.5	16.0	6.2	4.8	0.87
P-EX-9	RCP	24	41.1	0.5	16.0	6.2	4.8	0.86
P-EX-10	RCP	24	97.4	0.5	16.0	6.1	4.8	0.86
P-EX-11	RCP	12	29.4	3.8	6.9	0.0	0.0	0.0
P-EX-12	RCP	18	12.8	10.6	34.2	6.4	3.6	1.50
P-EX-13	RCP	24	41.8	0.5	16.0	11.9	5.6	1.29
P-EX-14	RCP	18	64.8	14.8	40.4	1.6	11.1	0.21
P-EX-15	RCP	18	67.8	15.0	40.7	1.6	11.2	0.20
P-EX-16	RCP	24	61.1	1.4	26.8	13.4	8.5	1.00
P-EX-17	RCP	12	30.8	10.0	11.3	0.0	0.0	0.0
P-EX-18	RCP	24	129.8	2.0	32.0	13.3	9.7	0.90
P-EX-19	RCP	18	60.3	2.0	14.9	0.0	0.0	0.0

4 Conclusion

The Monument Academy project will include extensions to the existing drainage system and creation of new drainage swales. Existing flow patterns are maintained and relocated outfalls will be protected with riprap to reduce erosion. The project should not adversely impact downstream facilities or surrounding development.

5 References

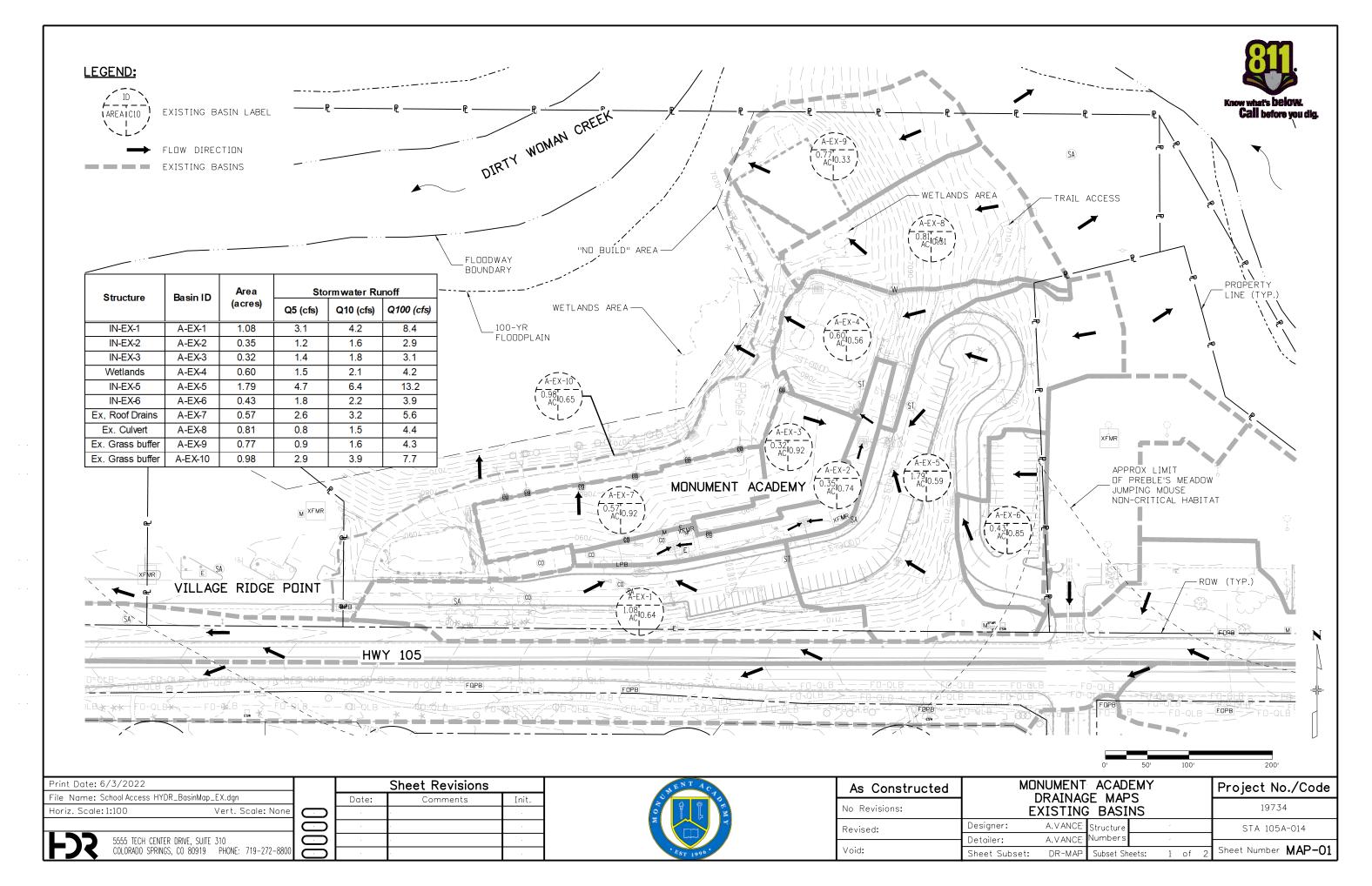
- A. Colorado Department of Public Health and Environment, MS4 General Permits, https://cdphe.colorado.gov/wg-municipal-ms4-general-permits
- B. Colorado Department of Transportation, 2019 Drainage Design Manual, https://www.codot.gov/business/hydraulics/drainage-design-manual
- C. Colorado Springs, Drainage Basin Planning Studies, Dirty Woman and Crystal Creek DBPS, https://coloradosprings.gov/dbps
- D. El Paso County, Drainage Criteria Manual, Version: Oct. 31, 2018 (Current), https://publicworks.elpasoco.com/stormwater/
- E. El Paso County, Engineering Criteria Manual, Version: Oct. 14, 2020 (Current), https://publicworks.elpasoco.com/stormwater/
- F. Mile High Flood District, Detention Design MHFD-Detention v4.04, Feb. 2021, https://mhfd.org/resources/software
- G. US Department of Agriculture, NRCS (2021). Web Soil Survey. https://websoilsurvey.sc.egov.usda.gov/App/WebSoilSurvey.aspx
- H. US Department of Commerce, NOAA (2021). NOAA Atlas 14: Precipitation-Frequency Atlas of the United States, Volume 8, Version 2. https://hdsc.nws.noaa.gov/hdsc/pfds/pfds map cont.html

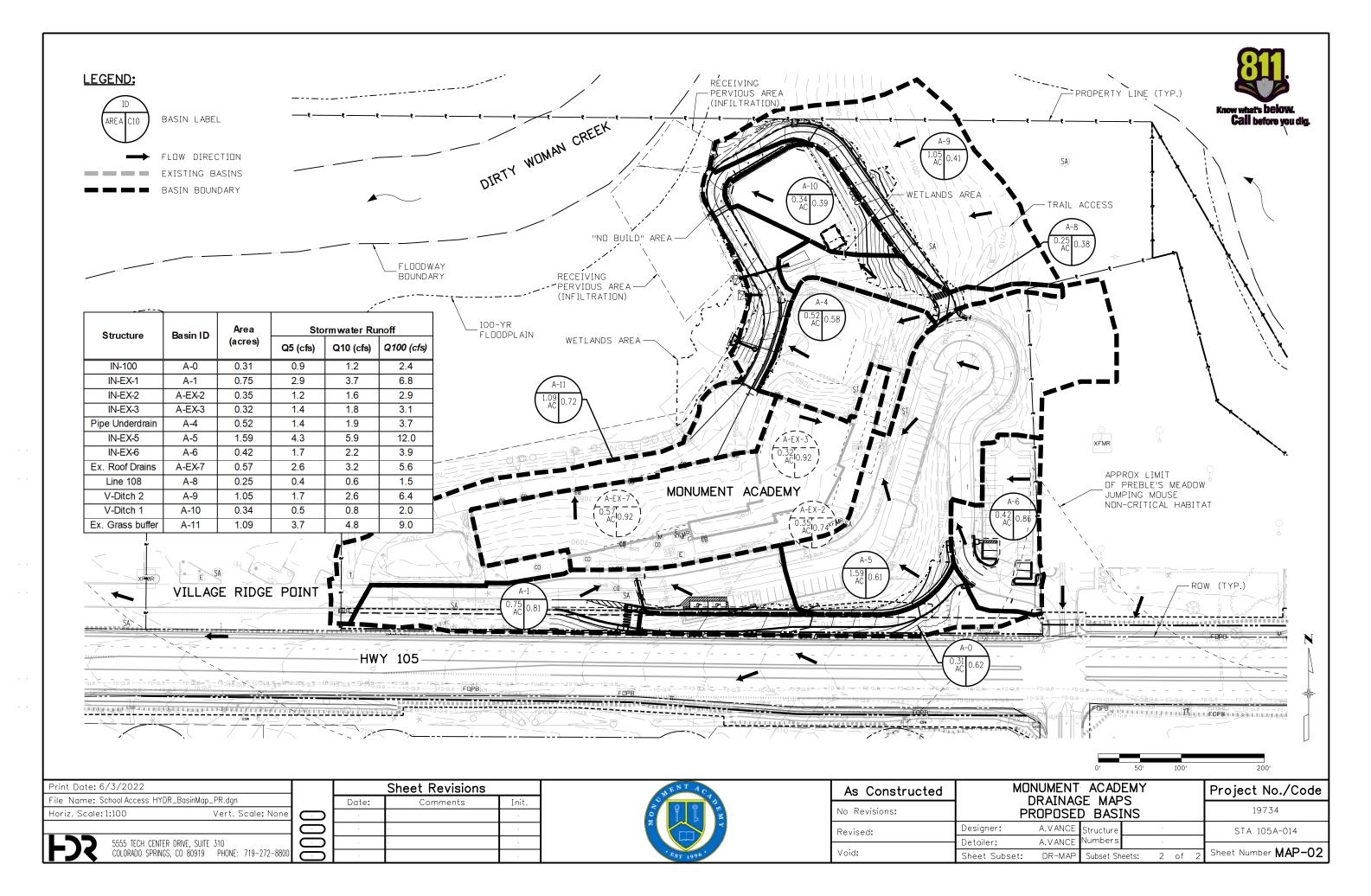
6 Appendices

- A. Drainage Basin Maps
- B. Historic Plans (2007)
- C. Soils Report
- D. Floodplain Map
- E. Precipitation Data
- F. Calculations

Appendix A

Drainage Basin Maps





Appendix B

Historic Plans (2007)

MONUMENT ACADEMY (TRACT A, KNOLLWOOD VILLAGE FILING NO. 2)

Town of monument, county of El Paso, state of colorado

PRIVATE STORM SEWER PLANS

AUGUST 2007

GENERAL NOTES

- ALL MATERIALS AND INSTALLATION PROCEDURES SHALL BE IN COMPLIANCE WITH EL PASO COUNTY STANDARD SPECIFICATIONS.

- UNLESS OTHERWISE NOTED, STORM SEWER PIPE SHALL BE CLASS IF REINFORCED CONCRETE PIPE WITH 6000 PSI CONCRETE AND CLASS-C BEDDING. UNLESS OTHERWISE NOTED, ALL STORM SEWER BENDS, WYES AND TEES SHOWN ON PLANS SHALL BE PREFABRICATED.

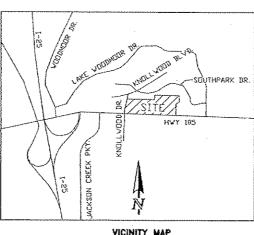
- THE CONTRACTOR SHALL SECURE ALL APPLICABLE LICENSES AND PERMITS TO COMPLETE THE CONSTRUCTION IN COMPUNICE WITH ALL LOCAL, STATE, AND FEDERAL REGULATIONS.
- CONTRACTOR TO OBTAIN COPIES OF THE SOILS REPORT FROM THE GEOTECHNICAL ENGINEER AND TO BE KEPT ON-SITE DURING ALL EARTHWORK OPERATIONS.
- 10, BENCHMARKS:

LEGEND

EXISTING WATER LINE EXISTING FIRE HYDRANT EXISTING GATE VALVE ---- EXISTING SANITARY SEVER · -- - - - EXISTING SAN. MANHOLE EXISTING GAS MAIN PROPOSED GATE VALVE PROPOSED WATER MAIN PROPOSED STORM SEWER PROPOSED STORM INLET

SHEET INDEX:

24" PRIVATE STORM SEWER PLAN 2 OF 3 18" PERFORATED RISER OUTFALL PAPE STORN LATERALS #1 & #2



AGENCIES:

DEVELOPER

ELECTRIC COMPANY TELEPHONE COMPANY:

MOCOMOOR WATER AND SANSTATION DISTRICT 1845 WOODWOOR DRIVE

1845 WORDINGUE LIPOVE P.O. BOX 1407 MONUMENT, CO 80132 MR. RAHDY CELLETTE (719) 468-2525

KEY MAP

STATE HIGHWAY 108

48 HOURS BEFORE YOU DIG,
CALL UTILITY LOCATORS
1-800-922-1987
STY OF COLORADO SPRINGS DEPT. OF UTILITIES

GAS, ELECTRIC, WATER AND WASTEWATER

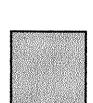
1 100% DD SET - NOT FOR CONSTRUCTION
2 FINAL PLAT REVISIONS

CONSULTING ENGINEERS & SURVEYORS

MONUMENT ACADEMY CIVIL CONSTRUCTION PLANS PRIVATE STORM SEWER PLANS ESIGNED BY JAW SCALE

DATE 08/16/07 RAWN BY JWW (H) !"= YARKES SHEET | OF 3 Colorado Springs, Colorado 80919 (1947)195-0794(Fax) CHECKED BY

Suite 300, Denver Colorado 80203 303, 861, 1600 tel



PROJECT: MONUMENT ACADEMY CHARTER

SCHOOL

Shelin Group, Inc. 6638 W. Onoway A Utileton, CO 80128 303 973 0757 (303 973 3256

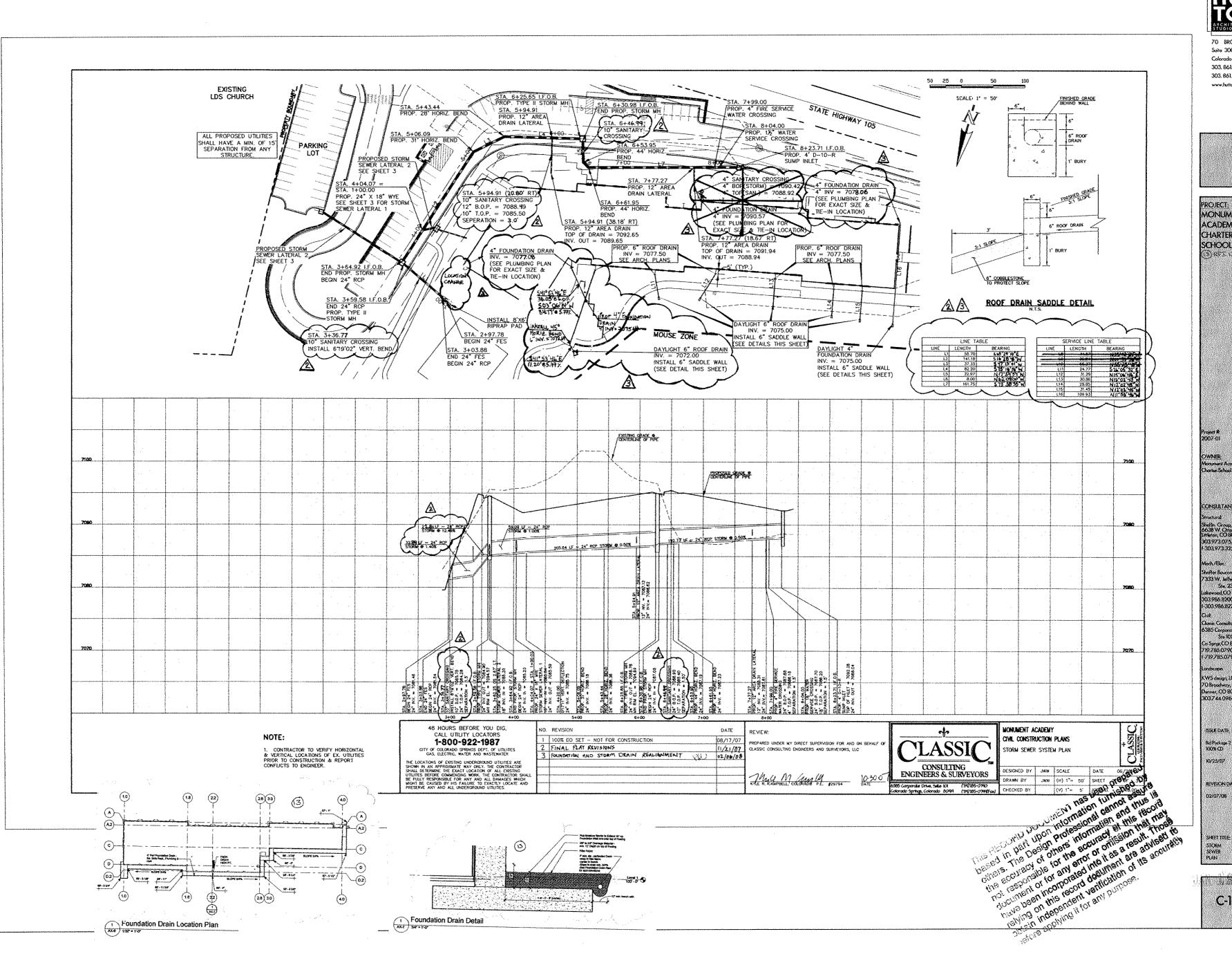
303.986.8200

Co Sprgs,CO 80719 719:785-0790 7-719:785-0799

KWS design, ILC 70 Broadway, See 3SC Dervier, CO 80203 303.744.0966

ISSUEDATE. BidPockings 2 100% CD

10/25/07



70 BROADWAY Suite 300, Denver Colorado 80203 303, 861, 1600 tel 303, 861, 1602 fax www.huttonford.com



PROJECT: MONUMENT ACADEMY CHARTER SCHOOL (3) RFI 13 - VIEVCE

Monument Acad Charles-School

CONSULTANTS: Shellin Group, Inc. 6638 W Orlows A. 19theron, CO 80178 303,973,0757

F303.973.3250

Shelfer Boucon Eng. 7333 W. Jefferson A Sis. 230 66d.CO 80235

1-303.986.8222 Classic Consultants 6385 Corporate Dr. Ste 101 Co Sprgs CO 80919 719:785.0790

1-719-785-0799 KWS design, ILC 70 Broadway, Ste 350 Deriver, CO 80203 303744.0966

ISSLEDATE

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NOTE: 1. CONTRACTOR TO VERIFY HORIZONTAL & VERTICAL LOCATIONS OF EX. UTILITIES PRIOR TO CONSTRUCTION & REPORT CONFLICTS TO ENGINEER. STA 1+02.67 LF O.B. REPRAP PAD STA. 1+13.66 BEGIN 16" FES $\sqrt{2}\sqrt{3}$ SCALE: 1' = 50' EXISTING GRADE OF PER 100-TRINGE # 7076 CENTRALINE OF PIPE MONUMENT ACADEMY 48 HOURS BEFORE YOU DIG, DATE REVIEW. CALL UTILITY LOCATORS

1-800-922-1987

GITY OF COLORDOS SPRINGS DEPT. OF UTILITIES
GAS. ELECTRIC, WATER AND WASTEWATER CIVIL CONSTRUCTION PLANS 08/17/07 100% DO SET - NOT FOR CONSTRUCTION PREPARED UNDER MY DIRECT SUPERMISION FOR AND ON BEHALF C CLASSIC CONSULTING ENGINEERS AND SURVEYORS, LLC The Record of th 2 FINAL PLAT REVISIONS 11/21/07 LATERALS 1, 2 & OUTFALL 3 FOUNDATION AND STORM DRAIN REALIGNMENT THE LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY ONLY. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, THE CONTRACTOR SHOWN OF A SHALL SHOW OF A SHALL SHOW OF A SHALL SHOW OF A SHALL SHOW OF A SHALL 6365 Corporate Drive, Suite 101 (719)785-0790 CHECKED BY (719)785-0799(Fax)

70 BROADWAY Suite 300, Denver Colorado 80203 303, 861, 1600 tel 303, 861, 1602 fex

PROJECT: MONUMENT ACADEMY CHARTER SCHOOL

Project #. 2007-01

CONSULTANTS Shelfin Group, Inc. 5638 W. Ortone A. Unistan, CO 80128 303.973.0757 † 303.973.3250

ech/Elec: Seaffer Boucom Eng. 7-333 W. Jefferson Ave. See, 2-30 Lokeward, CO-80205 303-986-8220 1-303-986-8222

Classic Consultants 6385 Corporate Dr Sie 101 Co Sproj, CO 90919 719:785.0790 (-719,785,0799

KWS design, U.C. 70 Broadway, Sie 350 Denver, CO 80203 303.744.0966

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REVISION DATES.

SHEET TITLE:

Appendix C

Soils Report



NRCS

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for El Paso County Area, Colorado

Monument Academy



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2 053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Map Unit Polygons

-

Soil Map Unit Lines

Soil Map Unit Points

Special Point Features

(0)

Blowout

 \boxtimes

Borrow Pit

36

Clay Spot

364

Closed Depression

Š

.

۰

Gravelly Spot

0

Landfill Lava Flow

Gravel Pit

٨

Marsh or swamp

2

Mine or Quarry

_

Miscellaneous Water
Perennial Water

0

Rock Outcrop

4

Saline Spot

. .

Sandy Spot

_

Severely Eroded Spot

.

Sinkhole

8

Slide or Slip

Ø

Sodic Spot

۵

Spoil Area Stony Spot



Very Stony Spot



Wet Spot Other



Special Line Features

Water Features

_

Streams and Canals

Transportation

ransp

Rails

~

Interstate Highways

~

US Routes



Major Roads



Local Roads

Background

Marie Control

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24.000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service Web Soil Survey URL:

Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 19, Aug 31, 2021

Soil map units are labeled (as space allows) for map scales 1:50.000 or larger.

Date(s) aerial images were photographed: Aug 19, 2018—Sep 23, 2018

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
1	Alamosa loam, 1 to 3 percent slopes	25.0	42.7%
92	Tomah-Crowfoot loamy sands, 3 to 8 percent slopes	33.5	57.3%
Totals for Area of Interest		58.5	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however,

Custom Soil Resource Report

onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

El Paso County Area, Colorado

1—Alamosa loam, 1 to 3 percent slopes

Map Unit Setting

National map unit symbol: 3670 Elevation: 7,200 to 7,700 feet

Farmland classification: Prime farmland if irrigated and reclaimed of excess salts

and sodium

Map Unit Composition

Alamosa and similar soils: 85 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Alamosa

Setting

Landform: Flood plains, fans Down-slope shape: Linear Across-slope shape: Linear Parent material: Alluvium

Typical profile

A - 0 to 6 inches: loam

Bt - 6 to 14 inches: clay loam Btk - 14 to 33 inches: clay loam

Cg1 - 33 to 53 inches: sandy clay loam Cg2 - 53 to 60 inches: sandy loam

Properties and qualities

Slope: 1 to 3 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Poorly drained

Runoff class: Very high

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20

to 0.60 in/hr)

Depth to water table: About 12 to 18 inches

Frequency of flooding: FrequentNone

Frequency of ponding: None

Calcium carbonate, maximum content: 5 percent

Maximum salinity: Very slightly saline to strongly saline (2.0 to 16.0 mmhos/cm)

Available water supply, 0 to 60 inches: High (about 10.2 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 5w

Hydrologic Soil Group: D

Ecological site: R048AY241CO - Mountain Meadow

Hydric soil rating: Yes

Minor Components

Other soils

Percent of map unit: Hydric soil rating: No

92—Tomah-Crowfoot loamy sands, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 36b9 Elevation: 7,300 to 7,600 feet

Farmland classification: Not prime farmland

Map Unit Composition

Tomah and similar soils: 50 percent Crowfoot and similar soils: 30 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Tomah

Setting

Landform: Hills, alluvial fans

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Alluvium derived from arkose and/or residuum weathered from

arkose

Typical profile

A - 0 to 10 inches: loamy sand E - 10 to 22 inches: coarse sand

Bt - 22 to 48 inches: stratified coarse sand to sandy clay loam

C - 48 to 60 inches: coarse sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high

(0.60 to 2.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 4.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4e

Hydrologic Soil Group: B

Ecological site: R049XY216CO - Sandy Divide

Hydric soil rating: No

Description of Crowfoot

Setting

Landform: Alluvial fans, hills

Landform position (three-dimensional): Side slope, crest

Down-slope shape: Linear Across-slope shape: Linear Parent material: Alluvium

Typical profile

A - 0 to 12 inches: loamy sand E - 12 to 23 inches: sand

Bt - 23 to 36 inches: sandy clay loam C - 36 to 60 inches: coarse sand

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high

(0.60 to 2.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 4.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4e

Hydrologic Soil Group: B

Ecological site: R049XY216CO - Sandy Divide

Hydric soil rating: No

Minor Components

Other soils

Percent of map unit: Hydric soil rating: No

Pleasant

Percent of map unit: Landform: Depressions Hydric soil rating: Yes

Soil Information for All Uses

Soil Properties and Qualities

The Soil Properties and Qualities section includes various soil properties and qualities displayed as thematic maps with a summary table for the soil map units in the selected area of interest. A single value or rating for each map unit is generated by aggregating the interpretive ratings of individual map unit components. This aggregation process is defined for each property or quality.

Soil Erosion Factors

Soil Erosion Factors are soil properties and interpretations used in evaluating the soil for potential erosion. Example soil erosion factors can include K factor for the whole soil or on a rock free basis, T factor, wind erodibility group and wind erodibility index.

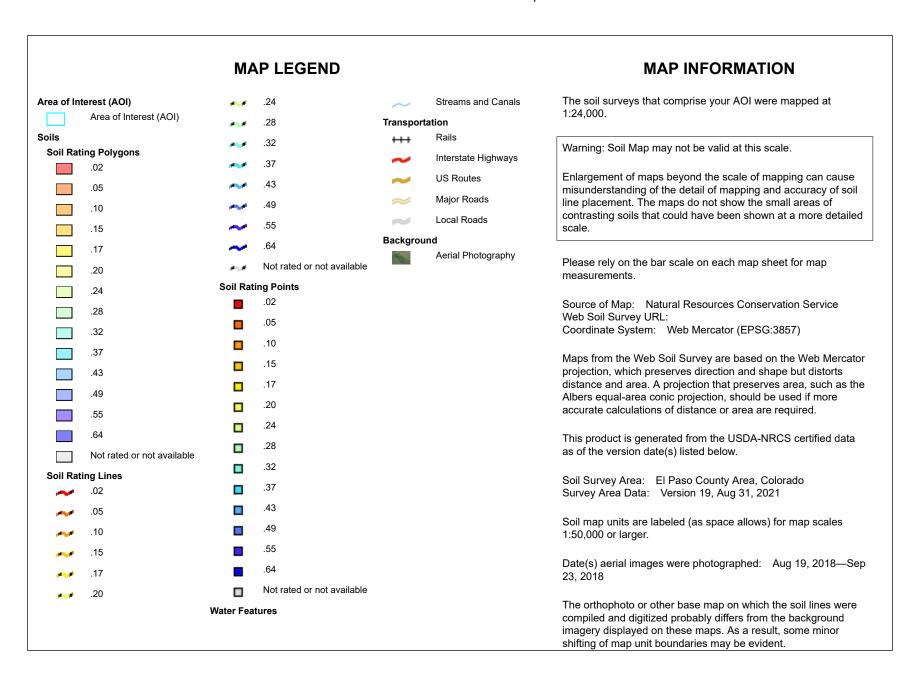
K Factor, Whole Soil

Erosion factor K indicates the susceptibility of a soil to sheet and rill erosion by water. Factor K is one of six factors used in the Universal Soil Loss Equation (USLE) and the Revised Universal Soil Loss Equation (RUSLE) to predict the average annual rate of soil loss by sheet and rill erosion in tons per acre per year. The estimates are based primarily on percentage of silt, sand, and organic matter and on soil structure and saturated hydraulic conductivity (Ksat). Values of K range from 0.02 to 0.69. Other factors being equal, the higher the value, the more susceptible the soil is to sheet and rill erosion by water.

"Erosion factor Kw (whole soil)" indicates the erodibility of the whole soil. The estimates are modified by the presence of rock fragments.

Factor K does not apply to organic horizons and is not reported for those layers.





Table—K Factor, Whole Soil

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
1	Alamosa loam, 1 to 3 percent slopes	.20	25.0	42.7%
92	Tomah-Crowfoot loamy sands, 3 to 8 percent slopes	.17	33.5	57.3%
Totals for Area of Interes	st	58.5	100.0%	

Rating Options—K Factor, Whole Soil

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Higher

Layer Options (Horizon Aggregation Method): Surface Layer (Not applicable)

Soil Qualities and Features

Soil qualities are behavior and performance attributes that are not directly measured, but are inferred from observations of dynamic conditions and from soil properties. Example soil qualities include natural drainage, and frost action. Soil features are attributes that are not directly part of the soil. Example soil features include slope and depth to restrictive layer. These features can greatly impact the use and management of the soil.

Hydrologic Soil Group

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained

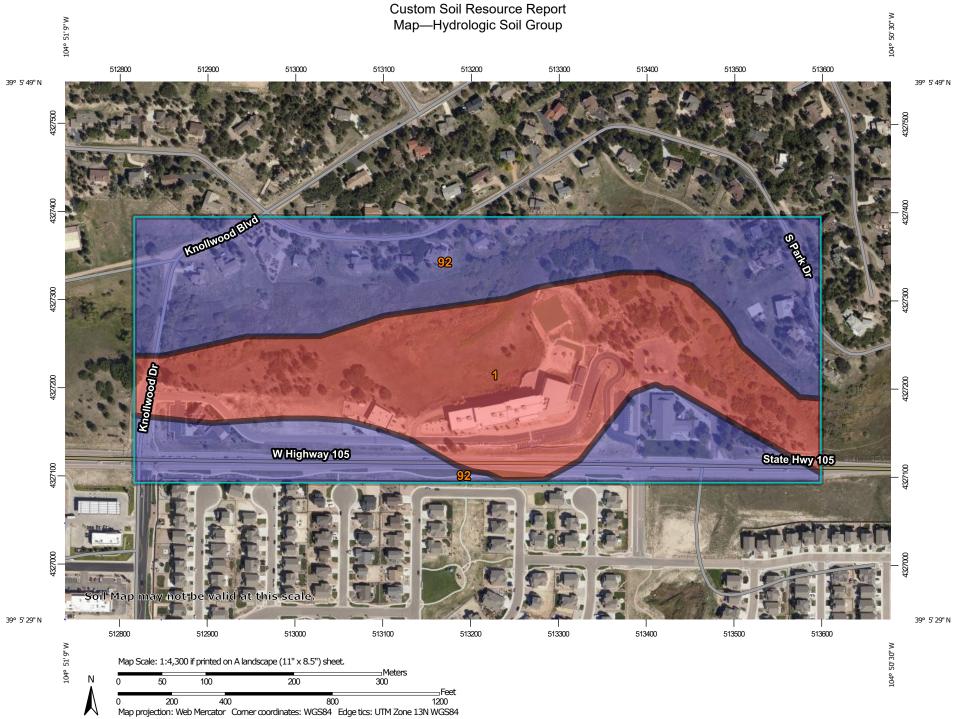
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soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.



MAP LEGEND MAP INFORMATION Area of Interest (AOI) The soil surveys that comprise your AOI were mapped at С 1:24.000. Area of Interest (AOI) C/D Soils D Warning: Soil Map may not be valid at this scale. Soil Rating Polygons Not rated or not available Α Enlargement of maps beyond the scale of mapping can cause **Water Features** A/D misunderstanding of the detail of mapping and accuracy of soil Streams and Canals line placement. The maps do not show the small areas of В contrasting soils that could have been shown at a more detailed Transportation scale. B/D Rails ---Interstate Highways Please rely on the bar scale on each map sheet for map C/D **US Routes** measurements. Major Roads Source of Map: Natural Resources Conservation Service Not rated or not available Local Roads Web Soil Survey URL: -Coordinate System: Web Mercator (EPSG:3857) Soil Rating Lines Background Aerial Photography Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. Soil Survey Area: El Paso County Area, Colorado Not rated or not available Survey Area Data: Version 19, Aug 31, 2021 **Soil Rating Points** Soil map units are labeled (as space allows) for map scales Α 1:50.000 or larger. A/D Date(s) aerial images were photographed: Aug 19, 2018—Sep 23. 2018 B/D The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Table—Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
1	Alamosa loam, 1 to 3 percent slopes	D	25.0	42.7%
92	Tomah-Crowfoot loamy sands, 3 to 8 percent slopes	В	33.5	57.3%
Totals for Area of Interes	st		58.5	100.0%

Rating Options—Hydrologic Soil Group

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Higher

Soil Reports

The Soil Reports section includes various formatted tabular and narrative reports (tables) containing data for each selected soil map unit and each component of each unit. No aggregation of data has occurred as is done in reports in the Soil Properties and Qualities and Suitabilities and Limitations sections.

The reports contain soil interpretive information as well as basic soil properties and qualities. A description of each report (table) is included.

Soil Erosion

This folder contains a collection of tabular reports that present soil erosion factors and groupings. The reports (tables) include all selected map units and components for each map unit. Soil erosion factors are soil properties and interpretations used in evaluating the soil for potential erosion. Example soil erosion factors can include K factor for the whole soil or on a rock free basis, T factor, wind erodibility group and wind erodibility index.

RUSLE2 Related Attributes

This report summarizes those soil attributes used by the Revised Universal Soil Loss Equation Version 2 (RUSLE2) for the map units in the selected area. The report includes the map unit symbol, the component name, and the percent of the component in the map unit. Soil property data for each map unit component include the hydrologic soil group, erosion factor Kf for the surface horizon, erosion factor T, and the representative percentage of sand, silt, and clay in the mineral surface horizon. Missing surface data may indicate the presence of an organic layer.

Report—RUSLE2 Related Attributes

Soil properties and interpretations for erosion runoff calculations. The surface mineral horizon properties are displayed or the first mineral horizon below an organic surface horizon. Organic horizons are not displayed.

RUSLE2 Related Attributes-El Paso County Area, Colorado													
Map symbol and soil name	Pct. of	Slope	Hydrologic group	Kf	T factor	Repre	esentative	value					
	map unit	length (ft)				% Sand	% Silt	% Clay					
1—Alamosa loam, 1 to 3 percent slopes													
Alamosa	85	_	D	.20	5	43.0	38.5	18.5					
92—Tomah-Crowfoot loamy sands, 3 to 8 percent slopes													
Tomah	50	_	В	.17	4	84.3	9.2	6.5					
Crowfoot	30	_	В	.15	3	85.7	4.3	10.0					

References

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.

Federal Register. July 13, 1994. Changes in hydric soils of the United States.

Federal Register. September 18, 2002. Hydric soils of the United States.

Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.

National Research Council. 1995. Wetlands: Characteristics and boundaries.

Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2 054262

Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2 053577

Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2 053580

Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.

United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.

United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/home/?cid=nrcs142p2 053374

United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/landuse/rangepasture/?cid=stelprdb1043084

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United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf

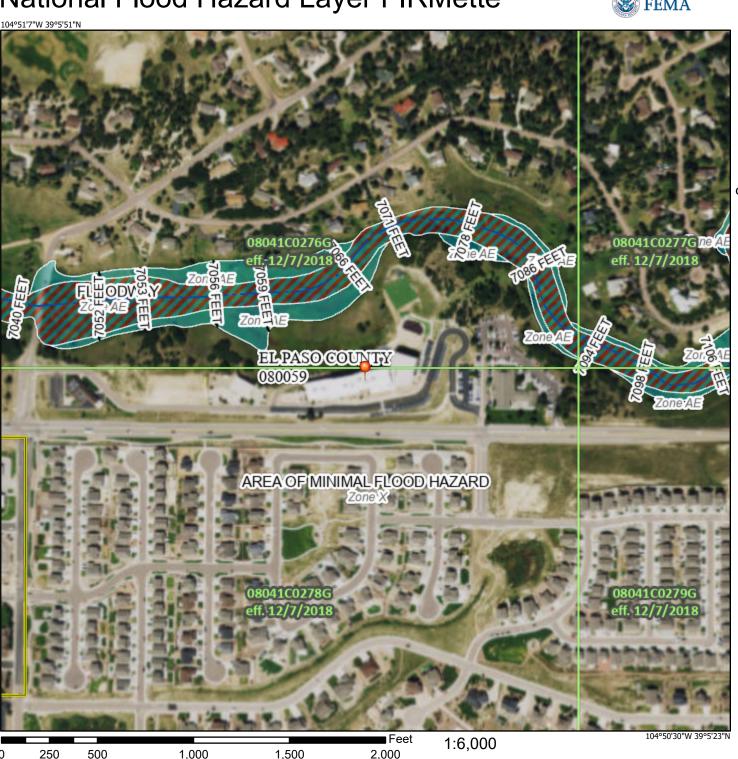
Appendix D

Floodplain Map

National Flood Hazard Layer FIRMette



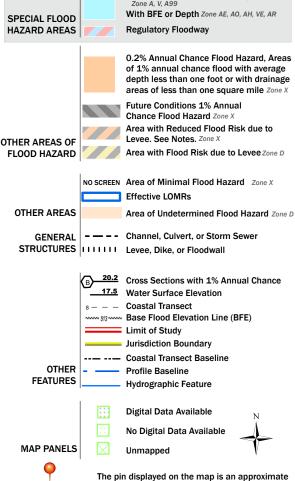
Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020



Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

Without Base Flood Elevation (BFE)



This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap

accuracy standards

point selected by the user and does not represent

an authoritative property location.

The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 1/12/2022 at 3:38 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

Appendix E

Precipitation Data



POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

PF tabular | PF graphical | Maps & aerials

PF tabular

PDS-	based poi	nt precipi	tation frec	uency es	timates w	ith 90% co	onfidence	intervals	(in inches	/hour) ¹
				•	ge recurren				•	<u> </u>
Duration	1	2	5	10	25	50	100	200	500	1000
5-min	2.88 (2.36-3.49)	3.65 (2.99-4.44)	4.94 (4.04-6.04)	6.06 (4.92-7.42)	7.64 (5.99-9.66)	8.89 (6.80-11.4)	10.2 (7.52-13.3)	11.5 (8.16-15.3)	13.4 (9.07-18.2)	14.8 (9.78-20.3)
10-min	2.11 (1.73-2.56)	2.67 (2.19-3.25)	3.62 (2.96-4.42)	4.43 (3.60-5.43)	5.59 (4.39-7.07)	6.52 (4.98-8.31)	7.46 (5.51-9.71)	8.45 (5.97-11.2)	9.79 (6.65-13.3)	10.8 (7.16-14.9)
15-min	1.71 (1.40-2.08)	2.17 (1.78-2.64)	2.94 (2.40-3.59)	3.61 (2.93-4.41)	4.55 (3.57-5.75)	5.30 (4.05-6.76)	6.06 (4.48-7.90)	6.87 (4.85-9.13)	7.96 (5.40-10.8)	8.81 (5.82-12.1)
30-min	1.16 (0.948-1.40)	1.47 (1.20-1.78)	1.99 (1.62-2.43)	2.44 (1.98-2.98)	3.07 (2.41-3.88)	3.58 (2.73-4.56)	4.09 (3.02-5.33)	4.63 (3.27-6.16)	5.37 (3.64-7.30)	5.94 (3.92-8.16)
60-min	0.735 (0.602-0.893)	0.903 (0.740-1.10)	1.20 (0.978-1.46)	1.46 (1.19-1.79)	1.85 (1.46-2.36)	2.18 (1.67-2.80)	2.52 (1.87-3.30)	2.89 (2.05-3.86)	3.41 (2.32-4.65)	3.82 (2.52-5.25)
2-hr	0.446 (0.368-0.538)	0.536 (0.442-0.647)	0.701 (0.576-0.848)	0.854 (0.696-1.04)	1.09 (0.867-1.38)	1.28 (0.996-1.64)	1.50 (1.12-1.95)	1.73 (1.24-2.31)	2.07 (1.42-2.81)	2.34 (1.55-3.19)
3-hr	0.332 (0.275-0.399)	0.390 (0.323-0.469)	0.501 (0.413-0.603)	0.608 (0.498-0.734)	0.777 (0.626-0.990)	0.925 (0.722-1.18)	1.09 (0.819-1.42)	1.27 (0.914-1.69)	1.53 (1.06-2.09)	1.75 (1.17-2.38)
6-hr	0.201 (0.168-0.239)	0.233 (0.194-0.277)	0.295 (0.245-0.352)	0.357 (0.294-0.427)	0.456 (0.371-0.579)	0.545 (0.429-0.694)	0.644 (0.488-0.835)	0.754 (0.547-0.999)	0.916 (0.638-1.24)	1.05 (0.706-1.42)
12-hr	0.121 (0.101-0.142)	0.142 (0.118-0.167)	0.181 (0.151-0.214)	0.218 (0.181-0.259)	0.277 (0.225-0.347)	0.328 (0.259-0.413)	0.384 (0.292-0.492)	0.445 (0.325-0.584)	0.535 (0.374-0.716)	0.608 (0.411-0.816)
24-hr	0.073 (0.061-0.085)	0.086 (0.072-0.101)	0.110 (0.092-0.129)	0.132 (0.110-0.155)	0.165 (0.135-0.204)	0.194 (0.153-0.240)	0.224 (0.171-0.284)	0.257 (0.188-0.333)	0.304 (0.213-0.403)	0.342 (0.233-0.456)
2-day	0.043 (0.036-0.050)	0.050 (0.043-0.058)	0.064 (0.054-0.074)	0.075 (0.063-0.088)	0.093 (0.076-0.114)	0.108 (0.086-0.133)	0.124 (0.095-0.156)	0.141 (0.104-0.182)	0.166 (0.117-0.218)	0.185 (0.127-0.245)
3-day	0.031 (0.026-0.036)	0.036 (0.031-0.042)	0.046 (0.039-0.053)	0.054 (0.046-0.063)	0.067 (0.055-0.081)	0.077 (0.062-0.095)	0.088 (0.068-0.110)	0.100 (0.074-0.128)	0.117 (0.083-0.153)	0.130 (0.090-0.172)
4-day	0.025 (0.021-0.028)	0.029 (0.025-0.033)	0.036 (0.031-0.042)	0.043 (0.036-0.050)	0.053 (0.043-0.064)	0.061 (0.049-0.074)	0.069 (0.054-0.086)	0.079 (0.058-0.100)	0.091 (0.065-0.119)	0.102 (0.070-0.134)
7-day	0.016 (0.014-0.019)	0.019 (0.016-0.022)	0.023 (0.020-0.027)	0.027 (0.023-0.032)	0.033 (0.028-0.040)	0.038 (0.031-0.046)	0.044 (0.034-0.054)	0.049 (0.037-0.062)	0.057 (0.041-0.074)	0.063 (0.044-0.083)
10-day	0.013 (0.011-0.015)	0.015 (0.013-0.017)	0.018 (0.016-0.021)	0.021 (0.018-0.024)	0.026 (0.021-0.031)	0.029 (0.024-0.035)	0.033 (0.026-0.041)	0.037 (0.028-0.047)	0.043 (0.031-0.056)	0.048 (0.033-0.062)
20-day	0.009 (0.007-0.010)	0.010 (0.009-0.011)	0.012 (0.010-0.014)	0.014 (0.012-0.016)	0.016 (0.014-0.019)	0.019 (0.015-0.022)	0.021 (0.016-0.025)	0.023 (0.017-0.029)	0.026 (0.019-0.034)	0.029 (0.020-0.037)
30-day	0.007 (0.006-0.008)	0.008 (0.007-0.009)	0.010 (0.008-0.011)	0.011 (0.009-0.012)	0.013 (0.011-0.015)	0.014 (0.012-0.017)	0.016 (0.013-0.019)	0.018 (0.013-0.022)	0.020 (0.014-0.025)	0.022 (0.015-0.028)
45-day	0.006 (0.005-0.006)	0.006 (0.006-0.007)	0.008 (0.007-0.009)	0.009 (0.008-0.010)	0.010 (0.009-0.012)	0.011 (0.009-0.013)	0.012 (0.010-0.015)	0.014 (0.010-0.017)	0.015 (0.011-0.019)	0.016 (0.011-0.021)
60-day	0.005 (0.004-0.005)	0.005 (0.005-0.006)	0.007 (0.006-0.007)	0.007 (0.006-0.008)	0.009 (0.007-0.010)	0.010 (0.008-0.011)	0.010 (0.008-0.012)	0.011 (0.008-0.014)	0.012 (0.009-0.015)	0.013 (0.009-0.017)

¹ Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

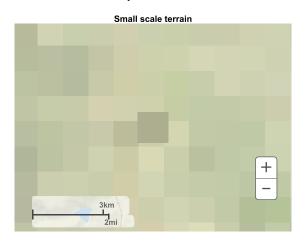
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PF graphical

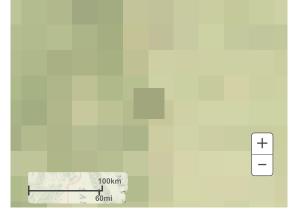


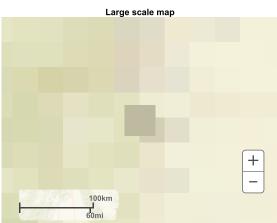
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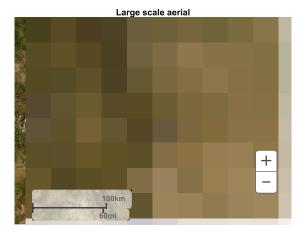
Maps & aerials



Large scale terrain







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US Department of Commerce
National Oceanic and Atmospheric Administration
National Weather Service
National Water Center
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

<u>Disclaimer</u>

Appendix F

Calculations

- 1. Rational Method
- 2. Bentley InRoads
- 3. FHWA Hydraulic Toolbox Riprap
 - 4. Permanent Water Quality

Corridor / Design Package: Hwy 105 / Monument Academy

System Name: Monument Academy

Computed: AMV Checked: JLS

Date: 3/10/2022 UpDated: 6/2/2022
Date: 3/23/2022 UpDated: 6/7/2022

	Sub-Basin Da	ta		Co	mposite	e C	Stree	ts (Paved) or Roof	s 100%	Unde	veloped A	reas or Pa	rks 7%	Busine	Business (Neighborhood Ar		
Basin ID	Description	Hydr Soils Group	Total Area (ac)	C ₅	C ₁₀	C ₁₀₀	C ₅	C ₁₀	C ₁₀₀	Area (ac)	C ₅	C ₁₀	C ₁₀₀	Area (ac)	C ₅	C ₁₀	C ₁₀₀	Area (ac)
Existing																		
A-EX-1	Front Circulation Road	D	1.08	0.58	0.64	0.76	0.90	0.92	0.96	0.5939	0.19	0.29	0.52	0.487	0.53	0.57	0.68	
A-EX-2	Front of Building(s)	D	0.35	0.71	0.74	0.81	0.90	0.92	0.96	0.1656	0.19	0.29	0.52		0.53	0.57	0.68	0.182
A-EX-3	Roof drains to Front	D	0.32	0.90	0.92	0.96	0.90	0.92	0.96	0.3191	0.19	0.29	0.52		0.53	0.57	0.68	
A-EX-4	Playground	D	0.60	0.52	0.56	0.69	0.90	0.92	0.96	0.0840	0.19	0.29	0.52	0.117	0.53	0.57	0.68	0.403
A-EX-5	Front Circulation Road	B/D	1.79	0.53	0.59	0.72	0.90	0.92	0.96	0.7151	0.19	0.29	0.52	0.764	0.53	0.57	0.68	0.312
A-EX-6	Upper Parking Lot	В	0.43	0.83	0.85	0.90	0.90	0.92	0.96	0.3528	0.12	0.20	0.39		0.49	0.53	0.62	0.077
A-EX-7	Roof drains to Back	D	0.57	0.90	0.92	0.96	0.90	0.92	0.96	0.5738	0.19	0.29	0.52		0.53	0.57	0.68	
A-EX-8	Channel & undeveloped	D	0.81	0.21	0.31	0.53	0.90	0.92	0.96		0.19	0.29	0.52	0.758	0.53	0.57	0.68	0.050
A-EX-9	Athletic field & undeveloped	D	0.77	0.24	0.33	0.55	0.90	0.92	0.96	0.0111	0.19	0.29	0.52	0.657	0.53	0.57	0.68	0.097
A-EX-10	Back of School Road	D	0.98	0.60	0.65	0.76	0.90	0.92	0.96	0.4488	0.19	0.29	0.52	0.274	0.53	0.57	0.68	0.262
		subtotal	7.70							3.2642				3.06				1.38
Proposed																		1
A-0	Front Circulation Road	B/D	0.31	0.56	0.62	0.75	0.90	0.92	0.96	0.1616	0.19	0.29	0.52	0.147	0.53	0.57	0.68	
A-1	Front Circulation Road	D	0.75	0.77	0.81	0.88	0.90	0.92	0.96	0.6208	0.19	0.29	0.52	0.133	0.53	0.57	0.68	
A-EX-2	Front of Building(s)	D	0.35	0.71	0.74	0.81	0.90	0.92	0.96	0.1656	0.19	0.29	0.52		0.53	0.57	0.68	0.182
A-EX-3	Roof drains to Front	D	0.32	0.90	0.92	0.96	0.90	0.92	0.96	0.3191	0.19	0.29	0.52		0.53	0.57	0.68	
A-4	Playground	D	0.52	0.54	0.58	0.70	0.90	0.92	0.96	0.0719	0.19	0.29	0.52	0.063	0.53	0.57	0.68	0.388
A-5	Front Circulation Road	B/D	1.59	0.55	0.61	0.74	0.90	0.92	0.96	0.7092	0.19	0.29	0.52	0.668	0.53	0.57	0.68	0.217
A-6	Upper Parking Lot	В	0.42	0.83	0.86	0.90	0.90	0.92	0.96	0.3545	0.12	0.20	0.39		0.49	0.53	0.62	0.070
A-EX-7	Roof drains to Back	D	0.57	0.90	0.92	0.96	0.90	0.92	0.96	0.5738	0.19	0.29	0.52		0.53	0.57	0.68	
A-8	Infield & Back Circ.Rd.	D	0.25	0.29	0.38	0.58	0.90	0.92	0.96	0.0264	0.19	0.29	0.52	0.210	0.53	0.57	0.68	0.018
A-9	Undeveloped & Back Circ.Rd.	D	1.05	0.32	0.41	0.60	0.90	0.92	0.96	0.1852	0.19	0.29	0.52	0.834	0.53	0.57	0.68	0.027
A-10	Athletic Field & Ditch	D	0.34	0.31	0.39	0.58	0.90	0.92	0.96	0.0115	0.19	0.29	0.52	0.236	0.53	0.57	0.68	0.097
A-11	Back of School & Circ. Rd.	D	1.09	0.68	0.72	0.81	0.90	0.92	0.96	0.6097	0.19	0.29	0.52	0.180	0.53	0.57	0.68	0.303
		subtotal	7.58							3.8093				2.47				1.00
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													-				1	

Standard Form SF-2 . Storm Drainage System Design (Rational Method Procedure)

Corridor / Design Package: Hwy 105 / Monument Academy

System Name: Monument Academy

 Computed:
 AMV
 Date:
 3/10/2022

 Checked:
 JLS
 Date:
 3/23/2022

Design Storm: 5-yr

		DIRECT RUNOFF					TOT	AL RUN	OFF			
Basin Area Description	AREA DESIGN (name)	AREA (AC)	RUNOFF	t _c (MIN)	C.A. (AC)	I IN / HR	a (CFS)	t _c (MIN)	SUM (C*A) (AC)	I (IN / HR)	Q (CFS)	REMARKS
(1)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	
Front Circulation Road	A-EX-1	1.081	0.58	5.00	0.63	4.94	3.10					
Front of Building(s)	A-EX-2	0.347	0.71	5.00	0.25	4.94	1.21					
Roof drains to Front	A-EX-3	0.319	0.90	5.00	0.29	4.94	1.42					
Playground	A-EX-4	0.604	0.52	5.00	0.31	4.94	1.54	SEE I	NPOAR	S CAL	CULATIONS	
Front Circulation Road	A-EX-5	1.791	0.53	5.00	0.95	4.94	4.71			OW RO		
Upper Parking Lot	A-EX-6	0.430	0.83	5.00	0.36	4.94	1.75] '	UK FL	OVV RU	UTING	
Roof drains to Back	A-EX-7	0.574	0.90	5.00	0.52	4.94	2.55	1				
Channel & undeveloped	A-EX-8	0.808	0.21	5.00	0.17	4.94	0.84					
Athletic field & undeveloped	A-EX-9	0.766	0.24	5.00	0.19	4.94	0.92					
Back of School Road	A-EX-10	0.985	0.60	5.00	0.59	4.94	2.94					
5 (0) (1) 5	1.0	0.000	0.50	5.00	0.47	4.04	0.00					
Front Circulation Road	A-0	0.309	0.56	5.00	0.17	4.94	0.86					
Front Circulation Road	A-1	0.754	0.77	5.00	0.58	4.94	2.88					
Front of Building(s)	A-EX-2	0.347	0.71	5.00	0.25	4.94	1.21					
Roof drains to Front	A-EX-3	0.319	0.90	5.00	0.29	4.94	1.42					
Playground	A-4	0.523	0.54	5.00	0.28	4.94	1.40	SEEI	NROAD	S CAL	CULATIONS	
Front Circulation Road	A-5	1.594	0.55	5.00	0.88	4.94	4.35	_		OW RO		
Upper Parking Lot	A-6	0.424	0.83	5.00	0.35	4.94	1.74	•				
Roof drains to Back	A-EX-7	0.574	0.90	5.00	0.52	4.94	2.55					
Infield & Back Circ.Rd.	A-8	0.254	0.29	5.00	0.07	4.94	0.36					
Undeveloped & Back Circ.Rd.	A-9	1.046	0.32	5.00	0.34	4.94	1.68					
Athletic Field & Ditch	A-10	0.345	0.31	5.00	0.11	4.94	0.53					
Back of School & Circ. Rd.	A-11	1.093	0.68	5.00	0.74	4.94	3.67	1				
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1	11	Basin	Descrir	otion	linked	tο	C-Value	Shee	

- (14) Additional Street Overland Flow
- (15) Additonal Street Overland Flow
- (16) Design Pipe Flow
- (17) Pipe Slope
- (18) Pipe Size

⁽²⁾ Basin Design Point (not designated)

⁽³⁾ Enter the Basin Name from C Value Sheet

⁽⁴⁾ Basin Area linked to C-Value Sheet

⁽⁵⁾ Composite C linked to C-Value Sheet

⁽⁶⁾ Time of Concentration linked to C-Value Sheet

^{(7) =}Column 4 x Column 5

^{(8) =28.5*}P/(10+Column 6)^0.786

^{(9) =}Column 7 x Column 8

^{(10) =}Column 6 + Column 21

⁽¹¹⁾ Add the Basin Areas (7) to get the combined basin AC

^{(12) =28.5*}P/(10+Column 10)^0.786

⁽¹³⁾ Sum of Qs

Standard Form SF-2 . Storm Drainage System Design (Rational Method Procedure)

Corridor / Design Package: Hwy 105 / Monument Academy

System Name: Monument Academy

 Computed:
 AMV
 Date:
 3/10/2022

 Checked:
 JLS
 Date:
 3/23/2022

Design Storm: 10-yr

		DIRECT RUNOFF							TOTA	L RUN	OFF	
Basin Area Description	AREA DESIGN (name)	AREA (AC)	RUNOFF	t_c (MIN)	C.A. (AC)	I IN / HR	Q (CFS)	t_c (MIN)	SUM (C*A) (AC)	I (IN / HR)	a (CFS)	REMARKS
(1)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	
Front Circulation Road	A-EX-1	1.08	0.64	5.00	0.69	6.06	4.17					
Front of Building(s)	A-EX-2	0.35	0.74	5.00	0.26	6.06	1.55					
Roof drains to Front	A-EX-3	0.32	0.92	5.00	0.29	6.06	1.78					
Playground	A-EX-4	0.60	0.56	5.00	0.34	6.06	2.07					
Front Circulation Road	A-EX-5	1.79	0.59	5.00	1.06	6.06	6.41	SEE	NROAD	S CALC	ULATIONS	
Upper Parking Lot	A-EX-6	0.43	0.85	5.00	0.37	6.06	2.21	F	OR FL	OW RO	JTING	
Roof drains to Back	A-EX-7	0.57	0.92	5.00	0.53	6.06	3.20					
Channel & undeveloped	A-EX-8	0.81	0.31	5.00	0.25	6.06	1.50					
Athletic field & undeveloped	A-EX-9	0.77	0.33	5.00	0.26	6.06	1.55					
Back of School Road	A-EX-10	0.98	0.65	5.00	0.64	6.06	3.89					
Front Circulation Road	A-0	0.309	0.62	5.00	0.19	6.06	1.16					
Front Circulation Road	A-1	0.754	0.81	5.00	0.61	6.06	3.69					
Front of Building(s)	A-EX-2	0.347	0.74	5.00	0.26	6.06	1.55					
Roof drains to Front	A-EX-3	0.319	0.92	5.00	0.29	6.06	1.78					
Playground	A-4	0.5229	0.58	5.00	0.31	6.06	1.85					
Front Circulation Road	A-5	1.5942	0.608	5.00	0.97	6.06	5.88	_			ULATIONS	
Upper Parking Lot	A-6	0.4240	0.856	5.00	0.36	6.06	2.20	,	OR FL	OW RO	JING	
Roof drains to Back	A-EX-7	0.5738	0.920	5.00	0.53	6.06	3.20					
Infield & Back Circ.Rd.	A-8	0.2541	0.376	5.00	0.10	6.06	0.58					
Undeveloped & Back Circ.Rd.	A-9	1.0460	0.409	5.00	0.43	6.06	2.59					
Athletic Field & Ditch	A-10	0.3447	0.390	5.00	0.13	6.06	0.81					
Back of School & Circ. Rd.	A-11	1.0928	0.719	5.00	0.79	6.06	4.76					
											l	

⁽²⁾ Basin Design Point (not designated)

- (17) Pipe Slope
- (18) Pipe Size

⁽³⁾ Enter the Basin Name from C Value Sheet

⁽⁴⁾ Basin Area linked to C-Value Sheet

⁽⁵⁾ Composite C linked to C-Value Sheet

⁽⁶⁾ Time of Concentration linked to C-Value Sheet

^{(8) =28.5*}P/(10+Column 6)^0.786

^{(9) =}Column 7 x Column 8

^{(10) =}Column 6 + Column 21

⁽¹¹⁾ Add the Basin Areas (7) to get the combined basin AC

^{(12) =28.5*}P/(10+Column 10)^0.786

⁽¹⁴⁾ Additional Street Overland Flow

⁽¹⁵⁾ Additional Street Overland Flow

⁽¹⁶⁾ Design Pipe Flow

Standard Form SF-2 . Storm Drainage System Design (Rational Method Procedure)

Corridor / Design Package: Hwy 105 / Monument Academy

System Name: Monument Academy

 Computed:
 AMV
 Date:
 3/10/2022

 Checked:
 JLS
 Date:
 3/23/2022

Design Storm: 100-yr

		DIRECT RUNOFF							TOTA	AL RUNG	OFF	
Basin Area Description	AREA DESIGN (name)	AREA (AC)	RUNOFF	t _c (MIN)	C.A. (AC)	I IN / HR	Q (CFS)	t_c (MIN)	SUM (C*A) (AC)	I (IN / HR)	a (CFS)	REMARKS
(1)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	
Front Circulation Road	A-EX-1	1.081	0.76	5.00	0.82	10.20	8.40					
Front of Building(s)	A-EX-2	0.347	0.81	5.00	0.28	10.20	2.88					
Roof drains to Front	A-EX-3	0.319	0.96	5.00	0.31	10.20	3.12					
Playground	A-EX-4	0.604	0.69	5.00	0.42	10.20	4.24					
Front Circulation Road	A-EX-5	1.791	0.72	5.00	1.30	10.20	13.22	SEEI	NROAD	S CALC	ULATIONS	
Upper Parking Lot	A-EX-6	0.430	0.90	5.00	0.39	10.20	3.94	1 1	FOR FL	OW RO	UTING	
Roof drains to Back	A-EX-7	0.574	0.96	5.00	0.55	10.20	5.62	1				
Channel & undeveloped	A-EX-8	0.808	0.53	5.00	0.43	10.20	4.37	1				
Athletic field & undeveloped	A-EX-9	0.766	0.55	5.00	0.42	10.20	4.27	1				
Back of School Road	A-EX-10	0.985	0.76	5.00	0.75	10.20	7.67					
Front Circulation Road	A-0	0.309	0.75	5.00	0.23	10.20	2.36					
Front Circulation Road	A-1	0.754	0.88	5.00	0.67	10.20	6.78	1				
Front of Building(s)	A-EX-2	0.347	0.81	5.00	0.28	10.20	2.88	1				
Roof drains to Front	A-EX-3	0.319	0.96	5.00	0.31	10.20	3.12	1				
Playground	A-4	0.523	0.70	5.00	0.37	10.20	3.73	l				
Front Circulation Road	A-5	1.594	0.74	5.00	1.18	10.20	11.99	_			ULATIONS	
Upper Parking Lot	A-6	0.424	0.90	5.00	0.38	10.20	3.91		FOR FL	OW RO	UTING	
Roof drains to Back	A-EX-7	0.574	0.96	5.00	0.55	10.20	5.62	1				
Infield & Back Circ.Rd.	A-8	0.254	0.58	5.00	0.15	10.20	1.50					
Undeveloped & Back Circ.Rd.	A-9	1.046	0.60	5.00	0.63	10.20	6.42					
Athletic Field & Ditch	A-10	0.345	0.58	5.00	0.20	10.20	2.04					
Back of School & Circ. Rd.	A-11	1.093	0.81	5.00	0.88	10.20	9.03					
Busicer control & Circ. 14d.	7. 11	1.000	0.01	0.00	0.00	10.20	0.00					
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⁽²⁾ Basin Design Point (not designated)

- (17) Pipe Slope
- (18) Pipe Size

⁽³⁾ Enter the Basin Name from C Value Sheet

⁽⁴⁾ Basin Area linked to C-Value Sheet

⁽⁵⁾ Composite C linked to C-Value Sheet

⁽⁶⁾ Time of Concentration linked to C-Value Sheet

^{(8) =28.5*}P/(10+Column 6)^0.786

^{(9) =}Column 7 x Column 8

^{(10) =}Column 6 + Column 21

⁽¹¹⁾ Add the Basin Areas (7) to get the combined basin AC

^{(12) =28.5*}P/(10+Column 10)^0.786

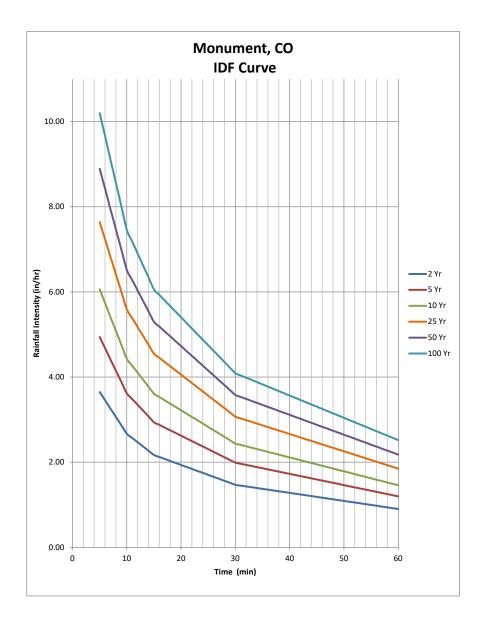
⁽¹⁴⁾ Additional Street Overland Flow

⁽¹⁵⁾ Additional Street Overland Flow

⁽¹⁶⁾ Design Pipe Flow

Monument	IDF	Computed:	AMV		Date:	1/12/2022
1		Checked:	JLS		Date:	2/2/2022
Time				/hr)		
(min)	2 Yr	5 Yr	10 Yr	25 Yr	50 Yr	100 Yr
5	3.65	4.94	6.06	7.64	8.89	10.20
6	3.45	4.68	5.73	7.23	8.42	9.65
7	3.26	4.41	5.41	6.82	7.94	9.10
8	3.06	4.15	5.08	6.41	7.47	8.56
9	2.87	3.88	4.76	6.00	6.99	8.01
10	2.67	3.62	4.43	5.59	6.52	7.46
11	2.57	3.48	4.27	5.38	6.28	7.18
12	2.47	3.35	4.10	5.17	6.03	6.90
13	2.37	3.21	3.94	4.97	5.79	6.62
14	2.27	3.08	3.77	4.76	5.54	6.34
15	2.17	2.94	3.61	4.55	5.30	6.06
16	2.12	2.88	3.53	4.45	5.19	5.93
17	2.08	2.81	3.45	4.35	5.07	5.80
18	2.03	2.75	3.38	4.25	4.96	5.67
19	1.98	2.69	3.30	4.16	4.84	5.53
20	1.94	2.62	3.22	4.06	4.73	5.40
21	1.89	2.56	3.14	3.96	4.61	5.27
22	1.84	2.50	3.06	3.86	4.50	5.14
23	1.80	2.43	2.99	3.76	4.38	5.01
24	1.75	2.37	2.91	3.66	4.27	4.88
25	1.70	2.31	2.83	3.56	4.15	4.75
26	1.66	2.24	2.75	3.46	4.04	4.62
27	1.61	2.18	2.67	3.37	3.92	4.48
28	1.56	2.12	2.60	3.27	3.81	4.35
29	1.52	2.05	2.52	3.17	3.69	4.22
30	1.47	1.99	2.44	3.07	3.58	4.09
31	1.45	1.96	2.41	3.03	3.53	4.04
32	1.43	1.94	2.37	2.99	3.49	3.99
33	1.41	1.91	2.34	2.95	3.44	3.93
34	1.39	1.88	2.31	2.91	3.39	3.88
35	1.38	1.86	2.28	2.87	3.35	3.83
36	1.36	1.83	2.24	2.83	3.30	3.78
37	1.34	1.81	2.21	2.79	3.25	3.72
38	1.32	1.78	2.18	2.74	3.21	3.67
39	1.30	1.75	2.15	2.70	3.16	3.62
40	1.28	1.73	2.11	2.66	3.11	3.57
41	1.26	1.70	2.08	2.62	3.07	3.51
42 43	1.24 1.22	1.67	2.05 2.02	2.58 2.54	3.02 2.97	3.46 3.41
43		1.65		2.54		
44	1.21 1.19	1.62 1.60	1.98 1.95	2.50 2.46	2.93 2.88	3.36 3.31
45	1.19	1.57	1.95	2.46	2.88	3.25
46	1.17	1.57	1.92	2.42	2.83	3.25
48	1.13	1.54	1.85	2.36	2.79	3.15
49	1.13	1.49	1.82	2.34	2.69	3.10
50	1.09	1.49	1.79	2.26	2.65	3.10
51	1.03	1.44	1.75	2.22	2.60	2.99
52	1.05	1.41	1.72	2.18	2.55	2.94
53	1.04	1.38	1.69	2.13	2.51	2.89
54	1.02	1.36	1.66	2.09	2.46	2.83
55	1.02	1.33	1.62	2.05	2.40	2.78
56	0.98	1.31	1.59	2.01	2.37	2.73
57	0.96	1.28	1.56	1.97	2.32	2.73
58	0.94	1.25	1.53	1.93	2.27	2.62
59	0.92	1.23	1.49	1.89	2.23	2.57
60	0.92	1.20	1.49	1.85	2.23	2.52
- 00	0.90	1.20	1.40	1.00	2.10	2.02





BENTLEY INROADS REPORT - 10 YEAR PROPOSED

Areas

Drainage Reports

Element Type: Area

Date: Friday, June 3, 2022 12:56:48 PM

Drainage Data File: 10y_105_School

Basin ID	Inlet ID	Peak Flow	Area	Runoff Coef	Intensity	Time of Conc
		(cfs)	(ac)	0	(in/h)	(min)
A-EX-1	-	4.17	1.08	0.64	6.06	5
A-EX-2	IN-EX-2	1.55	0.35	0.74	6.06	5
A-EX-3	IN-EX-3	1.78	0.32	0.92	6.06	5
A-EX-4	-	2.07	0.6	0.56	6.06	5
A-EX-5	-	6.41	1.79	0.59	6.06	5
A-EX-6	-	2.21	0.43	0.85	6.06	5
A-EX-7	-	3.2	0.57	0.92	6.06	5
A-EX-8	-	1.5	0.81	0.31	6.06	5
A-EX-9	-	1.55	0.77	0.33	6.06	5
A-0	IN-100	1.16	0.31	0.62	6.06	5
A-1	IN-EX-1	3.69	0.75	0.81	6.06	5
A-4	-	1.85	0.52	0.58	6.06	5
A-5	IN-EX-5	5.88	1.59	0.61	6.06	5
A-6	IN-EX-6	2.2	0.42	0.86	6.06	5
A-8	P-108	0.58	0.25	0.38	6.06	5
A-9	-	2.59	1.05	0.41	6.06	5
A-10	-	0.81	0.34	0.39	6.06	5

Inlets

Drainage Reports

Element Type: Inlet

Date: Friday, June 3, 2022 1:44:43 PM

Drainage Data File: 10y_105_School

ID	Туре	InletClass	GrateLen (ft)	GrateWid (ft)	Location	VaultShape	Clogging (%)	Bypass To	Bypass Flow (cfs)	Inlet Depr (in)	Spread (ft)
IN-100	Curb Opening	TYPE R	5	0	On-Grade	Box	30	IN-EX-1	0.22	3	2.784
IN-EX-1	Curb Opening	TYPE R	5	0	Sump	Box	30	N/A	0	3	4.694
IN-EX-2	Grate	Grate	1.33	1.33	On-Grade	Box	30	IN-EX-3	0.85	0	5.43
IN-EX-3	Grate	Grate	1.33	1.33	On-Grade	Box	30	IN-EX-4	1.57	0	6.273
IN-EX-4	Grate	Grate	1.33	1.33	On-Grade	Box	30	IN-EX-7	0	0	0
IN-EX-5	Curb Opening	TYPE R	5	0	Sump	Box	30	N/A	0	3	6.229
IN-EX-6	Curb Opening	TYPE R	5	0	On-Grade	Box	30	IN-EX-5	0.55	3	3.092
IN-EX-7	Grate	Grate	1.33	1.33	On-Grade	Box	30	-	0	0	0

Manholes

Drainage Reports

Element Type: Manhole

Date: Friday, June 3, 2022 1:44:58 PM

Drainage Data File: 10y_105_School

Manhole ID	Shape	Rim (ft)	Width (ft)	Length (ft)	Туре	Total Flow (cfs)	Headloss (ft)
MH-102	Circular	7077.2	4	4	13.28	0	0
MH-EX-1	Circular	7094.69	4	4	5.31	0	0
MH-EX-2	Circular	7094.17	4	4	13.36	0	0
MH-EX-3	Box	7083.93	3	6	13.36	0	0
UD-C-1	Circular	7077.19	0.5	0.5	0	0	0
UD-C-2	Circular	7078.99	0.5	0.5	0	0	0

Pipes

Drainage Reports

0 0

0

Element Type: Pipe 0

Date: Friday, June 3, 2022 1:45:27 PM 0

Drainage Data File: 10y_105_School

	0																		
Pipe ID	U/S ID	D/S ID	Material	Mannings n	Height	Width	Length	Invert In	Invert Out	Slope	Total Flow	Capacity	Velocity	Flow Depth	U/S HGL	D/S HGL	U/S EGL	D/S EGL	Headloss
	0	0	0 0	0	(in)	(in)	(ft)	(ft)	(ft)	(%)	(cfs)	(cfs)	(ft/s)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)
P-101	IN-100	IN-EX-1	RCP	0.013	18	18	39.35	7088.85	7087.08	4.5	0.94	22.28	6.24	0.21	7089.06	7087.29	7089.66	7087.89	0
P-102	MH-102	FREE_EXT	RCP	0.013	24	24	20.54	7068.21	7068	1	13.14	22.62	7.46	1.09	7069.3	7069.09	7070.16	7069.96	0
P-104	P-EX-19	FREE_EXT	RCP	0.013	18	18	20.78	7069.58	7068.4	5.67	0	0	0	0	0	0	0	0	0
P-108	FREE_ENT	FREE_EXT	RCP	0.013	18	18	61.5	7071.33	7068.36	4.83	0.58	23.09	5.51	0.16	7071.49	7068.52	7071.96	7068.99	0
P-EX-1	IN-EX-1	P-EX-3	RCP	0.013	24	24	45.85	7085.67	7085.44	0.5	4.8	16	4.45	0.75	7086.42	7086.19	7086.72	7086.49	0
P-EX-2	IN-EX-2	P-EX-3	RCP	0.013	12	12	18.74	7086	7085.44	3	0.7	6.17	5.19	0.23	7086.23	7086.11	7086.64	7086.53	0
P-EX-3	Y_JUNCT	P-EX-4	RCP	0.013	24	24	113.1	7085.44	7084.88	0.5	5.44	16	4.6	0.8	7086.24	7085.69	7086.57	7086.02	0
P-EX-4	P-EX-3	P-EX-5	RCP	0.013	24	24	14.51	7084.88	7084.81	0.5	5.32	16	4.57	0.79	7085.68	7085.61	7086	7085.93	0
P-EX-5	P-EX-4	MH-EX-1	RCP	0.013	24	24	20.39	7084.81	7084.71	0.5	5.31	16	4.57	0.79	7085.6	7085.5	7085.93	7085.83	0
P-EX-6	MH-EX-1	P-EX-8	RCP	0.013	24	24	34.81	7083.7	7083.53	0.5	5.28	16	4.56	0.79	7084.49	7084.32	7084.81	7084.64	0
P-EX-7	IN-EX-3	P-EX-8	RCP	0.013	12	12	37.48	7089.65	7083.53	16.34	1.04	14.4	10.58	0.18	7089.83	7084.24	7091.57	7085.98	0
P-EX-8	Y_JUNCT	P-EX-9	RCP	0.013	24	24	45.39	7083.53	7083.31	0.5	6.23	16	4.77	0.87	7084.39	7084.17	7084.75	7084.53	0
P-EX-9	P-EX-8	P-EX-10	RCP	0.013	24	24	41.07	7083.31	7083.1	0.5	6.18	16	4.76	0.86	7084.17	7083.97	7084.52	7084.32	0
P-EX-10	P-EX-9	P-EX-13	RCP	0.013	24	24	97.41	7083.1	7082.62	0.5	6.14	16	4.75	0.86	7083.96	7084.25	7084.31	7084.6	0
P-EX-11	IN-EX-4	IN-EX-5	RCP	0.013	12	12	29.39	7088.18	7087.06	3.81	0	0	0	0	0	0	0	0	0
P-EX-12	IN-EX-5	P-EX-13	RCP	0.013	18	18	12.76	7083.97	7082.62	10.6	6.41	34.21	3.63	1.5	7087.14	7086.8	7087.34	7087.29	0.05
P-EX-13	Y_JUNCT	MH-EX-2	RCP	0.013	24	24	41.79	7082.62	7082.41	0.5	11.94	16	5.58	1.29	7084.25	7084.03	7084.73	7084.51	0.22
P-EX-14	IN-EX-6	P-EX-15	RCP	0.013	18	18	64.8	7105.07	7095.48	14.8	1.64	40.41	11.13	0.21	7105.28	7095.69	7107.2	7097.61	0
P-EX-15	P-EX-14	MH-EX-2	RCP	0.013	18	18	67.78	7095.48	7085.31	15	1.63	40.68	11.23	0.2	7095.68	7085.52	7097.64	7087.48	0
P-EX-16	MH-EX-2	MH-EX-3	RCP	0.013	24	24	61.12	7081.41	7080.55	1.4	13.36	26.77	8.52	1	7082.41	7081.55	7083.53	7082.68	0
P-EX-17	IN-EX-7	MH-EX-3	RCP	0.013	12	12	30.8	7081.02	7077.94	10	0	0	0	0	0	0	0	0	0
P-EX-18	MH-EX-3	P-102	RCP	0.013	24	24	127.81	7075.38	7072.83	2	13.28	31.99	9.7	0.9	7076.28	7073.73	7077.74	7075.19	0
P-EX-19	FREE_ENT	P-104	RCP	0.013	18	18	60.34	7070.79	7069.58	2	0	0	0	0	0	0	0	0	0
UD-P-2	UD-C-2	FREE_EXT	PVC	0.009	6	6	46.07	7070.41	7069.72	1.5	0	0	0	0	0	0	0	0	0
UD-P-1A	UD-C-1	UD-P-1B	PVC	0.009	6	6	5.41	7070.97	7070.89	1.5	0	0	0	0	0	0	0	0	0
UD-P-1B	UD-P-1A	UD-P-1C	PVC	0.009	6	6	27	7070.89	7070.49	1.5	0	0	0	0	0	0	0	0	0
UD-P-1C	UD-P-1B	UD-C-2	PVC	0.009	6	6	5.41	7070.49	7070.41	1.5	0	0	0	0	0	0	0	0	0
	0																		

Hydraulic Analysis Report

Project Data

Project Title: Monument Academy

Designer:

Project Date: Friday, June 3, 2022
Project Units: U.S. Customary Units

Notes:

Channel Analysis: Channel Analysis

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 3.0000 ft/ft Side Slope 2 (Z2): 3.0000 ft/ft Longitudinal Slope: 0.1905 ft/ft

Manning's n: 0.0727

Lining Type: Rock Riprap - 300 mm (12-inch)

Flow: 2.5900 cfs

Result Parameters

Depth: 0.5018 ft

Area of Flow: 0.7555 ft^2 Wetted Perimeter: 3.1738 ft Hydraulic Radius: 0.2380 ft Average Velocity: 3.4283 ft/s

Top Width: 3.0109 ft

Froude Number: 1.2061
Critical Depth: 0.5409 ft
Critical Velocity: 2.9510 ft/s
Critical Slope: 0.1277 ft/ft
Critical Top Width: 3.25 ft

Calculated Max Shear Stress: 5.9653 lb/ft^2 Calculated Avg Shear Stress: 2.8296 lb/ft^2

Channel Lining Analysis: Channel Lining Design Analysis

Notes:

Lining Input Parameters

Channel Lining Type: Riprap, Cobble, or Gravel

D50: 1 ft

Riprap Specific Weight: 165 lb/ft³ Water Specific Weight: 62.4 lb/ft³

Riprap Shape is Angular

Safety Factor: 1

Calculated Safety Factor: 1.32551

Lining Results

Angle of Repose: 41.7 degrees Relative Flow Depth: 0.250733 ft Manning's n method: Bathurst

Manning's n: 0.0726643

Channel Bottom Shear Results

V*: 1.75387

Reynold's Number: 144114 Shield's Parameter: 0.114023

shear stress on channel bottom: 5.96103 lb/ft^2

Permissible shear stress for channel bottom: 8.83223 lb/ft^2

channel bottom is stable Stable D50: 0.894612 ft

Channel Side Shear Results

K1: 0.868

K2: 1 Kb: 1.05

shear stress on side of channel: 5.96103 lb/ft^2

Permissible shear stress for side of channel: 8.83223 lb/ft^2

Stable Side D50: 0.776523 lb/ft^2

side of channel is stable

Channel Bend Shear Results

Curvature Radius: 39 ft

No further correction will occur once R/T > 10

shear stress on bottom of channel in bend: 6.25909 lb/ft^2

bottom of bend of the channel is stable

Length of Protection beyond PT: 1.54603 ft

Additional Freeboard required because of Superelevation: 0.0282626 ft

Channel Bend Side Shear Results

shear stress on side of channel in bend: 5.43289 lb/ft^2

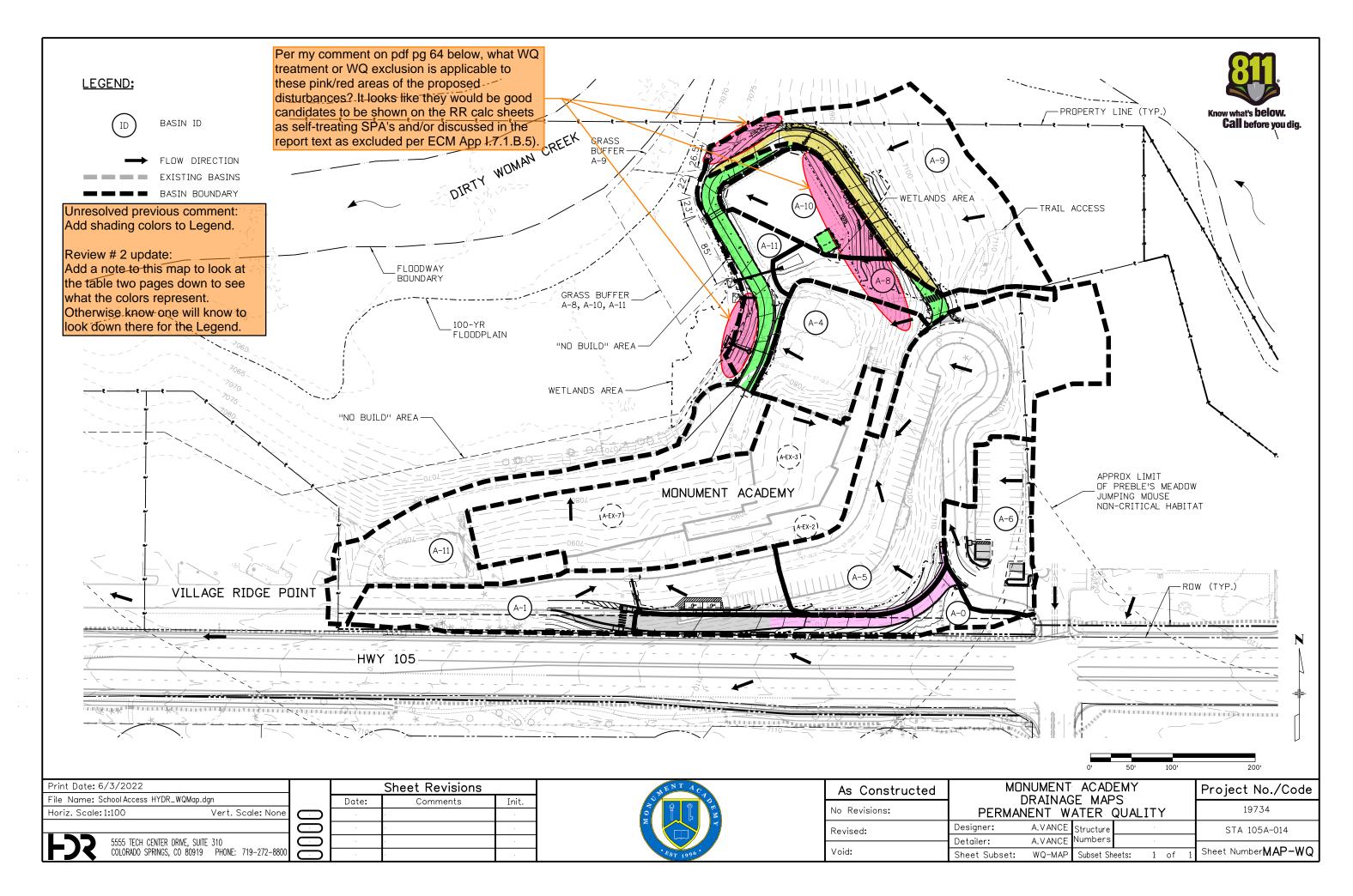
The side of the bend of the channel is stable

Channel Lining Stability Results

the channel is stable

Channel Summary

Name of Selected Channel: Channel Analysis



Monument Academy

WQCV by new impervious area & Runoff Reduction standard for 60% WQCV

Calc	AMV
Check	JLS

Date 6/3/2022 Date 6/7/2022

Area Within MS4 Boundary	Existing Impervious Area (ac)	Proposed Impervious Area (ac)	Difference in Impervious Area (ac)	20% Exclusion Area (ac)	New Impervious Area to be Treated(ac)	WQCV - New Impervious Area (ac-ft)	WQCV - New Impervious Area (cu.ft)
Yes	2.12	2.70	0.58	-0.12	0.47	0.019	844

60%
wqcv
(cu.ft)
507

WQCV (cu.ft)	507
60% Impervious Area (ac)	0.279

Note: WQCV = 0.5*Area/12

I think this column can be deleted because it is irrelevant since the disturbed area needs to be treated, not just the impervious area.

Monument Academy

WQCV by new impervious area & Runoff Reduction

Per the MS4 Permit, WQ treatment or exclusion has to be provided for all <u>disturbed</u> areas. The permit req's are not based on impervious surfaces. The table I provided on 61 of Review 1 shows this if you want to look at it for reference.

Calc AMV
Check JLS

Date 6/1/2022 Date 6/7/2022

Basin Color	Basin ID	Existing Impervious Tributary Area (acres)	Proposed Impervious Tributary Area (acres)	Required Area to be Treated: (New Impervious MS4 Area) (acres)	60% Required Area to be Treated (acres)	Actual Area Treated by CM: (Impervious) (acres)	Type of PWQ CM	Comments
Gray & Rose	A-0,1 A-5,6	1.66	1.89	0.23	0.14	0.00		
Green	A-8, A-10 A-11	0.45	0.63	0.18	0.11	0.18	Runoff Reduction	100% DCIA treated with Runoff Reduction
Yellow	A-9	0.01	0.18	0.17	0.10	0.12	Runoff Reduction	71% DCIA treated with Runoff Reduction
Subt	Subtotals		2.70	0.58	0.35	0.30		
Rose	Allowed 20% Exclusion			-0.12	-0.07			Per ECM I.7.1.C.1
PROJECT TOTALS		2.12 ac	2.70 ac	0.47 ac	0.279 ac	0.300 ac		Area Treated ≥ 60% Area Required

Basins A-EX-2, A-EX-3, and A-EX-7 are not included in the applicable site as they will not be disturbed during construction.

Is this the same Rose area as the first row above?

What about the pink/red color shown in basins A-8, A-10, and A-11?

Move this to the Comments column on the right and then add in what Basins have these exclusions.

This column really should be deleted. Use the areas from the column to the left in the RR calcs and then make sure that there is 60% treatment of the that (ie: 60% of the whole non-excluded disturbed area).

Make sure that these calcs get updated per the comments above. And make sure that 60% treatment is still achieved.

Design Procedure Form: Runoff Reduction UD-BMP (Version 3.07, March 2018) Sheet 1 of 1 Designer: A.Vance HDR Company: June 1, 2022 Date: Monument Academy Project: Monument Academy Location: **SITE INFORMATION (User Input in Blue Cells)** WQCV Rainfall Depth 0.60 Depth of Average Runoff Producing Storm, d₆ = 0.43 inches (for Watersheds Outside of the Denver Region, Figure 3-1 UIA:RPA UIA:RPA Area Type **DCIA** Area ID Front (0,1,5,6) Back (9) Back (8,10,11) Downstream Design Point ID 102-Out Grass Buffer-9 Grass-8,10,11 Downstream BMP Type None None None DCIA (ft²) 4,991 7,575 7,701 UIA (ft2) 14,287 RPA (ft2) 3.200 SPA (ft2) 0% 0% HSG A (%) HSG B (%) 0% 0% HSG C/D (%) 100% 100% 0.100 0.100 Average Slope of RPA (ft/ft) 26.50 130.00 UIA:RPA Interface Width (ft) **CALCULATED RUNOFF RESULTS** Area ID Front (0,1,5,6) Back (9) Back (8,10,11) UIA:RPA Area (ft2) --10,775 21,988 L / W Ratio 15.34 1.30 --0.3502 UIA / Area 0.7030 0.50 0.00 Runoff (in) 0.13 208 118 Runoff (ft3) 0 Runoff Reduction (ft3) 0 197 321 **CALCULATED WQCV RESULTS** Area ID Front (0,1,5,6) Back (9) Back (8,10,11) WQCV (ft3) 208 316 321 WQCV Reduction (ft3) 0 197 321 0% 63% 100% WQCV Reduction (%) 208 118 0 Untreated WQCV (ft3) CALCULATED DESIGN POINT RESULTS (sums results from all columns with the same Downstream Design Point ID) Downstream Design Point ID 102-Out Grass Buffer-9 Grass-8,10,11 DCIA (ft2) 4,991 0 UIA (ft²) 0 7,575 7,701 0 3,200 14,287 RPA (ft2) 0 0 0 SPA (ft2) Total Area (ft2) 4,991 10,775 21,988 Total Impervious Area (ft2) 4,991 7,575 7,701 WQCV (ft3) 208 316 321 197 321 WQCV Reduction (ft3) 0 0% 63% WQCV Reduction (%) 100% 208 118 Untreated WQCV (ft3) 0 CALCULATED SITE RESULTS (sums results from all columns in worksheet) Total Area (ft2) 37,755 Total Impervious Area (ft2) 20,268 844 WQCV (ft3) WQCV Reduction (ft3) 518

WQCV Reduction (%)

Untreated WQCV (ft3)

61.38%

326

Post Construction Stormwater Management Applicability Evaluation Form

This form is to be used by the Engineer of Record to evaluate applicable construction activities to determine if the activities are eligible for an exclusion to permanent stormwater quality management requirements. Additionally, Part III of the form is used to identify and document which allowable control measure design standard is used for the structure.

Part I. Project Information	
1. Project Name: Monument Academy	
2. El Paso County Project #: CDR2314	3. ESQCP #:
4. Project Location: North of Hwy 105 and East of	Project Location in MS4 Permit Area (Y or N): Y
Knollwood Dr.	
1150 Village Ridge Point, Monument, CO	
5. Project Description: Add two private access road	s to improve circulation of traffic around the school property. It
also includes a modular block wall, private storm system	updates, grading, and landscaping restoration.
If project is located within the El Paso County MS4	Permit Area, please provide copy of this completed form
to the Stormwater Quality Coordinator for reporting	g purposes; and save completed form with project file.

Part II. Exclusion Evaluation: Determine if Post-Construction Stormwater Management exclusion criteria are met. Note: Questions A thru K directly correlate to the MS4 permit Part I.E.4.a.i (A) thru (K). If Yes, to any of the following questions, then mark Not Applicable in Part III, Question 2. Yes No Not Notes: Questions Applicable A. Is this project a "Pavement This exclusion applies to "roadways" only. Areas used primarily for Management Site" as defined in Permit Χ Part I E.4.a.i.(A)? parking or access to parking are not included. B. Is the project "Excluded Roadway Development"? • Does the site add less than 1 acre of Χ paved area per mile? • Does the site add 8.25 feet or less of paved width at any location to the Χ existing roadway? C. Does the project increase the width of For redevelopment of existing the existing roadway by less than 2 times roadways, only the area of the existing roadway is excluded from the existing width? post-construction requirements Χ when the site does not increase the width by two times or more. This exclusion only excludes the original roadway area it does NOT apply to entire project. D. Is the project considered an Activity can NOT permanently alter aboveground and Underground Utilities the terrain, ground cover or Χ drainage patterns from those activity? present prior to the activity E. Is the project considered a "Large Lot Must be a single-residential lot or Single-Family Site"? agricultural zoned land, > 2.5 acres Χ per dwelling and total lot impervious area < 10 percent.

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Questions (cont'd)	Yes	No	Not	Notes
Questions (com u)			Applicable	1,000
F. Do Non-Residential or Non-Commercial Infiltration Conditions exist? Post-development surface conditions do not result in concentrated stormwater flow or surface water discharge during an 80th percentile stormwater runoff event.			X	Exclusion does not apply to residential or commercial sites for buildings. A site-specific study is required and must show: rainfall and soil conditions; allowable slopes; surface conditions; and ratios of imperviousness area to pervious area.
G. Is the project land disturbance to Undeveloped Land where undeveloped land remains undeveloped following the activity?			X	Project must be on land with no human made structures such as buildings or pavement.
H. Is the project a Stream Stabilization Site?			Х	Standalone stream stabilization projects are excluded.
I. Is the project a bike or pedestrian trail?			X	Bike lanes for roadways are not included in this exclusion but may qualify if part of larger roadway activity is excluded in A, B or C above.
J. Is the project Oil and Gas Exploration?			Х	Activities and facilities associated with oil and gas exploration are excluded.
K. Is the project in a County Growth Area?				Note, El Paso County does not apply this exclusion. All Applicable Construction Activity in El Paso County must comply the Post-Construction Stormwater Management criteria.

Part III. Post Construction (Permanent) Stormwater Control Determination						
Questions	Yes	No				
1. Is project an Applicable Construction Activity?	X					
2. Do any of the Exclusions (A-K in Part II) apply? Other exclusions apply - see		Х				
notes.						

If the project is an Applicable Construction Activity and no Exclusions apply then Post-Construction (Permanent) Stormwater Management is required.

Complete the applicable sections of Part IV below and then coordinate signatures for form and place in project file.

If the project is not an Applicable Construction Activity, or Exclusion(s) apply then Post-Construction (Permanent) Stormwater Management is NOT required. Coordinate signatures for form and place in project file.

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Par	t IV: Onsite PWQ Requirements, Documentation and Considerations	Yes	No
1.	Check which Design Standard(s) the project will utilize. Standards align with Control Measure Requirements identified in permit Part I.E.4.a.iv.	Х	
A.	Water Quality Capture Volume (WQCV) Standard		
В.	Pollutant Removal/80% Total Suspended Solids Removal (TSS)		
С.	Runoff Reduction Standard	Χ	
D.	Applicable Development Site Draining to a Regional WQCV Control Measure		
E.	Applicable Development Site Draining to a Regional WQCV Facility		
F.	Constrained Redevelopment Sites Standard		
G.	Previous Permit Term Standard		
2.	Will any of the project permanent stormwater control measure(s) be maintained by another MS4? If Yes, you must obtain a structure specific maintenance agreement with the other MS4 prior to advertisement.		Х
3.	Will any of the project permanent stormwater control measures be maintained by a private entity or quasi-governmental agency (e.g. HOA or Special District, respectively)? If Yes, a Private Detention Basin/Stormwater Quality Best Management Practice Maintenance Agreement and Easement must be recorded with the El Paso County Clerk and Recorder.		Х

Part V Notes (attach an additional sheet if you need more space)

The geographic location of the project is within the El Paso County urbanized area.

Existing impervious area = 2.12 acres
Proposed impervious area = 2.70 acres
New Impervious area = 0.58 acres

Treat 60% of the new impervious area = 0.35 acres

Allow up to a 20% exclusion = 0.07 acres (20% of 0.35) Required treatment area = 0.28 acres

Signature of El Paso County Project Engineer

Revise this whole Notes section to list areas of <u>disturbance</u>, not impervious area. Utilize Exclusion G above if needed (where applicable).

Date

Treating 0.30 acres with runoff reduction.

Project design is complete to include the project design, construction plans, drainage report, specifications, and maintenance and access agreements as required. The engineering, drainage considerations and information used to complete these documents is complete, true, and accurate to the best of my belief and knowledge.

38974

Signature and Stamp of Engineer of Record

Post-Construction Stormwater Management Applicability Form has been reviewed and the project design, construction plans, drainage report, specifications, and maintenance and access agreements as required, have been reviewed for compliance with the Post Construction Stormwater Management process and MS4 Permit requirements.

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