August 31, 2021



ENTECH ENGINEERING, INC.

505 ELKTON DRIVE COLORADO SPRINGS, CO 80907 PHONE (719) 531-5599 FAX (719) 531-5238

> APPROVED Engineering Department 07/06/2022 10:52:52 AM dsdnijkamp EPC Planning & Community Development Department

Gorilla Capital Colorado c/o Saddlehorn Ranch, LLC 1345 High Street Eugen, OR 97401

Attn: John Helmick

Re: Pavement Recommendations Saddlehorn Ranch, Filing 1, Phase 1 El Paso County, Colorado

Dear Mr. Helmick:

As requested, Entech Engineering, Inc. obtained samples of the pavement subgrade soils from the proposed roadways at the above referenced site. Laboratory testing was performed in order to determine the pavement support characteristics of the soil. This letter presents the results of the laboratory testing and pavement recommendations for the roadways.

Project Description

The project will consist of paving of the proposed Truchas Trail, Oscuro Trail, El Raiceno Trail, Del Cerro Trail, Zaragoza Trail, and Carranza Trail, Filing 1, Phase 1, subdivision in El Paso County, Colorado. Subsurface Soil Investigation and laboratory testing were performed to determine the pavement support characteristics on the soils. The general layout of the site is presented in the Test Boring Location Maps in Figures 1 and 2.

Subgrade Conditions

A total of eighteen (18) test borings were drilled along the roadways to depths of approximately 5 and 10 feet below the existing subgrade surface at the required sample frequency. At the time of our field investigation the subgrade was in good condition and adequate for vehicle traffic.

The soils at the roadway subgrade depth consisted of Soil Type 1: slightly silty to silty sand and clayey sand, and Soil Type 2: very sandy silt and very silty sand. The Test Boring Logs are presented in Appendix A. Sieve Analyses and Atterberg Limit testing were performed on subgrade soil samples obtained from the test borings for the purpose of classification. The percent passing the No. 200 sieve for the Type 1 soils ranged from approximately 8 to 32 percent. The percent passing the No. 200 sieve for the Type 2 soils ranged from 42 to 53 percent. Atterberg Testing on the Type 1 Soils resulted in Liquid Limits of No Value to 27 percent and Plastic Indexes of Non-Plastic to 8 Percent. The Type 2 soils were non-plastic.

The site soils classified as A-2-4, A-1-b, and A-4, which commonly exhibit good pavement support characteristics. Groundwater was not encountered in the test borings. Sulfate testing resulted in less than 0.01 percent soluble sulfate by weight, indicating a negligible potential for below grade concrete degradation due to sulfate attack.

Swell/Consolidation tests were not required on the Type 1 or 2 soils, due to their classification and plastic indexes. The Type 1 and Type 2 soils are anticipated to have a low expansion potential. Mitigation for expansive soils is not required on this site.

EPC Project No. SF-1912

Gorilla Capital Colorado c/o Saddlehorn Ranch, LLC Pavement Recommendations Saddlehorn Ranch, Filing 1, Phase 1 El Paso County, Colorado

California Bearing Ratio (CBR) testing was performed on a representative subgrade sample of the Type 1 materials to determine the support characteristics for the roadway sections. Due to the similarity of the Type 1 and the Type 2 soils, all sections were determined using the Type 1 values. The results of the CBR testing, are presented in Appendix B and summarized as follows:

Soil Type 1 - Silty Sand	<u>d</u>
<u>CBR 1</u>	
R @ 90% = 50.0	
R @ 95% = 76.0	
Classification Testing	
Classification Testing	
Liquid Limit	NV
Plasticity Index	NP
Percent Passing 200	21.6
AASHTO Classification	A-2-4
Group Index	0
Unified Soils Classification	SM

Pavement Design

CBR testing was used to determine pavement sections for the roadways. Pavement sections were determined utilizing El Paso County Pavement Design Criteria Manual. All roadways in the development, as shown in the Test Boring Location Maps in Figure 1 and 2, classify as rural local roads, which used an 18k ESAL value of 36,500 for design purposes. Pavement sections were determined for an asphalt on aggregate basecourse composite section.

Design parameters used in the pavement analysis for the roadways are as follows:

Reliability	
Rural Local	75%
Δpsi	2.0
"R" Value Subgrade (Soil Type 1)	50.0
Resilient Modulus (Soil Type 1)	13,168 psi
Hot Bituminous Pavement	0.44
Aggregate Basecourse	0.11

The pavement design calculations are presented in Appendix C. Pavement section alternatives for the roadway sections are presented below. Any additional grading may result in subgrade soils with different support characteristics. The following pavement sections should be re-evaluated if additional grading is performed.

Gorilla Capital Colorado c/o Saddlehorn Ranch, LLC Pavement Recommendations Saddlehorn Ranch, Filing 1, Phase 1 El Paso County, Colorado

<u>Pavement Sections</u> ESAL = 13,168 – Truchas Trail, Oscuro Trail, El Raiceno Trail, Del Cerro Trail, Zaragoza Trail, and Carranza Trail <u>Soil Type 1</u>

Alternative	<u>Asphalt (in)</u>	<u>Basecourse (in)</u>
1. Asphalt Over Basecourse	3.0*	4.0*

*Minimum sections required per the El Paso County Engineering Criteria Manual.

Mitigation

The El Paso County Engineering Criteria Manual requires mitigation of subgrade soils that have a swell of 2.0 percent or greater with a 150 pound per square foot surcharge. None of the soil types exceeded the threshold. Mitigation of the subgrade soils is not required. Due to the clay content of the Type 2 soils, moisture-conditioning and recompaction are recommended. Personnel of Entech Engineering, Inc. should be consulted to determine the need for and extents of scarification, if required.

Roadway Construction - Asphalt on Aggregate Basecourse

Prior to placement of the asphalt, the Type 1 subgrade should be scarified, moisture conditioned, and compacted to a minimum of 95 percent of the soils maximum Modified Proctor Dry Density, ASTM D-1557 at \pm 2 percent of optimum moisture content. The Type 2 soils subgrade should be scarified, moisture conditioned, and compacted to a minimum of 95% of the soils maximum Standard Proctor Dry Density, ASTM D-698 at 0 to 4 percent over optimum moisture content and properly compacted. Any loose or soft areas should be removed and replaced with suitable materials. Basecourse materials should be compacted to a minimum of 95 percent of its maximum Modified Proctor Dry Density, ASTM D-1557 at \pm 2 percent of optimum moisture content. Special attention should be given to areas adjacent to manholes, inlet structures, and valves. It is noted that full-depth asphalt is currently not allowed, per El Paso County specifications

In addition to the above guidance, the asphalt, subgrade conditions, compaction of materials and roadway construction methods shall meet the El Paso County specifications.

We trust that this has provided you with the information you required. The pavement sections provided are based on general site soil types. If you have any questions or need additional information, please do not hesitate to contact us.

Respectfully Submitted,

ENTECH ENGINEERING, INC.

Daniel P. Stegman DPS/jr Encl. Entech Job No. 211922 AAprojects/2021/211922 pr



EPC Project No. SF-1912

TABLE

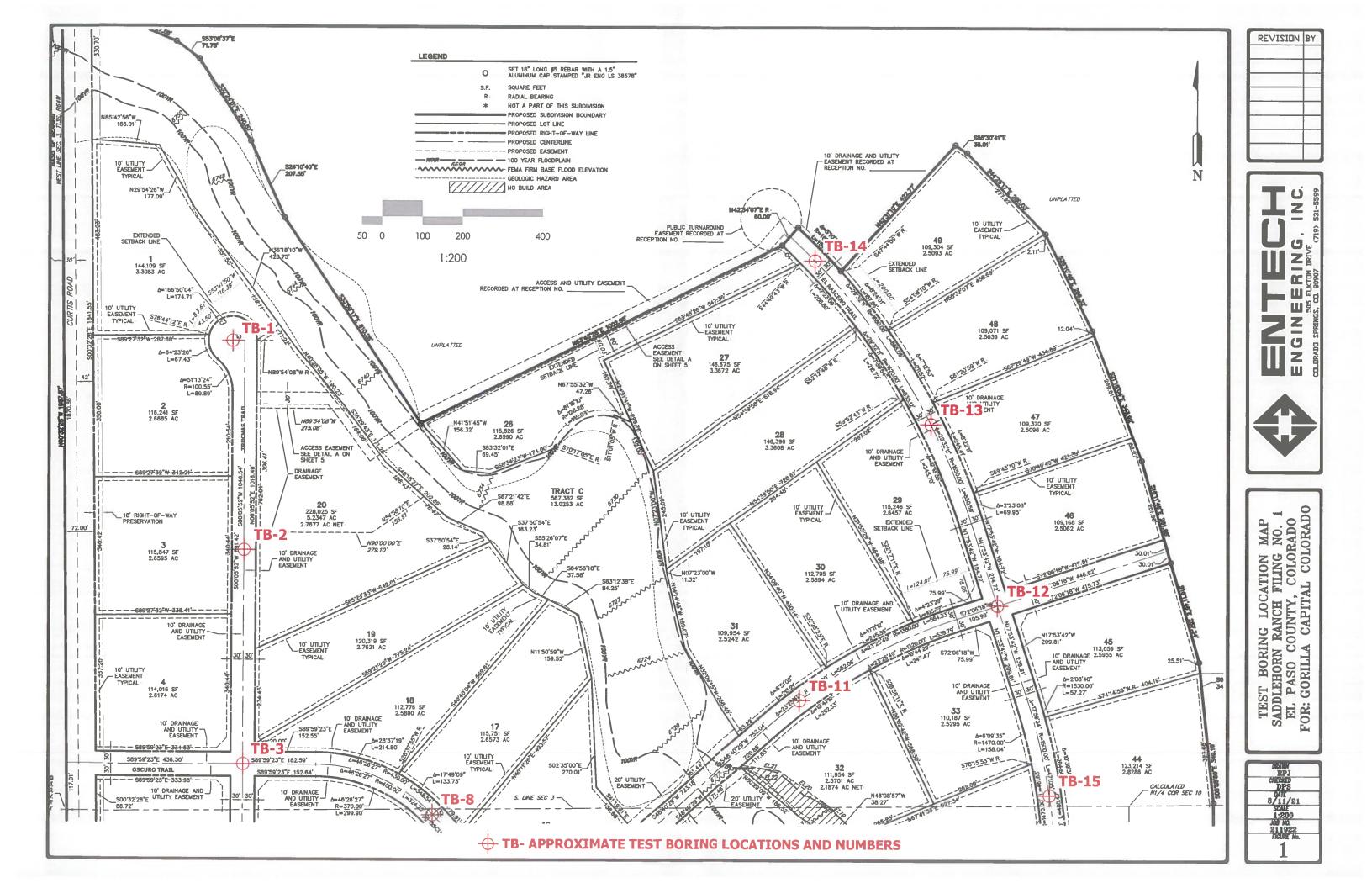
TABLE 1

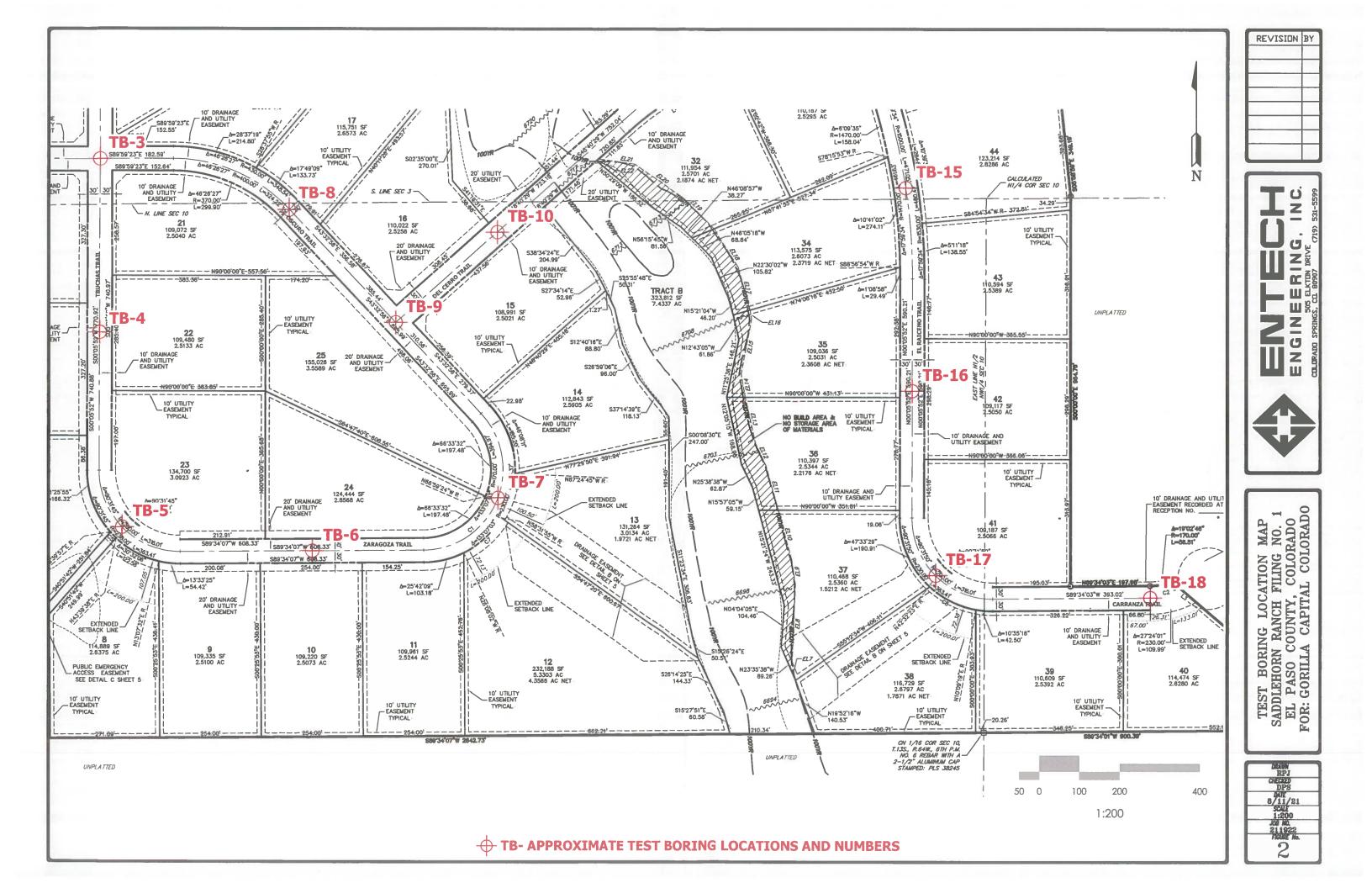
SUMMARY OF LABORATORY TEST RESULTS

CLIENTGORILLA CAPITALPROJECTSADDLEHORN RANCH, F-1JOB.NO.211922

	TEST			DRY	PASSING	LIQUID	PLASTIC			SWELL/		
SOIL	BORING	DEPTH	WATER	DENSITY	NO. 200 SIEVE	LIMIT	INDEX	SULFATE	AASHTO	CONSOL	UNIFIED	
TYPE	NO.	(FT)	(%)	(PCF)	(%)	(%)	(%)	(WT %)	CLASS.	(%)	CLASSIFICATION	SOIL DESCRIPTION
1, CBR	1	0-3			21.6	NV	NP		A-2-4		SM	SAND, SILTY
1	1	1-2			15.6	NV	NP		A-2-4		SM	SAND, SILTY
1	2	1-2			14.0	NV	NP		A-2-4		SM	SAND, SILTY
1	3	1-2			29.9	NV	NP		A-2-4		SM	SAND, SILTY
1	4	1-2			22.4	NV	NP	<0.01	A-2-4		SM	SAND, SILTY
1	5	1-2			16.0	NV	NP		A-2-4		SM	SAND, SILTY
1	6	1-2			14.4	NV	NP		A-2-4		SM	SAND, SILTY
1	7	1-2			32.1	NV	NP		A-2-4		SM	SAND, SILTY
1	8	1-2			10.4	NV	NP		A-1-b		SM-SW	SAND, SLIGHTLY SILTY
1	9	1-2			15.9	NV	NP	<0.01	A-2-4		SM	SAND, SILTY
1	10	1-2			15.1	NV	NP		A-2-4		SM	SAND, SILTY
1	11	1-2			23.2	NV	NP		A-2-4		SM	SAND, SILTY
1	12	1-2			10.1	NV	NP		A-1-b		SM-SW	SAND, SLIGHTLY SILTY
1	14	1-2			7.7	NV	NP		A-1-b		SM-SW	SAND, SLIGHTLY SILTY
1	15	1-2			31.2	NV	NP		A-2-4		SM	SAND, SILTY
1	17	1-2			17.7	NV	NP		A-2-4		SM	SAND, SILTY
1	18	1-2			32.4	27	8		A-2-4		SC	SAND, CLAYEY
2	13	1-2			41.9	NV	NP		A-4		SM	SAND, VERY SILTY
2	16	1-2			53.3	NV	NP		A-4		SM	SILT, VERY SANDY

FIGURES





APPENDIX A: Test Boring Logs

TEST BORING NO. 1 DATE DRILLED 7/15/2021 Job # 211922 REMARKS		TEST BORING NO DATE DRILLED CLIENT LOCATION REMARKS	2 7/15/2021 GORILLA CAPITAL SADDLEHORN RAI	
DRY TO 10', 7/15/21 SAND, SILTY, FINE TO MEDIUM	Depth (ft) Symbol Samples Blows per foot	Natercontent %	Depth (ft) Symbol	Samples Blows per foot Watercontent % Soil Type
GRAINED, BROWN TO TAN, MEDIUM DENSE TO DENSE, MOIST	27 7. 28 5. 10 45 5. 20	0 1		16 5.4 1 28 5.6 1
ENTECH ENGINEERING, I 505 ELKTON DRIVE COLORADO SPRINGS, COLO	DE			JOB NO.: 211922 FIG NO.: A- 1

DRY TO 5, 7/15/21 Image: state s	DATE DRIL Job #	ING NO. LED	3 7/15/202 211922	1						TEST BORING NO. DATE DRILLED CLIENT LOCATION REMARKS	4 7/15/202 GORILLA SADDLE	1 A CAP			F-1	
GRAINED, TAN, DENSE, MOIST 44 8.7 1 GRAINED, BROWN TO TAN, MEDIUM DENSE TO DENSE, MOIST TO DRY 31 7.8 1 5 31 4.3 1 MOIST TO DRY 5 15 1.5 1 10 10 1 1 1 10	DRY TO 5',	7/15/21		Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	DRY TO 10', 7/15/21		Depth (ft)	Symbol	Samples Blows per foot	Watercontent %	Soil Type
				10						GRAINED, BROWN TO T MEDIUM DENSE TO DEI	AN,	- - - - - - - - - - - - - - - - - - -		15	1.5	1

TEST BORING NO. 5 DATE DRILLED 7/15/202 Job # 211922 REMARKS	1		TEST BORING NO. DATE DRILLED CLIENT LOCATION REMARKS	6 7/15/2021 GORILLA SADDLEH		CH, F-	1	_
DRY TO 5', 7/15/21	Depth (ft) Symbol Samples Blows per foot	Watercontent % Soil Type	DRY TO 5', 7/15/21		Deptn (II) Symbol Samoles	Blows per foot	Watercontent % Soil Type	
SAND, SILTY, FINE TO MEDIUM GRAINED, TAN, DENSE TO MEDIUM DENSE, MOIST			SAND, SILTY, FINE TO M GRAINED, TAN, DENSE T MEDIUM DENSE, MOIST	70 1		33	8.0 1	
ENTECH ENGINEERING, 505 ELKTON DRIVE COLORADO SPRINGS, COL		DRAWN:	TEST BC		992521		JOB N 21192 FIG NG A-	

	TEST BORING NO. 7 DATE DRILLED 7/15/2021 Job # 211922							TEST BORING NO. 8 DATE DRILLED 7/15/2021 CLIENT GORILLA CAPITAL								
	REMARKS								LOCATION	SADDLE				H, F	-1	
	DRY TO 10',		Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	REMARKS DRY TO 5', 7/15/21		Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
		FINE TO MEDIUM DWN TO TAN,	10 15 20			13 24 27	9.7 5.4 8.9	1	SAND, SLIGHTLY SILTY, TO COARSE GRAINED, T MEDIUM DENSE, MOIST	AN.				29	4.8	1
*	ENTECH ENGINEERING, INC. 505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907					DRAW	/N:	DATE:		1	2 ^{TE} 2))]	21	DB NO.: 1922 FIG NO.: A- 4	

TEST BORING NO. 9 DATE DRILLED 7/27/202 Job # 211922 REMARKS	1					1	TEST BORING NO. DATE DRILLED CLIENT LOCATION REMARKS	10 7/15/202 GORILLA SADDLE	1 \ CAP	ITAL		H, F	-1	
DRY TO 5', 7/27/21	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	DRY TO 5', 7/15/21		Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
SAND, SILTY, FINE TO MEDIUM GRAINED, TAN, MEDIUM DENSE, MOIST	5 10 15 20			23	6.8	1	SAND, SILTY, FINE TO N GRAINED, TAN, DENSE MEDIUM DENSE, MOIST	ТО	5 10 15 20			33	7.8	1
ENTECH ENGINEERING, 505 ELKTON DRIVE COLORADO SPRINGS, COL					DRAW	/N:				 			21	B NO.: 1922 3 NO.: A- 5

ob # 211922 REMARKS							CLIENT LOCATION REMARKS	GORILLA SADDLE				H, F		
)RY TO 5', 7/15/21	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	DRY TO 5', 7/27/21		Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
AND, SILTY, FINE TO MEDIUM BRAINED, TAN, DENSE TO IEDIUM DENSE, MOIST TO DRY				28	8.1	1	SAND, SLIGHTLY SILTY, MEDIUM GRAINED, TAN, DENSE, DRY TO MOIST		-			13	1.4	1
	5			31	2.5	1			5			12	10.8	1
	10						9 3 -		10					
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	15								15					
	20								20					
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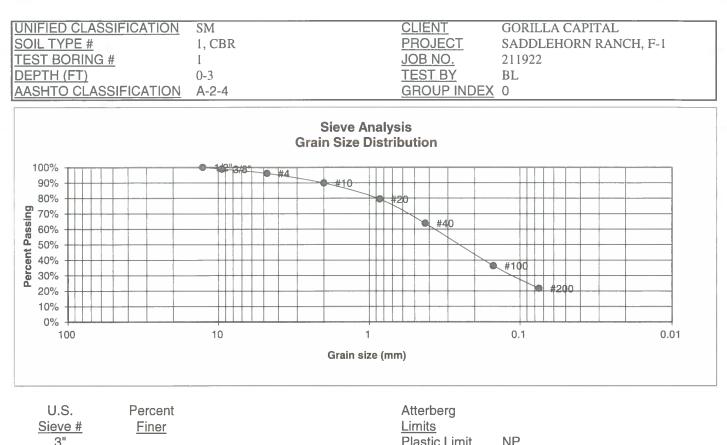
ENTECH ENGINEERING, INC. 505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907

	TES	T BORING LO	G	JOB NO.: 211922
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TEST BORING NO. 13 DATE DRILLED 7/27/202 Job # 211922 REMARKS	:1		
DRY TO 5', 7/27/21 SAND, VERY SILTY, FINE	Depth (ft) Symbol Samples Blows per foot Watercontent %	ed L IIIO S DRY TO 10', 7/27/21	Depth (ft) Symbol Samples Blows per foot Watercontent % Soil Type
GRAINED, TAN, LOOSE, MOIST	5 8 8.0 5 8 11.3	SAND, SLIGHTLY SILTY, FINE TO MEDIUM GRAINED, TAN, MEDIUM DENSE, DRY TO MOIST 2	15 1.3 1 5 21 3.3 1
	10 - 15		10 17 3.9 1 15
	20		20
ENTECH ENGINEERING, 505 ELKTON DRIVE COLORADO SPRINGS, COL	DRAW	N: DATE: CHECKED:	G 211922 Fig No.: A- 7

TEST BO DATE DR Job #	RING NO. ILLED	17 7/27/202 211922	1						TEST BORING N DATE DRILLED CLIENT LOCATION	0. 18 7/27/202 GORILL/ SADDLE	1 A CAF				-1	
REMARK	S		-			foot	ent %		REMARKS	0.00022						
DRY TO 5	5', 7/27/21		Depth (ft)	Symbol	Samples	Blows per foot	Watercontent	Soil Type	DRY TO 10', 7/27	/21	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
SAND, SIL1	TY, FINE TO N TAN, MEDIUM		-			19	7.6	1	SAND, CLAYEY, FIN GRAINED, TAN, MED TO DENSE, MOIST	E TO MEDIUM	-	·/-		19	9.6	1
			5			16	5.0	1			5			39	9.3	1
			10								10			10	3.0	1
			-								-					
			15								15					
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6>		ECH ERING,	INC.						TEST	BORING LO	G				21	B NO.: 1922
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APPENDIX B: Laboratory Test Results

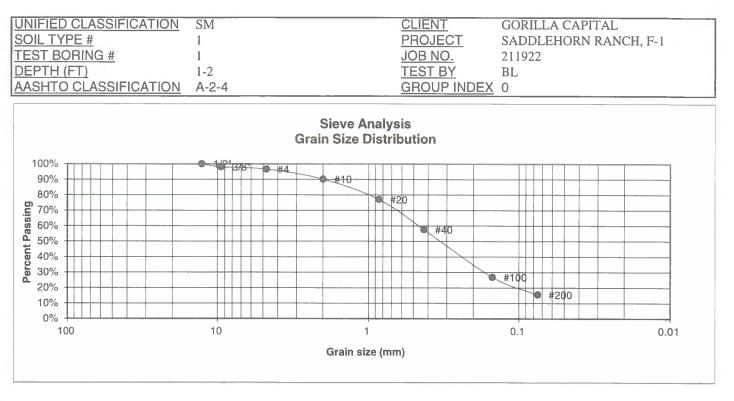


3" 1 1/2" 2/4"		Plastic Limit NP Liquid Limit NV Plastic Index NP
3/4" 1/2" 3/8"	100.0% 98.9%	Flastic Index INF
4	96.1%	<u>Swell</u>
10	89.9%	Moisture at start
20	79.4%	Moisture at finish
40	63.8%	Moisture increase
100	36.3%	Initial dry density (pcf)
200	21.6%	Swell (psf)

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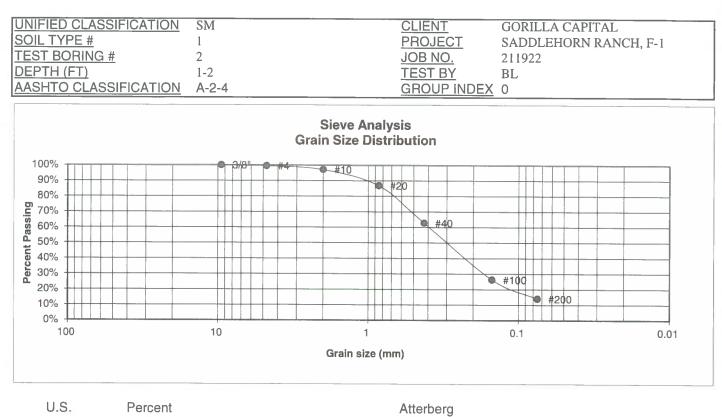
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JOB NO.:
211922
FIG NO.:
B-1



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit NP Liquid Limit NV Plastic Index NP
1/2" 3/8"	100.0% 98.1%	
4	96.6%	<u>Swell</u>
10	90.2%	Moisture at start
20	77.0%	Moisture at finish
40	57.6%	Moisture increase
100	26.8%	Initial dry density (pcf)
200	15.6%	Swell (psf)

\Leftrightarrow	ENTECH ENGINEERING, INC.		JOB NO.: 211922 FIG NO.:			
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907	DRAWN:	DATE:	CHECKED:	PATE 12/21	B- 2.



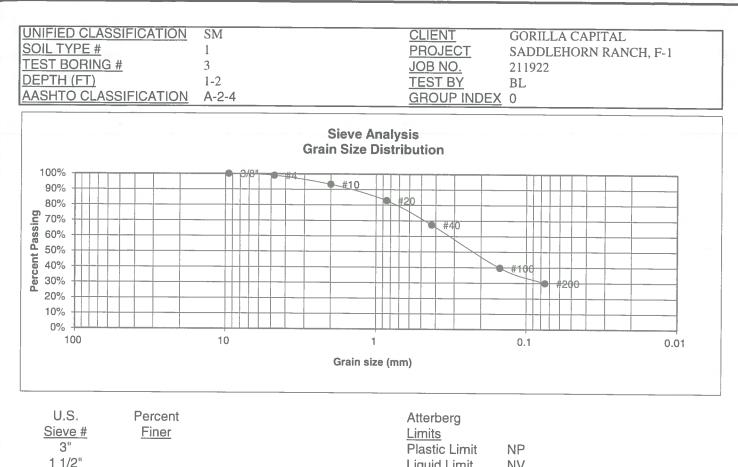
U.S.	Percent	Atterberg
<u>Sieve #</u>	Finer	Limits
3"		Plastic Limit NP
1 1/2"		Liquid Limit NV
3/4"		Plastic Index NP
1/2"		
3/8"	100.0%	
4	99.5%	Swell
10	97.1%	Moisture at start
20	86.8%	Moisture at finish
40	62.8%	Moisture increase
100	26.3%	Initial dry density (pcf)
200	14.0%	Swell (psf)

DRAWN:



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JOB NO :
211922
FIG NO.:
B-3

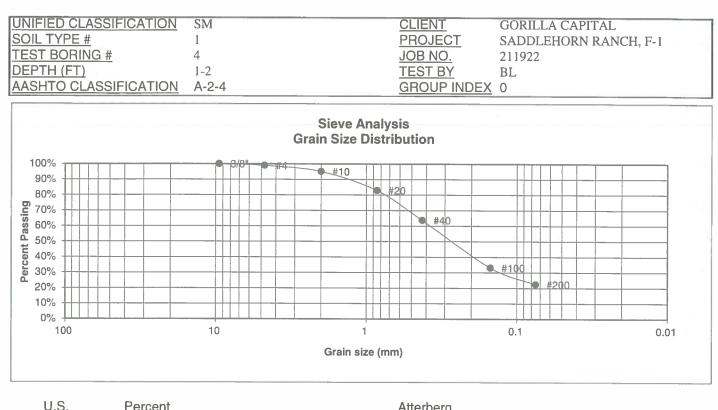


3/4"		Plastic Index NP
1/2"	100.00	
3/8"	100.0%	
4	98.9%	Swell
10	93.3%	Moisture at start
20	82.9%	Moisture at finish
40	67.3%	Moisture increase
100	39.6%	Initial dry density (pcf)
200	29.9%	Swell (psf)

ENTECH ENGINEERING, INC.
505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907

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JOB NO.;
211922
FIG NO.:
0-11

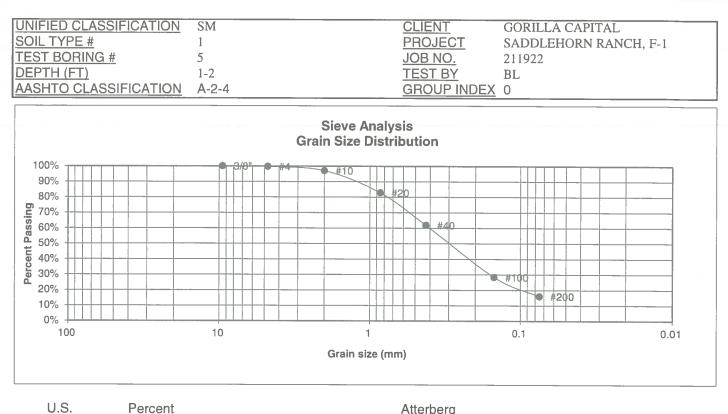


U.S.	Percent	Atterberg
Sieve #	<u>Finer</u>	Limits
3"		Plastic Limit NP
1 1/2"		Liquid Limit NV
3/4"		Plastic Index NP
1/2"		
3/8"	100.0%	
4	98.9%	Swell
10	95.1%	Moisture at start
20	82.9%	Moisture at finish
40	63.7%	Moisture increase
100	33.1%	Initial dry density (pcf)
200	22.4%	Swell (psf)

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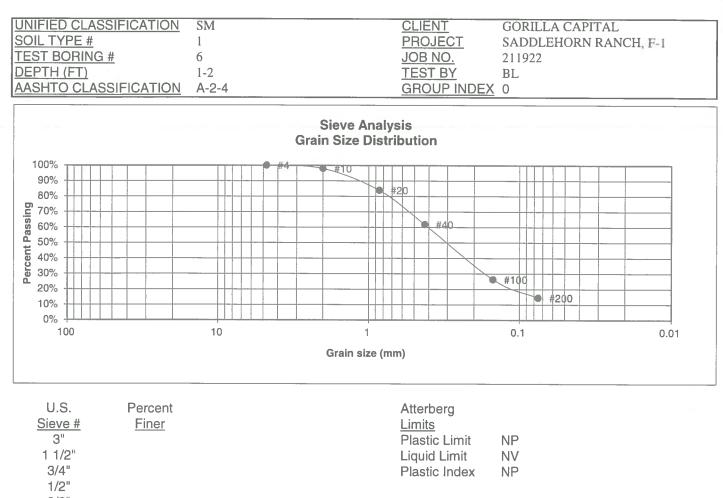


U.S.	Percent	Atterberg
<u>Sieve #</u>	<u>Finer</u>	Limits
3"		Plastic Limit NP
1 1/2"		Liquid Limit NV
3/4"		Plastic Index NP
1/2"		
3/8"	100.0%	
4	99.7%	Swell
10	97.0%	Moisture at start
20	82.8%	Moisture at finish
40	61.9%	Moisture increase
100	28.3%	Initial dry density (pcf)
200	16.0%	Swell (psf)

\Leftrightarrow	ENTECH ENGINEERING, INC.
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907

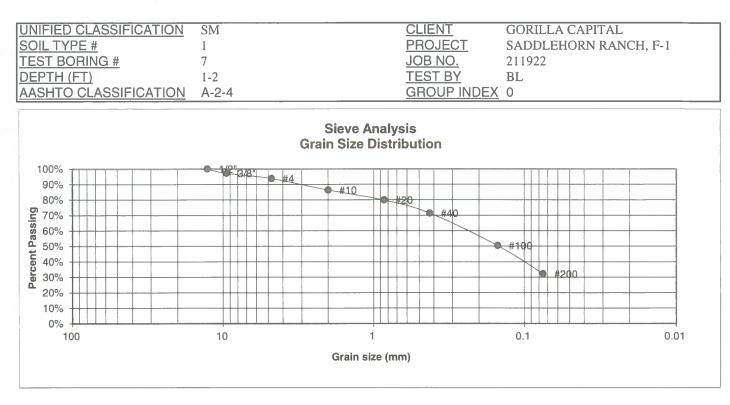
LABORATORY TEST RESULTS			
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FIG NO.:



3/8" 4 100.0% Swell 10 98.0% Moisture at start 20 83.9% Moisture at finish 40 61.8% Moisture increase 100 26.2% Initial dry density (pcf) 200 14.4% Swell (psf)

\Leftrightarrow			LABORATORY TEST RESULTS			
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907	DRAWN:	DATE:	CHECKED	912121	FIG NO.

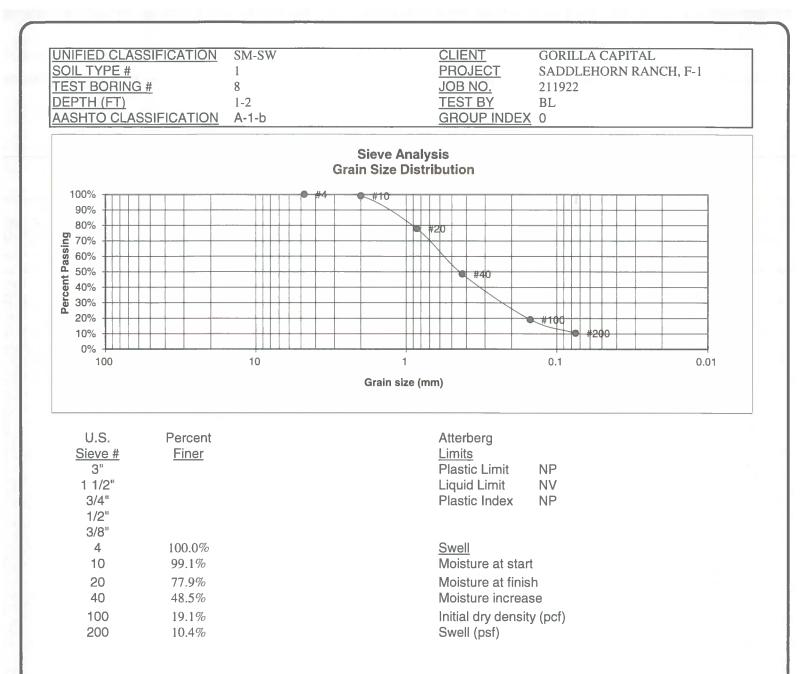


U.S.	Percent	Atterberg
<u>Sieve #</u> 3"	<u>Finer</u>	<u>Limits</u> Plastic Limit NP
1 1/2"		Liquid Limit NV
3/4"		Plastic Index NP
1/2"	100.0%	
3/8"	97.2%	
4	93.8%	Swell
10	86.3%	Moisture at start
20	79.8%	Moisture at finish
40	71.4%	Moisture increase
100	50.4%	Initial dry density (pcf)
200	32.1%	Swell (psf)

\Leftrightarrow	ENTECH ENGINEERING, INC.
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907

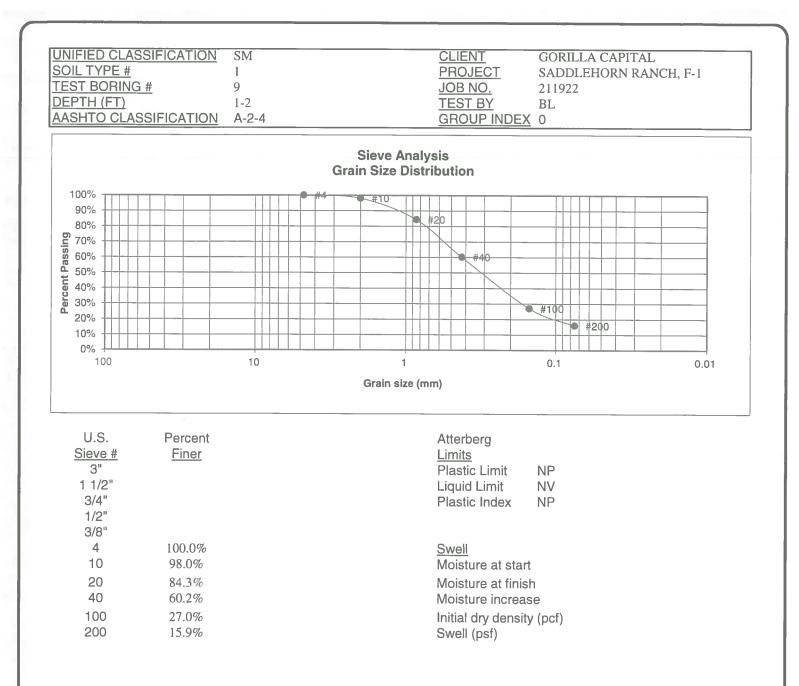
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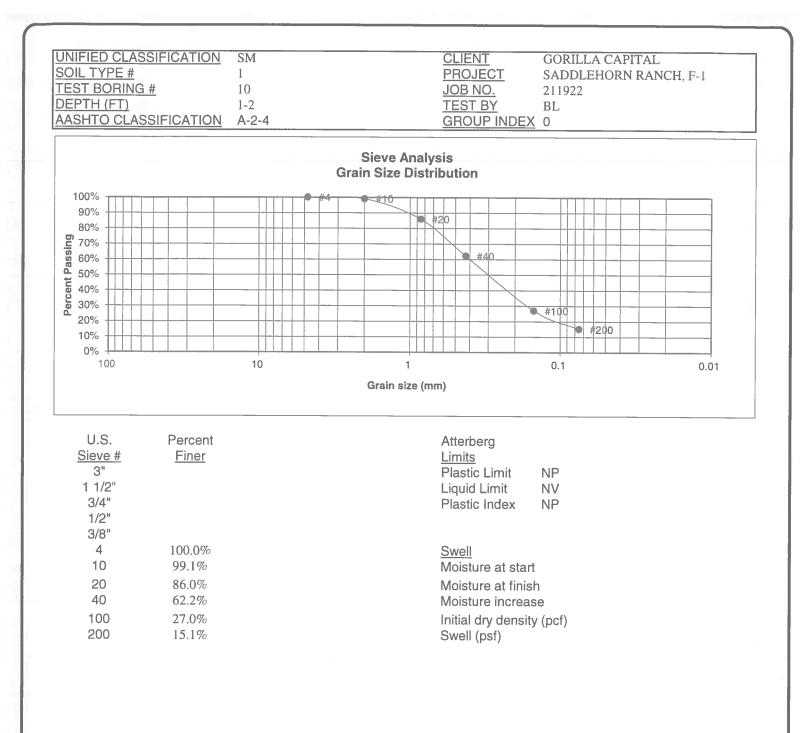


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	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907

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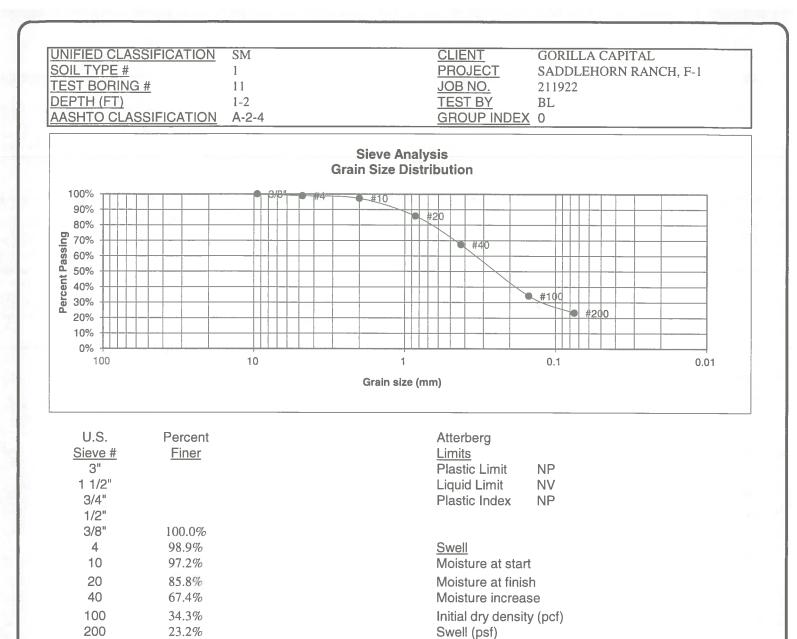
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211922
FIG NO
5-16



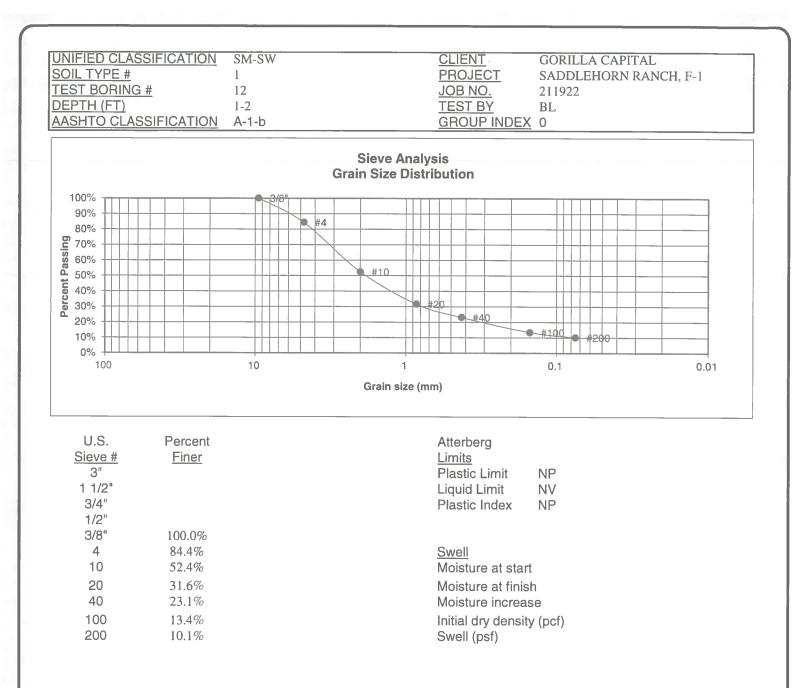
\Leftrightarrow	ENTECH ENGINEERING, INC.
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907

r	LABOF	RATORY TEST	г	JOB NO.
-	RESUL	TS		211922 FIG NO.
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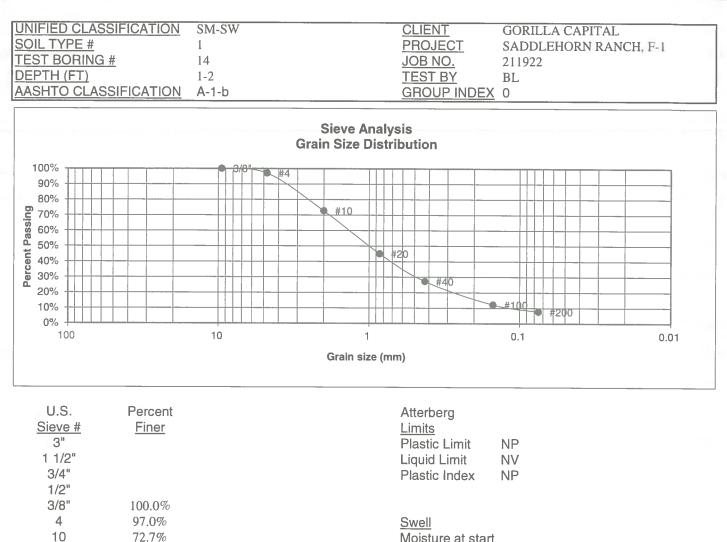


\diamond	ENTECH ENGINEERING, INC.
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907

ſ	LABOF	RATORY TES	г	JOB NO.:
	RESUL	TS		211922 FIG NO.:
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\mathbf{c}	ENTECH ENGINEERING, INC.		LABOF RESUL	RATORY TES	Т	JOB NO. 211922 FIG NO.
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907	DRAWN	DATE	CHECKED S	912121	B-13

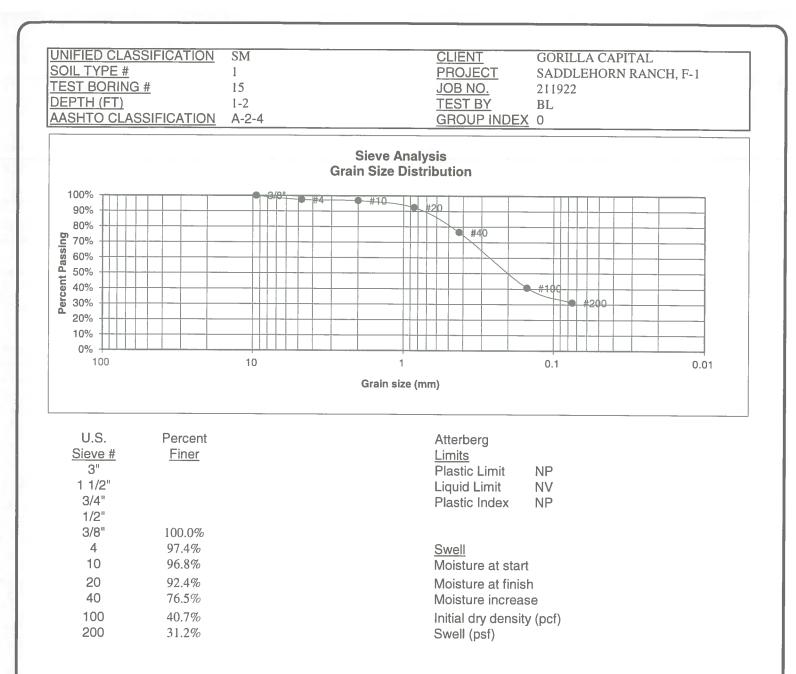


72.7% Moisture at start 45.0% Moisture at finish 27.2% Moisture increase 100 12.2% Initial dry density (pcf) 200 7.7% Swell (psf)

20

40

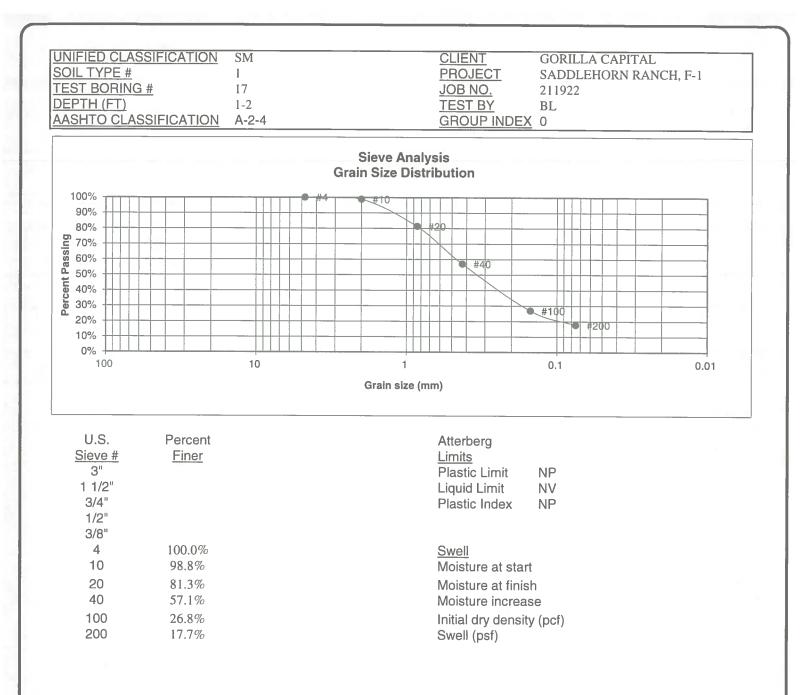
ENTECH		LABORATORY TEST				
ENGINEERING, INC.		RESULTS				
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907	DRAWN	DATE		912121	FIG NO.



\Rightarrow	ENTECH ENGINEERING, INC. 505 ELKTON DRIVE
	COLORADO SPRINGS, COLORADO 80907

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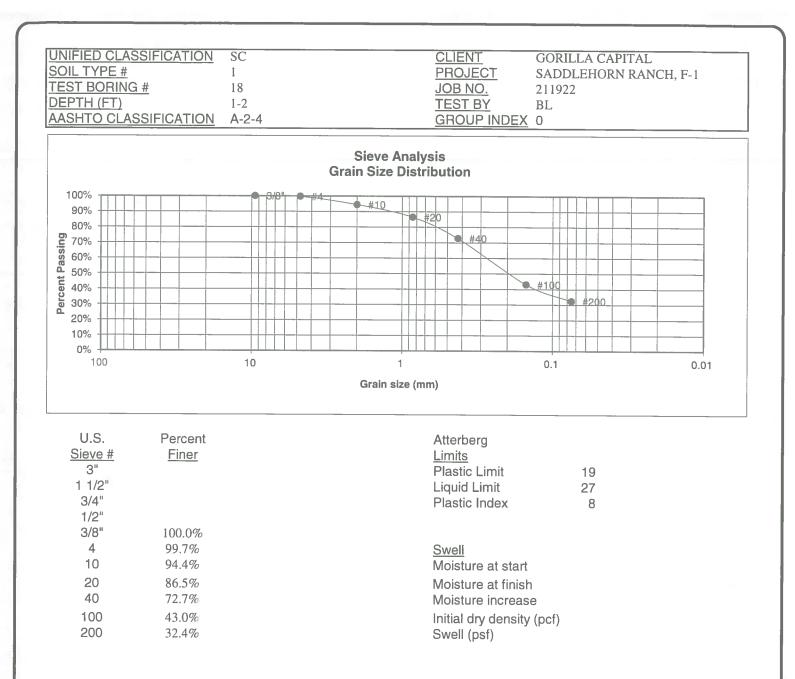
JOB NO.1	
211922	
FIG NO.	
8-15	



\bigcirc	ENTECH ENGINEERING, INC.
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907

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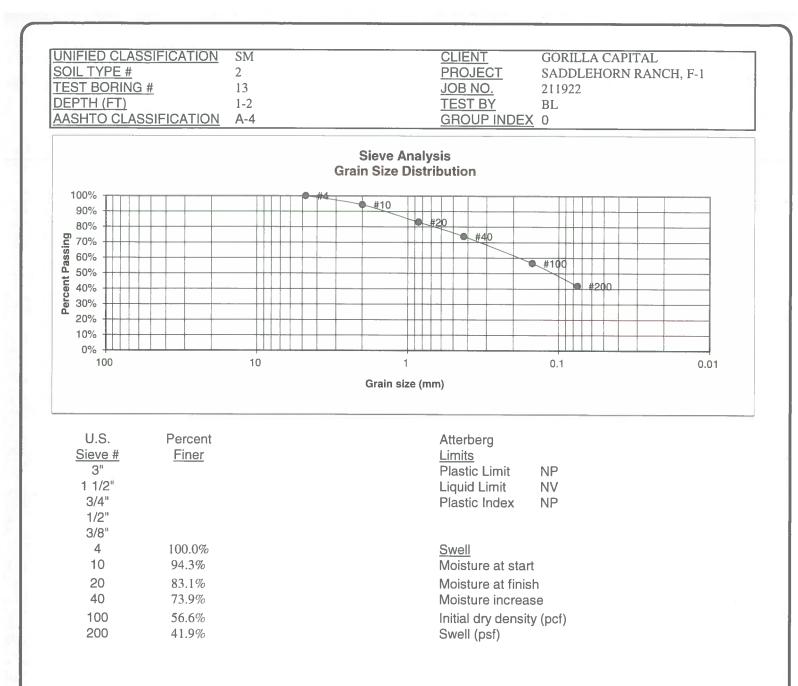
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ENTECH ENGINEERING, INC.
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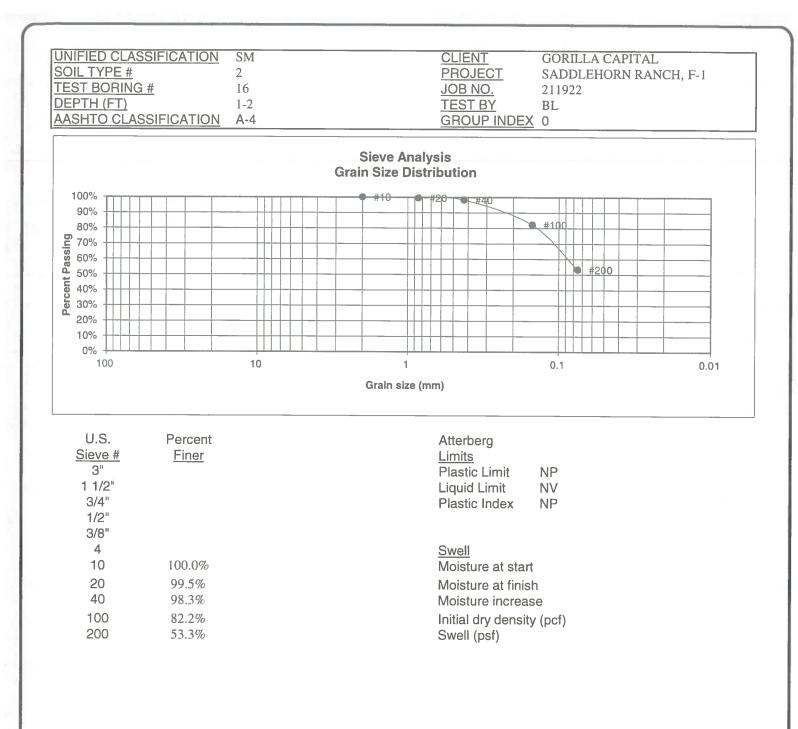
JOB NO.:
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FIG NO.:
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\Leftrightarrow	ENTECH ENGINEERING, INC.
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907

		RATORY TES	Г	JOE
	RESUL	TS		2111 FIG
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JOB NO	
211922	
FIG NO.	
B-18	35



\bigcirc	ENTECH ENGINEERING, INC.		LABORAT RESULTS	ORY TEST		JOB NO.: 211922 FIG NO.:
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907	DRAWN;	DATE	CHECKED:	DATE:	B-19

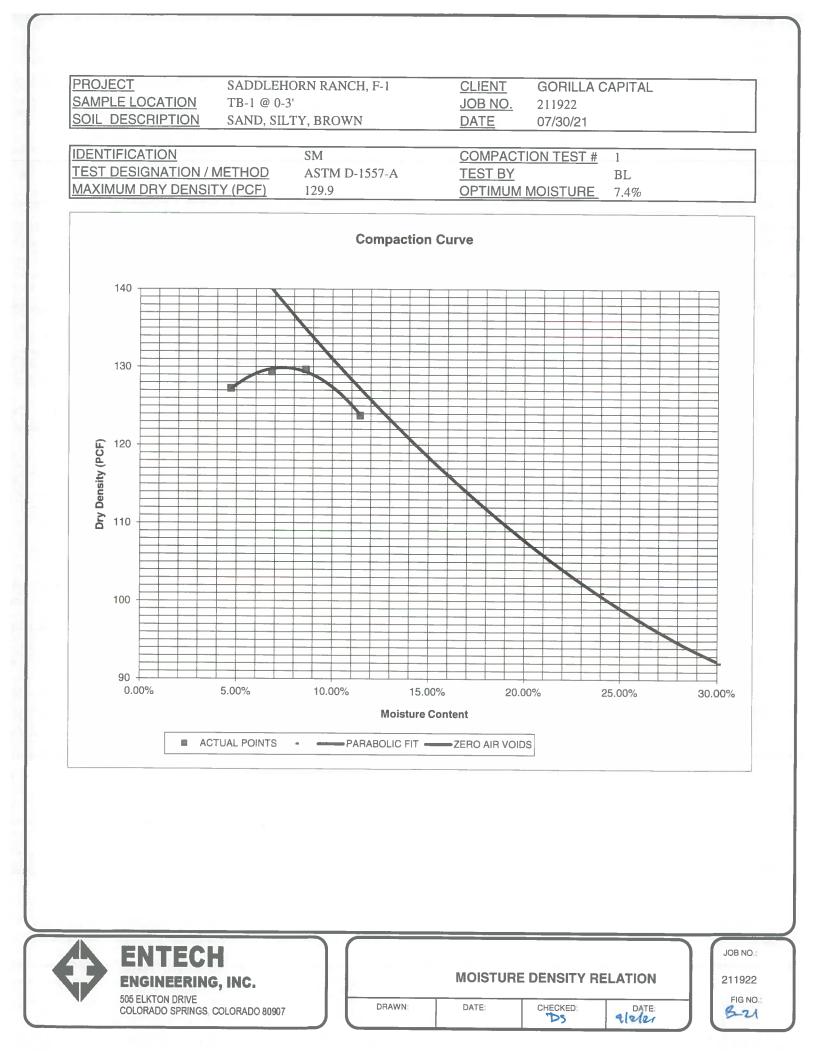
CLIENT	GORILLA CAPITAL	JOB NO.	211922
PROJECT	SADDLEHORN RANCH, F-1	DATE	7/30/2021
LOCATION	SADDLEHORN RANCH, F-1	TEST BY	BL

DEPTH, (ft)	SOIL TYPE NUMBER	UNIFIED CLASSIFICATION	WATER SOLUBLE SULFATE, (wt%)
1-2	1	SM	<0.01
1-2	1	SM	<0.01
		·	
			· · · · · · · · · · · · · · · · · · ·
	1-2	1-2 1	DEPTH, (ft) NUMBER CLASSIFICATION 1-2 1 SM

QC BLANK PASS



		RATORY TEST ATE RESULTS		JOB 2 FIG
DRAWN	DATE	CHECKED:	DATE:	3-



CBR TEST LOAD DATA

PISTON

PISTON

JOB NO: 211922 CLIENT: GORILLA CAPITAL PROJECT: SADDLEHORN RANCH, F-1 SOIL TYPE

DIAMETER (cm)	AREA (in ²)	SOIL TYPE: 1			-1	
4.958	2.993		OOIL THE.	1		
	10 BLOWS		25 BLOWS		56 BLOWS	
PENETRATION	MOLD #	1	MOLD #	2	MOLD #	3
DEPTH	LOAD(LBS)	STRESS	LOAD(LBS)	STRESS	LOAD(LBS)	STRESS
(INCHES)	(LBS)	(PSI)	(LBS)	(PSI)	(LBS)	(PSI)
0.000	0	0.00	0	0.00	0	0.00
0.025	95	31.75	168	56.14	289	96.57
0.050	121	40.43	317	105.93	726	242.61
0.075	136	45.45	416	139.01	1380	461.15
0.100	146	48.79	517	172.76	1532	511.94
0.125	155	51.80	636	212.53	2148	717.79
0.150	163	54.47	736	245.95	2696	900.92
0.175	171	57.14	834	278.70	3146	1051.29
0.200	178	59.48	933	311.78	3560	1189.64
0.300	201	67.17	1180	394.32	3790	1266.50
0.400	223	74.52	1211	404.68	3929	1312.95
0.500	248	82.87	1241	414.70	4165	1391.81

FINAL MOISTURE CONTENT

	MOLD #	1	MOLD #	2	MOLD #	3
CAN #		312		314		304
WT. CAN		7.78		7.98		8.08
WT. CAN+WET		352.3		321		280.57
WT. CAN+DRY		315.2		281.3		256.5
<u>WT. H20</u>		37.1		39.7		24.07
WT. DRY SOIL	1	307.42		273.32		248.42
MOISTURE CONTENT		12.07%		14.53%		9.69%
WET DENSITY (PCF)		119.4		126.5		131.8
DRY DENSITY (PCF)		111.1		117.8		122.7
BEARING RATIO		4.88		17.28		51.19
90% OF DRY DENSITY	116.9					
95% OF DRY DENSITY	123.4					
BEARING RATIO AT 90% OF MAX		15.58	~ R VALUE	50		
BEARING RATIO AT 95% OF MAX		55.92	~ R VALUE	76		
					1	

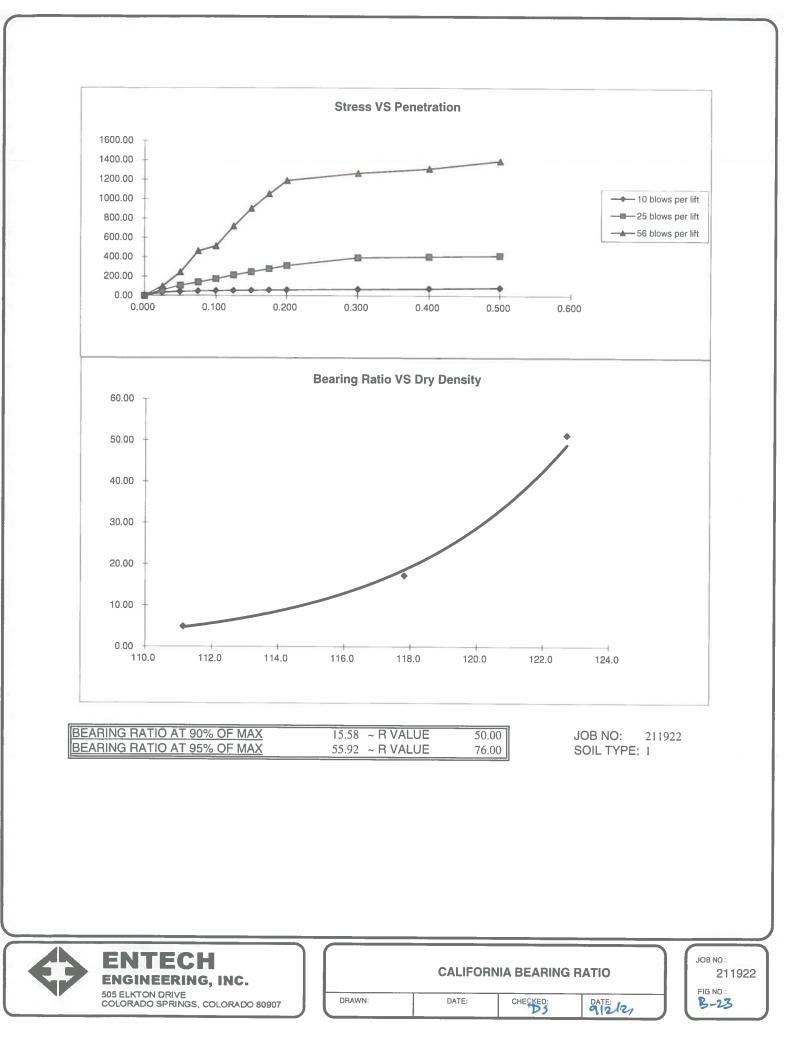


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ENGINEERING, INC.	
505 ELKTON DRIVE	DRAWN:
COLORADO SPRINGS, COLORADO 80907	^

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	CBR	TEST DATA	
4	DATE:		9/2/21

JOB NO .: 211922 FIG NO 8-22



APPENDIX C: Design Calculations

FLEXIBLE PAVEMENT DESIGN

DESIGN DATA GORILLA CAPITAL COLORADO SADDLEHORN RANCH, LLC SADDLE HORN RANCH

Equivalent (18 kip) Single Axle Load Applications (ESAL): $ESAL(W_{18}) =$ 36,500 Hveem Stabilometer (R Value) Results: R = 50 Standard Deviation $S_0 =$ 0.44 Loss in Serviceability 2.0 $\Delta psi =$ Reliability Reliability = 75 Reliability (z-statistic) $Z_R =$ -0.674 Soil Resilient Modulus $M_R =$ 13168

Weighted Structural Number (WSN):

WSN = 1.41

DESIGN TABLES AND EQUATIONS

 $S_1 = [(R - 5) / 11.29] + 3$ $M_R = 10^{[(S_1 + 18.72) / 6.24]}$

 $k = M_R / 19.4$

Where:

M_R = resilient modulus (psi)

 S_1 = the soil support value

R = R-value obtained from the Hveem stabilometer

CBR = California Bearing Ratio

Reliability (%)	Z_{R} (z-statistic)
50	0
60	-0.253
70	-0.524
75	-0.674
80	-0.841
90	-1.282
95	-1.65
97	-1.88
98	-2.05
99	-2.33
99.9	-3.09
99.99	-3.75

$$\log_{10}W_{18} = Z_R^* S_C^+ 9.36^* \log_{10}(SN+1) - 0.20 + \frac{\log_{10}\left[\frac{\Delta PSI}{4.2 - 1.5}\right]}{0.40 + \frac{1094}{(SN+1)^{5.19}}} + 2.32^* \log_{10}M_R^- 8.07$$

LeftRightDifference4.564.560.0

Job No. 211922 Fig. No. C-1

DESIGN CALCULATIONS

DESIGN DATA GORILLA CAPITAL COLORADO SADDLEHORN RANCH, LLC SADDLE HORN RANCH

Equivalent (18 kip) Single Axle Load Applications (ESAL):	ESAL =	36,500
Hveem Stabilometer (R Value) Results:	R =	50
Weighted Structural Number (WSN):	WSN =	1.41

DESIGN EQUATION

$$WSN = C_1D_1 + C_2D_2$$

 $C_1 = 0.44$ Strength Coefficient - Hot Bituminous Asphalt $C_2 = 0.11$ Strength Coefficient - Base Course

 D_1 = Depth of Asphalt (inches) D_2 = Depth of Base Course (inches)

FOR FULL DEPTH ASPHALT SECTION

 $D_1 = (WSN)/C_1 = 3.2$ inches of Full Depth Asphalt Use 5.0 inches Full Depth

FOR ASPHALT + AGGREGATE BASE COURSE SECTION

Asphalt Thickness (t) = 3 inches $D_2 = ((WSN) - (t)(C_1))/C_2 = 0.9$ inches of Aggregate Base Course, use 4.0 inches

RECOMMENDED ALTERNATIVES

- 1. 3.0 inches of Asphalt +
- 4.0 inches of Aggregate Base Course, or
- 2. 5.0 inches of Full-Depth Asphalt

Job No. 211922 Fig. No. C-2