

**PRELIMINARY DRAINAGE REPORT AND MDDP ADDENDUM
FOR
HOMESTEAD NORTH AT STERLING RANCH PRELIMINARY PLAN**

Prepared For:

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Project No. 25188.00**

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ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

Mike Bramlett, Colorado P.E. 38861
For and On Behalf of JR Engineering, LLC

DEVELOPER'S STATEMENT:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Business Name: SR Land, LLC

By: _____

Title: _____

Address: 20 Boulder Crescent, Suite 200
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El Paso County:

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volumes 1 and 2 and Engineering Criteria Manual, as amended.

Jennifer Irvine, P.E.
County Engineer/ ECM Administrator

Date

Conditions:



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- Appendix B – Hydrologic Calculations
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PURPOSE

This document is the Preliminary Drainage Report and MDDP Addendum for Homestead North at Sterling Ranch. The purpose of this report is to identify on-site and off-site drainage patterns, storm sewer, culvert and inlet locations, areas tributary to the site, and to safely route developed storm water to adequate outfall facilities. It is important to note that Homestead North at Sterling Ranch is intended to be constructed in two phases with both phases being evaluated in this report. Assumptions have been made with regards to Phase 2 in order to size and evaluate the site drainage infrastructure. This report will be confirmed or amended in the event that the phase 2 lot configuration has significant changes.

GENERAL SITE DESCRIPTION

GENERAL LOCATION

Homestead North at Sterling Ranch and the undeveloped land to the north (hereby referred to as the “site”) is a proposed development with a total area of approximately 88 acres.

The site is located in the northeast quarter of Section 33 and the southeast quarter of section 28, Township 12 South, Range 65 West of the Sixth Principal Meridian in the County of El Paso, State of Colorado. The site is located immediately east of Vollmer Road. The site is bounded by Briargate Parkway to the south, an unplatted vacant residential parcel to the north, and Sand Creek borders the site to east. The parcels are planned to be platted after approval of the Preliminary Plan. Refer to the vicinity map in Appendix A for additional information.

DESCRIPTION OF PROPERTY

The site is currently being designed to accommodate approximately 228 single-family residential lots and development is to be completed in two phases (totaling approximately 88 acres). The site is comprised of variable sloping grasslands that generally slope(s) downward to the east at 3 to 7% towards the Sand Creek tributary basin.

Soil characteristics are comprised of Type B hydrologic Soil groups. Refer to the soil survey map in Appendix A for additional information.

The Soils and Geology study on the site showed a potentially unstable region directly adjacent to the western bank of Sand Creek on the northeast corner of the site. At the time of final design, specifications from a Geotechnical Engineer will be implemented to ensure that the developed site is safe.

The Sand Creek is within the eastern portion of the site. Currently, JR Engineering is performing studies and plans to address Sand Creek stabilization.

There are no known irrigation facilities located on the project site.

FLOODPLAIN STATEMENT

Based on the FEMA Firm Maps Number 08041C0533G and 08041C0535G revised December 7, 2018, the vast majority of the development is located within Zone X, or areas area outside the Special Flood Hazard Area (SFHA) and higher than the elevation of the 0.2-percent-annual-chance (or 500-year) flood. The eastern property boundary will be platted to the center of Sand creek placing a portion of the site within Zone AE. The area of disturbance for site grading is located outside of the delineated floodway within Zone X. The FEMA map containing the site has been presented in Appendix A.

EXISTING DRAINAGE CONDITIONS

MAJOR BASIN DESCRIPTIONS

The site lies within the Sand Creek Drainage Basin based on the “Sand Creek Drainage Basin Planning Study” (DBPS) completed by Kiowa Engineering Corporation in January 1993, revised March 1996. The Sand Creek Drainage Basin covers approximately 54 square miles and is divided into major sub-basins. The site is within the respective sub-basin is shown in Appendix E.

The Sand Creek DBPS assumed the Homestead North at Sterling Ranch property to have a "large lot residential" use for the majority of the site. However, the proposed Sterling Ranch master plan is a mix of; school, multi-family, single-family, and commercial land uses, resulting in higher runoff. The site generally drains from north to south consisting of rolling hills. Currently, the site is used as pasture land for cattle. Sand Creek is located in the east portion of the site running north to south. This reach of drainage conveyance is not currently improved. There are a few stock ponds within the creek channel used for cattle watering. Currently, JR engineering is performing studies and plans to address Sand Creek stabilization adjacent to the site.

The proposed drainage on the site closely follows the approved "Master Development Drainage Plan for Sterling Ranch", (MMDP) prepared by M&S Civil Consultants, Inc., dated October 24, 2018. The MMDP “Developed Hydrologic Conditions Map” as shown within Appendix E, shows the estimated detention for the site. The site is tributary to basins SC3-18, SC3-17, and a portion of basin SC-322. Full-spectrum detention in the MMDP was previously analyzed and corresponds to ponds FSD18 and FSD17 for the site. Pond FSD17 is associated with ponds A and B within this report. Pond FSD18 is associated with ponds B and C within this report. Runoff as shown in the proposed M&S conduit RT-10A will be detained within pond C, whereas the 2018 MDDP specified that this runoff outfall directly into Sand Creek. This represents a more conservative approach to attenuate and treat water



quality for the offsite runoff going into Sand Creek. The total estimated/projected detention and estimated outflows from the MDDP are shown in Table 1 on the following page.

Table 1.

FSD17						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	41.8	59.6	85.2	119.0	149.1	180.6
ALLOWABLE RELEASE (CFS)	0.7	11.1	22.5	52	67.2	86.3
MODELED RELEASE (CFS)	0.7	8.4	22.4	52	67.2	86.1
STORED VOLUME (AC-FT)	2.6	2.6	2.8	3.4	4.0	4.7

FSD18						
STORM EVENT (YR)	2	5	10	25	50	100
PEAK INFLOW (CFS)	49.3	67.1	91.0	121.2	147.3	174
ALLOWABLE RELEASE (CFS)	0.6	9.2	18.4	42.2	54.6	69.9
MODELED RELEASE (CFS)	0.6	6.3	18.4	42.2	54.6	69.6
STORED VOLUME (AC-FT)	3.2	3.2	3.4	4.0	4.7	5.3

The MMDP plans for additional detention to the north of the site, as shown in Appendix E. No future offsite detention is necessary for the site.

In summary, the site will have three detention ponds A, B, and C. Ponds A and B associated with pond FSD17 of the M&S MDDP and Ponds B and C associated with pond FSD18. The release rates of these ponds will be below 90% of the historic drainage in continuity with the approved M&S MDDP. The report deviates with MDDP and detains and treats water quality from the existing offsite runoff from basin SC3-19 of the M&S MDDP via proposed storm pipe along Vollmer road and Briargate parkway that goes into pond C and outfalls within Sand Creek. The MDDP showed the runoff going into Sand Creek undetained. The total net detention being stored onsite in the 100 year event is 14.2 Acre-ft, as shown in Tables 2.1-2.3 of this report. The total runoff released from the detention ponds is 209.3 cfs in the 100 year event for the three ponds, as shown in Tables 2.1-2.3 of this report. The drainage for Vollmer and the corresponding offsite tributary area is detained treated for water quality within pond C.

EXISTING SUB-BASIN DRAINAGE

The existing/ predeveloped site consists of 3 onsite basins (H1, H2, and H3) and one offsite basin (2). This historic basins outfall to Sand Creek at 2 outfalls as shown in the Historic Drainage Map in Appendix D. A sub-division to the north of the site is being developed called “Retreat at Timberidge”. Runoff from this sub-division will be detained and will not impact storm-water runoff on the Sterling Ranch Homestead site.

Basin E-1 ($Q_5 = 1.1$ cfs, $Q_{100} = 5.2$ cfs) is 4.5 acres of undeveloped land adjacent to the northwest portion of Vollmer Road. Runoff from this basin drains to a 24” CMP pipe and outfalls on the eastern side of Vollmer Road and outfalls into Sand Creek.

Basin E-2 ($Q_5 = 28.1$ cfs, $Q_{100} = 192.9$ cfs) is 180.3 acres of undeveloped land adjacent to the northwest portion of Vollmer Road. Runoff from this basin drains to a 24” CMP pipe and outfalls on the eastern side of Vollmer Road and outfalls into Sand Creek.

Basin E-3 ($Q_5 = 2.1$ cfs, $Q_{100} = 13.6$ cfs) is 12.39 acres of undeveloped land adjacent to the western portion of Vollmer Road. Runoff from this basin drains offsite into a road side swale adjacent to Vollmer Road.

Basin E-4 ($Q_5 = 9.9$ cfs, $Q_{100} = 72.3$ cfs) is 70.5 acres of undeveloped land to the south of Retreat at Timber Ridge and on the eastern side of sand creek. Runoff from this basin drains to design point 4o.

Basin E-5 ($Q_5 = 3.4$ cfs, $Q_{100} = 24.9$ cfs) is 18.8 acres of undeveloped land adjacent to the eastern portion of Sand Creek. Runoff from this basin sheet flow to the south and ultimately drains to Sand Creek in confluence with flow from basin E-4 **at Design Point 5o.**

Basin E-6.1 ($Q_5 = 17.8$ cfs, $Q_{100} = 130.4$ cfs) is 124.9 acres of undeveloped land that drains to the south directly into sand creek at design point 6.1o.

Basin E-6.2 ($Q_5 = 7.5$, $Q_{100} = 55.4$ cfs) is 49.61 acres of undeveloped land that drains to a low point directly adjacent to basin E-6.1 at design point 6.2o. Runoff from this basin then drains to Sand Creek directly south of design point 6.1o in confluence with runoff from E-6.1.

Basin H1 ($Q_5 = 8.9$ cfs, $Q_{100} = 61.1$ cfs) is 45.3 acres of undeveloped land covered in native prairie grass at DP 1h.

Basin H2 ($Q_5 = 3.5$ cfs, $Q_{100} = 25.7$ cfs) is 15.9 acres of undeveloped land covered in native prairie grass. This basin drains directly into Sand Creek. The basin is to the south east of Vollmer road. This basin drains directly into Sand Creek at DP 2h.



29.1?

Basin H3 (Q5 = 6.1 cfs, Q100 = 41.8 cfs) is 21.9 acres of undeveloped land covered in native prairie grass. This basin drains directly into Sand Creek at DP 3h. The basin is to the south east of Vollmer road and North of Briargate Parkway.

INTERIM DRAINAGE CONDITIONS

An Interim Condition Drainage map has been provided for the early grading area of Homestead, and a map is provided in Appendix D. The early grading area consists of the southern portion of Homestead as well as Briargate Parkway and Sterling Ranch Road. This area was split into 2 basins corresponding to the two proposed sediment basins and the areas tributary to them.

Basin C-1 (Q5 = 3.6 cfs, Q100 = 26.8 cfs) is 2% impervious and 22.3 Acres. This basin includes early grading from Sterling Ranch Homestead North. Runoff from this basin will drain into a temporary sediment basin at pond C. ← 1

4.3

Basin C-2 (Q5 = 0.6 cfs, Q100 = 4.5 cfs) is 2% impervious and is 2.66 Acres. This basin is part of a temporary channel that diverts off site runoff in continuity with the Historic condition; directly to Sand Creek.

Basin OS (Q5 = 13.3 cfs, Q100 = 97.2 cfs) is an offsite basin that is 124.2 Acres and 2% impervious. This basin is directly tributary to sediment basin number 2.

O-S1 (Q5 = 1.1 cfs, Q100 = 7.1 cfs) is an offsite basin that is 3.6 % impervious and 5.40 Acres. This basin diverts offsite runoff away from the lots to the Sand Creek drainage way.

O-S2 (Q5 = 28.1 cfs, Q100 = 192.9 cfs) is an offsite basin that is 2.8% impervious and 180.3 Acres. This basin drains to an existing 24" CMP pipe and outfalls into the temporary swale that diverts the runoff around the site and into the sand creek tributary.

How does this flow fit into a 24" pipe?

O-S3 (Q5 = 0.9 cfs, Q100 = 3.2 cfs) is an offsite basin that is 18.1 % impervious and is 1.16 Acres. This basin drains into the temporary swale that diverts runoff away from the site.

O-S4 (Q5 = 12.7 cfs, Q100 = 93.5 cfs) is an offsite basin that is 2% impervious, the area is 69.42 Acres. This basin drains to a temporary 24" RCP pipe under the earth work for future Briargate Road and then this runoff ultimately goes to temporary sediment basin number 2.

2.6?

O-S5 (Q5 = 12.8 cfs, Q100 = 18.9 cfs) is an offsite basin that is 2% impervious; the basin has an area of 8.57 acres. The runoff drains to a temporary 24" RCP that goes under the earthwork for future Briargate Road and then the runoff goes to temporary sediment basin number 2.



O-S6 ($Q_5 = 7.8$ cfs, $Q_{100} = 57.1$ cfs) is an offsite basin that is 2% impervious; the basin has an area of 51.2 acres. The runoff drains to a temporary 24" RCP that goes under the earthwork for future Briargate Road and then the runoff goes to temporary sediment basin number 2.

Basin D ($Q_5 = 2.5$ cfs, $Q_{100} = 18.6$ cfs) is 2% impervious and 17.29 Acres. This basin includes Briargate Parkway and Sterling Ranch Road. Runoff from this basin will drain into a temporary sediment basin at pond D. The stormwater requirements for Briargate parkway and Sterling Ranch Road are included with the drainage report for the interim condition, the roads and will be detailed and designed in the Final Drainage Report when it is time to plat the ROW.

PROPOSED DRAINAGE CONDITIONS

PROPOSED SUB-BASIN DRAINAGE

The proposed site was broken up and delineated into three major basins: Basin A (upper-portion), Basin B (mid-portion), and Basin C (lower-portion) of the site. It should be noted that Basin A will be constructed as part of phase 2 of this development and Basins B and C will be constructed as part of Phase 1. Basin A is tributary to Pond A, Basin B is Tributary to Pond B and Basin C is tributary to Pond C. The proposed basin (and sub-basin) delineation is shown on the drainage basin map within Appendix D and is described as follows.

Basin A1 3.67 acres and 52% percent impervious is comprised of single-family residential lots, a residential road Jesse Evans Drive, and a Cul de Sac. Runoff ($Q_5=6.9$ cfs, $Q_{100}=14.7$ cfs) from this basin A1 drains to design point 1A to a 15' type R on-grade inlet. Runoff is then by-passed in the 100 year event to DP 3A.

Basin A2 3.27 acres and 56% percent impervious is comprised of single-family residential lots, a residential road Jesse Evans Drive, and a Cul de Sac. Runoff ($Q_5=6.4$ cfs, $Q_{100}=13.3$ cfs) from this basin drains to design point 2A to a 15' type R on-grade inlet. Runoff is then by-passed in the 100 year event to DP 4A.

Basin A3 4.79 acres and 50% percent impervious is comprised of single-family residential lots, a residential road David Rudabaugh Drive, and a Cul de Sac. Runoff ($Q_5=8.5$ cfs, $Q_{100}=18.4$ cfs) from this basin drains to design point 3A a 15' type R on-grade inlet in confluence with upstream by-pass flow from basin A1.

Basin A4 3.95 acres and 54% percent impervious is comprised of single-family residential lots, a residential road David Rudabaugh Drive, and a Cul de Sac. Runoff ($Q_5=7.4$ cfs, $Q_{100}=15.6$ cfs) from this basin drains to design point 4A a 15' type R on-grade inlet in confluence with upstream by-pass runoff from basin A2.



Basin A5 5.43 acres and 50% percent impervious is comprised of single-family residential lots, a residential road William Downing Drive, and an urban knuckle. Runoff ($Q_5=10.5$ cfs, $Q_{100}=22.6$ cfs) from this basin drains to design point 5A in confluence with upstream by-pass runoff from basin A3 and A1.

Basin A6 3.97 acres and 53% percent impervious is comprised of single-family residential lots, a residential road William Downing Drive, and a cul de sac. Runoff ($Q_5=7.7$ cfs, $Q_{100}=16.3$ cfs) from this basin drains to design point 6A at an on grade inlet in confluence with upstream by-pass runoff from basin A4 and A2.

Basin A7 1.97 acres and 15% percent impervious is comprised of open grass area, and a portion of a residential road Aspen Valley Road. The runoff ($Q_5=1.3$ cfs, $Q_{100}=4.8$ cfs) from this basin drains to design point 7A a 20' type R sump inlet. The runoff from the sump inlet collects tributary runoff basins A7, A5, A3, and A1.

Basin A8 0.46 acres and 52% percent impervious is comprised of a portion of a residential road Aspen Valley Road. The runoff ($Q_5=1.2$ cfs, $Q_{100}=2.6$ cfs) from this basin drains to design point 8A a 15' type R sump inlet. From here on runoff is piped for basin A1-A8 to detention pond A and detained for the water-quality event and up to the 100-year event. In the event the inlet clogs in the 100 year event, runoff will overflow across the curb and gutter and spill directly into pond A.

Basin A9 2.78 acres and 16% percent impervious is comprised of pond A, grass and walk-out lots facing the detention area. Runoff ($Q_5=2.1$ cfs, $Q_{100}=7.4$ cfs) generated in Basin A9 sheet flows into Pond A where it is treated for water-quality and is detained up until the 100 year-event. The UD Detention sheet for pond A is shown in Appendix C of this report.

Pond A has a total tributary area of 30.29 Acres, the net percent impervious area of pond A is 46.3%. Pond A has been conceptually graded in to fit the design volume, as shown in Appendix C of this report. This pond will be built in phase 2 of Homestead North at Sterling Ranch. Pond A will outfall directly into the Sand Creek basin. The WQCV, 5 year and 100 year volumes, releases rates and stages for pond A are shown in Table 2.1 below. These results correspond to the Routed Hydrograph results, as shown in Appendix C of this report.

TABLE 2.1 Pond A			
	Stage -ft	Volume (Acres)	Release Rate (cfs)
WQCV	2.81	0.498	0.2
5 Year	4.99	1.516	7.8
100 Year	7.45	2.989	32.6

Basin B1.1 3.36 acres 45% percent impervious is comprised of single-family residential lots, a local roads Billy Claiborne Drive, Perry Owens Drive and an urban knuckle. The runoff ($Q_5=5.5$ cfs, $Q_{100}=12.5$ cfs) from basin B1.1 drains to design point 1.1B.

Basin B1.2 1.81 acres and 54% percent impervious is comprised of single-family residential lots, a local roads Claiborne Drive, Perry Owens Drive and an urban knuckle. The runoff ($Q_5=3.5$ cfs, $Q_{100}=7.4$ cfs) from basin B1.2 drains to design point 1.2B.

Basin B1.3 0.47 acres and 47% percent impervious is comprised of single-family residential lots and a local roads Aspen Valley Road and Perry Owens Drive. The runoff ($Q_5=1.0$ cfs, $Q_{100}=2.2$ cfs) from basin B1.3 drains to design point 1.3B.

Basin B2 0.82 acres and 58% percent impervious is comprised of the northern portion of a local residential road Sam Bass Drive adjacent to the intersecting at Vollmer road. Runoff ($Q_5=2.3$ cfs, $Q_{100}=4.9$ cfs) from basin B2 drains to design point 2B and confluences with runoff from basins B1.1, B1.2 and B1.3.

Basin B3 0.24 acres and 79% percent impervious is comprised of the southern portion of a local residential road Sam Bass Drive adjacent to the intersection of Vollmer road. Runoff ($Q_5=0.9$ cfs, $Q_{100}=1.7$ cfs) from basin B3 drains to design point 3B.

Basin B4 4.21 acres and 41.3% percent impervious is comprised of single-family residential lots, a local residential road Wheatland Drive and a Cul de Sac. Runoff ($Q_5=7.1$ cfs, $Q_{100}=16.8$ cfs) from this basin drains to design point 4B.

Basin B5 1.75 acres and 58% percent impervious is comprised of single-family residential lots, a residential road Wheatland Drive, and a Cul de Sac. Runoff ($Q_5=4.3$ cfs, $Q_{100}=8.9$ cfs) from basin B5 drains to design point 5B.

Basin B6 3.66 acres and 57% percent impervious is comprised of single-family residential lots and a local residential roads Sam Bass Drive, Aspen Valley Road, Perry Owens Drive and Wheatland Drive. Runoff ($Q_5=9.5$ cfs, $Q_{100}=19.9$ cfs) from basin 6B drains to design point 6B. In total, the flow at design point 6B collects flow from basins B1, B2, B3, B4, and B6.

Basin B7 1.30 acres and 59% percent impervious is comprised of single-family lots, local roads and a Cul de Sac Robert Allison Circle. Runoff ($Q_5=3.1$ cfs, $Q_{100}=6.4$ cfs) from basin B7 drains to design point 7B in confluence with runoff from B5.

Basin B8 1.74 acres and 55% percent impervious is comprised of single-family lots, local road and a Cul de Sac. Runoff ($Q_5=4.1$ cfs, $Q_{100}=8.5$ cfs) from basin B8 drains to design point B8 in confluence with runoff from basins B8, B7 and B5.

Basin B9 3.69 acres and 65% percent impervious is comprised of single-family lots, and an urban knuckle, and local roads Willey Picket Drive and Wheatland Drive. Runoff ($Q_5=6.9$ cfs, $Q_{100}=14.8$ cfs) from Basin B9 drains to design point 9B in a 15' type R sump inlet. In total the runoff from the sump inlet collects runoff from basins B1, B2, B3, B4, B6 and B9.

Basin B10 0.22 acres and 80% percent impervious is comprised of the southeastern side of the local road Wheatland Drive. The runoff from this basin drains to design point B10 ($Q_5=0.8$ cfs, $Q_{100}=1.6$ cfs) a 10' type R sump inlet. The total runoff at design point B10 collected at this site is from basins B5, B7, B8, and B10. The runoff will then ultimately go directly into the pond. In the event the inlet clogs in the 100 year event, runoff will over flow across the curb and gutter and spill directly into pond B.

Basin B11 1.77 acres and 15% percent impervious is comprised of pond B. Runoff ($Q_5=0.9$ cfs, $Q_{100}=3.9$ cfs) generated in Basin B11 sheet flows into Pond B where it is treated for water-quality and is detained up until the 100 year-event. The UD Detention sheet for pond B is shown in Appendix C of this report.

Basin B12 is 2.40 Acres this basin is 40% percent impervious and is comprised of single family walk out lots facing Sand Creek. The runoff ($Q_5=1.5$ cfs, $Q_{100}=4.1$ cfs) from these lots is collected into area inlets. The runoff is then piped directly into pond B.

Pond B has a tributary area 27.87 acres and is 50.0 % impervious. Pond B has been conceptually graded in to fit the design volume, as shown in Appendix C of this report. This pond will be built in phase 1 of Homestead North at Sterling Ranch. The pond B emergency overflow spillway will drain



directly into Sand Creek. The WQCV, 5 year and 100 year volumes, releases rates and stages for pond B are shown in Table 2.2 below. These results correspond to the Routed Hydrograph results, as shown in Appendix C of this report.

TABLE 2.2 Pond B			
	Stage –ft	Volume (Acres)	Release Rate (cfs)
WQCV	3.13	0.483	0.2
5 Year	4.35	1.705	3.5
100 Year	5.20	2.896	28.5

Basin C1 2.82 acres and 69% percent impervious is comprised of single-family lots, and the northwestern side of the local residential roads Texas Jack Drive and Harvey Logan Drive. Runoff ($Q_5=5.4$ cfs, $Q_{100}=11.4$ cfs) from basin C1 drains to design point 1C at Wheatland Drive.

Basin C2.1 0.20 acres and 91% percent impervious is comprised of single-family lots, and the north western side of the residential road Texas Jack Drive. Runoff ($Q_5=0.8$ cfs, $Q_{100}=1.6$ cfs) from basin C2.1 drains to design point 2.1C a 5' on grade type R inlet.

Basin C2.2 4.69 acres and 73% percent impervious is comprised of local roads, single-family lots, and the north western side of the residential road Wheatland Drive. Runoff ($Q_5=9.9$ cfs, $Q_{100}=20.3$ cfs) from basin C2.2 drains to design point 2.2C in confluence with bypass runoff from basin C2.3. The runoff ultimately drains to design point 4C a 20' type R sump inlet. The total runoff from basins C1, C2.1, C2.2, C2.3 and C4.1 is collected within the sump inlet.

Basin C2.3 0.83 acres and 67% percent impervious is comprised of local roads Tom Ketchum Drive Jack Helm Drive and Harvey Logan Drive, single-family lots, and the north western side of the residential road Wheatland Drive. Runoff ($Q_5=1.9$ cfs, $Q_{100}=3.9$ cfs) from basin C2.3 drains to design point 2.3C in confluence with runoff from basin C1 at an on grade 15' Type R inlet.

Basin C3.1 0.35 acres and 73% percent impervious is comprised of single-family lots, and the southeastern side of the residential road Wheatland Drive. Runoff ($Q_5=1.2$ cfs, $Q_{100}=2.4$ cfs) from basin C3.1 drains to design point 3.1C.

Basin C3.2 1.46 acres and 71% percent impervious is comprised of local roads, single-family lots, and the southeastern side of the residential road Wheatland Drive and Tom Ketchum Drive. Runoff ($Q_5=3.6$ cfs, $Q_{100}=7.4$ cfs) from basin C3.2 drains to design point 3.2C.

Basin C4.1 6.35 acres and 65.3% percent impervious is comprised of single-family lots, and the northwestern side of the local residential road Texas Jack Drive, a right in lane and Nat Love Drive. Runoff ($Q_5=12.1$ cfs, $Q_{100}=25.9$ cfs) from basin C4.1 drains to design point 4C a 20' type R sump inlet. The total runoff from basins C1, C2.1, C2.2, C2.3 and C4.1 is collected within the sump inlet.

Basin C4.2 3.44 acres and 58.5% percent impervious is comprised of a local road Texas Jack Drive and single-family lots. Runoff ($Q_5=6$ cfs, $Q_{100}=13.3$ cfs) from basin C4.2 drains to design point 4.2C a 15' type R on grade inlet.

Basin C5 0.16 acres and 81% percent impervious is comprised of the northwestern side of a residential road Wheatland Drive. Runoff ($Q_5=0.6$ cfs, $Q_{100}=1.1$ cfs) from basin C5 drains to design point 5C, a 5' type R sump inlet. Basin C5 collects runoff from basin C3.2 and C5. The runoff from basin C ultimately outfalls into pond C. In the event the inlet clogs at Basin C5 the runoff will overflow to pond C. A berm has been graded to ensure that the overflow path will go into pond C.

Basin C6 2.48 acres and 21% percent impervious is comprised of pond C and some single-family residential area. Runoff ($Q_5=2.5$ cfs, $Q_{100}=8.8$ cfs) generated in Basin B11 sheet flows into Pond C where it is treated for water-quality and is detained up until the 100 year-event. The MHFD Detention sheet for pond C is shown in Appendix C of this report.

Pond C has a tributary area of 224.34 acres and is 10.3 % impervious. Pond C has been conceptually graded in to fit the design volume, as shown in Appendix C of this report. This pond will be built in phase 1 of Homestead North at Sterling Ranch. The Pond C overflow emergency spillway will overflow into Sand Creek. The WQCV, 5 year and 100 year volumes, releases rates and stages for pond C are shown in Table 2.3 below. These results correspond to the Routed Hydrograph results, as shown in Appendix C of this report.

TABLE 2.3 Pond C			
	Stage -ft	Volume (Acres)	Release Rate (cfs)
WQCV	3.38	1.288	0.6
5 Year	5.39	3.069	35.9
100 Year	9.95	8.339	148.2

The following basins are tributary to the adjacent portion of Vollmer Road being designed by JR Engineering. Runoff will be detained within pond C and the runoff will then be released into Sand Creek adjacent to the crossing of Briargate road and Sand Creek.

Basin D1 has a tributary area of 1.22 acres and is 57.1% impervious. Basin D1 consists of the northwest portion of Vollmer road (Rural Cross Section). Runoff from basin D1 ($Q_5=2.1$ cfs, $Q_{100}=4.5$ cfs) drains to an adjacent roadside swale and drains into a type C inlet at design point 1D. From here on the runoff is piped with upstream runoff from basin OS1 into the Vollmer storm sewer system.

Basin D2 has a tributary area of 1.77 acres and is 43.33% impervious. Basin D2 consists of the northeast portion of Vollmer road (Rural Cross Section). Runoff from basin D2 ($Q_5=2.6$ cfs, $Q_{100}=6.3$ cfs) drains to an adjacent roadside swale and drains into a type C inlet at design point 2D. From here on the runoff is piped with upstream runoff from basin OS1 and basin D1 into the Vollmer storm sewer system.

Basin D3 has a tributary area of 0.18 acres and is 67.6% impervious. Basin D3 ($Q_5=0.5$ cfs, $Q_{100}=1.1$ cfs) consists of the northeast portion of Vollmer road. Runoff on from this basin drains to an on grade 5' type R inlet at DP 3D.

Basin D4 has a tributary area of 0.18 acres and is 57.5% impervious. Basin D4 ($Q_5=0.5$ cfs, $Q_{100}=1.1$ cfs) consists of the northwest portion of Vollmer road. Runoff on from this basin drains to an on grade 5' type R inlet at D P4D. 0.3 cfs is by-passed down to DP 6D. Runoff is piped from basin(s) D3 and D4 to the Vollmer storm within the street's R.O.W.

Basin D5 has a tributary area of 0.91 Acres and is 77% impervious. Basin D5 ($Q_5=3.1$ cfs, $Q_{100}=6.1$ cfs) consists of the northeast portion of Vollmer road. Runoff from this basin drains to an on grade type R 10' inlet at the intersection of Vollmer and a right in right out at DP 5D, 0.7 cfs is by-passed downstream to design point D7 in the 100 year event.

Basin D6 has a tributary area of 0.83 Acres and is 69% impervious. Basin D6 ($Q_5=2.5$ cfs, $Q_{100}=5.2$ cfs) consists of the northwestern portion of Vollmer road and the runoff drains into a 10' on grade type R inlet at DP 6D. 0.4 cfs is by-passed to the downstream design point D8 in the 100 yr event.

Basin D7 has a tributary area of 0.73 Acres and is 81.4 % impervious. Basin D7 ($Q_5=2.0$ cfs, $Q_{100}=3.8$ cfs) consists of the northeast portion of Vollmer road. Runoff from this basin drains to an on grade type R 10' inlet at the intersection of Vollmer and Briargate at DP 7D. All of the runoff received by this inlet is captured within the 100 year event.

Basin D8 has a tributary area of 0.66 Acres and is 75.2% impervious. Basin D8 ($Q_5=2.4$ cfs, $Q_{100}=4.6$ cfs) consists of the northwestern portion of Vollmer road and the runoff drains into a 20' on grade type R inlet at DP 8D. 0.7 cfs is by-passed downstream and will drain into a roadside swale in continuity with the current condition.



Basin OS1 has a tributary area of 2.9 Acres and is 2.0% impervious. The runoff from basin OS1 ($Q_5=0.8$ cfs, $Q_{100}=6.0$ cfs) drains into a depression adjacent to on the northwest portion of Vollmer road. The runoff from basin OS1 is captured in a type C inlet at DP 1o, from there on runoff is piped within Vollmer road and outfalls into sand Creek.

Basin OS2 has a tributary area of 179.61 Acres and is 2.0 % impervious. The runoff from the basin ($Q_5=27.1$ cfs, $Q_{100}=179.61$ cfs) drains into a local depression at DP 2o near the northwest portion of Vollmer road. The runoff from the basin is piped within Vollmer Road and outfalls directly into Sand Creek.

Basin OS3 has a tributary area of 11.98 Acres is 2.0 % impervious. The runoff from this basin ($Q_5=2.2$ cfs, $Q_{100}=16.1$ cfs) sheet flows onto Vollmer road and is captured within a 20' type R inlet that is on grade and corresponds to design point 8D.

DRAINAGE DESIGN CRITERIA

DEVELOPMENT CRITERIA REFERENCE

Storm drainage analysis and design criteria for this project were taken from the “*City of Colorado Springs/El Paso County Drainage Criteria Manual*” Volumes 1 and 2 (EPCDCM), dated October 12, 1994, the “*Urban Storm Drainage Criteria Manual*” Volumes 1 to 3 (USDCM) and Chapter 6 and Section 3.2.1 of Chapter 13 of the “*Colorado Springs Drainage Criteria Manual*” (CSDCM), dated May 2014, as adopted by El Paso County.

HYDROLOGIC CRITERIA

All hydrologic data was obtained from the “*El Paso Drainage Criteria Manual*” Volumes 1 and 2, and the “*Urban Drainage and Flood Control District Urban Storm Drainage Criteria Manual*” Volumes 1, 2, and 3. Onsite drainage improvements were designed based on the 5 year (minor) storm event and the 100-year (major) storm event. Runoff was calculated using the Rational Method, and rainfall intensities for the 5-year and the 100-year storm return frequencies were obtained from Table 6-2 of the CSDCM. One hour point rainfall data for the storm events is identified in the chart below. Runoff coefficients were determined based on proposed land use and from data in Table 6-6 from the CSDCM. Time of concentrations were developed using equations from CSDCM. All runoff calculations and applicable charts and graphs are included in the Appendices.

Table 3 - 1-hr Point Rainfall Data

Storm	Rainfall (in.)
5-year	1.50
100-year	2.52



HYDRAULIC CRITERIA

The Rational Method and USDCM's SF-2 and SF-3 forms were used to determine the runoff from the minor and major storms on the site, and the UDFCD MHFD-Detention v4.03 spreadsheet was utilized for evaluating proposed detention and water quality pond. Sump and on-grade inlets were sized using UDFCD UD-Inlet v2.07. Manning's equation was used to size the proposed pipes in this report and StormCAD will be used to model the proposed storm sewer system and to analyze the proposed HGL calculations for Construction Drawings. **and will be included in the Final Drainage Report.**

Nothing in appendix to show any proposed pipe sizes.

Include discussion on proposed swales.

Include discussion on Sand Creek Improvements & how they are same or different from DBPS recommendations.

DRAINAGE FACILITY DESIGN

FOUR STEP PROCESS TO MINIMIZE ADVERSE IMPACTS OF URBANIZATION

In accordance with the El Paso County Drainage Criteria Manual Volume 2, this site has implemented the four step process to minimize adverse impacts of urbanization. The four step process includes reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainage ways, and implementing long-term source controls.

Step 1 – Reducing Runoff Volumes: The Homestead North at Sterling Ranch development project consists single -family homes with open spaces and lawn areas interspersed within the development which helps disconnect impervious areas and reduce runoff volumes. Roof drains from the structures will discharge to lawn areas, where feasible, to allow for infiltration and runoff volume reduction.

Step 2 – Stabilize Drainageways: The site lies within the Sand Creek Drainage Basin. Basin and bridge fees will be due at time of platting. These funds will be used for the channel stabilization being designed by JR Engineering adjacent to the site and on future projects within the basin to stabilize drainageways. The Soils and Geology study on the site showed a potentially unstable region directly adjacent to the western bank of Sand Creek on the northeast corner of the site. At the time of final design, specifications from a Geotechnical Engineer will be implemented to ensure that the developed site is safe. Homestead North lots will discharge into Full Spectrum Detention Ponds, and outflows will be less than or equal to historic flows. Existing flows from the northwest of Vollmer road and runoff from the Vollmer Road improvements will be piped under Vollmer Road and then along the north side of Briargate Parkway and will be detained and treated for water quality directly on-site. The subdivision improvement agreement (SIA) for Sterling Ranch Filing 1 states that “bank stabilization of the Sand Creek channel shall be required prior to any replats of other final plats adjacent to the channel. The design and installation of said improvements shall be accomplished and guaranteed through the normal subdivision review and collateralization process.” Additionally, “Other drainage improvements in Tract D and future tracts containing the Sand Creek Channel, such as drop structures, check structures and similar stabilization or protection improvements, will be designed and constructed by the District with the final construction drawings to be approved by the



County no later than the final platting of the 700th single family lot within the boundaries of the approved Sterling Ranch Sketch Plan and the completion of all said improvements no later than the 800th single family lot with the boundaries of the approved Sterling Ranch Sketch Plan.”

Step 3 – Treat the WQCV: Water Quality treatment for this site is provided in three proposed full spectrum water quality detention ponds: Pond A, B, and Pond C. The runoff from this site will be collected within inlets and conveyed to the proposed ponds via storm sewer. Upon entrance to the ponds, flows will be captured in a forebay designed to promote settlement of suspended solids. A trickle channel is also incorporated into the ponds to minimize the amount of standing water. The outlet structure has been designed to detain the water quality capture volume (WQCV) for 40 hours, and the extended urban runoff volume (EURV) for 72 hours. All flows released from the ponds will be reduced to less than historic rates.

Step 4 – Consider Need for Industrial and Commercial BMPs: There are no commercial or industrial components to this development; therefore no BMPs of this nature are required. BMPs will be utilized to minimize off-site contaminants and to protect the downstream receiving waters. The site is a residential subdivision (ie: not a high-risk site per Figure I-1 in ECM Appendix I), therefore specialized BMPs do not need to be considered. Site specific temporary source control BMPs that will be implemented include, but are not limited to, silt fencing placed around downstream areas of disturbance, construction vehicle tracking pads at the entrances, designated concrete truck washout basin, designated vehicle fueling areas, covered storage areas, spill containment and control, etc. The permanent erosion control BMPs include asphalt drives and parking, storm inlets and storm pipe, three full spectrum water quality and detention ponds, and permanent vegetation.

WATER QUALITY

The site is split into three major basins A, B, and C. Each major basin is serviced by an extended full spectrum water quality / detention pond. For this preliminary drainage report the design points, pipes and inlets are discussed in the Proposed Drainage Conditions section of this report. The corresponding design points, pipes and basin are shown within the Proposed Drainage Map within Appendix D. All the ponds have been designed per Section 13.3.2.1 of Resolution 15-042 of the El Paso County Drainage Criteria Manual. For additional information on pond storage and outlet characteristics see the MHFD sheets within appendix C.

EROSION CONTROL PLAN

We respectfully request that the Erosion Control Plan and Cost Estimate be submitted in conjunction with the grading and erosion control plan and construction assurances posted prior to obtaining a grading permit.

OPERATION & MAINTENANCE

In order to ensure the function and effectiveness of the stormwater infrastructure, maintenance activities such as inspection, routine maintenance, restorative maintenance, rehabilitation and repair, are required. The property owner shall be responsible for the inspection, maintenance, rehabilitation and repair of stormwater and erosion control facilities located on the property unless another party accepts such responsibility in writing and responsibility is properly assigned through legal documentation. Access is provided from onsite facilities and easements for proposed infrastructure located offsite. We respectfully request that the Operation & Maintenance Manual be submitted in conjunction with the construction documents, prior to obtaining a grading permit.

DRAINAGE AND BRIDGE FEES

The site lies within the Sand Creek Drainage Basin. Anticipated drainage and bridge fees will be provided at time of final drainage report and will be due at time of platting (depending on date of plat submittal):

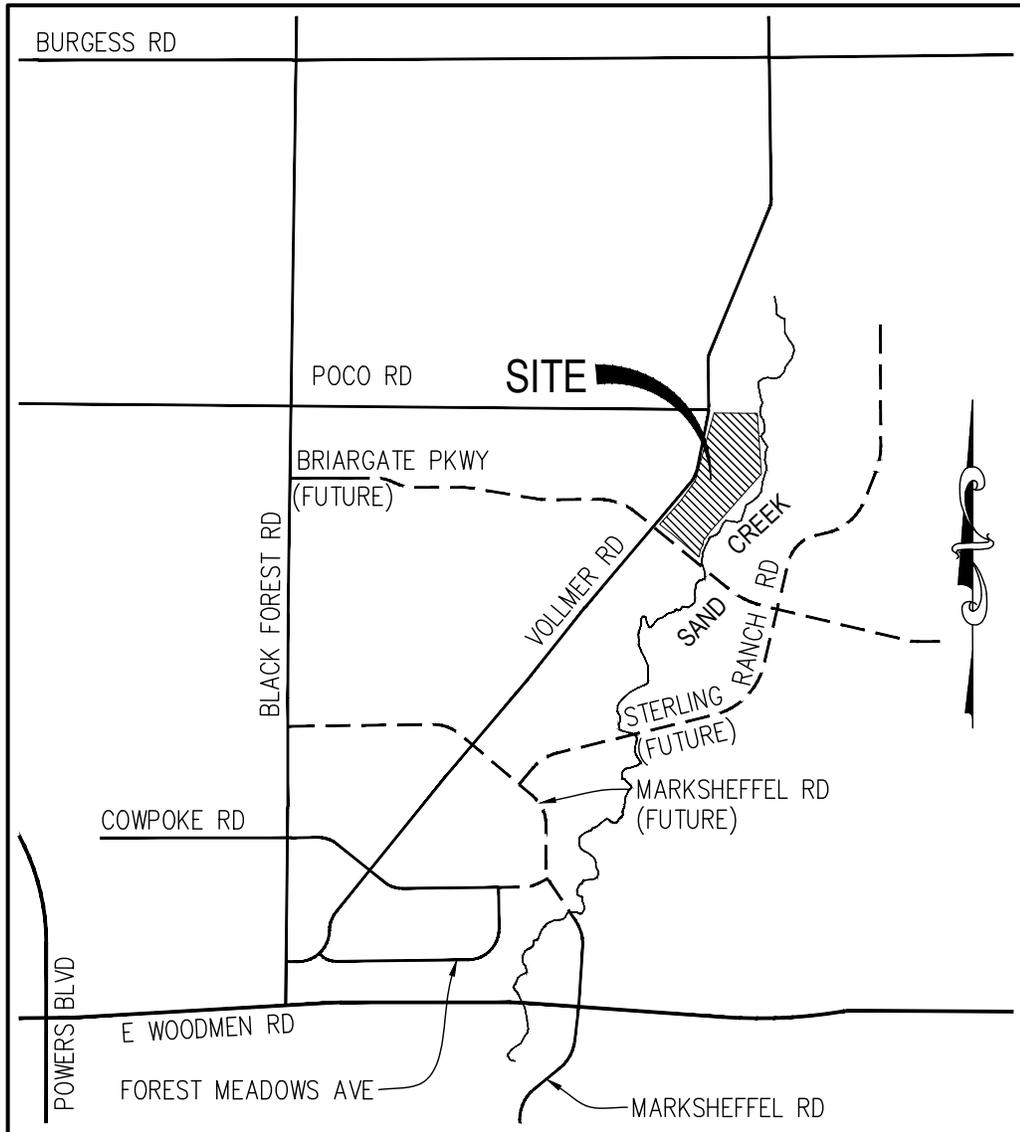
SUMMARY

The proposed Homestead North at Sterling Ranch drainage improvements were designed to meet or exceed the El Paso County Drainage Criteria. The proposed development's ponds are designed to release less than 90% of the predeveloped runoff study associated with the subject site. The proposed development will not adversely affect the offsite drainageways or surrounding development. This report is in conformance and meets the latest El Paso County Storm Drainage Criteria requirements.

REFERENCES

1. "El Paso County and City of Colorado Springs Drainage Criteria Manual, Vol I & II".
 2. El Paso County ECM, 2019
 3. El Paso County DCM Vol. 1 Update, 2015
 4. Urban Storm Drainage Criteria Manual (Volumes 1, 2, and 3), Urban Drainage and Flood Control District, June 2001.
 5. Upper Sand Creek Detention Evaluation Study, Wilson and Company'
 6. Final Drainage Report For Retreat at Timberridge Filing No. 1, Classic Consulting Engineers & Surveyors
 7. Sand Creek Channel Design Report JR Engineering, 2021- Draft
-

Appendix A
Vicinity Map, Soil Descriptions, FEMA Floodplain Map



VICINITY MAP

N.T.S.

VICINITY MAP
 HOMESTEAD FIL. 3
 JOB NO. 25188.00
 04/20/20
 SHEET 1 OF 1



J·R ENGINEERING

A Westrian Company

Centennial 303-740-9393 • Colorado Springs 719-593-2593
 Fort Collins 970-491-9888 • www.jrengineering.com

Hydrologic Soil Group—El Paso County Area, Colorado



Map Scale: 1:5,730 if printed on A portrait (8.5" x 11") sheet.
0 50 100 200 300 Meters
0 250 500 1000 1500 Feet
Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84



MAP LEGEND

- Area of Interest (AOI)**
 -  Area of Interest (AOI)
- Soils**
 - Soil Rating Polygons**
 -  A
 -  A/D
 -  B
 -  B/D
 -  C
 -  C/D
 -  D
 -  Not rated or not available
 - Soil Rating Lines**
 -  A
 -  A/D
 -  B
 -  B/D
 -  C
 -  C/D
 -  D
 -  Not rated or not available
 - Soil Rating Points**
 -  A
 -  A/D
 -  B
 -  B/D
- Water Features**
 -  Streams and Canals
- Transportation**
 -  Rails
 -  Interstate Highways
 -  US Routes
 -  Major Roads
 -  Local Roads
- Background**
 -  Aerial Photography
- Other**
 -  C
 -  C/D
 -  D
 -  Not rated or not available

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.
 Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
 Survey Area Data: Version 17, Sep 13, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 8, 2018—May 26, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
71	Pring coarse sandy loam, 3 to 8 percent slopes	B	90.2	100.0%
Totals for Area of Interest			90.2	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations (BFEs)** and/or **floodways** have been determined, users are encouraged to consult the **Flood Profiles and Floodway Data** and/or **Summary of Stillwater Elevations** tables contained within the **Flood Insurance Study (FIS)** report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The horizontal datum was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988 (NAVD88). These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov> or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, NWS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at <http://www.ngs.noaa.gov>.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, and Anderson Consulting Engineers, Inc. These data are current as of 2009.

This map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the **Flood Profiles and Floodway Data** tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

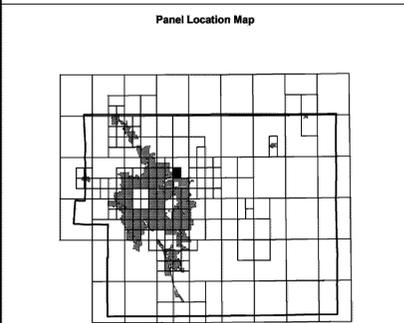
Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels, community map repository addresses, and a listing of communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

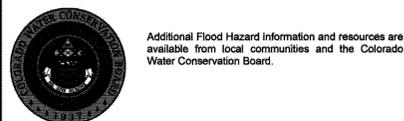
Contact **FEMA Map Service Center (MSC)** via the FEMA Map Information eXchange (FIMX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at <http://www.msc.fema.gov>.

If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA-MAP (1-877-336-2627) or visit the FEMA website at <http://www.fema.gov/business/nfp>.

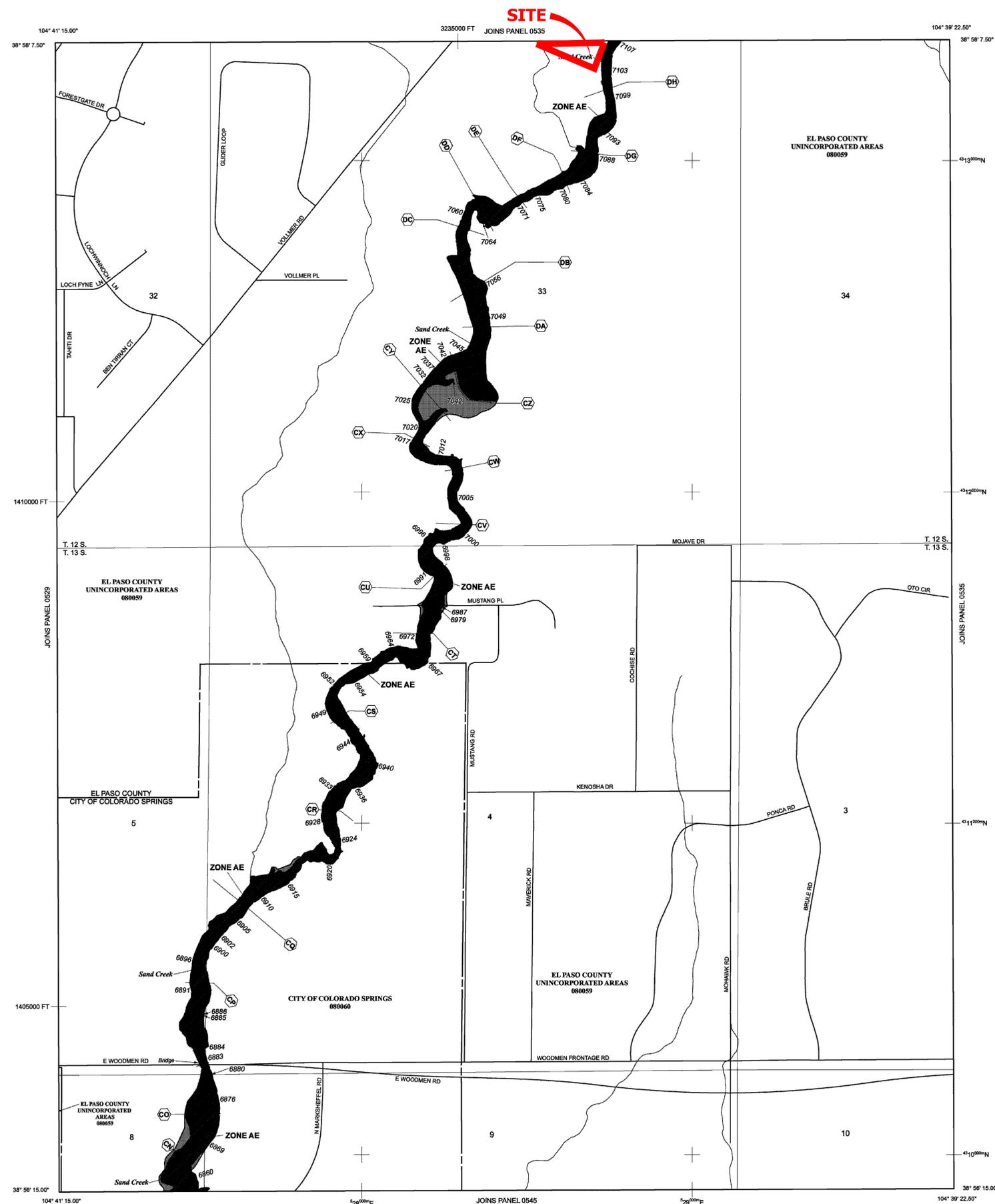
El Paso County Vertical Datum Offset Table	
Flooding Source	Vertical Datum Offset (ft)
REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION	



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).



Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 12 SOUTH, RANGE 65 WEST, AND TOWNSHIP 13 SOUTH, RANGE 66 WEST.

LEGEND

- SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD
- The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.
- ZONE A** No Base Flood Elevations determined.
- ZONE AE** Base Flood Elevations determined.
- ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
- ZONE AO** Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of sheet flow flooding, velocities also determined.
- ZONE AR** Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently identified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
- ZONE A99** Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
- ZONE V** Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
- ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.
- FLOODWAY AREAS IN ZONE AE
- The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.
- OTHER FLOOD AREAS
- ZONE X** Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.
- OTHER AREAS
- ZONE X** Areas determined to be outside the 0.2% annual chance floodplain.
- ZONE D** Areas in which flood hazards are undetermined, but possible.
- COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS
- OTHERWISE PROTECTED AREAS (OPAs)
- CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.
- Floodplain boundary
- Floodway boundary
- Zone D boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
- Base Flood Elevation line and value; elevation in feet*
(EL 887)
- Base Flood Elevation value where uniform within zone; elevation in feet*
- * Referenced to the North American Vertical Datum of 1988 (NAVD 88)
- Cross section line
- Transsect line
- 97° 07' 30.00" 32° 22' 30.00" Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
- 4759000M 1000-meter Universal Transverse Mercator grid ticks, zone 13
- 6000000 FT 5000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0902), Lambert Conformal Conic Projection
- DX5510 Bench mark (see explanation in Notes to Users section of this FIRM panel)
- M1.5 River Mile
- MAP REPOSITORIES
Refer to Map Repositories list on Map Index
- EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
MARCH 17, 1997
- EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.
- For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.
- To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

NATIONAL FLOOD INSURANCE PROGRAM

PANEL 0533G

FIRM
FLOOD INSURANCE RATE MAP
EL PASO COUNTY,
COLORADO
AND INCORPORATED AREAS

PANEL 533 OF 1300
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
COLORADO SPRINGS CITY OF	08000	0533	0
EL PASO COUNTY	08059	0533	0

Notice to User: The Map Number shown below should be used when placing map orders. The Community Number shown above should be used on insurance applications for the subject community.

MAP NUMBER
08041C0533G

MAP REVISED
DECEMBER 7, 2018

Federal Emergency Management Agency

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NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or Floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The horizontal datum was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988 (NAVD88). These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at <http://www.ngs.noaa.gov/> or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, NIMS-12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at <http://www.ngs.noaa.gov/>.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, and Anderson Consulting Engineers, Inc. These data are current as of 2008.

This map reflects more detailed and up-to-date stream channel configurations and floodplain delineations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels, community map repository addresses, and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

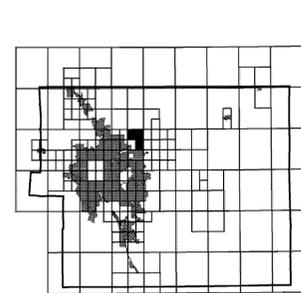
Contact **FEMA Map Service Center (MSC)** via the FEMA Map Information eXchange (FMX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at <http://www.msc.fema.gov/>.

If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2627) or visit the FEMA website at <http://www.fema.gov/business/nfp>.

El Paso County Vertical Datum Offset Table	
Flooding Source	Vertical Datum Offset (ft)

REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION

Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).



Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

- ZONE A** No Base Flood Elevations determined.
- ZONE AE** Base Flood Elevations determined.
- ZONE AH** Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
- ZONE AO** Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
- ZONE AR** Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently deteriorated. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
- ZONE A99** Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
- ZONE V** Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
- ZONE VE** Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

- Floodplain boundary
- Floodway boundary
- Zone D boundary
- CBRS and OPA boundary
- Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities

513 Base Flood Elevation line and value; elevation in feet* (EL 987)
Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

A-A Cross section line

23-23 Transsect line

97° 07' 30.00" 32° 22' 30.00" Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)

4750000N 1000-meter Universal Transverse Mercator grid ticks, zone 13

6000000 FT 5000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0903), Lambert Conformal Conic Projection

DX5510 Bench mark (see explanation in Notes to Users section of this FIRM panel)

M1.5 River Mile

MAP REPOSITORIES Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP MARCH 17, 1997

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 1000'

500 0 1000 2000 FEET

300 0 300 600 METERS

Panel 0535

FIRM FLOOD INSURANCE RATE MAP

EL PASO COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 535 OF 1300 (SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

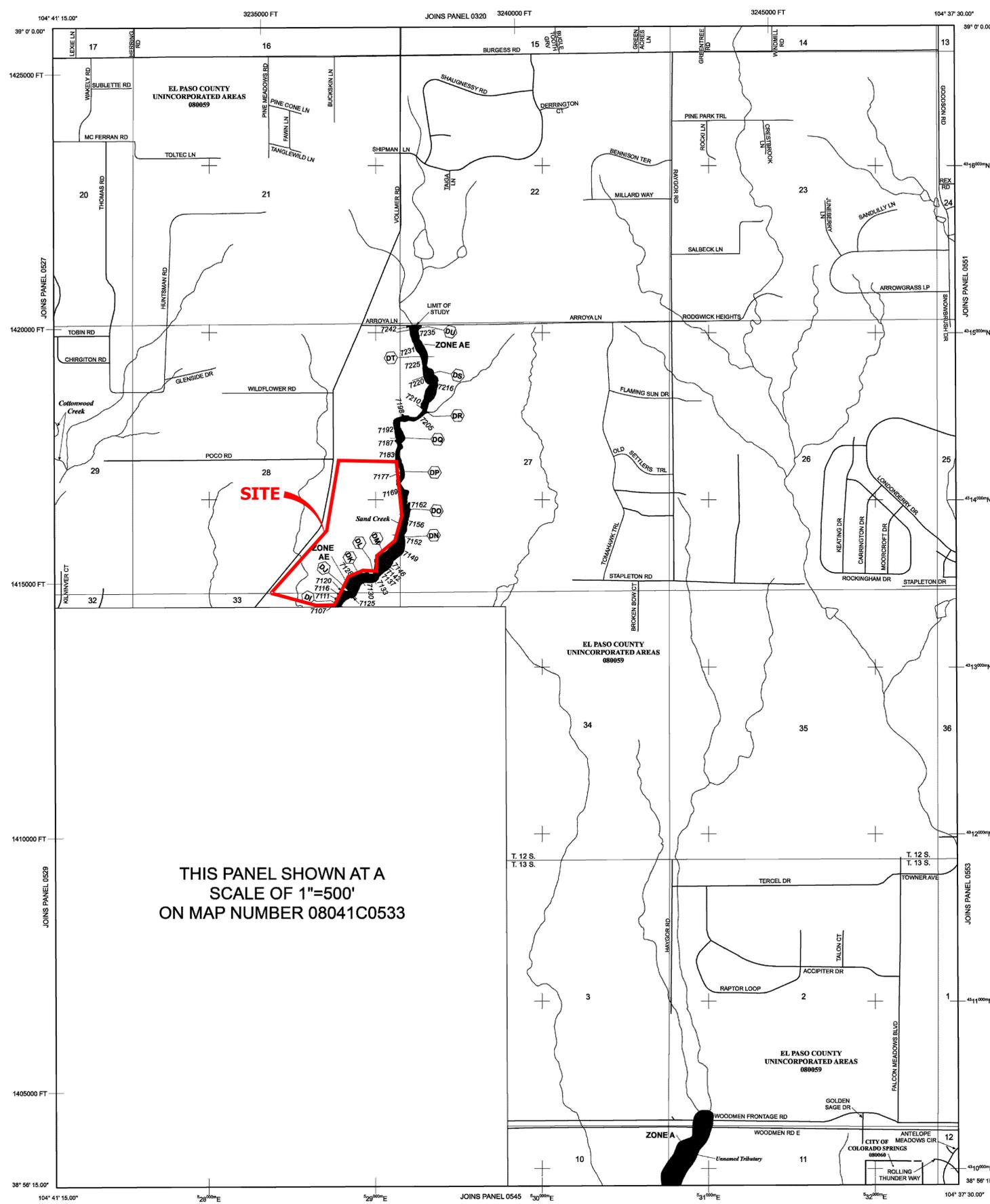
COMMUNITY	NUMBER	PANEL	SUFFIX
COLORADO SPRING CITY OF	09090	0535	0
EL PASO COUNTY	09059	0535	0

Notice to User: The Map Number shown below should be used when placing map orders. The Community Number shown above should be used on insurance applications for the subject community.

MAP NUMBER 08041C0535G

MAP REVISED DECEMBER 7, 2018

Federal Emergency Management Agency



THIS PANEL SHOWN AT A SCALE OF 1"=500' ON MAP NUMBER 08041C0533

NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 12 SOUTH, RANGE 65 WEST, AND TOWNSHIP 13 SOUTH, RANGE 65 WEST.

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Appendix B

Hydrologic Calculations

STANDARD FORM SF-2 TIME OF CONCENTRATION

Subdivision: Existing Conditions Homestead Fil. 3
Location: El Paso County

Project Name: Homestead North
Project No.: 25188.00
Calculated By: ARJ
Checked By:
Date: 9/15/21

SUB-BASIN DATA						INITIAL/OVERLAND (T _i)			TRAVEL TIME (T _t)					t _c CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C _s	C ₁₀₀	L (ft)	S _o (%)	t _i (min)	L _t (ft)	S _t (%)	K	VEL. (ft/s)	t _t (min)	COMP. t _c (min)	TOTAL LENGTH (ft)	Urbanized t _c (min)	t _c (min)
E-1	4.50	B	9%	0.14	0.39	600	1.0%	42.6	3006	4.1%	7.0	3.2	15.7	58.3	3606.0	48.7	48.7
E-2	180.30	B	3%	0.09	0.35	300	1.0%	31.7	3007	1.7%	7.0	3.2	15.7	47.4	3307.0	66.1	47.4
E-3	12.39	B	4%	0.10	0.37	300	1.0%	31.3	3008	1.8%	7.0	3.2	15.7	46.9	3308.0	64.3	46.9
E-4	70.50	B	2%	0.08	0.35	500	1.0%	41.2	2300	3.1%	7.0	4.2	9.1	50.3	2800.0	49.0	49.0
E-5	18.80	B	2%	0.08	0.35	300	1.0%	31.9	930	1.5%	7.0	5.2	3.0	34.9	1230.0	39.3	34.9
E6.1	125.30	B	2%	0.08	0.35	500	1.0%	41.2	2584	1.9%	7.0	6.2	6.9	48.1	3084.0	59.4	48.1
E6.2	49.61	B	2%	0.08	0.35	370	1.0%	35.4	3783	2.5%	7.0	7.2	8.8	44.2	4153.2	68.6	44.2
H1	45.30	B	3%	0.09	0.36	150	2.0%	17.8	1074	2.3%	7.0	1.1	16.9	34.7	1224.0	38.1	34.7
H2	15.90	B	2%	0.08	0.35	150	2.0%	17.9	425	2.0%	7.0	1.0	7.2	25.1	575.0	31.1	25.1
H3	29.10	B	3%	0.09	0.35	150	1.4%	20.3	645	1.9%	7.0	1.0	11.1	31.3	795.0	33.8	31.3

NOTES:

$$t_c = t_i + t_t$$

Where:

- t_c = computed time of concentration (minutes)
- t_i = overland (initial) flow time (minutes)
- t_t = channelized flow time (minutes).

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$

Where:

- t_t = channelized flow time (travel time, min)
- L_t = waterway length (ft)
- S_o = waterway slope (ft/ft)
- V_t = travel time velocity (ft/sec) = K√S_o
- K = NRCS conveyance factor (see Table 6-2).

$$\text{Equation 6-2} \quad t_i = \frac{0.395(1.1 - C_s)\sqrt{L_i}}{S_o^{0.33}}$$

Where:

- t_i = overland (initial) flow time (minutes)
- C_s = runoff coefficient for 5-year frequency (from Table 6-4)
- L_i = length of overland flow (ft)
- S_o = average slope along the overland flow path (ft/ft).

$$\text{Equation 6-4} \quad t_c = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$$

Where:

- t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.
- L_t = length of channelized flow path (ft)
- i = imperviousness (expressed as a decimal)
- S_t = slope of the channelized flow path (ft/ft).

Equation 6-3

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

Equation 6-5

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Existing Conditions Homestead Fil. 3
Location: El Paso County
Design Storm: 5-Year

Project Name: Homestead North
Project No.: 25188.00
Calculated By: ARJ
Checked By:
Date: 9/15/21

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS	
		Basin ID	Area (Ac)	Runoff Coeff.	t _c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q _{street/swale} (cfs)	C*A (ac)	Slope (%)	Q _{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t _t (min)		
	1o	E-1	4.50	0.14	48.7	0.61	1.76	1.1																
	1h	H1	45.30	0.09	34.7	3.93	2.26	8.9	48.7	4.54	1.76	8.0												Drains to swale H1 and E1
	2h	H2	15.90	0.08	25.1	1.27	2.75	3.5	48.7	5.81	1.76	10.2												Accepts runoff from H1, H2 and E-1
	2o	E-2	180.30	0.09	47.4	15.62	1.80	28.1																
	3h	H3	29.10	0.09	31.3	2.51	2.42	6.1	47.4	18.13	1.80	32.6												Total Runoff; E-2 and H3
	3o	E-3	12.39	0.10	46.9	1.24	1.81	2.2																Runoff: E-3 Runoff in Vollmer rd side swale
	4o	E-4	70.50	0.08	49.0	5.64	1.75	9.9																
	5o	E-5	18.80	0.08	34.9	1.50	2.26	3.4	49.0	7.14	1.75	12.5												Total Runoff; E-4 and E-5
	6.2o	E6.2	49.61	0.08	44.2	3.97	1.90	7.5																To low point
	6.1o	E6.1	125.30	0.08	48.1	10.02	1.77	17.8	49.0	21.13	1.75	36.9												Total Runoff E-6, E-4, E-5 Runoff makes it's way into sand creek

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Existing Conditions Homestead Fil. 3
 Location: El Paso County
 Design Storm: 100-Year

Project Name: Homestead North
 Project No.: 25188.00
 Calculated By: ARJ
 Checked By:
 Date: 9/15/21

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q _{street/swale} (cfs)	C*A (ac)	Slope (%)	Q _{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t _t (min)	
	1o	E-1	4.50	0.39	48.7	1.76	2.94	5.2															
	1h	H1	45.30	0.36	34.7	16.08	3.80	61.1	48.7	17.84	2.94	52.5											Drains to swale H1 and E1
	2h	H2	15.90	0.35	25.1	5.57	4.61	25.7	48.7	23.41	2.94	68.9											Accepts runoff from H1, H2 and E-1
	2o	E-2	180.30	0.35	47.4	64.00	3.01	192.9															Total Runoff: E-2 and H3
	3h	H3	29.10	0.35	31.3	10.32	4.05	41.8	47.4	74.32	3.01	224.0											
	3o	E-3	12.39	0.37	46.9	4.52	3.04	13.7															Runoff: E-3 Runoff in Vollmer rd side swale
	4o	E-4	70.50	0.35	49.0	24.68	2.93	72.3															Total Runoff: E-4 and E-5
	5o	E-5	18.80	0.35	34.9	6.58	3.78	24.9	49.0	31.26	2.93	91.6											
	6.2o	E6.2	49.61	0.35	44.2	17.36	3.19	55.4															To low point
	6.1o	E6.1	125.30	0.35	48.1	43.86	2.97	130.4	49.0	92.48	2.93	270.9											Total Runoff E-6, E-4, E-5 Runoff makes it's way into sand creek

Notes:
 Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
 All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

COMPOSITE % IMPERVIOUS & COMPOSITE RUNOFF COEFFICIENT CALCULATIONS

Subdivision: Homestead Fil. 3 -Interim Conditons Object Name: Homestead North
 Location: El Paso County Project No.: 25188.00
 Calculated By: ARJ
 Checked By: _____
 Date: 9/15/21

Basin ID	Total Area (ac)	Streets/Paved (100% Impervious)				Residential (45%-65% Impervious)				Lawns (2% Impervious)				Basins Total Weighted C Values		Basins Total Weighted % Imp.
		C ₅	C ₁₀₀	Area (ac)	Weighted % Imp.	C ₅	C ₁₀₀	Area (ac)	Weighted % Imp.	C ₅	C ₁₀₀	Area (ac)	Weighted % Imp.	C ₅	C ₁₀₀	
C-1	22.30	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	22.30	2.0%	0.08	0.35	2.0%
C-2	2.70	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	2.70	2.0%	0.08	0.35	2.0%
D	17.29	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	17.29	2.0%	0.08	0.35	2.0%
OS	124.20	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	124.20	2.0%	0.08	0.35	2.0%
O-S1	5.40	0.90	0.96	0.09	1.7%	0.45	0.59	0.00	0.0%	0.08	0.35	5.31	2.0%	0.09	0.36	3.6%
O-S2	180.30	0.90	0.96	1.46	0.8%	0.45	0.59	0.00	0.0%	0.08	0.35	178.84	2.0%	0.09	0.35	2.8%
O-S3	1.16	0.90	0.96	0.19	16.4%	0.45	0.59	0.00	0.0%	0.08	0.35	0.97	1.7%	0.21	0.45	18.1%
O-S4	69.42	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	69.42	2.0%	0.08	0.35	2.0%
O-S5	8.57	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	8.57	2.0%	0.08	0.35	2.0%
O-S6	51.20	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	51.20	2.0%	0.08	0.35	2.0%

STANDARD FORM SF-2 TIME OF CONCENTRATION

Subdivision: Homestead Fil. 3 **-Interim Condions**
 Location: El Paso County

Project Name: Homestead North
 Project No.: 25188.00
 Calculated By: ARJ
 Checked By: _____
 Date: 9/15/21

SUB-BASIN DATA						INITIAL/OVERLAND (T _i)			TRAVEL TIME (T _t)					t _c CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C ₅	C ₁₀₀	L (ft)	S _o (%)	t _i (min)	L _t (ft)	S _t (%)	K	VEL. (ft/s)	t _t (min)	COMP. t _c (min)	TOTAL LENGTH (ft)	Urbanized t _c (min)	t _c (min)
C-1	22.30	B	2%	0.08	0.35	150	2.0%	17.9	1378	2.2%	7.0	1.0	22.1	40.1	1528.0	42.3	40.1
C-2	2.66	B	2%	0.08	0.35	30	2.0%	8.0	1000	2.0%	7.0	1.0	16.8	24.9	1030.0	38.4	24.9
D	17.29	B	2%	0.08	0.35	30	102.0%	2.2	6925	14.0%	7.0	2.6	44.1	46.3	6955.0	58.9	46.3
OS	124.20	B	2%	0.08	0.35	600	2.0%	35.9	2899.91	1.8%	7.0	0.9	51.5	87.4	3499.9	64.5	64.5
O-S1	5.40	B	4%	0.09	0.36	300	1.5%	27.5	999	2.5%	7.0	1.1	15.0	42.6	1299.0	36.5	36.5
O-S2	180.30	B	3%	0.09	0.35	300	1.0%	31.7	3007	1.7%	7.0	3.2	15.7	47.4	3307.0	66.1	47.4
O-S3	1.16	B	18%	0.21	0.45	30	2.0%	7.0	580	3.9%	7.0	1.4	7.0	14.0	610.0	27.2	14.0
O-S4	69.42	B	2%	0.08	0.35	500	1.0%	41.2	645	1.9%	7.0	1.0	11.1	52.3	1145.0	34.0	34.0
O-S5	8.57	B	2%	0.08	0.35	300	150.0%	6.1	400	2.0%	7.0	1.0	6.7	12.8	700.0	30.7	12.8
O-S6	51.20	B	2%	0.08	0.35	370	1.0%	35.4	3783	2.5%	7.0	7.2	8.8	44.2	4153.2	68.6	44.2

NOTES:

$$t_c = t_i + t_t$$

Where:

- t_c = computed time of concentration (minutes)
- t_i = overland (initial) flow time (minutes)
- t_t = channelized flow time (minutes).

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60V_t}$$

Where:

- t_t = channelized flow time (travel time, min)
- L_t = waterway length (ft)
- S_o = waterway slope (ft/ft)
- V_t = travel time velocity (ft/sec) = K√S_o
- K = NRCS conveyance factor (see Table 6-2).

Equation 6-2

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S_o^{0.33}}$$

Where:

- t_i = overland (initial) flow time (minutes)
- C₅ = runoff coefficient for 5-year frequency (from Table 6-4)
- L = length of overland flow (ft)
- S_o = average slope along the overland flow path (ft/ft).

Equation 6-4

$$t_c = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$$

Where:

- t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.
- L_t = length of channelized flow path (ft)
- i = imperviousness (expressed as a decimal)
- S_t = slope of the channelized flow path (ft/ft).

Equation 6-3

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

Basin D - requirements will be detailed and designed in the Final Drainage Report when its time to plat the ROW for Sterling Ranch Road and Briargate Parkway.

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Homestead Fil. 3
Location: El Paso County
Design Storm: 5-Year

-Interim Conditons

Project Name: Homestead North
Project No.: 25188.00
Calculated By: ARJ
Checked By:
Date: 9/15/21

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	t _c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q _{street/swale} (cfs)	C*A (ac)	Slope (%)	Q _{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t _t (min)	
		O-S3	1.16	0.21	14.0	0.25	3.63	0.9															
	2	O-S2	180.30	0.09	47.4	15.62	1.80	28.1	47.4	15.87	1.80	28.5											Tributary Basins: O-S2 and O-S3 Drains to swale
	2.1	C-2	2.66	0.08	24.9	0.21	2.76	0.6	47.4	16.08	1.80	28.9											Tributary Basins: C-2, O-S2 and O-S3 To Sand Creek
	1	C-1	22.30	0.08	40.1	1.78	2.05	3.6															Tributary Basins: C-1 Pond C
	3	O-S1	5.40	0.09	36.5	0.51	2.19	1.1															To sand creek
	4	O-S4	69.42	0.08	34.0	5.55	2.29	12.7															Runoff conveyed from 24" RCP under interim grading
	5	O-S5	8.57	0.08	12.8	0.69	3.75	2.6															Runoff conveyed from 24" RCP under interim grading
	6	O-S6	51.20	0.08	44.2	4.10	1.90	7.8															Runoff conveyed from 24" RCP under interim grading
	7	OS	124.20	0.08	64.5	9.94	1.33	13.3	64.5	20.79	1.33	27.7											Runoff from basins OS, OS-4-OS-6
	8	D	17.29	0.08	46.3	1.38	1.83	2.5															Runoff from basin D
	0								64.5	21.66	1.33	28.9											Runoff from OS, OS-6,O-S5 and OS-4

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

-Interim Conditions

Subdivision: Homestead Fil. 3
Location: El Paso County
Design Storm: T00-Year

Project Name: Homestead North
Project No.: 25188.00
Calculated By: ARJ
Checked By:
Date: 9/15/21

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q _{street/swale} (cfs)	C*A (ac)	Slope (%)	Q _{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t _t (min)	
		O-S3	1.16	0.45	14.0	0.52	6.09	3.2															
	2	O-S2	180.30	0.35	47.4	64.00	3.01	192.9	47.4	64.52	3.01	194.5											Tributary Basins: O-S2 and O-S3 Drains to swale
	2.1	C-2	2.66	0.35	24.9	0.93	4.64	4.3	47.4	65.45	3.01	197.3											Tributary Basins: C-2, O-S2 and O-S3 To Sand Creek
	1	C-1	22.30	0.35	40.1	7.81	3.44	26.8															Tributary Basins: C-1 Pond C
	3	O-S1	5.40	0.36	36.5	1.94	3.67	7.1															To sand creek
	4	O-S4	69.42	0.35	34.0	24.30	3.85	93.5															Runoff conveyed from 24" RCP under interim grading
	5	O-S5	8.57	0.35	12.8	3.00	6.30	18.9															Runoff conveyed from 24" RCP under interim grading
	6	O-S6	51.20	0.35	44.2	17.92	3.19	57.1															Runoff conveyed from 24" RCP under interim grading
	7	OS	124.20	0.35	64.5	43.47	2.24	97.2	64.5	90.63	2.24	202.6											Runoff from basins OS, OS-4-OS-6
	8	D	17.29	0.35	46.3	6.05	3.07	18.6															Runoff from basin D
	0								64.5	96.68	2.24	216.2											Runoff from OS, OS-6,O-S5 and OS-4

Notes:
Street and Pipe C*A values are determined by Q/i using the catchment's intensity value.
All pipes are private and RCP unless otherwise noted. Pipe size shown in table column.

COMPOSITE % IMPERVIOUS & COMPOSITE RUNOFF COEFFICIENT CALCULATIONS

Subdivision: Homestead Fil. 3
 Location: El Paso County
 Calculated By: ARJ
 Checked By:
 Date: 10/8/21

-Proposed Conditions

Basin ID	Total Area (ac)	Streets/Paved (100% Impervious)				Residential (45%-65% Impervious)				Lawns (2% Impervious)				Basins Total Weighted C Values		Basins Total Weighted % Imp.
		C _s	C ₁₀₀	Area (ac)	Weighted % Imp.	C _s	C ₁₀₀	Area (ac)	Weighted % Imp.	C _s	C ₁₀₀	Area (ac)	Weighted % Imp.	C _s	C ₁₀₀	
A1	3.67	0.90	0.96	0.82	22.4%	0.45	0.59	2.41	29.5%	0.08	0.35	0.44	0.2%	0.51	0.64	52.2%
A2	3.27	0.90	0.96	0.84	25.6%	0.45	0.59	2.19	30.1%	0.08	0.35	0.24	0.1%	0.54	0.67	55.9%
A3	4.79	0.90	0.96	0.79	16.4%	0.45	0.59	3.56	33.4%	0.08	0.35	0.45	0.2%	0.49	0.63	50.0%
A4	3.95	0.90	0.96	0.77	19.6%	0.45	0.59	2.99	34.1%	0.08	0.35	0.18	0.1%	0.52	0.65	53.8%
A5	5.43	0.90	0.96	0.67	12.4%	0.45	0.59	4.47	37.0%	0.08	0.35	0.29	0.1%	0.49	0.62	49.5%
A6	3.97	0.90	0.96	0.67	17.0%	0.45	0.59	3.17	36.0%	0.08	0.35	0.12	0.1%	0.51	0.65	53.0%
A7	1.97	0.90	0.96	0.22	11.0%	0.45	0.59	0.12	2.7%	0.08	0.35	1.63	1.7%	0.19	0.43	15.4%
A8	0.46	0.90	0.96	0.21	45.6%	0.45	0.59	0.05	5.4%	0.08	0.35	0.20	0.8%	0.50	0.66	51.8%
A9	2.78	0.90	0.96	0.00	0.0%	0.45	0.59	0.93	15.1%	0.08	0.35	1.85	1.3%	0.20	0.43	16.4%
Pond A	30.29															46.4%
B1.1	3.36	0.90	0.96	0.48	14.2%	0.45	0.59	2.29	30.7%	0.08	0.35	0.59	0.4%	0.45	0.60	45.2%
B1.2	1.81	0.90	0.96	0.32	17.9%	0.45	0.59	1.43	35.5%	0.08	0.35	0.06	0.1%	0.52	0.65	53.5%
B1.3	0.47	0.90	0.96	0.20	41.4%	0.45	0.59	0.05	5.0%	0.08	0.35	0.22	0.9%	0.46	0.63	47.4%
B2	0.82	0.90	0.96	0.33	40.2%	0.45	0.59	0.32	17.3%	0.08	0.35	0.17	0.4%	0.55	0.69	57.9%
B3	0.24	0.90	0.96	0.19	78.7%	0.45	0.59	0.00	0.0%	0.08	0.35	0.05	0.4%	0.73	0.83	79.1%
B4	4.21	0.90	0.96	0.46	10.8%	0.45	0.59	2.63	28.1%	0.08	0.35	1.13	0.5%	0.40	0.57	39.4%
B5	1.75	0.90	0.96	0.44	25.1%	0.45	0.59	1.26	32.4%	0.08	0.35	0.05	0.1%	0.55	0.68	57.5%
B6	3.66	0.90	0.96	1.25	34.2%	0.45	0.59	1.85	22.8%	0.08	0.35	0.55	0.3%	0.55	0.68	57.3%
B7	1.30	0.90	0.96	0.38	29.5%	0.45	0.59	0.84	29.1%	0.08	0.35	0.08	0.1%	0.56	0.69	58.7%
B8	2.30	0.90	0.96	0.53	22.9%	0.45	0.59	1.63	31.9%	0.08	0.35	0.14	0.1%	0.53	0.66	54.9%
B9	3.69	0.90	0.96	0.80	21.7%	0.45	0.59	2.43	42.7%	0.08	0.35	0.47	0.3%	0.50	0.64	64.6%
B10	0.22	0.90	0.96	0.18	79.1%	0.45	0.59	0.00	0.0%	0.08	0.35	0.05	0.4%	0.73	0.83	79.5%
B11	1.65	61.50	0.96	0.00	0.0%	0.45	0.59	0.35	13.7%	0.08	0.35	1.30	1.6%	0.16	0.40	15.2%
B12	2.40	0.90	0.96	0.00	0.0%	0.45	0.59	1.45	39.3%	0.08	0.35	0.95	0.8%	0.30	0.50	40.1%
Pond B	27.87															50.0%
C1	2.82	0.90	0.96	0.49	17.2%	0.45	0.59	2.25	51.7%	0.08	0.35	0.09	0.1%	0.52	0.65	69.0%
C2.1	0.20	0.90	0.96	0.18	90.5%	0.45	0.59	0.00	0.0%	0.08	0.35	0.02	0.2%	0.82	0.90	90.7%
C2.2	4.69	0.90	0.96	1.26	26.9%	0.45	0.59	3.33	46.1%	0.08	0.35	0.10	0.0%	0.56	0.68	73.0%
C2.3	0.83	0.90	0.96	0.28	34.1%	0.45	0.59	0.41	32.4%	0.08	0.35	0.13	0.3%	0.54	0.68	66.9%
C3.1	0.35	0.90	0.96	0.25	72.8%	0.45	0.59	0.00	0.0%	0.08	0.35	0.09	0.5%	0.68	0.79	73.3%
C3.2	1.46	0.90	0.96	0.42	28.4%	0.45	0.59	0.96	42.8%	0.08	0.35	0.08	0.1%	0.56	0.68	71.3%
C4.1	6.35	0.90	0.96	1.04	16.4%	0.45	0.59	4.76	48.8%	0.08	0.35	0.55	0.2%	0.49	0.63	65.3%
C4.2	3.44	0.90	0.96	0.59	17.1%	0.45	0.59	2.20	41.6%	0.08	0.35	0.65	0.4%	0.46	0.61	59.1%
C5	0.16	0.90	0.96	0.13	80.9%	0.45	0.59	0.00	0.0%	0.08	0.35	0.03	0.4%	0.74	0.84	81.3%
C6	2.48	0.90	0.96	0.27	11.0%	0.45	0.59	0.32	8.5%	0.08	0.35	1.89	1.5%	0.22	0.45	21.0%
D1	1.83	0.90	0.96	0.69	37.5%	0.45	0.59	0.00	0.0%	0.08	0.35	1.14	1.2%	0.39	0.58	38.8%
D2	1.77	0.90	0.96	0.75	42.1%	0.45	0.59	0.00	0.0%	0.08	0.35	1.02	1.2%	0.43	0.61	43.3%
D3	0.18	0.90	0.96	0.12	67.0%	0.45	0.59	0.00	0.0%	0.08	0.35	0.06	0.7%	0.63	0.76	67.6%
D4	0.19	0.90	0.96	0.11	56.6%	0.45	0.59	0.00	0.0%	0.08	0.35	0.08	0.9%	0.54	0.70	57.5%
D5	0.91	0.90	0.96	0.70	76.5%	0.45	0.59	0.00	0.0%	0.08	0.35	0.21	0.5%	0.71	0.82	77.0%
D6	0.83	0.90	0.96	0.57	68.4%	0.45	0.59	0.00	0.0%	0.08	0.35	0.26	0.6%	0.64	0.77	69.0%
D7	0.73	0.90	0.96	0.59	81.0%	0.45	0.59	0.00	0.0%	0.08	0.35	0.14	0.4%	0.74	0.84	81.4%
D8	0.66	0.90	0.96	0.49	74.7%	0.45	0.59	0.00	0.0%	0.08	0.35	0.17	0.5%	0.69	0.81	75.2%
OffSite Basins																
OS1	2.85	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	2.85	2.0%	0.08	0.35	2.0%
OS2	179.61	0.90	0.96	0.91	0.5%	0.45	0.59	0.00	0.0%	0.08	0.35	178.71	2.0%	0.08	0.35	2.5%
OS3	11.99	0.90	0.96	0.00	0.0%	0.45	0.59	0.00	0.0%	0.08	0.35	11.99	2.0%	0.08	0.35	2.0%
Pond C	224.34															10.3%

STANDARD FORM SF-2 TIME OF CONCENTRATION

Subdivision: Homestead Fil. 3

-Proposed Conditions

Location: El Paso County

Project Name: Homestead North

Project No.: 25188.00

Calculated By: ARJ

Checked By: _____

Date: 10/8/21

SUB-BASIN						INITIAL/OVERLAND			TRAVEL TIME					t _c CHECK			FINAL
DATA						(T _i)			(T _t)					(URBANIZED BASINS)			
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C ₅	C ₁₀₀	L (ft)	S _o (%)	t _i (min)	L _t (ft)	S _t (%)	K	VEL. (ft/s)	t _t (min)	COMP. t _c (min)	TOTAL LENGTH (ft)	Urbanized t _c (min)	
A1	3.67	B	52%	0.51	0.64	150	2.0%	10.4	589	3.0%	20.0	3.5	2.8	13.3	739.0	20.6	13.3
A2	3.27	B	56%	0.54	0.67	150	2.0%	9.9	595	1.6%	20.0	2.5	3.9	13.8	745.0	21.2	13.8
A3	4.79	B	50%	0.49	0.63	150	2.0%	10.7	645	2.9%	20.0	3.4	3.2	13.9	795.0	21.5	13.9
A4	3.95	B	54%	0.52	0.65	150	2.0%	10.2	653	1.9%	20.0	2.7	4.0	14.2	803.0	21.7	14.2
A5	5.43	B	50%	0.49	0.62	187	7.0%	8.0	531	2.1%	20.0	2.9	3.1	11.1	718.0	21.5	11.1
A6	3.97	B	53%	0.51	0.65	230	4.5%	9.8	435	1.6%	20.0	2.6	2.8	12.6	665.0	20.4	12.6
A7	1.97	B	15%	0.19	0.43	240	4.9%	15.1	125	0.6%	20.0	1.5	1.4	16.5	365.0	25.9	16.5
A8	0.46	B	52%	0.50	0.66	9.5	2.0%	2.7	230	1.9%	20.0	2.8	1.4	4.1	239.5	18.9	5.0
A9	2.78	B	16%	0.20	0.43	30	2.0%	7.0	535	0.5%	20.0	1.4	6.3	13.4	565.0	34.4	13.4
B1.1	3.36	B	45%	0.45	0.60	125	2.0%	10.5	610	3.1%	20.0	3.5	2.9	13.4	735.0	22.1	13.4
B1.2	1.81	B	54%	0.52	0.65	150	2.0%	10.2	577	3.4%	20.0	3.7	2.6	12.8	727.0	20.1	12.8
B1.3	0.47	B	47%	0.46	0.63	50	2.0%	6.5	270	2.0%	20.0	2.8	1.6	8.1	320.0	20.0	8.1
B2	0.82	B	58%	0.55	0.69	9.5	2.0%	2.4	368	3.4%	20.0	3.7	1.7	4.1	377.5	18.1	5.0
B3	0.24	B	79%	0.73	0.83	9.5	2.0%	1.7	360	3.7%	20.0	3.9	1.6	3.2	369.5	14.1	5.0
B4	4.21	B	39%	0.40	0.57	25	2.0%	5.0	680	1.6%	20.0	2.5	4.5	9.5	705.0	25.5	9.5
B5	1.75	B	58%	0.55	0.68	25	2.0%	3.9	590	1.6%	20.0	2.6	3.8	7.8	615.0	20.7	7.8
B6	3.66	B	57%	0.55	0.68	9.5	2.0%	2.4	855	3.0%	20.0	3.5	4.1	6.6	864.5	21.1	6.6
B7	1.30	B	59%	0.56	0.69	50	1.0%	6.9	315	1.5%	20.0	2.4	2.1	9.0	365.0	18.5	9.0
B8	2.30	B	55%	0.53	0.66	50	1.0%	7.3	280	1.0%	20.0	2.0	2.4	9.6	330.0	19.5	9.6
B9	3.69	B	65%	0.50	0.64	140	2.0%	10.2	600	2.9%	20.0	3.4	2.9	13.1	740.0	18.3	13.1
B10	0.22	B	80%	0.73	0.83	9.5	2.0%	1.6	200	0.5%	20.0	1.4	2.4	4.1	209.5	14.9	5.0
B11	1.65	B	15%	0.16	0.40	30	2.0%	7.4	250	0.1%	20.0	0.4	9.3	16.7	280.0	40.1	16.7
B12	2.40	B	40%	0.30	0.50	30	2.0%	6.3	900	0.1%	20.0	0.4	33.5	39.8	930.0	65.1	39.8
C1	2.82	B	69%	0.52	0.65	130	2.0%	9.6	690	2.6%	20.0	3.2	3.6	13.1	820.0	18.1	13.1
C2.1	0.20	B	91%	0.82	0.90	7.5	2.0%	1.1	300	1.0%	20.0	2.0	2.5	3.6	307.5	12.9	5.0
C2.2	4.69	B	73%	0.56	0.68	150	2.0%	9.5	630	2.5%	20.0	3.2	3.3	12.8	780.0	17.0	12.8
C2.3	0.83	B	67%	0.54	0.68	100	2.0%	8.0	462	3.3%	20.0	3.6	2.1	10.1	562.0	16.9	10.1

STANDARD FORM SF-2 TIME OF CONCENTRATION

Subdivision: Homestead Fil. 3 **-Proposed Conditions**
 Location: El Paso County

Project Name: Homestead North
 Project No.: 25188.00
 Calculated By: ARJ
 Checked By: _____
 Date: 10/8/21

SUB-BASIN						INITIAL/OVERLAND			TRAVEL TIME					t _c CHECK			FINAL
DATA						(T _i)			(T _t)					(URBANIZED BASINS)			
BASIN ID	D.A. (ac)	Hydrologic Soils Group	Impervious (%)	C ₅	C ₁₀₀	L (ft)	S _o (%)	t _i (min)	L _t (ft)	S _t (%)	K	VEL. (ft/s)	t _t (min)	COMP. t _c (min)	TOTAL LENGTH (ft)	Urbanized t _c (min)	
C3.1	0.35	B	73%	0.68	0.79	9.5	2.0%	1.9	460	2.6%	20.0	3.2	2.4	4.2	469.5	16.0	5.0
C3.2	1.46	B	71%	0.56	0.68	50	2.0%	5.5	365	1.1%	20.0	2.1	2.9	8.4	415.0	16.9	8.4
C4.1	6.35	B	65%	0.49	0.63	150	2.0%	10.7	366	4.8%	21.0	4.6	1.3	12.0	516.0	16.4	12.0
C4.2	3.44	B	59%	0.46	0.61	150	2.0%	11.3	367	4.6%	22.0	4.7	1.3	12.6	517.0	17.6	12.6
C5	0.16	B	81%	0.74	0.84	9.5	2.0%	1.6	368	0.3%	23.0	1.3	4.9	6.4	377.5	17.7	6.4
C6	2.48	B	21%	0.22	0.45	15	2.0%	4.9	160	0.5%	20.0	1.4	1.9	6.8	175.0	25.6	6.8
D1	1.83	B	39%	0.39	0.58	30	1.0%	7.0	1365	2.5%	15.0	2.4	9.7	16.7	1395.0	29.5	16.7
D2	1.77	B	43%	0.43	0.61	30	1.0%	6.7	1365	2.5%	15.0	2.4	9.6	16.3	1395.0	28.2	16.3
D3	0.18	B	68%	0.63	0.76	30	1.0%	4.7	150	1.7%	20.0	3.2	0.8	5.4	180.0	15.5	5.4
D4	0.19	B	57%	0.54	0.70	30	1.0%	5.5	150	1.7%	20.0	3.2	0.8	6.3	180.0	17.4	6.3
D5	0.91	B	77%	0.71	0.82	15	2.0%	2.2	740	3.4%	20.0	3.2	3.9	6.0	755.0	16.3	6.0
D6	0.83	B	69%	0.64	0.77	15	2.0%	2.6	740	3.4%	20.0	3.2	3.9	6.4	755.0	17.8	6.4
D7	0.52	B	81%	0.74	0.84	15	2.0%	2.0	550	2.0%	20.0	4.2	2.2	4.2	565.0	15.3	5.0
D8	0.66	B	75%	0.69	0.81	15	2.0%	2.3	550	2.0%	20.0	5.2	1.8	4.0	565.0	16.5	5.0
OS1	2.85	B	2%	0.08	0.35	50	1.0%	13.0	280	3.9%	7.0	3.2	1.5	14.5	330.0	28.2	14.5
OS2	179.61	B	2%	0.08	0.35	300	1.0%	31.8	3007	1.7%	7.0	3.2	15.7	47.4	3307.0	66.3	47.4
OS3	11.99	B	2%	0.08	0.35	300	1.0%	31.9	3008	1.8%	7.0	3.2	15.7	47.6	3308.0	66.2	47.6

NOTES:

$$t_c = t_i + t_t$$

Where:

t_c = computed time of concentration (minutes)

t_i = overland (initial) flow time (minutes)

t_t = channelized flow time (minutes).

$$t_t = \frac{L_t}{60K\sqrt{S_o}} = \frac{L_t}{60P_t}$$

Where:

t_t = channelized flow time (travel time, min)

L_t = waterway length (ft)

S_o = waterway slope (ft/ft)

P_t = travel time velocity (ft/sec) = K√S_o

K = NRCS conveyance factor (see Table 6-2).

Equation 6-2

$$t_i = \frac{0.395(1.1 - C_5)\sqrt{L}}{S_o^{0.33}}$$

Where:

t_i = overland (initial) flow time (minutes)

C₅ = runoff coefficient for 5-year frequency (from Table 6-4)

L = length of overland flow (ft)

S_o = average slope along the overland flow path (ft/ft).

Equation 6-4

$$t_c = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_o}}$$

Where:

t_c = minimum time of concentration for first design point when less than t_c from Equation 6-1.

L_t = length of channelized flow path (ft)

i = imperviousness (expressed as a decimal)

S_o = slope of the channelized flow path (ft/ft).

Equation 6-3

Table 6-2. NRCS Conveyance factors, K

Type of Land Surface	Conveyance Factor, K
Heavy meadow	2.5
Tillage/field	5
Short pasture and lawns	7
Nearly bare ground	10
Grassed waterway	15
Paved areas and shallow paved swales	20

Equation 6-5

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Homestead Fil. 3
Location: El Paso County
Design Storm: 5-Year

-Proposed Conditions

Project Name: Homestead North
Project No.: 25188.00
Calculated By: ARJ
Checked By:
Date: 10/8/21

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE			TRAVEL TIME			REMARKS	
		Basin ID	Area (Ac)	Runoff Coeff.	t _c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q _{street/swale} (cfs)	C*A (ac)	Slope (%)	Q _{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)		t _t (min)
	1a	A1	3.67	0.51	13.3	1.86	3.70	6.9					0.00	0	2.84					335	3.4	1.7	On-grade Type R Inlet, Bypass to DP 3a
	3a	A3	4.79	0.49	13.9	2.34	3.63	8.5	14.9	2.34	3.53	8.3	0.20	0.06	2.8					110	3.3	0.5	On-grade Type R Inlet, Bypass to DP 5a
	5a	A5	5.43	0.49	11.1	2.64	3.98	10.5	14.9	2.70	3.53	9.5											Street Flow
	7a	A7	1.97	0.19	16.5	0.38	3.38	1.3	16.5	3.08	3.38	10.4											Flow Confluences at sump inlet
	2a	A2	3.27	0.54	13.8	1.76	3.64	6.4					0.00	0	2.84					335	3.4	1.7	On-grade Type R Inlet, Bypass to DP 4a
	4a	A4	3.95	0.52	14.2	2.06	3.61	7.4	15.5	2.06	3.47	7.2	3.60	1.04	2.8					110	3.3	0.5	On-grade Type R Inlet, Bypass to DP 6a
	6a	A6	3.97	0.51	12.6	2.04	3.79	7.7	15.5	3.08	3.47	10.7											Street Flow
	8a	A8	0.46	0.50	5.0	0.23	5.17	1.2	16.0	3.31	3.42	11.3											Flow Confluences at sump inlet
	9A	A9	2.78	0.20	13.4	0.57	3.70	2.1	16.5	6.38	3.38	21.6											Flows into Pond A. All of Pond A.
	1.1b	B1.1	3.36	0.45	13.4	1.50	3.69	5.5					0.00	0	2.6					210	3.2	1.1	On-grade Type R Inlet, Bypass to DP 2B
	1.2b	B1.2	1.81	0.52	12.8	0.94	3.75	3.5					0.00	0	2.6					235	3.2	1.2	On-grade Type R Inlet, Bypass to DP 2B
	1.3b	B1.3	0.47	0.46	8.1	0.22	4.45	1.0															Street flow
	2b	B2	0.82	0.55	5.0	0.45	5.17	2.3	14.5	0.67	3.58	2.40											Street flow
	3b	B3	0.24	0.73	5.0	0.18	5.17	0.9															Street flow
	4b	B4	4.21	0.40	9.5	1.68	4.20	7.1					0.1	0.02	2.5					340	3.2	1.8	Type R Inlet, Bypass to DP 6B
	6b	B6	3.66	0.55	6.6	2.00	4.76	9.5	14.5	2.87	3.58	10.28											Recives by-pass flows from Basins (B1.1, B1.2 and B4), Direct Runoff from B1.3,B2,B3, and B6
	9b	B9	3.69	0.50	13.1	1.85	3.72	6.9	14.5	3.37	3.58	12.07											Sump inlet Recives by-pass flows from (B1.1, B1.2 and B4) Direct Runoff from B1.3,B2,B3, B6 and B9
	5b	B5	1.75	0.55	7.8	0.96	4.51	4.3															Street flow
	7b	B7	1.30	0.56	9.0	0.73	4.28	3.1	9.0	1.69	4.28	7.2	0.1	0.05	1.6					340	2.5	2.2	On-grade Type R Inlet, Bypass to DP 8B
	8b	B8	2.30	0.53	9.6	1.22	4.19	5.1	11.3	1.27	3.95	5.0											Street Flow, Recives bypass flow from DP 7B

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Homestead Fil. 3
Location: El Paso County
Design Storm: 5-Year

-Proposed Conditions

Project Name: Homestead North
Project No.: 25188.00
Calculated By: ARJ
Checked By:
Date: 10/8/21

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE			TRAVEL TIME			REMARKS	
		Basin ID	Area (Ac)	Runoff Coeff.	t _c (min)	C*A (Ac)	I (in/hr)	Q (cfs)	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q _{street/swale} (cfs)	C*A (ac)	Slope (%)	Q _{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)		t _t (min)
	10b	B10	0.22	0.73	5.0	0.16	5.17	0.8	11.3	1.43	3.95	5.7											Sump inlet recives by-pass flow from 7b and runoff from 5b,8b, and 10b
	11b	B11	1.65	0.16	16.7	0.26	3.36	0.9	14.5	5.07	3.58	18.1											Pond B
	12b	B12	2.40	0.30	39.8	0.73	2.06	1.5															Runoff Collected from walk out lots facing sand creek
	2.5								39.8	12.88	2.06	26.5											Flow confluences into Pond B. All of Basin B
	1c	C1	2.82	0.52	13.1	1.46	3.72	5.4															
	2.3c	C2.3	0.83	0.54	10.1	0.45	4.11	1.9	13.1	1.91	3.72	7.1	0.1	0.03	1.6				185	2.5	1.2		On-Grade Type R Inlet, Street runoff from basin C1 and basin C2.3
	2.1c	C2.1	0.20	0.82	5.0	0.16	5.17	0.8					0.0	0	2.83				630	3.4	3.1		On-Grade Type R Inlet
	2.2c	C2.2	4.69	0.56	12.8	2.64	3.76	9.9	13.1	2.64	3.72	9.8											Runoff from basins 1c, 2.3c, 2.1c and 2.2c
	4.2c	C4.2	3.44	0.46	12.6	1.57	3.78	5.9					0.00	0	2.84				1010	3.4	5.0		On-Grade Type R Inlet, by pass to 4.2c
	4c	C4.1	6.35	0.49	12.0	3.13	3.85	12.1	17.6	5.77	3.28	18.9											Sump Inlet
	3.1c	C3.1	0.35	0.68	5.0	0.24	5.17	1.2					0.00	0	2.84				200	3.4	1.0		On-Grade Type R inlet, By pass flow to DP 3.2c
	3.2c	C3.2	1.46	0.56	8.4	0.82	4.39	3.6	8.4	0.82	4.39	3.6											Recives by-pass flow from DP 3.1c
	5c	C5	0.16	0.74	6.4	0.12	4.79	0.6	8.4	0.94	4.39	4.1											Sump Inlet
	6c	C6	2.48	0.22	6.8	0.54	4.71	2.5															Conluenced flow for Pond C
	3.5								13.1	11.13	3.72	41.4											
	o1	OS1	2.85	0.08	14.5	0.23	3.57	0.8															offsite basin to type D inlet
	1d	D1	1.83	0.39	16.7	0.71	3.36	2.4															Tributary basin D1 NW portion of Vollmer in Swale
	1.1d								16.7	0.94	3.36	3.2											Tributary basin D1 and OS1 NW portion of Vollmer in Swale
	2d	D2	1.77	0.43	16.3	0.75	3.40	2.5															Tributary basin D2 SE portion of Vollmer in Swale

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
 (RATIONAL METHOD PROCEDURE)

Subdivision: Homestead Fil. 3
 Location: El Paso County
 Design Storm: 100-Year

-Proposed Conditions

Project Name: Homestead North
 Project No.: 25188.00
 Calculated By: ARJ
 Checked By:
 Date: 10/8/21

Description	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET/SWALE			PIPE				TRAVEL TIME			REMARKS
		Basin ID	Area (ac)	Runoff Coeff.	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	t _c (min)	C*A (ac)	I (in/hr)	Q (cfs)	Q _{street/swale} (cfs)	C*A (ac)	Slope (%)	Q _{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	t _r (min)	
	1a	A1	3.67	0.64	13.3	2.36	6.22	14.7					2.80	0.45	2.84					335	3.4	1.7	On-grade Type R Inlet, Bypass to DP 3a
	3a	A3	4.79	0.63	13.9	3.01	6.10	18.4	14.9	3.46	5.92	20.5	6.10	1.03	2.8					110	3.3	0.5	On-grade Type R Inlet, Bypass to DP 5a
	5a	A5	5.43	0.62	11.1	3.38	6.67	22.6	14.9	4.41	5.92	26.1											Street Flow
	7a	A7	1.97	0.43	16.5	0.85	5.68	4.8	16.5	5.26	5.68	29.9											Flow Confluences at sump inlet
	2a	A2	3.27	0.67	13.8	2.18	6.12	13.3					1.60	0.26	2.84					335	3.4	1.7	On-grade Type R Inlet, Bypass to DP 4a
	4a	A4	3.95	0.65	14.2	2.57	6.05	15.6	15.5	2.83	5.83	16.5	3.60	0.62	2.8					110	3.3	0.5	On-grade Type R Inlet, Bypass to DP 6a
	6a	A6	3.97	0.65	12.6	2.56	6.35	16.3	15.5	3.18	5.83	18.5											Street Flow
	8a	A8	0.46	0.66	5.0	0.30	8.68	2.6	16.0	3.48	5.75	20.0											Flow Confluences at sump inlet
	9A	A9	2.78	0.43	13.4	1.20	6.20	7.4	16.5	8.74	5.68	49.6											Flows into Pond A. All of Pond A.
	1.1b	B1.1	3.36	0.60	13.4	2.01	6.20	12.5					1.50	0.24	2.6					210	3.2	1.1	On-grade Type R Inlet, Bypass to DP 2B
	1.2b	B1.2	1.81	0.65	12.8	1.17	6.30	7.4					0.20	0.03	2.6					235	3.2	1.2	On-grade Type R Inlet, Bypass to DP 2B
	1.3b	B1.3	0.47	0.63	8.1	0.30	7.47	2.2															Street flow
	2b	B2	0.82	0.69	5.0	0.56	8.68	4.9	14.5	1.13	6.00	6.80702											Street flow, Recives bypass flow from 1.1b,1.2b and direct runoff from basin 1.3b
	3b	B3	0.24	0.83	5.0	0.20	8.68	1.7															Street flow
	4b	B4	4.21	0.57	9.5	2.38	7.05	16.8					4.1	0.58	2.5					340	3.2	1.8	Type R Inlet, Bypass to DP 6B
	6b	B6	3.66	0.68	6.6	2.49	8.00	19.9	14.5	4.41	6.00	26.4519											Recives by-pass flows from Basins (B1.1, B1.2 and B4) , Direct Runoff from B1.3,B2,B3, and B6
	9b	B9	3.69	0.64	13.1	2.36	6.25	14.8	14.5	5.05	6.00	30.2946											Sump inlet Recives by-pass flows from (B1.1, B1.2 and B4) Direct Runoff from B1.3,B2,B3, B6 and B9
	5b	B5	1.75	0.68	7.8	1.18	7.57	8.9															Street flow
	7b	B7	1.30	0.69	9.0	0.89	7.19	6.4	9.0	2.07	7.19	14.9	3.2	0.44	1.6					340	2.5	2.2	On-grade Type R Inlet, Bypass to DP 8B

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
 (RATIONAL METHOD PROCEDURE)

Subdivision: Homestead Fil. 3
 Location: El Paso County
 Design Storm: 100-Year

-Proposed Conditions

Project Name: Homestead North
 Project No.: 25188.00
 Calculated By: ARJ
 Checked By:
 Date: 10/8/21

Description	Design Point	DIRECT RUNOFF						TOTAL RUNOFF				STREET/SWALE			PIPE			TRAVEL TIME			REMARKS		
		Basin ID	Area (ac)	Runoff Coeff.	t _c (min)	C*A (ac)	I _t (in/hr)	Q (cfs)	t _c (min)	C*A (ac)	I _t (in/hr)	Q (cfs)	Q _{street/swale} (cfs)	C*A (ac)	Slope (%)	Q _{pipe} (cfs)	C*A (ac)	Slope (%)	Pipe Size (inches)	Length (ft)		Velocity (fps)	t _t (min)
	8b	B8	2.30	0.66	9.6	1.52	7.03	10.7	11.3	1.96	6.63	13.0											Street Flow, Recives bypass flow from DP 7B
	10b	B10	0.22	0.83	5.0	0.19	8.68	1.6	11.3	2.15	6.63	14.3											Sump inlet recives by-pass flow from 7b and runoff from 5b,8b, and 10b
	11b	B11	1.65	0.40	16.7	0.66	5.64	3.7															
	12b	B12	2.40	0.50	39.8	1.19	3.45	4.1															
	2.5								39.8	17.10	3.45	59.0											Flow confluences into Pond B. All of Basin B
	1c	C1	2.82	0.65	13.1	1.82	6.25	11.4															
	2.3c	C2.3	0.83	0.68	10.1	0.56	6.91	3.9	13.1	2.38	6.25	14.9	3.6	0.58	1.6					185	2.5	1.2	On-Grade Type R Inlet, Street runoff from basin C1 and basin C2.3
	2.1C	C2.1	0.20	0.90	5.0	0.18	8.68	1.6				0.1	0.01	2.83						630	3.4	3.1	On-Grade Type R Inlet
	2.2C	C2.2	4.69	0.68	12.8	3.21	6.32	20.3	13.1	3.22	6.25	20.1											Runoff from basins 1c, 2.3c, 2.1c and 2.2c
	4.2c	C4.2	3.44	0.61	12.6	2.09	6.35	13.3				2.60	0.41	2.84						1010	3.4	5.0	On-Grade Type R Inlet, by pass to 4.2c
	4C	C4.1	6.35	0.63	12.0	4.00	6.47	25.9	17.6	7.63	5.51	42.0											Sump Inlet
	3.1c	C3.1	0.35	0.79	5.0	0.28	8.68	2.4				0.60	0.07	2.84						200	3.4	1.0	On-Grade Type R inlet, By pass flow to DP 3.2c
	3.2c	C3.2	1.46	0.68	8.4	1.00	7.37	7.4	8.4	1.07	7.37	7.9											Recives by-pass flow from DP 3.1c
	5C	C5	0.16	0.84	6.4	0.13	8.04	1.0	8.4	1.20	7.37	8.8											Sump Inlet
	6C	C6	2.48	0.45	6.8	1.11	7.91	8.8	17.6														
	3.5								17.6	14.38	5.51	79.2											Conluenced flow for Pond C
	o1	OS1	2.85	0.35	14.5	1.00	6.00	6.0															offsite basin to type D inlet
	1d	D1	1.83	0.58	16.7	1.06	5.64	6.0															Tributary basin D1 NW portion of Vollmer in Swale
	1.1d								16.7	2.06	5.64	11.6											Tributary basin D1 and OS1 NW portion of Vollmer in Swale
	2d	D2	1.77	0.61	16.3	1.07	5.71	6.1															Tributary basin D2 SE portion of Vollmer in Swale

Appendix C

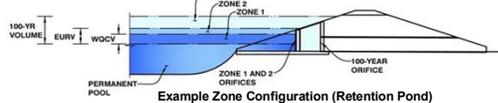
Hydraulic Calculations

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD- Detention, Version 4.03 (May 2020)

Project: **Homestead North at Sterling Ranch**

Basin ID: **Pond A**



Watershed Information

Selected BMP Type =	EDB
Watershed Area =	30.29 acres
Watershed Length =	1,963 ft
Watershed Length to Centroid =	1,178 ft
Watershed Slope =	0.030 ft/ft
Watershed Imperviousness =	46.40% percent
Percentage Hydrologic Soil Group A =	0.0% percent
Percentage Hydrologic Soil Group B =	100.0% percent
Percentage Hydrologic Soil Groups C/D =	0.0% percent
Target WQCV Drain Time =	40.0 hours
Location for 1-hr Rainfall Depths =	User Input

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Water Quality Capture Volume (WQCV) =	0.496 acre-feet
Excess Urban Runoff Volume (EURV) =	1.494 acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	1.435 acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	2.081 acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	2.652 acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	3.437 acre-feet
50-yr Runoff Volume (P1 = 2.25 in.) =	4.065 acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	4.868 acre-feet
500-yr Runoff Volume (P1 = 4 in.) =	8.766 acre-feet
Approximate 2-yr Detention Volume =	1.118 acre-feet
Approximate 5-yr Detention Volume =	1.542 acre-feet
Approximate 10-yr Detention Volume =	2.068 acre-feet
Approximate 25-yr Detention Volume =	2.278 acre-feet
Approximate 50-yr Detention Volume =	2.385 acre-feet
Approximate 100-yr Detention Volume =	2.685 acre-feet

Optional User Overrides	acre-feet
	acre-feet
	inches

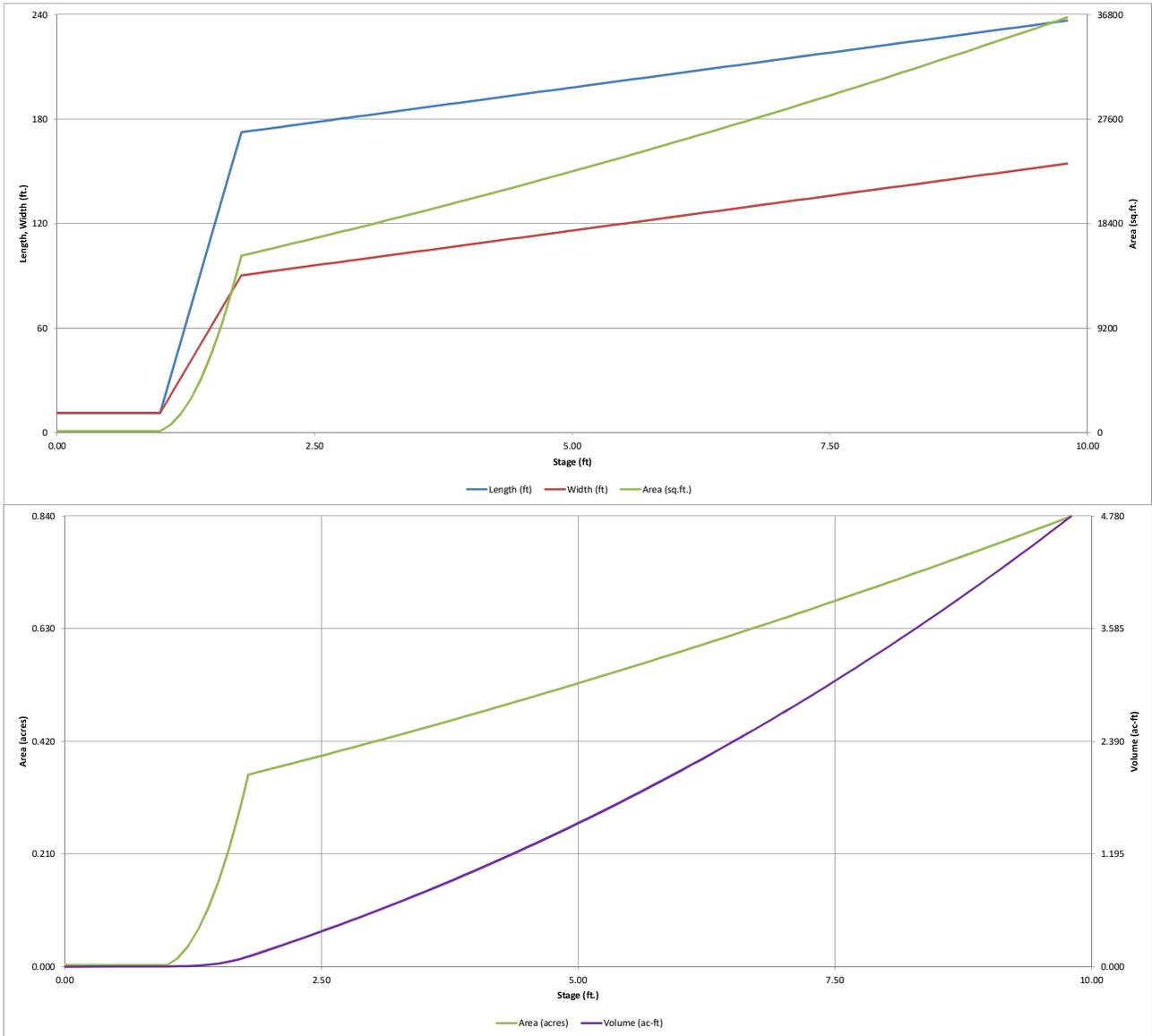
Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.496 acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.997 acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	1.192 acre-feet
Total Detention Basin Volume =	2.685 acre-feet
Initial Surcharge Volume (ISV) =	65 ft ³
Initial Surcharge Depth (ISD) =	0.50 ft
Total Available Detention Depth (H _{total}) =	7.00 ft
Depth of Trickle Channel (H _{TC}) =	0.50 ft
Slope of Trickle Channel (S _{TC}) =	0.005 ft/ft
Slopes of Main Basin Sides (S _{main}) =	4 H:V
Basin Length-to-Width Ratio (R _{LW}) =	2
Initial Surcharge Area (A _{ISV}) =	130 ft ²
Surcharge Volume Length (L _{ISV}) =	11.4 ft
Surcharge Volume Width (W _{ISV}) =	11.4 ft
Depth of Basin Floor (H _{FLOOR}) =	0.79 ft
Length of Basin Floor (L _{FLOOR}) =	172.5 ft
Width of Basin Floor (W _{FLOOR}) =	90.4 ft
Area of Basin Floor (A _{FLOOR}) =	15,597 ft ²
Volume of Basin Floor (V _{FLOOR}) =	4,516 ft ³
Depth of Main Basin (H _{MAIN}) =	5.21 ft
Length of Main Basin (L _{MAIN}) =	214.2 ft
Width of Main Basin (W _{MAIN}) =	132.1 ft
Area of Main Basin (A _{MAIN}) =	28,293 ft ²
Volume of Main Basin (V _{MAIN}) =	112,703 ft ³
Calculated Total Basin Volume (V _{total}) =	2.694 acre-feet

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft ²)	Optional Override Area (ft ²)	Area (acre)	Volume (ft ³)	Volume (ac-ft)
Top of Micropool	0.00		11.4	11.4	130		0.003		
ISV	0.50		11.4	11.4	130		0.003	65	0.001
	0.60		11.4	11.4	130		0.003	78	0.002
	0.70		11.4	11.4	130		0.003	91	0.002
	0.80		11.4	11.4	130		0.003	104	0.002
	0.90		11.4	11.4	130		0.003	117	0.003
	1.00		11.4	11.4	130		0.003	130	0.003
	1.10		31.8	21.4	680		0.016	167	0.004
	1.20		52.2	31.4	1,638		0.038	279	0.006
	1.30		72.6	41.4	3,004		0.069	508	0.012
	1.40		93.0	51.4	4,779		0.110	894	0.021
	1.50		113.4	61.4	6,961		0.160	1,477	0.034
	1.60		133.8	71.4	9,551		0.219	2,300	0.053
	1.70		154.2	81.4	12,549		0.288	3,401	0.078
Floor	1.79		172.5	90.4	15,597		0.358	4,665	0.107
	1.80		172.6	90.5	15,618		0.359	4,821	0.111
	1.90		173.4	91.3	15,829		0.363	6,394	0.147
	2.00		174.2	92.1	16,041		0.368	7,987	0.183
	2.10		175.0	92.9	16,255		0.373	9,602	0.220
	2.20		175.8	93.7	16,470		0.378	11,238	0.258
	2.30		176.6	94.5	16,686		0.383	12,896	0.296
	2.40		177.4	95.3	16,904		0.388	14,575	0.335
	2.50		178.2	96.1	17,122		0.393	16,277	0.374
	2.60		179.0	96.9	17,342		0.398	18,000	0.413
	2.70		179.8	97.7	17,564		0.403	19,745	0.453
	2.80		180.6	98.5	17,786		0.408	21,513	0.494
Zone 1 (WQCV)	2.81		180.7	98.5	17,809		0.409	21,691	0.498
	2.90		181.4	99.3	18,010		0.413	23,303	0.535
	3.00		182.2	100.1	18,236		0.419	25,115	0.577
	3.10		183.0	100.9	18,462		0.424	26,950	0.619
	3.20		183.8	101.7	18,690		0.429	28,807	0.661
	3.30		184.6	102.5	18,919		0.434	30,688	0.704
	3.40		185.4	103.3	19,149		0.440	32,591	0.748
	3.50		186.2	104.1	19,381		0.445	34,518	0.792
	3.60		187.0	104.9	19,614		0.450	36,467	0.837
	3.70		187.8	105.7	19,848		0.456	38,440	0.882
	3.80		188.6	106.5	20,083		0.461	40,437	0.928
	3.90		189.4	107.3	20,320		0.466	42,457	0.975
	4.00		190.2	108.1	20,558		0.472	44,501	1.022
	4.10		191.0	108.9	20,797		0.477	46,569	1.069
	4.20		191.8	109.7	21,038		0.483	48,660	1.117
	4.30		192.6	110.5	21,280		0.489	50,776	1.166
	4.40		193.4	111.3	21,523		0.494	52,916	1.215
	4.50		194.2	112.1	21,767		0.500	55,081	1.264
	4.60		195.0	112.9	22,013		0.505	57,270	1.315
	4.70		195.8	113.7	22,260		0.511	59,483	1.366
	4.80		196.6	114.5	22,508		0.517	61,722	1.417
	4.90		197.4	115.3	22,758		0.522	63,985	1.469
Zone 2 (EURV)	4.95		197.8	115.7	22,883		0.525	65,126	1.495
	5.00		198.2	116.1	23,008		0.528	66,273	1.521
	5.10		199.0	116.9	23,260		0.534	68,587	1.575
	5.20		199.8	117.7	23,514		0.540	70,926	1.628
	5.30		200.6	118.5	23,768		0.546	73,290	1.682
	5.40		201.4	119.3	24,024		0.552	75,679	1.737
	5.50		202.2	120.1	24,282		0.557	78,095	1.793
	5.60		203.0	120.9	24,540		0.563	80,536	1.849
	5.70		203.8	121.7	24,800		0.569	83,003	1.905
	5.80		204.6	122.5	25,061		0.575	85,496	1.963
	5.90		205.4	123.3	25,323		0.581	88,015	2.021
	6.00		206.2	124.1	25,587		0.587	90,560	2.079
	6.10		207.0	124.9	25,852		0.593	93,132	2.138
	6.20		207.8	125.7	26,118		0.600	95,731	2.198
	6.30		208.6	126.5	26,385		0.606	98,356	2.258
	6.40		209.4	127.3	26,654		0.612	101,008	2.319
	6.50		210.2	128.1	26,924		0.618	103,687	2.380
	6.60		211.0	128.9	27,195		0.624	106,393	2.442
	6.70		211.8	129.7	27,468		0.631	109,126	2.505
	6.80		212.6	130.5	27,742		0.637	111,886	2.569
	6.90		213.4	131.3	28,017		0.643	114,674	2.633
Zone 3 (100-year)	6.99		214.1	132.0	28,265		0.649	117,207	2.691
	7.00		214.2	132.1	28,293		0.650	117,490	2.697
	7.10		215.0	132.9	28,571		0.656	120,333	2.762
	7.20		215.8	133.7	28,850		0.662	123,204	2.828
	7.30		216.6	134.5	29,130		0.669	126,103	2.895
	7.40		217.4	135.3	29,412		0.675	129,030	2.962
	7.50		218.2	136.1	29,694		0.682	131,985	3.030
	7.60		219.0	136.9	29,978		0.688	134,969	3.098
	7.70		219.8	137.7	30,264		0.695	137,981	3.168
	7.80		220.6	138.5	30,550		0.701	141,021	3.237
	7.90		221.4	139.3	30,838		0.708	144,091	3.308
	8.00		222.2	140.1	31,128		0.715	147,189	3.379
	8.10		223.0	140.9	31,418		0.721	150,316	3.451
	8.20		223.8	141.7	31,710		0.728	153,473	3.523
	8.30		224.6	142.5	32,003		0.735	156,658	3.596
	8.40		225.4	143.3	32,297		0.741	159,873	3.670
	8.50		226.2	144.1	32,593		0.748	163,118	3.745
	8.60		227.0	144.9	32,890		0.755	166,392	3.820
	8.70		227.8	145.7	33,188		0.762	169,696	3.896
	8.80		228.6	146.5	33,487		0.769	173,030	3.972
	8.90		229.4	147.3	33,788		0.776	176,393	4.049
	9.00		230.2	148.1	34,090		0.783	179,787	4.127
	9.10		231.0	148.9	34,393		0.790	183,211	4.206
	9.20		231.8	149.7	34,698		0.797	186,666	4.285
	9.30		232.6	150.5	35,004		0.804	190,151	4.365
	9.40		23						

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.03 (May 2020)

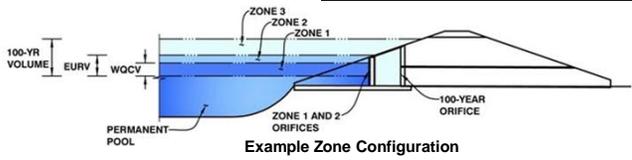


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.03 (May 2020)

Project: Homestead North at Sterling Ranch

Basin ID: Pond A



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
V)	2.81	0.496	
V)	4.95	0.997	
ir)	6.99	1.192	
Total (all zones)		2.685	

User Input: Orifice at Underdrain Outlet (typically used to drain WOCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
 Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = ft²
 Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WOCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice = 0.00 ft (relative to basin bottom at Stage = 0 ft)
 Depth at top of Zone using Orifice Plate = 4.27 ft (relative to basin bottom at Stage = 0 ft)
 Orifice Plate: Orifice Vertical Spacing = N/A inches
 Orifice Plate: Orifice Area per Row = N/A inches

Calculated Parameters for Plate

WO Orifice Area per Row = N/A ft²
 Elliptical Half-Width = N/A feet
 Elliptical Slot Centroid = N/A feet
 Elliptical Slot Area = N/A ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (optional)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.42	2.85	3.85				
Orifice Area (sq. inches)	2.00	2.00	2.00	2.00				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = Not Selected Not Selected ft (relative to basin bottom at Stage = 0 ft)
 Depth at top of Zone using Vertical Orifice = N/A N/A ft (relative to basin bottom at Stage = 0 ft)
 Vertical Orifice Diameter = N/A N/A inches

Calculated Parameters for Vertical Orifice

Vertical Orifice Area = Not Selected Not Selected ft²
 Vertical Orifice Centroid = N/A N/A feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

Overflow Weir Front Edge Height, Ho = Not Selected Not Selected ft (relative to basin bottom at Stage = 0 ft)
 Overflow Weir Front Edge Length = 5.00 N/A feet
 Overflow Weir Grate Slope = 4.00 N/A H:V
 Horiz. Length of Weir Sides = 5.00 N/A feet
 Overflow Grate Open Area % = 75% N/A % , grate open area/total area
 Debris Clogging % = 50% N/A %

Calculated Parameters for Overflow Weir

Height of Grate Upper Edge, H₁ = Not Selected Not Selected feet
 Overflow Weir Slope Length = 5.15 N/A feet
 Grate Open Area / 100-yr Orifice Area = 23.19 N/A
 Overflow Grate Open Area w/o Debris = 19.33 N/A ft²
 Overflow Grate Open Area w/ Debris = 9.66 N/A ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe = Not Selected Not Selected ft (distance below basin bottom at Stage = 0 ft)
 Circular Orifice Diameter = 24.00 N/A inches
 5.00

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

Outlet Orifice Area = Not Selected Not Selected ft²
 Outlet Orifice Centroid = 0.21 N/A feet
 Half-Central Angle of Restrictor Plate on Pipe = N/A N/A radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = 7.00 ft (relative to basin bottom at Stage = 0 ft)
 Spillway Crest Length = 23.00 feet
 Spillway End Slopes = 4.00 H:V
 Freeboard above Max Water Surface = 1.00 feet

Calculated Parameters for Spillway

Spillway Design Flow Depth = 0.91 feet
 Stage at Top of Freeboard = 8.91 feet
 Basin Area at Top of Freeboard = 0.78 acres
 Basin Volume at Top of Freeboard = 4.06 acre-ft

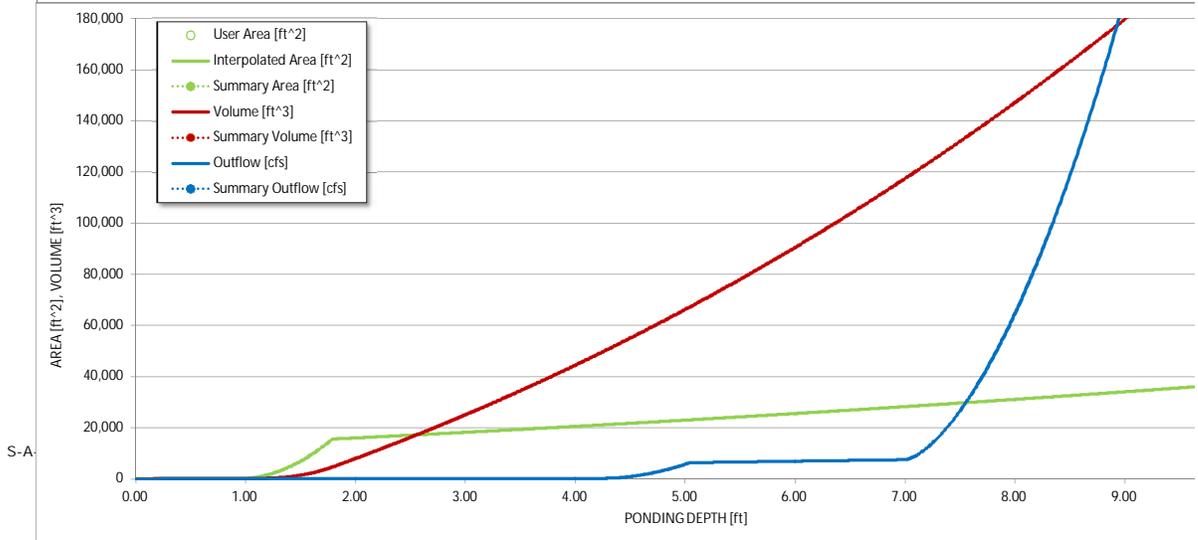
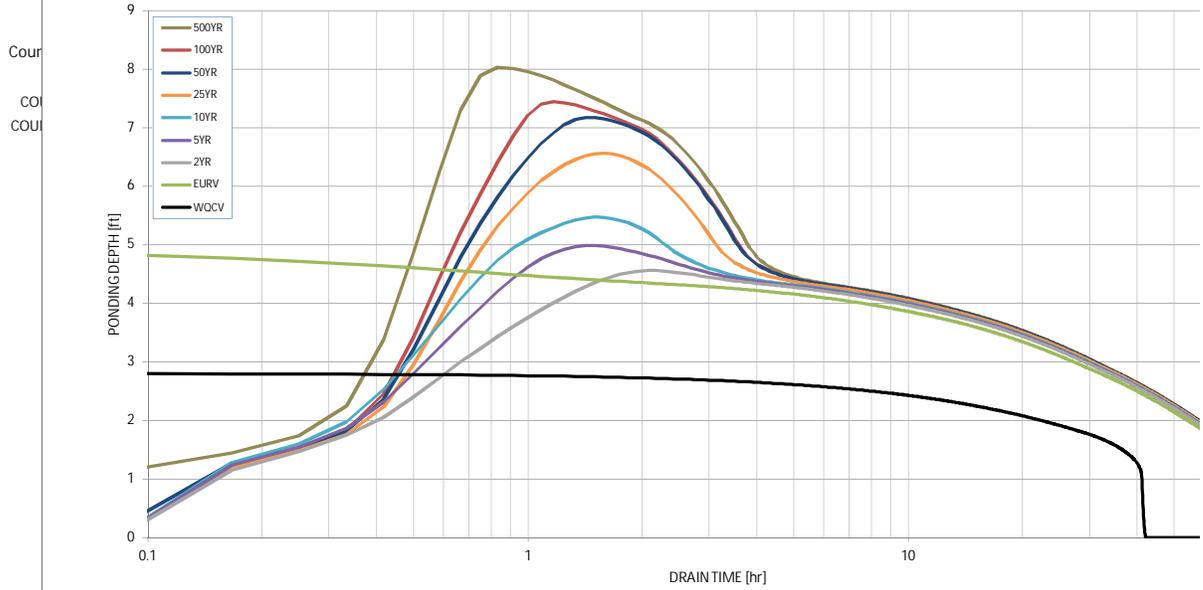
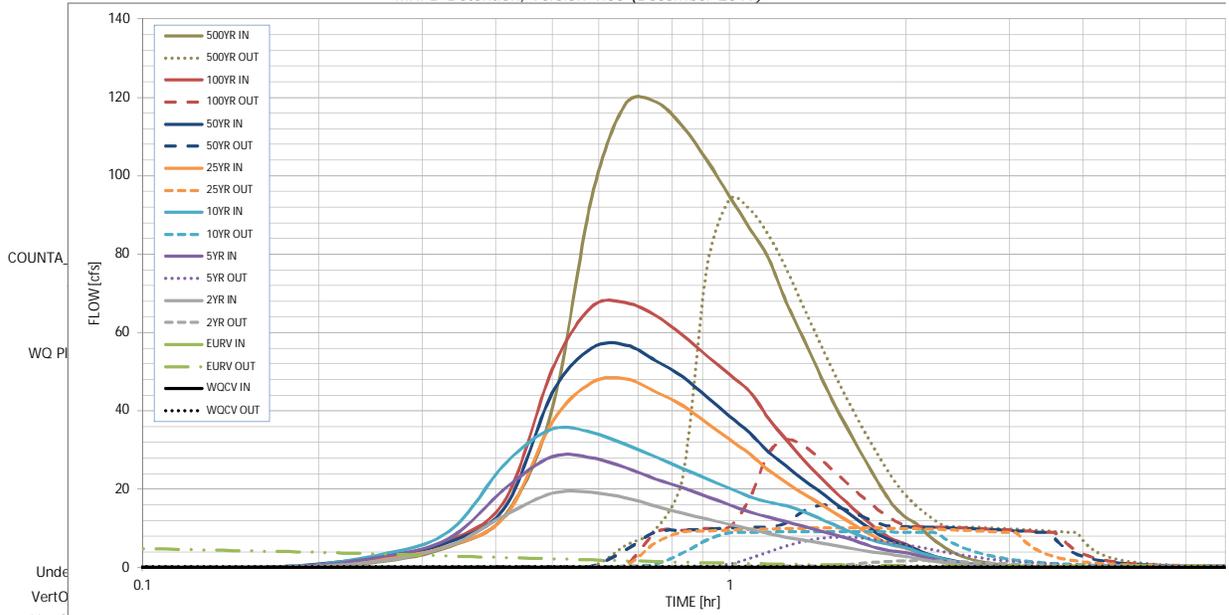
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WOCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =									
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	4.00
CUHP Runoff Volume (acre-ft) =	0.496	1.494	1.435	2.081	2.652	3.437	4.065	4.868	8.766
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	1.435	2.081	2.652	3.437	4.065	4.868	8.766
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	2.8	7.9	12.0	21.5	27.0	34.6	68.1
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.09	0.26	0.39	0.71	0.89	1.14	2.25
Peak Inflow Q (cfs) =	N/A	N/A	19.2	28.3	35.4	48.2	56.8	67.6	118.9
Peak Outflow Q (cfs) =	0.2	6.3	1.8	7.8	9.2	10.1	15.8	32.6	93.8
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	1.0	0.8	0.5	0.6	0.9	1.4
Structure Controlling Flow =	Plate	Overflow Weir	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Outlet Plate 1	Spillway	Spillway	Spillway
Max Velocity through Gate 1 (fps) =	N/A	0.34	0.07	0.4	0.5	0.5	0.5	0.5	0.5
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	39	67	69	68	66	65	63	61	52
Time to Drain 99% of Inflow Volume (hours) =	41	71	73	73	72	72	71	71	67
Maximum Ponding Depth (ft) =	2.81	4.95	4.56	4.99	5.47	6.56	7.18	7.45	8.03
Area at Maximum Ponding Depth (acres) =	0.41	0.53	0.50	0.53	0.56	0.62	0.66	0.68	0.72
Maximum Volume Stored (acre-ft) =	0.498	1.495	1.295	1.516	1.776	2.418	2.809	2.989	3.400

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.00 (December 2019)



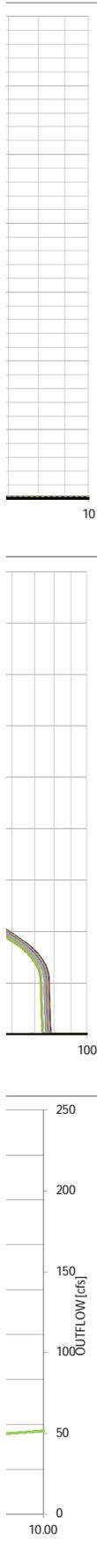
S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

Outflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.



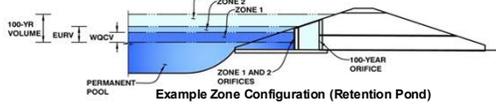
SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.17	0.02	1.30
0:15:00	0.00	0.00	1.51	2.47	3.06	2.06	2.59	2.52	5.32
0:20:00	0.00	0.00	5.56	7.41	9.24	5.53	6.48	6.91	13.71
0:25:00	0.00	0.00	13.50	20.37	26.75	13.30	15.79	17.62	40.65
0:30:00	0.00	0.00	18.98	28.34	35.37	37.22	44.63	50.66	94.40
0:35:00	0.00	0.00	19.16	27.99	34.48	47.21	56.00	66.54	118.45
0:40:00	0.00	0.00	17.76	25.47	31.41	48.19	56.81	67.57	118.88
0:45:00	0.00	0.00	15.65	22.58	28.22	44.83	52.80	64.29	112.59
0:50:00	0.00	0.00	13.84	20.32	25.25	41.35	48.66	59.30	103.94
0:55:00	0.00	0.00	12.37	18.13	22.65	36.90	43.53	54.01	94.88
1:00:00	0.00	0.00	11.03	16.05	20.26	32.73	38.70	49.29	86.64
1:05:00	0.00	0.00	9.85	14.18	18.12	29.02	34.37	44.93	78.97
1:10:00	0.00	0.00	8.66	12.86	16.71	24.83	29.47	38.10	67.80
1:15:00	0.00	0.00	7.76	11.74	15.77	21.75	25.90	32.63	58.95
1:20:00	0.00	0.00	7.03	10.61	14.40	18.99	22.62	27.78	50.30
1:25:00	0.00	0.00	6.37	9.54	12.70	16.62	19.76	23.58	42.60
1:30:00	0.00	0.00	5.75	8.54	11.08	14.27	16.92	19.97	35.96
1:35:00	0.00	0.00	5.13	7.58	9.56	12.10	14.30	16.66	29.91
1:40:00	0.00	0.00	4.53	6.41	8.18	10.10	11.88	13.62	24.38
1:45:00	0.00	0.00	4.00	5.35	6.98	8.25	9.65	10.86	19.36
1:50:00	0.00	0.00	3.62	4.58	6.16	6.65	7.74	8.51	15.34
1:55:00	0.00	0.00	3.18	4.14	5.60	5.59	6.50	6.96	12.79
2:00:00	0.00	0.00	2.83	3.80	5.06	4.95	5.76	6.00	11.16
2:05:00	0.00	0.00	2.31	3.11	4.14	3.95	4.58	4.68	8.75
2:10:00	0.00	0.00	1.85	2.47	3.29	3.05	3.54	3.53	6.63
2:15:00	0.00	0.00	1.47	1.95	2.60	2.37	2.74	2.65	4.99
2:20:00	0.00	0.00	1.16	1.54	2.03	1.83	2.11	1.97	3.71
2:25:00	0.00	0.00	0.91	1.21	1.58	1.41	1.62	1.47	2.77
2:30:00	0.00	0.00	0.71	0.93	1.20	1.08	1.24	1.12	2.10
2:35:00	0.00	0.00	0.56	0.71	0.91	0.82	0.93	0.85	1.58
2:40:00	0.00	0.00	0.43	0.53	0.69	0.62	0.70	0.65	1.21
2:45:00	0.00	0.00	0.33	0.41	0.53	0.48	0.54	0.51	0.94
2:50:00	0.00	0.00	0.24	0.30	0.40	0.36	0.41	0.39	0.71
2:55:00	0.00	0.00	0.17	0.21	0.28	0.26	0.30	0.28	0.51
3:00:00	0.00	0.00	0.11	0.14	0.19	0.18	0.20	0.19	0.34
3:05:00	0.00	0.00	0.07	0.09	0.11	0.11	0.13	0.12	0.21
3:10:00	0.00	0.00	0.03	0.05	0.06	0.06	0.07	0.06	0.11
3:15:00	0.00	0.00	0.01	0.02	0.02	0.03	0.03	0.02	0.04
3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD- Detention, Version 4.03 (May 2020)

Project: **Homestead North at Sterling Ranch**

Basin ID: **Pond B**



Watershed Information

Selected BMP Type =	EDB
Watershed Area =	27.87 acres
Watershed Length =	1,290 ft
Watershed Length to Centroid =	775 ft
Watershed Slope =	0.020 ft/ft
Watershed Imperviousness =	50.00% percent
Percentage Hydrologic Soil Group A =	0.0% percent
Percentage Hydrologic Soil Group B =	100.0% percent
Percentage Hydrologic Soil Groups C/D =	0.0% percent
Target WQCV Drain Time =	40.0 hours
Location for 1-hr Rainfall Depths =	User Input

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Water Quality Capture Volume (WQCV) =	0.479	acre-feet
Excess Urban Runoff Volume (EURV) =	1.490	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	1.408	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	2.013	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	2.544	inches
25-yr Runoff Volume (P1 = 2 in.) =	3.257	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	3.835	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	4.568	inches
500-yr Runoff Volume (P1 = 4 in.) =	8.154	inches
Approximate 2-yr Detention Volume =	1.124	acre-feet
Approximate 5-yr Detention Volume =	1.541	acre-feet
Approximate 10-yr Detention Volume =	2.041	acre-feet
Approximate 25-yr Detention Volume =	2.234	acre-feet
Approximate 50-yr Detention Volume =	2.335	acre-feet
Approximate 100-yr Detention Volume =	2.607	acre-feet

Optional User Overrides

	1.19	inches
	1.50	inches
	1.75	inches
	2.00	inches
	2.25	inches
	2.52	inches
	4.00	inches

Define Zones and Basin Geometry

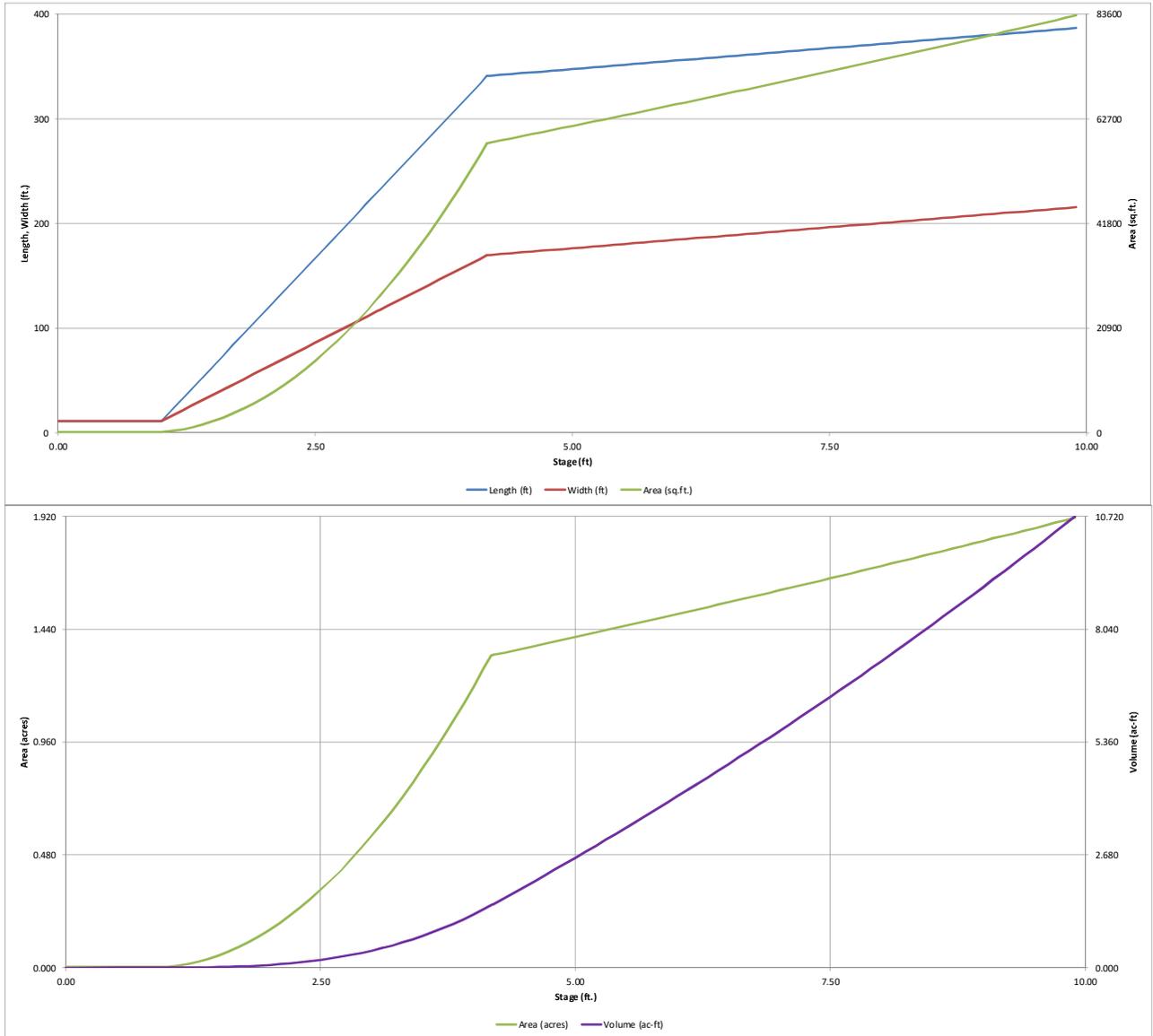
Zone 1 Volume (WQCV) =	0.479	acre-feet
Zone 2 Volume (EURV - Zone 1) =	1.011	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	1.117	acre-feet
Total Detention Basin Volume =	2.607	acre-feet
Initial Surcharge Volume (ISV) =	63	ft ³
Initial Surcharge Depth (ISD) =	0.50	ft
Total Available Detention Depth (H _{total}) =	5.00	ft
Depth of Trickle Channel (H _{TC}) =	0.50	ft
Slope of Trickle Channel (S _{TC}) =	0.010	ft/ft
Slopes of Main Basin Sides (S _{main}) =	4	H:V
Basin Length-to-Width Ratio (R _{LW}) =	2	
Initial Surcharge Area (A _{ISV}) =	125	ft ²
Surcharge Volume Length (L _{ISV}) =	11.2	ft
Surcharge Volume Width (W _{ISV}) =	11.2	ft
Depth of Basin Floor (H _{FLOOR}) =	3.17	ft
Length of Basin Floor (L _{FLOOR}) =	340.9	ft
Width of Basin Floor (W _{FLOOR}) =	169.7	ft
Area of Basin Floor (A _{FLOOR}) =	57,842	ft ²
Volume of Basin Floor (V _{FLOOR}) =	64,095	ft ³
Depth of Main Basin (H _{MAIN}) =	0.83	ft
Length of Main Basin (L _{MAIN}) =	347.5	ft
Width of Main Basin (W _{MAIN}) =	176.3	ft
Area of Main Basin (A _{MAIN}) =	61,276	ft ²
Volume of Main Basin (V _{MAIN}) =	49,427	ft ³
Calculated Total Basin Volume (V _{total}) =	2,609	acre-feet

Depth Increment = **0.10** ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft ²)	Optional Override Area (ft ²)	Area (acre)	Volume (ft ³)	Volume (ac-ft)
Top of Micropool	0.00		11.2	11.2	125		0.003		
ISV	0.50		11.2	11.2	125		0.003	63	0.001
	0.60		11.2	11.2	125		0.003	75	0.002
	0.70		11.2	11.2	125		0.003	88	0.002
	0.80		11.2	11.2	125		0.003	100	0.002
	0.90		11.2	11.2	125		0.003	113	0.003
	1.00		11.2	11.2	125		0.003	125	0.003
	1.10		21.6	16.2	350		0.008	148	0.003
	1.20		32.0	21.2	678		0.016	199	0.005
	1.30		42.4	26.2	1,110		0.025	287	0.007
	1.40		52.8	31.2	1,646		0.038	424	0.010
	1.50		63.2	36.2	2,287		0.052	620	0.014
	1.60		73.6	41.2	3,031		0.070	885	0.020
	1.70		84.0	46.2	3,879		0.089	1,230	0.028
	1.80		94.4	51.2	4,832		0.111	1,664	0.038
	1.90		104.8	56.2	5,888		0.135	2,199	0.050
	2.00		115.2	61.2	7,048		0.162	2,845	0.065
	2.10		125.6	66.2	8,313		0.191	3,612	0.083
	2.20		136.0	71.2	9,681		0.222	4,511	0.104
	2.30		146.4	76.2	11,153		0.256	5,552	0.127
	2.40		156.8	81.2	12,730		0.292	6,745	0.155
	2.50		167.2	86.2	14,410		0.331	8,101	0.186
	2.60		177.6	91.2	16,194		0.372	9,631	0.221
	2.70		188.0	96.2	18,082		0.415	11,344	0.260
	2.80		198.4	101.2	20,075		0.461	13,251	0.304
	2.90		208.8	106.2	22,171		0.509	15,362	0.353
	3.00		219.2	111.2	24,371		0.559	17,688	0.406
	3.10		229.6	116.2	26,676		0.612	20,240	0.465
Zone 1 (WQCV)	3.13		232.7	117.7	27,387		0.629	21,051	0.483
	3.20		240.0	121.2	29,084		0.668	23,027	0.529
	3.30		250.4	126.2	31,596		0.725	26,060	0.598
	3.40		260.8	131.2	34,213		0.785	29,350	0.674
	3.50		271.2	136.2	36,933		0.848	32,906	0.755
	3.60		281.6	141.2	39,757		0.913	36,740	0.843
	3.70		292.0	146.2	42,686		0.980	40,861	0.938
	3.80		302.4	151.2	45,718		1.050	45,280	1.039
	3.90		312.8	156.2	48,854		1.122	50,008	1.148
	4.00		323.2	161.2	52,095		1.196	55,055	1.264
	4.10		333.6	166.2	55,439		1.273	60,431	1.387
Floor	4.17		340.9	169.7	57,842		1.328	64,935	1.478
Zone 2 (EURV)	4.18		340.9	169.8	57,883		1.329	64,974	1.492
	4.20		341.1	169.9	57,964		1.331	66,132	1.518
	4.30		341.9	170.7	58,374		1.340	71,949	1.652
	4.40		342.7	171.5	58,785		1.350	77,807	1.786
	4.50		343.5	172.3	59,197		1.359	83,706	1.922
	4.60		344.3	173.1	59,610		1.368	89,646	2.058
	4.70		345.1	173.9	60,025		1.378	95,628	2.195
	4.80		345.9	174.7	60,440		1.388	101,651	2.334
	4.90		346.7	175.5	60,858		1.397	107,716	2.473
Zone 3 (100-year)	5.00		347.5	176.3	61,276		1.407	113,823	2.613
	5.10		348.3	177.1	61,696		1.416	119,971	2.754
	5.20		349.1	177.9	62,117		1.426	126,162	2.896
	5.30		349.9	178.7	62,539		1.436	132,395	3.039
	5.40		350.7	179.5	62,962		1.445	138,670	3.183
	5.50		351.5	180.3	63,387		1.455	144,987	3.328
	5.60		352.3	181.1	63,813		1.465	151,347	3.474
	5.70		353.1	181.9	64,241		1.475	157,750	3.621
	5.80		353.9	182.7	64,669		1.485	164,196	3.769
	5.90		354.7	183.5	65,099		1.494	170,684	3.918
	6.00		355.5	184.3	65,531		1.504	177,216	4.068
	6.10		356.3	185.1	65,963		1.514	183,790	4.219
	6.20		357.1	185.9	66,397		1.524	190,408	4.371
	6.30		357.9	186.7	66,832		1.534	197,070	4.524
	6.40		358.7	187.5	67,268		1.544	203,775	4.678
	6.50		359.5	188.3	67,706		1.554	210,523	4.833
	6.60		360.3	189.1	68,145		1.564	217,316	4.989
	6.70		361.1	189.9	68,585		1.574	224,152	5.146
	6.80		361.9	190.7	69,027		1.585	231,033	5.304
	6.90		362.7	191.5	69,469		1.595	237,958	5.463
	7.00		363.5	192.3	69,913		1.605	244,927	5.623
	7.10		364.3	193.1	70,359		1.615	251,940	5.784
	7.20		365.1	193.9	70,805		1.625	258,999	5.946
	7.30		365.9	194.7	71,253		1.636	266,102	6.109
	7.40		366.7	195.5	71,702		1.646	273,249	6.273
	7.50		367.5	196.3	72,153		1.656	280,442	6.438
	7.60		368.3	197.1	72,604		1.667	287,680	6.604
	7.70		369.1	197.9	73,057		1.677	294,963	6.771
	7.80		369.9	198.7	73,512		1.688	302,291	6.940
	7.90		370.7	199.5	73,967		1.698	309,665	7.109
	8.00		371.5	200.3	74,424		1.709	317,085	7.279
	8.10		372.3	201.1	74,882		1.719	324,550	7.451
	8.20		373.1	201.9	75,342		1.730	332,061	7.623
	8.30		373.9	202.7	75,802		1.740	339,619	7.797
	8.40		374.7	203.5	76,264		1.751	347,222	7.971
	8.50		375.5	204.3	76,727		1.761	354,871	8.147
	8.60		376.3	205.1	77,192		1.772	362,567	8.323
	8.70		377.1	205.9	77,658		1.783	370,310	8.501
	8.80		377.9	206.7	78,125		1.793	378,099	8.680
	8.90		378.7	207.5	78,593		1.804	385,935	8.860
	9.00		379.5	208.3	79,063		1.815	393,818	9.041
	9.10		380.3	209.1	79,534		1.826	401,748	

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.03 (May 2020)

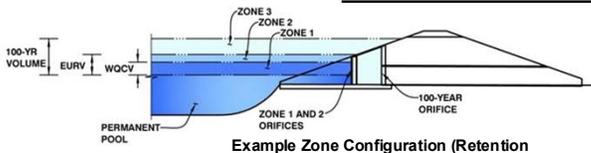


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.03 (May 2020)

Project: Homestead North at Sterling Ranch

Basin ID: Pond B



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
1 (WQCV)	3.13	0.479	Orifice Plate
2 (EURV)	4.18	1.011	Orifice Plate
(100-year)	5.00	1.117	Weir&Pipe (Restrict)
Total (all zones)		2.607	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft ²
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	4.11	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft ²
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft ²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.37	2.74	3.20				
Orifice Area (sq. inches)	1.50	1.50	1.50	9.00				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =		N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =		N/A	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =		N/A	ft ²
Vertical Orifice Centroid =		N/A	feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe))

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, H _o =	4.11	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	5.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	5.00	N/A	feet
Overflow Grate Open Area % =	70%	N/A	%, grate open area/total area
Debris Clogging % =	70%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H _t =	4.11	N/A	feet
Overflow Weir Slope Length =	5.00	N/A	feet
Grate Open Area / 100-yr Orifice Area =	5.76	N/A	
Overflow Grate Open Area w/o Debris =	17.50	N/A	ft ²
Overflow Grate Open Area w/ Debris =	5.25	N/A	ft ²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.25	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	33.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	16.80		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	3.04	N/A	ft ²
Outlet Orifice Centroid =	0.80	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	1.59	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	5.16	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	23.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

Spillway Design Flow Depth =	0.91	feet
Stage at Top of Freeboard =	7.07	feet
Basin Area at Top of Freeboard =	1.61	acres
Basin Volume at Top of Freeboard =	5.74	acre-ft

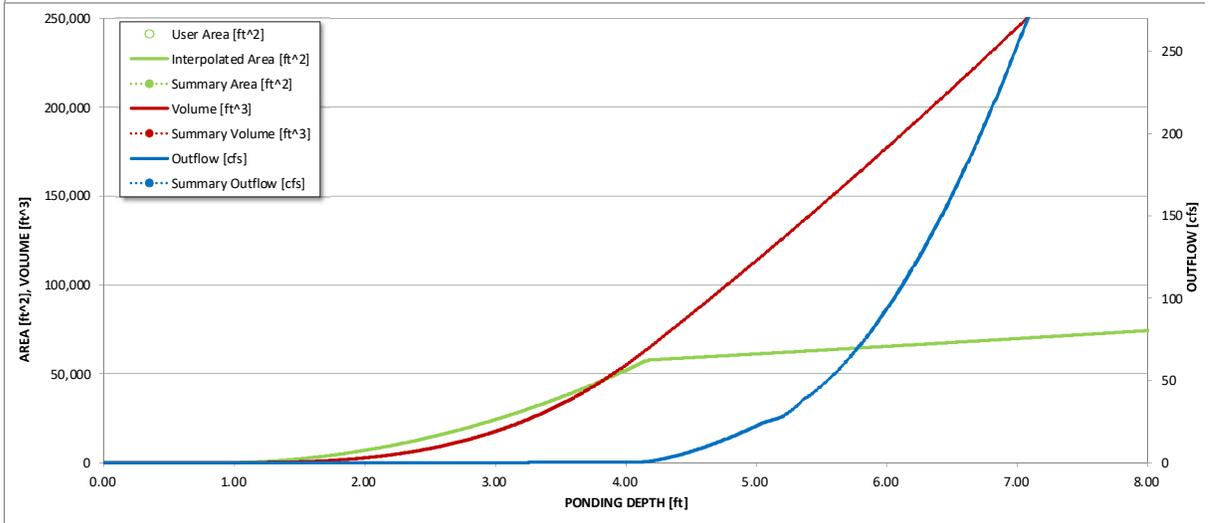
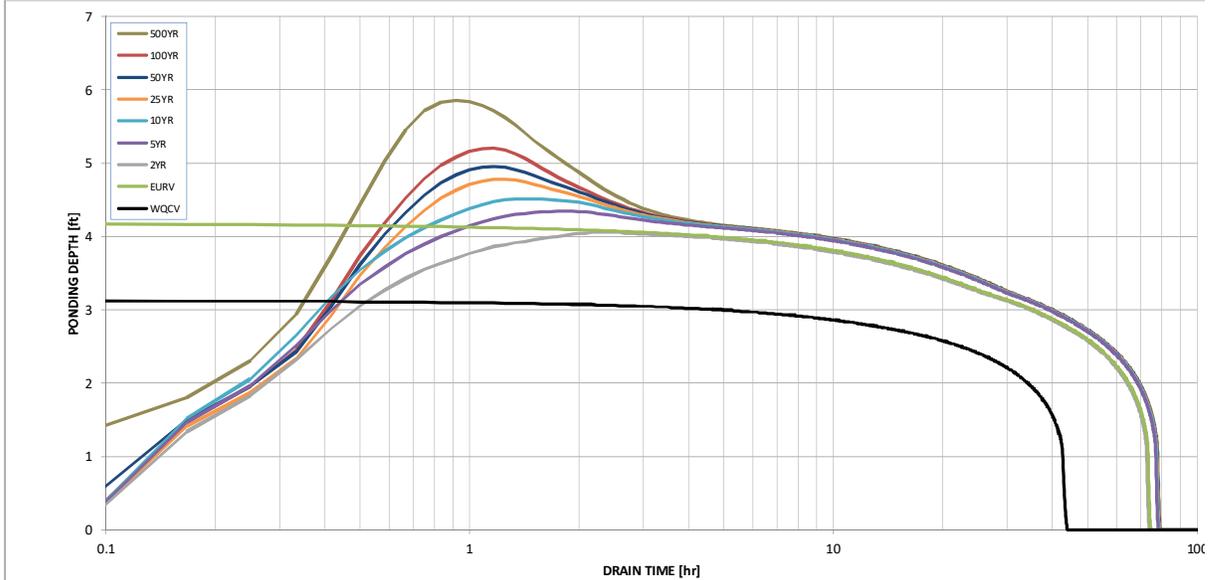
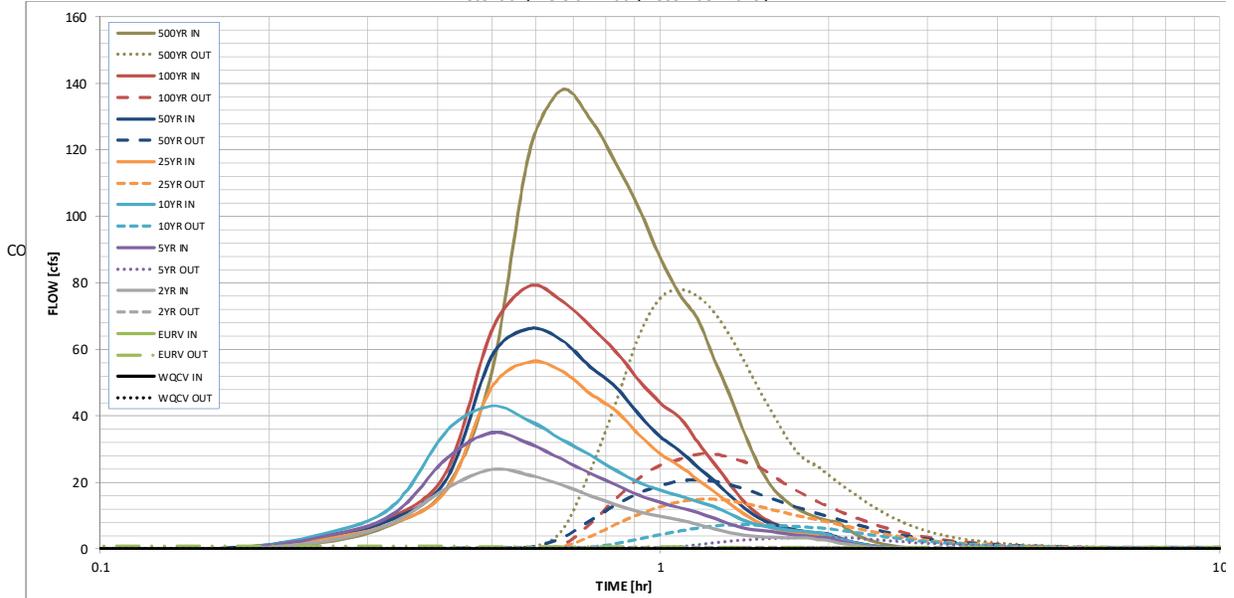
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	4.00
One-Hour Rainfall Depth (in) =	0.479	1.490	1.408	2.013	2.544	3.257	3.835	4.568	8.154
CUHP Runoff Volume (acre-ft) =	N/A	N/A	1.408	2.013	2.544	3.257	3.835	4.568	8.154
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	3.2	9.1	13.8	24.2	30.4	38.6	75.6
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A							
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.12	0.33	0.49	0.87	1.09	1.39	2.71
Peak Inflow Q (cfs) =	N/A	N/A	23.9	34.9	42.9	56.2	66.3	79.0	138.0
Peak Outflow Q (cfs) =	0.2	1.0	0.5	3.5	7.3	14.9	20.7	28.5	78.0
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.4	0.5	0.6	0.7	0.7	1.0
Structure Controlling Flow =	Plate	Overflow Weir 1	Plate	overflow Weir	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Spillway	Spillway
Max Velocity through Grate 1 (fps) =	N/A	0.03	N/A	0.2	0.4	0.8	1.1	1.5	1.9
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	41	67	67	69	68	66	64	62	53
Time to Drain 99% of Inflow Volume (hours) =	42	71	71	74	74	73	72	72	68
Maximum Ponding Depth (ft) =	3.13	4.18	4.05	4.35	4.51	4.78	4.95	5.20	5.86
Area at Maximum Ponding Depth (acres) =	0.63	1.33	1.23	1.34	1.36	1.39	1.40	1.43	1.49
Maximum Volume Stored (acre-ft) =	0.483	1.492	1.325	1.705	1.935	2.306	2.543	2.896	3.844

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.00 (December 2019)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

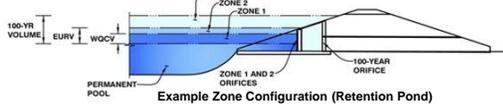
Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]	
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.26	0.03	1.94
	0:15:00	0.00	0.00	2.25	3.68	4.56	3.07	3.82	3.74	7.60
	0:20:00	0.00	0.00	7.96	10.47	12.93	7.76	9.03	9.69	18.74
	0:25:00	0.00	0.00	18.42	27.42	35.63	18.08	21.37	23.78	53.17
	0:30:00	0.00	0.00	23.89	34.92	42.92	48.56	57.93	65.61	119.03
	0:35:00	0.00	0.00	22.18	31.67	38.54	56.20	66.25	79.03	138.00
	0:40:00	0.00	0.00	19.33	26.99	32.94	53.56	62.78	74.65	129.30
	0:45:00	0.00	0.00	16.02	22.73	28.27	46.71	54.73	67.10	116.09
	0:50:00	0.00	0.00	13.30	19.31	23.64	41.49	48.62	59.31	102.41
	0:55:00	0.00	0.00	11.20	16.14	20.01	34.39	40.36	50.66	87.69
	1:00:00	0.00	0.00	9.79	14.01	17.72	28.70	33.80	43.90	76.55
	1:05:00	0.00	0.00	8.75	12.42	15.96	24.95	29.48	39.53	69.13
	1:10:00	0.00	0.00	7.35	10.97	14.29	20.83	24.68	32.16	56.89
	1:15:00	0.00	0.00	6.06	9.22	12.69	17.21	20.45	25.65	46.03
	1:20:00	0.00	0.00	4.95	7.48	10.52	13.53	16.04	19.30	34.53
	1:25:00	0.00	0.00	4.15	6.23	8.46	10.42	12.31	14.00	25.12
	1:30:00	0.00	0.00	3.71	5.58	7.21	7.93	9.35	10.26	18.74
	1:35:00	0.00	0.00	3.50	5.23	6.44	6.43	7.55	8.05	14.86
	1:40:00	0.00	0.00	3.40	4.64	5.89	5.49	6.42	6.66	12.34
	1:45:00	0.00	0.00	3.33	4.18	5.51	4.88	5.67	5.69	10.58
	1:50:00	0.00	0.00	3.27	3.85	5.24	4.47	5.16	5.03	9.37
	1:55:00	0.00	0.00	2.86	3.60	4.89	4.20	4.82	4.56	8.50
	2:00:00	0.00	0.00	2.51	3.32	4.37	4.01	4.59	4.26	7.93
	2:05:00	0.00	0.00	1.89	2.48	3.24	3.00	3.42	3.16	5.86
	2:10:00	0.00	0.00	1.38	1.80	2.33	2.16	2.46	2.28	4.19
	2:15:00	0.00	0.00	1.00	1.30	1.67	1.56	1.77	1.65	3.04
	2:20:00	0.00	0.00	0.72	0.93	1.20	1.12	1.27	1.20	2.19
	2:25:00	0.00	0.00	0.51	0.64	0.84	0.79	0.89	0.84	1.54
	2:30:00	0.00	0.00	0.35	0.43	0.58	0.55	0.62	0.59	1.07
	2:35:00	0.00	0.00	0.23	0.30	0.39	0.38	0.43	0.40	0.73
	2:40:00	0.00	0.00	0.14	0.19	0.24	0.24	0.27	0.26	0.46
	2:45:00	0.00	0.00	0.07	0.10	0.13	0.14	0.15	0.14	0.25
	2:50:00	0.00	0.00	0.03	0.05	0.05	0.06	0.07	0.06	0.11
	2:55:00	0.00	0.00	0.01	0.01	0.01	0.01	0.02	0.01	0.02
	3:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)

Project: Pond C with offsite flow

Basin ID: _____



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	EDB
Watershed Area =	224.34 acres
Watershed Length =	5,645 ft
Watershed Length to Centroid =	3,387 ft
Watershed Slope =	0.034 ft/ft
Watershed Imperviousness =	10.30% percent
Percentage Hydrologic Soil Group A =	0.0% percent
Percentage Hydrologic Soil Group B =	100.0% percent
Percentage Hydrologic Soil Groups C/D =	0.0% percent
Target WOCV Drain Time =	40.0 hours
Location for 1-hr Rainfall Depths =	User Input

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Water Quality Capture Volume (WOCV) =	1.285 acre-feet
Excess Urban Runoff Volume (EURV) =	2.177 acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	3.053 acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	6.690 acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	10.314 acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	16.752 acre-feet
50-yr Runoff Volume (P1 = 2.25 in.) =	21.154 acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	27.479 acre-feet
500-yr Runoff Volume (P1 = 4 in.) =	55.481 acre-feet
Approximate 2-yr Detention Volume =	1.394 acre-feet
Approximate 5-yr Detention Volume =	2.181 acre-feet
Approximate 10-yr Detention Volume =	4.470 acre-feet
Approximate 25-yr Detention Volume =	6.213 acre-feet
Approximate 50-yr Detention Volume =	6.504 acre-feet
Approximate 100-yr Detention Volume =	8.393 acre-feet

Optional User Overrides

acre-feet	acre-feet
inches	inches
1.19 inches	1.19 inches
1.50 inches	1.50 inches
1.75 inches	1.75 inches
2.00 inches	2.00 inches
2.25 inches	2.25 inches
2.52 inches	2.52 inches
4.00 inches	4.00 inches

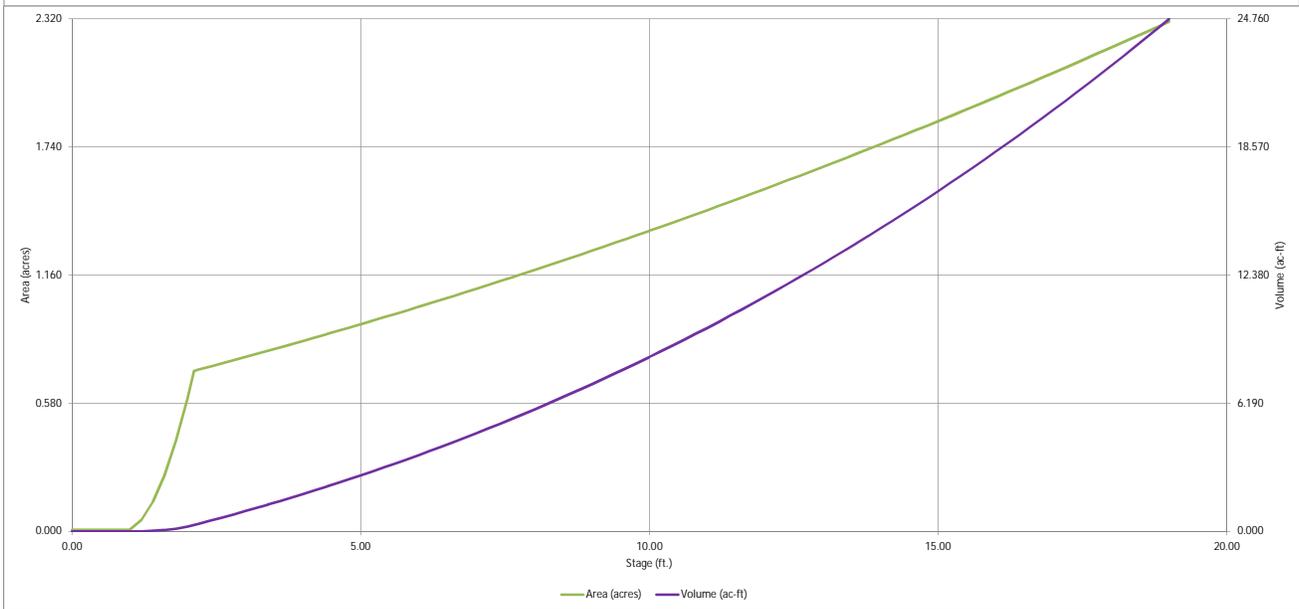
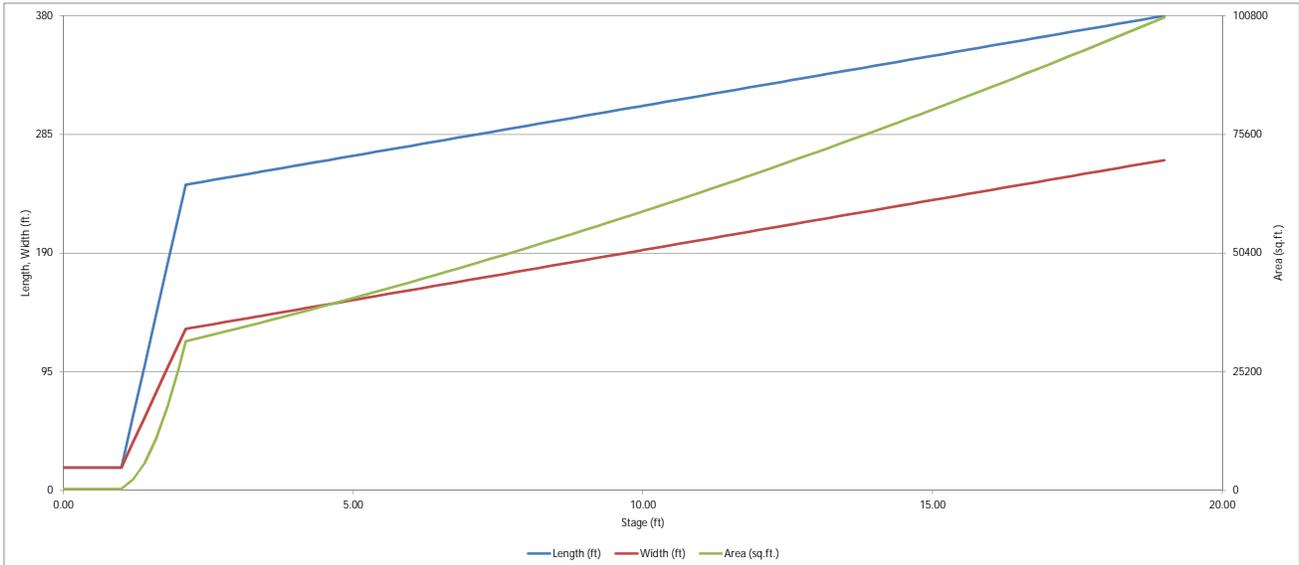
Define Zones and Basin Geometry

Zone 1 Volume (WOCV) =	1.285 acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.892 acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	6.216 acre-feet
Total Detention Basin Volume =	8.393 acre-feet
Initial Surcharge Volume (ISV) =	168 ft ³
Initial Surcharge Depth (ISD) =	0.50 ft
Total Available Detention Depth (H _{total}) =	9.99 ft
Depth of Trickle Channel (H _{TC}) =	0.50 ft
Slope of Trickle Channel (S _{TC}) =	0.005 ft/ft
Slopes of Main Basin Sides (S _{main}) =	4 H:V
Basin Length-to-Width Ratio (R _{L/W}) =	2
Initial Surcharge Area (A _{ISV}) =	336 ft ²
Surcharge Volume Length (L _{ISV}) =	18.3 ft
Surcharge Volume Width (W _{ISV}) =	18.3 ft
Depth of Basin Floor (H _{FLOOR}) =	1.11 ft
Length of Basin Floor (L _{FLOOR}) =	244.8 ft
Width of Basin Floor (W _{FLOOR}) =	129.3 ft
Area of Basin Floor (A _{FLOOR}) =	31,653 ft ²
Volume of Basin Floor (V _{FLOOR}) =	13,042 ft ³
Depth of Main Basin (H _{MAIN}) =	7.88 ft
Length of Main Basin (L _{MAIN}) =	307.8 ft
Width of Main Basin (W _{MAIN}) =	192.4 ft
Area of Main Basin (A _{MAIN}) =	59,210 ft ²
Volume of Main Basin (V _{MAIN}) =	352,381 ft ³
Calculated Total Basin Volume (V _{total}) =	8.397 acre-feet

Depth Increment = 0.20 ft		Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft ²)	Optional Override Area (ft ²)	Area (acre)	Volume (ft ³)	Volume (ac-ft)
Top of Micropool	0.00		18.3	18.3	336		0.008		
ISV	0.50		18.3	18.3	336		0.008	168	0.004
	0.60		18.3	18.3	336		0.008	201	0.005
	0.80		18.3	18.3	336		0.008	269	0.006
	1.00		18.3	18.3	336		0.008	336	0.008
	1.20		59.1	38.3	2,266		0.052	569	0.013
	1.40		99.9	58.3	5,828		0.134	1,351	0.031
	1.60		140.7	78.3	11,022		0.253	3,009	0.069
	1.80		181.5	98.3	17,848		0.410	5,868	0.135
	2.00		222.3	118.3	26,306		0.604	10,257	0.236
	Floor	2.11		244.8	129.3	31,653		0.727	13,440
2.20		245.5	130.0	31,923		0.733	16,301	0.374	
2.40		247.1	131.6	32,527		0.747	22,746	0.522	
2.60		248.7	133.2	33,135		0.761	29,312	0.673	
2.80		250.3	134.8	33,749		0.775	36,000	0.826	
3.00		251.9	136.4	34,368		0.789	42,812	0.983	
3.20		253.5	138.0	34,991		0.803	49,747	1.142	
Zone 1 (WOCV)	3.38		254.9	139.5	35,557		0.816	56,997	1.288
3.40		255.1	139.6	35,620		0.818	56,808	1.304	
3.60		256.7	141.2	36,255		0.832	63,996	1.469	
3.80		258.3	142.8	36,894		0.847	71,311	1.637	
4.00		259.9	144.4	37,538		0.862	78,754	1.808	
4.20		261.5	146.0	38,188		0.877	86,324	1.982	
4.40		263.1	147.6	38,842		0.892	94,029	2.159	
Zone 2 (EURV)	4.43		263.3	147.9	38,941		0.894	95,196	2.185
4.60		264.7	149.2	39,502		0.907	101,864	2.338	
4.80		266.3	150.8	40,167		0.922	109,830	2.521	
5.00		267.9	152.4	40,837		0.937	117,931	2.707	
5.20		269.5	154.0	41,512		0.953	126,165	2.896	
5.40		271.1	155.6	42,192		0.969	134,536	3.089	
5.60		272.7	157.2	42,877		0.984	143,043	3.284	
5.80		274.3	158.8	43,568		1.000	151,687	3.482	
6.00		275.9	160.4	44,263		1.016	160,470	3.684	
6.20		277.5	162.0	44,964		1.032	169,393	3.889	
6.40		279.1	163.6	45,670		1.048	178,456	4.097	
6.60		280.7	165.2	46,381		1.065	187,661	4.308	
6.80		282.3	166.8	47,097		1.081	197,009	4.523	
7.00		283.9	168.4	47,818		1.098	206,500	4.741	
7.20		285.5	170.0	48,544		1.114	216,136	4.962	
7.40		287.1	171.6	49,276		1.131	225,918	5.186	
7.60		288.7	173.2	50,012		1.148	235,847	5.414	
7.80		290.3	174.8	50,754		1.165	245,923	5.646	
8.00		291.9	176.4	51,501		1.182	256,149	5.880	
8.20		293.5	178.0	52,252		1.200	266,524	6.119	
8.40		295.1	179.6	53,009		1.217	277,050	6.360	
8.60		296.7	181.2	53,772		1.234	287,728	6.605	
8.80		298.3	182.8	54,539		1.252	298,559	6.854	
9.00		299.9	184.4	55,311		1.270	309,544	7.106	
9.20		301.5	186.0	56,089		1.288	320,684	7.362	
9.40		303.1	187.6	56,871		1.306	331,980	7.621	
9.60		304.7	189.2	57,659		1.324	343,433	7.884	
9.80		306.3	190.8	58,452		1.342	355,044	8.151	
Zone 3 (100-year)	9.98		307.7	192.3	59,170		1.358	365,630	8.394
10.00		307.9	192.4	59,250		1.360	366,814	8.421	
10.20		309.5	194.0	60,053		1.379	378,744	8.695	
10.40		311.1	195.6	60,861		1.397	390,835	8.972	
10.60		312.7	197.2	61,674		1.416	403,089	9.254	
10.80		314.3	198.8	62,493		1.435	415,505	9.539	
11.00		315.9	200.4	63,316		1.454	428,086	9.828	
11.20		317.5	202.0	64,145		1.473	440,933	10.120	
11.40		319.1	203.6	64,979		1.492	453,945	10.417	
11.60		320.7	205.2	65,818		1.511	466,824	10.717	
11.80		322.3	206.8	66,662		1.530	480,072	11.021	
12.00		323.9	208.4	67,511		1.550	493,489	11.329	
12.20		325.5	210.0	68,365		1.569	507,077	11.641	
12.40		327.1	211.6	69,225		1.589	520,836	11.957	
12.60		328.7	213.2	70,089		1.609	534,767	12.277	
12.80		330.3	214.8	70,959		1.629	548,872	12.600	
13.00		331.9	216.4	71,834		1.649	563,151	12.928	
13.20		333.5	218.0	72,714		1.669	577,605	13.260	
13.40		335.1	219.6	73,599		1.690	592,237	13.596	
13.60		336.7	221.2	74,489		1.710	607,045	13.936	
13.80		338.3	222.8	75,384		1.731	622,032	14.280	
14.00		339.9	224.4	76,284		1.751	637,199	14.628	
14.20		341.5	226.0	77,190		1.772	652,547	14.980	
14.40		343.1	227.6	78,100		1.793	668,075	15.337	
14.60		344.7	229.2	79,016		1.814	683,787	15.698	
14.80		346.3	230.8	79,937		1.835	699,682	16.062	
15.00		347.9	232.4	80,863		1.856	715,762	16.432	
15.20		349.5	234.0	81,794		1.878	732,028	16.805	
15.40		351.1	235.6	82,730		1.899	748,480	17.183	
15.60		352.7	237.2	83,671		1.921	765,120	17.565	
15.80		354.3	238.8	84,618		1.943	781,949	17.951	
16.00		355.9	240.4	85,569		1.964	798,968	18.342	
16.20		357.5	242.0	86,526		1.986	816,177	18.737	
16.40		359.1	243.6	87,488		2.008	833,578	19.136	
16.60		360.7	245.2	88,455		2.031	851,173	19.540	
16.80		362.3	246.8	89,427		2.053	868,961	19.949	
17.00		363.9	248.4	90,404		2.075	886,944	20.361	
17.20		365.5	250.0	91,386		2.098	905,123	20.779	
17.40		367.1	251.6	92,373		2.121	923,499	21.201	
17.60		368.7	253.2	93,366		2.143	942,072	21.627	
17.80		370.3	254.8	94,364		2.166	960,845	22.058	
18.00		371.9	256.4	95,367		2.189	979,818	22.494	
18.20		373.5	258.0	96,375		2.212	998,992	22.934	
18.40		375.1	259.6	97,388		2.236	1,018,368	23.379	
18.60		376.7	261.2	98,406		2.259	1,037,948	23.828	
18.80		378.3	262.8	99,429		2.283	1,057,731	24.282	
19.00		379.9	264.4	100,457		2.306	1,077,720	24.741	

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.04 (February 2021)

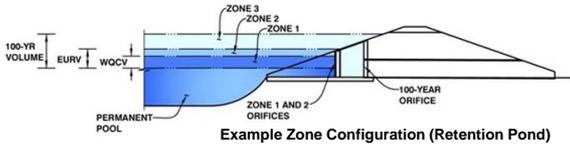


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.04 (February 2021)

Project: **Pond C with offsite flow**

Basin ID: _____



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WOCV)	3.38	1.285	Orifice Plate
Zone 2 (EURV)	4.43	0.892	Orifice Plate
Zone 3 (100-year)	9.98	6.216	Weir&Pipe (Restrict)
Total (all zones)		8.393	

User Input: Orifice at Underdrain Outlet (typically used to drain WOCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain	
Underdrain Orifice Area =	N/A ft ²
Underdrain Orifice Centroid =	N/A feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WOCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	4.43	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	17.72	inches
Orifice Plate: Orifice Area per Row =	4.62	sq. inches (use rectangular openings)

Calculated Parameters for Plate	
WQ Orifice Area per Row =	3.208E-02 ft ²
Elliptical Half-Width =	N/A feet
Elliptical Slot Centroid =	N/A feet
Elliptical Slot Area =	N/A ft ²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.50	3.00					
Orifice Area (sq. inches)	4.62	4.62	4.62					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orifice	
Vertical Orifice Area =	N/A ft ²
Vertical Orifice Centroid =	N/A feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir (and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	4.39	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	15.00	N/A	feet
Overflow Weir Gate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	6.00	N/A	feet
Overflow Gate Type =	Close Mesh Gate	N/A	
Debris Clogging % =	75%	N/A	%

Calculated Parameters for Overflow Weir	
Height of Gate Upper Edge, H ₁ =	4.39 feet
Overflow Weir Slope Length =	6.00 feet
Gate Open Area / 100-yr Orifice Area =	7.25 N/A
Overflow Gate Open Area w/o Debris =	71.19 ft ²
Overflow Gate Open Area w/ Debris =	17.80 ft ²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	1.50	0.00	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	48.00		inches
Restrictor Plate Height Above Pipe Invert =	35.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate	
Outlet Orifice Area =	9.82 ft ²
Outlet Orifice Centroid =	1.62 feet
Half-Central Angle of Restrictor Plate on Pipe =	2.05 radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	10.00	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	81.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway	
Spillway Design Flow Depth =	0.97 feet
Stage at Top of Freeboard =	11.97 feet
Basin Area at Top of Freeboard =	1.55 acres
Basin Volume at Top of Freeboard =	11.28 acre-ft

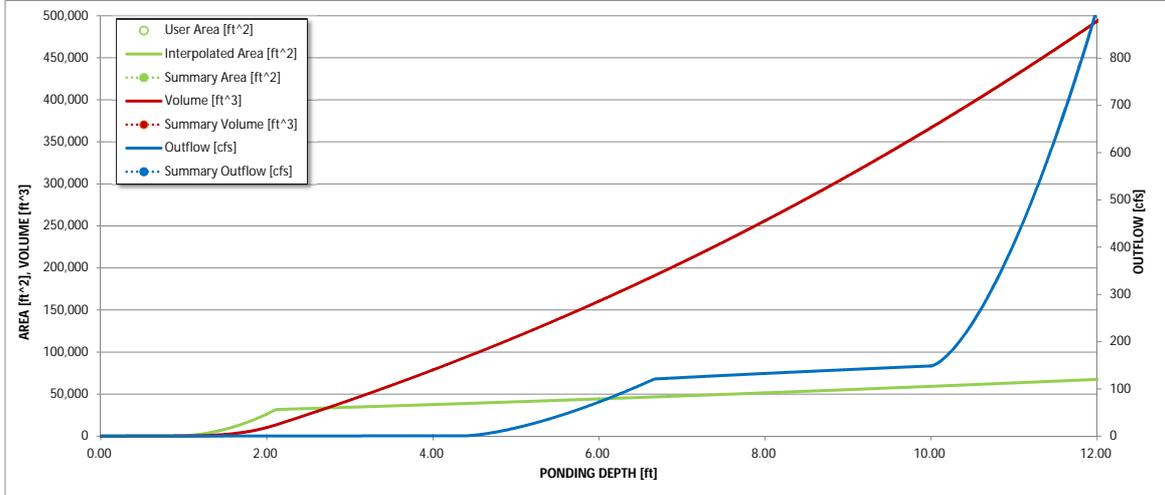
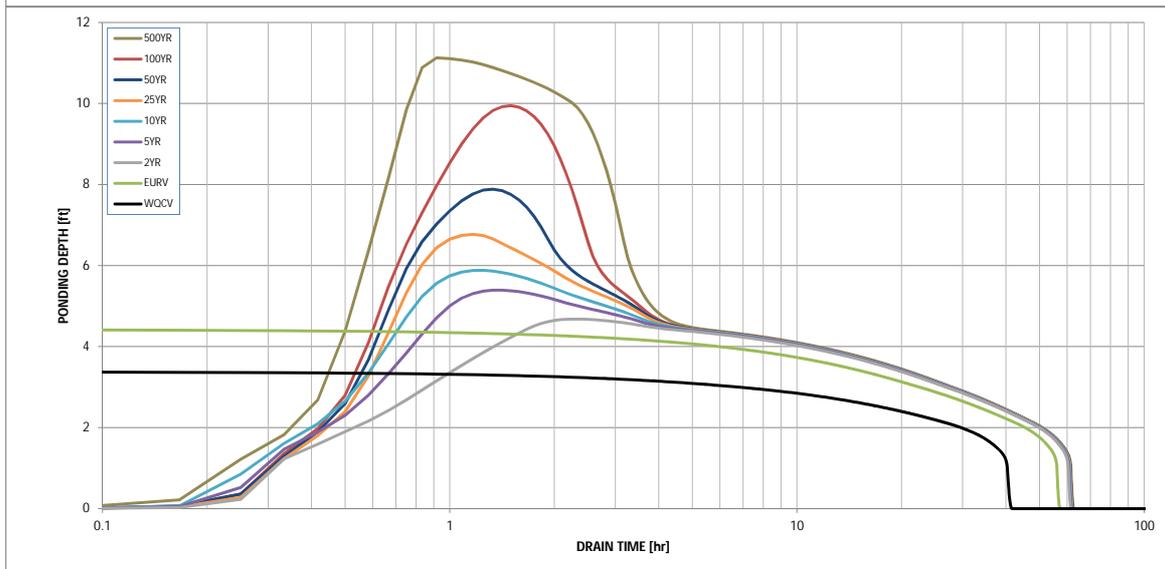
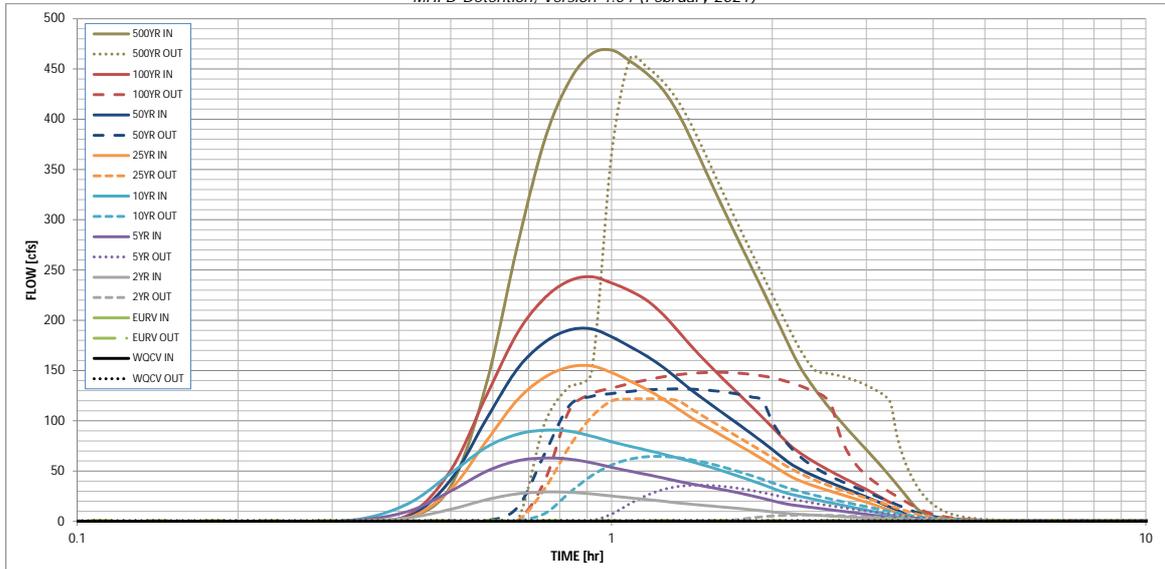
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WOCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =									
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	4.00
CUHP Runoff Volume (acre-ft) =	1.285	2.177	3.053	6.690	10.314	16.752	21.154	27.479	55.481
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	3.053	6.690	10.314	16.752	21.154	27.479	55.481
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	17.6	49.5	77.1	142.3	179.0	229.8	455.6
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.08	0.22	0.34	0.63	0.80	1.02	2.03
Peak Inflow Q (cfs) =	N/A	N/A	29.2	62.9	90.7	154.6	191.5	243.2	468.8
Peak Outflow Q (cfs) =	0.6	0.9	6.2	35.9	64.5	121.9	131.7	148.2	460.2
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.7	0.8	0.9	0.7	0.6	1.0
Structure Controlling Flow	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Outlet Plate 1	Outlet Plate 1	Spillway
Max Velocity through Gate 1 (fps) =	N/A	0.00	0.08	0.5	0.9	1.7	1.8	2.1	2.2
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	53	56	51	48	41	37	32	14
Time to Drain 99% of Inflow Volume (hours) =	40	55	59	57	56	53	52	49	40
Maximum Ponding Depth (ft) =	3.38	4.43	4.68	5.39	5.88	6.77	7.89	9.95	11.13
Area at Maximum Ponding Depth (acres) =	0.82	0.89	0.91	0.97	1.01	1.08	1.17	1.35	1.46
Maximum Volume Stored (acre-ft) =	1.288	2.185	2.402	3.079	3.563	4.490	5.739	8.339	10.003

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-*Detention*, Version 4.04 (February 2021)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

Inflow Hydrographs

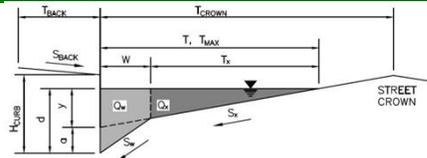
The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.08
	0:15:00	0.00	0.00	0.09	0.15	0.19	0.13	0.17	0.16	0.44
	0:20:00	0.00	0.00	0.46	1.05	1.75	0.51	0.62	0.65	3.37
	0:25:00	0.00	0.00	3.52	9.98	17.68	3.45	4.47	6.28	34.88
	0:30:00	0.00	0.00	11.92	30.04	47.68	31.82	41.26	50.74	137.01
	0:35:00	0.00	0.00	21.51	49.75	73.74	79.89	101.79	125.03	274.15
	0:40:00	0.00	0.00	27.37	60.19	86.63	120.85	151.63	187.24	379.49
	0:45:00	0.00	0.00	29.24	62.95	90.68	143.30	178.16	222.16	437.55
	0:50:00	0.00	0.00	28.90	61.88	89.65	153.54	190.25	239.24	464.73
	0:55:00	0.00	0.00	27.30	58.28	84.80	154.61	191.49	243.24	468.76
	1:00:00	0.00	0.00	25.17	53.74	79.17	147.99	183.59	237.12	457.76
	1:05:00	0.00	0.00	23.29	49.81	74.62	139.41	173.77	229.21	445.04
	1:10:00	0.00	0.00	21.66	46.44	70.63	130.89	164.01	219.46	428.91
	1:15:00	0.00	0.00	19.99	43.11	66.77	121.59	153.14	205.51	405.76
	1:20:00	0.00	0.00	18.32	39.84	62.89	111.70	141.25	189.08	377.53
	1:25:00	0.00	0.00	16.88	37.04	59.15	102.45	129.91	173.15	348.76
	1:30:00	0.00	0.00	15.70	34.68	55.36	94.62	120.15	159.22	321.92
	1:35:00	0.00	0.00	14.61	32.41	51.51	87.41	111.09	146.56	296.87
	1:40:00	0.00	0.00	13.57	30.11	47.69	80.68	102.59	135.01	273.59
	1:45:00	0.00	0.00	12.53	27.72	43.93	74.24	94.46	124.14	251.54
	1:50:00	0.00	0.00	11.50	25.30	40.26	68.02	86.61	113.65	230.41
	1:55:00	0.00	0.00	10.47	22.89	36.63	61.89	78.90	103.44	209.92
	2:00:00	0.00	0.00	9.42	20.49	32.97	55.86	71.34	93.51	190.03
	2:05:00	0.00	0.00	8.40	18.24	29.53	49.92	63.86	83.78	170.94
	2:10:00	0.00	0.00	7.56	16.53	26.95	44.49	57.05	74.95	154.24
	2:15:00	0.00	0.00	6.99	15.32	24.94	40.52	52.06	68.28	140.98
	2:20:00	0.00	0.00	6.50	14.25	23.11	37.29	47.92	62.75	129.60
	2:25:00	0.00	0.00	6.05	13.24	21.39	34.50	44.30	57.84	119.33
	2:30:00	0.00	0.00	5.61	12.26	19.77	31.95	40.97	53.38	109.93
	2:35:00	0.00	0.00	5.19	11.33	18.20	29.60	37.91	49.27	101.22
	2:40:00	0.00	0.00	4.78	10.42	16.69	27.34	34.98	45.41	93.06
	2:45:00	0.00	0.00	4.38	9.53	15.23	25.17	32.17	41.81	85.44
	2:50:00	0.00	0.00	3.99	8.66	13.83	23.06	29.47	38.40	78.24
	2:55:00	0.00	0.00	3.60	7.80	12.48	20.98	26.81	35.01	71.22
	3:00:00	0.00	0.00	3.22	6.96	11.17	18.93	24.19	31.65	64.33
	3:05:00	0.00	0.00	2.84	6.13	9.88	16.88	21.59	28.30	57.45
	3:10:00	0.00	0.00	2.46	5.30	8.60	14.84	19.00	24.94	50.60
	3:15:00	0.00	0.00	2.09	4.48	7.32	12.80	16.41	21.60	43.76
	3:20:00	0.00	0.00	1.72	3.67	6.05	10.77	13.83	18.26	36.95
	3:25:00	0.00	0.00	1.34	2.86	4.79	8.74	11.26	14.92	30.16
	3:30:00	0.00	0.00	0.98	2.05	3.55	6.72	8.69	11.60	23.41
	3:35:00	0.00	0.00	0.62	1.28	2.37	4.71	6.16	8.31	16.94
	3:40:00	0.00	0.00	0.36	0.78	1.68	2.85	3.84	5.33	11.65
	3:45:00	0.00	0.00	0.24	0.58	1.32	1.82	2.56	3.56	8.31
	3:50:00	0.00	0.00	0.19	0.45	1.05	1.19	1.76	2.42	6.00
	3:55:00	0.00	0.00	0.15	0.37	0.84	0.80	1.23	1.61	4.27
	4:00:00	0.00	0.00	0.12	0.29	0.67	0.52	0.84	1.04	2.97
	4:05:00	0.00	0.00	0.10	0.23	0.52	0.36	0.60	0.63	2.00
	4:10:00	0.00	0.00	0.08	0.18	0.39	0.24	0.41	0.35	1.29
	4:15:00	0.00	0.00	0.06	0.13	0.28	0.16	0.28	0.20	0.83
	4:20:00	0.00	0.00	0.05	0.10	0.20	0.12	0.20	0.15	0.60
	4:25:00	0.00	0.00	0.04	0.07	0.14	0.08	0.15	0.12	0.44
	4:30:00	0.00	0.00	0.03	0.05	0.11	0.06	0.12	0.09	0.35
	4:35:00	0.00	0.00	0.02	0.04	0.08	0.05	0.09	0.07	0.27
	4:40:00	0.00	0.00	0.02	0.02	0.06	0.03	0.06	0.05	0.20
	4:45:00	0.00	0.00	0.01	0.01	0.04	0.02	0.05	0.04	0.14
	4:50:00	0.00	0.00	0.01	0.01	0.02	0.02	0.03	0.03	0.09
	4:55:00	0.00	0.00	0.00	0.01	0.01	0.01	0.02	0.01	0.06
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01	0.03
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Homestead North**
 Inlet ID: **Street at DP 5A**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

T_{BACK} = 9.5 ft
 S_{BACK} = 0.020 ft/ft
 n_{BACK} = 0.013

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown
 Gutter Width
 Street Transverse Slope
 Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)
 Street Longitudinal Slope - Enter 0 for sump condition
 Manning's Roughness for Street Section (typically between 0.012 and 0.020)

H_{CURB} = 6.00 inches
 T_{CROWN} = 18.0 ft
 W = 2.00 ft
 S_X = 0.002 ft/ft
 S_W = 0.083 ft/ft
 S_0 = 0.028 ft/ft
 n_{STREET} = 0.016

Max. Allowable Spread for Minor & Major Storm
 Max. Allowable Depth at Gutter Flowline for Minor & Major Storm
 Allow Flow Depth at Street Crown (leave blank for no)

	Minor Storm	Major Storm	
T_{MAX}	18.0	18.0	ft
d_{MAX}	6.0	8.3	inches
	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	check = yes

MINOR STORM Allowable Capacity is based on Depth Criterion
MAJOR STORM Allowable Capacity is based on Depth Criterion

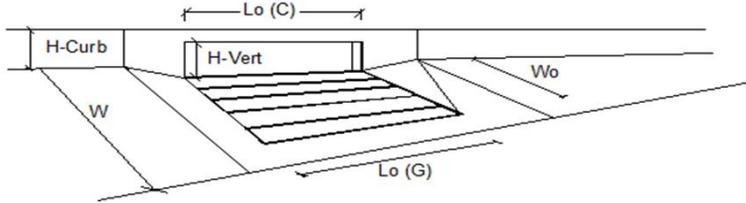
	Minor Storm	Major Storm	
Q_{allow}	34.9	63.3	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Provide 2nd page?

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

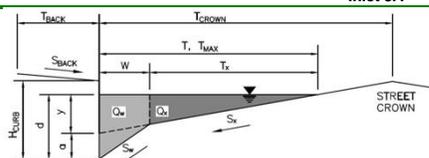


Design Information (Input)	CDOT Type R Curb Opening	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening	Type =	CDOT Type R Curb Opening	
Local Depression (additional to continuous gutter depression 'a')		$a_{LOCAL} =$	3.0	3.0
Total Number of Units in the Inlet (Grate or Curb Opening)		No =	3	3
Length of a Single Unit Inlet (Grate or Curb Opening)		$L_o =$	5.00	5.00
Width of a Unit Grate (cannot be greater than W, Gutter Width)		$W_o =$	N/A	N/A
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)		$C_r-G =$	N/A	N/A
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)		$C_r-C =$	0.10	0.10
Street Hydraulics: OK - $Q < \text{Allowable Street Capacity}$.				
Total Inlet Interception Capacity		$Q =$	7.1	12.0
Total Inlet Carry-Over Flow (flow bypassing inlet)		$Q_b =$	0.0	2.8
Capture Percentage = $Q_s/Q_o =$		C% =	100	81
				%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Homestead North**
 Inlet ID: **Inlet 3A**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

$T_{BACK} =$ ft
 $S_{BACK} =$ ft/ft
 $n_{BACK} =$

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown

$H_{CURB} =$ inches
 $T_{CROWN} =$ ft

Gutter Width

$W =$ ft

Street Transverse Slope

$S_X =$ ft/ft

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

$S_W =$ ft/ft

Street Longitudinal Slope - Enter 0 for sump condition

$S_0 =$ ft/ft

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

$n_{STREET} =$

Max. Allowable Spread for Minor & Major Storm

	Minor Storm	Major Storm	
$T_{MAX} =$	<input type="text" value="18.0"/>	<input type="text" value="18.0"/>	ft

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

	Minor Storm	Major Storm	
$d_{MAX} =$	<input type="text" value="6.0"/>	<input type="text" value="8.0"/>	inches

Allow Flow Depth at Street Crown (leave blank for no)

check = yes

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Spread Criterion

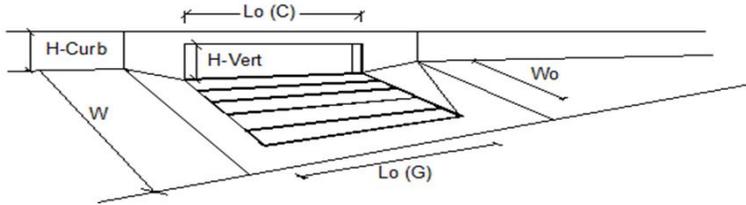
	Minor Storm	Major Storm	
$Q_{allow} =$	<input type="text" value="18.1"/>	<input type="text" value="21.1"/>	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

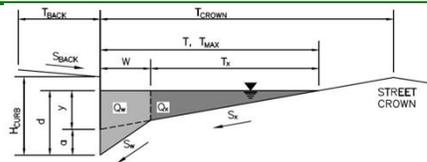


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity.			
Total Inlet Interception Capacity	8.2	14.3	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.2	6.1	cfs
Capture Percentage = Q_s/Q_o =	98	70	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Homestead North**
 Inlet ID: **Street at DP 5A**



DP5A was already included on first sheet of this section. Please remove one of the two.

Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

T_{BACK} = 9.5 ft
 S_{BACK} = 0.020 ft/ft
 n_{BACK} = 0.013

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown
 Gutter Width
 Street Transverse Slope
 Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)
 Street Longitudinal Slope - Enter 0 for sump condition
 Manning's Roughness for Street Section (typically between 0.012 and 0.020)

H_{CURB} = 6.00 inches
 T_{CROWN} = 18.0 ft
 W = 2.00 ft
 S_X = 0.002 ft/ft
 S_W = 0.083 ft/ft
 S_0 = 0.028 ft/ft
 n_{STREET} = 0.016

Max. Allowable Spread for Minor & Major Storm
 Max. Allowable Depth at Gutter Flowline for Minor & Major Storm
 Allow Flow Depth at Street Crown (leave blank for no)

	Minor Storm	Major Storm	
T_{MAX}	18.0	18.0	ft
d_{MAX}	6.0	8.3	inches
	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>	check = yes

MINOR STORM Allowable Capacity is based on Depth Criterion
MAJOR STORM Allowable Capacity is based on Depth Criterion

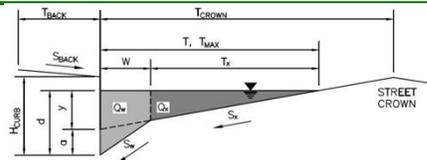
	Minor Storm	Major Storm	
Q_{allow}	34.9	63.3	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Homestead North**
 Inlet ID: **Inlet 7A**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

$T_{BACK} = 9.5$ ft
 $S_{BACK} = 0.020$ ft/ft
 $n_{BACK} = 0.016$

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown
 Gutter Width

$H_{CURB} = 6.00$ inches
 $T_{CROWN} = 18.0$ ft
 $W = 2.00$ ft

Street Transverse Slope
 Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)
 Street Longitudinal Slope - Enter 0 for sump condition
 Manning's Roughness for Street Section (typically between 0.012 and 0.020)

$S_X = 0.020$ ft/ft
 $S_W = 0.083$ ft/ft
 $S_0 = 0.000$ ft/ft
 $n_{STREET} = 0.016$

Max. Allowable Spread for Minor & Major Storm
 Max. Allowable Depth at Gutter Flowline for Minor & Major Storm
 Check boxes are not applicable in SUMP conditions

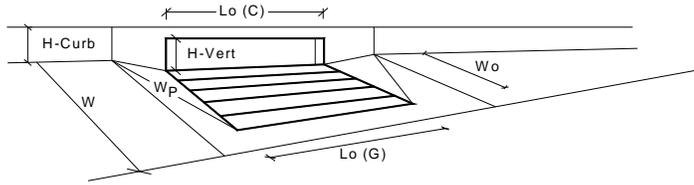
	Minor Storm	Major Storm	
$T_{MAX} =$	18.0	18.0	ft
$d_{MAX} =$	6.0	8.0	inches
	<input type="checkbox"/>	<input type="checkbox"/>	

MINOR STORM Allowable Capacity is based on Depth Criterion
MAJOR STORM Allowable Capacity is based on Depth Criterion

	Minor Storm	Major Storm	
$Q_{allow} =$	SUMP	SUMP	cfs

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



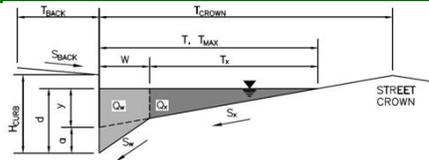
Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	4	4	
Water Depth at Flowline (outside of local depression)	6.0	8.3	inches
Grate Information	MINOR	MAJOR	<input checked="" type="checkbox"/> Override Depths
Length of a Unit Grate	N/A	N/A	feet
Width of a Unit Grate	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	N/A	N/A	
Curb Opening Information	MINOR	MAJOR	
Length of a Unit Curb Opening	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	0.67	0.67	
Low Head Performance Reduction (Calculated)	MINOR	MAJOR	
Depth for Grate Midwidth	N/A	N/A	ft
Depth for Curb Opening Weir Equation	0.33	0.53	ft
Combination Inlet Performance Reduction Factor for Long Inlets	0.57	0.78	
Curb Opening Performance Reduction Factor for Long Inlets	0.79	0.91	
Grated Inlet Performance Reduction Factor for Long Inlets	N/A	N/A	
Total Inlet Interception Capacity (assumes clogged condition)	MINOR	MAJOR	
Q_a	18.2	39.7	cfs
Q _{PEAK REQUIRED}	10.5	29.7	cfs

Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

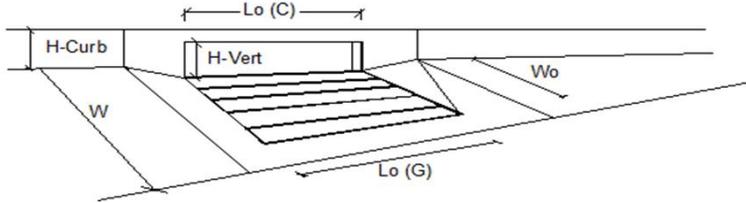
Project: **Homestead North**
 Inlet ID: **Inlet 2A**



Gutter Geometry (Enter data in the blue cells)					
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 9.5$ ft				
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft				
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.016$				
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches				
Distance from Curb Face to Street Crown	$T_{CROWN} = 18.0$ ft				
Gutter Width	$W = 2.00$ ft				
Street Transverse Slope	$S_X = 0.020$ ft/ft				
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_W = 0.083$ ft/ft				
Street Longitudinal Slope - Enter 0 for sump condition	$S_0 = 0.284$ ft/ft				
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$				
Max. Allowable Spread for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>$T_{MAX} = 18.0$</td> <td>$T_{MAX} = 18.0$</td> </tr> </table> ft	Minor Storm	Major Storm	$T_{MAX} = 18.0$	$T_{MAX} = 18.0$
Minor Storm	Major Storm				
$T_{MAX} = 18.0$	$T_{MAX} = 18.0$				
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>$d_{MAX} = 6.0$</td> <td>$d_{MAX} = 8.0$</td> </tr> </table> inches	Minor Storm	Major Storm	$d_{MAX} = 6.0$	$d_{MAX} = 8.0$
Minor Storm	Major Storm				
$d_{MAX} = 6.0$	$d_{MAX} = 8.0$				
Allow Flow Depth at Street Crown (leave blank for no)	<input type="checkbox"/> <input type="checkbox"/> check = yes				
MINOR STORM Allowable Capacity is based on Depth Criterion					
MAJOR STORM Allowable Capacity is based on Depth Criterion					
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'					
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'					
	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>$Q_{allow} = 9.0$</td> <td>$Q_{allow} = 18.6$</td> </tr> </table> cfs	Minor Storm	Major Storm	$Q_{allow} = 9.0$	$Q_{allow} = 18.6$
Minor Storm	Major Storm				
$Q_{allow} = 9.0$	$Q_{allow} = 18.6$				

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

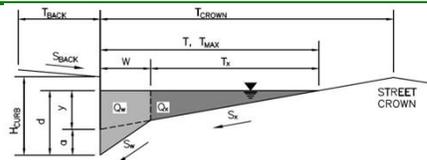


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity.			
Total Inlet Interception Capacity	6.4	11.5	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.0	1.7	cfs
Capture Percentage = Q_i/Q_o =	100	87	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Homestead North**
 Inlet ID: **Inlet 4A**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

$T_{BACK} = 9.5$ ft
 $S_{BACK} = 0.020$ ft/ft
 $n_{BACK} = 0.013$

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown

$H_{CURB} = 6.00$ inches
 $T_{CROWN} = 18.0$ ft

Gutter Width

$W = 2.00$ ft

Street Transverse Slope

$S_X = 0.020$ ft/ft

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

$S_W = 0.083$ ft/ft

Street Longitudinal Slope - Enter 0 for sump condition

$S_0 = 0.028$ ft/ft

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

$n_{STREET} = 0.016$

Max. Allowable Spread for Minor & Major Storm

	Minor Storm	Major Storm	
$T_{MAX} =$	18.0	18.0	ft
$d_{MAX} =$	6.0	8.0	inches

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

Allow Flow Depth at Street Crown (leave blank for no)

check = yes

MINOR STORM Allowable Capacity is based on Depth Criterion

MAJOR STORM Allowable Capacity is based on Spread Criterion

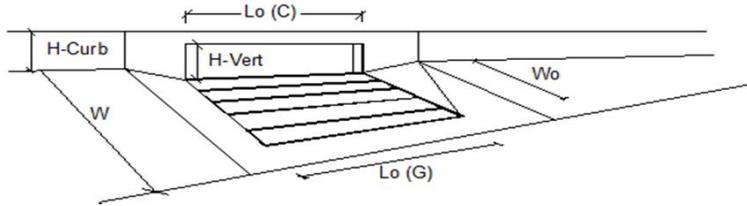
	Minor Storm	Major Storm	
$Q_{allow} =$	18.1	21.0	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

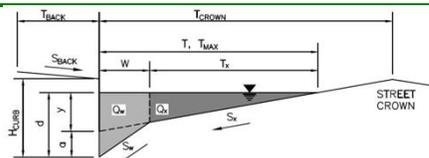


Design Information (Input)		
Type of Inlet	CDOT Type R Curb Opening	
Local Depression (additional to continuous gutter depression 'a')	Type =	CDOT Type R Curb Opening
Total Number of Units in the Inlet (Grate or Curb Opening)	$a_{LOCAL} =$	3.0 3.0 inches
Length of a Single Unit Inlet (Grate or Curb Opening)	No =	3 3
Width of a Unit Grate (cannot be greater than W, Gutter Width)	$L_o =$	5.00 5.00 ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	$W_o =$	N/A N/A ft
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	$C_r-G =$	N/A N/A
Street Hydraulics: OK - $Q < Q_{allowable}$ Street Capacity.	$C_r-C =$	0.10 0.10
Total Inlet Interception Capacity		MINOR MAJOR
Total Inlet Carry-Over Flow (flow bypassing inlet)	$Q =$	7.0 12.6 cfs
Capture Percentage = $Q_i/Q_o =$	$Q_b =$	0.0 3.6 cfs
	$C\% =$	100 78 %

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Homestead North**
 Inlet ID: **Street at DP 6A**



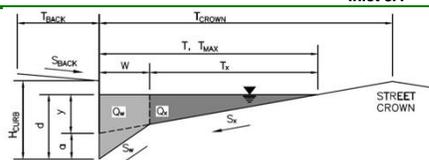
Gutter Geometry (Enter data in the blue cells)					
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 9.5$ ft				
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft				
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.013$				
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches				
Distance from Curb Face to Street Crown	$T_{CROWN} = 18.0$ ft				
Gutter Width	$W = 2.00$ ft				
Street Transverse Slope	$S_X = 0.002$ ft/ft				
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_W = 0.083$ ft/ft				
Street Longitudinal Slope - Enter 0 for sump condition	$S_0 = 0.028$ ft/ft				
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$				
Max. Allowable Spread for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>$T_{MAX} = 18.0$</td> <td>$T_{MAX} = 18.0$</td> </tr> </table> ft	Minor Storm	Major Storm	$T_{MAX} = 18.0$	$T_{MAX} = 18.0$
Minor Storm	Major Storm				
$T_{MAX} = 18.0$	$T_{MAX} = 18.0$				
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>$d_{MAX} = 6.0$</td> <td>$d_{MAX} = 8.0$</td> </tr> </table> inches	Minor Storm	Major Storm	$d_{MAX} = 6.0$	$d_{MAX} = 8.0$
Minor Storm	Major Storm				
$d_{MAX} = 6.0$	$d_{MAX} = 8.0$				
Allow Flow Depth at Street Crown (leave blank for no)	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td><input checked="" type="checkbox"/></td> <td><input checked="" type="checkbox"/></td> </tr> </table> check = yes	Minor Storm	Major Storm	<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>
Minor Storm	Major Storm				
<input checked="" type="checkbox"/>	<input checked="" type="checkbox"/>				
MINOR STORM Allowable Capacity is based on Depth Criterion					
MAJOR STORM Allowable Capacity is based on Depth Criterion					
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'					
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'					
	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>$Q_{allow} = 34.9$</td> <td>$Q_{allow} = 57.7$</td> </tr> </table> cfs	Minor Storm	Major Storm	$Q_{allow} = 34.9$	$Q_{allow} = 57.7$
Minor Storm	Major Storm				
$Q_{allow} = 34.9$	$Q_{allow} = 57.7$				

Provide 2nd page?

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Homestead North**
 Inlet ID: **Inlet 8A**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

$T_{BACK} = 9.5$ ft
 $S_{BACK} = 0.020$ ft/ft
 $n_{BACK} = 0.016$

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown
 Gutter Width

$H_{CURB} = 6.00$ inches
 $T_{CROWN} = 18.0$ ft
 $W = 2.00$ ft

Street Transverse Slope
 Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)
 Street Longitudinal Slope - Enter 0 for sump condition
 Manning's Roughness for Street Section (typically between 0.012 and 0.020)

$S_x = 0.020$ ft/ft
 $S_w = 0.083$ ft/ft
 $S_o = 0.000$ ft/ft
 $n_{STREET} = 0.016$

Max. Allowable Spread for Minor & Major Storm
 Max. Allowable Depth at Gutter Flowline for Minor & Major Storm
 Check boxes are not applicable in SUMP conditions

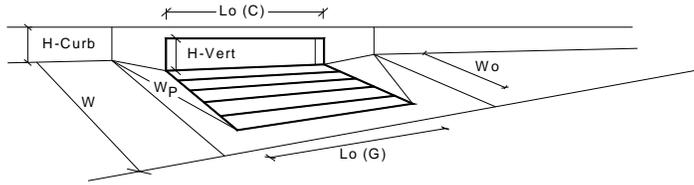
	Minor Storm	Major Storm	
$T_{MAX} =$	18.0	18.0	ft
$d_{MAX} =$	6.0	12.0	inches

MINOR STORM Allowable Capacity is based on Depth Criterion
MAJOR STORM Allowable Capacity is based on Depth Criterion

	Minor Storm	Major Storm	
$Q_{allow} =$	SUMP	SUMP	cfs

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017

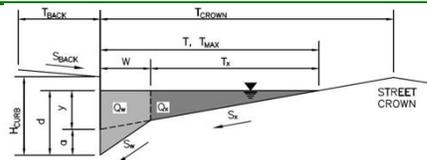


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	3	3	
Water Depth at Flowline (outside of local depression)	5.8	8.3	inches
Grate Information	MINOR	MAJOR	<input checked="" type="checkbox"/> Override Depths
Length of a Unit Grate	N/A	N/A	feet
Width of a Unit Grate	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	N/A	N/A	
Curb Opening Information	MINOR	MAJOR	
Length of a Unit Curb Opening	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	0.67	0.67	
Low Head Performance Reduction (Calculated)	MINOR	MAJOR	
Depth for Grate Midwidth	N/A	N/A	ft
Depth for Curb Opening Weir Equation	0.32	0.53	ft
Combination Inlet Performance Reduction Factor for Long Inlets	0.55	0.78	
Curb Opening Performance Reduction Factor for Long Inlets	0.78	0.91	
Grated Inlet Performance Reduction Factor for Long Inlets	N/A	N/A	
Total Inlet Interception Capacity (assumes clogged condition)	MINOR	MAJOR	
Q_a	12.5	29.4	cfs
Q _{PEAK REQUIRED}	11.3	19.9	cfs
Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)			

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Homestead North**
 Inlet ID: **Inlet DP 1.1B**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

$T_{BACK} = 9.5$ ft
 $S_{BACK} = 0.020$ ft/ft
 $n_{BACK} = 0.020$

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown

$H_{CURB} = 6.00$ inches
 $T_{CROWN} = 18.0$ ft

Gutter Width

$W = 1.17$ ft

Street Transverse Slope

$S_X = 0.020$ ft/ft

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

$S_W = 0.083$ ft/ft

Street Longitudinal Slope - Enter 0 for sump condition

$S_0 = 0.026$ ft/ft

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

$n_{STREET} = 0.016$

Max. Allowable Spread for Minor & Major Storm

	Minor Storm	Major Storm	
$T_{MAX} =$	18.0	18.0	ft

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

	Minor Storm	Major Storm	
$d_{MAX} =$	6.0	8.0	inches

Allow Flow Depth at Street Crown (leave blank for no)

check = yes

MINOR STORM Allowable Capacity is based on Spread Criterion

MAJOR STORM Allowable Capacity is based on Spread Criterion

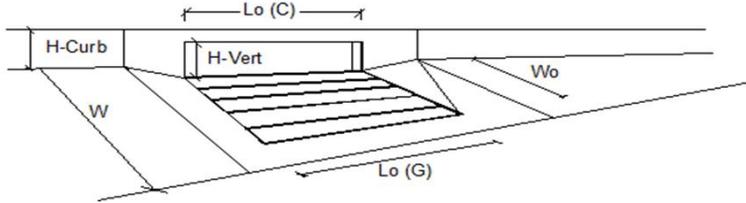
	Minor Storm	Major Storm	
$Q_{allow} =$	19.1	19.1	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

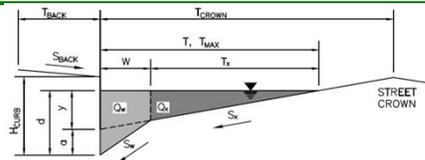


Design Information (Input)	CDOT Type R Curb Opening	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening	Type =	CDOT Type R Curb Opening	
Local Depression (additional to continuous gutter depression 'a')		$a_{LOCAL} =$	3.0	3.0
Total Number of Units in the Inlet (Grate or Curb Opening)		No =	3	3
Length of a Single Unit Inlet (Grate or Curb Opening)		$L_o =$	5.00	5.00
Width of a Unit Grate (cannot be greater than W, Gutter Width)		$W_o =$	N/A	N/A
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)		$C_r-G =$	N/A	N/A
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)		$C_r-C =$	0.10	0.10
Street Hydraulics: OK - $Q < \text{Allowable Street Capacity}$.				
Total Inlet Interception Capacity		$Q =$	5.3	9.6
Total Inlet Carry-Over Flow (flow bypassing inlet)		$Q_b =$	0.0	1.5
Capture Percentage = $Q_c/Q_o =$		C% =	100	87
				%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Homestead North**
 Inlet ID: **Inlet DP 1.2B**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

$T_{BACK} = 9.5$ ft
 $S_{BACK} = 0.020$ ft/ft
 $n_{BACK} = 0.020$

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown

$H_{CURB} = 6.00$ inches
 $T_{CROWN} = 18.0$ ft

Gutter Width

$W = 1.17$ ft

Street Transverse Slope

$S_X = 0.020$ ft/ft

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

$S_W = 0.083$ ft/ft

Street Longitudinal Slope - Enter 0 for sump condition

$S_0 = 0.026$ ft/ft

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

$n_{STREET} = 0.016$

Max. Allowable Spread for Minor & Major Storm

	Minor Storm	Major Storm	
$T_{MAX} =$	9.5	18.0	ft

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

	Minor Storm	Major Storm	
$d_{MAX} =$	6.0	8.3	inches

Allow Flow Depth at Street Crown (leave blank for no)

check = yes

MINOR STORM Allowable Capacity is based on Spread Criterion

MAJOR STORM Allowable Capacity is based on Spread Criterion

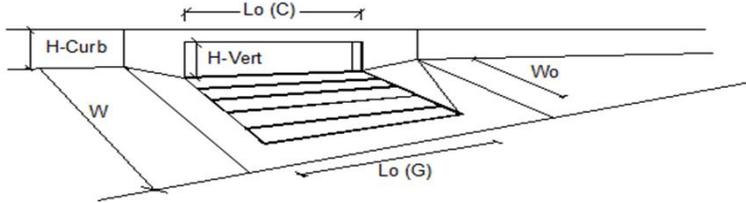
	Minor Storm	Major Storm	
$Q_{allow} =$	3.7	19.1	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

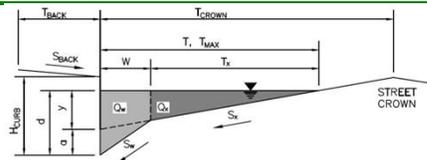


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity.			
Total Inlet Interception Capacity	3.6	7.4	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.0	0.2	cfs
Capture Percentage = Q_i/Q_o =	100	97	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

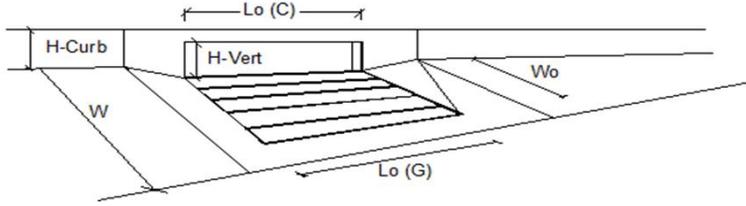
Project: **Homestead North**
 Inlet ID: **Inlet DP 4b**



Gutter Geometry (Enter data in the blue cells)													
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 9.5$ ft												
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft												
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.020$												
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches												
Distance from Curb Face to Street Crown	$T_{CROWN} = 18.0$ ft												
Gutter Width	$W = 1.17$ ft												
Street Transverse Slope	$S_x = 0.020$ ft/ft												
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_w = 0.083$ ft/ft												
Street Longitudinal Slope - Enter 0 for sump condition	$S_o = 0.016$ ft/ft												
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$												
Max. Allowable Spread for Minor & Major Storm	<table border="1"> <thead> <tr> <th></th> <th>Minor Storm</th> <th>Major Storm</th> <th></th> </tr> </thead> <tbody> <tr> <td>$T_{MAX} =$</td> <td>18.0</td> <td>18.0</td> <td>ft</td> </tr> <tr> <td>$d_{MAX} =$</td> <td>6.0</td> <td>8.0</td> <td>inches</td> </tr> </tbody> </table>		Minor Storm	Major Storm		$T_{MAX} =$	18.0	18.0	ft	$d_{MAX} =$	6.0	8.0	inches
	Minor Storm	Major Storm											
$T_{MAX} =$	18.0	18.0	ft										
$d_{MAX} =$	6.0	8.0	inches										
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm													
Allow Flow Depth at Street Crown (leave blank for no)	<input type="checkbox"/> <input checked="" type="checkbox"/> check = yes												
MINOR STORM Allowable Capacity is based on Spread Criterion													
MAJOR STORM Allowable Capacity is based on Depth Criterion													
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'													
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	Minor Storm	Major Storm											
$Q_{allow} =$	15.0	52.4	cfs										

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

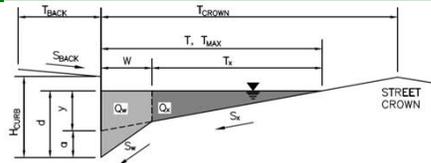


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity.			
Total Inlet Interception Capacity	6.7	11.9	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.1	4.1	cfs
Capture Percentage = Q_i/Q_o =	99	74	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

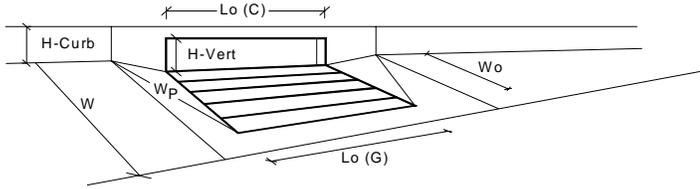
Project: Homestead North
 Inlet ID: Inlet DP 9b



Gutter Geometry (Enter data in the blue cells)							
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 9.5$ ft						
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft						
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.020$						
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches						
Distance from Curb Face to Street Crown	$T_{CROWN} = 18.0$ ft						
Gutter Width	$W = 2.00$ ft						
Street Transverse Slope	$S_X = 0.020$ ft/ft						
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_W = 0.083$ ft/ft						
Street Longitudinal Slope - Enter 0 for sump condition	$S_O = 0.000$ ft/ft						
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$						
Max. Allowable Spread for Minor & Major Storm	<table border="1" style="display: inline-table; border-collapse: collapse;"> <tr> <th style="padding: 2px;">Minor Storm</th> <th style="padding: 2px;">Major Storm</th> <th style="padding: 2px;">ft</th> </tr> <tr> <td style="text-align: center; padding: 2px;">$T_{MAX} = 18.0$</td> <td style="text-align: center; padding: 2px;">$T_{MAX} = 18.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	ft	$T_{MAX} = 18.0$	$T_{MAX} = 18.0$	
Minor Storm	Major Storm	ft					
$T_{MAX} = 18.0$	$T_{MAX} = 18.0$						
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table border="1" style="display: inline-table; border-collapse: collapse;"> <tr> <th style="padding: 2px;">Minor Storm</th> <th style="padding: 2px;">Major Storm</th> <th style="padding: 2px;">inches</th> </tr> <tr> <td style="text-align: center; padding: 2px;">$d_{MAX} = 6.0$</td> <td style="text-align: center; padding: 2px;">$d_{MAX} = 8.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	inches	$d_{MAX} = 6.0$	$d_{MAX} = 8.0$	
Minor Storm	Major Storm	inches					
$d_{MAX} = 6.0$	$d_{MAX} = 8.0$						
Check boxes are not applicable in SUMP conditions	<input type="checkbox"/> <input type="checkbox"/>						
MINOR STORM Allowable Capacity is based on Depth Criterion							
MAJOR STORM Allowable Capacity is based on Depth Criterion							
Q _{allow} =	<table border="1" style="display: inline-table; border-collapse: collapse;"> <tr> <th style="padding: 2px;">Minor Storm</th> <th style="padding: 2px;">Major Storm</th> <th style="padding: 2px;">cfs</th> </tr> <tr> <td style="text-align: center; padding: 2px;">SUMP</td> <td style="text-align: center; padding: 2px;">SUMP</td> <td></td> </tr> </table>	Minor Storm	Major Storm	cfs	SUMP	SUMP	
Minor Storm	Major Storm	cfs					
SUMP	SUMP						

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



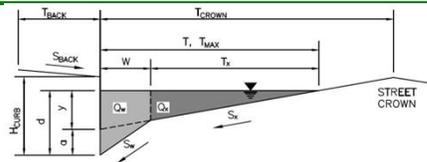
Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	3	3	
Water Depth at Flowline (outside of local depression)	6.0	12.0	inches
Grate Information	MINOR	MAJOR	<input checked="" type="checkbox"/> Override Depths
Length of a Unit Grate	N/A	N/A	feet
Width of a Unit Grate	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	N/A	N/A	
Curb Opening Information	MINOR	MAJOR	
Length of a Unit Curb Opening	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	0.67	0.67	
Low Head Performance Reduction (Calculated)	MINOR	MAJOR	
Depth for Grate Midwidth	N/A	N/A	ft
Depth for Curb Opening Weir Equation	0.33	0.83	ft
Combination Inlet Performance Reduction Factor for Long Inlets	0.57	1.00	
Curb Opening Performance Reduction Factor for Long Inlets	0.79	1.00	
Grated Inlet Performance Reduction Factor for Long Inlets	N/A	N/A	
Total Inlet Interception Capacity (assumes clogged condition)	MINOR	MAJOR	
Q_a	13.5	39.1	cfs
Q_{PEAK REQUIRED}	12.5	30.9	cfs

Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

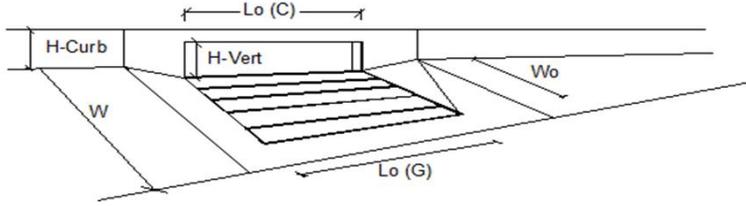
Project: **Homestead North**
 Inlet ID: **Inlet DP 7b**



Gutter Geometry (Enter data in the blue cells)					
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 9.5$ ft				
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft				
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.020$				
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches				
Distance from Curb Face to Street Crown	$T_{CROWN} = 18.0$ ft				
Gutter Width	$W = 1.17$ ft				
Street Transverse Slope	$S_X = 0.020$ ft/ft				
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_W = 0.083$ ft/ft				
Street Longitudinal Slope - Enter 0 for sump condition	$S_0 = 0.016$ ft/ft				
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$				
Max. Allowable Spread for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>$T_{MAX} = 18.0$</td> <td>$T_{MAX} = 18.0$</td> </tr> </table> ft	Minor Storm	Major Storm	$T_{MAX} = 18.0$	$T_{MAX} = 18.0$
Minor Storm	Major Storm				
$T_{MAX} = 18.0$	$T_{MAX} = 18.0$				
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>$d_{MAX} = 6.0$</td> <td>$d_{MAX} = 8.0$</td> </tr> </table> inches	Minor Storm	Major Storm	$d_{MAX} = 6.0$	$d_{MAX} = 8.0$
Minor Storm	Major Storm				
$d_{MAX} = 6.0$	$d_{MAX} = 8.0$				
Allow Flow Depth at Street Crown (leave blank for no)	<input type="checkbox"/> <input checked="" type="checkbox"/> check = yes				
MINOR STORM Allowable Capacity is based on Spread Criterion					
MAJOR STORM Allowable Capacity is based on Depth Criterion					
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'					
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'					
	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>$Q_{allow} = 15.0$</td> <td>$Q_{allow} = 52.3$</td> </tr> </table> cfs	Minor Storm	Major Storm	$Q_{allow} = 15.0$	$Q_{allow} = 52.3$
Minor Storm	Major Storm				
$Q_{allow} = 15.0$	$Q_{allow} = 52.3$				

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

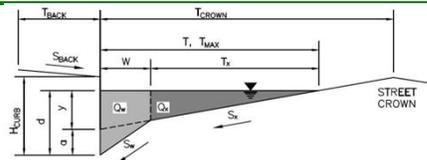


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity.			
Total Inlet Interception Capacity	7.0	11.3	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.1	3.2	cfs
Capture Percentage = Q_b/Q_o =	98	78	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Homestead North**
 Inlet ID: **Inlet DP 10b**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

$T_{BACK} = 9.5$ ft
 $S_{BACK} = 0.020$ ft/ft
 $n_{BACK} = 0.020$

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown
 Gutter Width

$H_{CURB} = 6.00$ inches
 $T_{CROWN} = 18.0$ ft
 $W = 2.00$ ft

Street Transverse Slope
 Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)
 Street Longitudinal Slope - Enter 0 for sump condition

$S_x = 0.020$ ft/ft
 $S_w = 0.083$ ft/ft
 $S_o = 0.000$ ft/ft

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

$n_{STREET} = 0.016$

Max. Allowable Spread for Minor & Major Storm
 Max. Allowable Depth at Gutter Flowline for Minor & Major Storm
 Check boxes are not applicable in SUMP conditions

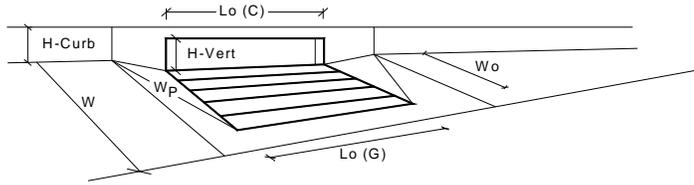
	Minor Storm	Major Storm	
$T_{MAX} =$	18.0	18.0	ft
$d_{MAX} =$	6.0	8.0	inches
	<input type="checkbox"/>	<input type="checkbox"/>	

MINOR STORM Allowable Capacity is based on Depth Criterion
MAJOR STORM Allowable Capacity is based on Depth Criterion

	Minor Storm	Major Storm	
$Q_{allow} =$	SUMP	SUMP	cfs

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



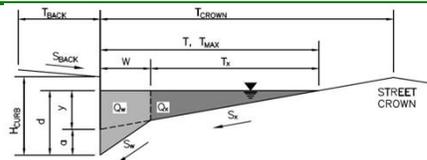
Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	3	3	
Water Depth at Flowline (outside of local depression)	5.8	8.0	inches
Grate Information	MINOR	MAJOR	<input checked="" type="checkbox"/> Override Depths
Length of a Unit Grate	N/A	N/A	feet
Width of a Unit Grate	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	N/A	N/A	
Curb Opening Information	MINOR	MAJOR	
Length of a Unit Curb Opening	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	0.67	0.67	
Low Head Performance Reduction (Calculated)	MINOR	MAJOR	
Depth for Grate Midwidth	N/A	N/A	ft
Depth for Curb Opening Weir Equation	0.32	0.50	ft
Combination Inlet Performance Reduction Factor for Long Inlets	0.55	0.75	
Curb Opening Performance Reduction Factor for Long Inlets	0.78	0.89	
Grated Inlet Performance Reduction Factor for Long Inlets	N/A	N/A	
Total Inlet Interception Capacity (assumes clogged condition)	MINOR	MAJOR	
Q_a	12.5	27.9	cfs
Q _{PEAK REQUIRED}	5.0	12.5	cfs

Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

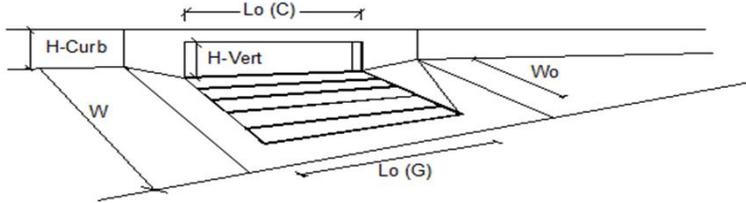
Project: **Homestead North**
 Inlet ID: **Inlet DP 2.3C**



Gutter Geometry (Enter data in the blue cells)	
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 9.5$ ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.020$
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches
Distance from Curb Face to Street Crown	$T_{CROWN} = 18.0$ ft
Gutter Width	$W = 1.17$ ft
Street Transverse Slope	$S_x = 0.020$ ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_w = 0.083$ ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	$S_o = 0.027$ ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$
Max. Allowable Spread for Minor & Major Storm	$T_{MAX} = \begin{matrix} \text{Minor Storm} & \text{Major Storm} \\ 18.0 & 18.0 \end{matrix}$ ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	$d_{MAX} = \begin{matrix} \text{Minor Storm} & \text{Major Storm} \\ 6.0 & 8.0 \end{matrix}$ inches
Allow Flow Depth at Street Crown (leave blank for no)	<input checked="" type="checkbox"/> <input checked="" type="checkbox"/> check = yes
MINOR STORM Allowable Capacity is based on Depth Criterion	
MAJOR STORM Allowable Capacity is based on Depth Criterion	
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'	
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'	
	$Q_{allow} = \begin{matrix} \text{Minor Storm} & \text{Major Storm} \\ 24.2 & 44.5 \end{matrix}$ cfs

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

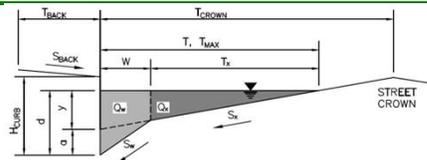


Design Information (Input)	MINOR		MAJOR	
Type of Inlet	CDOT Type R Curb Opening			
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches	
Total Number of Units in the Inlet (Grate or Curb Opening)	3	3		
Length of a Single Unit Inlet (Grate or Curb Opening)	5.00	5.00	ft	
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft	
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A		
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10		
Street Hydraulics: OK - Q < Allowable Street Capacity.				
Total Inlet Interception Capacity	7.2	11.6	cfs	
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.1	3.6	cfs	
Capture Percentage = Q_c/Q_o =	98	76	%	

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Homestead North**
 Inlet ID: **Inlet DP 2.1C**



Gutter Geometry (Enter data in the blue cells)

Maximum Allowable Width for Spread Behind Curb
 Side Slope Behind Curb (leave blank for no conveyance credit behind curb)
 Manning's Roughness Behind Curb (typically between 0.012 and 0.020)

$T_{BACK} = 7.5$ ft
 $S_{BACK} = 0.020$ ft/ft
 $n_{BACK} = 0.020$

Height of Curb at Gutter Flow Line
 Distance from Curb Face to Street Crown

$H_{CURB} = 6.00$ inches
 $T_{CROWN} = 18.0$ ft

Gutter Width

$W = 1.17$ ft

Street Transverse Slope

$S_X = 0.020$ ft/ft

Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)

$S_W = 0.083$ ft/ft

Street Longitudinal Slope - Enter 0 for sump condition

$S_0 = 0.020$ ft/ft

Manning's Roughness for Street Section (typically between 0.012 and 0.020)

$n_{STREET} = 0.016$

Max. Allowable Spread for Minor & Major Storm

	Minor Storm	Major Storm	
$T_{MAX} =$	18.0	18.0	ft

Max. Allowable Depth at Gutter Flowline for Minor & Major Storm

	Minor Storm	Major Storm	
$d_{MAX} =$	6.0	8.0	inches

Allow Flow Depth at Street Crown (leave blank for no)

check = yes

MINOR STORM Allowable Capacity is based on Spread Criterion

MAJOR STORM Allowable Capacity is based on Spread Criterion

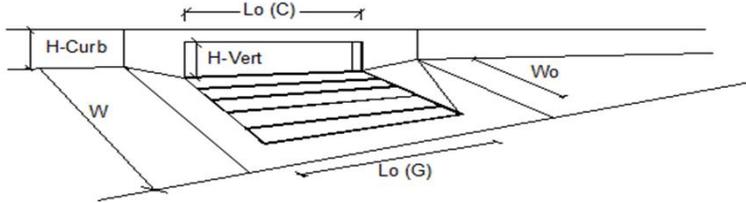
	Minor Storm	Major Storm	
$Q_{allow} =$	16.7	16.7	cfs

Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

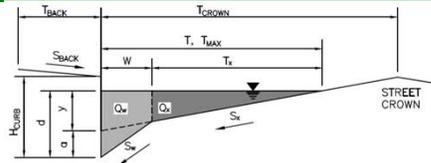


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity.			
Total Inlet Interception Capacity	0.8	1.5	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.0	0.1	cfs
Capture Percentage = Q_s/Q_o =	100	91	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

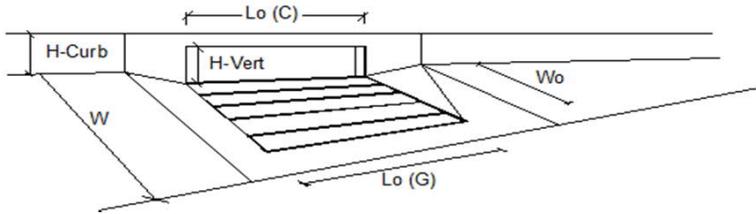
Project: Homestead North
 Inlet ID: Inlet DP 4.2C



Gutter Geometry (Enter data in the blue cells)							
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 7.5$ ft						
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft						
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.020$						
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches						
Distance from Curb Face to Street Crown	$T_{CROWN} = 18.0$ ft						
Gutter Width	$W = 1.17$ ft						
Street Transverse Slope	$S_X = 0.020$ ft/ft						
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_W = 0.083$ ft/ft						
Street Longitudinal Slope - Enter 0 for sump condition	$S_O = 0.020$ ft/ft						
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$						
Max. Allowable Spread for Minor & Major Storm	<table border="1"> <thead> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>ft</th> </tr> </thead> <tbody> <tr> <td>$T_{MAX} = 18.0$</td> <td>$T_{MAX} = 18.0$</td> <td></td> </tr> </tbody> </table>	Minor Storm	Major Storm	ft	$T_{MAX} = 18.0$	$T_{MAX} = 18.0$	
Minor Storm	Major Storm	ft					
$T_{MAX} = 18.0$	$T_{MAX} = 18.0$						
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table border="1"> <thead> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>inches</th> </tr> </thead> <tbody> <tr> <td>$d_{MAX} = 6.0$</td> <td>$d_{MAX} = 8.0$</td> <td></td> </tr> </tbody> </table>	Minor Storm	Major Storm	inches	$d_{MAX} = 6.0$	$d_{MAX} = 8.0$	
Minor Storm	Major Storm	inches					
$d_{MAX} = 6.0$	$d_{MAX} = 8.0$						
Allow Flow Depth at Street Crown (leave blank for no)	<input type="checkbox"/> <input type="checkbox"/> check = yes						
MINOR STORM Allowable Capacity is based on Spread Criterion							
MAJOR STORM Allowable Capacity is based on Spread Criterion							
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'	<table border="1"> <thead> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>cfs</th> </tr> </thead> <tbody> <tr> <td>$Q_{allow} = 16.7$</td> <td>$Q_{allow} = 16.7$</td> <td></td> </tr> </tbody> </table>	Minor Storm	Major Storm	cfs	$Q_{allow} = 16.7$	$Q_{allow} = 16.7$	
Minor Storm	Major Storm	cfs					
$Q_{allow} = 16.7$	$Q_{allow} = 16.7$						
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'							

INLET ON A CONTINUOUS GRADE

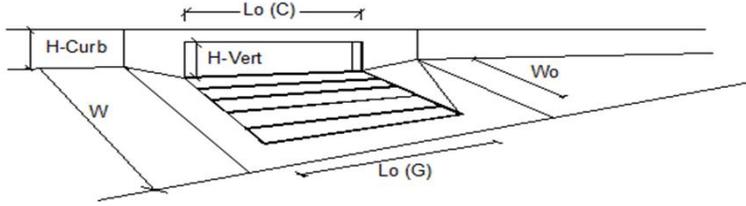
Version 4.05 Released March 2017



Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	3	3	
Length of a Single Unit Inlet (Grate or Curb Opening)	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity*			
Total Inlet Interception Capacity	5.9	10.7	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.0	2.6	cfs
Capture Percentage = Q_i/Q_o =	100	81	%

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

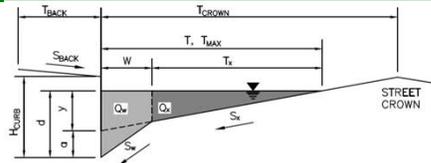


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity.			
Total Inlet Interception Capacity	1.3	2.0	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.0	0.6	cfs
Capture Percentage = Q_i/Q_o =	97	79	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

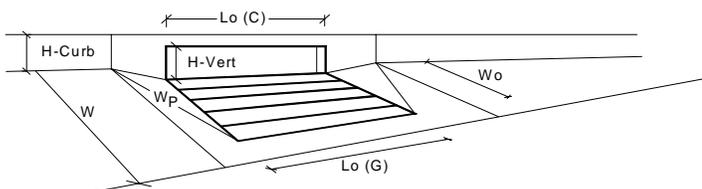
Project: Homestead North
 Inlet ID: Inlet DP 4C



Gutter Geometry (Enter data in the blue cells)																	
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = $ <input style="width: 50px;" type="text" value="9.5"/> ft																
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = $ <input style="width: 50px;" type="text" value="0.020"/> ft/ft																
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = $ <input style="width: 50px;" type="text" value="0.020"/>																
Height of Curb at Gutter Flow Line	$H_{CURB} = $ <input style="width: 50px;" type="text" value="6.00"/> inches																
Distance from Curb Face to Street Crown	$T_{CROWN} = $ <input style="width: 50px;" type="text" value="18.0"/> ft																
Gutter Width	$W = $ <input style="width: 50px;" type="text" value="2.00"/> ft																
Street Transverse Slope	$S_X = $ <input style="width: 50px;" type="text" value="0.020"/> ft/ft																
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_W = $ <input style="width: 50px;" type="text" value="0.083"/> ft/ft																
Street Longitudinal Slope - Enter 0 for sump condition	$S_O = $ <input style="width: 50px;" type="text" value="0.000"/> ft/ft																
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = $ <input style="width: 50px;" type="text" value="0.016"/>																
Max. Allowable Spread for Minor & Major Storm	<table style="width: 100%; border-collapse: collapse;"> <tr> <th style="width: 50%;"></th> <th style="width: 25%; text-align: center;">Minor Storm</th> <th style="width: 25%; text-align: center;">Major Storm</th> <th style="width: 10%;"></th> </tr> <tr> <td>$T_{MAX} =$</td> <td style="text-align: center;"><input style="width: 40px;" type="text" value="18.0"/></td> <td style="text-align: center;"><input style="width: 40px;" type="text" value="18.0"/></td> <td style="text-align: right;">ft</td> </tr> <tr> <td>$d_{MAX} =$</td> <td style="text-align: center;"><input style="width: 40px;" type="text" value="6.0"/></td> <td style="text-align: center;"><input style="width: 40px;" type="text" value="8.3"/></td> <td style="text-align: right;">inches</td> </tr> <tr> <td></td> <td style="text-align: center;"><input type="checkbox"/></td> <td style="text-align: center;"><input type="checkbox"/></td> <td></td> </tr> </table>		Minor Storm	Major Storm		$T_{MAX} = $	<input style="width: 40px;" type="text" value="18.0"/>	<input style="width: 40px;" type="text" value="18.0"/>	ft	$d_{MAX} = $	<input style="width: 40px;" type="text" value="6.0"/>	<input style="width: 40px;" type="text" value="8.3"/>	inches		<input type="checkbox"/>	<input type="checkbox"/>	
	Minor Storm	Major Storm															
$T_{MAX} = $	<input style="width: 40px;" type="text" value="18.0"/>	<input style="width: 40px;" type="text" value="18.0"/>	ft														
$d_{MAX} = $	<input style="width: 40px;" type="text" value="6.0"/>	<input style="width: 40px;" type="text" value="8.3"/>	inches														
	<input type="checkbox"/>	<input type="checkbox"/>															
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm																	
Check boxes are not applicable in SUMP conditions																	
MINOR STORM Allowable Capacity is based on Depth Criterion																	
MAJOR STORM Allowable Capacity is based on Depth Criterion																	
$Q_{allow} = $	<table style="width: 100%; border-collapse: collapse;"> <tr> <th style="width: 50%;"></th> <th style="width: 25%; text-align: center;">Minor Storm</th> <th style="width: 25%; text-align: center;">Major Storm</th> <th style="width: 10%;"></th> </tr> <tr> <td></td> <td style="text-align: center;"><input style="width: 40px;" type="text" value="SUMP"/></td> <td style="text-align: center;"><input style="width: 40px;" type="text" value="SUMP"/></td> <td style="text-align: right;">cfs</td> </tr> </table>		Minor Storm	Major Storm			<input style="width: 40px;" type="text" value="SUMP"/>	<input style="width: 40px;" type="text" value="SUMP"/>	cfs								
	Minor Storm	Major Storm															
	<input style="width: 40px;" type="text" value="SUMP"/>	<input style="width: 40px;" type="text" value="SUMP"/>	cfs														

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



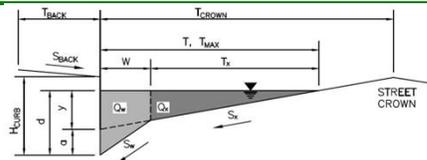
Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	4	4	
Water Depth at Flowline (outside of local depression)	6.0	12.0	inches
Grate Information	MINOR	MAJOR	<input checked="" type="checkbox"/> Override Depths
Length of a Unit Grate	N/A	N/A	feet
Width of a Unit Grate	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	N/A	N/A	
Curb Opening Information	MINOR	MAJOR	
Length of a Unit Curb Opening	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	0.67	0.67	
Low Head Performance Reduction (Calculated)	MINOR	MAJOR	
Depth for Grate Midwidth	N/A	N/A	ft
Depth for Curb Opening Weir Equation	0.33	0.83	ft
Combination Inlet Performance Reduction Factor for Long Inlets	0.57	1.00	
Curb Opening Performance Reduction Factor for Long Inlets	0.79	1.00	
Grated Inlet Performance Reduction Factor for Long Inlets	N/A	N/A	
Total Inlet Interception Capacity (assumes clogged condition)	MINOR	MAJOR	
Q_a	18.2	52.7	cfs
Q_{PEAK REQUIRED}	18.9	42.0	cfs

WARNING: Inlet Capacity less than Q Peak for Minor Storm

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

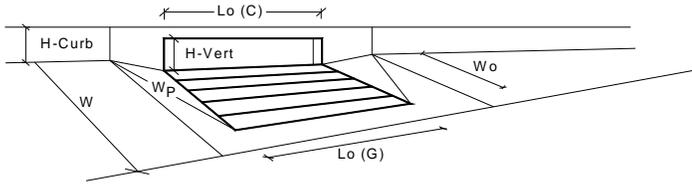
Project: **Homestead North**
 Inlet ID: **Inlet DP 5C**



Gutter Geometry (Enter data in the blue cells)					
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 9.5$ ft				
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft				
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.020$				
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches				
Distance from Curb Face to Street Crown	$T_{CROWN} = 18.0$ ft				
Gutter Width	$W = 2.00$ ft				
Street Transverse Slope	$S_X = 0.020$ ft/ft				
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_W = 0.083$ ft/ft				
Street Longitudinal Slope - Enter 0 for sump condition	$S_0 = 0.000$ ft/ft				
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$				
Max. Allowable Spread for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>$T_{MAX} = 18.0$</td> <td>$T_{MAX} = 18.0$</td> </tr> </table> ft	Minor Storm	Major Storm	$T_{MAX} = 18.0$	$T_{MAX} = 18.0$
Minor Storm	Major Storm				
$T_{MAX} = 18.0$	$T_{MAX} = 18.0$				
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>$d_{MAX} = 6.0$</td> <td>$d_{MAX} = 8.3$</td> </tr> </table> inches	Minor Storm	Major Storm	$d_{MAX} = 6.0$	$d_{MAX} = 8.3$
Minor Storm	Major Storm				
$d_{MAX} = 6.0$	$d_{MAX} = 8.3$				
Check boxes are not applicable in SUMP conditions	<input type="checkbox"/> <input type="checkbox"/>				
MINOR STORM Allowable Capacity is based on Depth Criterion					
MAJOR STORM Allowable Capacity is based on Depth Criterion					
$Q_{allow} =$	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> </tr> <tr> <td>SUMP</td> <td>SUMP</td> </tr> </table> cfs	Minor Storm	Major Storm	SUMP	SUMP
Minor Storm	Major Storm				
SUMP	SUMP				

INLET IN A SUMP OR SAG LOCATION

Version 4.05 Released March 2017



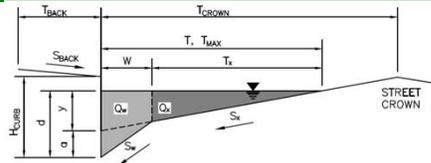
Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	2	2	
Water Depth at Flowline (outside of local depression)	5.8	5.8	inches
Grate Information	MINOR	MAJOR	<input type="checkbox"/> Override Depths
Length of a Unit Grate	N/A	N/A	feet
Width of a Unit Grate	N/A	N/A	feet
Area Opening Ratio for a Grate (typical values 0.15-0.90)	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	N/A	N/A	
Curb Opening Information	MINOR	MAJOR	
Length of a Unit Curb Opening	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	6.00	6.00	inches
Angle of Throat (see USDCM Figure ST-5)	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	2.00	2.00	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	0.67	0.67	
Low Head Performance Reduction (Calculated)	MINOR	MAJOR	
Depth for Grate Midwidth	N/A	N/A	ft
Depth for Curb Opening Weir Equation	0.32	0.32	ft
Combination Inlet Performance Reduction Factor for Long Inlets	0.55	0.55	
Curb Opening Performance Reduction Factor for Long Inlets	0.92	0.92	
Grated Inlet Performance Reduction Factor for Long Inlets	N/A	N/A	
Total Inlet Interception Capacity (assumes clogged condition)	MINOR	MAJOR	
Q_a	9.7	9.7	cfs
Q _{PEAK REQUIRED}	4.2	9.0	cfs

Inlet Capacity IS GOOD for Minor and Major Storms(>Q PEAK)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

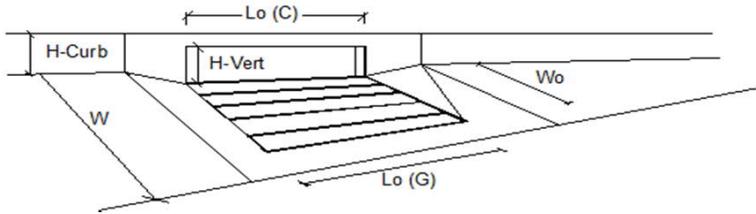
Project: Homestead North
 Inlet ID: Inlet DP 3D



Gutter Geometry (Enter data in the blue cells)							
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 25.0$ ft						
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft						
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.016$						
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches						
Distance from Curb Face to Street Crown	$T_{CROWN} = 26.5$ ft						
Gutter Width	$W = 2.00$ ft						
Street Transverse Slope	$S_x = 0.020$ ft/ft						
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_w = 0.083$ ft/ft						
Street Longitudinal Slope - Enter 0 for sump condition	$S_o = 0.023$ ft/ft						
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$						
Max. Allowable Spread for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>ft</th> </tr> <tr> <td>$T_{MAX} = 15.0$</td> <td>$T_{MAX} = 25.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	ft	$T_{MAX} = 15.0$	$T_{MAX} = 25.0$	
Minor Storm	Major Storm	ft					
$T_{MAX} = 15.0$	$T_{MAX} = 25.0$						
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>inches</th> </tr> <tr> <td>$d_{MAX} = 6.0$</td> <td>$d_{MAX} = 8.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	inches	$d_{MAX} = 6.0$	$d_{MAX} = 8.0$	
Minor Storm	Major Storm	inches					
$d_{MAX} = 6.0$	$d_{MAX} = 8.0$						
Allow Flow Depth at Street Crown (leave blank for no)	<input type="checkbox"/> <input type="checkbox"/> check = yes						
MINOR STORM Allowable Capacity is based on Spread Criterion							
MAJOR STORM Allowable Capacity is based on Depth Criterion							
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'	$Q_{allow} = 12.0$ cfs						
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'	$Q_{allow} = 41.9$ cfs						

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

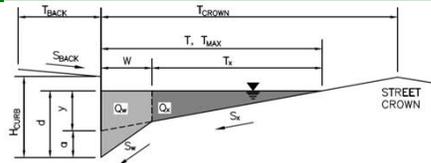


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity*			
Total Inlet Interception Capacity	0.5	1.1	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.0	0.0	cfs
Capture Percentage = Q_i/Q_o =	100	100	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

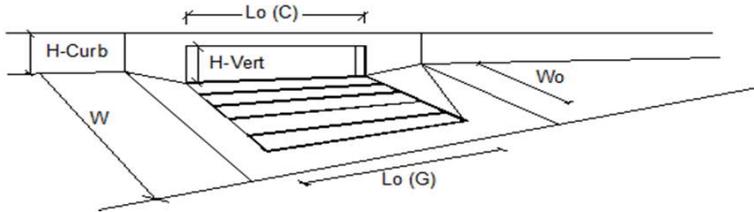
Project: _____
 Inlet ID: _____
Homestead North
Inlet DP 4D



Gutter Geometry (Enter data in the blue cells)									
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} =$ <input style="width: 50px;" type="text" value="15.0"/> ft								
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} =$ <input style="width: 50px;" type="text" value="0.020"/> ft/ft								
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} =$ <input style="width: 50px;" type="text" value="0.016"/>								
Height of Curb at Gutter Flow Line	$H_{CURB} =$ <input style="width: 50px;" type="text" value="6.00"/> inches								
Distance from Curb Face to Street Crown	$T_{CROWN} =$ <input style="width: 50px;" type="text" value="36.0"/> ft								
Gutter Width	$W =$ <input style="width: 50px;" type="text" value="2.00"/> ft								
Street Transverse Slope	$S_x =$ <input style="width: 50px;" type="text" value="0.020"/> ft/ft								
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_w =$ <input style="width: 50px;" type="text" value="0.083"/> ft/ft								
Street Longitudinal Slope - Enter 0 for sump condition	$S_o =$ <input style="width: 50px;" type="text" value="0.023"/> ft/ft								
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} =$ <input style="width: 50px;" type="text" value="0.016"/>								
Max. Allowable Spread for Minor & Major Storm	<table style="width: 100%; border: none;"> <tr> <td style="text-align: center; border: none;">$T_{MAX} =$</td> <td style="text-align: center; border: none;">Minor Storm</td> <td style="text-align: center; border: none;">Major Storm</td> <td style="border: none;"></td> </tr> <tr> <td style="border: 1px solid black; width: 50px; text-align: center;">15.0</td> <td style="border: 1px solid black; width: 50px; text-align: center;">25.0</td> <td style="border: 1px solid black; width: 50px; text-align: center;">ft</td> <td style="border: none;"></td> </tr> </table>	$T_{MAX} =$	Minor Storm	Major Storm		15.0	25.0	ft	
$T_{MAX} =$	Minor Storm	Major Storm							
15.0	25.0	ft							
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table style="width: 100%; border: none;"> <tr> <td style="text-align: center; border: none;">$d_{MAX} =$</td> <td style="text-align: center; border: none;">Minor Storm</td> <td style="text-align: center; border: none;">Major Storm</td> <td style="border: none;"></td> </tr> <tr> <td style="border: 1px solid black; width: 50px; text-align: center;">6.0</td> <td style="border: 1px solid black; width: 50px; text-align: center;">6.0</td> <td style="border: 1px solid black; width: 50px; text-align: center;">inches</td> <td style="border: none;"></td> </tr> </table>	$d_{MAX} =$	Minor Storm	Major Storm		6.0	6.0	inches	
$d_{MAX} =$	Minor Storm	Major Storm							
6.0	6.0	inches							
Allow Flow Depth at Street Crown (leave blank for no)	<table style="width: 100%; border: none;"> <tr> <td style="text-align: center; border: none;"><input type="checkbox"/></td> <td style="text-align: center; border: none;"><input type="checkbox"/></td> <td style="border: none;">check = yes</td> </tr> </table>	<input type="checkbox"/>	<input type="checkbox"/>	check = yes					
<input type="checkbox"/>	<input type="checkbox"/>	check = yes							
MINOR STORM Allowable Capacity is based on Spread Criterion									
MAJOR STORM Allowable Capacity is based on Depth Criterion									
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'	$Q_{allow} =$ <table style="display: inline-table; border: none;"><tr><td style="text-align: center; border: none;">Minor Storm</td><td style="text-align: center; border: none;">Major Storm</td></tr><tr><td style="border: 1px solid black; width: 50px; text-align: center;">12.0</td><td style="border: 1px solid black; width: 50px; text-align: center;">19.4</td></tr></table> cfs	Minor Storm	Major Storm	12.0	19.4				
Minor Storm	Major Storm								
12.0	19.4								
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'									

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

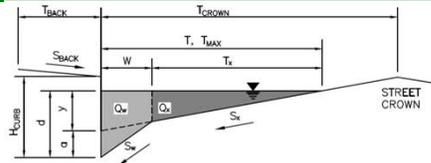


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	5.00	5.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity*			
Total Inlet Interception Capacity	1.1	1.8	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.0	0.3	cfs
Capture Percentage = Q_i/Q_o =	100	86	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

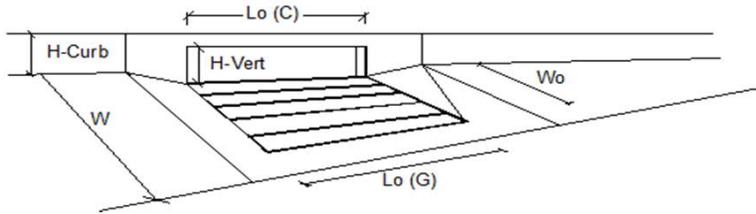
Project: Homestead North
 Inlet ID: Inlet DP 5D



Gutter Geometry (Enter data in the blue cells)							
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 15.0$ ft						
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft						
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.016$						
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches						
Distance from Curb Face to Street Crown	$T_{CROWN} = 33.0$ ft						
Gutter Width	$W = 2.00$ ft						
Street Transverse Slope	$S_x = 0.020$ ft/ft						
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_w = 0.083$ ft/ft						
Street Longitudinal Slope - Enter 0 for sump condition	$S_o = 0.030$ ft/ft						
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$						
Max. Allowable Spread for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>ft</th> </tr> <tr> <td>$T_{MAX} = 33.0$</td> <td>$T_{MAX} = 33.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	ft	$T_{MAX} = 33.0$	$T_{MAX} = 33.0$	
Minor Storm	Major Storm	ft					
$T_{MAX} = 33.0$	$T_{MAX} = 33.0$						
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>inches</th> </tr> <tr> <td>$d_{MAX} = 6.0$</td> <td>$d_{MAX} = 6.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	inches	$d_{MAX} = 6.0$	$d_{MAX} = 6.0$	
Minor Storm	Major Storm	inches					
$d_{MAX} = 6.0$	$d_{MAX} = 6.0$						
Allow Flow Depth at Street Crown (leave blank for no)	<input type="checkbox"/> <input type="checkbox"/> check = yes						
MINOR STORM Allowable Capacity is based on Depth Criterion							
MAJOR STORM Allowable Capacity is based on Depth Criterion							
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>cfs</th> </tr> <tr> <td>$Q_{allow} = 17.8$</td> <td>$Q_{allow} = 17.8$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	cfs	$Q_{allow} = 17.8$	$Q_{allow} = 17.8$	
Minor Storm	Major Storm	cfs					
$Q_{allow} = 17.8$	$Q_{allow} = 17.8$						
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'							

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

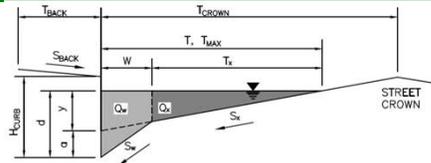


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	10.00	10.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity*			
Total Inlet Interception Capacity	3.1	5.4	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.0	0.7	cfs
Capture Percentage = Q_i/Q_o =	100	88	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

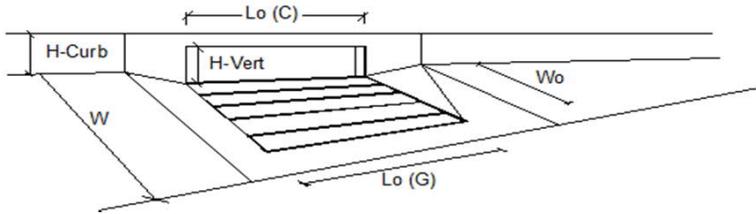
Project: Homestead North
 Inlet ID: Inlet DP 6D



Gutter Geometry (Enter data in the blue cells)							
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 20.0$ ft						
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft						
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.016$						
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches						
Distance from Curb Face to Street Crown	$T_{CROWN} = 33.0$ ft						
Gutter Width	$W = 2.00$ ft						
Street Transverse Slope	$S_x = 0.020$ ft/ft						
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_w = 0.083$ ft/ft						
Street Longitudinal Slope - Enter 0 for sump condition	$S_o = 0.030$ ft/ft						
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$						
Max. Allowable Spread for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>ft</th> </tr> <tr> <td>$T_{MAX} = 33.0$</td> <td>$T_{MAX} = 33.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	ft	$T_{MAX} = 33.0$	$T_{MAX} = 33.0$	
Minor Storm	Major Storm	ft					
$T_{MAX} = 33.0$	$T_{MAX} = 33.0$						
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>inches</th> </tr> <tr> <td>$d_{MAX} = 6.0$</td> <td>$d_{MAX} = 6.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	inches	$d_{MAX} = 6.0$	$d_{MAX} = 6.0$	
Minor Storm	Major Storm	inches					
$d_{MAX} = 6.0$	$d_{MAX} = 6.0$						
Allow Flow Depth at Street Crown (leave blank for no)	<input type="checkbox"/> <input type="checkbox"/> check = yes						
MINOR STORM Allowable Capacity is based on Depth Criterion							
MAJOR STORM Allowable Capacity is based on Depth Criterion							
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>cfs</th> </tr> <tr> <td>$Q_{allow} = 17.8$</td> <td>$Q_{allow} = 17.8$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	cfs	$Q_{allow} = 17.8$	$Q_{allow} = 17.8$	
Minor Storm	Major Storm	cfs					
$Q_{allow} = 17.8$	$Q_{allow} = 17.8$						
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'							

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

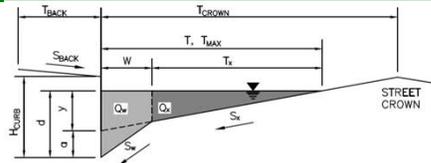


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	10.00	10.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity*			
Total Inlet Interception Capacity	2.5	4.8	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.0	0.4	cfs
Capture Percentage = Q_i/Q_o =	100	93	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

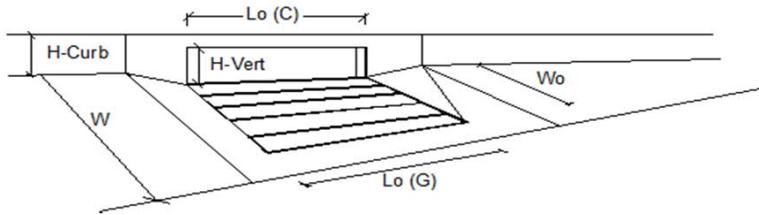
Project: Homestead North
 Inlet ID: Inlet DP 7D



Gutter Geometry (Enter data in the blue cells)							
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 15.0$ ft						
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft						
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.016$						
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches						
Distance from Curb Face to Street Crown	$T_{CROWN} = 33.0$ ft						
Gutter Width	$W = 2.00$ ft						
Street Transverse Slope	$S_x = 0.020$ ft/ft						
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_w = 0.083$ ft/ft						
Street Longitudinal Slope - Enter 0 for sump condition	$S_o = 0.020$ ft/ft						
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$						
Max. Allowable Spread for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>ft</th> </tr> <tr> <td>$T_{MAX} = 33.0$</td> <td>$T_{MAX} = 33.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	ft	$T_{MAX} = 33.0$	$T_{MAX} = 33.0$	
Minor Storm	Major Storm	ft					
$T_{MAX} = 33.0$	$T_{MAX} = 33.0$						
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>inches</th> </tr> <tr> <td>$d_{MAX} = 6.0$</td> <td>$d_{MAX} = 6.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	inches	$d_{MAX} = 6.0$	$d_{MAX} = 6.0$	
Minor Storm	Major Storm	inches					
$d_{MAX} = 6.0$	$d_{MAX} = 6.0$						
Allow Flow Depth at Street Crown (leave blank for no)	<input type="checkbox"/> <input type="checkbox"/> check = yes						
MINOR STORM Allowable Capacity is based on Depth Criterion							
MAJOR STORM Allowable Capacity is based on Depth Criterion							
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>cfs</th> </tr> <tr> <td>$Q_{allow} = 19.5$</td> <td>$Q_{allow} = 19.5$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	cfs	$Q_{allow} = 19.5$	$Q_{allow} = 19.5$	
Minor Storm	Major Storm	cfs					
$Q_{allow} = 19.5$	$Q_{allow} = 19.5$						
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'							

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017

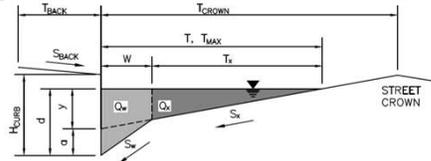


Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	10.00	10.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity*			
Total Inlet Interception Capacity	2.0	3.3	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.0	0.0	cfs
Capture Percentage = Q_i/Q_o =	100	100	%

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

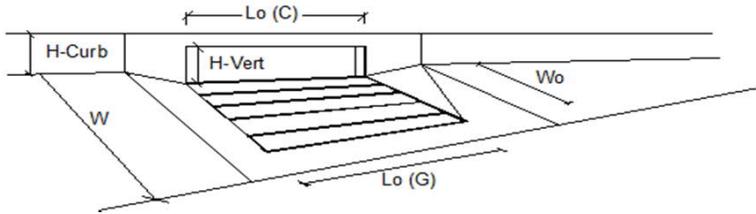
Project: Homestead North
 Inlet ID: Inlet DP 8D



Gutter Geometry (Enter data in the blue cells)							
Maximum Allowable Width for Spread Behind Curb	$T_{BACK} = 15.0$ ft						
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	$S_{BACK} = 0.020$ ft/ft						
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	$n_{BACK} = 0.013$						
Height of Curb at Gutter Flow Line	$H_{CURB} = 6.00$ inches						
Distance from Curb Face to Street Crown	$T_{CROWN} = 33.0$ ft						
Gutter Width	$W = 2.00$ ft						
Street Transverse Slope	$S_x = 0.020$ ft/ft						
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	$S_w = 0.083$ ft/ft						
Street Longitudinal Slope - Enter 0 for sump condition	$S_o = 0.020$ ft/ft						
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	$n_{STREET} = 0.016$						
Max. Allowable Spread for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>ft</th> </tr> <tr> <td>$T_{MAX} = 33.0$</td> <td>$T_{MAX} = 33.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	ft	$T_{MAX} = 33.0$	$T_{MAX} = 33.0$	
Minor Storm	Major Storm	ft					
$T_{MAX} = 33.0$	$T_{MAX} = 33.0$						
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	<table border="1"> <tr> <th>Minor Storm</th> <th>Major Storm</th> <th>inches</th> </tr> <tr> <td>$d_{MAX} = 6.0$</td> <td>$d_{MAX} = 6.0$</td> <td></td> </tr> </table>	Minor Storm	Major Storm	inches	$d_{MAX} = 6.0$	$d_{MAX} = 6.0$	
Minor Storm	Major Storm	inches					
$d_{MAX} = 6.0$	$d_{MAX} = 6.0$						
Allow Flow Depth at Street Crown (leave blank for no)	<input type="checkbox"/> <input type="checkbox"/> check = yes						
MINOR STORM Allowable Capacity is based on Depth Criterion							
MAJOR STORM Allowable Capacity is based on Depth Criterion							
Minor storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'	$Q_{allow} = 19.5$ cfs						
Major storm max. allowable capacity GOOD - greater than the design flow given on sheet 'Inlet Management'	$Q_{allow} = 19.5$ cfs						

INLET ON A CONTINUOUS GRADE

Version 4.05 Released March 2017



Design Information (Input)	MINOR	MAJOR	
Type of Inlet	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	20.00	20.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity*			
Total Inlet Interception Capacity	2.5	13.6	cfs
Total Inlet Carry-Over Flow (flow bypassing inlet)	0.0	0.7	cfs
Capture Percentage = Q_i/Q_o =	100	95	%

Channel Report

Interim Channel Section - AA

Trapezoidal

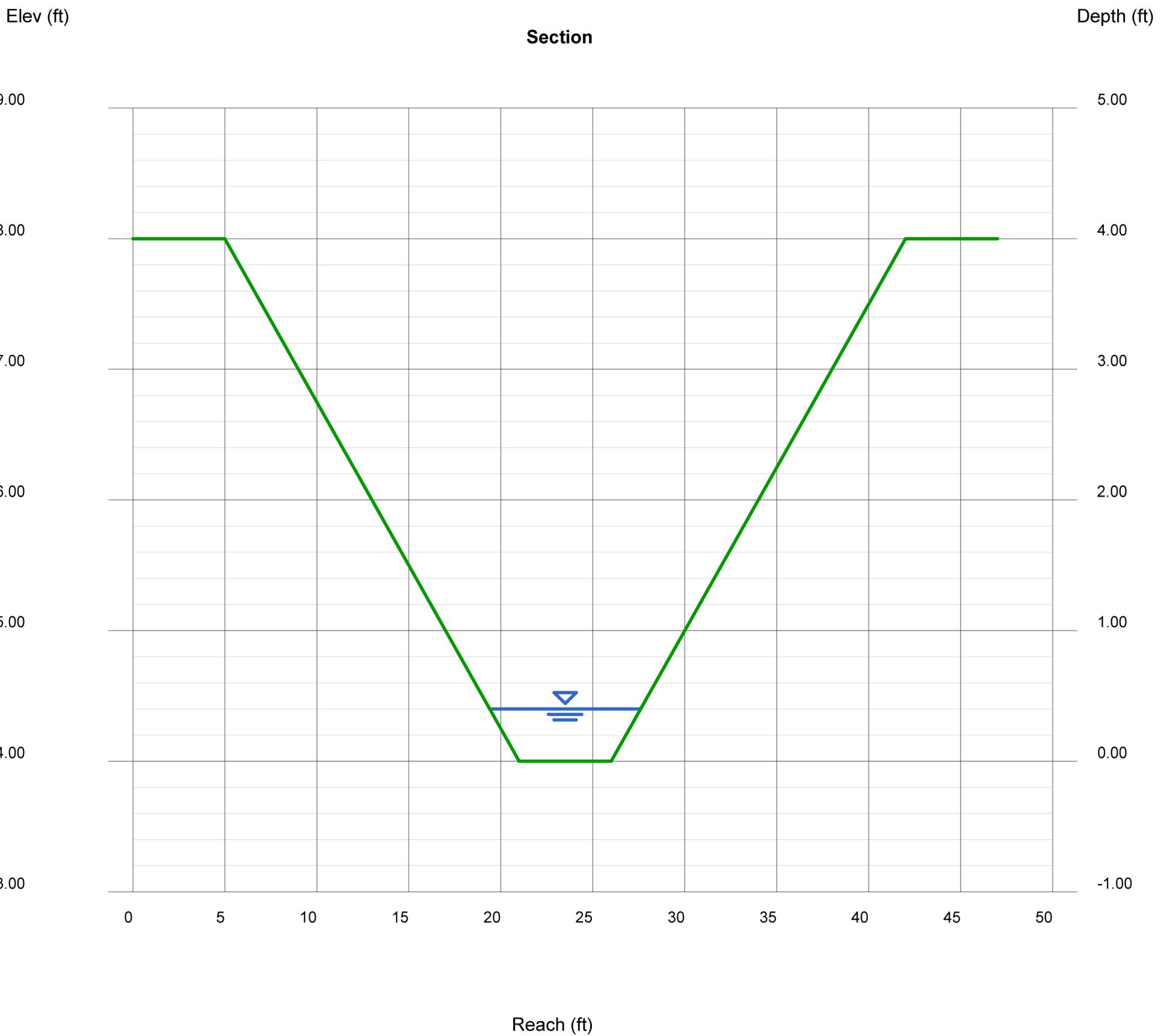
Bottom Width (ft) = 5.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 4.00
Invert Elev (ft) = 7134.00
Slope (%) = 2.62
N-Value = 0.040

Highlighted

Depth (ft) = 0.40
Q (cfs) = 7.100
Area (sqft) = 2.64
Velocity (ft/s) = 2.69
Wetted Perim (ft) = 8.30
Crit Depth, Yc (ft) = 0.36
Top Width (ft) = 8.20
EGL (ft) = 0.51

Calculations

Compute by: Known Q
Known Q (cfs) = 7.10



Channel Report

Interim Channel Section - BB

Trapezoidal

Bottom Width (ft) = 5.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 4.00
Invert Elev (ft) = 7098.00
Slope (%) = 2.00
N-Value = 0.040

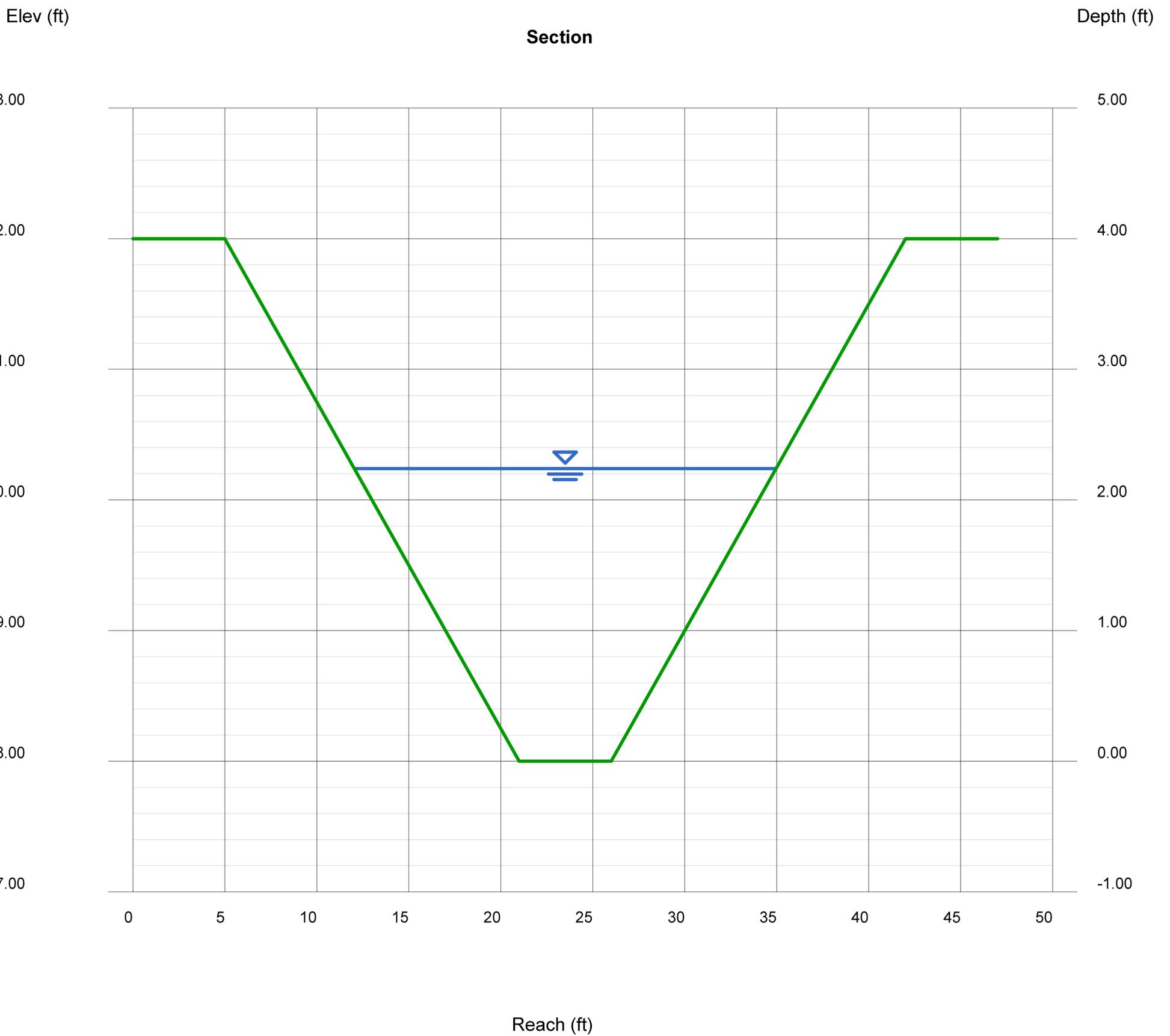
Calculations

Compute by: Known Q
Known Q (cfs) = 197.30

Highlighted

Depth (ft) = 2.24
Q (cfs) = 197.30
Area (sqft) = 31.27
Velocity (ft/s) = 6.31
Wetted Perim (ft) = 23.47
Crit Depth, Yc (ft) = 2.19
Top Width (ft) = 22.92
EGL (ft) = 2.86

State what lining is proposed



Channel Report

Interim Channel Section - CC

Trapezoidal

Bottom Width (ft) = 5.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 4.00
Invert Elev (ft) = 7010.00
Slope (%) = 2.18
N-Value = 0.040

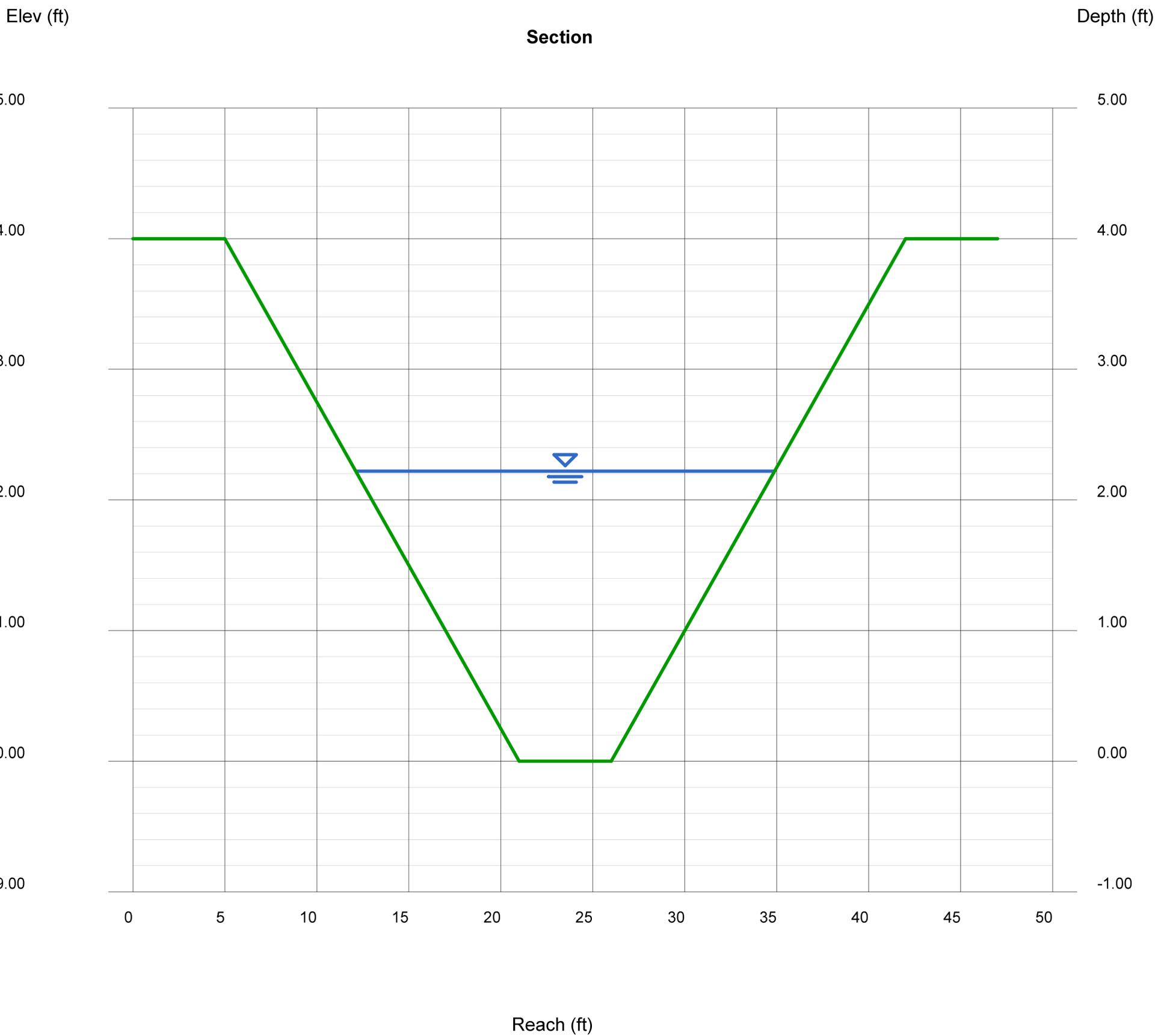
Calculations

Compute by: Known Q
Known Q (cfs) = 202.60

Highlighted

Depth (ft) = 2.22
Q (cfs) = 202.60
Area (sqft) = 30.81
Velocity (ft/s) = 6.58
Wetted Perim (ft) = 23.31
Crit Depth, Yc (ft) = 2.22
Top Width (ft) = 22.76
EGL (ft) = 2.89

State what lining is proposed



Channel Report

Interim Channel Section - DD

Trapezoidal

Bottom Width (ft) = 5.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 4.00
Invert Elev (ft) = 7057.00
Slope (%) = 3.65
N-Value = 0.040

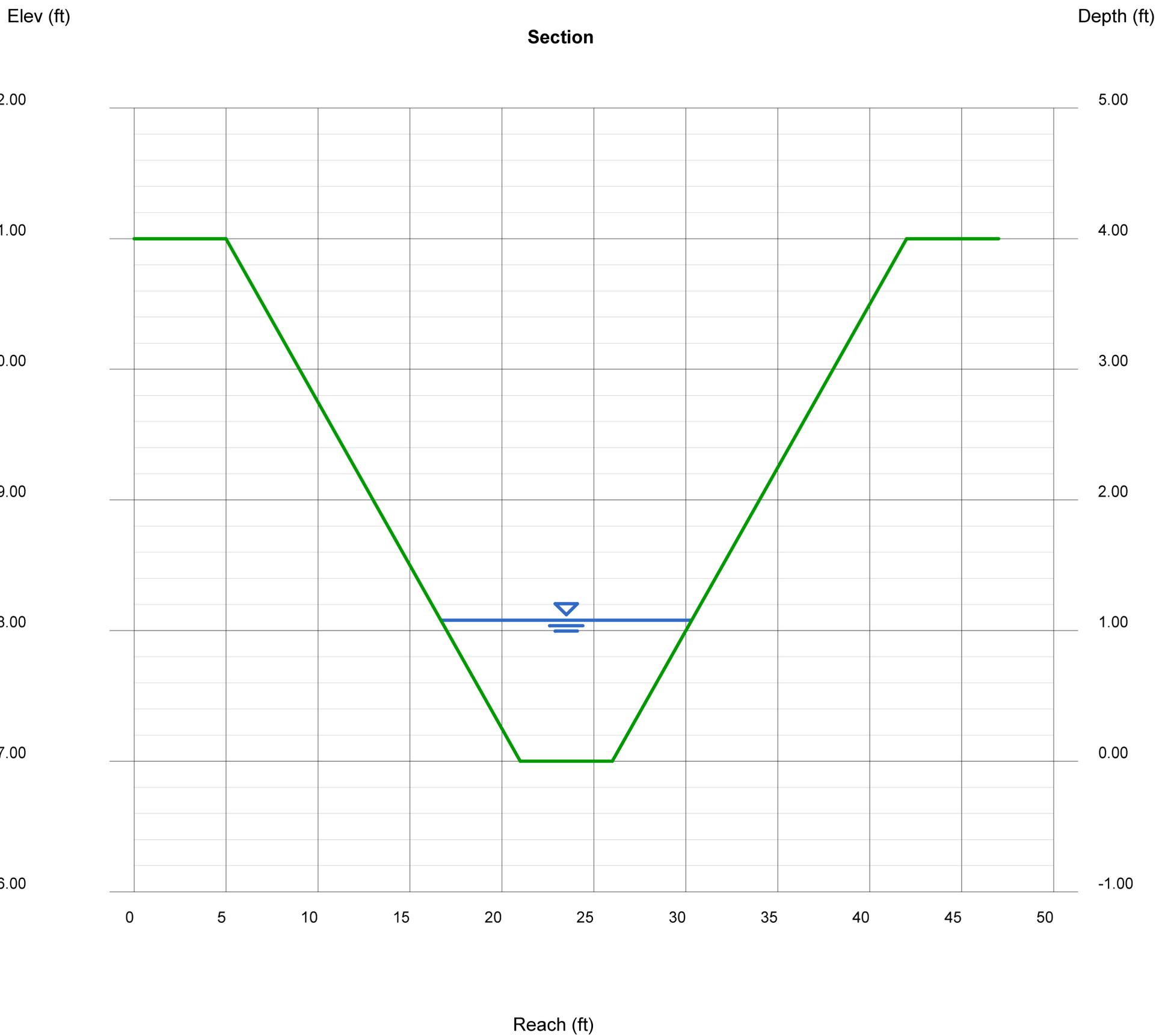
Calculations

Compute by: Known Q
Known Q (cfs) = 57.10

Highlighted

Depth (ft) = 1.08
Q (cfs) = 57.10
Area (sqft) = 10.07
Velocity (ft/s) = 5.67
Wetted Perim (ft) = 13.91
Crit Depth, Yc (ft) = 1.18
Top Width (ft) = 13.64
EGL (ft) = 1.58

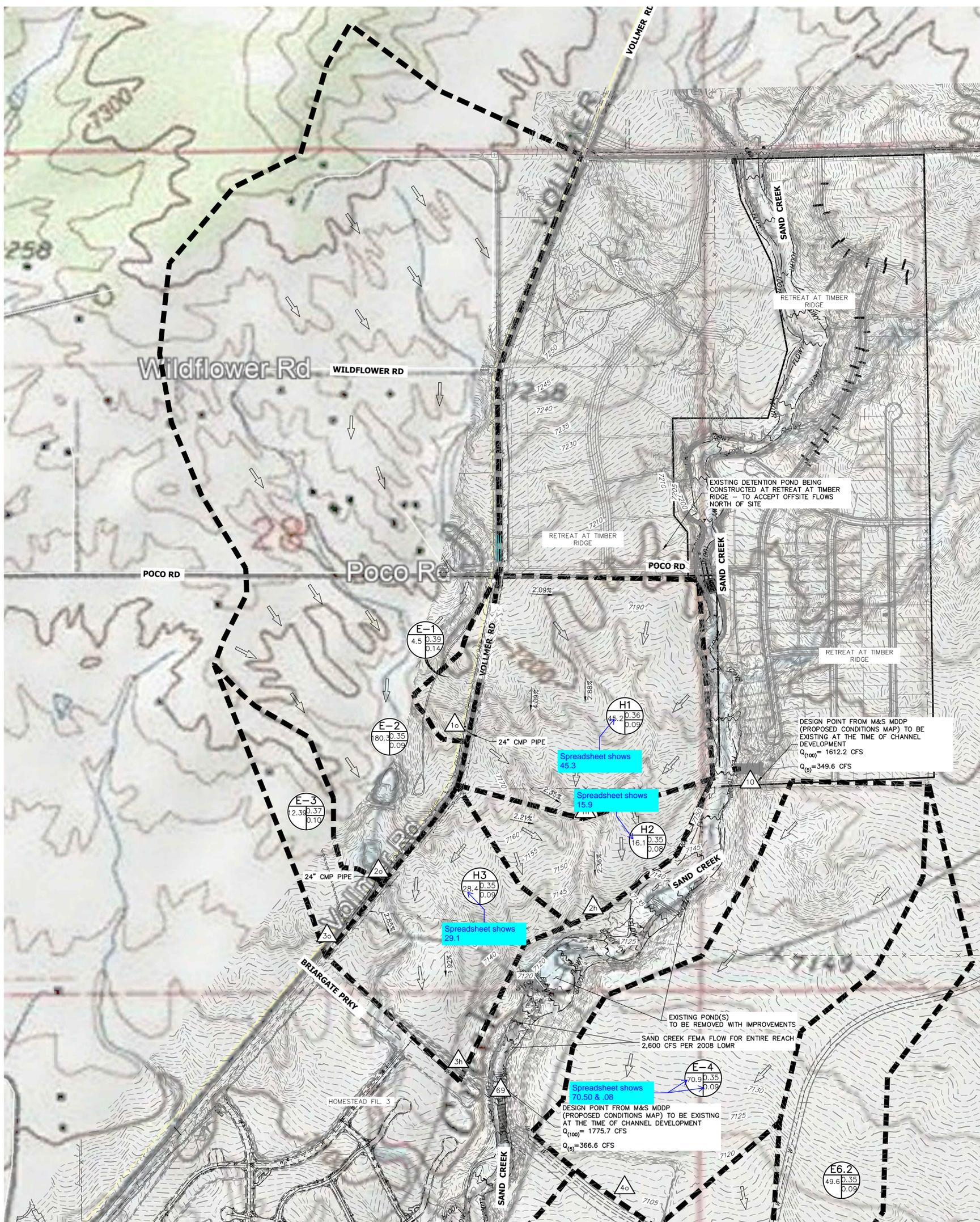
State what lining is proposed



Appendix D

Drainage Maps

EXISTING DRAINAGE MAP HOMESTEAD NORTH



SEE SHEET 2

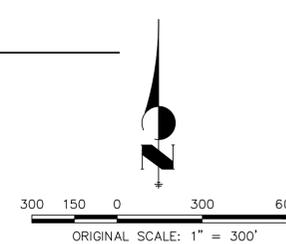
BASIN SUMMARY TABLE							
Tributary Sub-basin	Area (acres)	Percent Impervious	C _s	C ₁₀₀	t _c (min)	Q _s (cfs)	Q ₁₀₀ (cfs)
E-1	4.50	9%	0.14	0.39	48.7	1.1	5.2
E-2	180.30	3%	0.09	0.35	47.4	28.1	192.9
E-3	12.39	4%	0.10	0.37	46.9	2.2	13.7
E-4	70.50	2%	0.08	0.35	49.0	9.9	72.3
E-5	18.80	2%	0.08	0.35	34.9	3.4	24.9
E6.1	125.30	2%	0.08	0.35	48.1	17.8	130.4
E6.2	49.61	2%	0.08	0.35	44.2	7.5	55.4
H1	45.30	3%	0.09	0.36	34.7	8.9	61.1
H2	15.90	2%	0.08	0.35	25.1	3.5	25.7
H3	29.10	3%	0.09	0.35	31.3	6.1	41.8

DESIGN POINT		
DP	Q ₅ Total	Q ₁₀₀ Total
1h	8.0	52.5
2h	10.2	68.9
3h	32.6	224.0
1o	1.1	5.2
2o	28.1	192.9
3o	2.2	13.7
4o	9.9	72.3
5o	12.5	91.6
6.2o	7.5	55.4
6.1o	36.9	270.9

LEGEND

- BASIN ID
A: BASIN LABEL
B: AREA
C: C - 100 YR
D: C - 5 YR
- DESIGN POINT
- EXISTING FLOW DIRECTION
- BASIN DRAINAGE AREA
- EXISTING STORM SEWER
- EXISTING PROPERTY LINE
- ROW EXISTING
- FL EXISTING
- SIDEWALK EXISTING
- DRAINAGE ACCESS & MAINTENANCE EASEMENT

EXISTING



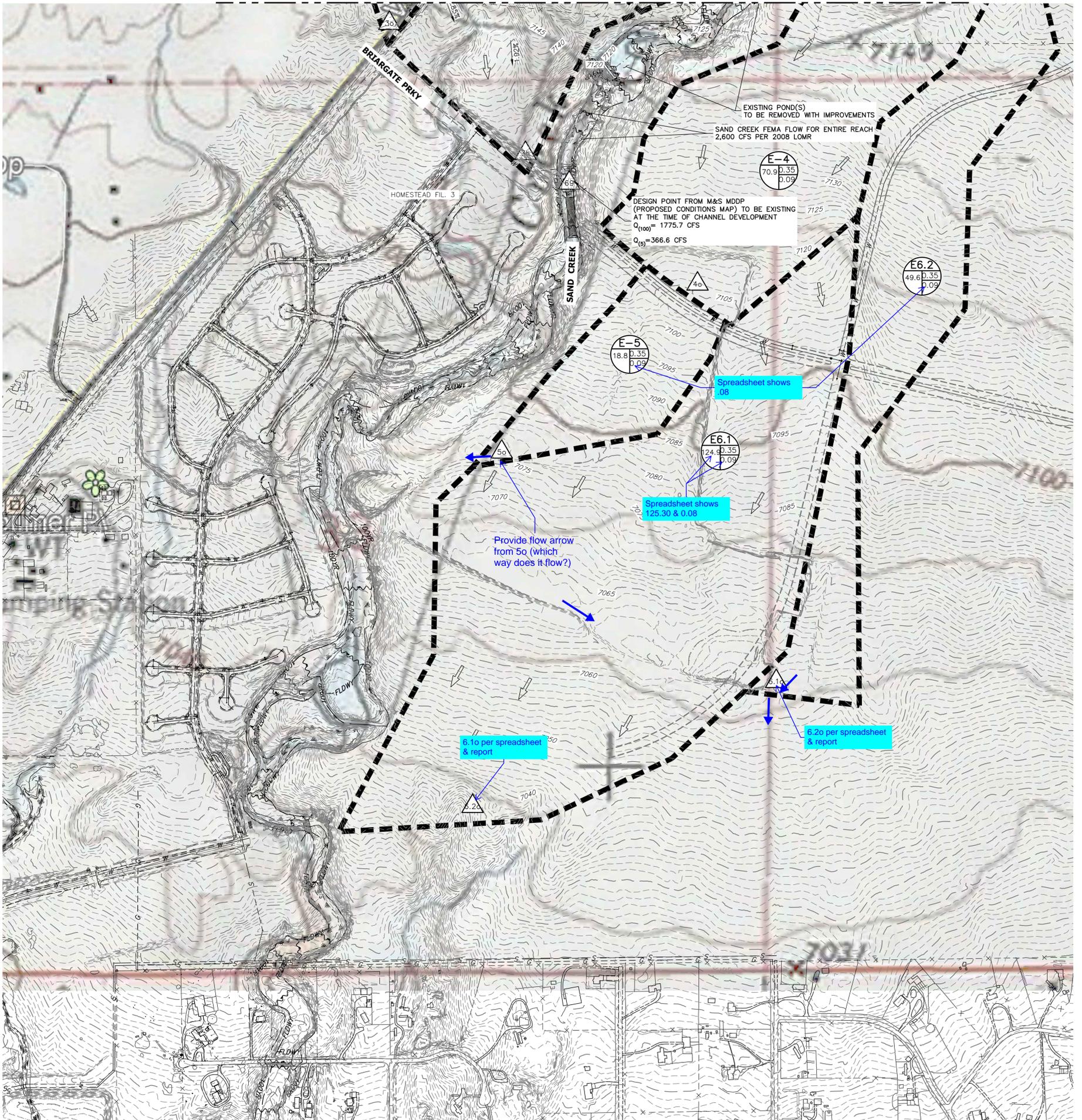
EX DRAINAGE MAP
HOMESTEAD NORTH
JOB NO. 25188.00
9-25-2021
SHEET 1 OF 2

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A Westrian Company

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EXISTING DRAINAGE MAP HOMESTEAD NORTH

SEE SHEET 1



Tributary Sub-basin	Area (acres)	Percent Impervious	C _s	C ₁₀₀	t _c (min)	Q _s (cfs)	Q ₁₀₀ (cfs)
E-1	4.50	9%	0.14	0.39	48.7	1.1	5.2
E-2	180.30	3%	0.09	0.35	47.4	28.1	192.9
E-3	12.39	4%	0.10	0.37	46.9	2.2	13.7
E-4	70.50	2%	0.08	0.35	49.0	9.9	72.3
E-5	18.80	2%	0.08	0.35	34.9	3.4	24.9
E6.1	125.30	2%	0.08	0.35	48.1	17.8	130.4
E6.2	49.61	2%	0.08	0.35	44.2	7.5	55.4
H1	45.30	3%	0.09	0.36	34.7	8.9	61.1
H2	15.90	2%	0.08	0.35	25.1	3.5	25.7
H3	29.10	3%	0.09	0.35	31.3	6.1	41.8

DP	Q5	Q100
	Total	Total
1h	8.0	52.5
2h	10.2	68.9
3h	32.6	224.0
1o	1.1	5.2
2o	28.1	192.9
3o	2.2	13.7
4o	9.9	72.3
5o	12.5	91.6
6.2o	7.5	55.4
6.1o	36.9	270.9

LEGEND

BASIN ID
A: BASIN LABEL
B: AREA
C: C - 100 YR
D: C - 5 YR



DESIGN POINT
EXISTING FLOW DIRECTION



BASIN DRAINAGE AREA



EXISTING STORM SEWER



EXISTING PROPERTY LINE



ROW EXISTING



FL EXISTING



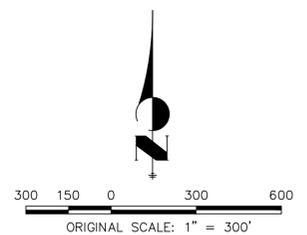
SIDEWALK EXISTING



DRAINAGE ACCESS & MAINTENANCE EASEMENT



EXISTING



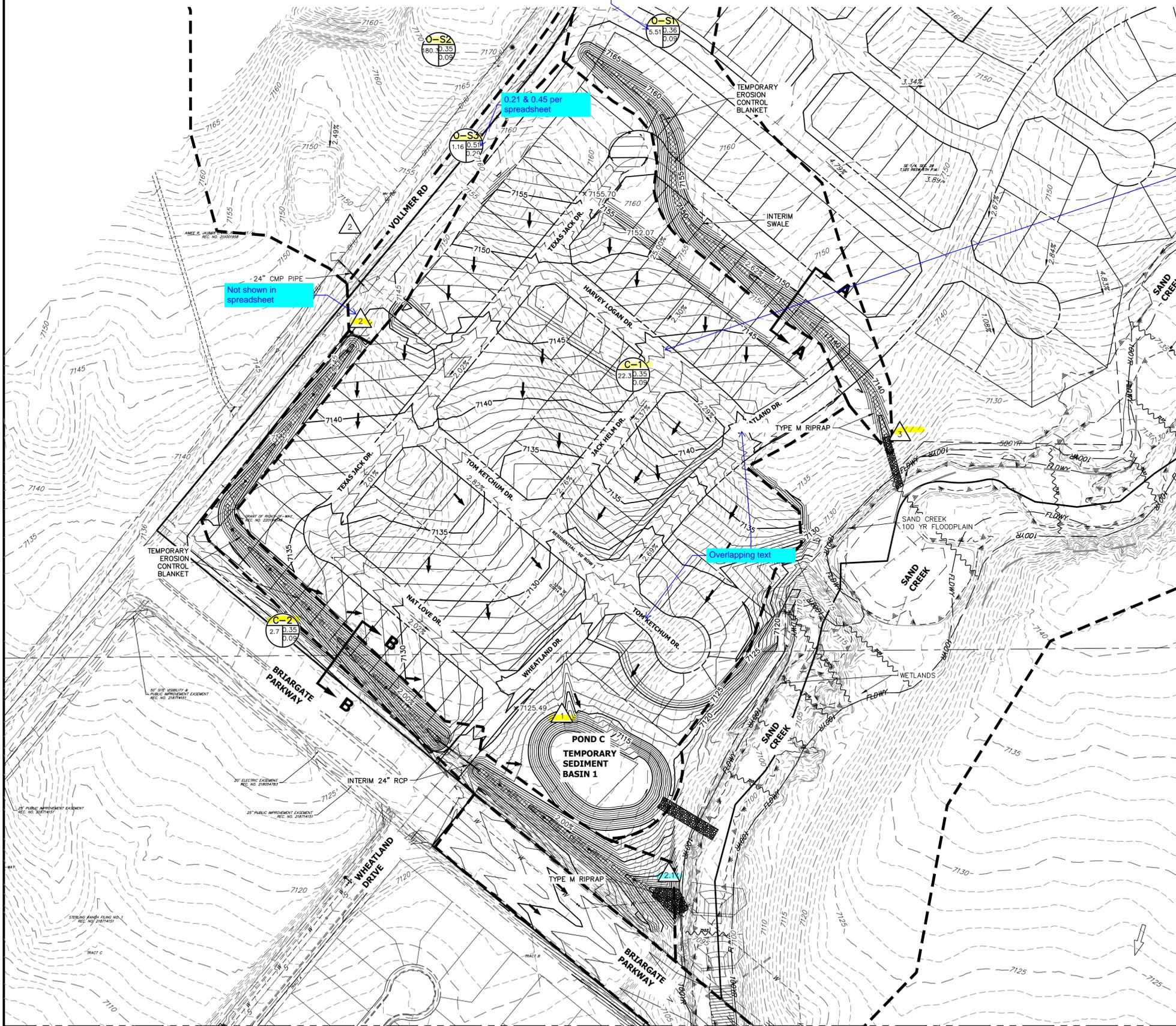
EX DRAINAGE MAP
HOMESTEAD NORTH
JOB NO. 25188.00
9-15-2021
SHEET 2 OF 2

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A Westrian Company

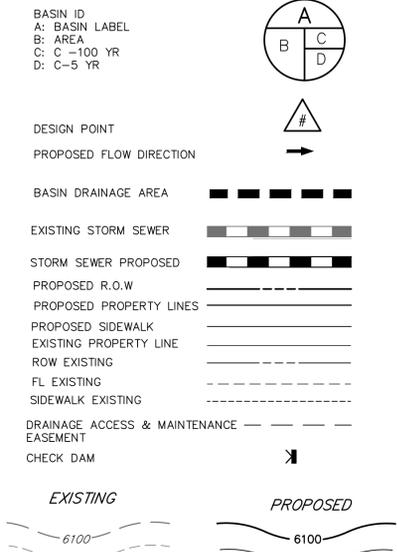
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EARLY GRADING - DRAINAGE MAP

5.40 in spreadsheet



LEGEND



SEDIMENT BASIN - SUMMARY TABLE

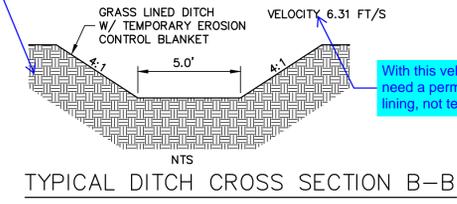
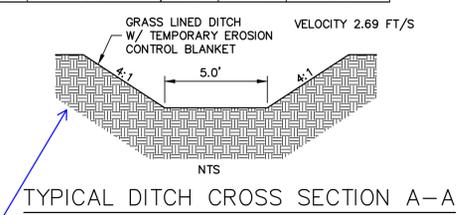
Temporary Sediment Basin	Contributing On-site Basin	Area (acres)	Percent Impervious	Contributing Off-site Basin	Area (acres)	Percent Impervious	Required Volume (cf)	Provided Volume (cf)
1	C-1	22.30	2%			2%	80,280	108,900
2	D	17.29	2%	OS, O-S4, O-S5, OS-6	253.39	2%	188,939	201,393

BASIN - SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C _s	C ₁₀₀	t _c (min)	Q _s (cfs)	Q ₁₀₀ (cfs)
C-1	22.30	2%	0.08	0.35	40.1	3.6	26.8
C-2	2.66	2%	0.08	0.35	24.9	0.6	4.3
D	17.29	2%	0.08	0.35	46.3	2.5	18.6
OS	124.20	2%	0.08	0.35	64.5	13.3	97.2
O-S1	5.40	4%	0.09	0.36	36.5	1.1	7.1
O-S2	180.30	3%	0.09	0.35	47.4	28.1	192.9
O-S3	1.16	18%	0.21	0.45	14.0	0.9	3.2
O-S4	69.42	2%	0.08	0.35	34.0	12.7	93.5
O-S5	8.57	2%	0.08	0.35	12.8	2.6	18.9
O-S6	51.20	2%	0.08	0.35	44.2	7.8	57.1

DESIGN POINT

DP	Q5		Q100	
	Total	Total	Total	Total
0	28.9	216.2		
1	3.6	26.8		
2	28.5	194.5		
2.1	28.9	197.3		
3	1.1	7.1		
4	12.7	93.5		
5	2.6	18.9		
6	7.8	57.1		
7	27.7	202.6		
8	2.5	18.6		



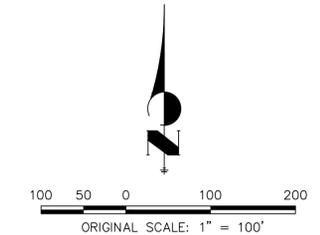
Do not show curb and gutter on grading

Overlapping text

Include a minimum depth on all swale details

With this velocity, need a permanent lining, not temporary

SEE SHEET 2

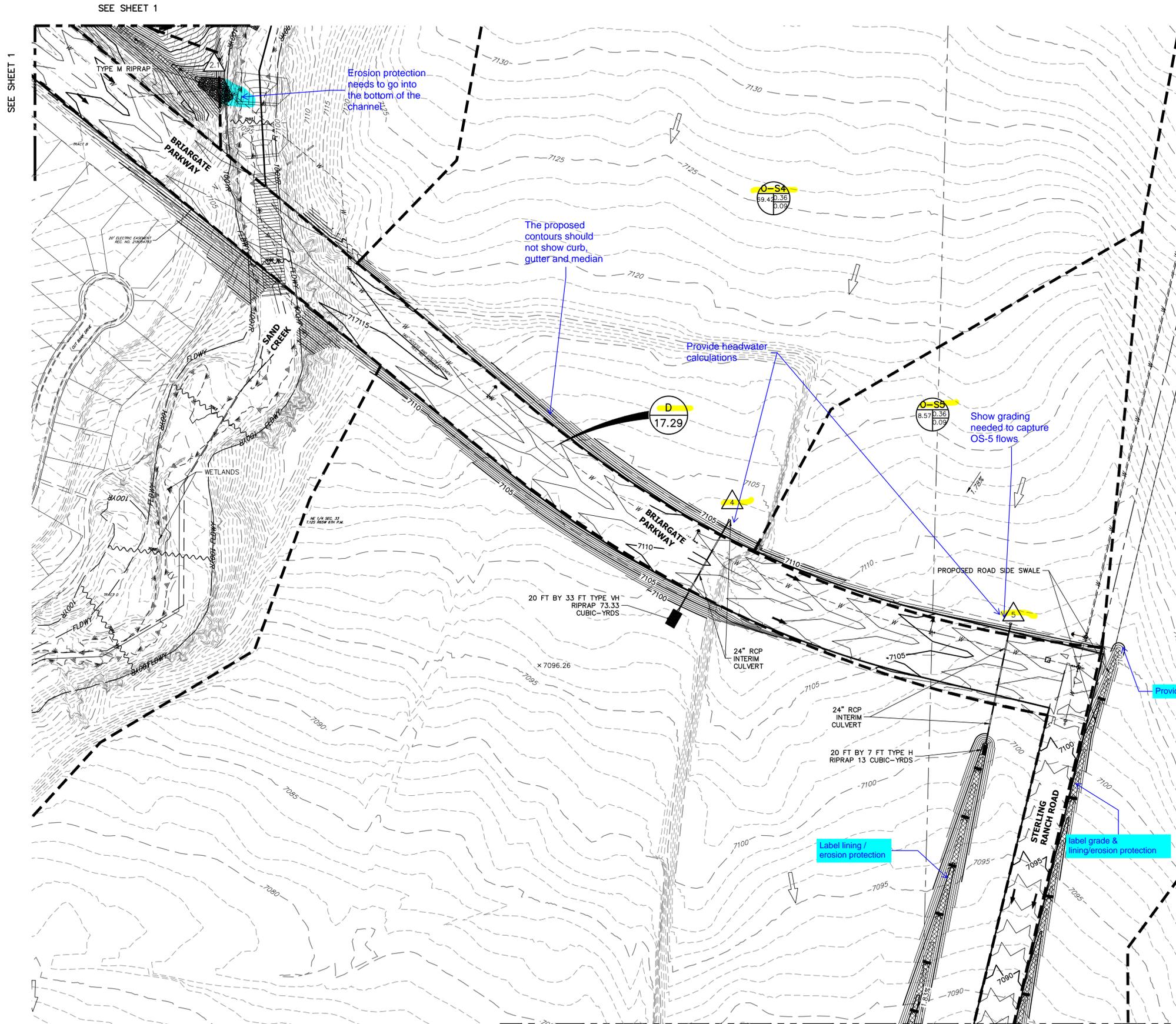


EARLY GRADING - DRAINAGE MAP
 HOMESTEAD NORTH
 JOB NO. 25188.00
 09/16/21
 SHEET 1 OF 4



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EARLY GRADING - DRAINAGE MAP



LEGEND

BASIN ID
 A: BASIN LABEL
 B: AREA
 C: C - 100 YR
 D: C-5 YR

DESIGN POINT
 PROPOSED FLOW DIRECTION

BASIN DRAINAGE AREA

EXISTING STORM SEWER

STORM SEWER PROPOSED

PROPOSED R.O.W

PROPOSED PROPERTY LINES

PROPOSED SIDEWALK

EXISTING PROPERTY LINE

ROW EXISTING

FL EXISTING

SIDEWALK EXISTING

DRAINAGE ACCESS & MAINTENANCE EASEMENT

CHECK DAM

EXISTING
 PROPOSED

SEDIMENT BASIN - SUMMARY TABLE

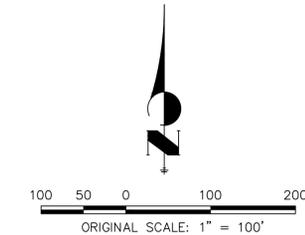
Temporary Sediment Basin	Contributing On-site Basin	Area (acres)	Percent Impervious	Contributing Off-site Basin	Area (acres)	Percent Impervious	Required Volume (cf)	Provided Volume (cf)
1	C-1	22.30	2%			2%	80,280	108,900
2	D	17.29	2%	OS, O-S4, O-S5, OS-6	253.39	2%	188,939	201,393

BASIN - SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C _s	C ₁₀₀	t _c (min)	Q _c (cfs)	Q ₁₀₀ (cfs)
C-1	22.30	2%	0.08	0.35	40.1	3.6	26.8
C-2	2.66	2%	0.08	0.35	24.9	0.6	4.3
D	17.29	2%	0.08	0.35	46.3	2.5	18.6
OS	124.20	2%	0.08	0.35	64.5	13.3	97.2
O-S1	5.40	4%	0.09	0.36	36.5	1.1	7.1
O-S2	180.30	3%	0.09	0.35	47.4	28.1	192.9
O-S3	1.16	18%	0.21	0.45	14.0	0.9	3.2
O-S4	69.42	2%	0.08	0.35	34.0	12.7	93.5
O-S5	8.57	2%	0.08	0.35	12.8	2.6	18.9
O-S6	51.20	2%	0.08	0.35	44.2	7.8	57.1

DESIGN POINT

DP	Q5		Q100	
	Total	Total	Total	Total
0	28.9	216.2		
1	3.6	26.8		
2	28.5	194.5		
2.1	28.9	197.3		
3	1.1	7.1		
4	12.7	93.5		
5	2.6	18.9		
6	7.8	57.1		
7	27.7	202.6		
8	2.5	18.6		



EARLY GRADING - DRAINAGE MAP
 HOMESTEAD NORTH
 JOB NO. 25188.00
 09/16/21
 SHEET 2 OF 4



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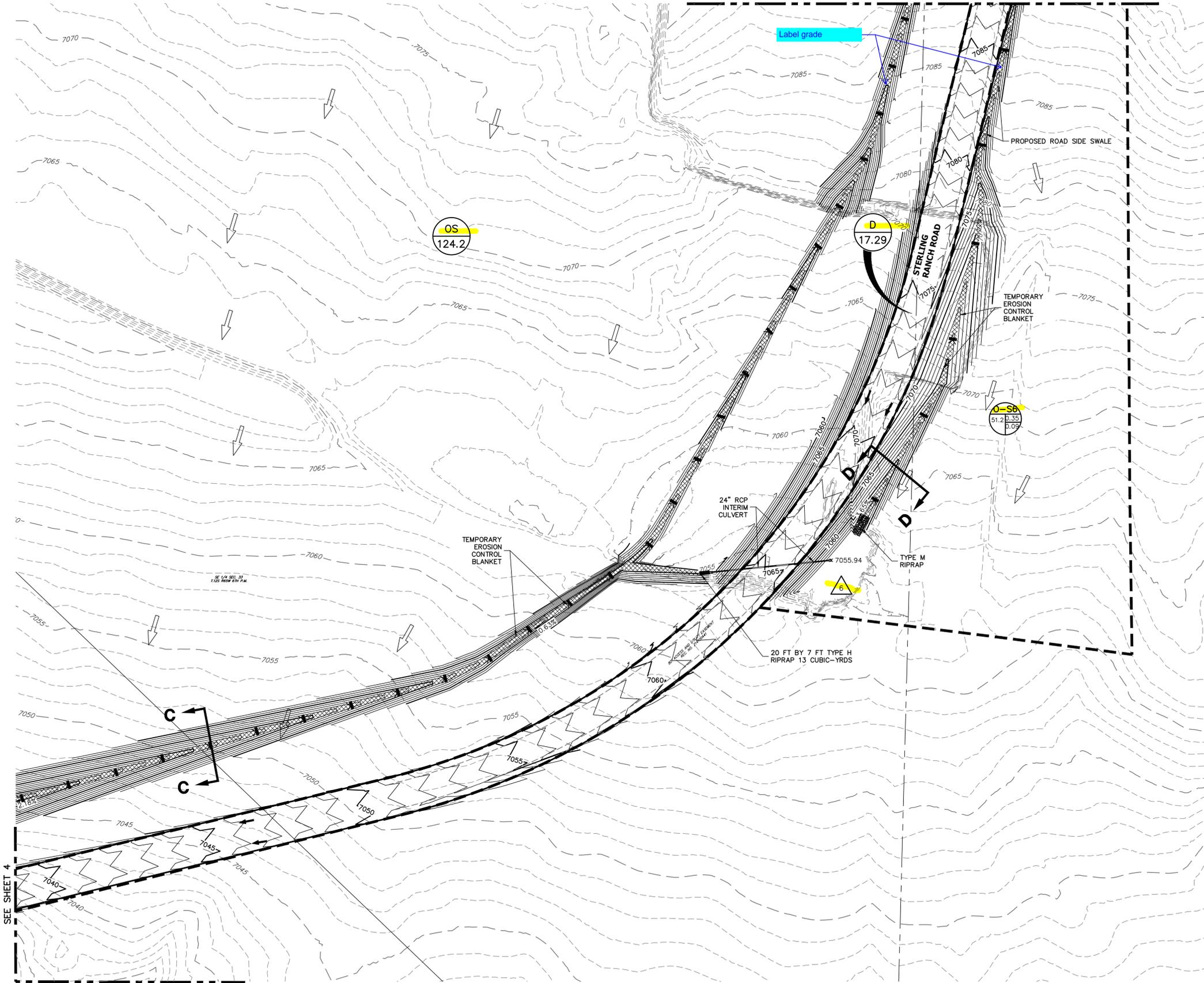
SEE SHEET 1

SEE SHEET 1

SEE SHEET 3

EARLY GRADING - DRAINAGE MAP

SEE SHEET 2



LEGEND

BASIN ID
 A: BASIN LABEL
 B: AREA
 C: C-100 YR
 D: C-5 YR

DESIGN POINT
 PROPOSED FLOW DIRECTION

BASIN DRAINAGE AREA
 EXISTING STORM SEWER
 STORM SEWER PROPOSED
 PROPOSED R.O.W
 PROPOSED PROPERTY LINES
 PROPOSED SIDEWALK
 EXISTING PROPERTY LINE
 ROW EXISTING
 FL EXISTING
 SIDEWALK EXISTING

DRAINAGE ACCESS & MAINTENANCE
 EASEMENT
 CHECK DAM

EXISTING
 PROPOSED

SEDIMENT BASIN - SUMMARY TABLE

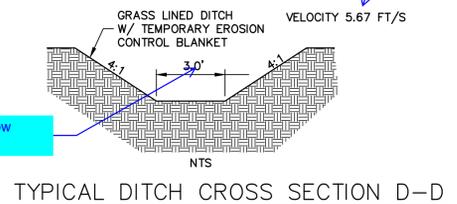
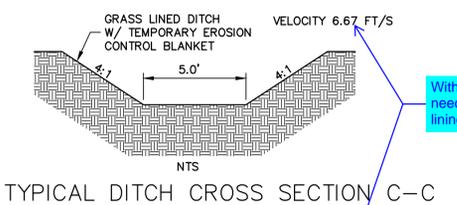
Temporary Sediment Basin	Contributing On-site Basin	Area (acres)	Percent Impervious	Contributing Off-site Basin	Area (acres)	Percent Impervious	Required Volume (cf)	Provided Volume (cf)
1	C-1	22.30	2%			2%	80,280	108,900
2	D	17.29	2%	O5,O-54,O-55,O5-6	253.39	2%	188,939	201,393

BASIN - SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C _s	C ₁₀₀	t _c (min)	Q _s (cfs)	Q ₁₀₀ (cfs)
C-1	22.30	2%	0.08	0.35	40.1	3.6	26.8
C-2	2.66	2%	0.08	0.35	24.9	0.6	4.3
D	17.29	2%	0.08	0.35	46.3	2.5	18.6
OS	124.20	2%	0.08	0.35	64.5	13.3	97.2
O-S1	5.40	4%	0.09	0.36	36.5	1.1	7.1
O-S2	180.30	3%	0.09	0.35	47.4	28.1	192.9
O-S3	1.16	18%	0.21	0.45	14.0	0.9	3.2
O-S4	69.42	2%	0.08	0.35	34.0	12.7	93.5
O-S5	8.57	2%	0.08	0.35	12.8	2.6	18.9
O-S6	51.20	2%	0.08	0.35	44.2	7.8	57.1

DESIGN POINT

DP	Q ₅ Total	Q ₁₀₀ Total
0	28.9	216.2
1	3.6	26.8
2	28.5	194.5
2.1	28.9	197.3
3	1.1	7.1
4	12.7	93.5
5	2.6	18.9
6	7.8	57.1
7	27.7	202.6
8	2.5	18.6



With this velocity, need a permanent lining, not temporary

5' per Hydraul spreadsheet

SEE SHEET 4

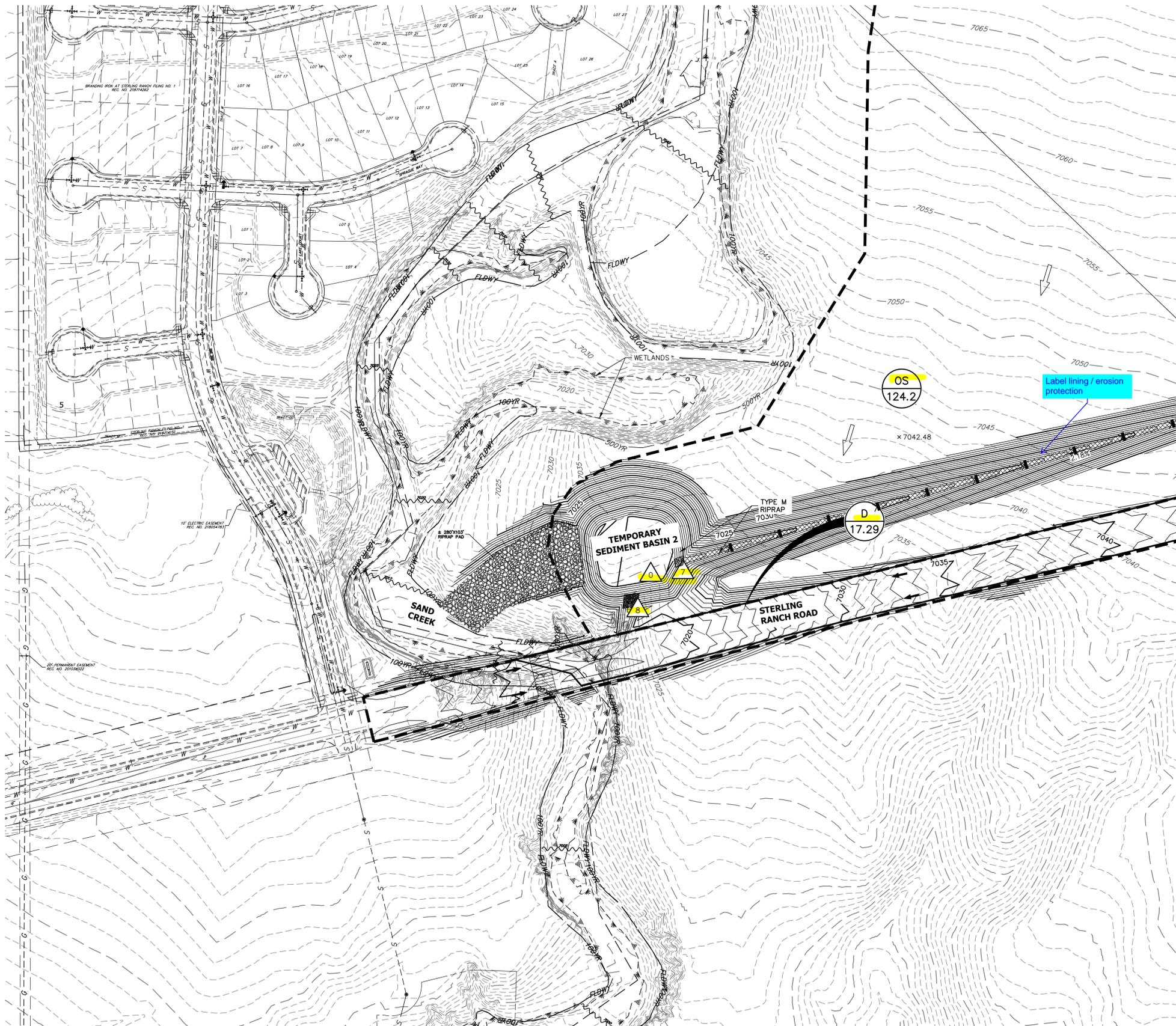
ORIGINAL SCALE: 1" = 100'

EARLY GRADING - DRAINAGE MAP
 HOMESTEAD NORTH
 JOB NO. 25188.00
 09/16/21
 SHEET 3 OF 4



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EARLY GRADING - DRAINAGE MAP



LEGEND

BASIN ID
 A: BASIN LABEL
 B: AREA
 C: C - 100 YR
 D: C - 5 YR

DESIGN POINT
 PROPOSED FLOW DIRECTION

BASIN DRAINAGE AREA
 EXISTING STORM SEWER
 STORM SEWER PROPOSED
 PROPOSED R.O.W
 PROPOSED PROPERTY LINES
 PROPOSED SIDEWALK
 EXISTING PROPERTY LINE
 ROW EXISTING
 FL EXISTING
 SIDEWALK EXISTING

DRAINAGE ACCESS & MAINTENANCE
 EASEMENT
 CHECK DAM

EXISTING
 PROPOSED

Verify size based on contributing area from east side of SRR

SEDIMENT BASIN - SUMMARY TABLE

Temporary Sediment Basin	Contributing On-site Basin	Area (acres)	Percent Impervious	Contributing Off-site Basin	Area (acres)	Percent Impervious	Required Volume (cf)	Provided Volume (cf)
1	C-1	22.30	2%			2%	80,280	108,900
2	D	17.29	2%	OS, O-S4, O-S5, OS-6	253.39	2%	188,939	201,393

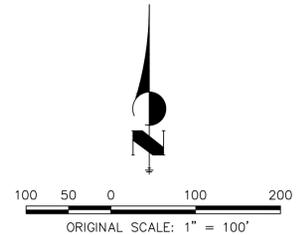
BASIN - SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C _s	C ₁₀₀	t _c (min)	Q _s (cfs)	Q ₁₀₀ (cfs)
C-1	22.30	2%	0.08	0.35	40.1	3.6	26.8
C-2	2.66	2%	0.08	0.35	24.9	0.6	4.3
D	17.29	2%	0.08	0.35	46.3	2.5	18.6
OS	124.20	2%	0.08	0.35	64.5	13.3	97.2
O-S1	5.40	4%	0.09	0.36	36.5	1.1	7.1
O-S2	180.30	3%	0.09	0.35	47.4	28.1	192.9
O-S3	1.16	18%	0.21	0.45	14.0	0.9	3.2
O-S4	69.42	2%	0.08	0.35	34.0	12.7	93.5
O-S5	8.57	2%	0.08	0.35	12.8	2.6	18.9
O-S6	51.20	2%	0.08	0.35	44.2	7.8	57.1

DESIGN POINT

DP	Q ₅ Total	Q ₁₀₀ Total
0	28.9	216.2
1	3.6	26.8
2	28.5	194.5
2.1	28.9	197.3
3	1.1	7.1
4	12.7	93.5
5	2.6	18.9
6	7.8	57.1
7	27.7	202.6
8	2.5	18.6

SEE SHEET 3



EARLY GRADING - DRAINAGE MAP
 HOMESTEAD NORTH
 JOB NO. 25188.00
 09/16/21
 SHEET 4 OF 4



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DRAINAGE MAP

LEGEND

BASIN ID
 A: BASIN LABEL
 B: AREA
 C: C-100 YR
 D: C-5 YR



DESIGN POINT
 PROPOSED FLOW DIRECTION



BASIN DRAINAGE AREA



EXISTING STORM SEWER



STORM SEWER PROPOSED



PROPOSED R.O.W



PROPOSED PROPERTY LINES
 PROPOSED SIDEWALK
 EXISTING PROPERTY LINE
 ROW EXISTING
 FL EXISTING
 SIDEWALK EXISTING
 DRAINAGE ACCESS & MAINTENANCE EASEMENT

EXISTING

PROPOSED

6100

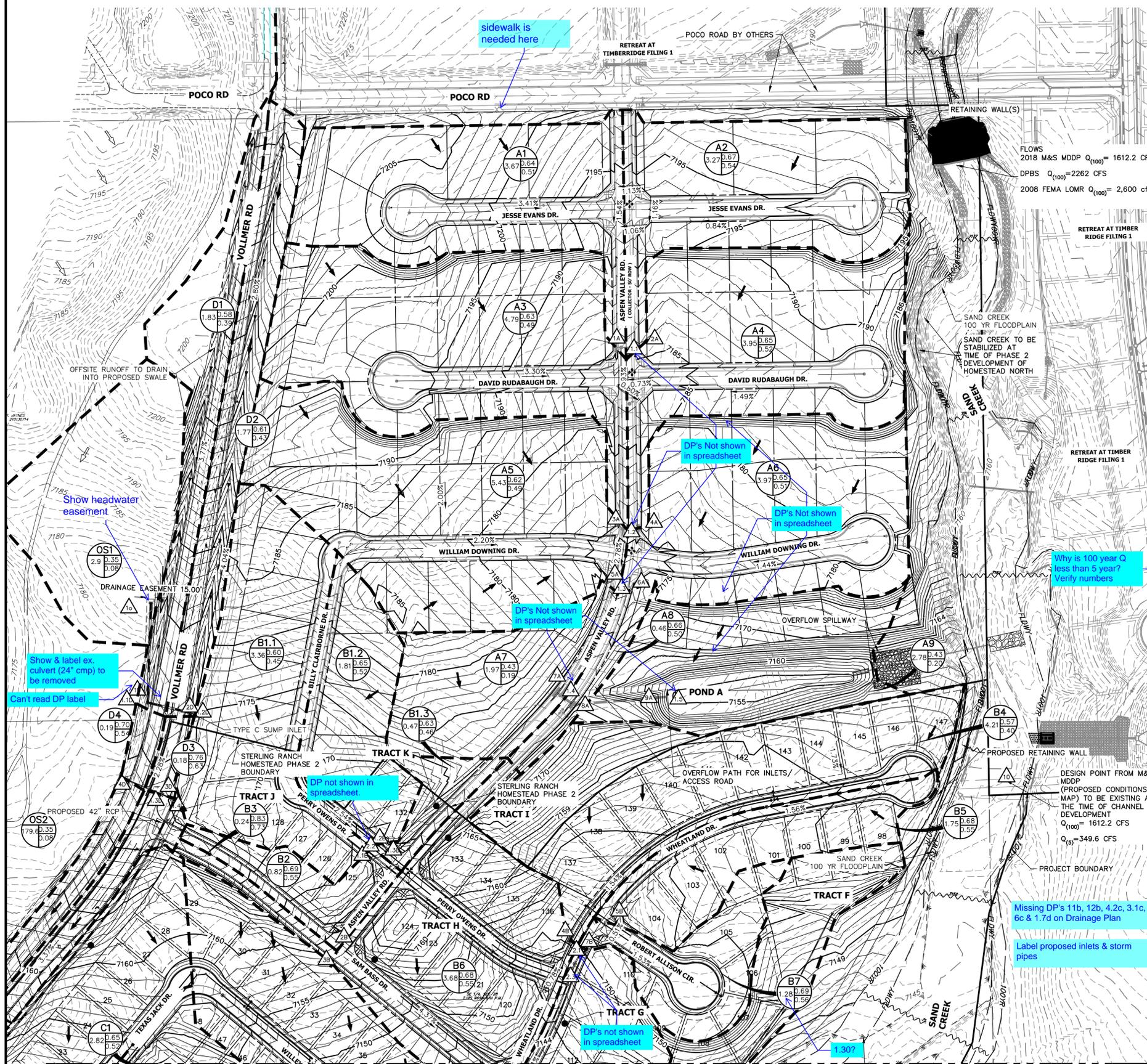
6100

DESIGN POINT SUMMARY TABLE

DP	Q5	Q100
Total	Total	
1a	6.9	14.7
2a	6.4	13.3
3a	8.3	20.5
4a	7.2	16.5
5a	9.5	26.1
6a	10.7	18.5
7a	10.4	29.9
8a	11.3	20.0
9a	21.6	49.6
1.1b	5.5	12.5
1.2b	3.5	7.4
1.3b	1.0	2.2
2b	2.4	6.8
3b	0.9	1.7
4b	7.1	16.8
5b	4.3	8.9
6b	10.3	26.5
7b	7.2	14.9
8b	5.0	13.0
9b	12.1	30.3
10b	5.7	14.3
11b	18.1	3.7
1c	5.4	11.4
2.1c	0.8	1.6
2.2c	9.8	20.1
2.3c	7.1	14.9
3.1c	1.2	2.4
3.2c	3.6	7.9
4c	18.9	42.0
4.2c	5.9	13.3
5c	4.1	8.8
6c	2.5	8.8
o1	0.8	6.0
1d	2.4	6.0
1.1d	3.2	11.6
2d	2.5	6.1
1.2d	5.7	17.7
4d	1.0	1.1
3d	0.6	1.2
1.3d	0.5	2.2
2o	27.1	190.9
5d	3.1	6.1
6d	2.5	4.6
1.4d	6.4	19.2
1.5d	29.2	195.0
3o	1.7	12.6
7d	2.0	3.3
8d	2.5	14.4
2.1d	3.2	15.5
1.6d	32.6	205.3
1.7d	35.8	220.3
OS1	2.85	2%
OS2	179.61	2%
OS3	11.99	2%

BASIN SUMMARY TABLE

Tributary	Area (acres)	Percent Impervious	C5	C100	tc (min)	Q5 (cfs)	Q100 (cfs)
A1	3.67	52%	0.51	0.64	13.3	6.9	14.7
A2	3.27	56%	0.54	0.67	13.8	6.4	13.3
A3	4.79	50%	0.49	0.63	13.9	8.5	18.4
A4	3.95	54%	0.52	0.65	14.2	7.4	15.6
A5	5.43	50%	0.49	0.62	11.1	10.5	22.6
A6	3.97	53%	0.51	0.65	12.6	7.7	16.3
A7	1.97	15%	0.19	0.43	16.5	1.3	4.8
A8	0.46	52%	0.50	0.66	5.0	1.2	2.6
A9	2.78	16%	0.20	0.43	13.4	2.1	7.4
B1.1	3.36	45%	0.45	0.60	13.4	5.5	12.5
B1.2	1.81	54%	0.52	0.65	12.8	3.5	7.4
B1.3	0.47	47%	0.46	0.63	8.1	1.0	2.2
B2	0.82	58%	0.55	0.69	5.0	2.3	4.9
B3	0.24	79%	0.73	0.83	5.0	0.9	1.7
B4	4.21	39%	0.40	0.57	9.5	7.1	16.8
B5	1.75	58%	0.55	0.68	7.8	4.3	8.9
B6	3.66	57%	0.55	0.68	6.6	9.5	19.9
B7	1.30	59%	0.56	0.69	9.0	3.1	6.4
B8	2.30	55%	0.53	0.66	9.6	5.1	10.7
B9	3.69	65%	0.50	0.64	13.1	6.9	14.8
B10	0.22	80%	0.73	0.83	5.0	0.8	1.6
B11	1.65	15%	0.16	0.40	16.7	0.9	3.7
B12	2.40	40%	0.30	0.50	39.8	1.5	4.1
C1	2.82	69%	0.52	0.65	13.1	5.4	11.4
C2.1	0.20	91%	0.82	0.90	5.0	0.8	1.6
C2.2	4.69	73%	0.56	0.68	12.8	9.9	20.3
C2.3	0.83	67%	0.54	0.68	10.1	1.9	3.9
C3.1	0.35	73%	0.68	0.79	5.0	1.2	2.4
C3.2	1.46	71%	0.56	0.68	8.4	3.6	7.4
C4.1	6.35	65%	0.49	0.63	12.0	12.1	25.9
C4.2	3.44	59%	0.46	0.61	12.6	5.9	13.3
C5	0.16	81%	0.74	0.84	6.4	0.6	1.0
C6	2.48	21%	0.22	0.45	6.8	2.5	8.8
D1	1.83	39%	0.39	0.58	16.7	2.4	6.0
D2	1.77	43%	0.43	0.61	16.3	2.5	6.1
D3	0.18	68%	0.63	0.76	5.4	0.6	1.2
D4	0.19	57%	0.54	0.70	6.3	0.5	1.1
D5	0.91	77%	0.71	0.82	6.0	3.1	6.1
D6	0.83	69%	0.64	0.77	6.4	2.5	5.2
D7	0.52	81%	0.74	0.84	5.0	2.0	3.8
D8	0.66	75%	0.69	0.81	5.0	2.4	4.6
OS1	2.85	2%	0.08	0.35	14.5	0.8	6.0
OS2	179.61	2%	0.08	0.35	47.4	27.1	190.9
OS3	11.99	2%	0.08	0.35	47.6	1.7	12.6



sidewalk is needed here

DP's Not shown in spreadsheet

DP's Not shown in spreadsheet

DP's Not shown in spreadsheet

DP not shown in spreadsheet

DP's not shown in spreadsheet

Why is 100 year Q less than 5 year? Verify numbers

Show headwater easement

Show & label ex-culvert (24" cmp) to be removed

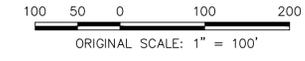
Can't read DP label

Missing DP's 11b, 12b, 4.2c, 3.1c, 6c & 1.7d on Drainage Plan

Label proposed inlets & storm pipes

Missing design points 3.5, 2.5 & 12b in table.

SEE SHEET 2



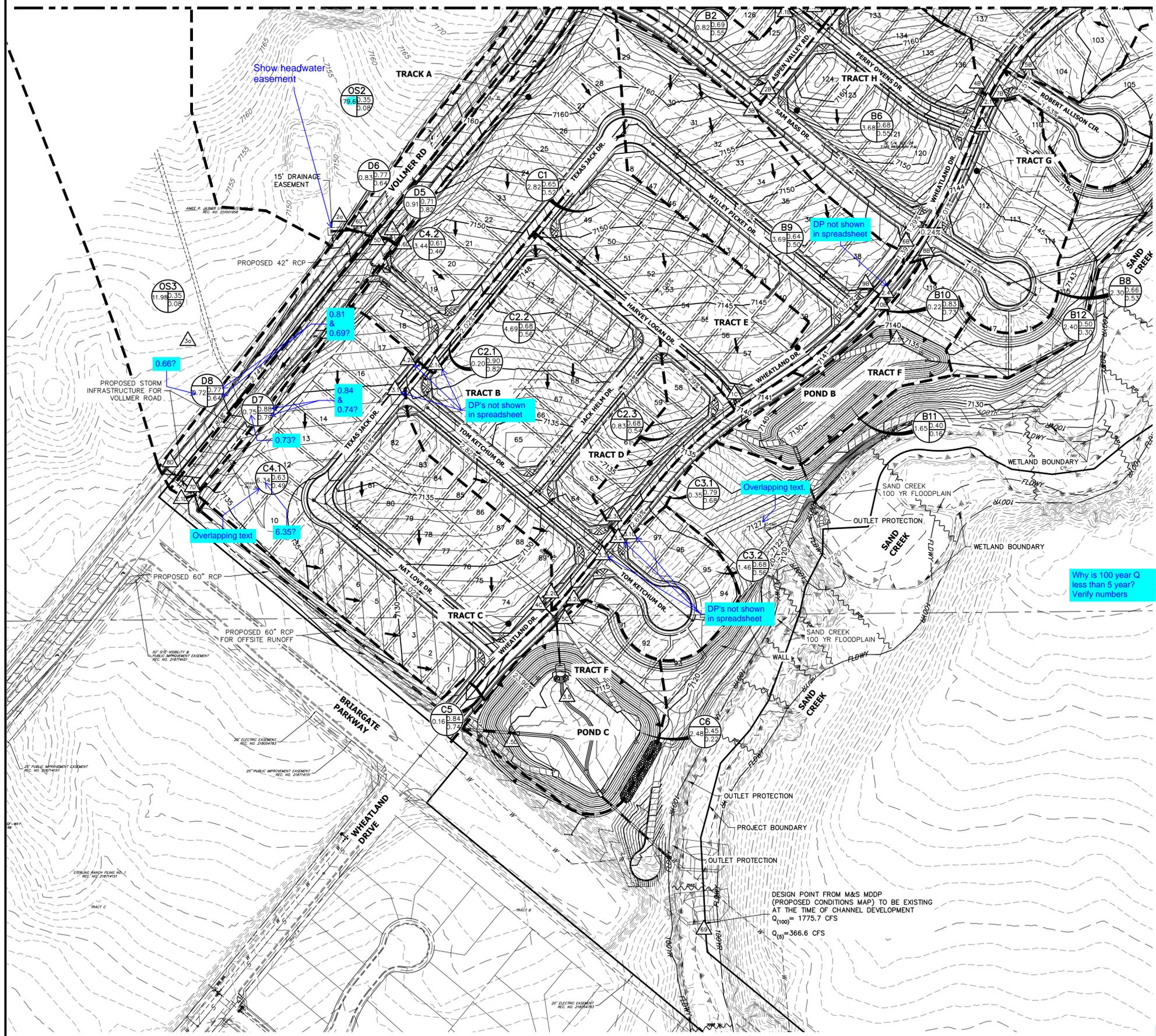
DRAINAGE MAP
 HOMESTEAD NORTH
 JOB NO. 25188.00
 10/08/21
 SHEET 1 OF 2



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DRAINAGE MAP

SEE SHEET 1



LEGEND

BASIN ID
 A: BASIN LABEL
 B: AREA
 C: C - 100 YR
 D: C - 5 YR

DESIGN POINT
 PROPOSED FLOW DIRECTION

BASIN DRAINAGE AREA
 EXISTING STORM SEWER
 STORM SEWER PROPOSED

PROPOSED R.O.W
 PROPOSED PROPERTY LINES
 PROPOSED SIDEWALK
 EXISTING PROPERTY LINE
 ROW EXISTING
 FL EXISTING
 SIDEWALK EXISTING
 DRAINAGE ACCESS & MAINTENANCE EASEMENT

EXISTING
 PROPOSED

Missing DP's 11b, 12b, 4.2c, 3.1c, 6c & 1.7d on Drainage Plan

Label proposed inlets & storm pipes

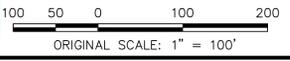
DESIGN POINT SUMMARY TABLE

DP	Q5	Q100
	Total	Total
1a	6.9	14.7
2a	6.4	13.3
3a	8.3	20.5
4a	7.2	16.5
5a	9.5	26.1
6a	10.7	18.5
7a	10.4	29.9
8a	11.3	20.0
9a	21.6	49.6
1.1b	5.5	12.5
1.2b	3.5	7.4
1.3b	1.0	2.2
2b	2.4	6.8
3b	0.9	1.7
4b	7.1	16.8
5b	4.3	8.9
6b	10.3	26.5
7b	7.2	14.9
8b	5.0	13.0
9b	12.1	30.3
10b	5.7	14.3
11b	18.1	3.7
1c	5.4	11.4
2.1c	0.8	1.6
2.2c	9.8	20.1
2.3c	7.1	14.9
3.1c	1.2	2.4
3.2c	3.6	7.9
4c	18.9	42.0
4.2c	5.9	13.3
5c	4.1	8.8
6c	2.5	8.8
o1	0.8	6.0
1d	2.4	6.0
1.1d	3.2	11.6
2d	2.5	6.1
1.2d	5.7	17.7
4d	1.0	1.1
3d	0.6	1.2
1.3d	0.5	2.2
2o	27.1	190.9
5d	3.1	6.1
6d	2.5	4.6
1.4d	6.4	19.2
1.5d	29.2	195.0
3o	1.7	12.6
7d	2.0	3.3
8d	2.5	14.4
2.1d	3.2	15.5
1.6d	32.6	205.3
1.7d	35.8	220.3

BASIN SUMMARY TABLE

Tributary Sub-basin	Area (acres)	Percent Impervious	C5	C100	tc (min)	Q5 (cfs)	Q100 (cfs)
A1	3.67	52%	0.51	0.64	13.3	6.9	14.7
A2	3.27	56%	0.54	0.67	13.8	6.4	13.3
A3	4.79	50%	0.49	0.63	13.9	8.5	18.4
A4	3.95	54%	0.52	0.65	14.2	7.4	15.6
A5	5.43	50%	0.49	0.62	11.1	10.5	22.6
A6	3.97	53%	0.51	0.65	12.6	7.7	16.3
A7	1.97	15%	0.19	0.43	16.5	1.3	4.8
A8	0.46	52%	0.50	0.66	5.0	1.2	2.6
A9	2.78	16%	0.20	0.43	13.4	2.1	7.4
B1.1	3.36	45%	0.45	0.60	13.4	5.5	12.5
B1.2	1.81	54%	0.52	0.65	12.8	3.5	7.4
B1.3	0.47	47%	0.46	0.63	8.1	1.0	2.2
B2	0.82	58%	0.55	0.69	5.0	2.3	4.9
B3	0.24	79%	0.73	0.83	5.0	0.9	1.7
B4	4.21	39%	0.40	0.57	9.5	7.1	16.8
B5	1.75	58%	0.55	0.68	7.8	4.3	8.9
B6	3.66	57%	0.55	0.68	6.6	9.5	19.9
B7	1.30	59%	0.56	0.69	9.0	3.1	6.4
B8	2.30	55%	0.53	0.66	9.6	5.1	10.7
B9	3.69	65%	0.50	0.64	13.1	6.9	14.8
B10	0.22	80%	0.73	0.83	5.0	0.8	1.6
B11	1.65	15%	0.16	0.40	16.7	0.9	3.7
B12	2.40	40%	0.30	0.50	39.8	1.5	4.1
C1	2.82	69%	0.52	0.65	13.1	5.4	11.4
C2.1	0.20	91%	0.82	0.90	5.0	0.8	1.6
C2.2	4.69	73%	0.56	0.68	12.8	9.9	20.3
C2.3	0.83	67%	0.54	0.68	10.1	1.9	3.9
C3.1	0.35	73%	0.68	0.79	5.0	1.2	2.4
C3.2	1.46	71%	0.56	0.68	8.4	3.6	7.4
C4.1	6.35	65%	0.49	0.63	12.0	12.1	25.9
C4.2	3.44	59%	0.46	0.61	12.6	5.9	13.3
C5	0.16	81%	0.74	0.84	6.4	0.6	1.0
C6	2.48	21%	0.22	0.45	6.8	2.5	8.8
D1	1.83	39%	0.39	0.58	16.7	2.4	6.0
D2	1.77	43%	0.43	0.61	16.3	2.5	6.1
D3	0.18	68%	0.63	0.76	5.4	0.6	1.2
D4	0.19	57%	0.54	0.70	6.3	0.5	1.1
D5	0.91	77%	0.71	0.82	6.0	3.1	6.1
D6	0.83	69%	0.64	0.77	6.4	2.5	5.2
D7	0.52	81%	0.74	0.84	5.0	2.0	3.8
D8	0.66	75%	0.69	0.81	5.0	2.4	4.6
OS1	2.85	2%	0.08	0.35	14.5	0.8	6.0
OS2	179.61	2%	0.08	0.35	47.4	27.1	190.9
OS3	11.99	2%	0.08	0.35	47.6	1.7	12.6

Missing design points 3.5, 2.5 & 12b in table.



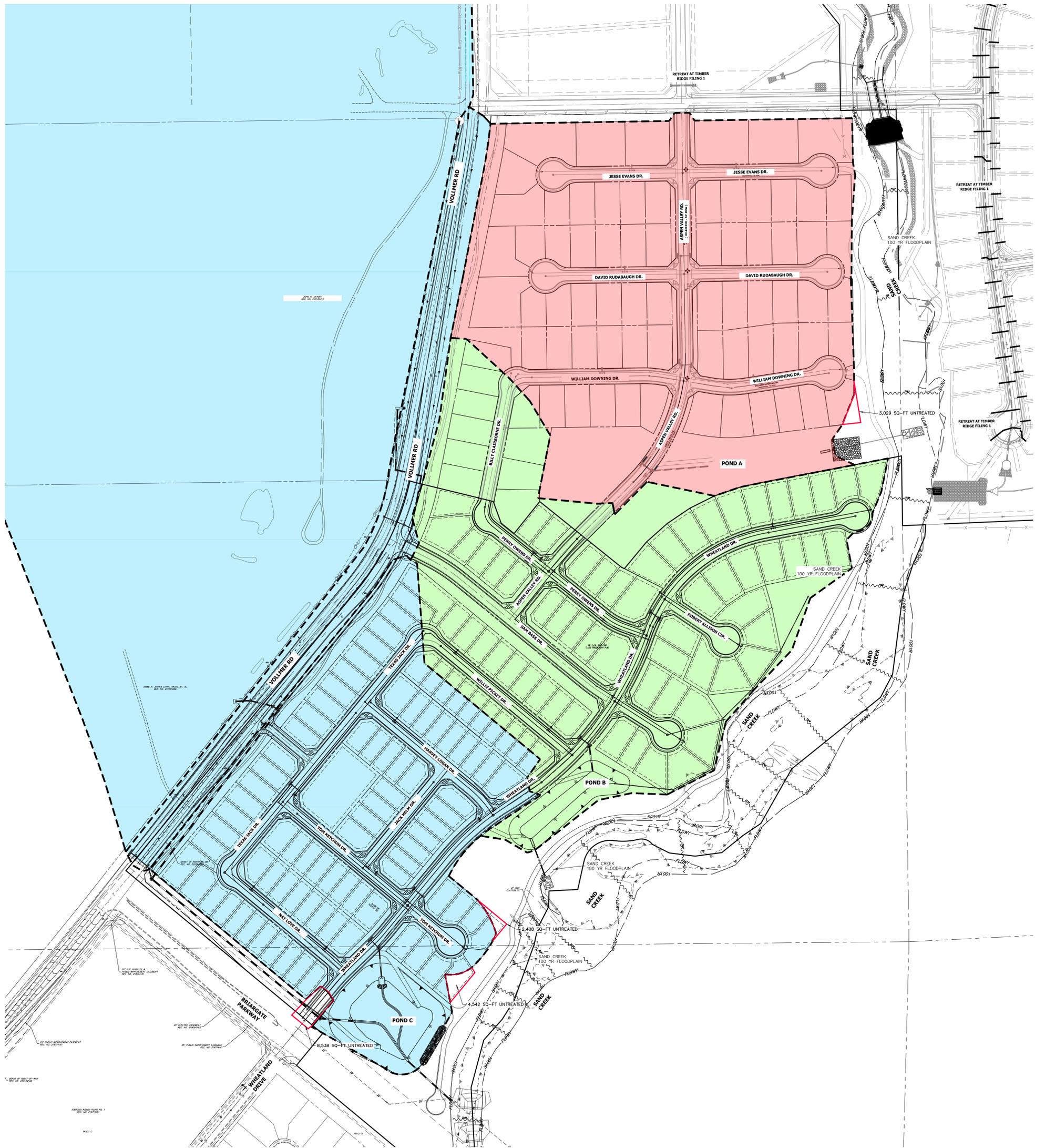
DRAINAGE MAP
 HOMESTEAD NORTH
 JOB NO. 25188.00
 10/08/21
 SHEET 2 OF 2



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WATER QUALITY CAPTURE PLAN

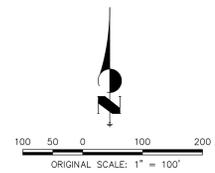
HOMESTEAD NORTH



- POND A 30.29 ACRES, 46.4% IMPERVIOUS**
- POND B 27.87 ACRES, 50.0% IMPERVIOUS**
- POND C 224.34 ACRES, 10.3% IMPERVIOUS**

NOTE:

1. A SEPARATE PLAN FOR STERLING RANCH ROAD AND BRIARGATE PKWY WILL BE PROVIDED IN A THE SEPARATE FDR REQUIRED FOR CONSTRUCTION OF THESE ROADWAYS.
2. A TOTAL OF 13,517 SQ-FT ON SITE IS LEFT UNTREATED.
3. POND C TREATS THE IMPROVEMENTS TO VOLLMER ROAD AND THE OFFSITE TRIBUTARY AREA

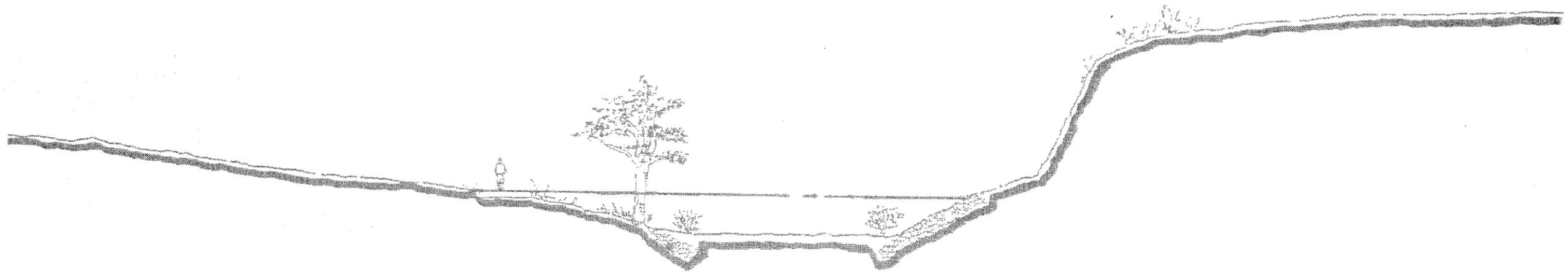


WQ -PONDS
 HOMESTEAD NORTH
 JOB NO. 25188.00
 6-29-2021
 SHEET 1 OF 1

Appendix E

Reference Material

SAND CREEK DRAINAGE BASIN PLANNING STUDY
PRELIMINARY DESIGN REPORT
CITY OF COLORADO SPRINGS, EL PASO COUNTY, COLORADO



PREPARED FOR:

City of Colorado Springs
Department of Comprehensive Planning, Development and Finance
Engineering Division
30 S. Nevada
Colorado Springs, Colorado 80903

PREPARED BY:

Kiowa Engineering Corporation
1011 North Weber
Colorado Springs, CO 80903

II. STUDY AREA DESCRIPTION

The Sand Creek drainage basin is a left-bank tributary to the Fountain Creek lying in the west-central portions of El Paso County. Sand Creek's drainage area at Fountain Creek is approximately 54 square miles of which approximately 18.8 square miles are inside the City of Colorado Springs corporate limits. The basin is divided into five major sub-basins, the Sand Creek mainstem, the East Fork Sand Creek, the Central Tributary to East Fork, the West Fork, and the East Fork Subtributary. Figure II-1 shows the location of the Sand Creek basin.

Basin Description

The Sand Creek basin covers a total of 54 square miles in unincorporated El Paso County and Colorado Springs, Colorado. Of this total, approximately 28 square miles is encompassed by the Sand Creek basin, and 26 square miles for the East Fork Sand Creek basin. The basin trends in generally a south to southwesterly direction, entering the Fountain Creek approximately two miles upstream of the Academy Boulevard bridge over Fountain Creek. Two main tributaries drain the basin, those being the mainstem of Sand Creek and East Fork Sand Creek. Development presence is most evident along the mainstream. At this time, approximately 25 percent of the basin is developed. This alternative evaluation focuses upon the Sand Creek basin only.

The maximum basin elevation is approximately 7,620 feet above mean sea level, and falls to approximately 5,790 feet at the confluence with Fountain Creek. The headwaters of the basin originate in the conifer covered areas of The Black Forest. The middle eastern portions of the basin are typified by rolling range land with fair to good vegetative cover associated with semi-arid climates.

Climate

This area of El Paso County can be described, in general as high plains, with total precipitation amounts typical of a semi-arid region. Winters are generally cold and dry. Precipitation ranges from 14 to 16 inches per year, with the majority of this precipitation occurring in spring and summer in the form of rainfall. Thunderstorms are common during the summer months, and are typified by quick-moving low pressure cells which draw moisture from the Gulf of Mexico into the region. Average temperatures range from about 30°F in the winter

to 75° in the summer. The relative humidity ranges from about 25 percent in the summer to 45 percent in the winter.

Soils and Geology

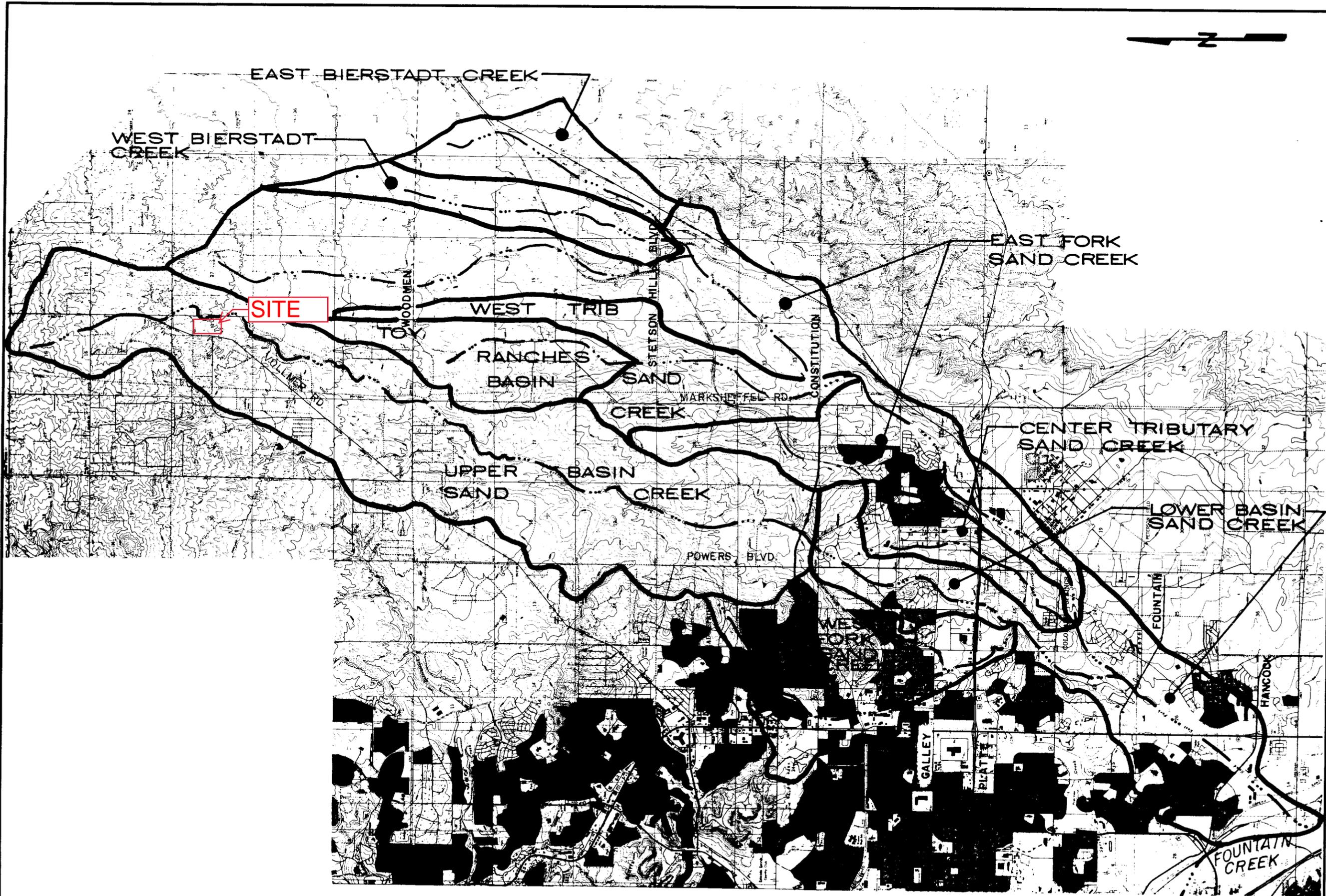
Soils within the Sand Creek basin vary between soil types A through D, as identified by the U. S. Department of Agriculture, Soil Conservation Service. The predominant soil groupings are in the Truckton and Bresser soil associations. The soils consist of deep, well drained soils that formed in alluvium and residuum, derived from sedimentary rock. The soils have high to moderate infiltration rates, and are extremely susceptible to wind and water erosion where poor vegetation cover exists. In undeveloped areas, the predominance of Type A and B soils give this basin a lower runoff per unit area as compared to basins with soils dominated by Types C and D. Presented on Figure II-2 is the Hydrologic Soil distribution map for the Sand Creek basin.

Property Ownership and Impervious Land Densities

Property ownership along the major drainageway within the Sand Creek basin vary from public to private. Along the developed reaches, drainage right-of-ways and greenbelts have been dedicated during the development of the adjacent residential and commercial land. Where development has not occurred, the drainageways remain under private ownership with no delineated drainage right-of-way or easements. There are several public parks which abut the mainstem of Sand Creek. Roadway and utility easements abutting or crossing the major drainageways occur most frequently in the developed portions of the basin.

Land use information for the existing and future conditions were reviewed as part of the planning effort. This information is used in the hydrologic analysis to predict runoff rates and volumes for the purposes of facility evaluation. The identification of land uses abutting the drainageways is also useful in the identification of feasible plans for stabilization and aesthetic treatment of the creek. Presented on Figure II-3 is the proposed land use map used in the evaluation of impervious land densities discussed in the hydrologic section of this report. Figure II-3 is not intended to reflect the future zoning or land use policies of the City or the County.

The land use information within the Banning-Lewis Ranch property was obtained from Aries Properties during the time the draft East Fork Sand Creek Drainage Basin Planning Study was being prepared. The land use information was again reviewed with the City of Colorado Springs Department of Planning and was found to be appropriate for use in the estimation of hydrology for the East Fork Basin. The location of future arterial streets and roadways within



Kiowa Engineering Corporation
 419 W. Bijou Street
 Colorado Springs, Colorado
 80905-1308

SAND CREEK DRAINAGE
 BASIN PLANNING STUDY
 REGIONAL SUB-BASINS

Project No	90-04-09
Date:	11/90
Design:	
Drawn:	EAK
Check:	
Revisions:	

SAND CREEK – SAND CREEK DRAINAGE BASIN PLANNING STUDY

Fee Development *For Information Only

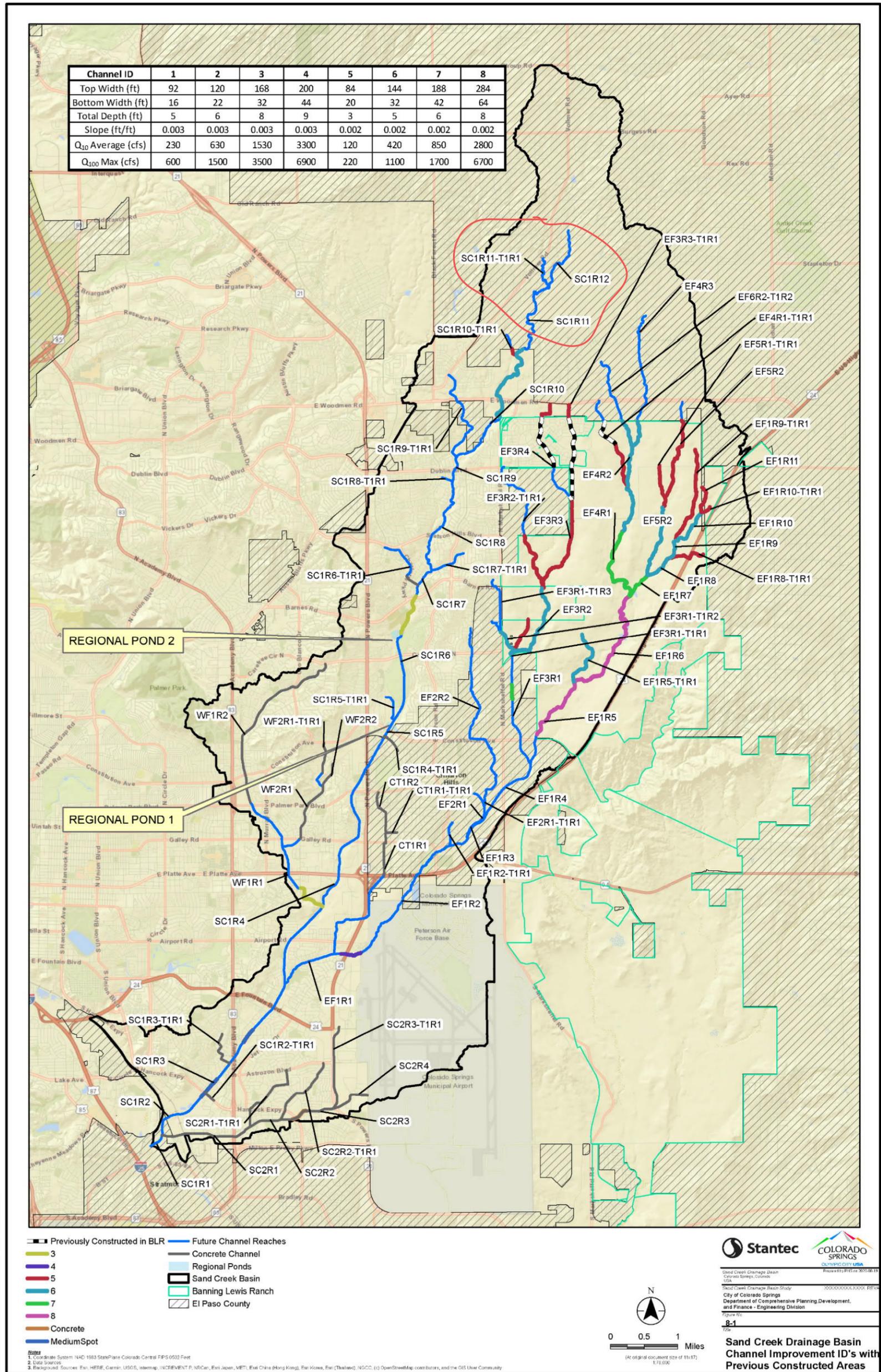


Figure 8-1. Sand Creek Drainage Basin Channel Improvement IDs with Previously Constructed Areas

HISTORIC CONDITION

BASIN SUMMARY		
BASIN	AREA (acres)	Q ₁₀₀ (cfs)
EX-1	24	3
EX-2	31	3
EX-3	111	49
EX-4	150	71
EX-5	162	118
EX-6	42	12
EX-7	132	11
EX-8	145	14
EX-9	200	19
EX-10	40	5
EX-11	80	8
EX-12	318	61
EX-13	33	8
OS-20	88	18
OS-21	78	18
OS-22	34	84
OS-23		

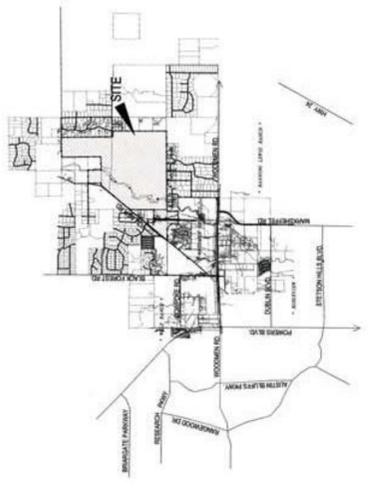
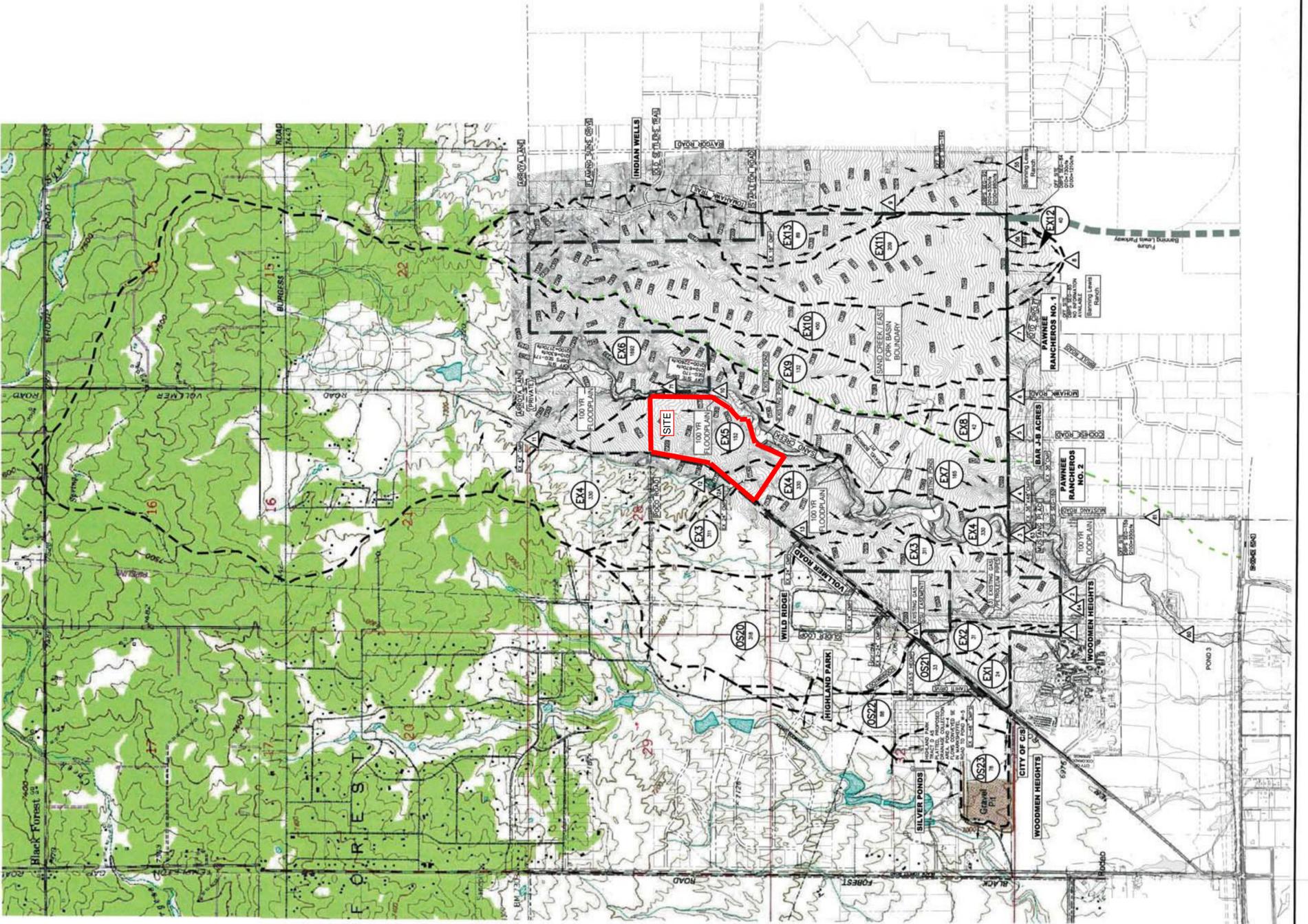
* NOTE: BASINS OS-22 & OS-23 NOT PART OF THIS REPORT. FLOWS FOLLOW HISTORIC PATTERNS ON THE WESTSIDE OF VOLLMER ROAD.

HISTORIC CONDITION

DESIGN POINTS		
DESIGN POINT	SO. MI.	SO. DBPS/DBPS
1	0.09	5
2	0.49	55
3	0.52	139
4	0.26	12
5	0.07	4
6	0.21	11
7	0.10	48
8	0.19	18
9	0.14	6
10	2.64	122
11	0.09	5
12	0.27	10
13	0.17	6
14	0.17	6
15	0.17	6

* NOTE: SO. MI. ARE NOT CONSTANT AT EACH DESIGN POINT DP-DBPS FOR THE EXISTING CONDITION
 * NOTE: DBPS FLOWS ARE FOR THE EXISTING CONDITION
 # NO DATA GIVEN IN DBPS

Runoff in attenuated in an existing pond. The existing release rate across the site is 16 cfs



STERLING RANCH
N.T.S.

STERLING RANCH MDDP
 HISTORIC - DRAINAGE MAP
 PROJECT NO. 09-001 FILE: \\dnp\p\m\09001-MDDP HISTORIC
 DESIGNED BY: WAS SCALE DATE: 03/16/15
 DRAWN BY: WAS HORIZ. 1"=500'
 CHECKED BY: WAS VERT. N/A SHEET 1 OF 1 D1

LEGEND

- EXISTING MDDP BASIN
- EXISTING FLOW RELEASE POINT
- FLOW DIRECTION
- BASIN BOUNDARY
- PROPERTY BOUNDARY
- EXISTING CONTOUR
- CULVERT PIPE

HISTORIC CONDITION

BASIN SUMMARY		
BASIN	AREA (acres)	Q ₁₀₀ (cfs)
EX-1	24	3
EX-2	31	3
EX-3	111	49
EX-4	150	71
EX-5	1682	118
EX-6	42	12
EX-7	132	11
EX-8	145	14
EX-9	200	19
EX-10	40	5
EX-11	80	6
EX-12	318	61
EX-13	33	8
OS-20	88	18
OS-21	78	18
OS-22	34	84
OS-23		

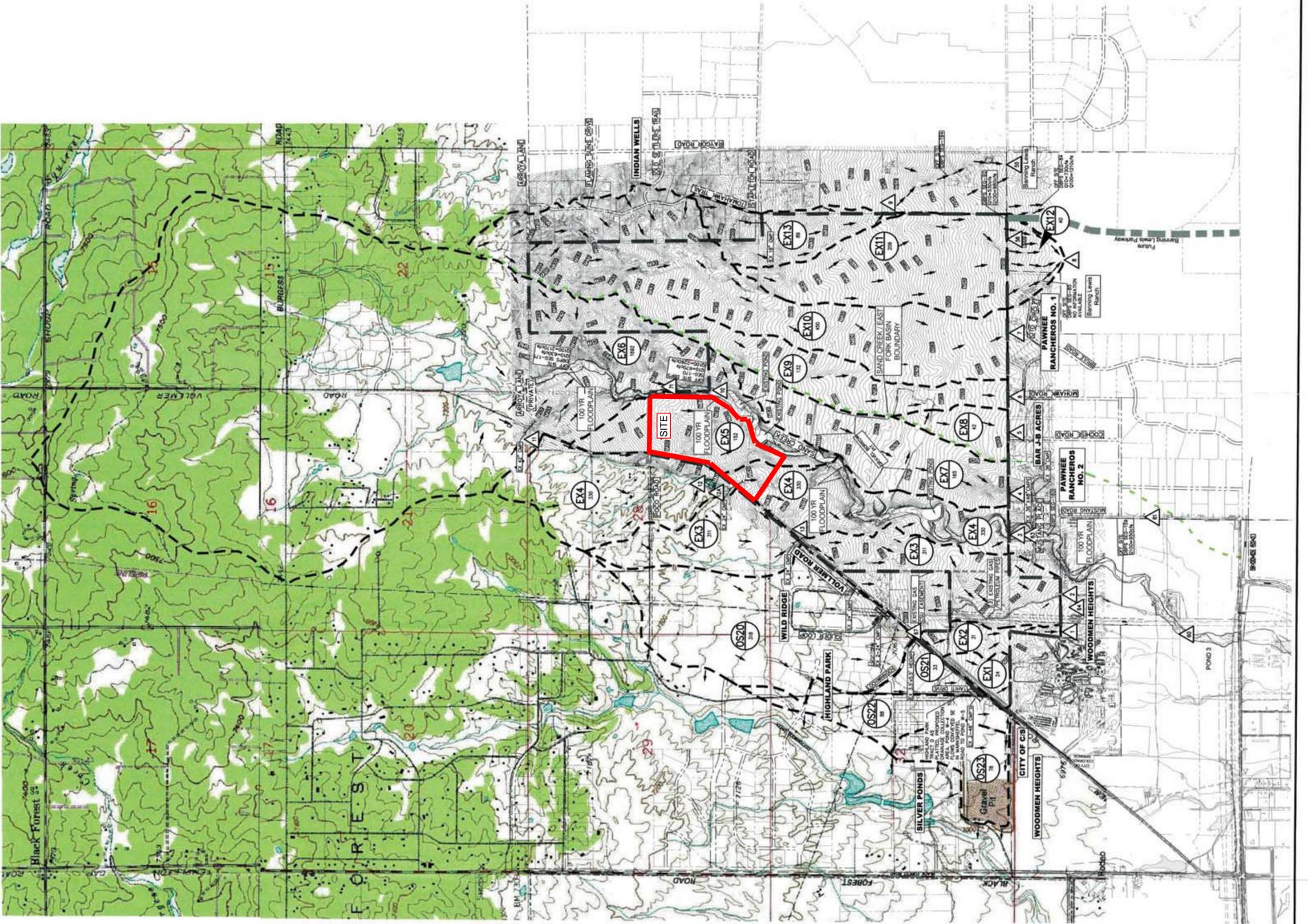
* NOTE: BASINS OS-22 & OS-23 NOT PART OF THIS REPORT. FLOWS FOLLOW HISTORIC PATTERNS ON THE WESTSIDE OF VOLLMER ROAD.

HISTORIC CONDITION

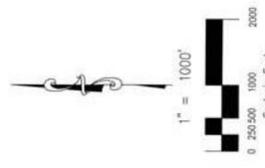
DESIGN POINTS		
DESIGN POINT	SO. MI.	SO. DBPS DBPS
1	0.09	5
2	0.49	55
3	0.52	139
4	0.26	12
5	0.07	4
6	0.21	11
7	0.10	48
8	0.19	18
9	0.14	6
10	2.64	122
11	0.09	5
12	0.17	10
13	0.17	6

* NOTE: SO. MI. ARE NOT CONSTANT AT EACH DESIGN POINT DP-DBPS FOR THE EXISTING CONDITION
 # NO DATA GIVEN IN DBPS

Runoff in attenuated in an existing pond. The existing release rate across the site is 16 cfs



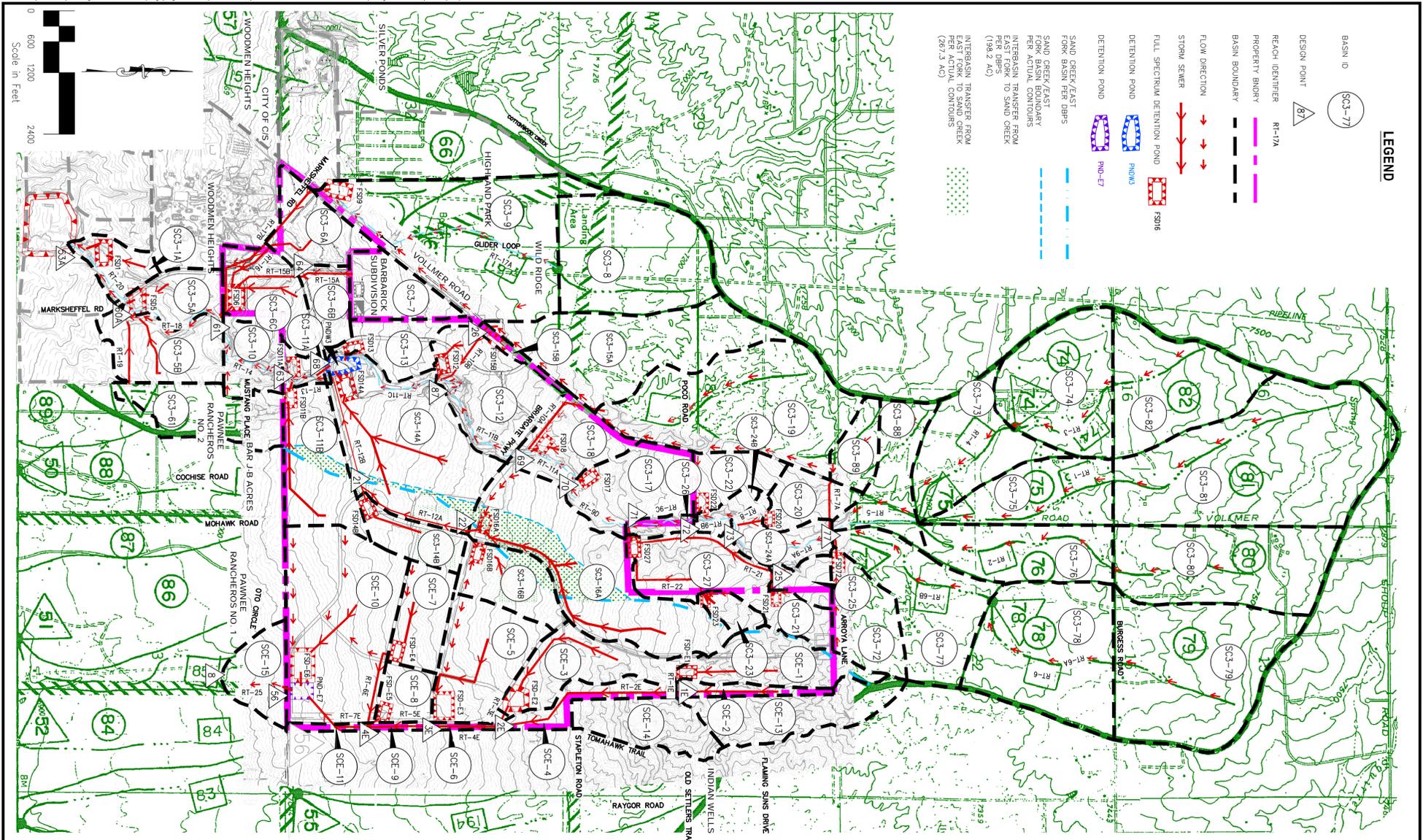
STERLING RANCH
N.T.S.



- LEGEND**
- EXISTING 100-YR FLOODPLAIN
 - EXISTING FLOW RELEASE POINT
 - FLOW DIRECTION
 - BASIN BOUNDARY
 - PROPERTY BOUNDARY
 - EXISTING CONTOUR
 - CULVERT PIPE

STERLING RANCH MDDP
HISTORIC - DRAINAGE MAP

PROJECT NO. 09-001 FILE: \\dnp\p\m\09001-MDDP HISTORIC
 DESIGNED BY: WAS SCALE DATE: 03/16/15
 DRAWN BY: WAS HORIZ. 1"=500'
 CHECKED BY: WAS VERT. N/A SHEET 1 OF 1 D1



BASIN SUMMARY											
BASIN	CN	AREA	Q ₁₀	Q ₅	Q ₂	Q ₁	Q _{0.5}	Q _{0.2}	Q _{0.1}	Q _{0.05}	Q _{0.02}
SC3-1A	73	27.8	0.044	16.3	33.0	45.8	57.1	68.9			
SC3-1B	84	39.1	0.061	40.6	53.7	71.0	92.4	110.6	129.1		
SC3-1C	81	63.0	0.098	53.8	73.0	98.5	130.8	158.6	187.0		
SC3-1D	88	49.3	0.077	61.4	79.3	102.2	130.1	153.6	177.1		
SC3-1E	85	30.9	0.048	32.9	43.4	57.0	73.9	88.2	102.7		
SC3-1F	82	58.0	0.099	53.9	72.5	97.1	128.0	154.5	181.5		
SC3-1G	88	45.7	0.071	54.0	69.9	90.3	115.2	136.2	157.2		
SC3-1H	66	143.4	0.224	25.4	42.1	66.7	100.7	132.3	166.2		
SC3-1I	63	217.4	0.340	45.8	71.5	108.6	158.9	204.9	258.0		
SC3-1J	63	36.0	0.056	7.6	12.3	19.4	29.1	38.0	47.7		
SC3-1K	70	10.7	0.017	5.3	7.8	10.3	15.9	20.0	24.3		
SC3-1L	80	76.6	0.120	59.4	78.3	110.8	148.1	180.5	213.7		
SC3-1M	81	88.2	0.138	77.8	105.6	142.5	189.1	229.1	270.0		
SC3-1N	85	41.0	0.064	43.9	57.8	76.0	99.5	117.6	136.9		
SC3-1O	77	164.9	0.258	127.6	175.4	239.8	321.9	393.2	466.3		
SC3-1P	77	34.7	0.054	24.6	33.4	47.4	64.2	79.0	94.1		
SC3-1Q	82	139.7	0.216	21.3	35.5	56.3	83.3	112.1	141.0		
SC3-1R	87	103.2	0.162	14.0	18.2	23.9	32.6	42.6	53.8		
SC3-1S	74	168.1	0.265	34.6	52.4	73.0	94.8	120.2	151.8		
SC3-1T	70	70.2	0.110	48.9	65.6	88.9	119.0	143.1	180.6		
SC3-1U	81	53.8	0.094	49.3	67.1	91.0	117.2	147.3	174.0		
SC3-1V	81	184.0	0.287	28.8	47.7	75.7	114.4	150.2	188.8		
SC3-1W	66	23.3	0.035	9.0	15.5	23.6	35.1	45.5	56.6		
SC3-1X	66	23.3	0.035	7.0	10.8	16.3	23.7	30.4	37.5		
SC3-1Y	65	14.5	0.023	5.5	8.3	12.4	18.0	23.0	28.4		
SC3-1Z	65	35.7	0.056	13.0	20.4	31.1	45.7	59.0	73.2		
SC3-2A	65	12.2	0.019	3.4	5.3	8.1	11.8	15.2	18.9		
SC3-2B	66	19.0	0.030	5.8	8.9	13.4	19.5	25.1	31.0		
SC3-2C	66	10.0	0.016	2.5	4.0	6.2	9.2	12.1	15.1		
SC3-2D	71	70.0	0.109	35.3	51.2	73.8	103.7	130.3	158.3		
SC3-2E	63	65.5	0.102	13.7	22.0	34.4	51.6	67.6	84.8		
SC3-2F	63	56.2	0.088	12.8	20.2	31.4	46.7	60.9	76.0		
SC3-2G	63	90.0	0.141	16.4	28.4	41.3	62.1	81.3	102.0		
SC3-2H	63	119.7	0.187	22.3	38.5	57.3	85.9	112.3	140.7		
SC3-2I	63	79.3	0.124	13.1	21.5	33.3	50.5	66.1	82.8		
SC3-2J	62	86.4	0.135	14.2	23.1	36.4	54.6	71.4	89.6		
SC3-2K	62	106.9	0.167	16.6	27.6	43.8	66.2	87.0	109.4		
SC3-2L	63	185.6	0.243	28.1	45.3	70.8	106.2	139.1	170.1		
SC3-2M	63	189.9	0.295	34.9	57.0	89.5	134.3	175.6	220.1		
SC3-2N	63	147.7	0.231	27.3	44.3	69.6	104.5	136.8	171.4		
SC3-2O	62	202.9	0.311	42.6	70.2	107.0	160.4	219.6	275.2		
SC3-2P	62	60.9	0.094	10.6	17.2	27.8	41.8	54.0	69.0		
SC3-2Q	62	27.5	0.043	6.1	10.0	15.7	23.6	30.8	38.9		
SC3-2R	65	64.4	0.101	23.3	35.9	53.8	79.1	102.4	127.4		
SC3-2S	64	15.0	0.023	4.4	7.0	10.8	15.9	20.7	25.7		
SC3-2T	70	67.5	0.105	50.6	65.2	85.9	118.0	143.9	178.2		
SC3-2U	70	29.5	0.046	13.3	19.6	28.6	40.6	52.8	67.6		
SC3-2V	67	85.5	0.134	10.4	13.0	16.6	21.4	28.8	38.4		
SC3-2W	64	3.8	0.006	1.6	2.5	3.7	5.4	7.0	8.7		
SC3-2X	89	44.9	0.070	58.9	8.8	48.4	60.7	75.2	93.7		
SC3-2Y	82	25.5	0.040	38.6	6.8	45.4	56.2	70.4	87.7		
SC3-2Z	64	4.0	0.006	1.6	2.4	3.6	5.3	6.8	8.5		
SC3-3A	63	174.3	0.272	7.6	18.4	19.4	29.1	39.8	46.5		
SC3-3B	64	5.8	0.009	2.3	3.3	4.8	7.0	10.3	12.8		
SC3-3C	63	78.6	0.123	19.6	31.3	48.7	73.1	95.7	120.0		
SC3-3D	63	52.5	0.082	21.2	33.3	49.9	65.2	81.9	98.7		
SC3-3E	51	39.7	0.062	2.2	5.1	10.3	17.7	25.1	33.4		

DESIGN POINT SUMMARY (VOLUME)											
DESIGN POINT	AREA	Q ₁₀	Q ₅	Q ₂	Q ₁	Q _{0.5}	Q _{0.2}	Q _{0.1}	Q _{0.05}	Q _{0.02}	LOCATION
DP-1	0.371	3.9	10.4	18.8	25.5	31.6					ARROYA LANE X-ING
DP-2	1.413	32.7	34.5	51.7	75.4	97.1	120.3				ARROYA LANE X-ING
DP-3	2.343	57.7	57.4	85.9	123.1	161.1	190.9				ARROYA LANE X-ING
DP-4	0.338	3.8	13.5	20.1	29.3	37.7	46.7				ARROYA LANE X-ING
DP-5	0.247	2.5	4.0	6.0	8.8	11.7	15.1				ARROYA LANE X-ING
DP-6	2.545	41.3	62.9	94.0	136.8	185.2	238.5				ARROYA LANE X-ING
DP-7	4.026	46.3	74.5	110.3	161.5	216.5	280.5				ARROYA LANE X-ING
DP-8	4.946	57.2	86.1	127.6	181.8	235.3	300.9				ARROYA LANE X-ING
DP-9	3.924	66.5	98.9	147.6	209.1	287.1	370.9				ARROYA LANE X-ING
DP-10	0.247	3.1	5.2	7.8	11.2	14.8	19.6				ARROYA LANE X-ING
DP-11	0.480	6.1	10.4	16.9	25.7	33.7	42.2				ARROYA LANE X-ING
DP-12	0.820	7.0	13.6	23.4	36.1	47.4	59.3				ARROYA LANE X-ING
DP-13	1.017	7.7	18.1	28.6	51.0	74.0	92.9				ARROYA LANE X-ING
DP-14	1.079	8.0	16.7	26.6	53.0	74.0	95.9				ARROYA LANE X-ING
DP-15	0.396	6.3	10.7	18.3	27.5	35.6	44.0				ARROYA LANE X-ING
DP-16	0.236	6.3	10.7	16.7	24.6	31.5	38.7				ARROYA LANE X-ING
DP-17	1.017	1.3	1.9	2.8	4.1	5.8	8.1				ARROYA LANE X-ING
DP-18	1.079	0.7	0.9	1.2	1.5	1.8	2.1				ARROYA LANE X-ING

WATER QUALITY & DETENTION POND SUMMARY											
DESIGN POINT	AREA	Q ₁₀	Q ₅	Q ₂	Q ₁	Q _{0.5}	Q _{0.2}	Q _{0.1}	Q _{0.05}	Q _{0.02}	LOCATION
DP-19	4.449	1468	1468	1468	1468	1468	1468	1468	1468	1468	STERLING RANCH SOUTHERN BRIDY
DP-20	4.333	1468	1468	1468	1468	1468	1468	1468	1468	1468	STERLING RANCH SOUTHERN BRIDY
DP-21	2.157	1612	1612	1612	1612	1612	1612	1612	1612	1612	STERLING RANCH SOUTHERN BRIDY
DP-22	4.449	1468	1468	1468	1468	1468	1468	1468	1468	1468	STERLING RANCH SOUTHERN BRIDY
DP-23	4.333	1468	1468	1468	1468	1468	1468	1468	1468	1468	STERLING RANCH SOUTHERN BRIDY
DP-24	2.157	1612	1612	1612	1612	1612	1612	1612	1612	1612	STERLING RANCH SOUTHERN BRIDY
DP-25	4.449	1468	1468	1468	1468	1468	1468	1468	1468	1468	STERLING RANCH SOUTHERN BRIDY
DP-26	4.333	1468	1468	1468	1468	1468	1468	1468	1468	1468	STERLING RANCH SOUTHERN BRIDY

PRELIMINARY DRAINAGE REPORT AND MDDP ADDENDUM FOR
HOMESTEAD NORTH AT STERLING RANCH

October 2021