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October 20, 2026

El Paso County Engineering Division
Department of Public Works
3275 Akers Drive
Colorado Springs, Colorado 80922

Re: Professional Engineer's Certification of Permanent Control Measure
Flying Horse North Filing No. 4, Pond B

This letter serves as a Pond Certification for the constructed Full Spectrum Detention Facility "Pond B" located in Flying Horse North Filing No. 4. The permanent control measure is located at the north central area of the subdivision and the northwestern part of the filing, adjacent and north of Old Stagecoach Road.

The detention facility was designed by a Colorado Licensed Professional Engineer of HR Green Development, LLC. and the design is within the County approved Construction Drawings dated February 20, 2025 and is referred to as "Pond B" within Tract B of the development. A Colorado Licensed Professional Surveyor conducted an as-built survey of the facility in September of 2026 to verify the basin and infrastructure dimensions, elevations, materials, inverts, and slopes as required for determination of inflow and outflow hydraulics.

A Stormwater Preliminary Acceptance Punchlist was issued by El Paso County Stormwater Engineering staff and inspections dated 6/11/2026 for a 1st Walkthrough dated 6/11/2026 for ESQCP No. ESQ258 with the related PCD File Nos. SF2422, CON258, and ASB2616.

The as-built conditions of the basin and its infrastructure were assessed and compared to the approved Construction Drawings and design worksheets within the approved Final Drainage Report. As-built conditions were assessed and redlined plans and reports were issued for field adjustments and verification of as-built conditions for the pond and its infrastructure, per the ASB documentation.

Preliminary Acceptance Punchlist items included the following and were directly addressed as described:

- Outfall Riprap
  - Outfall riprap is not installed per the approved plans. Plans show buried riprap and in the field it is installed as exposed riprap. Install per approved plans or consult the Engineer of Record. See Note 1. ***Outfall/spillway rip-rap is exposed rip-rap and is now reflected on the as-builts, as approved by the Engineer of Record.***
- Maintenance access road installed with crushed concrete, which per direction from CDPHE is not an acceptable means of Final Stabilization, nor was that material on the approved plans, so please remove and install access road material per plans. ***As-built plans and redlines now call out crushed concrete as approved by the Engineer of Record. This was discussed with Stormwater staff in July 2026 and is to be allowable for this filing.***

With the field adjustments and verification of as-built conditions as of July 22, 2026, the facility was found to be in general compliance with the design, and it is anticipated that the facility will function as intended with no negative impacts to surrounding and downstream developments, property, or infrastructure. General compliance allows for construction tolerances and is based on the information provided by the as-built survey.

The facility is in a stabilized condition and there are no proposed modifications required at this time.

The site and adjacent properties (as affected by work performed under the County permit) are stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover, and that the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements. The PCM(s) provide the required storage volume and meet the required release rates, stage areas, elevations, and outlet dimensions, as documented by the attached revised MHFD-Detention spreadsheet that shows the as-built conditions.

Sincerely,

HR GREEN, INC.



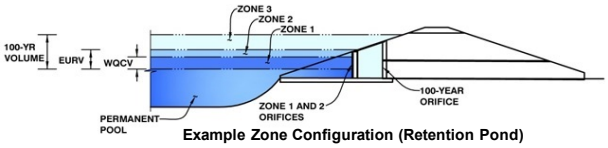
Richard D. Lyon, PE (No. 53921)
Senior Project Manager, Civil Engineer
HR Green Development, LLC.

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: **FLYING HORSE NORTH FILING NO. 4** (AS-BUILT CONDITION FOR SEPTEMBER 2026)

Basin ID: **POND B**



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	**EDB**	
Watershed Area =	105.65	acres
Watershed Length =	3,000	ft
Watershed Length to Centroid =	1,000	ft
Watershed Slope =	0.035	ft/ft
Watershed Imperviousness =	8.25%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

			Optional User Overrides	
Water Quality Capture Volume (WQCV) =	0.50	acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	0.81	acre-feet		acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	1.27	acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	2.93	acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	4.61	acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	7.66	acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	9.71	acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	12.69	acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 3.14 in.) =	18.09	acre-feet		inches
Approximate 2-yr Detention Volume =	0.50	acre-feet		
Approximate 5-yr Detention Volume =	0.80	acre-feet		
Approximate 10-yr Detention Volume =	1.83	acre-feet		
Approximate 25-yr Detention Volume =	2.63	acre-feet		
Approximate 50-yr Detention Volume =	2.73	acre-feet		
Approximate 100-yr Detention Volume =	3.57	acre-feet		

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.50	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.31	acre-feet
Zone 3 Volume (User Defined - Zones 1 & 2) =	1.51	acre-feet
Total Detention Basin Volume =	2.32	acre-feet

Total detention volume is less than 100-year volume.

Depth Increment = ft

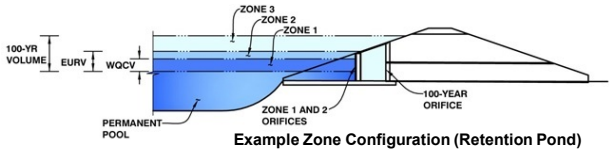
Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
7526 **Top of Micropool**	--	0.00	--	--	--	10	0.000		
7526.5	--	0.50	--	--	--	370	0.008	95	0.002
7527.0	--	1.00	--	--	--	3,250	0.075	1,000	0.023
7527.5	--	1.50	--	--	--	9,990	0.229	4,310	0.099
7528.0	--	2.00	--	--	--	16,760	0.385	10,997	0.252
7528.5	--	2.50	--	--	--	22,895	0.526	20,911	0.480
7529.0	--	3.00	--	--	--	27,077	0.622	33,404	0.767
7529.5	--	3.50	--	--	--	30,000	0.689	47,673	1.094
7530.0	--	4.00	--	--	--	32,290	0.741	63,246	1.452
7530.5	--	4.50	--	--	--	35,012	0.804	80,071	1.838
7531.0	--	5.00	--	--	--	37,820	0.868	98,279	2.256
7531.5	--	5.50	--	--	--	40,670	0.934	117,902	2.707
7532.0	--	6.00	--	--	--	43,900	1.008	139,044	3.192
7532.5	--	6.50	--	--	--	46,440	1.066	161,629	3.710
7533.0	--	7.00	--	--	--	57,630	1.323	187,647	4.308

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: **FLYING HORSE NORTH FILING NO. 4** (AS-BUILT CONDITION FOR SEPTEMBER 2026)

Basin ID: **POND B**



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.54	0.500	Orifice Plate
Zone 2 (EURV)	3.07	0.307	Orifice Plate
Zone 3 (User)	5.08	1.513	Weir&Pipe (Restrict)
Total (all zones)		2.320	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft²
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

Calculated Parameters for Underdrain

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	N/A	ft²
Depth at top of Zone using Orifice Plate =	2.28	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	N/A	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	N/A	sq. inches	Elliptical Slot Area =	N/A	ft²

Calculated Parameters for Plate

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.80	1.60					
Orifice Area (sq. inches)	1.65	1.65	1.65					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected			Not Selected	Not Selected
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	N/A	N/A
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	N/A	N/A
Vertical Orifice Diameter =	N/A	N/A	inches			

Calculated Parameters for Vertical Orif

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected			Zone 3 Weir	Not Selected
Overflow Weir Front Edge Height, Ho =	2.85	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	2.85	N/A
Overflow Weir Front Edge Length =	30.00	N/A	feet	Overflow Weir Slope Length =	5.00	N/A
Overflow Weir Grate Slope =	0.00	N/A	H:V	Grate Open Area / 100-yr Orifice Area =	5.89	N/A
Horiz. Length of Weir Sides =	5.00	N/A	feet	Overflow Grate Open Area w/o Debris =	104.40	N/A
Overflow Grate Type =	Type C Grate	N/A		Overflow Grate Open Area w/ Debris =	52.20	N/A
Debris Clogging % =	50%	N/A	%			

Calculated Parameters for Overflow W

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected			Zone 3 Restrictor	Not Selected
Depth to Invert of Outlet Pipe =	3.70	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	17.71	N/A
Outlet Pipe Diameter =	60.00	N/A	inches	Outlet Orifice Centroid =	2.28	N/A
Restrictor Plate Height Above Pipe Invert =	50.75		inches	Half-Central Angle of Restrictor Plate on Pipe =	2.33	N/A

Calculated Parameters for Outlet Pipe w/ Flow Restriction Pla

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	5.10	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.86	feet
Spillway Crest Length =	100.00	feet	Stage at Top of Freeboard =	6.96	feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	1.30	acres
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =	4.26	acre-ft

Calculated Parameters for Spillway

Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AI

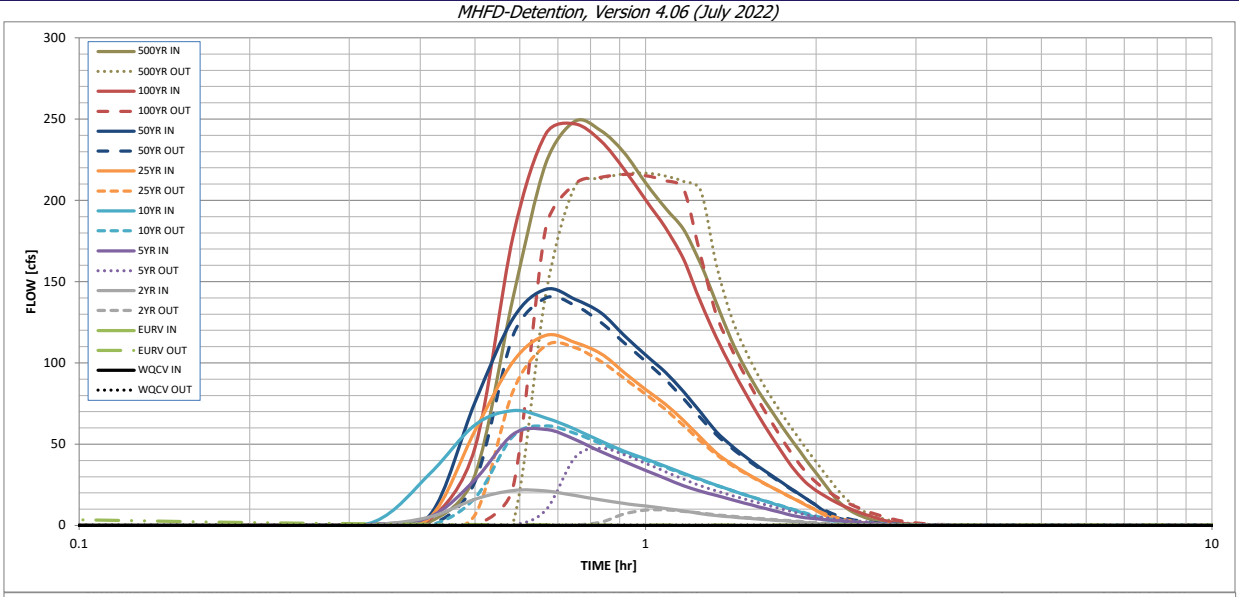
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
CUHP Runoff Volume (acre-ft) =	0.500	0.807	1.270	2.932	4.609	7.657	9.710	12.690
User Override Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	1.270	3.472	4.609	7.657	9.710	17.082
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	14.9	41.7	63.1	109.6	138.0	172.8
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A						216.0
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.14	0.39	0.60	1.04	1.31	2.04
Peak Inflow Q (cfs) =	N/A	N/A	21.5	59.1	70.7	116.9	145.3	247.1
Peak Outflow Q (cfs) =	0.2	8.1	9.5	47.5	61.2	110.6	139.9	215.9
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	1.1	1.0	1.0	1.0	1.0
Structure Controlling Flow =	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1
Max Velocity through Grate 1 (fps) =	N/A	0.11	0.09	0.5	0.6	1.1	1.3	2.1
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	39	48	48	41	37	30	26	13
Time to Drain 99% of Inflow Volume (hours) =	**41**	51	52	49	47	43	41	35
Maximum Ponding Depth (ft) =	2.54	3.07	3.04	3.41	3.51	3.84	4.00	4.99
Area at Maximum Ponding Depth (acres) =	0.53	0.63	0.63	0.68	0.69	0.72	0.74	0.87
Maximum Volume Stored (acre-ft) =	0.50	0.81	0.79	1.03	1.10	1.33	1.45	2.24
Elevation (ft) =	7528.54	7529.07						7530.99
Pond Bottom (ft) =	7526.00							

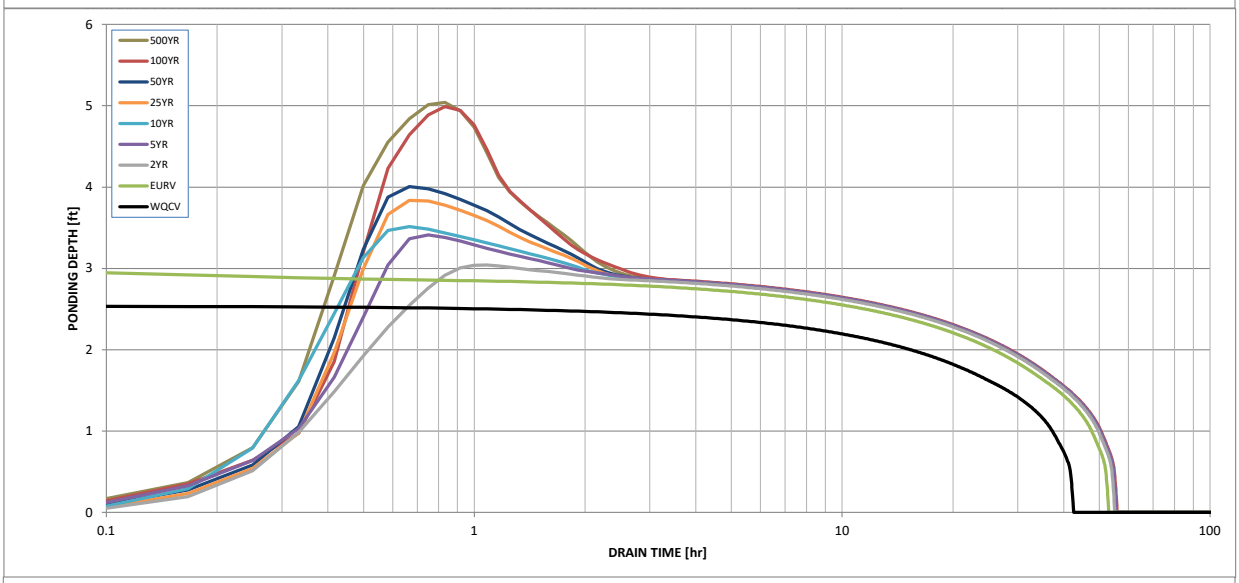
SWMM volume 103,808

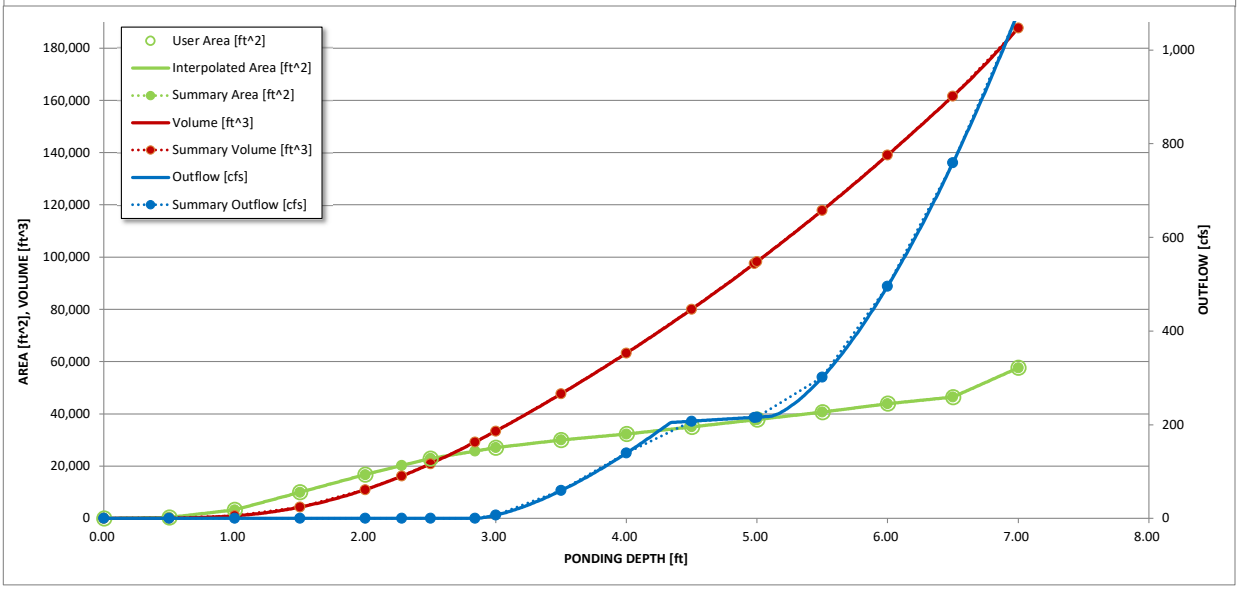
DOWNSTREAM OUTFALL AND CONVEYENCES WERE ASSESSED AS A PART OF THE FDR FOR THE MINOR AN MAJOR STORM EVENTS AND PROVEN TO BE STABLE FOR THE MAJOR STORM

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	USER	CUHP	CUHP	CUHP	USER	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.02	0.00	0.00	0.01	0.03	0.05
	0:15:00	0.00	0.00	0.13	0.29	0.25	0.17	0.22	0.32	0.32
	0:20:00	0.00	0.00	0.50	0.84	2.56	0.51	0.61	0.81	2.42
	0:25:00	0.00	0.00	5.28	5.28	32.00	5.01	6.58	4.04	31.16
	0:30:00	0.00	0.00	16.15	28.18	61.85	58.31	75.74	47.72	140.12
	0:35:00	0.00	0.00	21.48	55.99	70.65	100.78	127.30	177.86	222.60
	0:40:00	0.00	0.00	21.19	59.11	66.12	116.93	145.33	240.83	248.61
	0:45:00	0.00	0.00	18.41	52.91	59.32	112.63	139.24	247.06	242.92
	0:50:00	0.00	0.00	15.77	45.39	51.88	105.76	130.83	236.61	229.34
	0:55:00	0.00	0.00	13.51	39.31	45.74	94.10	116.93	219.08	210.73
	1:00:00	0.00	0.00	11.89	33.91	40.89	83.71	104.89	200.33	195.31
	1:05:00	0.00	0.00	10.45	28.93	36.47	74.57	94.22	183.22	182.16
	1:10:00	0.00	0.00	8.87	24.45	32.13	64.62	82.26	163.76	160.97
	1:15:00	0.00	0.00	7.27	21.11	28.23	54.18	69.62	137.51	137.05
	1:20:00	0.00	0.00	6.00	18.47	24.80	44.69	57.74	115.66	114.12
	1:25:00	0.00	0.00	5.21	15.99	21.68	37.99	49.23	97.45	96.67
	1:30:00	0.00	0.00	4.56	13.64	18.77	32.53	42.20	81.53	82.47
	1:35:00	0.00	0.00	3.98	11.47	16.17	27.90	36.24	67.41	70.56
	1:40:00	0.00	0.00	3.40	9.51	13.77	23.69	30.82	54.87	59.85
	1:45:00	0.00	0.00	2.83	7.57	11.49	19.86	25.89	43.86	50.04
	1:50:00	0.00	0.00	2.28	5.83	9.29	16.17	21.17	33.94	40.82
	1:55:00	0.00	0.00	1.71	4.79	7.07	12.63	16.64	26.61	32.21
	2:00:00	0.00	0.00	1.15	4.12	4.87	9.19	12.28	21.52	24.12
	2:05:00	0.00	0.00	0.67	3.56	3.31	5.81	7.96	17.61	16.50
	2:10:00	0.00	0.00	0.42	3.01	2.45	3.59	5.15	14.31	11.29
	2:15:00	0.00	0.00	0.30	2.51	1.89	2.28	3.44	11.60	7.89
	2:20:00	0.00	0.00	0.23	2.04	1.47	1.48	2.35	9.28	5.46
	2:25:00	0.00	0.00	0.18	1.60	1.14	0.95	1.58	7.30	3.68
	2:30:00	0.00	0.00	0.13	1.18	0.86	0.62	1.08	5.61	2.39
	2:35:00	0.00	0.00	0.10	0.81	0.63	0.40	0.71	4.07	1.45
	2:40:00	0.00	0.00	0.07	0.55	0.45	0.25	0.46	2.75	0.84
	2:45:00	0.00	0.00	0.06	0.40	0.31	0.17	0.31	1.87	0.55
	2:50:00	0.00	0.00	0.05	0.31	0.21	0.12	0.22	1.28	0.38
	2:55:00	0.00	0.00	0.04	0.24	0.15	0.09	0.17	0.88	0.30
	3:00:00	0.00	0.00	0.03	0.19	0.11	0.06	0.13	0.58	0.24
	3:05:00	0.00	0.00	0.02	0.15	0.08	0.05	0.09	0.37	0.18
	3:10:00	0.00	0.00	0.01	0.12	0.06	0.03	0.07	0.23	0.13
	3:15:00	0.00	0.00	0.01	0.09	0.03	0.02	0.05	0.15	0.09
	3:20:00	0.00	0.00	0.01	0.07	0.02	0.01	0.03	0.11	0.05
	3:25:00	0.00	0.00	0.00	0.05	0.01	0.01	0.02	0.08	0.03
	3:30:00	0.00	0.00	0.00	0.04	0.00	0.00	0.01	0.07	0.01
	3:35:00	0.00	0.00	0.00	0.03	0.00	0.00	0.00	0.05	0.00
	3:40:00	0.00	0.00	0.00	0.02	0.00	0.00	0.00	0.04	0.00
	3:45:00	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.03	0.00
	3:50:00	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.02	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00