HRGreen®

1975 Research Parkway | Suite 160
Colorado Springs, CO 80920
Main 719.300.4140 + **Fax** 713.965.0044

HRGREEN.COM

October 20, 2026

El Paso County Engineering Division
Department of Public Works
3275 Akers Drive
Colorado Springs, Colorado 80922

Re: Professional Engineer's Certification of Permanent Control Measure
Flying Horse North Filing No. 4, Pond A

This letter serves as a Pond Certification for the constructed Full Spectrum Detention Facility "Pond A" located in Flying Horse North Filing No. 4. The permanent control measure is located at the southeast area of the subdivision, near Black Forest Road and north of Rubble Drive.

The detention facility was designed by a Colorado Licensed Professional Engineer of HR Green Development, LLC. and the design is within the County approved Construction Drawings dated February 20, 2025 and is referred to as "Pond A" within Tract A of the development. A Colorado Licensed Professional Surveyor conducted an as-built survey of the facility in September of 2025 to verify the basin and infrastructure dimensions, elevations, materials, inverts, and slopes as required for determination of inflow and outflow hydraulics. A subsequent survey was conducted in September 2026 and the as-built MHFD Detention calculations within this letter reflect those conditions.

A Stormwater Preliminary Acceptance Punchlist was issued by El Paso County Stormwater Engineering staff and inspections dated 6/11/2026 for a 1st Walkthrough dated 6/11/2026 for ESQCP No. ESQ258 with the related PCD File Nos. SF2422, CON258, and ASB2616.

The as-built conditions of the basin and its infrastructure were assessed and compared to the approved Construction Drawings and design worksheets within the approved Final Drainage Report. As-built conditions were assessed and redlined plans and reports were issued for field adjustments and verification of as-built conditions for the pond and its infrastructure, per the ASB documentation.

Preliminary Acceptance Punchlist items included the following and were directly addressed as described:

- Inflow Riprap Upstream of Forebay
    - The riprap inflow is inundated with sediment and is not installed per the approved plans. Buried riprap is shown on the approved plans. Install per approved plans or consult the Engineer of Record. See Note 1. ***All exposed rip-rap is now buried soil rip-rap with top soil and permanent seeding for stabilization.***
    - Also, the riprap has no low-flow conveyance, such that as it has accumulated sediment in the middle of the width, the flows have been pushed to the outside of the riprap and is eroding around the sides of the riprap. Installing riprap with a V shape (per plans) or U shape would better prevent flows from causing erosion of the swale side slopes, and/or install riprap on side slopes. ***A more defined V-shaped swale has been constructed. The rip-rap pad is revised per the as-built redlines and the remaining channelized section is TRM lined with permanent seeding for stabilization.***
- Outlet Structure
    - The distance between orifice holes is not installed per the approved plans. The holes are 12in apart and should be 10.8in apart. Install per approved plans or consult the Engineer of

Record. See Note 1. ***As-built redlines supplied to reflect the revision to the orifice plate. MHFD Detention calculations supplied with the Pond Certification letter support the change.***

- Outfall Riprap
    - Outfall riprap is not installed per the approved plans. Plans show buried riprap and in the field it is installed as exposed riprap. Install per approved plans or consult the Engineer of Record. See Note 1. ***Outfall/spillway rip-rap is exposed rip-rap and is now reflected on the as-builts, as approved by the Engineer of Record.***
- Maintenance access road installed with crushed concrete, which per direction from CDPHE is not an acceptable means of Final Stabilization, nor was that material on the approved plans, so please remove and install access road material per plans. ***As-built plans and redlines now call out crushed concrete as approved by the Engineer of Record. This was discussed with Stormwater staff in July 2026 and is to be allowable for this filing.***

With the field adjustments and verification of as-built conditions as of September 18, 2026, the facility was found to be in general compliance with the design, and it is anticipated that the facility will function as intended with no negative impacts to surrounding and downstream developments, property, or infrastructure. General compliance allows for construction tolerances and is based on the information provided by the as-built survey.

The facility is in a stabilized condition and there are no proposed modifications required at this time.

The site and adjacent properties (as affected by work performed under the County permit) are stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover, and that the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements. The PCM(s) provide the required storage volume and meet the required release rates, stage areas, elevations, and outlet dimensions, as documented by the attached revised MHFD-Detention spreadsheet that shows the as-built conditions.

Sincerely,

HR GREEN, INC.



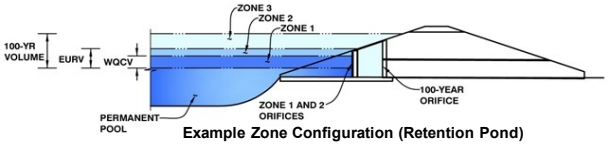
Richard D. Lyon, PE (No. 53921)
Senior Project Manager, Civil Engineer
HR Green Development, LLC.

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: **FLYING HORSE NORTH FILING NO. 4 (September 2026 As-Built Condition)**

Basin ID: **POND A**



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	**EDB**	
Watershed Area =	128.48	acres
Watershed Length =	3,560	ft
Watershed Length to Centroid =	1,400	ft
Watershed Slope =	0.040	ft/ft
Watershed Imperviousness =	8.96%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

			Optional User Overrides	
Water Quality Capture Volume (WQCV) =	0.653	acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	1.073	acre-feet		acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	1.617	acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	3.665	acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	5.721	acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	9.429	acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	11.939	acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	15.571	acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 3.14 in.) =	22.164	acre-feet		inches
Approximate 2-yr Detention Volume =	0.677	acre-feet		
Approximate 5-yr Detention Volume =	1.072	acre-feet		
Approximate 10-yr Detention Volume =	2.337	acre-feet		
Approximate 25-yr Detention Volume =	3.321	acre-feet		
Approximate 50-yr Detention Volume =	3.463	acre-feet		
Approximate 100-yr Detention Volume =	4.504	acre-feet		

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.653	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.420	acre-feet
Zone 3 Volume (User Defined - Zones 1 & 2) =	1.760	acre-feet
Total Detention Basin Volume =	2.833	acre-feet

Total detention volume is less than 100-year volume.

Depth Increment = ft

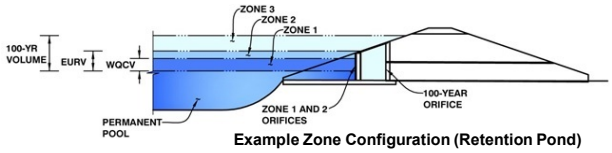
Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
7532 **Top of Micropool**	--	0.00	--	--	--	10	0.000		
7532.5	--	0.50	--	--	--	245	0.006	64	0.001
7533.0	--	1.00	--	--	--	1,310	0.030	452	0.010
7533.5	--	1.50	--	--	--	6,624	0.152	2,436	0.056
7534.0	--	2.00	--	--	--	15,944	0.366	8,078	0.185
7534.5	--	2.50	--	--	--	25,590	0.587	18,461	0.424
7535.0	--	3.00	--	--	--	33,340	0.765	33,194	0.762
7535.5	--	3.50	--	--	--	40,302	0.925	51,604	1.185
7536.0	--	4.00	--	--	--	46,714	1.072	73,358	1.684
7536.5	--	4.50	--	--	--	52,110	1.196	98,064	2.251
7537.0	--	5.00	--	--	--	55,511	1.274	124,970	2.869
7537.5	--	5.50	--	--	--	58,055	1.333	153,361	3.521
7538.0	--	6.00	--	--	--	61,402	1.410	183,225	4.206
7538.5	--	6.50	--	--	--	64,200	1.474	214,626	4.927
7539.0	--	7.00	--	--	--	69,384	1.593	248,022	5.694

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: FLYING HORSE NORTH FILING NO. 4 (September 2026 As-Built Condition)

Basin ID: POND A



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.86	0.65	Orifice Plate
Zone 2 (EURV)	3.38	0.42	Orifice Plate
Zone 3 (User)	4.98	1.76	Weir&Pipe (Restrict)
Total (all zones)		2.83	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft²
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

Calculated Parameters for Underdrain

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	N/A	ft²
Depth at top of Zone using Orifice Plate =	3.38	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	N/A	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	N/A	sq. inches	Elliptical Slot Area =	N/A	ft²

Calculated Parameters for Plate

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.00	2.00					
Orifice Area (sq. inches)	2.08	2.08	2.08					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected				
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	N/A	N/A
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	N/A	N/A
Vertical Orifice Diameter =	N/A	N/A	inches			

Calculated Parameters for Vertical Orif

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected				
Overflow Weir Front Edge Height, Ho =	3.48	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	3.48	N/A
Overflow Weir Front Edge Length =	22.00	N/A	feet	Overflow Weir Slope Length =	5.00	N/A
Overflow Weir Grate Slope =	0.00	N/A	H:V	Grate Open Area / 100-yr Orifice Area =	5.30	N/A
Horiz. Length of Weir Sides =	5.00	N/A	feet	Overflow Grate Open Area w/o Debris =	76.56	N/A
Overflow Grate Type =	Type C Grate	N/A		Overflow Grate Open Area w/ Debris =	38.28	N/A
Debris Clogging % =	50%	N/A	%			

Calculated Parameters for Overflow W

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected				
Depth to Invert of Outlet Pipe =	2.50	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	14.44	N/A
Outlet Pipe Diameter =	54.00	N/A	inches	Outlet Orifice Centroid =	2.06	N/A
Restrictor Plate Height Above Pipe Invert =	46.00		inches	Half-Central Angle of Restrictor Plate on Pipe =	2.35	N/A

Calculated Parameters for Outlet Pipe w/ Flow Restriction Pla

Zone 3 Restrictor	Not Selected
14.44	N/A
2.06	N/A
2.35	N/A

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	5.25	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.71	feet
Spillway Crest Length =	100.00	feet	Stage at Top of Freeboard =	6.96	feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	1.58	acres
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =	5.63	acre-ft

Calculated Parameters for Spillway

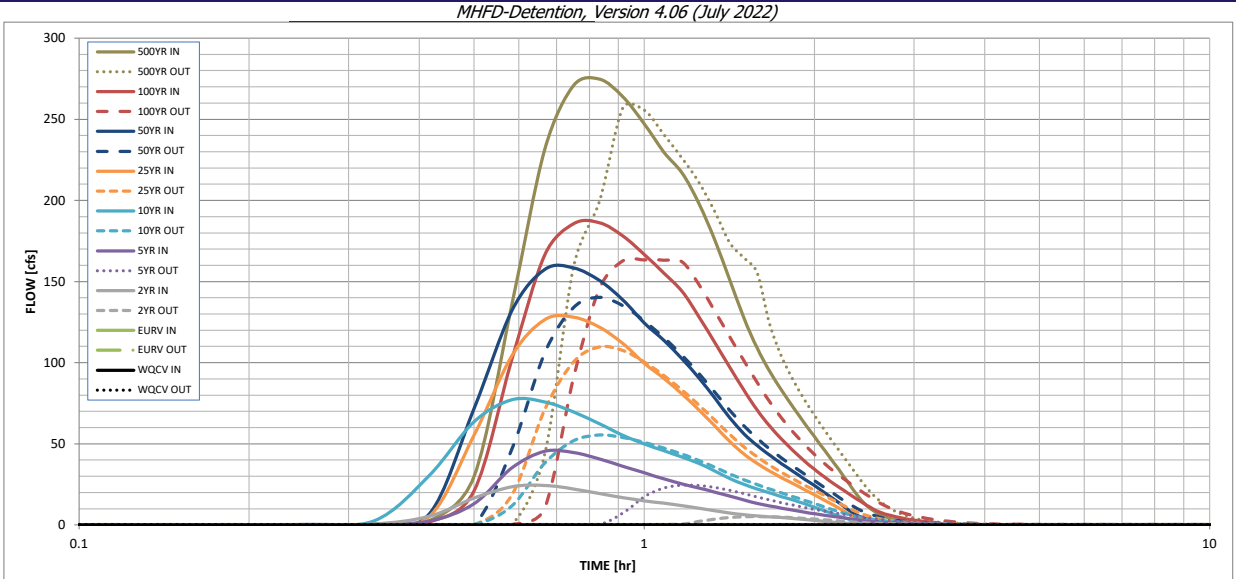
Routed Hydrograph Results

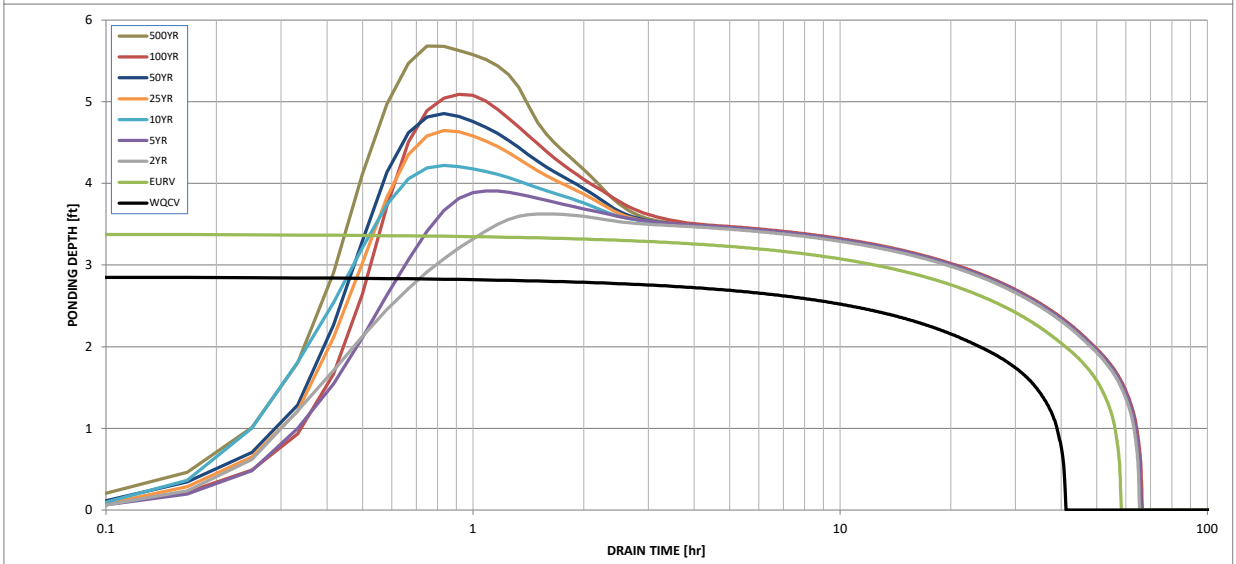
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AI

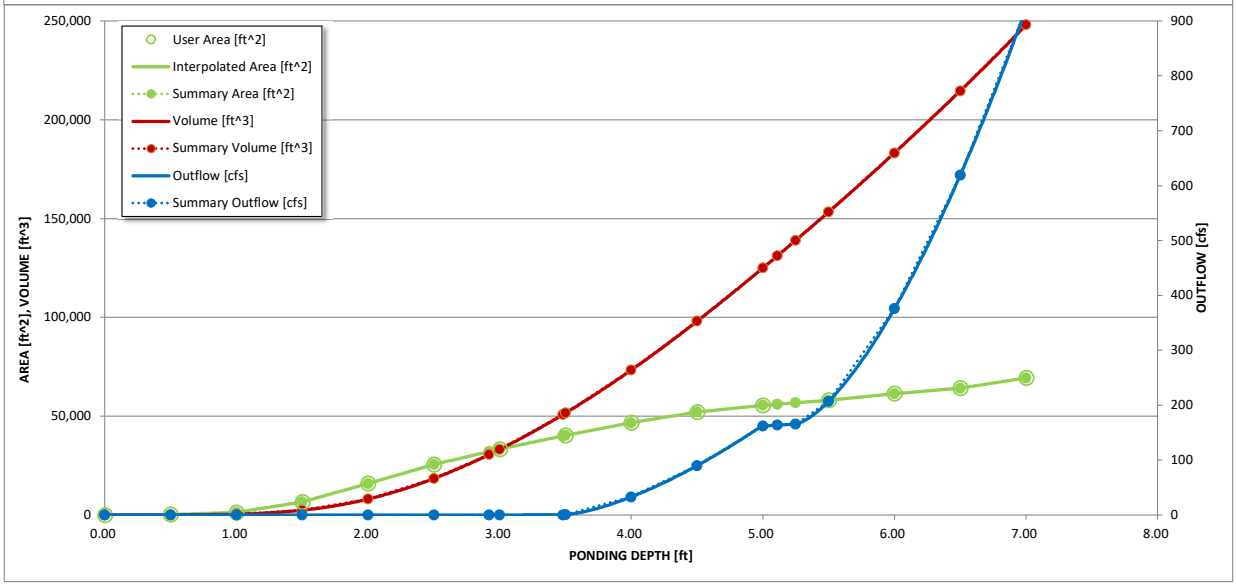
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
CUHP Runoff Volume (acre-ft) =	0.653	1.073	1.617	3.665	5.721	9.429	11.939	15.571
User Override Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	1.617	3.257	5.721	9.429	11.939	15.120
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	16.2	44.9	67.6	120.7	151.3	192.2
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A						160
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.13	0.35	0.53	0.94	1.18	1.25
Peak Inflow Q (cfs) =	N/A	N/A	24.4	45.5	77.1	128.1	158.4	186.3
Peak Outflow Q (cfs) =	0.3	0.3	5.1	24.5	55.4	109.7	140.3	163
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.5	0.8	0.9	0.9	1.0
Structure Controlling Flow =	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1
Max Velocity through Grate 1 (fps) =	N/A	N/A	0.06	0.3	0.7	1.4	1.8	2.1
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	54	59	56	50	43	39	35
Time to Drain 99% of Inflow Volume (hours) =	40	56	62	61	59	56	54	52
Maximum Ponding Depth (ft) =	2.86	3.38	3.62	3.91	4.22	4.64	4.85	5.09
Area at Maximum Ponding Depth (acres) =	0.72	0.89	0.96	1.04	1.12	1.22	1.25	1.28
Maximum Volume Stored (acre-ft) =	0.66	1.08	1.30	1.58	1.91	2.42	2.68	2.97
Elevation (ft) =	7534.86	7535.38		7535.91				7537.09
Pond Bottom (ft) =	7532.00							

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	USER	CUHP	CUHP	CUHP	USER	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.01	0.00	0.00	0.02	0.02	0.05
	0:15:00	0.00	0.00	0.13	0.10	0.27	0.18	0.24	0.11	0.36
	0:20:00	0.00	0.00	0.57	0.30	2.62	0.59	0.71	0.29	2.52
	0:25:00	0.00	0.00	5.36	2.41	30.47	5.12	6.71	1.88	29.80
	0:30:00	0.00	0.00	16.52	13.05	63.62	55.17	71.61	20.97	135.59
	0:35:00	0.00	0.00	23.52	35.13	77.06	104.24	131.89	101.89	231.36
	0:40:00	0.00	0.00	24.39	45.45	75.83	126.55	157.48	166.39	270.75
	0:45:00	0.00	0.00	22.19	44.62	69.58	128.06	158.42	185.68	274.76
	0:50:00	0.00	0.00	19.35	40.59	62.34	121.87	150.62	186.26	263.84
	0:55:00	0.00	0.00	16.90	36.20	55.25	111.77	138.68	178.12	247.25
	1:00:00	0.00	0.00	14.86	32.09	49.89	99.68	124.46	166.72	229.77
	1:05:00	0.00	0.00	13.41	28.35	45.72	90.36	113.74	155.23	216.91
	1:10:00	0.00	0.00	11.90	25.28	41.81	80.69	102.30	143.95	198.12
	1:15:00	0.00	0.00	10.32	22.68	37.94	70.85	90.47	128.32	175.41
	1:20:00	0.00	0.00	8.74	20.23	33.22	60.95	78.14	112.28	151.11
	1:25:00	0.00	0.00	7.33	17.78	28.60	51.51	66.12	97.19	127.77
	1:30:00	0.00	0.00	6.33	15.39	25.05	43.75	56.32	83.19	109.01
	1:35:00	0.00	0.00	5.65	13.34	22.22	37.92	48.97	70.84	94.69
	1:40:00	0.00	0.00	5.07	11.76	19.74	33.24	43.00	60.91	83.00
	1:45:00	0.00	0.00	4.52	10.39	17.46	29.12	37.73	52.90	72.60
	1:50:00	0.00	0.00	3.97	9.08	15.33	25.41	32.98	45.89	63.18
	1:55:00	0.00	0.00	3.42	7.91	13.19	21.93	28.52	39.70	54.39
	2:00:00	0.00	0.00	2.86	6.89	10.99	18.58	24.25	34.29	46.15
	2:05:00	0.00	0.00	2.28	6.00	8.77	15.28	20.00	29.57	38.16
	2:10:00	0.00	0.00	1.71	5.15	6.60	12.00	15.77	25.32	30.31
	2:15:00	0.00	0.00	1.15	4.35	4.57	8.75	11.60	21.47	22.62
	2:20:00	0.00	0.00	0.67	3.59	3.11	5.62	7.60	17.84	15.56
	2:25:00	0.00	0.00	0.41	2.88	2.30	3.47	4.89	14.39	10.60
	2:30:00	0.00	0.00	0.29	2.22	1.79	2.21	3.28	11.19	7.41
	2:35:00	0.00	0.00	0.23	1.72	1.40	1.44	2.24	8.60	5.16
	2:40:00	0.00	0.00	0.18	1.38	1.09	0.93	1.52	6.79	3.50
	2:45:00	0.00	0.00	0.14	1.16	0.83	0.61	1.04	5.51	2.29
	2:50:00	0.00	0.00	0.10	0.99	0.62	0.40	0.70	4.50	1.41
	2:55:00	0.00	0.00	0.08	0.85	0.45	0.25	0.46	3.69	0.83
	3:00:00	0.00	0.00	0.06	0.72	0.31	0.17	0.32	3.02	0.54
	3:05:00	0.00	0.00	0.05	0.60	0.22	0.12	0.23	2.46	0.39
	3:10:00	0.00	0.00	0.04	0.50	0.16	0.09	0.17	1.99	0.31
	3:15:00	0.00	0.00	0.03	0.40	0.12	0.07	0.13	1.60	0.24
	3:20:00	0.00	0.00	0.02	0.31	0.09	0.05	0.10	1.26	0.18
	3:25:00	0.00	0.00	0.02	0.23	0.06	0.04	0.07	0.94	0.13
	3:30:00	0.00	0.00	0.01	0.17	0.04	0.02	0.05	0.65	0.09
	3:35:00	0.00	0.00	0.01	0.12	0.02	0.02	0.03	0.46	0.06
	3:40:00	0.00	0.00	0.00	0.10	0.01	0.01	0.02	0.33	0.03
	3:45:00	0.00	0.00	0.00	0.08	0.00	0.00	0.01	0.24	0.01
	3:50:00	0.00	0.00	0.00	0.06	0.00	0.00	0.00	0.17	0.00
	3:55:00	0.00	0.00	0.00	0.05	0.00	0.00	0.00	0.12	0.00
	4:00:00	0.00	0.00	0.00	0.04	0.00	0.00	0.00	0.09	0.00
	4:05:00	0.00	0.00	0.00	0.03	0.00	0.00	0.00	0.06	0.00
	4:10:00	0.00	0.00	0.00	0.03	0.00	0.00	0.00	0.04	0.00
	4:15:00	0.00	0.00	0.00	0.02	0.00	0.00	0.00	0.03	0.00
	4:20:00	0.00	0.00	0.00	0.02	0.00	0.00	0.00	0.03	0.00
	4:25:00	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.02	0.00
	4:30:00	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.02	0.00
	4:35:00	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.01	0.00
	4:40:00	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.01	0.00
	4:45:00	0.00	0.00	0.00	0.01	0.00	0.00	0.00	0.01	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00