# Retreat at Timber Ridge Preliminary Plan Traffic Impact Analysis PCD File No: <br> $\qquad$ (LSC \#174030) <br> April 12, 2018 

## Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.


## Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.
 CONSULTANTS, INC.

April 12, 2018

Mr. Peter Martz
Arroyo Investments
P.O. Box 50223

Colorado Springs, CO 80949
E-mail: Isc@lsctrans.com
Website: http://www.Isctrans.com

RE: Retreat at Timber Ridge<br>Preliminary Plan<br>El Paso County, Colorado<br>Transportation Memorandum<br>LSC \#174030

Dear Mr. Peter:

LSC Transportation Consultants, Inc. has prepared this transportation memorandum to accompany the preliminary plan submittal for the Retreat at Timber Ridge residential development to be located generally east of Vollmer Road and south of Arroya Lane in El Paso County, Colorado. The site location is shown in Figure 1. LSC prepared a traffic impact study (TIS) for the entire Retreat at Timber Ridge PUD development plan dated January 25, 2018. A copy of this report is attached. The lot and street plan has not changed since completion of that report. This memorandum contains the following:

- The proposed land use and roadway improvement phasing.
- The projected average weekday and peak-hour vehicle-trips to be generated by the currently proposed preliminary plan land uses by phase.
- Recommendations for street functional classifications for roads and streets within the preliminary plan.
- The projected timing of the required roadway improvements and turn lane improvement on Vollmer Road.


## SITE DEVELOPMENT AND LAND USE

## Land Use

The currently proposed Retreat at Timber Ridge Preliminary Plan area includes the 203 lots for single-family homes located east of Vollmer Road and 2 lots for single family homes located west of Vollmer Road and south of Arroya Lane. The currently proposed preliminary plan does not include the area northwest of Vollmer Road/Arroya Lane. The lot layout, street network, and
access points from the plan shown in the January 2018 PUD TIS for this area have not changed; however, the phasing plan has changed. Figure 1 shows the currently proposed phasing plan.

As part of Phase 1, eighty feet of right-of-way will be dedicated for Arroya Lane; however, Arroya Lane will remain a gravel road until a later phase when the 200-average-daily-traffic threshold requiring paving is reached. The eighty-foot right-of-way is being dedicated to accommodate a future expansion of the roadway cross section if ever needed in the future. The storm sewer crossing under Arroya Lane will be built with Phase 1. The existing temporary turnaround on the east end of Arroya Lane will be moved to coincide with the intersection of Arroya/Nature Refuge Road. A 50-foot pavement apron will be constructed at the tie-in to Vollmer Road. Nature Refuge will be constructed as a gravel road with this phase.

Phases 2 and 3 are planned to be constructed concurrently. Poco Road will be extended east through the site including the creek crossing. The north/south street segments through Phase 4 will be constructed as a gravel road as part of Phases 2 and 3 to provide a secondary emergency access. This section will be upgraded to Urban Local standards with Phase 4. Phase 2 is also planned to include the construction of an interim access for the two lots west of Vollmer Road and south of Arroya Lane to Vollmer Road 440 feet south of the existing Arroya/Vollmer intersection. A deviation request for this interim access has been submitted and is currently under review.

Arroya will be upgraded to paved urban local roadway section, either the section from Vollmer to the Filing 5 access or the entire section from Vollmer to the temporary turnaround at Nature Refuge Road, depending on the timing of Phases 4 and 5 and the projected average daily traffic. (Once the average daily traffic exceeds 200 vehicles per day, upgrade would be required.)

## DEVIATION REQUESTS

The following deviation request are either currently under review or are being e submitted with this preliminary plan application:

- Deviation to allow for a temporary access to the west side Vollmer Road south of Arroya Lane (currently under review)
- Deviation to permit Nature Refuge Drive to be a gravel road
- Deviation to permit delay of 50-foot paved apron on Nature Refuge Way until Arroya Lane is paved
- Deviation to permit an interim cul-de-sac length of greater than $1 / 4$ mile for the 10 lots on Nature Refuge Way
- Deviation to permit an emergency access as a second access for lots east of Sand Creek Waiver of the LDC. More than 25 lots on a dead-end road (2nd access).


## TRIP GENERATION

Estimates of the traffic volumes expected to be generated by the site by phase have been made using the nationally published trip generation rates found in Trip Generation, $10^{\text {th }}$ Edition, 2017 by the Institute of Transportation Engineers (ITE). Table 1 shows the results of the trip generation estimates.

## TRIP ASSIGNMENT

Table 2 shows the projected average weekday traffic volumes on Poco Road and Arroya Lane just east of Vollmer Road following buildout of each phase of the preliminary plan. Table 2 also shows the projected short-term peak-hour entering and exiting traffic volumes at the intersections of Vollmer/Poco and Vollmer/Arroya for each phase. These volumes are based on the projected trip generation estimates shown in Table 1 and the short-term directional distribution estimate shown in Figure 7 from the Retreat at Timber Ridge Updated Traffic Impact Analysis dated January 25, 2018.

## SHORT-TERM AND 2040 TOTAL TRAFFIC

Please refer to the Retreat at Timber Ridge Updated Traffic Impact Analysis dated January 25, 2018 for the short-term and 2040 total traffic volumes and level of service analysis. The 2040 total traffic volumes include traffic estimated to be generated by the parcel northwest of Vollmer Road/Arroya Lane; however, this parcel is not part of the currently proposed preliminary plan.

## ROADWAY CLASSIFICATIONS

Figure 2 shows updated recommended street classifications for Vollmer Road and Arroya Lane adjacent to the site and for the internal streets within the Preliminary Plan area. The only change from the classifications shown in Figure 12 of the Retreat at Timber Ridge Updated Traffic Impact Analysis dated January 25, 2018 is that Arroya Lane is now shown as a Rural Local instead of a Minor Rural Collector.

## ROADWAY IMPROVEMENT FEE PROGRAM

This project will be required to participate in the El Paso County Road Improvement Fee Program. The details will be identified with each final plat.

## RECOMMENDED IMPROVEMENTS

- Table 3 contains a summary of the needed improvements. This represents an updated version from the PUD traffic report - primarily due to the new phasing plan.
- Vollmer Road:
- Short Term: As shown on Figure 10 of the TIS, the projected 2020 average weekday traffic volume on Vollmer Road just south of the site is 5,360 vehicles per day. This includes buildout of the currently proposed preliminary plan area and buildout of the Sterling Ranch parcel just south of the site. Currently, the MTCP indicates the capacity of existing Vollmer Road to be about 6,000 vehicles per day.
- Long Term: The County MTCP shows a Vollmer Road upgrade between Poco Road and Shoup Road to a county-standard, two-lane Rural Minor Arterial. Traffic volume estimates indicate this improvement will not be needed in the short-term horizon. The 2040 MTCP indicates the Vollmer project will be needed by 2040. The 2040 MTCP shows the Vollmer upgrade "project" as Project ID U-12. The Retreat at Timber Ridge will dedicate right-of-way to accommodate the future upgrade to Rural Minor Arterial standards (As shown in the MTCP and the Fee Study); The applicant will be required to participate in the County Road Impact Fee program.
- Vollmer/Poco Intersection: Based on the projected site-generated traffic volumes shown in Table 3, a northbound right-turn deceleration lane will likely be required on Vollmer Road approaching Poco Road with Phase 5. This assumes no development of the Sterling Ranch parcels located to the east of Retreat at Timber Ridge or just south of the site that will share this access point.


## - Arroya Lane:

- As part of Phase 1, 80 feet of right-of-way will be dedicated for Arroya Lane; however, Arroya Lane will remain a gravel road until a later phase when the 200 -vehicle-perday, average-daily-traffic threshold requiring paving is reached. The 80-foot right-ofway is being dedicated for purposes of accommodating a future expansion of the roadway cross section if ever needed in the future.
- The storm sewer crossing under Arroya Lane will be built with Phase 1. Phase 1 improvements would involve grading and improving the roadway to an interim allweather, gravel cross section acceptable to the County and the fire district and suitable for two-way traffic and emergency vehicles the from Vollmer to Nature Refuge Road.
- The existing temporary turnaround on the east end of Arroya Lane will be moved to coincide with the intersection of Arroya/Nature Refuge Road and a 50-foot apron will be constructed at the tie-in to Vollmer Road.
- Based on the projected average weekday traffic volumes shown in Table 2, Arroya Lane will need to be paved with Phase 4. Should Phase 5 precede Phase 4 it would only be necessary to pave between Vollmer Road and the Phase 5 access point.
- Nature Refuge Road: The applicant proposes to construct Nature Refuge Road as a permanent, County-standard Gravel road with Phase 1. Traffic volumes will remain under 200 ADT. A 50-foot paved apron at Arroya Lane would be constructed. The applicant proposes this improvement be delayed to coincide with the paving of Arroya Lane.
- As shown on Figure 10 of the TIS, all movements at the intersections of Poco/Vollmer and Arroya/Vollmer are projected to operate at LOS B or better during the peak hours based on the 2020 total traffic volumes assuming the existing cross section of Vollmer Road except for the addition of a northbound right-turn lane approaching Poco Road.

Please contact me if you have any questions regarding this report.
Respectfully Submitted,

LSC TRANSPORTATION CONSULTANTS, INC.

JCH:KDF:bjwb

Enclosures: Tables 1-3
Figures 1-2
Retreat at Timber Ridge Updated Traffic Impact Analysis dated January 25, 2018

| $\begin{gathered} \text { Table } 1 \\ \text { Trip Generation Estimate } \\ \text { Retreat at Timber Ridge Preliminary Plan } \end{gathered}$ |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Phase | Land <br> Use <br> Code | $\qquad$ | Trip Generation Units | Trip Generation Rates ${ }^{(1)}$ |  |  |  |  | Total Trips Generated |  |  |  |  |
|  |  |  |  | Average Weekday Traffic | Morning Peak Hour |  | Afternoon Peak Hour |  | Average Weekday Traffic | Morning Peak Hour |  | Afternoon Peak Hour |  |
|  |  |  |  |  |  |  | In | Out |  | In | Out | In | Out |
| Currently Proposed Preliminary Plan |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 1 | 210 | Single-Family Detached Housing | $10 \mathrm{DU}^{(2)}$ | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 94 | 2 | 6 | 6 | 4 |
| 2 | 210 | Single-Family Detached Housing | 13 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 123 | 2 | 7 | 8 | 5 |
| 3 | 210 | Single-Family Detached Housing | 59 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 557 | 11 | 33 | 37 | 22 |
| 4 | 210 | Single-Family Detached Housing | 33 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 312 | 6 | 18 | 21 | 12 |
| 5 | 210 | Single-Family Detached Housing | 15 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 142 | 3 | 8 | 9 | 5 |
| 6 | 210 | Single-Family Detached Housing | 75 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 708 | 14 | 42 | 47 | 27 |
|  |  |  | 205 DU |  |  |  |  |  | 1,935 | 38 | 114 | 128 | 75 |
| Future Filings (Part of the overall PUD but not a part of the currently proposed Preliminary Plan) |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  | 210 | Single-Family Detached Housing | 7 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 66 | 1 | 4 | 4 | 3 |
|  | Total at | Buildout of Retreat at Timber Ridge | 212 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 2,001 | 39 | 118 | 132 | 78 |
| Notes: <br> (1) Source: "Trip Generation, 10th Edition, 2017" by the Institute of Transportation Engineers (ITE) <br> (2) $\mathrm{DU}=$ dwelling unit |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Source: LSC Transportation Consultants, Inc. |  |  |  |  |  |  |  |  |  |  |  |  |  |


|  |  |  |  |  |  |  | Short reat at T | $\begin{array}{r} \mathrm{Tab} \\ \mathrm{t} \text {-Term } \mathrm{Tr} \end{array}$ Cimber Ri | ble 2 Trip Assign Ridge Preli | gnment liminary |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Phase | Weekday Traffic East of Vollmer (vehicles per day) |  | Morning Peak Hour (vehicles per hour) |  |  |  |  |  |  |  | Afternoon Peak Hour (vehicles per hour) |  |  |  |  |  |  |  |
|  |  |  | Entering Traffic |  |  |  | Exiting Traffic |  |  |  | Entering Traffic |  |  |  | Exiting Traffic |  |  |  |
|  |  |  | Poco Rd |  | Arroya Ln |  | Poco Rd Arroya Ln |  |  |  | Poco Rd |  | Arroya Ln |  | Poco Rd |  | Arroya Ln |  |
|  | Poco | Arroya | NB RT | SB LT | NB RT | SB LT | WB LT | WB RT | WB LT | WB RT | NB RT | SB LT | NB RT | SB LT | WB LT | WB RT | WB LT | WB RT |
| Existing | 0 | 30 | 0 | 0 | 1 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 1 | 0 |
| 1 | 0 | 124 | 0 | 0 | 3 | 0 | 0 | 0 | 7 | 1 | 0 | 0 | 7 | 1 | 0 | 0 | 4 | 1 |
| 2 | 123 | 124 | 2 | 0 | 3 | 0 | 5 | 1 | 7 | 1 | 6 | 1 | 7 | 1 | 3 | 1 | 4 | 1 |
| 3 | 680 | 124 | 11 | 1 | 3 | 0 | 33 | 6 | 7 | 1 | 37 | 6 | 7 | 1 | 22 | 3 | 4 | 1 |
| 4 | 858 | 258 | 14 | 1 | 5 | 0 | 43 | 6 | 12 | 3 | 49 | 7 | 13 | 4 | 29 | 4 | 7 | 2 |
| 5 | 905 | 334 | 15 | 1 | 6 | 0 | 47 | 6 | 15 | 4 | 53 | 7 | 16 | 5 | 31 | 4 | 9 | 3 |
| 6 | 1,587 | 355 | 28 | 4 | 4 | 2 | 82 | 11 | 13 | 6 | 93 | 12 | 14 | 7 | 55 | 7 | 10 | 4 |
| Source: LSC Transportation Consultants, Inc. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |


| Table 3Roadway ImprovementsRetreat at Timber RidgePreliminary Plan |  |  |
| :---: | :---: | :---: |
| Improvement | Timing | Responsibility ${ }^{\text {(1) }}$ |
| Arroya Lane Initial/interim: Dedicate $80^{\prime}$ of ROW or $40^{\prime}$ half ROW where applicable; Construct a storm sewer crossing under Arroya Lane; regrade and improve the roadway to an interim all-weather, gravel cross section for two-way traffic and emergency vehicles suitable to the County and the fire district the from Vollmer to Nature Refuge Road; move the existing temporary turnaround on the east end of Arroya Lane to coincide with the intersection of Arroya/Nature Refuge Road and construct a 50 -foot apron at the tie-in to Vollmer Road. | Phase 1 | The Retreat at Timber Ridge |
| Upgrade Arroya Lane to a Rural Local cross section (paved). Along with this improvement, construct a 50 -foot apron on Nature Refuge Road at the tie-in to Arroya Lane. | Once the average weekday traffic volume exeeds 200 vehicles per day. This is projected to occur with either Phase 4 or 5 | The Retreat at Timber Ridge |
| Realign Arroya Lane at the intersection of Vollmer Road/Arroya Lane so Arroya intersects Vollmer at a right angle. | Phases 4 or 5 | The Retreat at Timber Ridge |
| Extend Poco Road to the east including the creek crossing | Phases 2 and 3 | The Retreat at Timber Ridge |
| Construct a gravel road to provide secondary emergency access through the Phase 4 area to Arroya Lane (this gravel road would be replaced with the subdivision streets in Phase 4). | Phases 2 and 3 | The Retreat at Timber Ridge |
| Construct a northbound right-turn deceleration lane on Vollmer Road approaching Poco Road. | Design and installation with the applicable final plat(s) for The Retreat at Timber Ridge. This turn lane is projected to be required with Phase 5. | The Retreat at Timber Ridge |
| Potential improvement: Southbound left-turn lane at Arroyo | Evaluation with final plats. Although the anticipated traffic counts do not warrant it, the County Engineer may require a southbound left-turn lane at Arroyo based on unanticipated traffic patterns [from Staff Comments]. | The Retreat at Timber Ridge and/or possible-but-not-currently-anticipated-future development with access via Arroya |
| Possible future modern roundabout intersection control at Poco/Vollmer as an alternative to the two-way, Stopsign control (TWSC) shown in this TIS | Consideration of roundabout trafic control instead of TWSC could be addressed with the applicable final plat(s) for The Retreat at Timber Ridge and/or Sterling Ranch. Roundabouts would require signifant circulat right-of way around the center of the intersection. Curentig additional right-of-way to accomodate a roundabout(s) is not available on the west side of Vollmer. Also, the southeast torner of the intersection is not part of this project and is not owned by this applicant. It is owned by Sterling Ranch. The consideration is that although the TIS shows better sidide-street level of service with the roundataout, the projected approach traffic volumes are not close to being equal on all the intersection approaches. The northbound and southboound through volumes are significantly higher than the eastbound and westbound volumes. The balance of approach volumes is an element tod consider when evaluating a roundabout as a potential traffic control solution. | The Retreat at Timber Ridge andor Stering Ranch |
| As shown on the County MTCP: Vollmer Road upgrade between Poco Road and Shoup Road to a countystandard, two-lane Rural Minor Arterial. | Traffic <br> volume estimates indicate this improvement will not be needed in the short term horizon. The 2040 MTCP indicates the Vollmer project will be needed by 2040. The 2040 MTCP shows the Vollmer upgrade "project" as Project ID U-12. | The Retreat at Timber Ridge will dedicate right-of-way to accommodate the future upgrade to Rural Minor Arterial standards (As shown in the MTCP and the Fee Study); The applicant will be requred to participate in the County Road Impact Fee program. |
| Upgrade Vollmer Road between future Stapleton Drive and Poco Road to an Urban Minor Arterial cross section (five lanes) |  | (Sterling Ranch Metro District) MTCP Master-Planned MTCP Project ID U-12 |
| Upgrade Vollmer Road generally between the south boundary of Sterling Ranch and future Stapleton Drive to an Urban Minor Arterial cross section (five lanes) | Designed MTCP Project ID C-13 | Stering Ranch Metro District |
| Upgrade Vollmer Road generally between Cowpoke Road and the south boundary of Sterling Ranch to an Urban Minor Arterial cross section (five lanes) | Designed MTCP Project ID C-13 | Woodmen Heights Metro District |
| Construct section of Stapleton Road half section between Vollmer Road and the first Sterling Ranch access point | With development of Phase 1 of Sterling Ranch - Designed MTCP Project ID N-5 | Stering Ranch Metro District |
| Construct a northbound right-turn deceleration lane on Vollmer Road approaching Stapleton Road | With development of Phase 1 of Sterling Ranch - Designed MTCP Project ID C-13 | Sterring Ranch Metro District |
| Construct Briargate Parkway (four-lane Principal Arterial) between Black Forest Road and Vollmer Road. | $\begin{gathered} \text { Future - TBD } \\ \text { TBD with PPRTA }{ }^{(2)} \text { Corridor Study } \end{gathered}$ | TBD with PPRTA ${ }^{(2)}$ Corridor Study MTCP Project N-5 |
| Construct Stapleton Drive between Vollmer Road and Towner | Future <br> TBD with PPRTA ${ }^{(2)}$ Corridor Study | TBD with PPRTA ${ }^{(2)}$ Corridor Study MTCP Project N-5 |
| Southbound left-turn lanes on Vollmer Road approaching Burgess Road | Exisiting Deficiency | Existing Deficiency - Others (This development will not add volume to this turning movement.) |
| Northbound left-tur lane at BurgessVollmer | Projections indicate after 2020 but prior to 2040 the turning volume threshold warranting the turn lane ( 25 northbound left turns per hour) would be exceeded. | Based on the revised PUD plan, the afternoon peakhour traffic impact from this project on the northbound approach to this intersection is projected to be below 10 percent. The site volume on the roadway link (both directions of travel) south of the intersection is more than 10 percent, however the turn lane thresholds are shown to be exceeded on the northbound approach during the afternoon peak hour when the impact of this project is below 10 percent on this approach. This project will be participating in the Fee Progam and the MTCP Project ID is U-12. |
| Northbound right-tur lane at Burgess/Volmer | Projections indicate by 2020 the turning volume threshold warranting the turn lane ( 50 northbound right turns per hour) would be exceeded. | Based on the revised PUD plan, the afternoon peakhour traffic impact from this project on the northbound approach to this intersection is projected to be below 10 percent. The site volume on the roadway link (both directions of travel) south of the intersection is more than 10 percent, however the turn lane thresholds are shown to be exceeded on the northbound approach during the afternoon peak hour when the impact of this project is below 10 percent on this approach. This project will be participating in the Fee Progam and the MTCP Project ID is U-12. |
| Future trafic signal at StapletonNollmer | Once warrants are met; analysis to be included with final plat traffic reports; projections indicate by 2040 the intersection would be signaliized. | Escrow a fair-share amount toward the cost the signal (to be determined with final plats). Once the signal is constructed, a portion of the escrow amount used to fund the installation of the signal may have become creditable under the Fee Program (if this signal is added to the fee program list of signals eligible for credit (County signals not currently programmed in Fee Program). |
| Notes: <br> (1) Preliminary concept of responsibility; the actual construction responsibility would be determined through subdivision applications and cost recovery if applicable agreements. <br> (2) PPRTA = Pikes Peak Rural Transportation Authority. <br> Source: LSC Transportation Consultants, Inc. (date:4-11-18) |  |  |



Figure 2
Recommended Classifications

Note (4-12-18): This PUD report has been included for reference. The more recent Preliminary Plan report addresses phasing and other updates to TRANSPORTATIC match the Preliminary
Plan.

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Retreat at Timber Ridge Updated Traffic Impact Analysis<br>PCD File No: PUD-17-003<br>(LSC \#174030)<br>January 25, 2018

## Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.


## Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.


January 25, 2018
Mr. Peter Martz
Arroyo Investments
P.O. Box 50223

Colorado Springs, CO 80949

RE: Retreat at Timber Ridge<br>Updated Traffic Impact Analysis<br>El Paso County, CO<br>LSC \#174030

Dear Peter:

In response to your request, LSC Transportation Consultants, Inc. has prepared this updated traffic impact analysis for the proposed Retreat at Timber Ridge residential development to be located generally east of Vollmer Road and south of Arroya Lane in El Paso County, Colorado. Figure 1 shows the site location.

## REPORT CONTENTS

The report contains the following:

- The existing roadway and traffic conditions in the site's vicinity including the roadway widths, surface conditions, lane geometries, traffic controls, and posted speed limits, etc.
- The existing traffic volumes on the area roadways.
- The projected average weekday and peak-hour vehicle-trips to be generated by the proposed development.
- The assignment of the projected site-generated traffic volumes to the area roadways.
- The projected total traffic volumes on the area roadways.
- The projected levels of service at the site access points and the key adjacent intersections.
- The resulting traffic impacts.
- Recommendations for roadway improvements.


## SITE LAND USE

The site is located generally east of Vollmer Road and south of Arroya Lane. There are existing singlefamily homes west and north of the site. The vacant parcels south and east of the site are part of the planned Sterling Ranch development. These parcels are planned to be developed for single-family homes.

The site is planned to be developed with 212 lots for single-family homes. Nine of these lots are located west of Vollmer Road and 10 of these lots are located north of Arroya Lane. The site plan is shown in Figure 2.

Access for the lots east of Vollmer Road is proposed to Vollmer Road aligning with Poco Road and to Arroya Lane. Three full-movement access points are proposed to Arroya Lane. The spacing of the proposed Arroya Lane access points is shown in Figure 2. A deviation request for the location of these access points has been approved. The site plan also shows future connections through the Sterling Ranch development east and south of the site.

An interim access for the two lots west of Vollmer Road and south of Arroya Lane is planned to Vollmer Road 440 feet south of the existing Arroya/Vollmer intersection (which is planned to be realigned 110 feet to the north). A deviation request for this interim access has been submitted. Once Tract A, which is located west of Vollmer Road and north of Arroya Lane, is developed this interim access would be closed and the two lots south of Arroya and the seven lots planned for Tract A would have access somewhere to the north. This report assumes access for these lots will be to Vollmer Road aligning with Arroya Lane.

## EXISTING ROADWAY AND TRAFFIC CONDITIONS

## Area Roadways

The roadways in the study area are shown on Figure 1 and are described below.

- Vollmer Road is a two-lane, rural, paved roadway north of Cowpoke Road extending to north of Hodgen Road. Vollmer Road has a posted speed limit of 45 miles per hour (mph). It is currently a five-lane urban street within the City of Colorado Springs limits between Black Forest Road and Cowpoke Road The 2040 El Paso County Major Transportation Corridors Plan (MTCP) shows Vollmer Road as a two-lane Rural Minor Arterial adjacent to the site.
- Burgess Road is a two-lane Rural Minor Arterial that extends east from Milam Road to Goodson Road. The posted speed limit on Burgess Road in the vicinity of Vollmer Road is 45 mph .
- Briargate Parkway is a six-lane, Principal Arterial that extends east from I-25 to Grand Lawn Circle (about one-half mile east of Powers Boulevard). Briargate Parkway is planned to ultimately extend to Vollmer Road. The County MTCP shows Briargate/Stapleton east of Black Forest Road as a four-lane Principal Arterial.
- Stapleton Drive is shown as a four-lane Principal Arterial on the El Paso County MTCP. Stapleton Drive currently extends east from just west of Towner Drive across Eastonville Road to Curtis Road. Stapleton Drive is planned to be extended west to connect to Briargate Parkway in the future.


## Existing Traffic Conditions

Figure 3 shows the current morning and afternoon peak-hour traffic volumes at the intersections of Vollmer Road/Poco Road and Vollmer Road/Burgess Road based on counts conducted by LSC in February, March, and June 2017. The traffic count reports are attached.

## Existing Levels of Service

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection. Level of service is indicated on a scale from "A" to "F." LOS A represents control delay of less than

10 seconds for unsignalized and signalized intersections. LOS F represents control delay of more than 50 seconds for unsignalized intersections and more than 80 seconds for signalized intersections. Table 1 shows the level of service delay ranges.

| Table 1 <br> Intersection Levels of Service Delay Ranges |  |  |  |
| :---: | :---: | :---: | :---: |
| Signalized Intersections | Unsignalized Intersections |  |  |
|  | Average Control Delay <br> (seconds per vehicle) | V/C ${ }^{(1)}$ | Average Control Delay <br> (seconds per vehicle) ${ }^{(2)}$ |
|  | 10.0 sec or less | less than 0.60 | 10.0 sec or less |
| B | $10.1-20.0 \mathrm{sec}$ | $0.60-0.69$ | $10.1-15.0 \mathrm{sec}$ |
| C | $20.1-35.0 \mathrm{sec}$ | $0.70-0.79$ | $15.1-25.0 \mathrm{sec}$ |
| D | $35.1-55.0 \mathrm{sec}$ | $0.80-0.89$ | $25.1-35.0 \mathrm{sec}$ |
| E | $55.1-80.0 \mathrm{sec}$ | $0.90-0.99$ | $35.1-50.0 \mathrm{sec}$ |
| F | 80.1 sec or more | 1.00 and greater | 50.1 sec or more |

The intersections of Vollmer Road/Poco Road and Vollmer Road/Burgess Road were analyzed to determine the existing levels of service based on the unsignalized method of analysis procedures found in the Highway Capacity Manual, 6th Edition by the Transportation Research Board. Figure 3 shows the level of service analysis results. The level of service (LOS) reports are attached.

As shown on the figure, all movements the intersection of Vollmer/Poco are currently operating at a level of service A during the peak hours. All movements at the intersection of Vollmer/Burgess are currently operating at LOS C or better during the peak hours.

## SIGHT DISTANCE

Figure 4 shows the sight distance analysis for the Arroya Lane access points. The analysis is based on a design speed of 40 miles per hour.

## SHORT-TERM (2020) BACKGROUND TRAFFIC

Background traffic is the traffic estimated to be on the adjacent roadways and at adjacent intersections without the proposed development's trip generation and resulting site-generated traffic volumes. Background traffic includes increases in the through traffic and the traffic generated by adjacent and nearby developments, but assumes zero traffic generated by the site. Figure 5 shows the background traffic for the short term (Year 2020). The short-term background traffic volumes are based on some growth in existing through volumes on Vollmer Road shown in Figure 3, plus the addition of traffic generated by Phase 1 of the Sterling Ranch development located just east of Vollmer Road and south of the future Stapleton Drive, and traffic generated by Sterling Ranch North located east of Vollmer Road between the future Stapleton Drive and future extension of Poco Road. The short-term background volumes
assume Stapleton Drive and Briargate Parkway will not be constructed in the vicinity of the site in the short term other than the short segment needed for access for Phase 1 of Sterling Ranch.

## 2040 BACKGROUND TRAFFIC

Figure 6 shows the background traffic volumes for the year 2040. The 2040 background traffic volume estimates were based on 2040 volume projections in the El Paso County Major Transportation Corridors Plan (MTCP) and previous work completed in the area by LSC, including the Sterling Ranch Updated Traffic Impact Analysis by LSC dated June 5, 2008. The 2040 background traffic includes buildout of the Sterling Ranch development including the future connections to the Sterling Ranch parcels east of the Retreat at Timber Ridge Site. The 2040 background traffic also assumes a Stapleton Drive extension to the west to Vollmer Road and a Briargate Parkway extension east to Vollmer Road.

## TRIP GENERATION

Estimates of the traffic volumes expected to be generated by the existing and proposed land uses within the study area were made using the nationally published trip generation rates found in Trip Generation, 10th Edition, 2017 by the Institute of Transportation Engineers (ITE). Table 2 shows the trip generation estimates.

At buildout the site is projected to generate about 2,001 new external vehicle-trips on the average weekday, with about half entering and half exiting the site during a 24 -hour period. During the morning peak hour, which generally occurs for one hour between 6:30 and 8:30 a.m., about 39 vehicles would enter and 118 vehicles would exit the site. During the afternoon peak hour, which generally occurs for one hour between $4: 15$ and $6: 15 \mathrm{p} . \mathrm{m}$., about 131 vehicles would enter and 78 vehicles would exit the site.

## TRIP DISTRIBUTION AND ASSIGNMENT

The estimated directional distribution of the site-generated traffic volumes on the adjacent roadways is an important factor in determining the site's traffic impacts. Figure 7 shows the short-term and longterm directional distribution estimates for the site-generated traffic. The estimates have been based on the following factors: the site's location with respect to the City of Colorado Springs metropolitan area and other developed areas; the site's proposed land use; the site's proposed access points; the roadway system serving the site; and the existing traffic counts. The short-term directional distribution estimates assume the proposed future extensions of Stapleton Drive and Briargate Parkway will not be constructed in the vicinity of the site in the short term other than the short segments needed for access for Phase 1 of Sterling Ranch. The long-term directional distribution estimate assumes buildout of the future street network including a Stapleton Drive extension to the west to Vollmer Road and a Briargate Parkway extension east to Vollmer Road. The long-term distribution estimate also assumes the future connections on the east side of the site will connect to a new north/south collector street through the Sterling Ranch parcel just east of the site.

When the distribution percentages (from Figure 7) were applied to the trip generation estimates (from Table 2), the site-generated traffic volumes on the area roadways were determined. Figure 8 shows the
short-term site-generated traffic volumes for Phases A through D only. The short-term site-generated traffic volumes do not include estimates of traffic projected to be generated by future development of the seven lots planned for Tract A located west of Vollmer Road and north of Arroya Lane. Access for the two lots west of Vollmer Road and south of Arroya Lane was assumed at the proposed interim location 440 feet south of the existing Arroya/Vollmer intersection. Figure 9 shows the long-term buildout site-generated traffic volumes. The long-term site-generated traffic volumes assume the interim access has been closed and access for the nine lots located west of Vollmer Road aligning with Arroya Lane.

## SHORT-TERM TOTAL TRAFFIC

Figure 10 shows the short-term total traffic volumes at the access points and key intersections in the vicinity of the site. The volumes are the sum of the short-term background traffic volumes from Figure 5, plus the short-term site-generated traffic volumes from Figure 8.

Figure 10 also shows the lane geometry, traffic control, and level of service at the site access points and key intersections based on the short-term total volumes.

## 2040 TOTAL TRAFFIC

Figure 11 shows the 2040 total traffic volumes at the site access points and key intersections in the vicinity of the site. The volumes are the sum of the 2040 background traffic volumes from Figure 6, plus the long-term site-generated traffic volumes from Figure 9.

Figure 11 also shows the lane geometry, traffic control, and level of service at the key intersections based on the 2040 total volumes.

## PROJECTED LEVELS OF SERVICE

## Intersection Levels of Service

The site access point intersections and other key area intersections have been analyzed to determine the projected levels of service based on the short-term and 2040 total traffic volumes. The intersections were analyzed based on the unsignalized method of analysis procedures found in the Highway Capacity Manual, 6th Edition by the Transportation Research Board. The intersection of Vollmer/ Briargate/Stapleton was analyzed as a signalized intersection for the projected long-term conditions using Synchro. The level of service reports are attached. Figures 5, 6, 10, and 11 show the level of service analysis results.

The intersections of Vollmer/Poco and Vollmer/Arroya and the proposed site access point to Vollmer just south of Arroya are projected to operate at a satisfactory level of service (satisfactory according to the County standards, which is LOS D or better) as stop-sign-controlled intersections based on the projected short-term and 2040 total traffic volumes.

All movements at the intersection of Vollmer/Burgess are projected to operate at LOS D or better based on the projected 2020 total traffic volumes. By 2040 the eastbound and westbound approaches at this
intersection are projected to operate at LOS F during the afternoon peak hour based on both background and total traffic volumes assuming the current two-way stop-sign control (TWSC). If this intersection were converted to all-way, stop-sign control (AWSC) all movements are projected to operate at LOS C or better.

The intersection of Vollmer/Stapleton is projected to operate at a satisfactory level of service (LOS D or better) as a stop-sign-controlled intersection based on the short-term total traffic. This analysis assumes Stapleton Road has only been extended east of Vollmer Road to serve the planned Phase 1 development of Sterling Ranch. By 2040, it was assumed that Briargate Road would be extended east to Vollmer Road and Stapleton Drive would be extended east to connect to its current terminus. It was also assumed that the intersection of Vollmer/Briargate/Stapleton would be signal controlled by 2040. This intersection is projected to operate at an overall satisfactory level of service (LOS D or better) as a signalized intersection.

## Modern Roundabout Option

The levels of service assuming modern roundabout traffic control at the Poco/Vollmer intersection would be A overall and for all intersection approaches during the peak hours based on the projected 2040 total traffic volumes. The roundabout level of service reports are attached.

## CONCLUSIONS AND RECOMMENDATIONS

## Trip Generation

1. The site is projected to generate about 2,001 new external vehicle-trips on the average weekday, with about half entering and half exiting the site during a 24 -hour period. During the morning peak hour about 39 vehicles would enter and 118 vehicles would exit the site. During the afternoon peak hour about 131 vehicles would enter and 78 vehicles would exit the site.

## Projected Levels of Service

2. The intersections of Vollmer/Poco and Vollmer/Arroya and the site access point to Vollmer Road just south of Arroya are projected to operate at a satisfactory level of service (LOS D or better) as stop-sign-controlled intersections based on the projected short-term and 2040 total traffic volumes.
3. All movements at the intersection of Vollmer/Burgess are projected to operate at LOS D or better based on the projected 2020 total traffic volumes. By 2040, the eastbound and westbound approaches at this intersection are projected to operate at LOS F during the afternoon peak hour based on both background and total traffic volumes assuming the current two-way stop-sign control (TWSC). If this intersection were converted to all-way, stop-sign control (AWSC) all movements are projected to operate at LOS C or better.
4. The intersection of Vollmer/Stapleton is projected to operate at a satisfactory level of service (LOS D or better) as a stop-sign-controlled intersection based on the short-term total traffic. This intersection is projected to operate at an overall satisfactory level of service (LOS D or better) as a signalized intersection in 2040.

## Improvements

5. A summary of the needed improvements is shown in Table 3.

## Recommended Auxiliary Turn Lane Improvements at the Site Access Points

6. Based on the criteria contained in the El Paso County Engineering Criteria Manual, the classification of Vollmer Road as a Minor Arterial, and the projected short-term site-generated traffic volumes, a northbound right-turn deceleration lane will be required on Vollmer Road approaching the Poco Road intersection. Based on a $45-\mathrm{mph}$ posted speed limit ( $50-\mathrm{mph}$ design speed), this deceleration lane should be 235 feet long plus a 200 -foot taper. Depending on the timing of Sterling Ranch to the south, the anticipated Vollmer improvement adjacent to Sterling Ranch, and associated transitions to the rural road cross section, the right turn could potentially be incorporated into that transition section.
7. Based on the criteria contained in the El Paso County Engineering Criteria Manual, the classification of Vollmer Road as a Minor Arterial, and the projected 2040 total traffic volumes, a northbound right-turn deceleration lane will not be required on Vollmer Road approaching the Arroya Lane intersection.
8. Based on the criteria contained in the El Paso County Engineering Criteria Manual, the classification of Vollmer Road as a Minor Arterial, and the projected 2040 total traffic volumes, southbound left-turn lanes will not be required on Vollmer Road approaching both the Arroya Lane intersection and the Poco Road intersection.
9. Based on the criteria contained in the El Paso County Engineering Criteria Manual, the classification of Arroya Lane as a Minor Collector, and the projected 2040 total traffic volumes no auxiliary turn lanes would be required on Arroya Lane approaching the three site access points.
10. Actual timing of installation of these turn lanes can be determined with the final plats.
11. Modern Roundabout Option: Modern roundabout intersection control could be considered as an alternative to two-way, Stop-sign control (TWSC) at Poco/Vollmer. The levels of service assuming modern roundabout traffic control at the Poco/Vollmer intersection would be A overall and for all intersection approaches. This would represent significantly lower delay on the side-street approaches during peak periods, but would introduce some minimal delay for north/south through traffic on Vollmer.

Roundabouts would require significant circular right-of-way around the center of the intersection. Currently, additional right-of-way to accommodate a roundabout(s) is not available on the west side of Vollmer. The consideration is that although the TIS shows better side-street level of service with the roundabout, the projected approach traffic volumes are not close to being equal on all the intersection approaches. The northbound and southbound through volumes are significantly higher than the eastbound and westbound volumes. The balance of approach volumes is an element to consider when evaluating a roundabout as a potential traffic control solution.

## Off-Site Auxiliary Turn Lane Evaluation

12. Based on the criteria contained in the El Paso County Engineering Criteria Manual, the classification of Vollmer Road as a Minor Arterial, and the existing plus buildout site-generated, 2020 background, and 2020 total traffic volumes, northbound right-turn volume will exceed the turning volume thresholds requiring a northbound right-turn lane on Vollmer Road at the Burgess Road intersection. Based on the revised PUD plan with lower trip generation, the afternoon peakhour traffic impact from this project on the northbound approach to this intersection is projected to be below 10 percent. The site volume on the roadway link (both directions of travel) south of the intersection is more than 10 percent; however, the turn lane thresholds are shown to be exceeded on the northbound approach during the afternoon peak hour when the impact of this project is below 10 percent on this approach. This project will be participating in the Fee Program and the MTCP Project ID is U-12.
13. Based on the criteria contained in the El Paso County Engineering Criteria Manual, the classification of Vollmer Road as a Minor Arterial, and the existing traffic volumes, the minimum turning volume threshold for a southbound left-turn lane is currently exceeded on Vollmer Road approaching Burgess Road. This project will not add any left turning volume to this turning movement.
14. Based on the criteria contained in the El Paso County Engineering Criteria Manual, the classification of Vollmer Road as a Minor Arterial, and the 2040 background traffic volumes, the northbound leftturn volume would exceed the turning volume thresholds requiring a northbound left-turn lane on Vollmer Road at the Burgess Road intersection. Based on the revised PUD plan with lower trip generation, the afternoon peak-hour traffic impact from this project on the northbound approach to this intersection is projected to be below 10 percent. The site volume on the roadway link (both directions of travel) south of the intersection is more than 10 percent, however the turn lane thresholds are shown to be exceeded on the northbound approach during the afternoon peak hour when the impact of this project is below 10 percent on this approach. This project will be participating in the Fee Program and the MTCP Project ID is U-12.

## EI Paso County Roadway Improvement Fee Program

15. This project will be required to participate in the El Paso County Road Impact Fee Program.

## Street Classification

16. Figure 12 shows the recommended street classifications for Vollmer Road adjacent to the site and the internal streets based on the projected 2040 traffic volumes shown in Figure 10.

## Deviations

17. County deviation forms for the proposed intersection spacing along Arroya Lane to Vollmer have been approved.
18. A county deviation request form has been submitted for the interim access to Vollmer Road for lots R-11 and R-12.

Please contact me if you have any questions or need further assistance.
Sincerely,
LSC TRANSPORTATION CONSULTANTS, INC.


JCH:KDF:bjwb
Enclosures: Tables 2 and 3
Figures 1-12
Traffic Count Reports
Level of Service Reports

| Table 2 <br> Trip Generation Estimate Retreat at Timber Ridge |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Phase | Land Use Code | Land <br> Use Description | Trip Generation Units | Trip Generation Rates ${ }^{(1)}$ |  |  |  |  | Total Trips Generated |  |  |  |  |
|  |  |  |  | Average Weekday Traffic | Morning Peak Hour |  | Afternoon <br> Peak Hour |  | Average Weekday Traffic | Morning Peak Hour |  | Afternoon Peak Hour |  |
|  |  |  |  |  | In | Out | In | Out |  | In | Out | In | Out |
| A | 210 | Single-Family Detached Housing | $12 \mathrm{DU}^{(2)}$ | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 113 | 2 | 7 | 7 | 4 |
| B | 210 | Single-Family Detached Housing | 29 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 274 | 5 | 16 | 18 | 11 |
| C | 210 | Single-Family Detached Housing | 19 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 179 | 4 | 11 | 12 | 7 |
| D | 210 | Single-Family Detached Housing | 145 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 1,369 | 27 | 80 | 90 | 53 |
|  |  | Phase A-D | 205 DU |  |  |  |  |  | 1,935 | 38 | 114 | 127 | 75 |
| E | 210 | Single-Family Detached Housing | 7 DU | 9.44 | 0.19 | 0.56 | 0.62 | 0.37 | 66 | 1 | 4 | 4 | 3 |
|  |  | Buildout | 212 DU |  |  |  |  |  | 2,001 | 39 | 118 | 131 | 78 |
| 20182 |  |  |  |  |  |  |  |  |  |  |  |  |  |
| (1) Source: "Trip Generation, 10th Edition, 2017" by the Institute of Transportation Engineers (ITE) <br> (2) $\mathrm{DU}=$ dwelling unit |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Source: LSC Transportation Consultants, Inc. |  |  |  |  |  |  |  |  |  |  |  |  |  |

> Note (4-12-18): The Preliminary Plan Report presents and updated version to match the Preliminary Plan.

| Note (4-12-18): The Preliminary Plan Report presents and updated version to match the Preliminary Plan. |  |  |
| :---: | :---: | :---: |
| Improvement | Timing | Responsibility ${ }^{(1)}$ |
| Construct a northbound right-turn deceleration lane on Vollmer Road approaching Poco Road. | Design and installation with the applicable final plat(s) for The Retreat at Timber Ridge. | The Retreat at Timber Ridge |
| Potential improvement: Southbound left-turn lane at Arroyo | Evaluation with final plats. Although the anticipated traffic counts do not warrant it, the County Engineer may require a southbound left-turn lane at Arroyo based on unanticipated traffic patterns [from Staff Comments]. | The Retreat at Timber Ridge and/or possible-but-not-currently-anticipated-future development with access via Arroya |
| Possible future modern roundabout intersection control at Poco/Vollmer as an alternative to the two-way, Stopsign control (TWSC) shown in this TIS | Consideration of roundabout traffic control instead of TWSC could be addressed with the applicable final plat(s) for The Retreat at Timber Ridge and/or Sterling Ranch. Roundabouts would require signifcant circular right-ofway around the center of the intersection. Currently, additional right-of-way to accomodate a roundabout(s) is not available on the west side of Vollmer. Also, the southeast corner of the intersection is not part of this project and is not owned by this applicant. It is owned by Sterling Ranch. The consideration is that although the TIS shows better side-street level of service with the roundabout, the projected approach traffic volumes are not close to being equal on all the intersection approaches. The northbound and southbound through volumes are significantly higher than the eastbound and westbound volumes. The balance of approach volumes is an element to consider when evaluating a roundabout as a potential traffic control solution. | The Retreat at Timber Ridge and/or Sterling Ranch |
| Upgrade Vollmer Road between future Stapleton Drive and Poco Road to an Urban Minor Arterial cross section (five lanes) | Future <br> MTCP Project ID U-12 <br> (Note: MTCP indicates two-lane Rural Minor Arterial.) | (Sterling Ranch Metro District) MTCP Master-Planned MTCP Project ID U-12 |
| Upgrade Vollmer Road generally between the south boundary of Sterling Ranch and future Stapleton Drive to an Urban Minor Arterial cross section (five lanes) | Designed MTCP Project ID C-13 | Sterling Ranch Metro District |
| Upgrade Vollmer Road generally between Cowpoke Road and the south boundary of Sterling Ranch to an Urban Minor Arterial cross section (five lanes) | Designed MTCP Project ID C-13 | Woodmen Heights Metro District |
| Construct section of Stapleton Road half section between Vollmer Road and the first Sterling Ranch access point | With development of Phase 1 of Sterling Ranch - Designed MTCP Project ID N-5 | Sterling Ranch Metro District |
| Construct a northbound right-turn deceleration lane on Vollmer Road approaching Stapleton Road | With development of Phase 1 of Sterling Ranch - Designed MTCP Project ID C-13 | Sterling Ranch Metro District |
| Construct Briargate Parkway (four-lane Principal Arterial) between Black Forest Road and Vollmer Road. | Future - TBD TBD with PPRTA ${ }^{(2)}$ Corridor Study | TBD with PPRTA ${ }^{(2)}$ Corridor Study MTCP Project N-5 |
| Construct Stapleton Drive between Vollmer Road and Towner | Future <br> TBD with PPRTA ${ }^{(2)}$ Corridor Study | TBD with PPRTA ${ }^{(2)}$ Corridor Study MTCP Project N-5 |
| Southbound left-turn lanes on Vollmer Road approaching Burgess Road | Existing Deficiency | Existing Deficiency - Others <br> (This development will not add volume to this turning movement.) |
| Northbound left-turn lane at Burgess/Vollmer | Projections indicate after 2020 but prior to 2040 the turning volume threshold warranting the turn lane ( 25 northbound left turns per hour) would be exceeded. | Based on the revised PUD plan, the afternoon peak-hour traffic impact from this project on the northbound approach to this intersection is projected to be below 10 percent. The site volume on the roadway link (both directions of travel) south of the intersection is more than 10 percent, however the turn lane thresholds are shown to be exceeded on the northbound approach during the afternoon peak hour when the impact of this project is below 10 percent on this approach. This project will be participating in the Fee Progam and the MTCP Project ID is U-12. |
| Northbound right-turn lane at Burgess/Vollmer | Projections indicate by 2020 the turning volume threshold warranting the turn lane ( 50 northbound right turns per hour) would be exceeded. | Based on the revised PUD plan, the afternoon peak-hour traffic impact from this project on the northbound approach to this intersection is projected to be below 10 percent. The site volume on the roadway link (both directions of travel) south of the intersection is more than 10 percent, however the turn lane thresholds are shown to be exceeded on the northbound approach during the afternoon peak hour when the impact of this project is below 10 percent on this approach. This project will be participating in the Fee Progam and the MTCP Project ID is U-12. |
| Future traffic signal at Stapleton/Vollmer | Once warrants are met; analysis to be included with final plat traffic reports; projections indicate by 2040 the intersection would be signalilzed. | Escrow a fair-share amount toward the cost the signal (to be determined with final plats). Once the signal is constructed, a portion of the escrow amount used to fund the installation of the signal may have become creditable under the Fee Program (if this signal is added to the fee program list of signals eligible for credit (County signals not currently programmed in Fee Program). |
| Notes: <br> (1) Preliminary concept of responsibility; the actual construction responsibility would be determined through subdivision applications and cost recovery if applicable agreements. <br> (2) PPRTA = Pikes Peak Rural Transportation Authority. |  |  |
| Source: LSC Transportation Consultants, Inc. |  |  |




Figure 2
Site Plan

> Note (4-12-18): The Preliminary Plan Report shows updated phasing to match the Preliminary Plan.



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Figure 7






LSC Transportation Consultants, Inc.
545 E. Pikes Peak Ave., \#210
LSC Transportation Consultants, Inc. Colorado Springs, CO 80903 File Name : Vollmer Rd - Poco Rd AM

(719) 633-2868 | Site Code $: 00174030$ |
| :--- | :--- |
| Start Date :02/09/2017 |

Start Date : 02/09/2017
Page No : 1
Groups Printed- Unshifted

|  | Vollmer Rd From North |  |  |  | From East |  |  |  | Vollmer Rd From South |  |  |  | Poco Rd From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \hline \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 06:30 AM | 0 | 26 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 9 | 1 | 0 | 1 | 0 | 1 | 0 | 38 |
| 06:45 AM | 0 | 23 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 4 | 0 | 0 | 0 | 0 | 0 | 30 |
| Total | 0 | 49 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 5 | 0 | 1 | 0 | 1 | 0 | 68 |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | ---: |
| 07:00 AM | 0 | 21 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | 3 | 0 | 1 | 0 | 0 | 0 | 36 |
| $07: 15 \mathrm{AM}$ | 1 | 25 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 12 | 0 | 0 | 3 | 0 | 0 | 0 | 41 |
| 07:30 AM | 0 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 0 | 0 | 1 | 0 | 0 | 0 | 43 |
| $07: 45 \mathrm{AM}$ | 1 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 15 | 0 | 0 | 2 | 0 | 0 | 0 | 40 |
| Total | 2 | 90 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 58 | 3 | 0 | 7 | 0 | 0 | 0 | 160 |


| 08:00 AM | 0 | 20 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 8 | 0 | 0 | 2 | 0 | 0 | 0 | 30 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 08:15 AM | 1 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 2 | 0 | 0 | 0 | 0 | 0 | 51 |
| Grand Total | 3 | 188 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 97 | 10 | 0 | 10 | 0 | 1 | 0 | 309 |
| Apprch \% | 1.6 | 98.4 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 90.7 | 9.3 | 0.0 | 90.9 | 0.0 | 9.1 | 0.0 |  |
| Total \% | 1.0 | 60.8 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 31.4 | 3.2 | 0.0 | 3.2 | 0.0 | 0.3 | 0.0 |  |

LSC Transportation Consultants, Inc.
545 E. Pikes Peak Ave., \#210
Colorado Springs, CO 80903 File Name : Vollmer Rd - Poco Rd AM
(719) 633-2868 $\quad$ Site Code :00174030

Page No : 2

|  | Vollmer Rd From North |  |  |  |  | From East |  |  |  |  | Vollmer Rd From South |  |  |  |  | Poco Rd From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{r} \hline \text { Thr } \\ \mathrm{u} \end{array}$ | $\begin{array}{r} \text { Lef } \\ \mathrm{t} \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{r} \hline \text { Thr } \\ \mathrm{u} \end{array}$ | $\begin{array}{r} \text { Lef } \\ \mathrm{t} \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \hline \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{\|r\|} \hline \text { Thr } \\ u \end{array}$ | $\begin{array}{r\|} \hline \text { Lef } \\ \mathrm{t} \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{gathered} \hline \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{r} \hline \text { Thr } \\ \mathrm{u} \end{array}$ | Lef | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \end{aligned}$ | App. <br> Total | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |




LSC Transportation Consultants, Inc.
545 E. Pikes Peak Ave., \#210
LSC Transportation Consultants, Inc. Colorado Springs, CO 80903 File Name : Vollmer Rd - Poco Rd PM
(719) 633-2868 $\begin{array}{ll}\text { Site Code }: 00174030 \\ \text { Start Date }: 02 / 08 / 2017\end{array}$

Page No : 1
Groups Printed- Unshifted

|  | Vollmer Rd From North |  |  |  | From East |  |  |  | Vollmer Rd From South |  |  |  | Poco Rd From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 04:00 PM | 0 | 20 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 28 | 1 | 0 | 0 | 0 | 1 | 0 | 50 |
| 04:15 PM | 1 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | 0 | 0 | 0 | 0 | 0 | 0 | 35 |
| 04:30 PM | 0 | 19 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 36 | 1 | 0 | 0 | 0 | 0 | 0 | 56 |
| 04:45 PM | 0 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 36 | 1 | 0 | 2 | 0 | 0 | 0 | 52 |
| Total | 1 | 66 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 120 | 3 | 0 | 2 | 0 | 1 | 0 | 193 |


| 05:00 PM | 1 | 22 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 35 | 2 | 0 | 0 | 0 | 0 | 0 | 60 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 05:15 PM | 0 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 22 | 1 | 0 | 0 | 0 | 0 | 0 | 33 |
| 05:30 PM | 0 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 38 | 1 | 0 | 1 | 0 | 0 | 0 | 55 |
| 05:45 PM | 0 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 26 | 2 | 0 | 1 | 0 | 0 | 0 | 41 |
| Total | 1 | 59 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 121 | 6 | 0 | 2 | 0 | 0 | 0 | 189 |


| Grand Total | 2 | 125 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 241 | 9 | 0 | 4 | 0 | 1 | 0 | 382 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Apprch \% | 1.6 | 98.4 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 96.4 | 3.6 | 0.0 | 80.0 | 0.0 | 20.0 | 0.0 |  |
| Total \% | 0.5 | 32.7 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 63.1 | 2.4 | 0.0 | 1.0 | 0.0 | 0.3 | 0.0 |  |

LSC Transportation Consultants, Inc.
545 E. Pikes Peak Ave., \#210
Colorado Springs, CO 80903 File Name : Vollmer Rd - Poco Rd PM
(719) 633-2868 $\begin{array}{ll}\text { Site Code }: 00174030 \\ \text { Start Date }: 02 / 08 / 2017\end{array}$

Page No : 2


## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Vollmer Rd - Burgess Rd AM
Site Code : 00174030
Start Date : 06/13/2017
Page No : 1
Groups Printed- Unshifted

|  | Vollmer Rd From North |  |  |  | Burgess Rd From East |  |  |  | Vollmer Rd From South |  |  |  | Burgess Rd From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Int. Total |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 06:30 AM | 2 | 15 | 6 | 0 | 13 | 39 | 5 | 0 | 1 | 1 | 0 | 0 | 0 | 7 | 1 | 0 | 90 |
| 06:45 AM | 1 | 5 | 2 | 0 | 9 | 39 | 7 | 0 | 4 | 5 | 1 | 0 | 0 | 5 | 0 | 0 | 78 |
| Total | 3 | 20 | 8 | 0 | 22 | 78 | 12 | 0 | 5 | 6 | 1 | 0 | 0 | 12 | 1 | 0 | 168 |


| 07:00 AM | 7 | 18 | 2 | 0 | 17 | 51 | 12 | 0 | 6 | 13 | 2 | 0 | 0 | 7 | 0 | 0 | 135 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $07: 15 \mathrm{AM}$ | 6 | 7 | 4 | 0 | 21 | 52 | 8 | 0 | 1 | 6 | 2 | 0 | 0 | 2 | 0 | 0 | 109 |
| 07:30 AM | 2 | 12 | 4 | 0 | 17 | 60 | 11 | 0 | 6 | 12 | 1 | 0 | 1 | 5 | 2 | 0 | 133 |
| $07: 45 \mathrm{AM}$ | 7 | 12 | 6 | 0 | 13 | 44 | 5 | 0 | 0 | 12 | 3 | 0 | 1 | 9 | 2 | 0 | 114 |
| Total | 22 | 49 | 16 | 0 | 68 | 207 | 36 | 0 | 13 | 43 | 8 | 0 | 2 | 23 | 4 | 0 | 491 |


| 87 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| 08:00 AM | 2 | 14 | 7 | 0 | 9 | 30 | 7 | 0 | 5 | 5 | 1 | 0 | 0 | 6 | 1 | 0 | 87 |
| 08:15 AM | 3 | 6 | 10 | 0 | 9 | 38 | 9 | 0 | 7 | 6 | 2 | 0 | 1 | 9 | 1 | 0 | 101 |
| Grand Total | 30 | 89 | 41 | 0 | 108 | 353 | 64 | 0 | 30 | 60 | 12 | 0 | 3 | 50 | 7 | 0 | 847 |
| Apprch \% | 18.8 | 55.6 | 25.6 | 0.0 | 20.6 | 67.2 | 12.2 | 0.0 | 29.4 | 58.8 | 11.8 | 0.0 | 5.0 | 83.3 | 11.7 | 0.0 |  |
| Total \% | 3.5 | 10.5 | 4.8 | 0.0 | 12.8 | 41.7 | 7.6 | 0.0 | 3.5 | 7.1 | 1.4 | 0.0 | 0.4 | 5.9 | 0.8 | 0.0 |  |

File Name : Vollmer Rd - Burgess Rd AM
Site Code : 00174030
Start Date : 06/13/2017
Page No : 2

|  | Vollmer Rd From North |  |  |  |  | Burgess Rd From East |  |  |  |  | Vollmer Rd From South |  |  |  |  | Burgess Rd From West |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | $\begin{array}{r} \text { Rig } \\ \mathrm{ht} \\ \hline \end{array}$ | $\begin{array}{r} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | Lef | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \\ & \hline \end{aligned}$ | App. Total | $\begin{array}{r} \text { Rig } \\ \text { ht } \\ \hline \end{array}$ | $\begin{array}{r} \hline \mathrm{Thr} \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r} \text { Lef } \\ \mathrm{t} \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \\ & \hline \end{aligned}$ | App. Total | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{\|r\|} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | $\begin{array}{r} \hline \text { Lef } \\ \mathrm{t} \\ \hline \end{array}$ | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \\ & \hline \end{aligned}$ | App. Total | $\begin{gathered} \text { Rig } \\ \text { ht } \end{gathered}$ | $\begin{array}{\|r\|} \hline \text { Thr } \\ \mathrm{u} \\ \hline \end{array}$ | Lef t | $\begin{aligned} & \mathrm{Pe} \\ & \mathrm{ds} \\ & \hline \end{aligned}$ | App. Total | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Peak Hour From 06:30 AM to 08:15 AM - Peak 1 of 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Intersecti on | 07:00 AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Volume | 22 | 49 | 16 | 0 | 87 | 68 | 20 | 36 | 0 | 311 | 13 | 43 | 8 | 0 | 64 | 2 | 23 | 4 | 0 | 29 | 491 |
| Percent | 25. | $\begin{array}{r} 56 . \\ 3 \end{array}$ | $\begin{array}{r} 18 . \\ 4 \end{array}$ | 0.0 |  | 21. | $\begin{array}{r} 66 \\ 6 \end{array}$ | $\begin{array}{r} 11 . \\ 6 \end{array}$ | 0.0 |  | 20. | 67. | 12. | 0.0 |  | 6.9 |  |  | 0.0 |  |  |
| $\begin{array}{r} \text { 07:00 } \\ \text { Volume } \end{array}$ | 7 | 18 | 2 | 0 | 27 | 17 | 51 | 12 | 0 | 80 | 6 | 13 | 2 | 0 | 21 | 0 | 7 | 0 | 0 | 7 | 135 |
| Peak |  |  |  |  |  | 07:30 AM |  |  |  |  | 07:00 AM |  |  |  |  | 07:45 AM |  |  |  |  | 0.909 |
| Factor |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| High Int. | 07:00 | AM |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Volume | 7 | 18 | 2 | 0 | 27 | 17 | 60 | 11 | 0 | 88 |  |  |  |  |  | 6 | 13 | 2 | 0 | 21 | 1 | 9 | 2 | 0 | 12 |  |
| Peak |  |  |  |  | 0.80 |  |  |  |  | 0.88 |  |  |  |  |  |  |  |  |  | 0.76 |  |  |  |  | 0.60 |  |
| Factor |  |  |  |  | 6 |  |  |  |  | 4 |  |  |  |  | 2 |  |  |  |  | 4 |  |



## Counts by LSC

LSC Transportation Consultants, Inc.
File Name : Vollmer Rd - Burgess Rd PM
Site Code : 00174030
Start Date : 06/08/2017
Page No : 1
Groups Printed- Unshifted

|  | Vollmer Rd From North |  |  |  | Burgess Rd From East |  |  |  | Vollmer Rd From South |  |  |  | Burgess Rd From West |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | Right | Thru | Left | Peds | $\begin{array}{r} \text { Int. } \\ \text { Total } \end{array}$ |
| Factor | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 |  |
| 04:00 PM | 2 | 11 | 13 | 0 | 10 | 15 | 11 | 0 | 13 | 9 | 4 | 0 | 2 | 27 | 2 | 0 | 119 |
| 04:15 PM | 0 | 11 | 19 | 0 | 4 | 22 | 5 | 0 | 17 | 8 | 1 | 0 | 3 | 36 | 1 | 0 | 127 |
| 04:30 PM | 5 | 11 | 11 | 0 | 11 | 19 | 7 | 0 | 9 | 17 | 0 | 0 | 0 | 29 | 3 | 0 | 122 |
| 04:45 PM | 4 | 9 | 28 | 0 | 5 | 18 | 7 | 0 | 14 | 20 | 2 | 0 | 0 | 43 | 3 | 0 | 153 |
| Total | 11 | 42 | 71 | 0 | 30 | 74 | 30 | 0 | 53 | 54 | 7 | 0 | 5 | 135 | 9 | 0 | 521 |


| 05:00 PM | 1 | 8 | 15 | 0 | 7 | 10 | 7 | 0 | 9 | 17 | 0 | 0 | 2 | 40 | 2 | 0 | 118 |
| ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| $05: 15 ~ P M ~$ | 5 | 8 | 8 | 0 | 7 | 17 | 2 | 0 | 13 | 21 | 2 | 0 | 2 | 56 | 1 | 0 | 142 |
| 05:30 PM | 2 | 14 | 18 | 0 | 7 | 17 | 2 | 0 | 10 | 16 | 2 | 0 | 2 | 49 | 4 | 0 | 143 |
| 05:45 PM | 3 | 6 | 11 | 0 | 9 | 14 | 8 | 0 | 17 | 15 | 0 | 0 | 1 | 42 | 2 | 0 | 128 |
| Total | 11 | 36 | 52 | 0 | 30 | 58 | 19 | 0 | 49 | 69 | 4 | 0 | 7 | 187 | 9 | 0 | 531 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Grand Total | 22 | 78 | 123 | 0 | 60 | 132 | 49 | 0 | 102 | 123 | 11 | 0 | 12 | 322 | 18 | 0 | 1052 |
| Apprch \% | 9.9 | 35.0 | 55.2 | 0.0 | 24.9 | 54.8 | 20.3 | 0.0 | 43.2 | 52.1 | 4.7 | 0.0 | 3.4 | 91.5 | 5.1 | 0.0 |  |
| Total \% | 2.1 | 7.4 | 11.7 | 0.0 | 5.7 | 12.5 | 4.7 | 0.0 | 9.7 | 11.7 | 1.0 | 0.0 | 1.1 | 30.6 | 1.7 | 0.0 |  |

File Name : Vollmer Rd - Burgess Rd PM
Site Code : 00174030
Start Date : 06/08/2017
Page No : 2




| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 9.2 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | \& |  |  | \$ |  |  | $\ddagger$ |  |
| Traffic Vol, veh/h | 4 | 23 | 2 | 36 | 207 | 68 | 8 | 43 | 13 | 16 | 49 | 22 |
| Future Vol, veh/h | 4 | 23 | 2 | 36 | 207 | 68 | 8 | 43 | 13 | 16 | 49 | 22 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 97 | 97 | 97 | 76 | 76 | 76 | 81 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 4 | 23 | 2 | 37 | 213 | 70 | 11 | 57 | 17 | 20 | 60 | 27 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 0.2 |  |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | * |  |  | $\uparrow$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 0 | 2 | 4 | 127 | 68 | 2 |
| Future Vol, veh/h | 0 | 2 | 4 | 127 | 68 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 89 | 89 | 76 | 76 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 2 | 4 | 143 | 89 | 3 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \& |  |  | * |  |  | \$ |  |
| Traffic Vol, veh/h | 10 | 188 | 6 | 18 | 62 | 26 | 6 | 74 | 46 | 69 | 39 | 12 |
| Future Vol, veh/h | 10 | 188 | 6 | 18 | 62 | 26 | 6 | 74 | 46 | 69 | 39 | 12 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 88 | 88 | 88 | 88 | 88 | 88 | 73 | 73 | 73 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 10 | 188 | 6 | 20 | 70 | 30 | 7 | 84 | 52 | 95 | 53 | 16 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1.7 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{1}$ | 「 | 4 | 「 | ${ }^{7}$ | 4 |
| Traffic Vol, veh/h | 59 | 7 | 134 | 23 | 13 | 222 |
| Future Vol, veh/h | 59 | 7 | 134 | 23 | 13 | 222 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | S | None | - | None | - | None |
| Storage Length | 0 | 0 | - | 235 | 260 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 76 | 92 | 92 | 79 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 64 | 8 | 176 | 25 | 14 | 281 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 1.3 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | \$ |  |  | \& |  |  | \& |  |  | \$ |  |
| Traffic Vol, veh/h | 0 | 0 | 5 | 25 | 0 | 4 | 2 | 91 | 8 | 1 | 118 | 2 |
| Future Vol, veh/h | 0 | 0 | 5 | 25 | 0 | 4 | 2 | 91 | 8 | 1 | 118 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 92 | 92 | 92 | 76 | 76 | 92 | 92 | 79 | 79 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 0 | 5 | 27 | 0 | 4 | 3 | 120 | 9 | 1 | 149 | 3 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 0.2 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | * |  |  | $\uparrow$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 0 | 5 | 2 | 93 | 117 | 0 |
| Future Vol, veh/h | 0 | 5 | 2 | 93 | 117 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | S | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 76 | 79 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 5 | 2 | 122 | 148 | 0 |



| Intersection |  |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | :---: |
| Int Delay, s/veh | 0.1 |  |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |  |
| Lane Configurations | 1 |  | 1 |  |  |  |  |
| Traffic Vol, veh/h | 2 | 0 | 92 | 1 | 0 | 115 |  |
| Future Vol, veh/h | 2 | 0 | 92 | 1 | 0 | 115 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control | Stop | Stop | Free | Free | Free | Free |  |
| RT Channelized | - | None | - | None | - | None |  |
| Storage Length | 0 | - | - | - | - | - |  |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |  |
| Grade, \% | 0 | - | 0 | - | - | 0 |  |
| Peak Hour Factor | 92 | 92 | 76 | 92 | 92 | 79 |  |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Mvmt Flow | 2 | 0 | 121 | 1 | 0 | 146 |  |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 9.8 |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | ¢ |  |  | \& |  | ${ }^{*}$ | 4 | 「 | ${ }^{*}$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 4 | 25 | 7 | 46 | 226 | 74 | 15 | 58 | 25 | 17 | 61 | 24 |
| Future Vol, veh/h | 4 | 25 | 7 | 46 | 226 | 74 | 15 | 58 | 25 | 17 | 61 | 24 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | 0 | - | 0 | 0 | - | - |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 97 | 97 | 97 | 76 | 76 | 76 | 81 | 81 | 81 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 4 | 25 | 7 | 47 | 233 | 76 | 20 | 76 | 33 | 21 | 75 | 30 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 3.1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | ${ }^{7}$ | 「 | 4 | 「 | ${ }^{1}$ | 4 |
| Traffic Vol, veh/h | 113 | 43 | 319 | 69 | 36 | 163 |
| Future Vol, veh/h | 113 | 43 | 319 | 69 | 36 | 163 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | St | None | - | None | - | None |
| Storage Length | 0 | 0 | - | 235 | 260 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 123 | 47 | 347 | 75 | 39 | 177 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 0.7 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ¢ |  |  | ¢ |  |  | ¢ |  |  | ${ }_{\text {¢ }}$ |  |  |
| Traffic Vol, veh/h | 0 | 0 | 2 | 16 | 0 | 3 | 4 | 187 | 28 | 4 | 128 | 2 |
| Future Vol, veh/h | 0 | 0 | 2 | 16 | 0 | 3 | 4 | 187 | 28 | 4 | 128 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - |  |
| Veh in Median Storage, \# | - | 0 |  |  | 0 | - | - | 0 | - | - | 0 |  |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 |  |  | 0 |  |
| Peak Hour Factor | 100 | 100 | 100 | 92 | 92 | 92 | 89 | 89 | 92 | 92 | 76 | 76 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mumt Flow | 0 | 0 | 2 | 17 | 0 | 3 | 4 | 210 | 30 | 4 | 168 | 3 |


| Major/Minor | Minor2 |  | Minor1 |  |  |  |  | Major1 |  |  | Major2 |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 413 | 426 | 170 |  | 412 | 412 | 225 |  | 171 | 0 | 0 | 240 | 0 | 0 |
| Stage 1 | 178 | 178 | - |  | 233 | 233 | - |  | - | - | - | - | - |  |
| Stage 2 | 235 | 248 | - |  | 179 | 179 | - |  | - | - |  |  | - |  |
| Critical Hdwy | 7.12 | 6.52 | 6.22 |  | 7.12 | 6.52 | 6.22 |  | 4.12 | - | - | 4.12 | - |  |
| Critical Hdwy Stg 1 | 6.12 | 5.52 | - |  | 6.12 | 5.52 | - |  | - | - | - | - | - |  |
| Critical Hdwy Stg 2 | 6.12 | 5.52 | - |  | 6.12 | 5.52 | - |  | - | - | - | - | - |  |
| Follow-up Hdwy | 3.518 | 4.018 | 3.318 |  | 3.518 | 4.018 | 3.318 |  | 2.218 | - | - | 2.218 | - |  |
| Pot Cap-1 Maneuver | 549 | 520 | 874 |  | 550 | 530 | 814 |  | 1406 | - | - | 1327 | - |  |
| Stage 1 | 824 | 752 | - |  | 770 | 712 | - |  | - | - | - | - | - |  |
| Stage 2 | 768 | 701 | - |  | 823 | 751 | - |  | - | - | - | - | - |  |
| Platoon blocked, \% |  |  |  |  |  |  |  |  |  | - | - |  | - |  |
| Mov Cap-1 Maneuver | 544 | 517 | 874 |  | 546 | 527 | 814 |  | 1406 | - | - | 1327 | - |  |
| Mov Cap-2 Maneuver | 544 | 517 | - |  | 546 | 527 | - |  | - | - | - | - | - |  |
| Stage 1 | 822 | 750 | - |  | 768 | 710 | - |  | - | - | - | - | - |  |
| Stage 2 | 763 | 699 | - |  | 819 | 749 | - |  | - | - | - | - | - |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Approach | EB |  |  |  | WB |  |  |  | NB |  |  | SB |  |  |
| HCM Control Delay, s | 9.1 |  |  |  | 11.5 |  |  |  | 0.1 |  |  | 0.2 |  |  |
| HCM LOS | A |  |  |  | B |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt | NBL | NBT | NBR | EBLn1 | NBLn1 | SBL | SBT | SBR |  |  |  |  |  |  |
| Capacity (veh/h) | 1406 | - | - | 874 | 576 | 1327 | - |  |  |  |  |  |  |  |
| HCM Lane V/C Ratio | 0.003 | - | - | 0.002 | 0.036 | 0.003 | - |  |  |  |  |  |  |  |
| HCM Control Delay (s) | 7.6 | 0 | - | 9.1 | 11.5 | 7.7 | 0 |  |  |  |  |  |  |  |
| HCM Lane LOS | A | A | - | A | B | A | A | - |  |  |  |  |  |  |
| HCM 95th \%tile Q(veh) | 0 | - | - | 0 | 0.1 | 0 | - | - |  |  |  |  |  |  |


| Intersection |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 0.2 |  |  |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |  |
| Lane Configurations | \% |  |  | $\uparrow$ | F |  |  |
| Traffic Vol, veh/h | 0 |  | 5 | 184 | 131 | 0 |  |
| Future Vol, veh/h | 0 | 3 | 5 | 184 | 131 | 0 |  |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |  |
| Sign Control | Stop | Stop | Free | Free | Free | Free |  |
| RT Channelized | - | None | - | None | - | None |  |
| Storage Length | 0 | - | - | - | - | - |  |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |  |
| Grade, \% | 0 | - | - | 0 | 0 | - |  |
| Peak Hour Factor | 92 | 92 | 92 | 89 | 76 | 92 |  |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |  |
| Mvmt Flow | 0 | 3 | 5 | 207 | 172 | 0 |  |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | 1 |  | 18 |  |  |  |
| Traffic Vol, veh/h | 1 | 0 | 2 | 0 | 130 |  |
| Future Vol, veh/h | 1 | 0 | 182 | 2 | 0 | 130 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 89 | 92 | 92 | 76 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 1 | 0 | 204 | 2 | 0 | 171 |



| Intersection |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 11.9 |  |  |  |  |  |  |  |  |  |  |  |  |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations |  | 4 |  |  | \& |  | ${ }^{7}$ | 4 | 「 | ${ }^{7}$ | $\uparrow$ |  |
| Traffic Vol, veh/h | 11 | 205 | 17 | 44 | 68 | 28 | 18 | 101 | 70 | 75 | 67 | 13 |
| Future Vol, veh/h | 11 | 205 | 17 | 44 | 68 | 28 | 18 | 101 | 70 | 75 | 67 | 13 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Free | Free | Free | Free | Free | Free |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | 0 | - | 0 | 0 | - | - |
| Veh in Median Storage, \# | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, \% | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 88 | 88 | 88 | 88 | 88 | 88 | 73 | 73 | 73 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 11 | 205 | 17 | 50 | 77 | 32 | 20 | 115 | 80 | 103 | 92 | 18 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{T}$ | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{1}$ | 4 |
| Traffic Vol, veh/h | 59 | 7 | 167 | 23 | 13 | 318 |
| Future Vol, veh/h | 59 | 7 | 167 | 23 | 13 | 318 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | 0 | - | 235 | 260 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 76 | 92 | 92 | 79 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 64 | 8 | 220 | 25 | 14 | 403 |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.8 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | 1 |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 15 | 6 | 103 | 5 | 2 | 118 |
| Future Vol, veh/h | 15 | 6 | 103 | 5 | 2 | 118 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 76 | 92 | 92 | 79 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 16 | 7 | 136 | 5 | 2 | 149 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 292 | 139 | 0 | 0 | 141 | 0 |
| Stage 1 | 139 | - | - | - | - | - |
| Stage 2 | 153 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 699 | 909 | - | - | 1442 | - |
| Stage 1 | 888 | - | - | - | - | - |
| Stage 2 | 875 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 698 | 909 | - | - | 1442 | - |
| Mov Cap-2 Maneuver | 698 | - | - | - | - | - |
| Stage 1 | 886 | - | - | - | - | - |
| Stage 2 | 875 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 10 |  | 0 |  | 0.1 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 748 | 1442 | - |
| HCM Lane V/C Ratio |  | - | - | 0.031 | 0.002 | - |
| HCM Control Delay (s) |  | - | - | 10 | 7.5 | 0 |
| HCM Lane LOS |  | - | - | B | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.1 | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | $\mathbf{4}$ | Mr |  |
| Traffic Vol, veh/h | 5 | 2 | 0 | 17 | 4 | 0 |
| Future Vol, veh/h | 5 | 2 | 0 | 17 | 4 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 5 | 2 | 0 | 18 | 4 | 0 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |







| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | Yr |  |  | -1 | F |  |
| Traffic Vol, veh/h | 0 | 1 | 0 | 108 | 133 | 0 |
| Future Vol, veh/h | 0 | 1 | 0 | 108 | 133 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 1 | 0 | 117 | 145 | 0 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{T}$ | $\mathbf{7}$ | $\mathbf{4}$ | $\mathbf{7}$ | $\mathbf{1}$ | 4 |
| Traffic Vol, veh/h | 113 | 43 | 427 | 69 | 36 | 227 |
| Future Vol, veh/h | 113 | 43 | 427 | 69 | 36 | 227 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | 0 | - | 235 | 260 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 123 | 47 | 464 | 75 | 39 | 247 |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | 1 |  | $\uparrow$ |  |  | $\uparrow$ |
| Traffic Vol, veh/h | 11 | 4 | 190 | 16 | 7 | 142 |
| Future Vol, veh/h | 11 | 4 | 190 | 16 | 7 | 142 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 89 | 92 | 92 | 76 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 12 | 4 | 213 | 17 | 8 | 187 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 425 | 222 | 0 | 0 | 230 | 0 |
| Stage 1 | 222 | - | - | - | - | - |
| Stage 2 | 203 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 586 | 818 | - | - | 1338 | - |
| Stage 1 | 815 | - | - | - | - | - |
| Stage 2 | 831 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 582 | 818 | - | - | 1338 | - |
| Mov Cap-2 Maneuver | 582 | - | - | - | - | - |
| Stage 1 | 809 | - | - | - | - | - |
| Stage 2 | 831 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 10.9 |  | 0 |  | 0.3 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 631 | 1338 | - |
| HCM Lane V/C Ratio |  | - | - | 0.026 | 0.006 | - |
| HCM Control Delay (s) |  | - | - | 10.9 | 7.7 | 0 |
| HCM Lane LOS |  | - | - | B | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.1 | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.9 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | - | ric |  |
| Traffic Vol, veh/h | 18 | 5 | 0 | 11 | 4 | 0 |
| Future Vol, veh/h | 18 | 5 | 0 | 11 | 4 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 20 | 5 | 0 | 12 | 4 | 0 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.8 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | $\uparrow$ |  |  | - | Tr |  |
| Traffic Vol, veh/h | 8 | 10 | 0 | 5 | 6 | 0 |
| Future Vol, veh/h | 8 | 10 | 0 | 5 | 6 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 9 | 11 | 0 | 5 | 7 | 0 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.9 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | -1 | 1 |  | Mr |  |
| Traffic Vol, veh/h | 6 | 2 | 1 | 0 | 0 | 4 |
| Future Vol, veh/h | 6 | 2 | 1 | 0 | 0 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 7 | 2 | 1 | 0 | 0 | 4 |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0 |  |  |  |  |  |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | r |  |  | - | 个 |  |
| Traffic Vol, veh/h | 0 | 1 | 1 | 206 | 152 | 0 |
| Future Vol, veh/h | 0 | 1 | 1 | 206 | 152 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, $\#$ | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 1 | 1 | 224 | 165 | 0 |



|  | 4 | $\rightarrow$ |  | 7 |  | 4 | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊ | 个个 | 「 | ${ }^{1 *}$ | 个个 | 「 | ＊ | 个个 | 「 | ${ }^{*}$ | 个个 | F |
| Traffic Volume（vph） | 48 | 823 | 60 | 342 | 1441 | 66 | 90 | 124 | 121 | 143 | 287 | 99 |
| Future Volume（vph） | 48 | 823 | 60 | 342 | 1441 | 66 | 90 | 124 | 121 | 143 | 287 | 99 |
| Turn Type | pm＋pt | NA | Perm | Prot | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  | 2 |  |  | 6 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 5 | 2 | 2 | 1 | 6 | 6 | 3 | 8 | 8 | 7 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split（s） | 10.0 | 53.0 | 53.0 | 22.0 | 65.0 | 65.0 | 15.0 | 32.0 | 32.0 | 13.0 | 30.0 | 30.0 |
| Total Split（\％） | 8．3\％ | 44．2\％ | 44．2\％ | 18．3\％ | 54．2\％ | 54．2\％ | 12．5\％ | 26．7\％ | 26．7\％ | 10．8\％ | 25．0\％ | 25．0\％ |
| Yellow Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | None | None | None | None | None | None | None |
| Act Efft Green（s） | 42.9 | 37.6 | 37.6 | 15.0 | 50.4 | 50.4 | 23.2 | 14.4 | 14.4 | 23.4 | 17.4 | 17.4 |
| Actuated g／C Ratio | 0.45 | 0.39 | 0.39 | 0.16 | 0.52 | 0.52 | 0.24 | 0.15 | 0.15 | 0.24 | 0.18 | 0.18 |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.30 | 0.63 | 0.09 | 0.67 | 0.82 | 0.08 | 0.31 | 0.25 | 0.35 | 0.46 | 0.47 | 0.25 |
| Control Delay | 15.0 | 25.9 | 0.2 | 48.0 | 24.5 | 1.0 | 31.9 | 39.7 | 6.4 | 35.5 | 42.5 | 3.3 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 15.0 | 25.9 | 0.2 | 48.0 | 24.5 | 1.0 | 31.9 | 39.7 | 6.4 | 35.5 | 42.5 | 3.3 |
| LOS | B | C | A | D | C | A | C | D | A | D | D | A |
| Approach Delay |  | 23.7 |  |  | 28.0 |  |  | 25.6 |  |  | 33.3 |  |
| Approach LOS |  | C |  |  | C |  |  | C |  |  | C |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length： 120
Actuated Cycle Length： 96.4
Natural Cycle： 65
Control Type：Actuated－Uncoordinated
Maximum v／c Ratio： 0.82
Intersection Signal Delay： 27.5
Intersection LOS：C
Intersection Capacity Utilization 73．6\％
ICU Level of Service D
Analysis Period（min） 15
Splits and Phases：8：Vollmer Rd \＆Stapleton Dr




| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 1 |  |  |  |  |  |
| Movement V | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | \% |  | 4 | 「 |  | $\uparrow$ |
| Traffic Vol, veh/h | 30 | 18 | 186 | 10 | 6 | 357 |
| Future Vol, veh/h | 30 | 18 | 186 | 10 | 6 | 357 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | 235 | - | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 32 | 19 | 196 | 11 | 6 | 376 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 584 | 196 | 0 | 0 | 207 | 0 |
| Stage 1 | 196 | - | - | - | - | - |
| Stage 2 | 388 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 474 | 845 | - | - | 1364 | - |
| Stage 1 | 837 | - | - | - | - | - |
| Stage 2 | 686 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 471 | 845 | - | - | 1364 | - |
| Mov Cap-2 Maneuver | 471 | - | - | - | - | - |
| Stage 1 | 832 | - | - | - | - | - |
| Stage 2 | 686 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 12 |  | 0 |  | 0.1 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 565 | 1364 | - |
| HCM Lane V/C Ratio |  | - | - | 0.089 | 0.005 | - |
| HCM Control Delay (s) |  | - | - | 12 | 7.7 | 0 |
| HCM Lane LOS |  | - | - | B | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.3 | 0 | - |




| Intersection |  |
| :--- | ---: | :--- |
| Intersection Delay, s/veh | 15 |
| Intersection LOS | B |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | \& |  |  | ¢ |  | ${ }^{*}$ | 4 | 「 | ${ }^{*}$ | 4 | 「 |
| Traffic Vol, veh/h | 20 | 29 | 17 | 52 | 254 | 98 | 23 | 106 | 29 | 50 | 163 | 62 |
| Future Vol, veh/h | 20 | 29 | 17 | 52 | 254 | 98 | 23 | 106 | 29 | 50 | 163 | 62 |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 21 | 31 | 18 | 55 | 267 | 103 | 24 | 112 | 31 | 53 | 172 | 65 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 1 | 1 | 1 | 1 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  | 3 |  |  | 3 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 3 |  |  | 3 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 3 |  |  | 3 |  |  | 1 |  |  | 1 |  |  |
| HCM Control Delay | 10.2 |  |  | 20.3 |  |  | 10.5 |  |  | 11 |  |  |
| HCM LOS | B |  |  | C |  |  | B |  |  | B |  |  |


| Lane | NBLn1 | NBLn2 | NBLn3 | EBLn1 | WBLn1 | SBLn1 | SBLn2 | SBLn3 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $100 \%$ | $0 \%$ | $0 \%$ | $30 \%$ | $13 \%$ | $100 \%$ | $0 \%$ | $0 \%$ |
| Vol Thru, \% | $0 \%$ | $100 \%$ | $0 \%$ | $44 \%$ | $63 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Right, \% | $0 \%$ | $0 \%$ | $100 \%$ | $26 \%$ | $24 \%$ | $0 \%$ | $0 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 23 | 106 | 29 | 66 | 404 | 50 | 163 | 62 |
| LT Vol | 23 | 0 | 0 | 20 | 52 | 50 | 0 | 0 |
| Through Vol | 0 | 106 | 0 | 29 | 254 | 0 | 163 | 0 |
| RT Vol | 0 | 0 | 29 | 17 | 98 | 0 | 0 | 62 |
| Lane Flow Rate | 24 | 112 | 31 | 69 | 425 | 53 | 172 | 65 |
| Geometry Grp | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.048 | 0.204 | 0.05 | 0.125 | 0.686 | 0.101 | 0.305 | 0.103 |
| Departure Headway (Hd) | 7.101 | 6.588 | 5.87 | 6.472 | 5.809 | 6.901 | 6.389 | 5.672 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 503 | 543 | 607 | 552 | 621 | 518 | 561 | 630 |
| Service Time | 4.863 | 4.349 | 3.631 | 4.237 | 3.551 | 4.655 | 4.143 | 3.426 |
| HCM Lane V/C Ratio | 0.048 | 0.206 | 0.051 | 0.125 | 0.684 | 0.102 | 0.307 | 0.103 |
| HCM Control Delay | 10.2 | 11 | 8.9 | 10.2 | 20.3 | 10.4 | 11.9 | 9.1 |
| HCM Lane LOS | B | B | A | B | C | B | B | A |
| HCM 95th-tile Q | 0.2 | 0.8 | 0.2 | 0.4 | 5.4 | 0.3 | 1.3 | 0.3 |


|  | 4 | $\rightarrow$ |  | 7 |  | 4 | 4 | 4 | P |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 个个 | 「 | \％${ }^{1 / 4}$ | 个个 | 「 | \％ | 个个 | 「 | ${ }_{4}$ | 个4 | 「 |
| Trafic Volume（vph） | 159 | 1441 | 105 | 325 | 1194 | 114 | 205 | 398 | 357 | 142 | 202 | 92 |
| Future Volume（vph） | 159 | 1441 | 105 | 325 | 1194 | 114 | 205 | 398 | 357 | 142 | 202 | 92 |
| Turn Type | pm＋pt | NA | Perm | Prot | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  | 2 |  |  | 6 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 5 | 2 | 2 | 1 | 6 | 6 | 3 | 8 | 8 | 7 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split（s） | 20.0 | 50.0 | 50.0 | 28.0 | 58.0 | 58.0 | 22.0 | 28.0 | 28.0 | 14.0 | 20.0 | 20.0 |
| Total Split（\％） | 16．7\％ | 41．7\％ | 41．7\％ | 23．3\％ | 48．3\％ | 48．3\％ | 18．3\％ | 23．3\％ | 23．3\％ | 11．7\％ | 16．7\％ | 16．7\％ |
| Yellow Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | Max | Max | None | Max | Max | None | None | None | None | None | None |
| Act Effct Green（s） | 59.3 | 48.0 | 48.0 | 16.4 | 53.2 | 53.2 | 32.4 | 19.2 | 19.2 | 22.0 | 13.1 | 13.1 |
| Actuated g／C Ratio | 0.53 | 0.43 | 0.43 | 0.15 | 0.47 | 0.47 | 0.29 | 0.17 | 0.17 | 0.20 | 0.12 | 0.12 |
| $\mathrm{v} / \mathrm{c}$ Ratio | 0.63 | 0.97 | 0.14 | 0.68 | 0.75 | 0.14 | 0.62 | 0.70 | 0.72 | 0.64 | 0.52 | 0.27 |
| Control Delay | 28.8 | 50.7 | 0.4 | 53.3 | 28.9 | 1.7 | 40.7 | 50.9 | 18.0 | 46.5 | 52.3 | 1.8 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 28.8 | 50.7 | 0.4 | 53.3 | 28.9 | 1.7 | 40.7 | 50.9 | 18.0 | 46.5 | 52.3 | 1.8 |
| LOS | C | D | A | D | C | A | D | D | B | D | D | A |
| Approach Delay |  | 45.4 |  |  | 31.9 |  |  | 36.5 |  |  | 39.8 |  |
| Approach LOS |  | D |  |  | C |  |  | D |  |  | D |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length： 120
Actuated Cycle Length： 112.6
Natural Cycle： 90
Control Type：Semi Act－Uncoord
Maximum v／c Ratio： 0.97
Intersection Signal Delay： 38.3
Intersection LOS：D
Intersection Capacity Utilization 84．6\％
ICU Level of Service E
Analysis Period（min） 15

Splits and Phases：8：Vollmer Rd \＆Stapleton Dr




| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 0.7 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * |  | 4 | 7 |  | $\uparrow$ |
| Traffic Vol, veh/h | 19 | 12 | 483 | 33 | 20 | 333 |
| Future Vol, veh/h | 19 | 12 | 483 | 33 | 20 | 333 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | 235 | - | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 20 | 13 | 508 | 35 | 21 | 351 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 901 | 508 | 0 | 0 | 543 | 0 |
| Stage 1 | 508 | - | - | - | - | - |
| Stage 2 | 393 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 309 | 565 | - | - | 1026 | - |
| Stage 1 | 604 | - | - | - | - | - |
| Stage 2 | 682 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 301 | 565 | - | - | 1026 | - |
| Mov Cap-2 Maneuver | 301 | - | - | - | - | - |
| Stage 1 | 589 | - | - | - | - | - |
| Stage 2 | 682 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 15.8 |  | 0 |  | 0.5 |  |
| HCM LOS | C |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 367 | 1026 | - |
| HCM Lane V/C Ratio |  | - | - | 0.089 | 0.021 | - |
| HCM Control Delay (s) |  | - | - | 15.8 | 8.6 | 0 |
| HCM Lane LOS |  | - | - | C | A | A |
| HCM 95th \%tile Q(veh) |  | - | - | 0.3 | 0.1 | - |




| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh $\quad 16.1$ |  |
| Intersection LOS | C |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | \& |  |  | \$ |  | ${ }^{7}$ | 4 | 「 | ${ }^{*}$ | 4 | 「 |
| Traffic Vol, veh/h | 52 | 230 | 31 | 44 | 77 | 62 | 32 | 225 | 73 | 99 | 159 | 35 |
| Future Vol, veh/h | 52 | 230 | 31 | 44 | 77 | 62 | 32 | 225 | 73 | 99 | 159 | 35 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 55 | 245 | 33 | 47 | 82 | 66 | 34 | 239 | 78 | 105 | 169 | 37 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 1 | 1 | 1 | 1 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  | 3 |  |  | 3 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 3 |  |  | 3 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 3 |  |  | 3 |  |  | 1 |  |  | 1 |  |  |
| HCM Control Delay | 21.7 |  |  | 14.5 |  |  | 14.4 |  |  | 12.9 |  |  |
| HCM LOS | C |  |  | B |  |  | B |  |  | B |  |  |


| Lane | NBLn1 | NBLn2 | NBLn3 | EBLn1 | WBLn1 | SBLn1 | SBLn2 | SBLn3 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $100 \%$ | $0 \%$ | $0 \%$ | $17 \%$ | $24 \%$ | $100 \%$ | $0 \%$ | $0 \%$ |
| Vol Thru, \% | $0 \%$ | $100 \%$ | $0 \%$ | $73 \%$ | $42 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Right, \% | $0 \%$ | $0 \%$ | $100 \%$ | $10 \%$ | $34 \%$ | $0 \%$ | $0 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 32 | 225 | 73 | 313 | 183 | 99 | 159 | 35 |
| LT Vol | 32 | 0 | 0 | 52 | 44 | 99 | 0 | 0 |
| Through Vol | 0 | 225 | 0 | 230 | 77 | 0 | 159 | 0 |
| RT Vol | 0 | 0 | 73 | 31 | 62 | 0 | 0 | 35 |
| Lane Flow Rate | 34 | 239 | 78 | 333 | 195 | 105 | 169 | 37 |
| Geometry Grp | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.072 | 0.475 | 0.138 | 0.646 | 0.389 | 0.226 | 0.339 | 0.067 |
| Departure Headway (Hd) | 7.66 | 7.144 | 6.42 | 6.986 | 7.2 | 7.736 | 7.218 | 6.494 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 467 | 505 | 558 | 516 | 498 | 463 | 497 | 551 |
| Service Time | 5.41 | 4.893 | 4.169 | 4.729 | 4.952 | 5.487 | 4.969 | 4.245 |
| HCM Lane V/C Ratio | 0.073 | 0.473 | 0.14 | 0.645 | 0.392 | 0.227 | 0.34 | 0.067 |
| HCM Control Delay | 11 | 16.2 | 10.2 | 21.7 | 14.5 | 12.7 | 13.7 | 9.7 |
| HCM Lane LOS | B | C | $B$ | C | B | B | B | A |
| HCM 95th-tile Q | 0.2 | 2.5 | 0.5 | 4.6 | 1.8 | 0.9 | 1.5 | 0.2 |


|  | 4 | $\rightarrow$ | $\geqslant$ | 7 |  |  | 4 | 4 |  |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ＊ | 个个 | 「 | ${ }^{7} 1$ | 个个 | 「 | ＊ | 个个 | 「 | ＊ | 个个 | F |
| Traffic Volume（vph） | 58 | 825 | 60 | 361 | 1451 | 66 | 90 | 142 | 125 | 145 | 333 | 124 |
| Future Volume（vph） | 58 | 825 | 60 | 361 | 1451 | 66 | 90 | 142 | 125 | 145 | 333 | 124 |
| Turn Type | pm＋pt | NA | Perm | Prot | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  | 2 |  |  | 6 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 5 | 2 | 2 | 1 | 6 | 6 | 3 | 8 | 8 | 7 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split（s） | 10.0 | 53.0 | 53.0 | 22.0 | 65.0 | 65.0 | 15.0 | 30.0 | 30.0 | 15.0 | 30.0 | 30.0 |
| Total Split（\％） | 8．3\％ | 44．2\％ | 44．2\％ | 18．3\％ | 54．2\％ | 54．2\％ | 12．5\％ | 25．0\％ | 25．0\％ | 12．5\％ | 25．0\％ | 25．0\％ |
| Yellow Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | None | None | None | None | None | None | None |
| Act Efft Green（s） | 43.4 | 38.2 | 38.2 | 15.5 | 51.5 | 51.5 | 23.4 | 14.5 | 14.5 | 26.4 | 19.0 | 19.0 |
| Actuated g／C Ratio | 0.44 | 0.39 | 0.39 | 0.16 | 0.52 | 0.52 | 0.24 | 0.15 | 0.15 | 0.27 | 0.19 | 0.19 |
| v／c Ratio | 0.37 | 0.64 | 0.09 | 0.71 | 0.83 | 0.08 | 0.33 | 0.29 | 0.36 | 0.44 | 0.52 | 0.31 |
| Control Delay | 17.7 | 27.2 | 0.2 | 50.5 | 26.0 | 1.0 | 32.0 | 41.2 | 7.3 | 33.8 | 42.9 | 6.2 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 17.7 | 27.2 | 0.2 | 50.5 | 26.0 | 1.0 | 32.0 | 41.2 | 7.3 | 33.8 | 42.9 | 6.2 |
| LOS | B | C | A | D | C | A | C | D | A | C | D | A |
| Approach Delay |  | 24.9 |  |  | 29.8 |  |  | 27.0 |  |  | 33.1 |  |
| Approach LOS |  | C |  |  | C |  |  | C |  |  | C |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length： 120
Actuated Cycle Length： 99.1
Natural Cycle： 70
Control Type：Actuated－Uncoordinated
Maximum v／c Ratio： 0.83
Intersection Signal Delay： 28.9
Intersection LOS：C
Intersection Capacity Utilization 75．1\％
ICU Level of Service D
Analysis Period（min） 15
Splits and Phases：8：Vollmer Rd \＆Stapleton Dr






| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.4 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  |  | $\uparrow$ | Mr |  |
| Traffic Vol, veh/h | 21 | 1 | 0 | 60 | 4 | 0 |
| Future Vol, veh/h | 21 | 1 | 0 | 60 | 4 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 22 | 1 | 0 | 63 | 4 | 0 |


| Major/Minor M | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 23 | 0 | 86 | 23 |
| Stage 1 | - | - | - | - | 23 | - |
| Stage 2 | - | - | - | - | 63 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1592 | - | 915 | 1054 |
| Stage 1 | - | - | - | - | 1000 | - |
| Stage 2 | - | - | - | - | 960 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1592 | - | 915 | 1054 |
| Mov Cap-2 Maneuver | - | - | - | - | 915 | - |
| Stage 1 | - | - | - | - | 1000 | - |
| Stage 2 | - | - | - | - | 960 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 0 |  | 9 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBT | EBR | WBL WBT |  |
| Capacity (veh/h) |  | 15 | - | - | 1592 | - |
| HCM Lane V/C Ratio |  |  | - | - | - | - |
| HCM Control Delay (s) |  | 9 | - | - | 0 | - |
| HCM Lane LOS |  | A | - | - | A | - |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | 0 | - |




| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.9 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  |  | $\uparrow$ |  | Tr |  |
| Traffic Vol, veh/h | 2 | 17 | 48 | 0 | 1 | 5 |
| Future Vol, veh/h | 2 | 17 | 48 | 0 | 1 | 5 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 2 | 18 | 51 | 0 | 1 | 5 |


| Major/Minor M | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 51 | 0 | - | 0 | 73 | 51 |
| Stage 1 | - | - | - - | - | 51 | - |
| Stage 2 | - | - | - - | - | 22 | - |
| Critical Hdwy | 4.12 | - | - - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1555 | - | - - | - | 931 | 1017 |
| Stage 1 | - | - | - - | - | 971 | - |
| Stage 2 | - | - | - - | - | 1001 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1555 | - | - - | - | 930 | 1017 |
| Mov Cap-2 Maneuver | - | - | - - | - | 930 | - |
| Stage 1 | - | - | - - | - | 970 | - |
| Stage 2 | - | - | - - | - | 1001 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 0.8 |  | 0 |  | 8.6 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 |  |  |
| Capacity (veh/h) |  | 1555 | - | - | - | 1001 |
| HCM Lane V/C Ratio |  | 0.001 | - | - | - | 0.006 |
| HCM Control Delay (s) |  | 7.3 | 0 | - | - | 8.6 |
| HCM Lane LOS |  | A | A | - | - | A |
| HCM 95th \%tile Q(veh) |  | 0 | O | - | - | 0 |




| Intersection |  |
| :--- | ---: |
| Intersection Delay, s/veh $\quad 15.1$ |  |
| Intersection LOS | C |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | \& |  |  | ¢ |  | ${ }^{*}$ | 4 | 「 | ${ }^{*}$ | 4 | 「 |
| Traffic Vol, veh/h | 20 | 29 | 17 | 52 | 254 | 98 | 24 | 109 | 30 | 50 | 164 | 62 |
| Future Vol, veh/h | 20 | 29 | 17 | 52 | 254 | 98 | 24 | 109 | 30 | 50 | 164 | 62 |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 21 | 31 | 18 | 55 | 267 | 103 | 25 | 115 | 32 | 53 | 173 | 65 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 1 | 1 | 1 | 1 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  | 3 |  |  | 3 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 3 |  |  | 3 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 3 |  |  | 3 |  |  | 1 |  |  | 1 |  |  |
| HCM Control Delay | 10.2 |  |  | 20.5 |  |  | 10.6 |  |  | 11.1 |  |  |
| HCM LOS | B |  |  | C |  |  | B |  |  | B |  |  |


| Lane | NBLn1 | NBLn2 | NBLn3 | EBLn1 | WBLn1 | SBLn1 | SBLn2 | SBLn3 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $100 \%$ | $0 \%$ | $0 \%$ | $30 \%$ | $13 \%$ | $100 \%$ | $0 \%$ | $0 \%$ |
| Vol Thru, \% | $0 \%$ | $100 \%$ | $0 \%$ | $44 \%$ | $63 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Right, \% | $0 \%$ | $0 \%$ | $100 \%$ | $26 \%$ | $24 \%$ | $0 \%$ | $0 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 24 | 109 | 30 | 66 | 404 | 50 | 164 | 62 |
| LT Vol | 24 | 0 | 0 | 20 | 52 | 50 | 0 | 0 |
| Through Vol | 0 | 109 | 0 | 29 | 254 | 0 | 164 | 0 |
| RT Vol | 0 | 0 | 30 | 17 | 98 | 0 | 0 | 62 |
| Lane Flow Rate | 25 | 115 | 32 | 69 | 425 | 53 | 173 | 65 |
| Geometry Grp | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.05 | 0.21 | 0.052 | 0.125 | 0.688 | 0.101 | 0.307 | 0.103 |
| Departure Headway (Hd) | 7.107 | 6.594 | 5.875 | 6.495 | 5.826 | 6.912 | 6.4 | 5.684 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 502 | 543 | 607 | 550 | 621 | 517 | 559 | 628 |
| Service Time | 4.869 | 4.356 | 3.637 | 4.261 | 3.569 | 4.67 | 4.158 | 3.44 |
| HCM Lane V/C Ratio | 0.05 | 0.212 | 0.053 | 0.125 | 0.684 | 0.103 | 0.309 | 0.104 |
| HCM Control Delay | 10.2 | 11.1 | 9 | 10.2 | 20.5 | 10.5 | 12 | 9.1 |
| HCM Lane LOS | B | B | A | B | C | B | B | A |
| HCM 95th-tile Q | 0.2 | 0.8 | 0.2 | 0.4 | 5.4 | 0.3 | 1.3 | 0.3 |


| Intersection |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: |
| Intersection Delay, s/veh | 5.3 |  |  |  |
| Intersection LOS | A |  | WB | SB |
| Approach | EB | 1 | 1 | 1 |
| Entry Lanes | 1 | 1 | 1 | 1 |
| Conflicting Circle Lanes | 1 | 101 | 247 | 429 |
| Adj Approach Flow, veh/h | 5 | 103 | 252 | 437 |
| Demand Flow Rate, veh/h | 5 | 215 | 3 | 98 |
| Vehicles Circulating, veh/h | 531 | 40 | 533 | 0 |
| Vehicles Exiting, veh/h | 4 | 0 | 0 | 0 |
| Ped Vol Crossing Leg, \#/h | 0 | 1.000 | 1.000 | 1.000 |
| Ped Cap Adj | 1.000 | 4.1 | 4.2 | A |
| Approach Delay, s/veh | 4.5 | A | A |  |
| Approach LOS | A |  |  |  |


| Lane | Left | Left | Left | Left |
| :--- | ---: | ---: | ---: | ---: |
| Designated Moves | LTR | LTR | LTR | LTR |
| Assumed Moves | LTR | LTR | LTR |  |
| RT Channelized |  |  |  |  |
| Lane Util | 1.000 | 1.000 | 1.000 | 1.000 |
| Follow-Up Headway, s 2.609 | 2.609 | 2.609 | 4.976 |  |
| Critical Headway, s | 4.976 | 4.976 | 4.976 | 437 |
| Entry Flow, veh/h | 5 | 103 | 252 | 1249 |
| Cap Entry Lane, veh/h | 803 | 1108 | 1376 | 0.981 |
| Entry HV Adj Factor | 1.000 | 0.981 | 0.979 | 429 |
| Flow Entry, veh/h | 5 | 101 | 247 | 1224 |
| Cap Entry, veh/h | 803 | 1087 | 1347 | 0.350 |
| V/C Ratio | 0.006 | 0.093 | 0.183 | 6.3 |
| Control Delay, s/veh | 4.5 | 4.1 | 4.2 | A |
| LOS | A | A | A | 2 |


|  | 4 | $\rightarrow$ |  | $\dagger$ |  |  |  | 4 | $p$ |  | $\downarrow$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | \％ | 性 | \％ | \％${ }^{*}$ | 个个 | 「 | \％ | 个4 | \％ | ${ }^{7}$ | 个个 | F |
| Traffic Volume（vph） | 191 | 1449 | 105 | 337 | 1201 | 116 | 205 | 458 | 371 | 143 | 232 | 109 |
| Future Volume（vph） | 191 | 1449 | 105 | 337 | 1201 | 116 | 205 | 458 | 371 | 143 | 232 | 109 |
| Turn Type | pm＋pt | NA | Perm | Prot | NA | Perm | pm＋pt | NA | Perm | pm＋pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  | 2 |  |  | 6 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 5 | 2 | 2 | 1 | 6 | 6 | 3 | 8 | 8 | 7 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split（s） | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split（s） | 20.0 | 52.0 | 52.0 | 27.0 | 59.0 | 59.0 | 21.0 | 27.0 | 27.0 | 14.0 | 20.0 | 20.0 |
| Total Split（\％） | 16．7\％ | 43．3\％ | 43．3\％ | 22．5\％ | 49．2\％ | 49．2\％ | 17．5\％ | 22．5\％ | 22．5\％ | 11．7\％ | 16．7\％ | 16．7\％ |
| Yellow Time（s） | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All－Red Time（s） | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust（s） | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time（s） | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead／Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag |
| Lead－Lag Optimize？ | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | Max | Max | None | Max | Max | None | None | None | None | None | None |
| Act Effct Green（s） | 62.1 | 49.5 | 49.5 | 17.2 | 54.1 | 54.1 | 33.6 | 20.2 | 20.2 | 23.7 | 14.7 | 14.7 |
| Actuated g／C Ratio | 0.54 | 0.43 | 0.43 | 0.15 | 0.47 | 0.47 | 0.29 | 0.17 | 0.17 | 0.20 | 0.13 | 0.13 |
| $\mathrm{V} / \mathrm{c}$ Ratio | 0.74 | 0.98 | 0.14 | 0.70 | 0.77 | 0.15 | 0.65 | 0.76 | 0.77 | 0.70 | 0.54 | 0.31 |
| Control Delay | 39.2 | 52.4 | 0.4 | 54.8 | 30.2 | 1.8 | 43.2 | 54.4 | 24.2 | 52.3 | 53.2 | 2.1 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 39.2 | 52.4 | 0.4 | 54.8 | 30.2 | 1.8 | 43.2 | 54.4 | 24.2 | 52.3 | 53.2 | 2.1 |
| LOS | D | D | A | D | C | A | D | D | C | D | D | A |
| Approach Delay |  | 47.7 |  |  | 33.2 |  |  | 41.2 |  |  | 41.4 |  |
| Approach LOS |  | D |  |  | C |  |  | D |  |  | D |  |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |

Cycle Length： 120
Actuated Cycle Length： 116
Natural Cycle： 90
Control Type：Actuated－Uncoordinated
Maximum v／c Ratio： 0.98
Intersection Signal Delay： 40.8
Intersection LOS：D
Intersection Capacity Utilization 86．9\％
ICU Level of Service $E$
Analysis Period（min） 15
Splits and Phases：8：Vollmer Rd \＆Stapleton Dr






| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.2 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  |  | $\uparrow$ | Mr |  |
| Traffic Vol, veh/h | 66 | 4 | 0 | 38 | 3 | 0 |
| Future Vol, veh/h | 66 | 4 | 0 | 38 | 3 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 69 | 4 | 0 | 40 | 3 | 0 |


| Major/Minor M | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 73 | 0 | 111 | 71 |
| Stage 1 | - | - | - | - | 71 | - |
| Stage 2 | - | - | - | - | 40 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1527 | - | 886 | 991 |
| Stage 1 | - | - | - | - | 952 | - |
| Stage 2 | - | - | - | - | 982 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1527 | - | 886 | 991 |
| Mov Cap-2 Maneuver | - | - | - | - | 886 | - |
| Stage 1 | - | - | - | - | 952 | - |
| Stage 2 | - | - | - | - | 982 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 0 |  | 9.1 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBT | EBR | WBL WBT |  |
| Capacity (veh/h) |  | 886 | - | - | 1527 | WBT |
| HCM Lane V/C Ratio |  | 0.004 | - | - | - | - |
| HCM Control Delay (s) |  | 9.1 | - | - | 0 | - |
| HCM Lane LOS |  | A | - | - | A | - |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.3 |  |  |  |  |  |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | F |  |  | $-\uparrow$ | Mr |  |
| Traffic Vol, veh/h | 58 | 8 | 1 | 35 | 3 | 0 |
| Future Vol, veh/h | 58 | 8 | 1 | 35 | 3 | 0 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 61 | 8 | 1 | 37 | 3 | 0 |


| Major/Minor M | Major1 |  | Major2 |  | Minor1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 0 | 0 | 69 | 0 | 104 | 65 |
| Stage 1 | - | - | - | - | 65 | - |
| Stage 2 | - | - | - | - | 39 | - |
| Critical Hdwy | - | - | 4.12 | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - |  | - | 5.42 | - |
| Follow-up Hdwy | - | - | 2.218 | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | - | - | 1532 | - | 894 | 999 |
| Stage 1 | - | - | - | - | 958 | - |
| Stage 2 | - | - | - | - | 983 | - |
| Platoon blocked, \% | - | - |  | - |  |  |
| Mov Cap-1 Maneuver | - | - | 1532 | - | 893 | 999 |
| Mov Cap-2 Maneuver | - | - | - | - | 893 | - |
| Stage 1 | - | - | - | - | 957 | - |
| Stage 2 | - | - | - | - | 983 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | NB |  |
| HCM Control Delay, s | 0 |  | 0.2 |  | 9 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBLn1 | EBT | EBR | WBL WBT |  |
| Capacity (veh/h) |  | 3 | - | - | 1532 | , |
| HCM Lane V/C Ratio |  |  | - |  | 0.001 | - |
| HCM Control Delay (s) |  | 9 | - | - | 7.4 | 0 |
| HCM Lane LOS |  | A | - | - | A | A |
| HCM 95th \%tile Q(veh) |  | 0 | - | - | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.7 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | -1 | $\uparrow$ |  | Mr |  |
| Traffic Vol, veh/h | 5 | 53 | 32 | 1 | 0 | 4 |
| Future Vol, veh/h | 5 | 53 | 32 | 1 | 0 | 4 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 95 | 95 | 95 | 95 | 95 | 95 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 5 | 56 | 34 | 1 | 0 | 4 |


| Major/Minor M | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 35 | 0 | - | 0 | 101 | 35 |
| Stage 1 | - | - | - - | - | 35 | - |
| Stage 2 | - | - | - - | - | 66 | - |
| Critical Hdwy | 4.12 | - | - - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1576 | - | - - | - | 898 | 1038 |
| Stage 1 | - | - | - - | - | 987 | - |
| Stage 2 | - | - | - - | - | 957 | - |
| Platoon blocked, \% |  | - | - - | - |  |  |
| Mov Cap-1 Maneuver | 1576 | - | - - | - | 895 | 1038 |
| Mov Cap-2 Maneuver | - | - | - - | - | 895 | - |
| Stage 1 | - | - | - - | - | 984 | - |
| Stage 2 | - | - | - - | - | 957 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 0.6 |  | 0 |  | 8.5 |  |
| HCM LOS |  |  |  |  | A |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 |  |  |
| Capacity (veh/h) |  | 1576 | - | - | - | 1038 |
| HCM Lane V/C Ratio |  | 0.003 | 析 | - | - | 0.004 |
| HCM Control Delay (s) |  | 7.3 | 0 | - | - | 8.5 |
| HCM Lane LOS |  | A | A | - | - | A |
| HCM 95th \%tile Q(veh) |  | 0 | O | - | - | 0 |




| Intersection |  |
| :--- | ---: | :--- |
| Intersection Delay, s/veh | 16.2 |
| Intersection LOS | C |


| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Configurations |  | \$ |  |  | \$ |  | ${ }^{1}$ | 4 | 「 | ${ }^{*}$ | 4 | 「 |
| Traffic Vol, veh/h | 52 | 230 | 32 | 45 | 77 | 62 | 33 | 227 | 74 | 99 | 163 | 35 |
| Future Vol, veh/h | 52 | 230 | 32 | 45 | 77 | 62 | 33 | 227 | 74 | 99 | 163 | 35 |
| Peak Hour Factor | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 | 0.94 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 55 | 245 | 34 | 48 | 82 | 66 | 35 | 241 | 79 | 105 | 173 | 37 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 1 | 1 | 1 | 1 |
| Approach | EB |  |  | WB |  |  | NB |  |  | SB |  |  |
| Opposing Approach | WB |  |  | EB |  |  | SB |  |  | NB |  |  |
| Opposing Lanes | 1 |  |  | 1 |  |  | 3 |  |  | 3 |  |  |
| Conflicting Approach Left | SB |  |  | NB |  |  | EB |  |  | WB |  |  |
| Conflicting Lanes Left | 3 |  |  | 3 |  |  | 1 |  |  | 1 |  |  |
| Conflicting Approach Right | NB |  |  | SB |  |  | WB |  |  | EB |  |  |
| Conflicting Lanes Right | 3 |  |  | 3 |  |  | 1 |  |  | 1 |  |  |
| HCM Control Delay | 22 |  |  | 14.7 |  |  | 14.5 |  |  | 13 |  |  |
| HCM LOS | C |  |  | B |  |  | B |  |  | B |  |  |


| Lane | NBLn1 | NBLn2 | NBLn3 | EBLn1 | WBLn1 | SBLn1 | SBLn2 | SBLn3 |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | ---: |
| Vol Left, \% | $100 \%$ | $0 \%$ | $0 \%$ | $17 \%$ | $24 \%$ | $100 \%$ | $0 \%$ | $0 \%$ |
| Vol Thru, \% | $0 \%$ | $100 \%$ | $0 \%$ | $73 \%$ | $42 \%$ | $0 \%$ | $100 \%$ | $0 \%$ |
| Vol Right, \% | $0 \%$ | $0 \%$ | $100 \%$ | $10 \%$ | $34 \%$ | $0 \%$ | $0 \%$ | $100 \%$ |
| Sign Control | Stop | Stop | Stop | Stop | Stop | Stop | Stop | Stop |
| Traffic Vol by Lane | 33 | 227 | 74 | 314 | 184 | 99 | 163 | 35 |
| LT Vol | 33 | 0 | 0 | 52 | 45 | 99 | 0 | 0 |
| Through Vol | 0 | 227 | 0 | 230 | 77 | 0 | 163 | 0 |
| RT Vol | 0 | 0 | 74 | 32 | 62 | 0 | 0 | 35 |
| Lane Flow Rate | 35 | 241 | 79 | 334 | 196 | 105 | 173 | 37 |
| Geometry Grp | 7 | 7 | 7 | 7 | 7 | 7 | 7 | 7 |
| Degree of Util (X) | 0.075 | 0.481 | 0.141 | 0.651 | 0.394 | 0.227 | 0.349 | 0.067 |
| Departure Headway (Hd) | 7.689 | 7.172 | 6.448 | 7.019 | 7.242 | 7.764 | 7.246 | 6.522 |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Cap | 466 | 502 | 555 | 514 | 497 | 462 | 496 | 548 |
| Service Time | 5.443 | 4.926 | 4.201 | 4.765 | 4.996 | 5.52 | 5.002 | 4.278 |
| HCM Lane V/C Ratio | 0.075 | 0.48 | 0.142 | 0.65 | 0.394 | 0.227 | 0.349 | 0.068 |
| HCM Control Delay | 11.1 | 16.4 | 10.3 | 22 | 14.7 | 12.8 | 13.9 | 9.7 |
| HCM Lane LOS | B | C | $B$ | $C$ | $B$ | $B$ | B | A |
| HCM 95th-tile Q | 0.2 | 2.6 | 0.5 | 4.6 | 1.9 | 0.9 | 1.5 | 0.2 |


| Intersection |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: |
| Intersection Delay, s/veh 7.1 |  |  |  |  |
| Intersection LOS | A |  |  |  |
| Approach | EB | WB | SB |  |
| Entry Lanes | 1 | 1 | 1 | 1 |
| Conflicting Circle Lanes | 1 | 1 | 1 | 1 |
| Adj Approach Flow, veh/h | 2 | 67 | 689 | 390 |
| Demand Flow Rate, veh/h | 2 | 68 | 702 | 398 |
| Vehicles Circulating, veh/h | 459 | 582 | 8 | 67 |
| Vehicles Exiting, veh/h | 6 | 128 | 453 | 583 |
| Ped Vol Crossing Leg, \#/h | 0 | 0 | 0 | 0 |
| Ped Cap Adj | 1.000 | 1.000 | 1.000 | 1.000 |
| Approach Delay, s/veh | 4.2 | 5.7 | 8.0 | A |
| Approach LOS | A | A | A |  |
|  |  |  |  |  |


| Lane | Left | Left | Left | Left |
| :--- | ---: | ---: | ---: | ---: |
| Designated Moves | LTR | LTR | LTR | LTR |
| Assumed Moves | LTR | LTR | LTR |  |
| RT Channelized |  |  |  |  |
| Lane Util | 1.000 | 1.000 | 1.000 | 1.000 |
| Follow-Up Headway, s | 2.609 | 2.609 | 2.609 | 4.909 |
| Critical Headway, s | 4.976 | 4.976 | 4.976 | 398 |
| Entry Flow, veh/h | 2 | 68 | 702 | 1289 |
| Cap Entry Lane, veh/h | 864 | 762 | 1369 | 0.981 |
| Entry HV Adj Factor | 1.000 | 0.985 | 0.981 | 390 |
| Flow Entry, veh/h | 2 | 67 | 689 | 1264 |
| Cap Entry, veh/h | 864 | 751 | 1343 | 0.309 |
| V/C Ratio | 0.002 | 0.089 | 0.513 | 5.7 |
| Control Delay, s/veh | 4.2 | 5.7 | 8.0 | A |
| LOS | A | A | 3 | 1 |

## Markup Summary

| 4/12/2018 1:02:21 PM (2) |
| :--- |

## 4/12/2018 12:51:10 PM (1)



Subject: Text Box
Page Label: 25
Lock: Unlocked
Status: shows updated phasing to match the Preliminary

Checkmark: Unchecked
Author: jchodsdon
Date: 4/12/2018 12:51:10 PM
Color:

| 4/12/2018 12:51 | :36 PM (1) |  |
| :---: | :---: | :---: |
| Noinc\|erse | Subject: Text Box | Note(4-12-18): Updated Version in the Preliminary Plan Report |
| Uedated $\begin{aligned} & \text { Uersion in the } \\ & \text { Ver }\end{aligned}$ | Page Label: 35 Lock: Unlocked |  |
|  | Status: |  |
| Col | Checkmark: Unchecked |  |
|  | Author: jchodsdon |  |
|  | Date: 4/12/2018 12:51:36 PM |  |
|  | Color: ${ }^{\square}$ |  |

## 4/12/2018 12:53:15 PM (1)



Subject: Text Box Page Label: 23
Lock: Unlocked
Status:
Checkmark: Unchecked
Author: jchodsdon
Date: 4/12/2018 12:53:15 PM
Color:

Note (4-12-18): The Preliminary Plan Report presents and updated version to match the Preliminary Plan.

## 4/12/2018 12:54:14 PM (1)

|  | Subject: Text Box <br> Page Label: 22 <br> Lock: Unlocked <br> Status: <br> Checkmark: Unchecked <br> Author: jchodsdon <br> Date $4 / 2 / 2 / 2018$ 12:54:14 PM <br>  <br> Color: |
| :--- | :--- |

Note (4-12-18): The Preliminary Plan Report presents and updated version to match the Preliminary Plan.
Status:
Checkmark: Unchecked
Date: 4/12/2018 12:54:14 PM
Color:

## 5/11/2018 1:46:16 PM (1)

| Nill | Subject: Highlight <br> Page Label: 6 <br> Lock: Unlocked <br> Status: <br> Checkmark: Unchecked <br> Author: dsdrice <br> Date: $5 / 11 / 2018 ~ 1: 46: 16 ~ P M ~$ <br> Color: $\square$ |
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5/11/2018 2:08:26 PM (1)
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phase(s) | Subject: Callout |
| :--- |
| Page Label: 10 |
| Lock: Unlocked |
| Status: |$\quad$ Add paving phase(s)

