

PAVEMENT DESIGN REPORT FLYING HORSE NORTH, FILING NO. 3 EL PASO COUNTY, COLORADO

PCD File No. SF2326

Prepared for: Flying Horse North, LLC 2138 Flying Horse Club Drive Colorado Springs, CO 80904

Attn: Adam Doyle

November 4, 2024

Respectfully Submitted,

ENTECH ENGINEERING, INC.

1 mi

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Reviewed by:



Joseph C. Goode III, P.E. Sr. Engineer

LJM:JCG/ljm

Entech Job No. 231192



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1 Introduction

Entech Engineering, Inc. (Entech) completed a subsurface exploration program, laboratory testing, and pavement design for roadways within the Flying Horse North, Filing No. 3. This report describes the subsurface exploration program conducted for the proposed roadway improvements and provides pavement section alternatives and construction recommendations. Entech participated in this project as a subconsultant to Classic Communities. The contents of this report, including the pavement design recommendations, are subject to the limitations and assumptions presented in Section 7.

2 **Project Description**

The site is located southwest of the intersection of Hodgen Road and Black Forest Road within Flying Horse North, Filing No. 3, in El Paso County, Colorado (Figure 1). The proposed improvements include the paving of sections of Allen Ranch Road, Quartz Creek Drive, and the entirety of Bourbon Court. The extent of our investigation is shown in Figure 2.

At the time of our subsurface exploration program, the existing roadway had been rough-graded and utilities had been installed. Surrounding properties are comprised of vacant land, land being developed for future residential lots, and an existing subdivision. Based on the development plans, the roadways are designated as urban local roadways.

3 Subsurface Explorations and Laboratory Testing

3.1 Subsurface Exploration Program

Subsurface conditions at the project site were explored by 26 test borings, designated TB-1 through TB-26, drilled on October 16, 2024. The locations of the test borings are shown on the Site and Exploration Plan (Figure 2). The borings were drilled to depths of 5 to 10 feet below the existing ground surface (bgs). The drilling was performed using a truck-mounted, continuous flight auger drill rig supplied and operated by Entech. Descriptive boring logs providing the lithologies of the subsurface conditions encountered during drilling are presented in Appendix A. Groundwater levels were measured in each of the open boreholes at the conclusion of drilling.

Soil and bedrock samples were obtained from the borings utilizing the Standard Penetration Test (ASTM D1586) using a split-barrel California sampler. Results of the Standard Penetration Test (SPT) are included on the boring logs in terms of N-values expressed in blows per foot (bpf). Soil



and bedrock samples recovered from the borings were visually classified and recorded on the boring logs. The soil classifications were later verified utilizing laboratory testing and grouped by soil type. The soil type numbers are included on the boring logs. It should be understood that the soil descriptions shown on the boring logs may vary between boring location and sample depths. It should also be noted that the lines of stratigraphic separation shown on the boring logs represent approximate boundaries between soil types and the actual stratigraphic transitions may be more gradual or variable with location.

3.2 Geotechnical Index and Engineering Property Testing

Water content testing (ASTM D2216) was performed on the samples recovered from the borings, and the results are shown on the boring logs. Grain-Size Analysis (ASTM D422) and Atterberg Limits testing (ASTM D4318) were performed on selected samples to assist in classifying the materials encountered in the borings.

One-dimensional swell or collapse testing (ASTM D4546) was performed on select samples to determine the swell or collapse potential of the soil. For pavement design, a modified proctor (ASTM D1557) and California Bearing Ratio (CBR) test (ASTM D1883) were completed. Soluble sulfate testing was performed on select soil samples to evaluate the potential for below-grade degradation of concrete due to sulfate attack. The laboratory testing results are presented in Appendix B and summarized in Table B-1.

Strength testing was performed on two sets of soil/cement composite samples for both, Soil Type 1 and Soil Type 2. Testing was performed on soil samples prepared with 2% and 4% Portland Cement Type 1L. A compression strength of 125 pounds per square inch (psi) is recommended for cement-stabilized subgrade. The 6-day average strength value of the 2% mix was 205 psi and 184 psi, respectively and the 6-day strength of the 4% mix was 232 psi and 211 psi, respectively. A 2% mix is recommended based on the laboratory test results. A summary of the testing results is attached in Appendix B, Table B-2.

4 Subgrade Conditions

Two primary soil types and one bedrock type were encountered in the test borings drilled for the subsurface investigation. Each soil type was classified in accordance with the Unified Soil Classification System (USCS) and the American Association of State Highway and Transportation



Officials (AASHTO) soil classification system using the laboratory testing results and the observations made during drilling.

4.1 Subsurface Conditions

Subsurface conditions along the proposed roadways consisted of loose to dense silty sand and sand with silt fill (Soil Type 1, AASHTO A-1-b, A-2-4, and A-2-6) and loose to dense clayey to silty sand fill and stiff to very stiff sandy clay fill (Soil Type 2, AASHTO A-4 and A-6). Extremely weak to very weak sandstone bedrock, or very dense silty sand or sand with silt when classified as a soil (Soil Type 3, AASHTO A-1-b, A-2-6) was encountered in three of the test borings. Water soluble sulfate testing results showed 0.00% sulfates which indicates that the soils exhibit a negligible potential for sulfate attack.

4.2 Groundwater

Groundwater was not encountered in the test borings. Groundwater fluctuations are possible and will depend on seasonal variations, local precipitation, runoff, and other factors, however, we do not anticipate groundwater to affect the proposed construction.

5 Pavement Design Recommendations

Pavement design recommendations were made in accordance with the *El Paso County Engineering Criteria Manual (ECM).*

5.1 Subgrade Conditions

California Bearing Ratio (CBR) testing was performed on representative samples of the Soil Type 1 silty sand fill subgrade from TB-3 and from Soil Type 2 clayey sand fill from TB-21 to determine the support characteristic of the subgrade soils. The results of the CBR testing are presented in Appendix B and summarized in Exhibit 1.



Design Parameter	Value							
Soil Type	1 – Silty Sand Fill	2 – Clayey Sand Fill						
CBR at 95%	18.2	6.5						
Design CBR	10	6.5						
Liquid Limit	NV	28						
Plasticity Index	NP	7						
Percent Passing 200	27.9	48.3						
AASHTO Classification	A-2-4	A-4						
Unified Soils Classification	SC	SC						

Exhibit 1: Subsurface Laboratory Testing Summary

5.2 Swell Mitigation

El Paso County requires swell mitigation for soils with swell testing results greater than 2% under a 150 pounds per square foot (psf) surcharge. Based on the subgrade soils classification and swell testing, mitigation for expansive soils will not be required on this site.

5.3 Traffic Loading

Traffic data is not available for the future interior roads in the Flying Horse North, Filing No. 3 subdivision; however, the Quartz Creek Drive, Bourbon Court and portions of Allen Ranch Road, are classified as rural local roadways based on current development plans. Allen Ranch Road is also classified as an urban local roadway between STA 52+79.43 and STA 78+06.31 where the proposed roadway ties into the existing Allen Ranch Road. Refer to the Traffic Impact Study, *Flying Horse North Filing 3 / Traffic Generation Analysis PCD File No. SF2326, SM ROCHA, LLC TRAFFIC AND TRANSPORTATION CONSULTANTS* for additional information. The *El Paso County Engineering Criteria Manual* provides default 18-kip equivalent single axle loadings (ESAL) based on the street classifications (ECM Section D.3.3, Table D-2). For design, default ESAL values of 292,000 and 36,500 were used for the urban local roadway (Allen Ranch Road) and rural local roadway (Quartz Creek Drive, Bourbon Court, and portions of Allen Ranch Road) designations, respectively.

5.4 Pavement Design

The pavement sections were determined utilizing the *El Paso County Engineering Criteria Manual*, the CBR testing, and default ESALs. Design parameters used in the pavement analysis are presented in Exhibit 2.



Design Parameter	Value
Reliability - Urban Local	80%
Reliability – Rural Local	75%
Standard Deviation	0.45
Serviceability Loss (Δ psi)	2.5
Design CBR	6.5
Resilient Modulus	9,750 psi
Structural Coefficients	
Hot Bituminous Pavement	0.44
Aggregate Base Course	0.11
Recycled Concrete Base	0.11
Cement Stabilized Subgrade	0.11

Exhibit 2: Pavement Design Parameters

Pavement section alternatives recommended for the roadways included in this phase filing are summarized in Exhibit 3. The pavement design calculations are presented in Appendix C.

Exhibit 3: Recommended Pavement Sections

Pavement Area	Roadway Classification	Design ESAL	Alternative ¹
Allen Ranch Road	Urban Local	292,000	1. 3.5 inches HMA over 8.0 inches ABC/RCB
Allen Kanen Koad	Orban Local	232,000	2. 3.5 inches HMA over 8.0 inches CTS
Allen Ranch Road, Quartz Creek Drive,	Rural Local	36,500	1. 3.0 inches HMA over 4.0 inches ABC/RCB
Bourbon Court		30,300	2. 3.0 inches HMA over 8.0 inches CTS

ABC = Aggregate Base Course; ESAL = equivalent single axle loads; HMA = Hot Mix Asphalt; CTS = Cement Treated Soil; RCB= Recycled Concrete Base

Notes:

1. The use of CTS will require a deviation request approval.

6 Construction Recommendations

Pavement design recommendations provided herein are contingent on good construction practices, and poor construction techniques may result in poor performance. Our analyses assumed that this project will be constructed according to the *El Paso County Engineering Criteria Manual* and the *Pikes Peak Region Asphalt Paving Specifications*.

6.1 Earthwork Recommendations for Pavement Subgrade

Proper subgrade preparation is required for adequate pavement performance. Paving areas should be cleared of all deleterious materials including but not limited to: existing pavements, utility poles, and fence poles. Surface vegetation, if any, should be removed by stripping, with the



depth to be field determined. Isolated pockets of high cohesive soils such as those encountered in boring TB-22 should be removed and replaced with granular fill. Granular soils can be placed in accordance with Section 6.1.3.

6.1.1 Subgrade Preparation – Unbound Base Alternatives

If pavement section alternatives are selected utilizing aggregate base course (ABC) or Recycled Concrete Base (RCB), the final subgrade surface should be scarified to a depth of 8 inches, moisture conditioned within +/- 2% of the optimum water content, and recompacted to 95% of the Modified Proctor (ASTM 1557) maximum dry density.

The compacted surface below pavements should be proof-rolled with a fully loaded, tandem-axle, 10-yard dump truck or equivalent. Any areas, that are delineated to be soft, loose, or yielding during proof-rolling should be removed and reconditioned or replaced.

6.1.2 Subgrade Preparation – Cement Treated Subgrade

Prior to placement of cement stabilization a preliminary proof roll should be completed with a fully loaded, tandem-axle, 10-yard dump truck or equivalent. Any areas that are delineated to be soft, loose, or yielding during proof-rolling should be removed and reconditioned or replaced.

Following the preliminary proof roll, the subgrade shall be stabilized by the addition of cement. The amount of cement applied shall be a minimum of 2% (by weight) of the subgrade's maximum dry density as determined by the Modified Proctor (ASTM D1557) for granular soils or by the Standard Proctor (ASTM D698) for cohesive soils. The cement should be spread evenly on the subgrade surface and be thoroughly mixed into the subgrade such that a uniform blend of soil and cement is achieved to the CTS design depth. Compaction of the cement-stabilized subgrade should be completed to obtain at least 95% of the subgrade maximum dry density as determined by the Modified Proctor (ASTM D1557) or by the Standard Proctor (ASTM D698). Satisfactory compaction of the subgrade shall occur within 90 minutes from the time of mixing the cement into the subgrade.

The following conditions shall be observed as part of the subgrade stabilization:

- Type I/II or Type 1L cement as supplied; a local supplier shall be used. All cement used for stabilization should come from the same source. If cement sources are changed, a new laboratory mix design should be completed.
- Moisture conditioning of the subgrade and/or mixing of the cement into the subgrade shall not



occur when soil temperatures are below 40 degrees F. Cement treated subgrades should be maintained at a temperature of 40 degrees F or greater until the subgrade has been compacted as required.

- Cement placement, cement mixing, and compaction of the cement treated subgrade should be observed by Entech Engineering. Testing should include in-situ compaction tests and representative compacted specimens of the treated subgrade material for subsequent laboratory quality assurance testing. Testing reports will be provided to El Paso County as construction progresses.
- A minimum 7-day CTS compressive strength of 125 psi must be achieved.
- Soil strengths in excess of 275 psi will require microfracturing. Microfracturing will be completed using the Standard Method as defined by the *City of Colorado Springs Draft Standard Specification,* Section 305 Chemically Treated Subgrade. Microfracturing will be performed with the same (or equivalent tonnage) steel drum vibratory roller used for compaction of the CTS. A minimum of 12-ton roller shall be used. Three full passes with the roller operating at maximum amplitude and traveling at 2- 3 mph shall be applied. If the treated material breaks up excessively at the surface, the vibration amplitude shall be decreased or eliminated.

6.1.3 Fill Placement and Compaction

Granular fill placed as part of the pavement subgrade shall consist of non-expansive, granular soil, free of organic matter, unsuitable materials, debris, and cobbles greater than 3 inches in diameter. Additionally, any granular fill placed as part of the roadway subgrade should have a minimum CBR of 6. All granular fill placed within the pavement subgrade should be compacted to a minimum of 95% of the Modified Proctor (ASTM D1557) maximum dry density at +/-2% of optimum moisture content. Fill material should be placed in horizontal lifts such that each finished lift has a compacted thickness of 6 inches or less. Entech should approve any imported fill to be used within the pavement subgrade area prior to delivery to the site.

6.1.4 Aggregate Base Course and Recycled Concrete Base

ABC or RCB materials shall conform to the *El Paso County Standard Specifications Manual*, Section 300 Aggregate Base Course. ABC or RCB materials should be compacted to a minimum of 95% of the Modified Proctor (ASTM D1557) maximum dry density within +/-2% of optimum moisture content.



6.2 Concrete Degradation Due to Sulfate Attack

Sulfate solubility testing was conducted on several samples recovered from the test borings to evaluate the potential for sulfate attack on concrete. The test results indicated less than 0.01% soluble sulfate (by weight). The test results indicate the sulfate component of the in-place soils presents a negligible to severe exposure threat to concrete placed below the site grade.

As presented in *Evaluation of Selected Pavement Specifications and Responses to Questions Relevant to Design and Construction of Cement-Treated Soil and Aggregate Layers in El Paso County, Colorado* report from Spencer Gutherie and Robert Stevens dated March 13, 2024 soils with less than 3,000 ppm (0.3%) do not require special construction practices.

6.3 Construction Observation

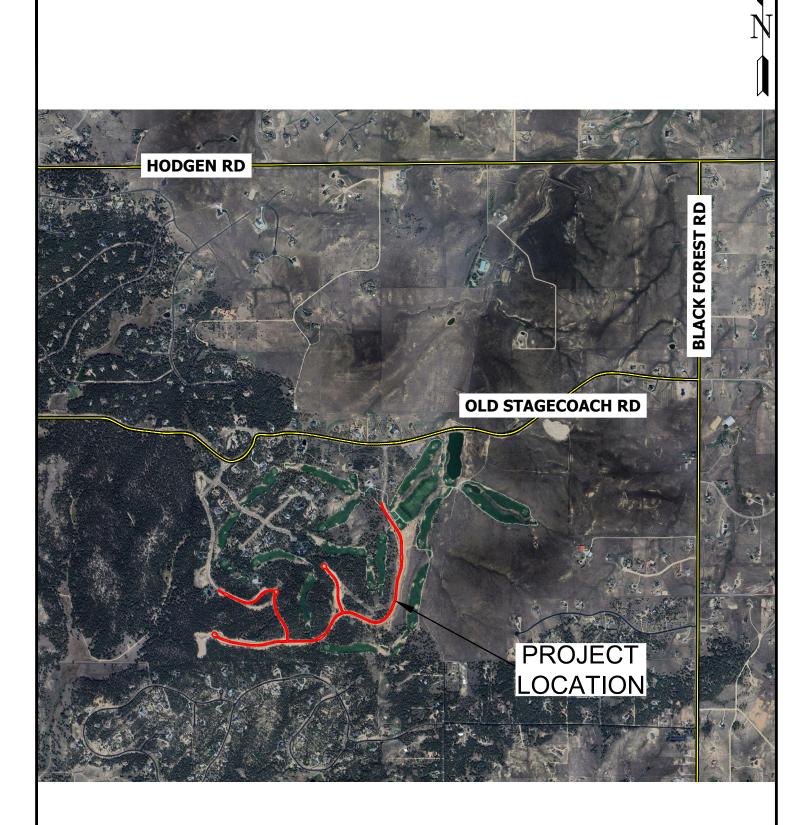
Subgrade preparation for pavement structures should be observed by Entech in order to verify that (1) no anomalies are present, (2) materials similar to those described in this report have been encountered or placed, and (3) no soft spots, expansive or organic soil, or debris are present in the pavement subgrade prior to paving. Construction observation requirements as presented in the Use of CTS for Paving Season Memorandum should be followed.

7 Closure

The subsurface investigation, geotechnical evaluation, and recommendations presented in this report are intended for use by Classic Communities with application to the paving of the Flying Horse North, Filing No. 3 project in El Paso County, Colorado. In conducting the subsurface investigation, laboratory testing, engineering evaluation, and reporting, Entech Engineering, Inc. endeavored to work in accordance with generally accepted professional geotechnical and geologic practices and principles consistent with the level of care and skill ordinarily exercised by members of the geotechnical profession currently practicing in the same locality and under similar conditions. No other warranty, expressed or implied, is made. During final design and/or construction, if conditions are encountered that appear different from those described in this report, Entech Engineering, Inc. requests to be notified so that the evaluation and recommendations presented herein can be reviewed and modified as appropriate.

If there are any questions regarding the information provided herein, or if Entech Engineering, Inc. can be of further assistance, please do not hesitate to contact us.

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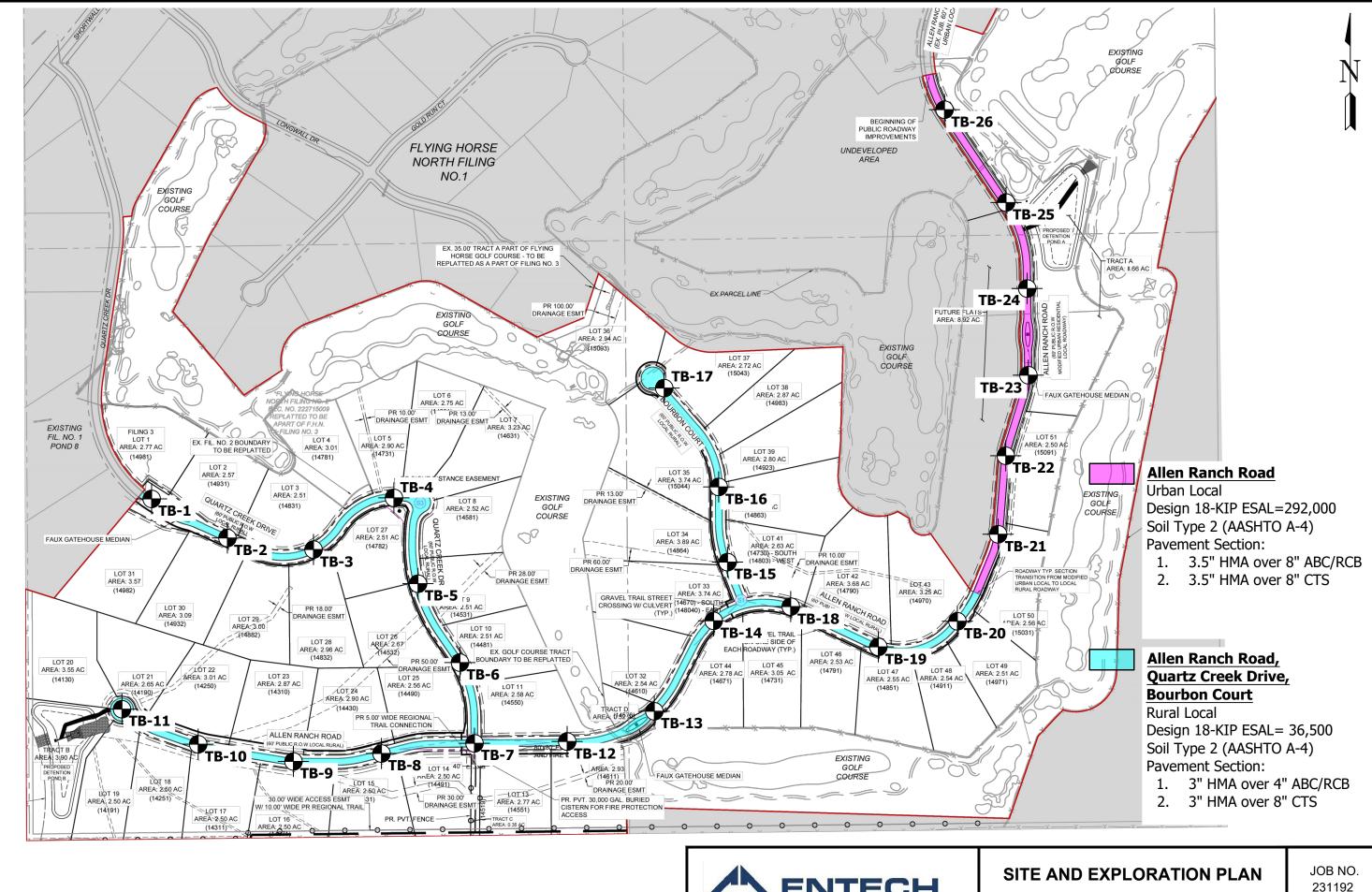




VICINITY MAP

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC JOB NO. 231192

FIG. 1





TB- APPROXIMATE TEST BORING LOCATION AND NUMBER

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

FIG. 2



APPENDIX A: Test Boring Logs

TEST BORING 1 DATE DRILLED 10/16/202						TEST BORING 2 DATE DRILLED 10/16/20					
REMARKS DRY TO 5', 10/16/24	Depth (ft)	Symbol Samles	Blows per foot	Watercontent %	Soil Type	REMARKS DRY TO 5', 10/16/24	Depth (ft)	Symbol Samples	Blows per foot	Watercontent %	Soil Type
FILL 0-5', SAND, WITH SILT, BROWN to TAN, MEDIUM DENSE to LOOSE, MOIST	10 15 20		16 7	3.2 9.1		FILL 0-5', SAND, SILTY, TAN, MEDIUM DENSE, MOIST	5 10 15 20		20 23		
							-	1			



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FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

TEST BORING 3 DATE DRILLED 10/16/202	24						TEST BORING 4 DATE DRILLED 10/16/2024
REMARKS	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	REMARKS
FILL 0-10', SAND, SILTY, TAN, MEDIUM DENSE, MOIST				m 25 25 11	8.6 7.4 13.1		DRY TO 5', 10/16/24 A B
						FL	TEST BORING LOGSJOB NO. 231192_YING HORSE NORTH, FILING NO. 3EIG A-2

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

TEST BORING 5 DATE DRILLED 10/16/202				TEST BORING 6 DATE DRILLED 10/16/20					
REMARKS DRY TO 5', 10/16/24	Depth (ft) Symbol Samples	Blows per foot Watercontent %	Soil Type	REMARKS DRY TO 10', 10/16/24	Depth (ft)	Symbol	earnples Blows per foot	Watercontent %	Soil Type
FILL 0-5', SAND, CLAYEY, TAN, MEDIUM DENSE, MOIST	5	11 8.1 20 9.3		FILL 0-10', SAND, CLAYEY, TAN, MEDIUM DENSE, MOIST	5	$\overline{)}$	21 25	8.4 6.9	2
	10				10	· / / / · · · ·	23	6.0	2
	15 - - 20 -				15 				
					-				



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7)24				DATE DRILLED 10/16/20					
Depth (ft) Symbol	Samples Blows per foot	Watercontent %	Soil Type		Jepth (ft)	Symbol	samples Blows per foot	Watercontent %	Soil Type
	16			FILL 0-5', SAND, CLAYEY, TAN,	-		20	7.6	2
5	19	8.2	1		5		15	8.7	2
10					10				
15					15				
20					20				
					-	1			
	024 10 15 10 15 10 15 10 10 15 10 10 10 10 10 10 10 10 10 10	10 10 10 10 10 10 10 10 10 10	10 15 10 15 10 15 10 15 10 15 10 15 10 10 15 10 10 10 15 10 10 15 10 10 10 10 10 10 10 10 10 10	12 12 12 12 12 14 15 15 15 15 15 16 16 16 16 16 16 16 16 17 17 17 17 17 17 17 17 17 17	D24 DATE DRILLED 10/16/203 (1) 10 10/16/203 REMARKS (1) 10 10 10 10 (1) 10 10 10 10 10 (1) 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 15 10 10 10 10 10 10 10 10 10 10 15 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10 10	D24 DATE DRILLED 10/16/2024 (I) I I I REMARKS IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	D24 DATE DRILLED 10/16/2024 (i) io	D24 DATE DRILLED 10/16/2024 (1) 10 10 10/16/2024 (1) 10 10 10 10 (1) 10 10 10 10 10 (1) 10 10 10 10 10 10 (1) 10 10 10 10 10 10 10 10 10 10 10 10 15 15 15 15	D24 DATE DRILLED 10/16/2024 (1) 100<



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TEST BORING LOGS

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TEST BORING S DATE DRILLED 10/16/20							TEST BORING 10 DATE DRILLED 10/16/20						
REMARKS DRY TO 10', 10/16/24	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	REMARKS DRY TO 5', 10/16/24	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
FILL 0-10', SAND, CLAYEY, SILTY, TAN, MEDIUM DENSE to DENSE, MOIST	5			15 33	6.6 12.1		FILL 0-5', SAND, SILTY, BROWN, MEDIUM DENSE to DENSE, MOIST	5			29 35	6.3 7.4	1
	10			21	5.4	1		10					
	15 20							15 20					



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FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

TEST BORING 11 DATE DRILLED 10/16/20				TEST BORING 12 DATE DRILLED 10/16/202					
REMARKS DRY TO 5', 10/16/24	Depth (ft) Symbol Samples	Blows per foot Watercontent %	Soil Type	REMARKS DRY TO 10', 10/16/24	Depth (ft)	Symbol Samples	Blows per foot	Watercontent %	Soil Type
FILL 0-5', SAND, SILTY, BROWN, MEDIUM DENSE to DENSE, MOIST		19 7.6		FILL 0-4', SAND, SILTY, TAN, DENSE, MOIST			36	15.2	
	5 <u>• [·]</u>	24 6.1		SANDSTONE, EXTREMELY WEAK, TAN, MODERATELY WEATHERED (SAND, SILTY, VERY DENSE, MOIST)	5		<u>50</u> 9"	8.3	3
	10 <u>-</u>				10		<u>50</u> 8"	11.1	3
	15				15				
	20				20				



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TEST BORING 13 DATE DRILLED 10/16/20			_	TEST BORING 14 DATE DRILLED 10/16/20			_		
REMARKS DRY TO 5', 10/16/24	Depth (ft) Symbol Samples	Blows per foot Watercontent %	Soil Type	REMARKS DRY TO 5', 10/16/24	Depth (ft)	Symbol Samples	Blows per foot	Watercontent %	Soil Type
FILL 0-5', SAND, CLAYEY, LIGHT BROWN, MEDIUM DENSE, MOIST		24 7.0		FILL 0-5', CLAY, SANDY, BROWN, VERY STIFF, MOIST	-		17	8.5	2
	5	18 6.8	1		5		17	7.1	2
					10				
	15 <mark>-</mark>				15				
	20				20				



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TEST BORING 15						TEST BORING 16	3			
DATE DRILLED 10/16/2024	Ļ					DATE DRILLED 10/16/20				
REMARKS				%		REMARKS			<i>°</i>	
			oot	ent 9				oot	ent 9	
(##)		se	per 1	conte	be			per1	conte	be
t t		Samples	Blows per foot	Watercontent %	Soil Type		Depth (ft) Symbol Samples	Blows per foot	Watercontent %	Soil Type
		v Sa	BIG	W8		DRY TO 5', 10/16/24			Ň	So
FILL 0-10', SAND, CLAYEY, BROWN to TAN, MEDIUM DENSE, MOIST		- 	24	8.0		FILL 0-5', SAND, WITH SILT, TAN, DENSE, MOIST		38	9.6	1
						- ,				
	5		11	6.7	2		5	40	14.6	1
1	ب <mark>ر</mark> 0	<u>,</u>	14	9.5	2		10			
1	5						15			
	-						-			
2										
2	.0						20			
	I	I						I	I	I
								Ι		
						TEST BORING LOG	S		JOB I 2311	
					FL	YING HORSE NORTH, FILING	NO. 3		2311	92
		101 10				FLYING HORSE NORTH. LL		1	FIG.	A-8

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TEST BORING 17 DATE DRILLED 10/16/202							TEST BORING 18 DATE DRILLED 10/16/20						
REMARKS	24						REMARKS	24					
DRY TO 5', 10/16/24	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	DRY TO 10', 10/16/24	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
FILL 0-5', SAND, SILTY, TAN,	-			18	6 1	4	FILL 0-6', SAND, CLAYEY, BROWN,	-	~		30	0.7	1
MEDIUM DENSE, MOIST	-			18	6.1	1	DENSE to MEDIUM DENSE, MOIST	-	/		30	9.7	1
	5			13	8.5	1		5	/		28	15.3	1
	-						SANDSTONE, VERY WEAK, TAN, MODERATELY WEATHERED						
	10						(SAND, WITH SILT, VERY DENSE, MOIST)	10			<u>50</u> 8"	8.1	3
	-							-			U		
	15							15					
	-							-					
	20							20					
							l						



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TEST BORING 19 DATE DRILLED 10/16/20					TEST BORING 20 DATE DRILLED 10/16/20					
REMARKS DRY TO 5', 10/16/24	Depth (ft) Symbol Samoles	Blows per foot	Watercontent %	Soil Type	REMARKS DRY TO 5', 10/16/24	Depth (ft)	Symbol	Blows per foot	Watercontent %	Soil Type
FILL 0-5', SAND, CLAYEY, TAN, MEDIUM DENSE, MOIST	5	17 16	5.5 4.5		FILL 0-5', SAND, CLAYEY, TAN, MEDIUM DENSE, MOIST	5		27 28	4.3	1
	10					10				
	15 - 20					15 20				
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TEST BORING 21 DATE DRILLED 10/16/202						TEST BORING 22 DATE DRILLED 10/16/2024			
	Symbol Symbol	Samples	o c c Blows per foot	% Matercontent %				% Matercontent %	
					FL	TEST BORING LOGS YING HORSE NORTH, FILING NO FLYING HORSE NORTH, LLC). 3	JOB I 2311 IG. /	92

TEST BORING 23 DATE DRILLED 10/16/202	24						TEST BORING 24 DATE DRILLED 10/16/20					
REMARKS DRY TO 5', 10/16/24	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	REMARKS DRY TO 10', 10/16/24	Depth (ft)	Symbol	Samples Blowe per foot	Watercontent %	Soil Type
FILL 0-5', SAND, CLAYEY, TAN, MEDIUM DENSE, MOIST	-	//		14	7.9		FILL 0-10', SAND, CLAYEY, TAN, MEDIUM DENSE to LOOSE, MOIST	1	///////////////////////////////////////		2 11.8	
	5	//		10	9.6	2		5	///////////////////////////////////////	ç	9 13.7	2
	10							10	·/· ·/· ·/·	1	1 10.1	2
	15							15				
	-							-				
	20							20				



JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

TEST BORING 25 DATE DRILLED 10/16/20							TEST BORING 2 DATE DRILLED 10/16/2	26 024			
REMARKS					9		REMARKS			%	
DRY TO 5', 10/16/24	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	DRY TO 5', 10/16/24	Depth (ft) Symbol	Samples Blows per foot	Watercontent %	Soil Type
FILL 0-5', CLAY, SANDY, TAN,				<u> </u>			FILL 0-5', CLAY, SANDY, TAN,				
STIFF, MOIST	5				10.6 9.9	2	STIFF, MOIST	5	8	8.3 8.9	2 2
	10							10			
	15							15			
	20							20			
	I			I			I	1 1 1	I	I	I
						FL	TEST BORING LOC YING HORSE NORTH, FILING FLYING HORSE NORTH, L	G NO. 3	F	JOB 2311 FIG. /	192



APPENDIX B: Laboratory Test Results

TABLE B-1SUMMARY OF LABORATORY TEST RESULTS



SOIL TYPE	TEST BORING		WATER		PASSING NO. 200 SIEVE	LIQUID LIMIT	PLASTIC LIMIT	PLASTIC INDEX	SULFATE	SWELL/ COLLAPSE	·		
	NO.	(FT)	(%)	(PCF)	(%)				(WT %)	(%)	INDEX)	USCS	SOIL DESCRIPTION
1, CBR	3	0-3	7.9		27.9	NV	NP	NP			A-2-4 (0)	SM	FILL, SAND, SILTY
1	1	1-2	3.1		8.2	NV	NP	NP	0.00		A-1-b (0)	SW-SM	FILL, SAND, WITH SILT
1	2	1-2	10.1		15.3	NV	NP	NP			A-1-b (0)	SM	FILL, SAND, SILTY
1	3	1-2	8.6		19.0	NV	NP	NP			A-1-b (0)	SM	FILL, SAND, SILTY
1	7	1-2	8.4		33.8	29	19	10			A-2-4 (0)	SC	FILL, SAND, CLAYEY
1	9	1-2	6.6		32.3	25	18	7			A-2-4 (0)	SC-SM	FILL, SAND, CLAYEY, SILTY
1	10	1-2	6.3		15.3	NV	NP	NP			A-1-b (0)	SM	FILL, SAND, SILTY
1	11	1-2	7.6		21.9	23	19	4			A-2-4 (0)	SM	FILL, SAND, SILTY
1	13	1-2	7.0		30.0	30	20	10			A-2-4 (0)	SC	FILL, SAND, CLAYEY
1	16	1-2	9.6		11.1	NV	NP	NP	0.00		A-1-b (0)	SW-SM	FILL, SAND, WITH SILT
1	17	1-2	6.1		12.3	NV	NP	NP			A-1-b (0)	SM	FILL, SAND, SILTY
1	18	1-2	9.7		16.9	39	23	16			A-2-6 (0)	SC	FILL, SAND, CLAYEY
1	19	1-2	5.5		24.1	27	19	8			A-2-4 (0)	SC	FILL, SAND, CLAYEY
2, CBR	21	0-3	14.0		48.3	28	21	7			A-4 (1)	SC	FILL, SAND, CLAYEY
2	5	1-2	8.1		43.6	27	19	8			A-4 (1)	SC	FILL, SAND, CLAYEY
2	6	1-2	8.4		38.0	31	22	9			A-4 (0)	SC	FILL, SAND, CLAYEY
2	8	1-2	9.8	103.9	44.4	29	19	10		0.1	A-6 (2)	SC	FILL, SAND, CLAYEY
2	12	1-2	16.6	105.6	37.7	NV	NP	NP		0.2	A-4 (0)	SM	FILL, SAND, SILTY
2	14	1-2	16.1	108.6	62.9	30	19	11		0.6	A-6 (5)	CL	FILL, CLAY, SANDY
2	15	1-2	8.0		42.7	28	20	8			A-4 (0)	SC	FILL, SAND, CLAYEY
2	20	1-2	4.3		45.4	23	14	9			A-4 (1)	SC	FILL, SAND, CLAYEY
2	21	1-2	14.0	107.5	54.0	30	21	9		0.7	A-4 (3)	CL	FILL, CLAY, SANDY
2	22	1-2	14.8	99.4	76.2	28	19	9	0.00	0.9	A-4 (5)	CL	FILL, CLAY, WITH SAND
2	23	1-2	7.6	106.2	44.4	23	14	9		-0.5	A-4 (1)	SC	FILL, SAND, CLAYEY
2	24	1-2	11.8		47.9	28	19	9			A-4 (2)	SC	SAND, CLAYEY
2	25	1-2	10.1	115.3	51.2	27	17	10		0.3	A-4 (2)	CL	FILL, CLAY, SANDY
2	26	1-2	8.3		54.9	28	20	8			A-4 (2)	CL	FILL, CLAY, SANDY
2	15	0-3	5.4		40.3	29	21	8			A-4 (0)	SC	FILL, SAND, CLAYEY
3	4	1-2	9.6		17.0	44	27	17	0.00		A-2-6 (0)	SM	SANDSTONE (SAND, SILTY)
3	12	10	11.1		14.3	NV	NP	NP	0.00		A-1-b (0)	SM	SANDSTONE (SAND, SILTY)
3	18	10	8.1		10.2	NV	NP	NP			A-1-b (0)	SW-SM	SANDSTONE (SAND, WITH SILT)



TABLE B-2SUMMARY OF CTS TEST RESULTS

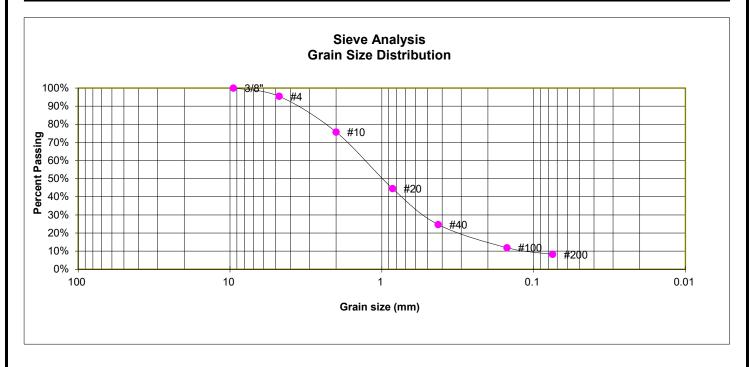
FIELD SAMPLE ID	SOIL ADDITIVE	ADDITIVE PERCENTAGE (%)	WATER CONTENT (%)	DENSITY (dry)	AGE (days)	STRENGTH (psi)
				120.4		204
TB-3 @ 0-3'	TYPE IL CEMENT	2	7.1	120.7	5	213
				120.5		199
				A	VERAGE:	205
TB-3 @ 0-3' TYPE IL CEMEN				121.1		244
	TYPE IL CEMENT	4	7.1	120.3	5	238
				120.6		214
				A	VERAGE:	232
				104.4		192
TB-21 @ 0-3'	TYPE IL CEMENT	2	14.9	104.5	5	176
				104.3		183
					VERAGE:	184
				104.6		210
TB-21 @ 0-3'	TYPE IL CEMENT	4	14.9	104.6	5	223
				104.6		200
				A	VERAGE:	211

Notes:

1. CURING METHOD: 100° HUMIDIFIED OVEN

TEST BORING 1 DEPTH (FT) 1-2

SOIL DESCRIPTION FILL, SAND, WITH SILT SOIL TYPE 1



GRAIN SIZE ANALYSIS

Percent
<u>Finer</u>
100.0%
95.5%
75.7%
44.6%
24.7%
11.9%
8.2%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SW-SM AASHTO CLASSIFICATION: A-1-b AASHTO GROUP INDEX: 0

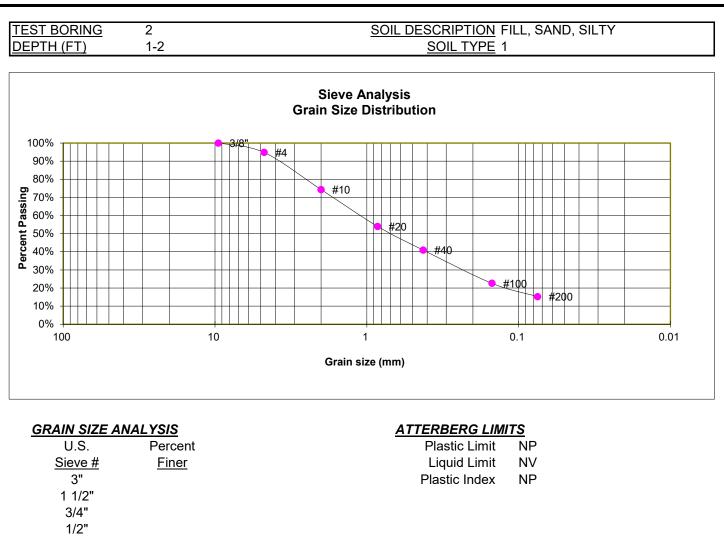
ATTERBERG LIMITS

Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



•.	
1/2"	
3/8"	100.0%
4	94.8%
10	74.3%
20	54.0%
40	40.9%
100	22.6%
200	15.3%

SOIL CLASSIFICATION

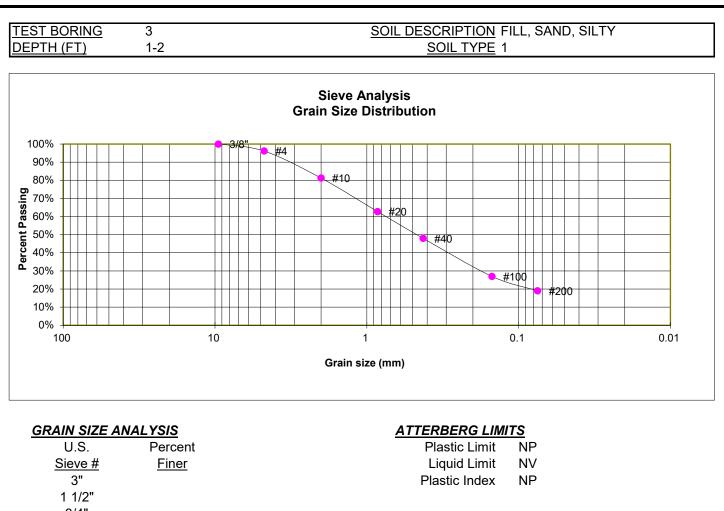
USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-1-b
AASHTO GROUP INDEX:	0



LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



3/4"	
1/2"	
3/8"	100.0%
4	96.2%
10	81.2%
20	62.8%
40	48.0%
100	27.0%
200	19.0%

SOIL CLASSIFICATION

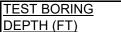
USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-1-b
AASHTO GROUP INDEX:	0



LABORATORY TEST RESULTS

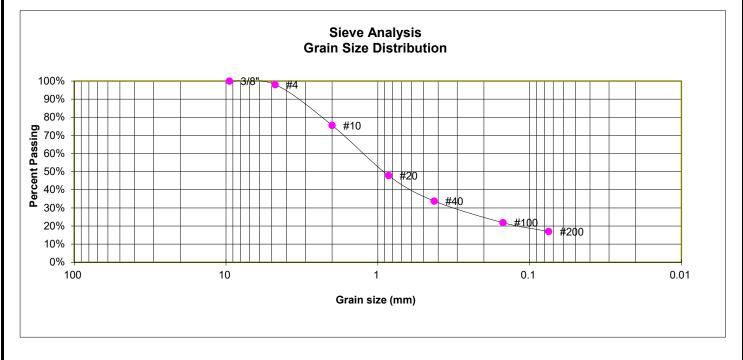
JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



4 1-2

SOIL DESCRIPTION SANDSTONE (SAND, SILTY) SOIL TYPE 3



GRAIN SIZE ANALYSIS

U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	98.1%
10	75.6%
20	47.9%
40	33.8%
100	21.9%
200	17.0%

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-2-6
AASHTO GROUP INDEX:	0

ATTERBERG LIMITS

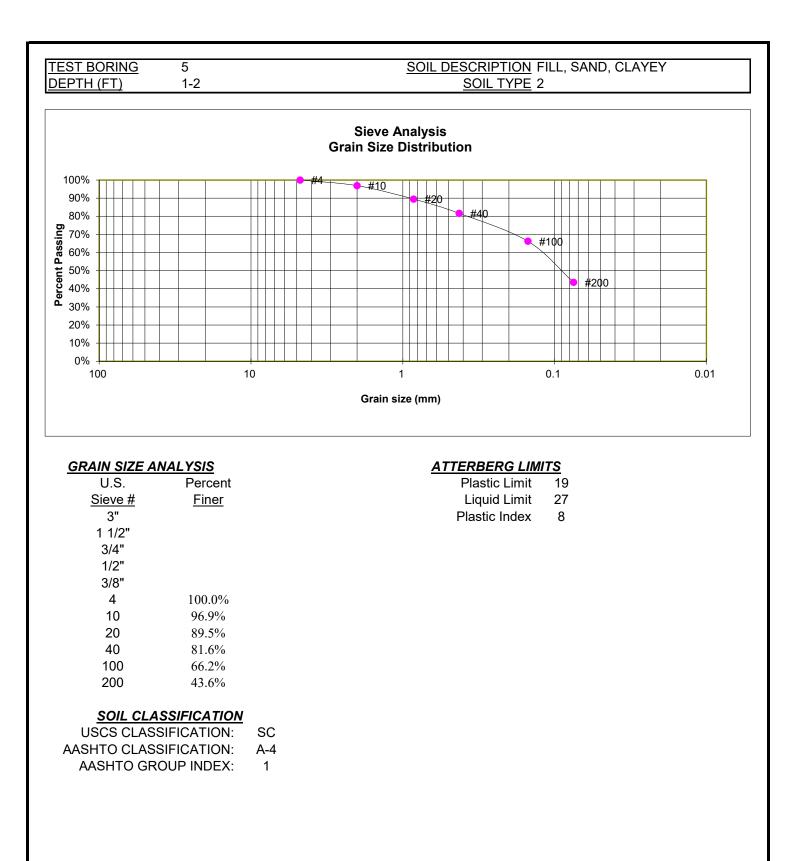
Plastic Limit	27

- Liquid Limit 44
- Plastic Index 17

LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC





LABORATORY TEST RESULTS

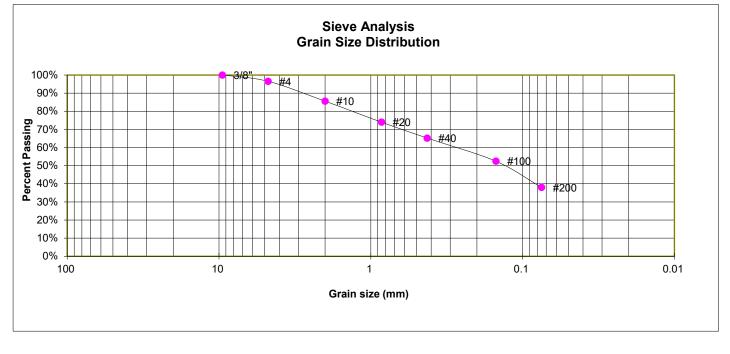
JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

TEST BORING	
DEPTH (FT)	

6 1-2

SOIL DESCRIPTION FILL, SAND, CLAYEY SOIL TYPE 2



GRAIN SIZE ANALYSIS

U.S.	Percent	
Sieve #	Finer	
3"		
1 1/2"		
3/4"		
1/2"		
3/8"	100.0%	
4	96.6%	
10	85.6%	
20	74.0%	
40	65.2%	
100	52.5%	
200	38.0%	

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-4
AASHTO GROUP INDEX:	0

ATTERBERG LIMITS

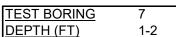
Pla	stic	Lir	nit	22
			••	~ 4

- Liquid Limit 31
- Plastic Index 9

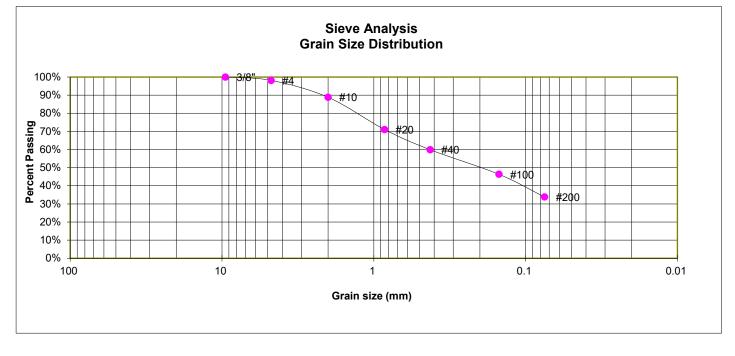
LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



SOIL DESCRIPTION FILL, SAND, CLAYEY SOIL TYPE 1



GRAIN SIZE ANALYSIS

U.S.	Percent
<u>Sieve #</u>	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	98.2%
10	88.9%
20	71.1%
40	59.9%
100	46.4%
200	33.8%

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-2-4
AASHTO GROUP INDEX:	0

ENGINEERING, INC.

ATTERBERG LIMITS

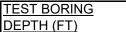
Plas	stic	Lim	nit	19

- Liquid Limit 29
- Plastic Index 10

LABORATORY TEST RESULTS

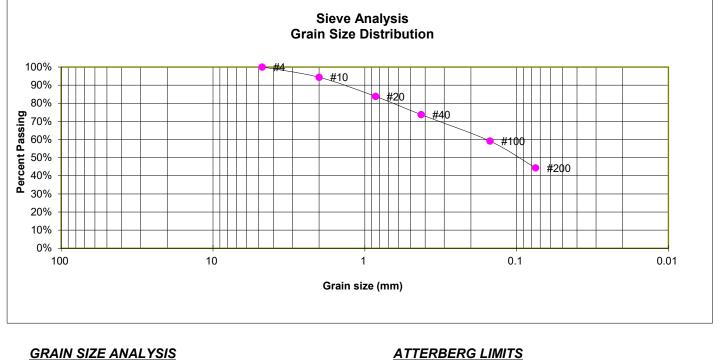
JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



8 1-2

SOIL DESCRIPTION FILL, SAND, CLAYEY SOIL TYPE 2



U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	94.4%
20	83.8%
40	73.7%
100	59.2%
200	44.4%

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-6
AASHTO GROUP INDEX:	2

ENGINEERING, INC.

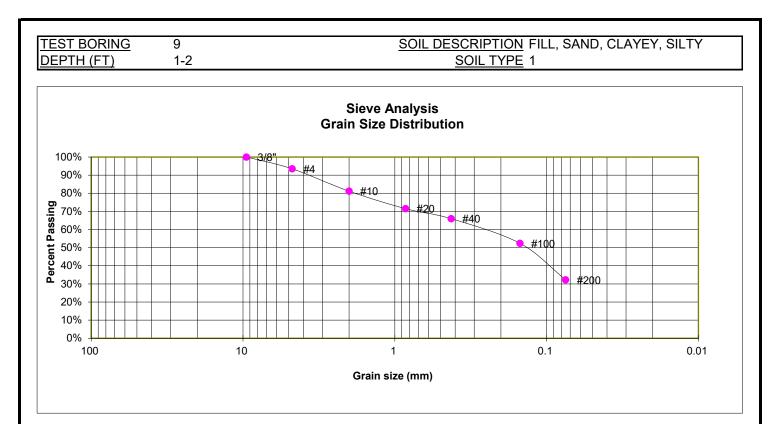
ATTERBERG LIMITS

Ρ	las	tic	Lir	nit	19

- Liquid Limit 29
- Plastic Index 10

LABORATORY TEST RESULTS

JOB NO. 231192



GRAIN SIZE ANALYSIS

U.S.	Percent	
<u>Sieve #</u>	<u>Finer</u>	
3"		
1 1/2"		
3/4"		
1/2"		
3/8"	100.0%	
4	93.5%	
10	81.2%	
20	71.5%	
40	66.0%	
100	52.4%	
200	32.3%	

SOIL CLASSIFICATION

USCS CLASSIFICATION: SC-SM AASHTO CLASSIFICATION: A-2-4 AASHTO GROUP INDEX: 0



ATTERBERG LIMITS

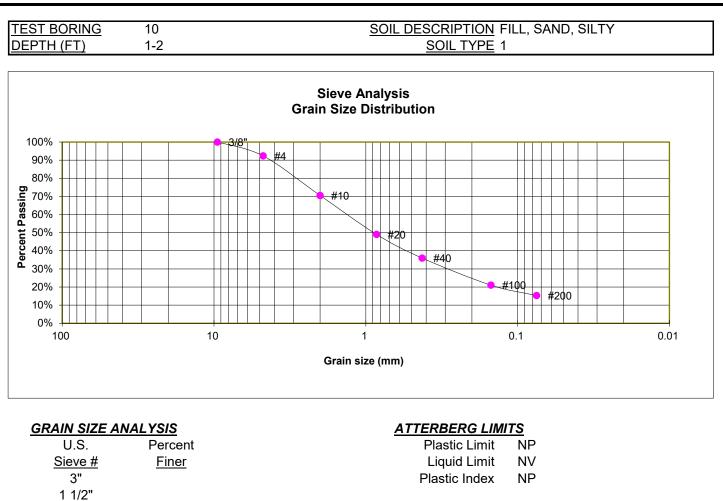
Plastic Limit	18
Liquid Limit	25

Plastic Index 7

LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



1 1/2	
3/4"	
1/2"	
3/8"	100.0%
4	92.4%
10	70.5%
20	49.1%
40	36.0%
100	21.0%
200	15.3%

SOIL CLASSIFICATION

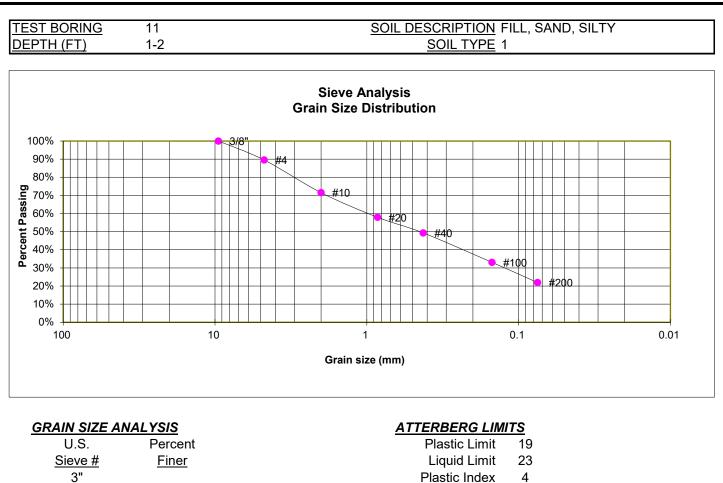
USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-1-b
AASHTO GROUP INDEX:	0



LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	89.6%
10	71.5%
20	57.9%
40	49.3%
100	33.1%
200	21.9%

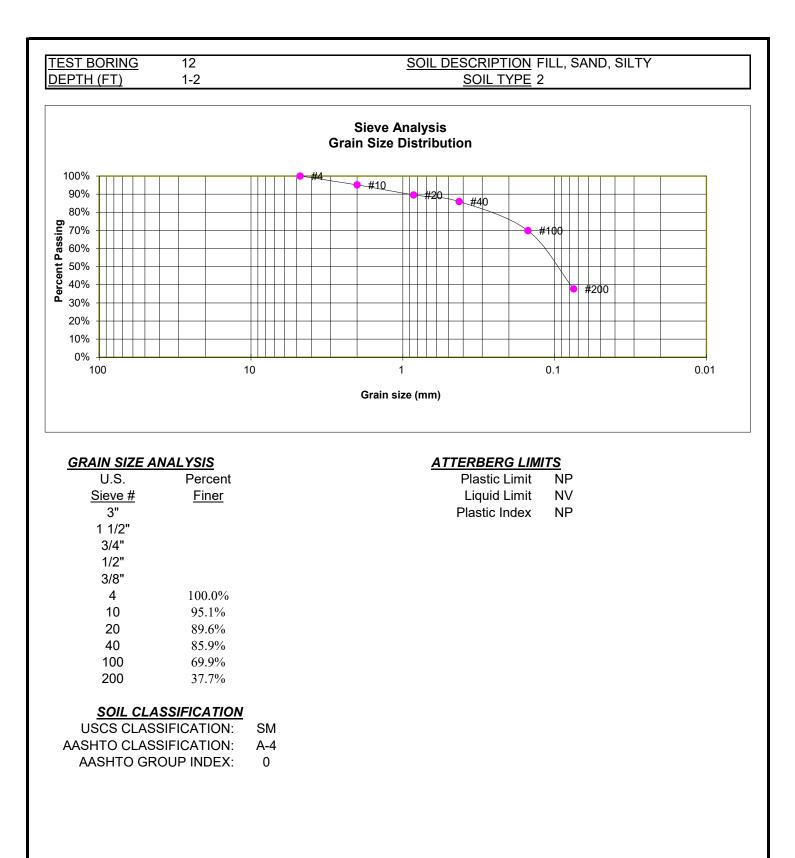
SOIL CLASSIFICATION

USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-2-4
AASHTO GROUP INDEX:	0

LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

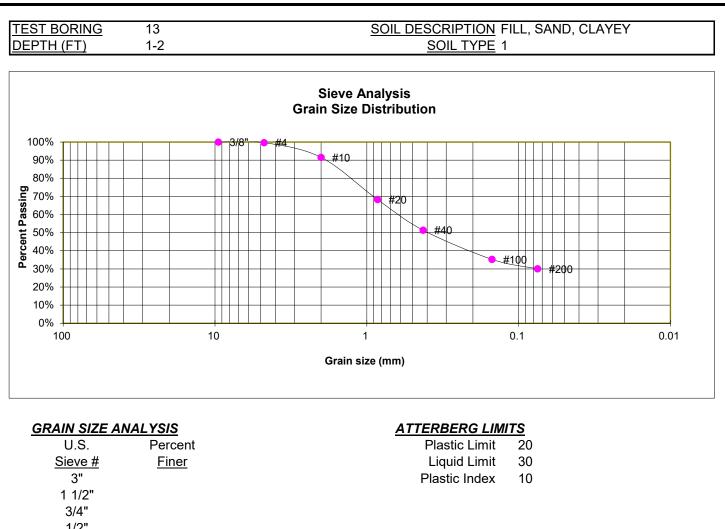




LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



1/2"	
3/8"	100.0%
4	99.7%
10	91.5%
20	68.3%
40	51.3%
100	35.3%
200	30.0%

SOIL CLASSIFICATION

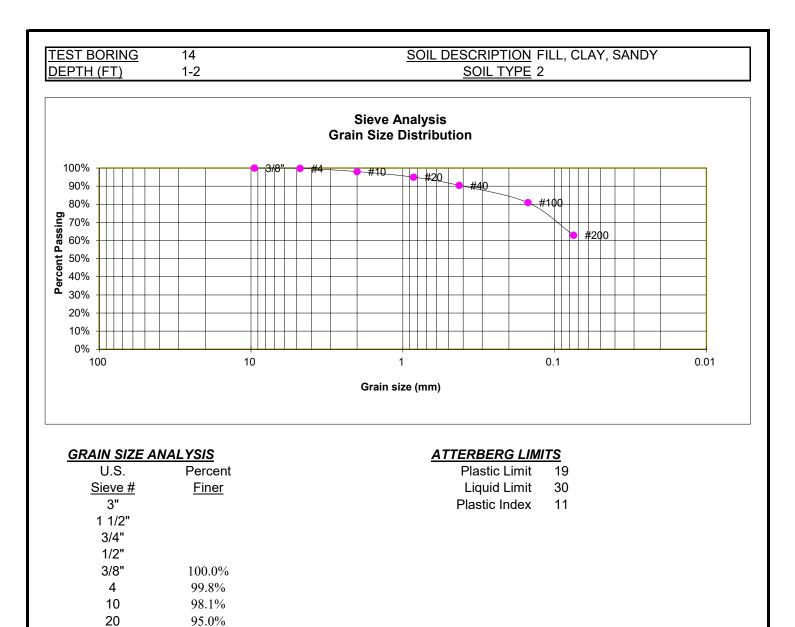
USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-2-4
AASHTO GROUP INDEX:	0



LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



100	80.9%
200	62.9%

90.4%

40

SOIL CLASSIFICATION

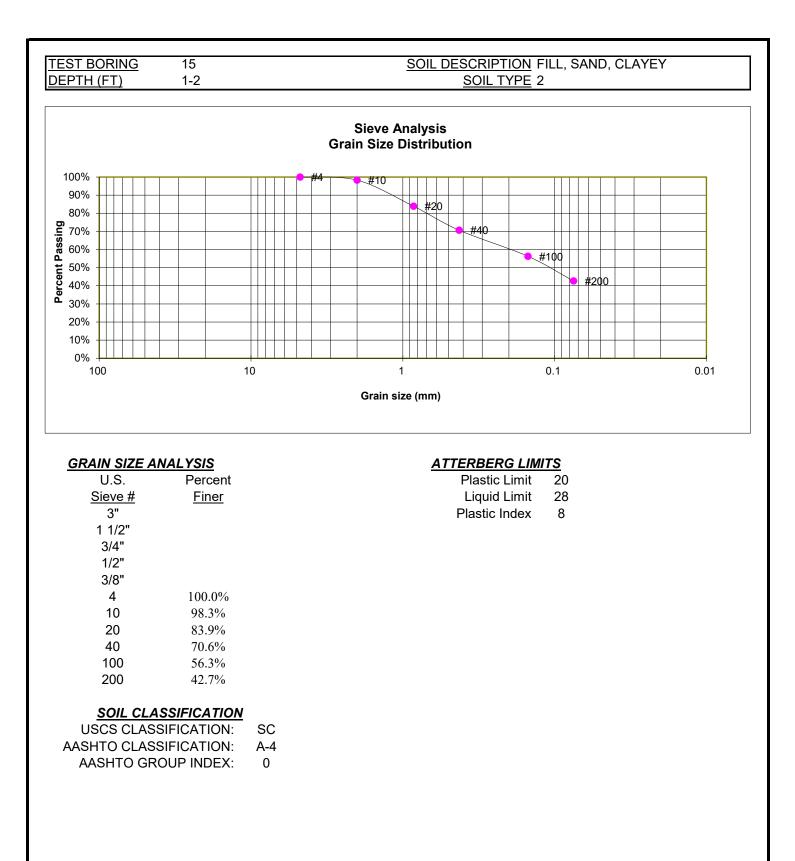
USCS CLASSIFICATION:	CL
AASHTO CLASSIFICATION:	A-6
AASHTO GROUP INDEX:	5



LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

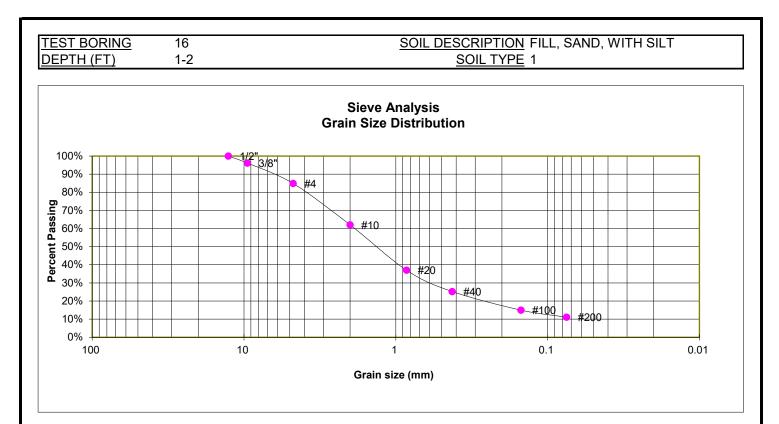




LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



GRAIN SIZE ANALYSIS

Percent
<u>Finer</u>
100.0%
96.1%
84.9%
62.0%
37.0%
25.2%
15.0%
11.1%

ATTERBERG LIMITS

Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

SOIL CLASSIFICATION

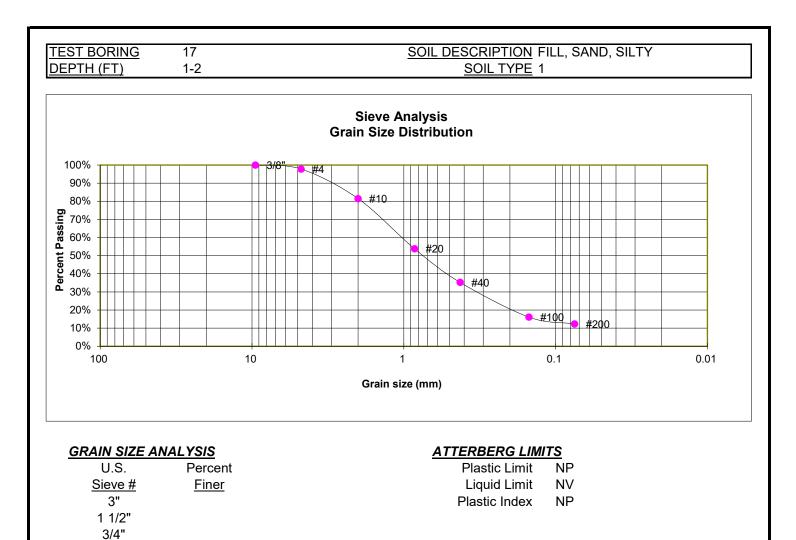
USCS CLASSIFICATION: SW-SM AASHTO CLASSIFICATION: A-1-b AASHTO GROUP INDEX: 0



LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



200	12.3%

100.0%

97.8% 81.5%

53.8%

35.2%

16.1%

1/2" 3/8"

4

10 20

40

100

SOIL CLASSIFICATION

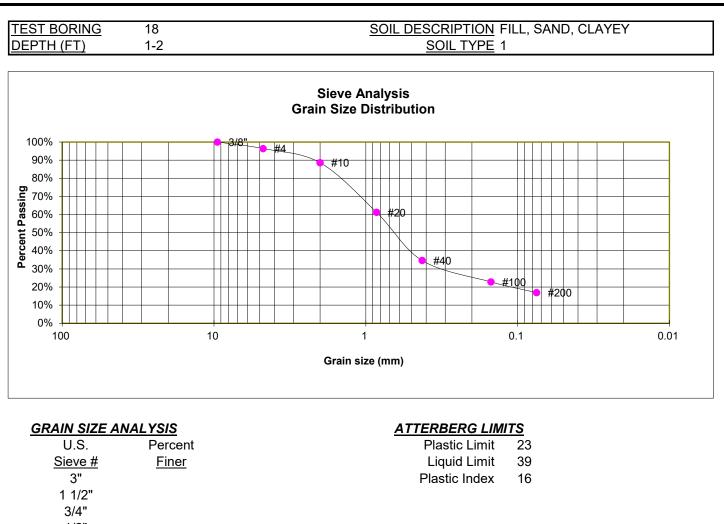
USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-1-b
AASHTO GROUP INDEX:	0



LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



1 1/2	
3/4"	
1/2"	
3/8"	100.0%
4	96.4%
10	88.6%
20	61.3%
40	34.6%
100	22.8%
200	16.9%

SOIL CLASSIFICATION

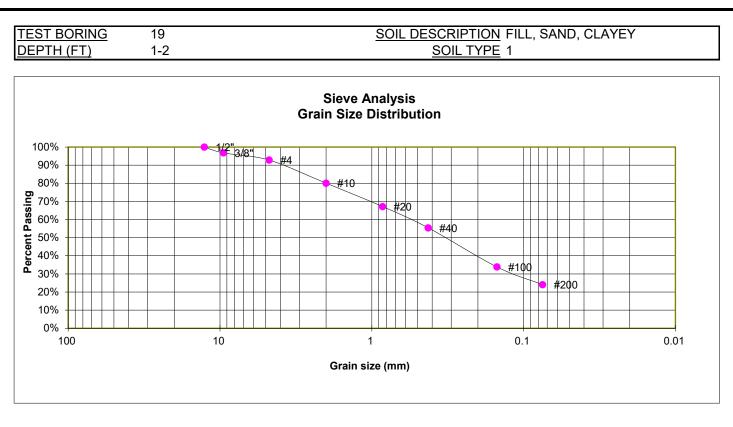
USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-2-6
AASHTO GROUP INDEX:	0



LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



GRAIN SIZE ANALYSIS

U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	96.8%
4	92.8%
10	80.1%
20	67.2%
40	55.4%
100	33.9%
200	24.1%

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-2-4
AASHTO GROUP INDEX:	0

ATTERBERG LIMITS

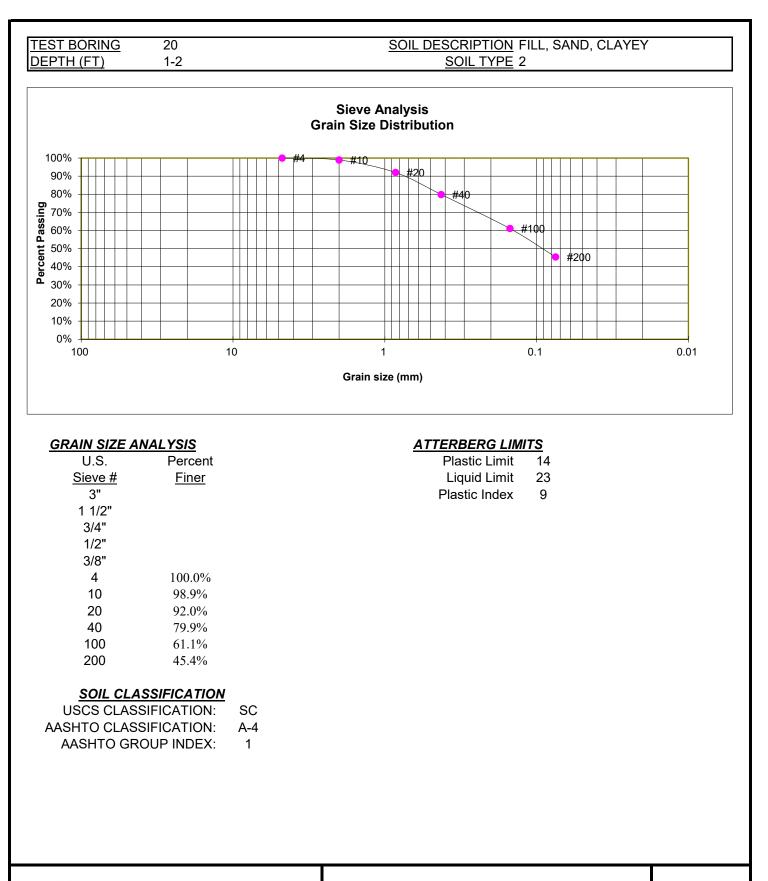
Plastic Limit	19
Liquid Limit	27

Plastic Index 8

LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

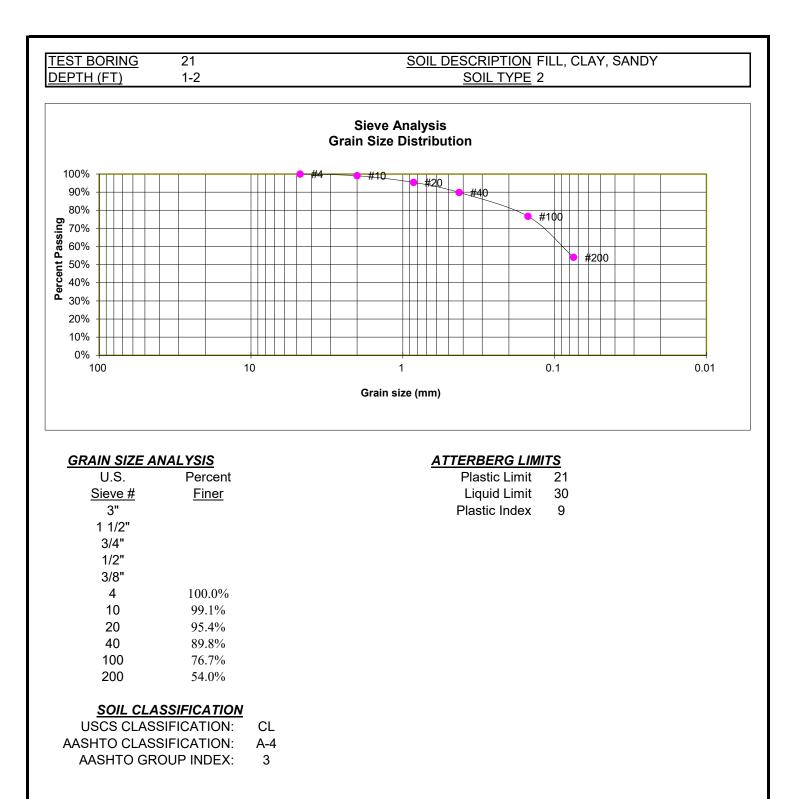




LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC





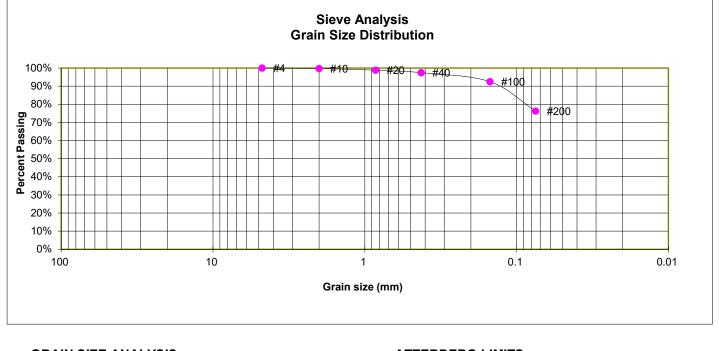
LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

TEST BORING	22
DEPTH (FT)	1-2

SOIL DESCRIPTION FILL, CLAY, WITH SAND SOIL TYPE 2



<u>GRAIN</u>	SIZE	<u>ANALYSIS</u>	
	0	-	

.

U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	
4	100.0%
10	99.7%
20	98.8%
40	97.4%
100	92.5%
200	76.2%

SOIL CLASSIFICATION

USCS CLASSIFICATION:	CL
AASHTO CLASSIFICATION:	A-4
AASHTO GROUP INDEX:	5

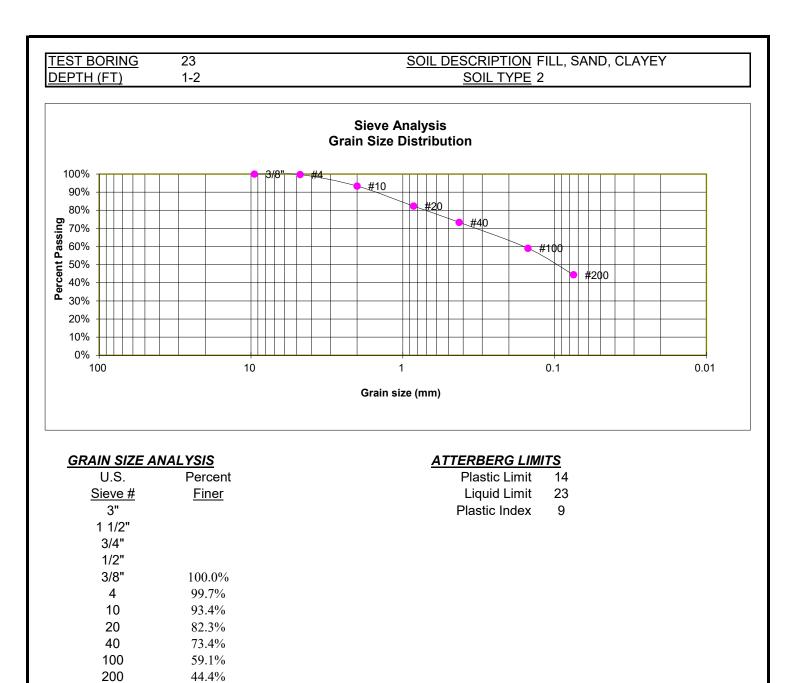
ATTERBERG LIMITS

Plastic Limit	19
لنعيبنا لنحينا	20

- Liquid Limit 28
- Plastic Index 9

LABORATORY TEST RESULTS

JOB NO. 231192



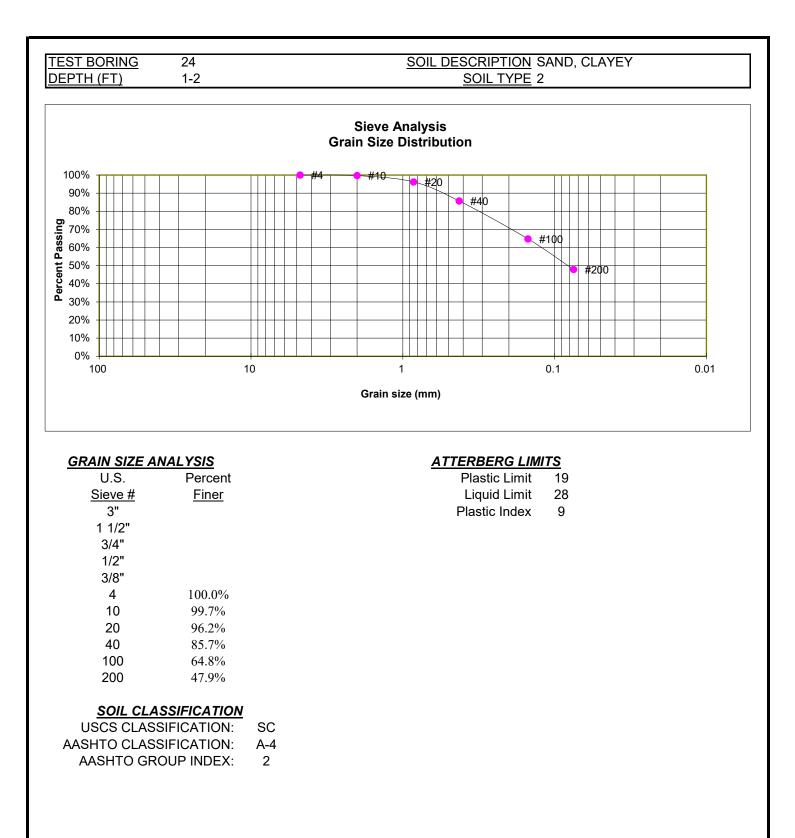
SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-4
AASHTO GROUP INDEX:	1

LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

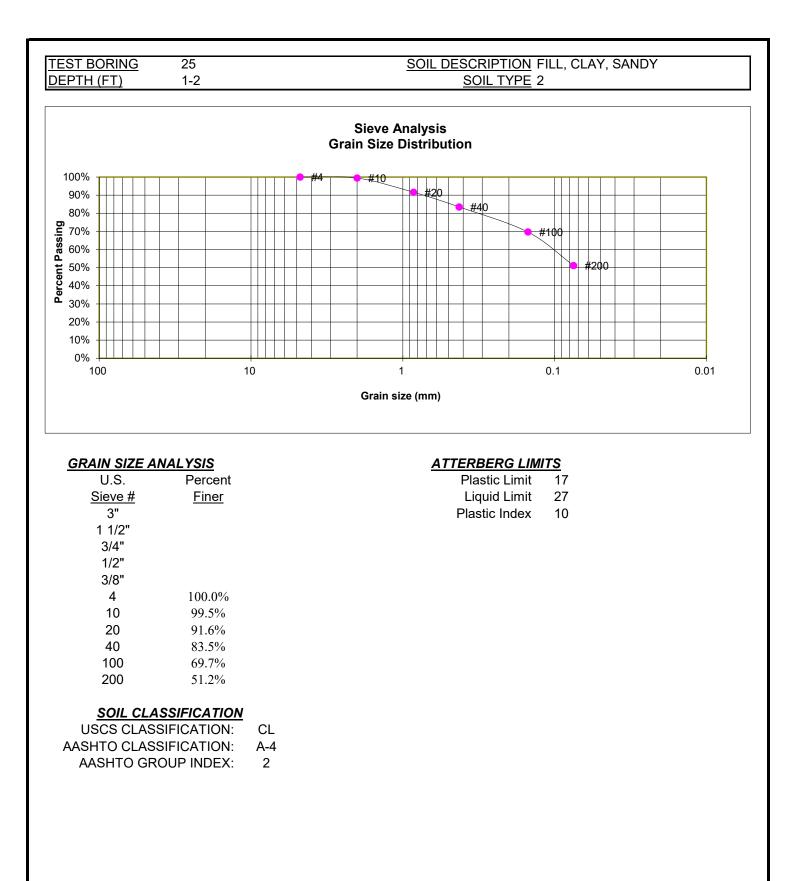




LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

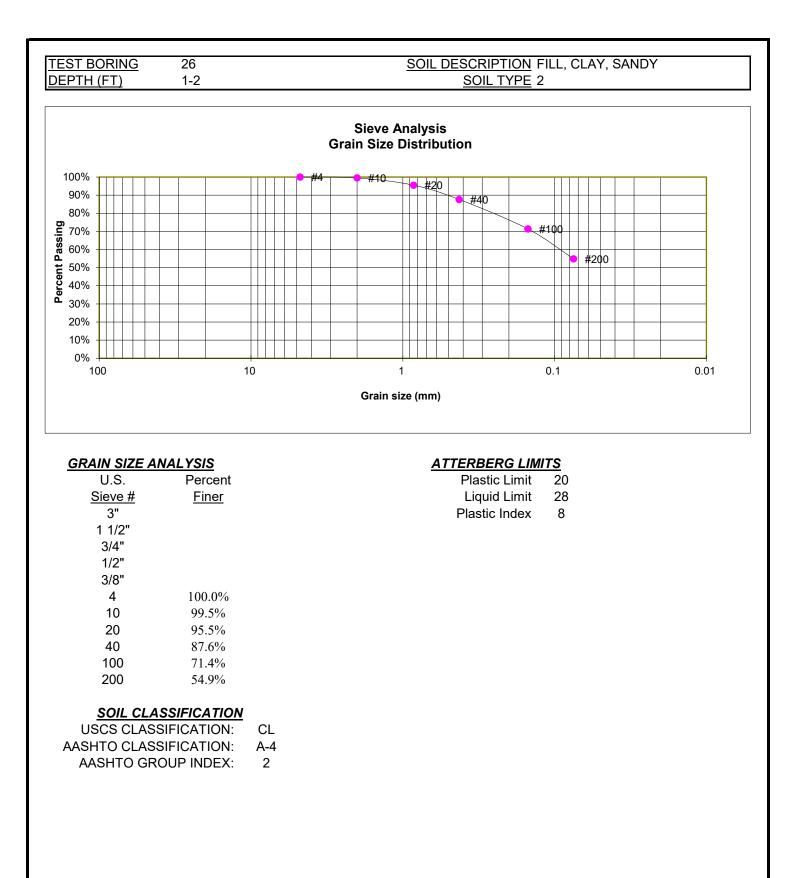


ENTECH ENGINEERING, INC.

LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

EST BORING DEPTH (FT)	12 10		RIPTION SANDSTONE (SAN I <u>L TYPE</u> 3	ND, SILTY)
		Sieve Analysis Grain Size Distribution		
100% 90% 80% 70% 60% 50% 40% 30% 20% 10% 0%		#4 #10 #20 #20	0 #100 #200	
100	10	1 Grain size (mm)	0.1	0.0

GRAIN	SIZE	ANALYSIS	
	~		

U.S.	Percent
Sieve #	Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	81.6%
10	56.6%
20	37.8%
40	28.1%
100	18.2%
200	14.3%

SUIL CLASSIFICATION	
USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-1-b
AASHTO GROUP INDEX:	0

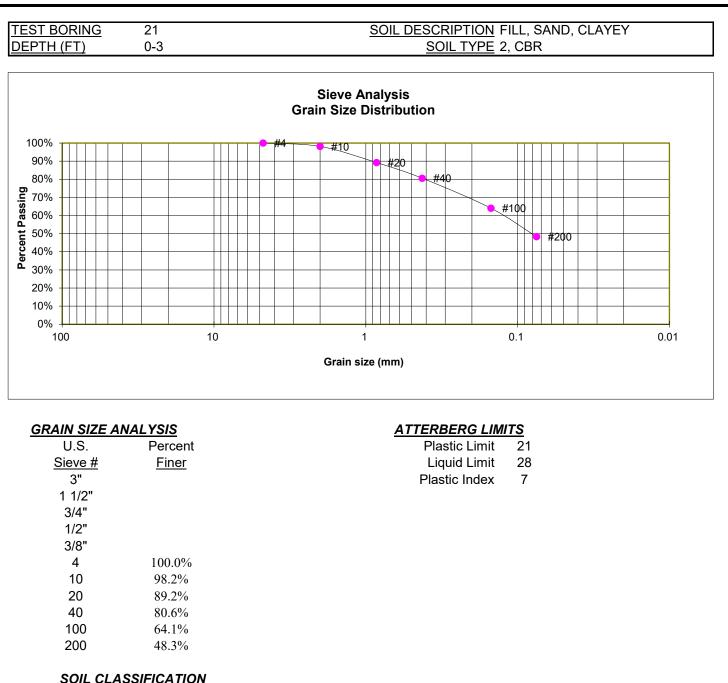
ATTERBERG LIMITS

Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-4
AASHTO GROUP INDEX:	1



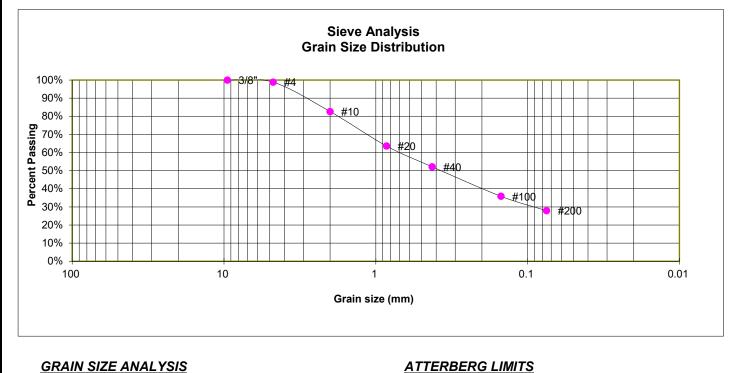
LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

TEST BORING	3
DEPTH (FT)	0-3

SOIL DESCRIPTION FILL, SAND, SILTY SOIL TYPE 1, CBR



GRAIN SIZE ANALYSIS

U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	98.8%
10	82.6%
20	63.6%
40	52.1%
100	35.9%
200	27.9%

SOIL CLASSIFICATION

USCS CLASSIFICATION:	SM
AASHTO CLASSIFICATION:	A-2-4
AASHTO GROUP INDEX:	0



LABORATORY TEST RESULTS

Plastic Limit

Liquid Limit

Plastic Index

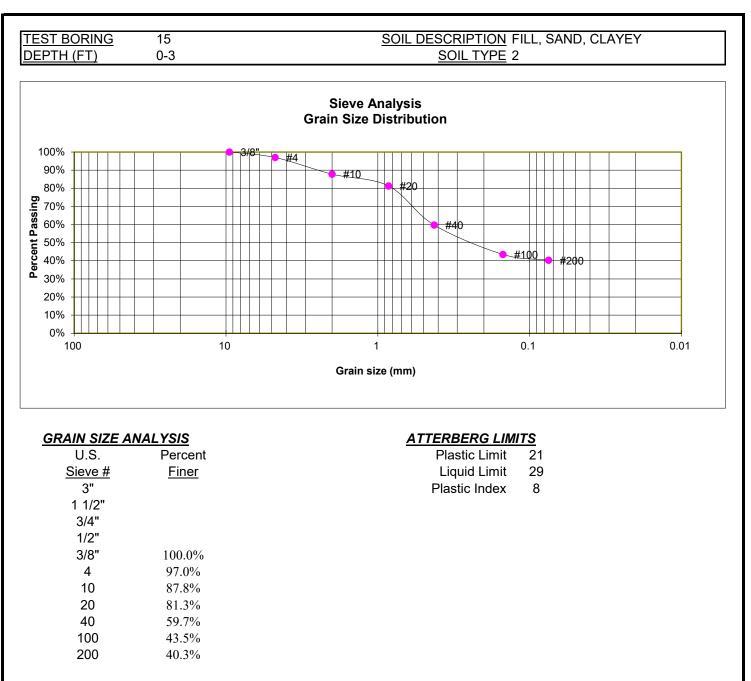
NP

NV

NP

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC



SOIL CLASSIFICATION

USCS CLASSIFICATION:	SC
AASHTO CLASSIFICATION:	A-4
AASHTO GROUP INDEX:	0



LABORATORY TEST RESULTS

JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

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			+	-	-		#	10			+			-							_		
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					3/8"		Gra	Grain S	Grain Size I	Grain Size Dis	Grain Size Distrib		Grain Size Distribution										

|--|

U.S.	Percent
Sieve #	<u>Finer</u>
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	92.5%
4	77.2%
10	53.1%
20	39.2%
40	27.0%
100	13.6%
200	10.2%

ATTERBERG LIMITS

Plastic Limit	NP
Liquid Limit	NV
Plastic Index	NP

SOIL CLASSIFICATION

USCS CLASSIFICATION: SW-SM AASHTO CLASSIFICATION: A-1-b AASHTO GROUP INDEX: 0



LABORATORY TEST RESULTS

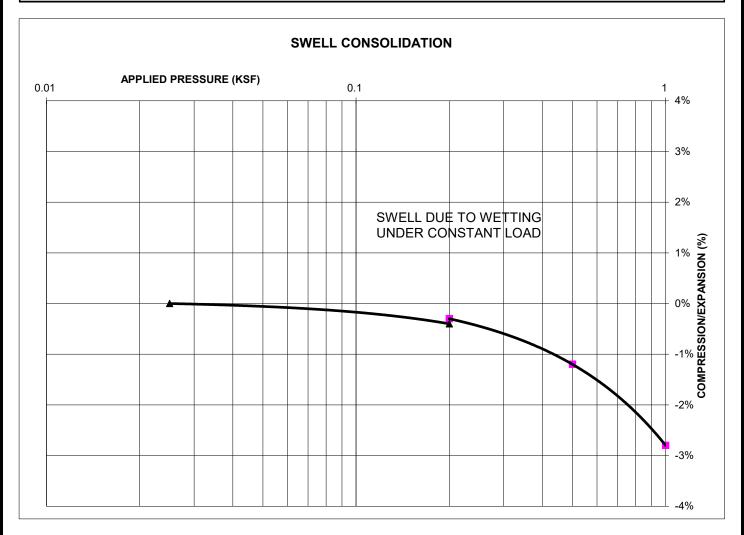
JOB NO. 231192

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC

SOIL DESCRIPTION FILL, SAND, CLAYEY SOIL TYPE 2

<u>TEST BORING</u> DEPTH (FT) 8

1-2



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF):	104
NATURAL MOISTURE CONTENT:	9.8%
SWELL/COLLAPSE (%):	0.1%



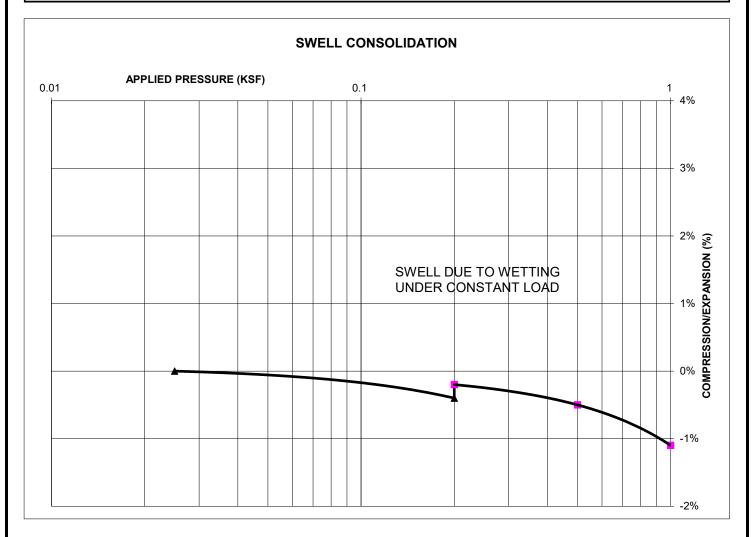
SWELL TEST RESULTS

JOB NO. 231192

TEST BORING	
DEPTH (FT)	

1-2

SOIL DESCRIPTION FILL, SAND, SILTY SOIL TYPE 2



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF):	106
NATURAL MOISTURE CONTENT:	16.6%
SWELL/COLLAPSE (%):	0.2%



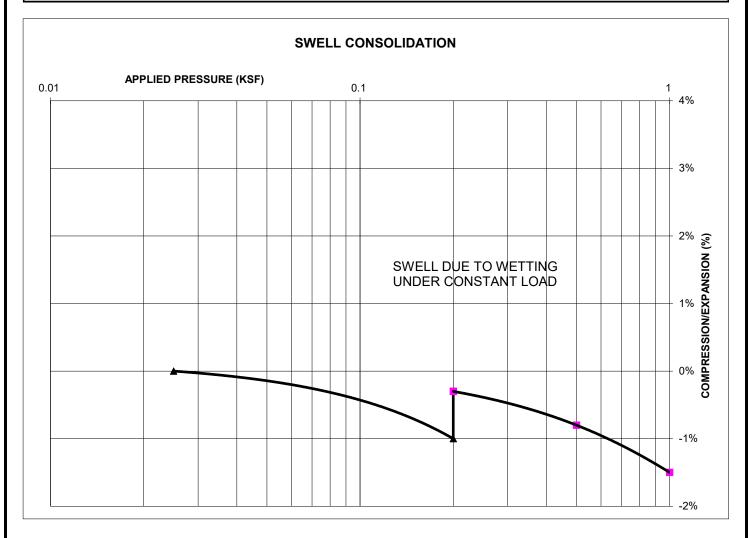
SWELL TEST RESULTS

JOB NO. 231192

TEST BORING	
DEPTH (FT)	

1-2

SOIL DESCRIPTION FILL, CLAY, SANDY SOIL TYPE 2



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF):	108
NATURAL MOISTURE CONTENT:	14.0%
SWELL/COLLAPSE (%):	0.7%



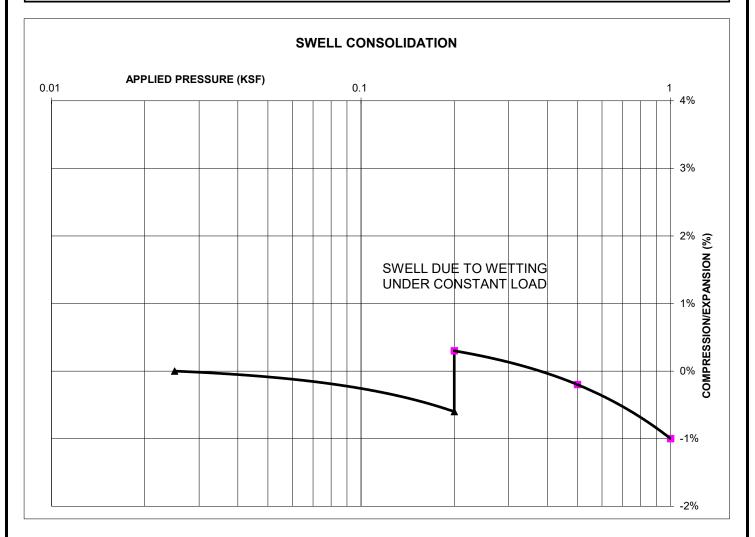
SWELL TEST RESULTS

JOB NO. 231192

TEST BORING	
DEPTH (FT)	

1-2

SOIL DESCRIPTION FILL, CLAY, SANDY SOIL TYPE 2



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF):	99
NATURAL MOISTURE CONTENT:	14.8%
SWELL/COLLAPSE (%):	0.9%



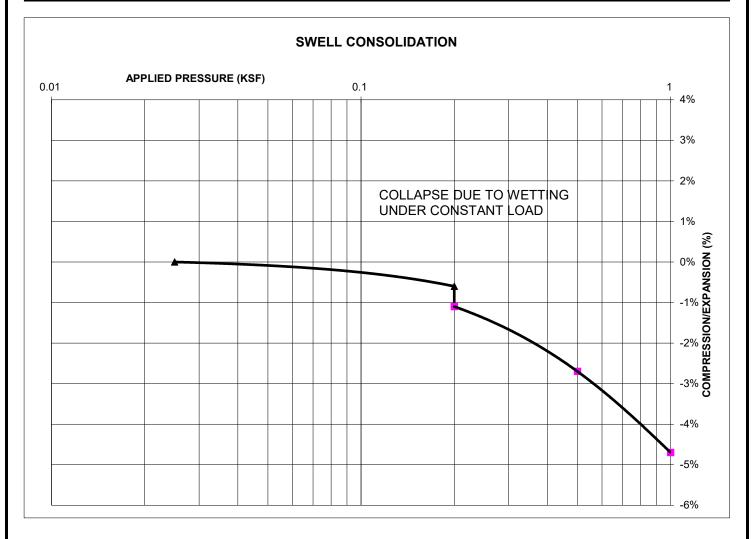
SWELL TEST RESULTS

JOB NO. 231192

<u>TEST BORING</u> DEPTH (FT) 23

1-2

SOIL DESCRIPTION FILL, SAND, CLAYEY SOIL TYPE 2



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF):	106
NATURAL MOISTURE CONTENT:	7.6%
SWELL/COLLAPSE (%):	-0.5%



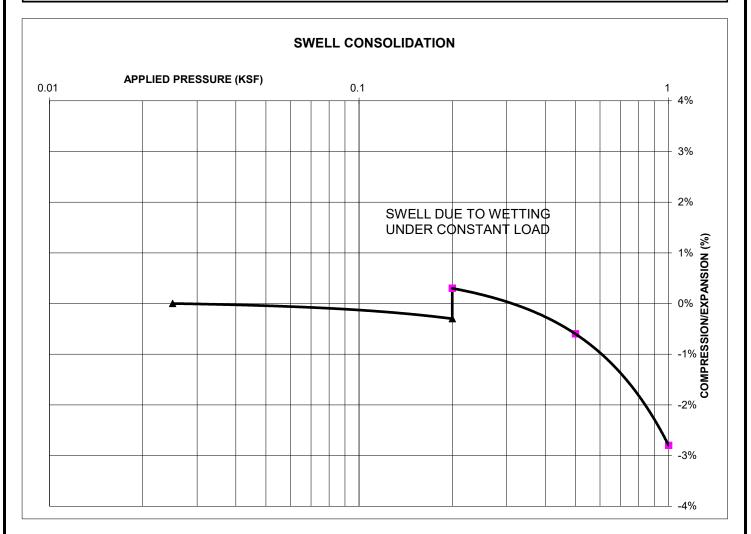
SWELL TEST RESULTS

JOB NO. 231192

TIEST BORING	
IDEPTH (FT)	

1-2

SOIL DESCRIPTION FILL, CLAY, SANDY SOIL TYPE 2



SWELL/COLLAPSE TEST RESULTS

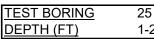
NATURAL UNIT DRY WEIGHT (PCF):	109
NATURAL MOISTURE CONTENT:	16.1%
SWELL/COLLAPSE (%):	0.6%



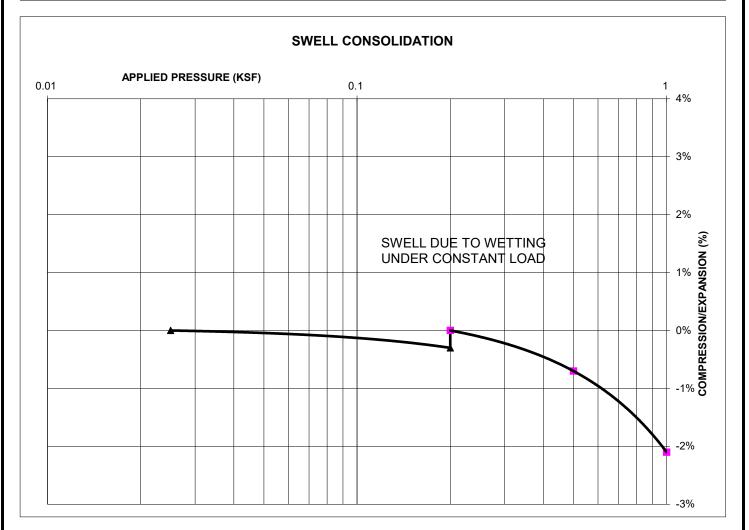
SWELL TEST RESULTS

JOB NO. 231192

SOIL DESCRIPTION FILL, CLAY, SANDY SOIL TYPE 2



1-2



SWELL/COLLAPSE TEST RESULTS

NATURAL UNIT DRY WEIGHT (PCF):	115
NATURAL MOISTURE CONTENT:	10.1%
SWELL/COLLAPSE (%):	0.3%



SWELL TEST RESULTS

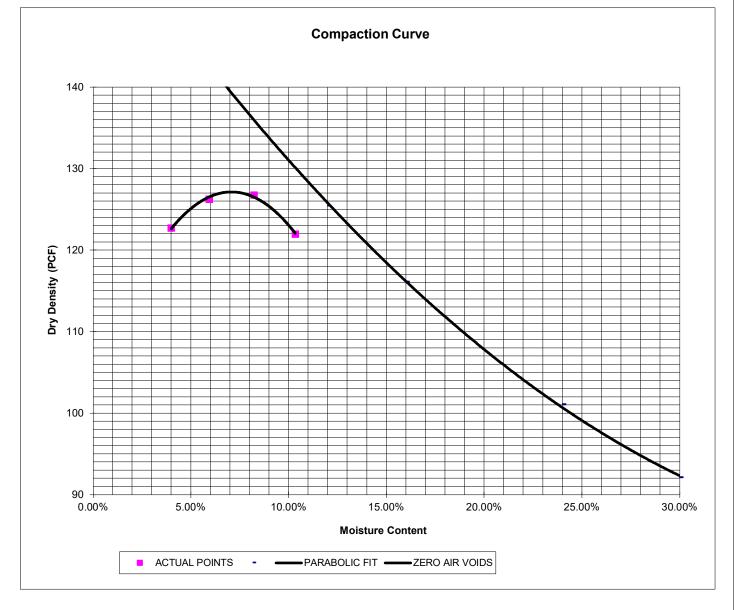
JOB NO. 231192



SOIL DESCRIPTION FILL, SAND, SILTY, BROWN SOIL TYPE 1

PROCTOR DATA

IDENTIFICATION:	SM
PROCTOR TEST #:	1
TEST BY:	PH
TEST DESIGNATION:	ASTM-1557-A
MAXIMUM DRY DENSITY (PCF):	127.1
OPTIMUM MOISTURE:	7.1





LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC JOB NO. 231192

SAMPLE LOCATION TB-3 @ 0-3'

SOIL DESCRIPTION FILL, SAND, SILTY, BROWN SOIL TYPE 1

CBR TEST LOAD DATA

Piston Diameter (cm): 4.958 Piston Area (in²): 2.993

	10 B	LOWS	25 B	LOWS	56 B	LOWS
Penetration	Мо	ld # 1	Mo	ld # 2	Mold # 3	
Depth	Load	Stress	Load	Stress	Load	Stress
(inches)	(lbs)	(psi)	(lbs)	(psi)	(lbs)	(psi)
0.000	0	0.00	0	0.00	0	0.00
0.025	117	39.10	197	65.83	263	87.89
0.050	222	74.19	278	92.90	427	142.69
0.075	271	90.56	379	126.65	540	180.45
0.100	289	96.57	508	169.76	653	218.21
0.125	322	107.60	701	234.25	823	275.02
0.150	331	110.61	804	268.67	914	305.43
0.175	347	115.96	1009	337.18	1215	406.01
0.200	374	124.98	1095	365.91	1469	490.89
0.300	427	142.69	1334	445.78	2338	781.28
0.400	480	160.40	1548	517.29	3012	1006.51
0.500	538	179.78	1737	580.45	3796	1268.50

MOISTURE AND DENSITY DATA

	Mold # 1	Mold # 2	Mold # 3
Can #	340	349	117
Wt. Can	8.67	8.93	8.6
Wt. Can+Wet	141.72	150.74	127.9
Wt. Can+Dry	124.62	133.57	112.92
Wt. H20	17.1	17.17	14.98
Wt. Dry Soil	115.95	124.64	104.32
Moisture Content	14.75%	13.78%	14.36%
Wet Density (PCF)	121.5	127.5	134.6
Dry Density (PCF)	113.5	119.0	125.6
% Compaction	89%	94%	99%
CBR	9.66	16.98	21.82

CBR at 90% of Max. Density =	10.9	~ R VALUE 30
CBR at 95% of Max. Density =	18.2	~ R VALUE 65

PROCTOR DATA

Maximum Dry Density (pcf)	127.1
Optimum Moisture	7.1
90% of Max. Dry Density (pcf)	114.4
95% of Max. Dry Density (pcf)	120.7

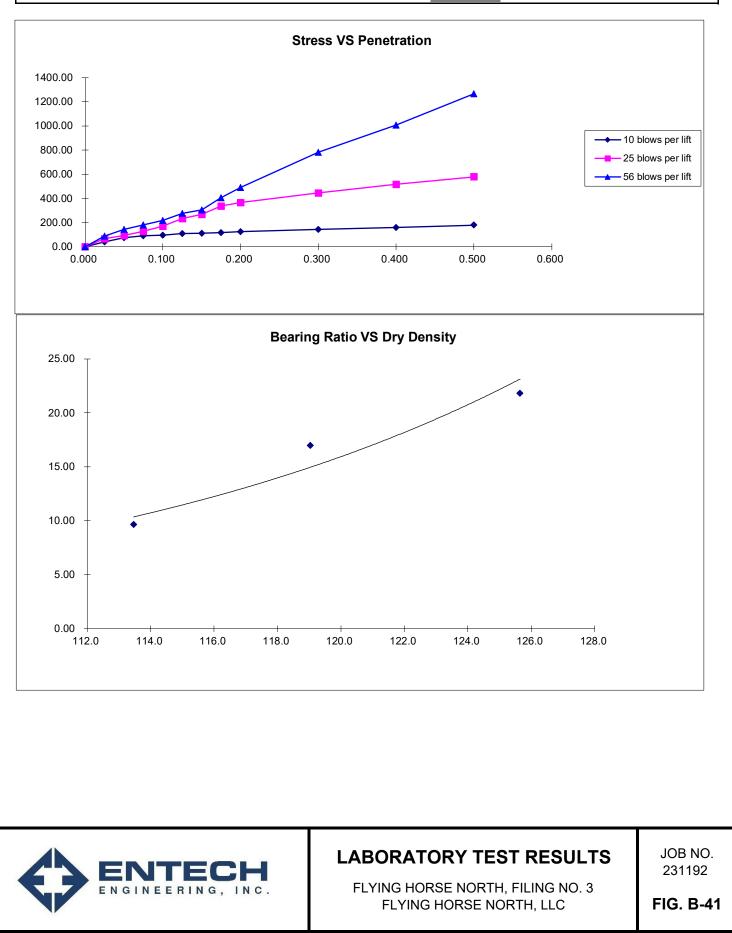


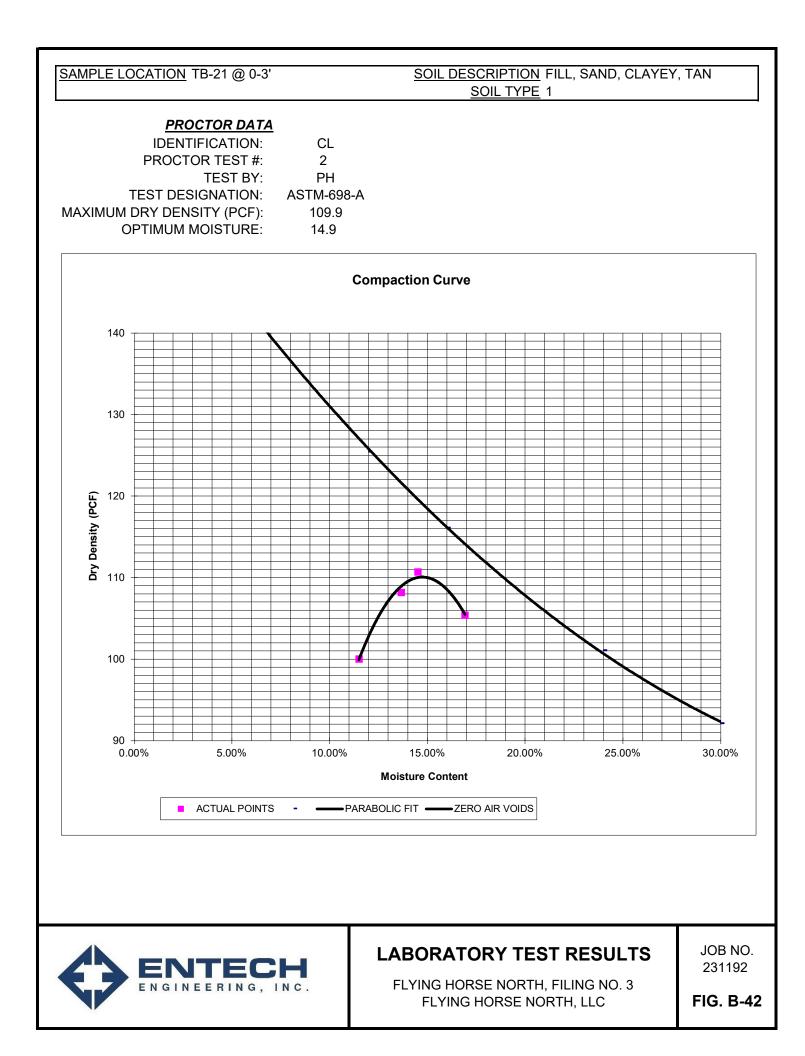
LABORATORY TEST RESULTS

FLYING HORSE NORTH, FILING NO. 3 FLYING HORSE NORTH, LLC JOB NO. 231192

SAMPLE LOCATION TB-3 @ 0-3'

SOIL DESCRIPTION FILL, SAND, SILTY, BROWN SOIL TYPE 1





SAMPLE LOCATION TB-21 @ 0-3'

SOIL DESCRIPTION FILL, SAND, CLAYEY, TAN SOIL TYPE 1

CBR TEST LOAD DATA

Piston Diameter (cm): 4.958 Piston Area (in²): 2.993

	10 BLOWS		25 BLOWS		25 BLOWS		56 BLOWS	
Penetration	Mol	d # 1	Mol	d # 2	Mold # 3			
Depth	Load	Stress	Load	Stress	Load	Stress		
(inches)	(lbs)	(psi)	(lbs)	(psi)	(lbs)	(psi)		
0.000	0	0.00	0	0.00	0	0.00		
0.025	18	6.02	97	32.41	145	48.45		
0.050	26	8.69	123	41.10	234	78.20		
0.075	28	9.36	136	45.45	267	89.22		
0.100	30	10.03	146	48.79	292	97.58		
0.125	33	11.03	154	51.46	320	106.93		
0.150	35	11.70	161	53.80	337	112.61		
0.175	38	12.70	168	56.14	347	115.96		
0.200	41	13.70	173	57.81	360	120.30		
0.300	46	15.37	186	62.16	407	136.01		
0.400	46	15.37	197	65.83	458	153.05		
0.500	48	16.04	211	70.51	510	170.43		

MOISTURE AND DENSITY DATA

	Mold # 1	Mold # 2	Mold # 3
Can #	347	354	361
Wt. Can	8.93	8.09	8.51
Wt. Can+Wet	202.45	155.01	169.18
Wt. Can+Dry	159.67	131.24	141.75
Wt. H20	42.78	23.77	27.43
Wt. Dry Soil	150.74	123.15	133.24
Moisture Content	28.38%	19.30%	20.59%
Wet Density (PCF)	112.3	117.9	124.2
Dry Density (PCF)	97.7	102.6	108.1
% Compaction	89%	93%	98%
CBR	1.00	4.88	9.76

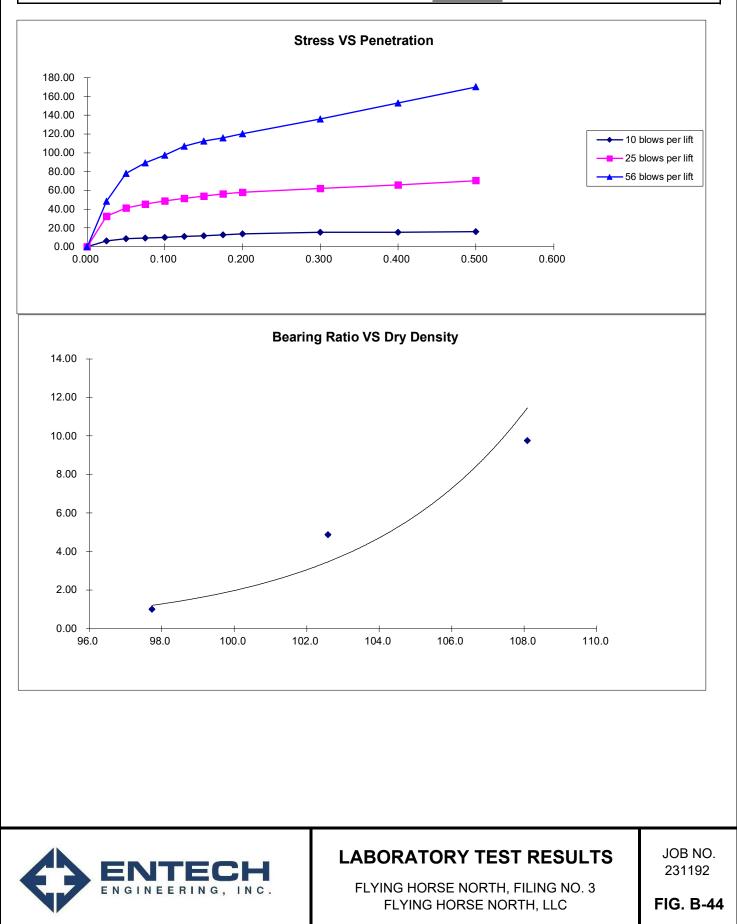
CBR at 90% of Max. Density = 1.9	~ R VALUE 1
CBR at 95% of Max. Density = 6.5	~ R VALUE 14

PROCTOR DATA

Maximum Dry Density (pcf)	109.9
Optimum Moisture	14.9
90% of Max. Dry Density (pcf)	98.9
95% of Max. Dry Density (pcf)	104.4



JOB NO. 231192





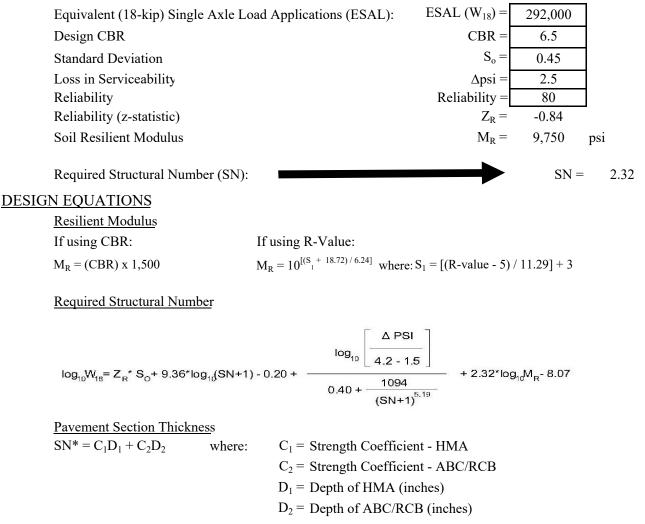
APPENDIX C: Pavement Design Calculations



PROJECT DATA

Project Location: Sterling Ranch, Filing No. 5 Job Number: 240368

DESIGN DATA



RECOMMENED THICKNESSES

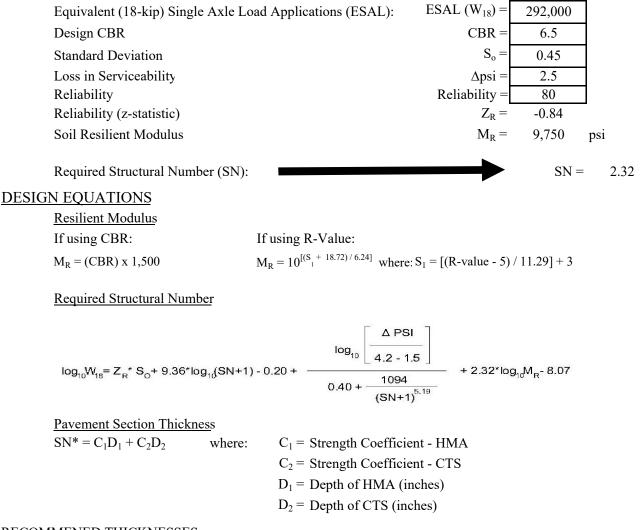
Layer	Material	Coefficient	Thickness (D*i)		Thickness (D* _i)		SN* _i	SN
1	HMA	$C_1 = 0.44$	3.5	inches	1.540			
2	ABC/RCB	$C_2 = 0.11$	8.0	inches	0.880	-		
				SN* =	2.420	2.32		



PROJECT DATA

Project Location: Sterling Ranch, Filing No. 5 Job Number: 240368

DESIGN DATA



RECOMMENED THICKNESSES

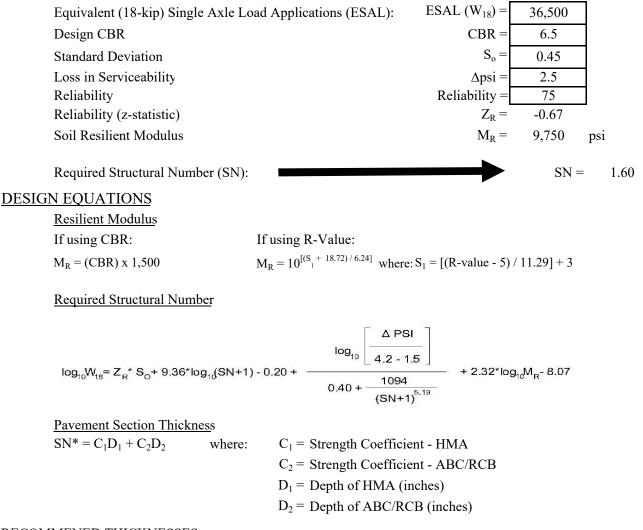
Layer	Material	Coefficient	Thickness (D*i)		Thickness (D*i)		SN* _i	SN
1	HMA	$C_1 = 0.44$	3.5	inches	1.540			
2	CTS	$C_2 = 0.11$	8.0	inches	0.880	-		
				SN* =	2.420	2.32		



PROJECT DATA

Project Location: Sterling Ranch, Filing No. 5 Job Number: 240368

DESIGN DATA



RECOMMENED THICKNESSES

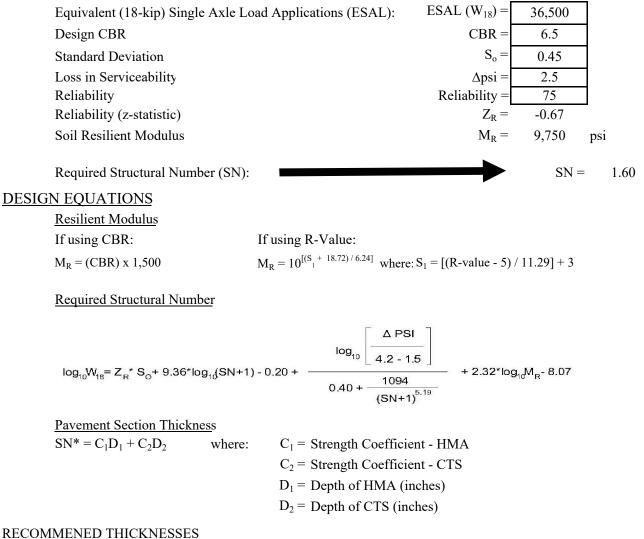
Layer	Material	Coefficient	Thickness (D*i)		Thickness (D* _i)		SN* _i	SN
1	HMA	$C_1 = 0.44$	3.0	inches	1.320			
2	ABC/RCB	$C_2 = 0.11$	4.0	inches	0.440	-		
				SN* =	1.760	1.60		



PROJECT DATA

Project Location: Sterling Ranch, Filing No. 5 Job Number: 240368

DESIGN DATA



Ommerced Thior (ESSES							
	Layer	Material	Coefficient	Thickne	ess (D*i)	SN* _i	SN
	1	HMA	$C_1 = 0.44$	3.0	inches	1.320	
	2	CTS	$C_2 = 0.11$	8.0	inches	0.880	-
					SN* =	2.200	1.60