

DUNKIN BENT GRASS

Lot 1A of Bent Grass East Commercial Filing No. 2A 8035 Meridian Park Drive, Peyton, CO 80831

FINAL DRAINAGE REPORT

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> Prepared For: CD Bent Grass, LLC 106 S Kyrene Road Chandler, AZ 85226 Contact: Bert Hayenga

> > Submitted To: El Paso County

WCC Project No.: 322002 County Project No.: PCD File # PPR-22-027

October 5, 2022



Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by El Paso County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Eric McKnight, PE, QSD Project Manager Registered Professional Engineer State of Colorado No. 55261

Developer's Statement:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Business Name

By:_____

Title:

Address: _____

El Paso County:

Filed in accordance with the requirements of the El Paso Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the Engineering Criteria Manual, as amended.

Joshua Palmer, Interim County Engineer / ECM Administrator

Date

Conditions:



Table of Contents

General Location	4
Description of Property	5
Existing Topography	5
Existing Soils	5
Existing Utilities	5
Drainage Design Criteria	5
Major Drainageways & Master Drainage Plans	5
Floodplain Statement	5
Drainage Facility Design	6
General Concept	
Offsite Flow Patterns	
Historic Drainage Patterns	6
Proposed Drainage Patterns	7
Detention Facility Capacity Analysis	7
Drainage and Bridge Fees	
Bridge Fees	Error! Bookmark not defined.
Bridge Fees Drainage Fees	
	Error! Bookmark not defined.
Drainage Fees	Error! Bookmark not defined.
Drainage Fees	Error! Bookmark not defined.
Drainage Fees Summary Appendix A	Error! Bookmark not defined.
Drainage Fees Summary Appendix A Vicinity Map	Error! Bookmark not defined. 10 11 11 13
Drainage Fees Summary Appendix A Vicinity Map Appendix B	
Drainage Fees Summary Appendix A Vicinity Map Appendix B Soils Map (NRCS Soils Study)	Error! Bookmark not defined. 10 11 11 13 14
Drainage Fees Summary Appendix A Vicinity Map Appendix B Soils Map (NRCS Soils Study) Appendix C	Error! Bookmark not defined. 10 11 11 13 13 14
Drainage Fees Summary Appendix A Vicinity Map Appendix B Soils Map (NRCS Soils Study) Appendix C FEMA Flood Insurance Rate Map	Error! Bookmark not defined. 10 11 11 13 13 14 14 15
Drainage Fees Summary Appendix A Vicinity Map Appendix B Soils Map (NRCS Soils Study) Appendix C FEMA Flood Insurance Rate Map Appendix D	Error! Bookmark not defined. 10 11 11 13 13 14 14 15
Drainage Fees Summary Appendix A Vicinity Map Appendix B Soils Map (NRCS Soils Study) Appendix C FEMA Flood Insurance Rate Map Drainage Maps	Error! Bookmark not defined. 10 11 11 13 13 14 14 15 15 16
Drainage Fees Summary Appendix A Vicinity Map Appendix B Soils Map (NRCS Soils Study) Appendix C FEMA Flood Insurance Rate Map Drainage Maps Appendix E	Error! Bookmark not defined. 10 11 11 13 13 14 14 15 15 16



General Location



The Dunkin Bent Grass project (Site) is located at 8035 Meridian Park Drive in Peyton, CO. The Site is located in a parcel of land situated in the Northeast Quarter of the Section 1, Township 13 South, Range 65 West of the Sixth Principal Meridian in El Paso County, Colorado. The site is bound on the north by Lot 1 of the Bent Grass East Commercial Development (a 7-Eleven gas station), to the west by Meridian Park Road right-of-way, to the east by Meridian Road right-of-way, and to the south by Lot 3 of the Bent Grass East Commercial Development (a dental office).



Description of Property

The Dunkin Bent Grass site, which will consist of a new Dunkin restaurant, parking lot, drive thru-aisle, and associated walks and landscaped areas, is comprised of 1.46 acres in area.

Existing Topography

The site is currently undeveloped and covered with natural grasses. The site generally slopes from north to south, with an existing drainage ditch that runs along the eastern edge of the property. Existing slopes on the majority site range from 1% to 3%, while the eastern edge that slopes down to the ditch slopes up to 25%.

Existing Soils

Existing soils on the site are 100% gravelly sandy loam, designated as NRCS hydrologic group A soils. Additional soils information has been provided in **Appendix B** of this report.

Existing Utilities

There are existing water and sanitary sewer stubouts for use on site, as well as existing electrical and communications equipment located at the southwest and southeast corners of the site. There is a public utility and drainage easement (width varies between 10 and 20 feet) that encircles the entirety of the site. There are no existing irrigation facilities present on site.

Drainage Design Criteria

All drainage calculations were performed in accordance with the El Paso County Drainage Criteria Manual (updated October 31, 2018, referred to as the DCM). Per Section 5.1 of the El Paso County DCM, the 5-Year and 100-Year storm frequencies were selected for analysis. Additionally, because the site is under 100 acres in area, the Rational Method has been selected as the runoff methodology for this analysis. The Mile High Flood District (MHFD) Drainage Criteria Manual was also consulted for additional hydrologic methodology further outlined in this report.

Major Drainageways & Master Drainage Plans

The site lies within the Middle Tributary Basin within the Falcon Drainage Basin. This site has been previously studied as part of the "Master Development Drainage Plan and Preliminary Drainage Plan for the Bent Grass Subdivision," prepared by Kiowa Engineering Corporation, approved in September 2007. More recently, the site was analyzed as part of "Preliminary Drainage Report for Bent Grass East Commercial – Phase 1 (Preliminary Plan) and Final Drainage Report for Bent Grass East Commercial Filing No. 1 – Lot 1 (Final Plat)," prepared by Classic Consulting Engineers & Surveyors, approved March 15, 2013 (referred to as the Phase 1 PDR), as well as "Final Drainage Report, Bent Grass Commercial Filing No. 2" dated July 2014 (refer to PCD File No. #SF1411).

Floodplain Statement

The Flood Insurance Rate Maps (FIRM) for El Paso County Flood Insurance Study (FIS) panel number 08041C0553G dated December 7, 2018 was reviewed to determine if any regulatory floodplains pass through the property. No portion of this proposed development is within a floodplain. A copy of the FIRM Map for this site has been included in **Appendix C** of this report.



Drainage Facility Design

General Concept

The proposed Dunkin Bent Grass site will consist of a new Dunkin restaurant, parking lot, drive thru-aisle, and associated walks and landscaped areas. All runoff is proposed to leave the site via surface flows (e.g.: sheet flow, curb and gutter), and no inlets or associated piping are proposed as part of the design. Existing drainage patterns (i.e.: some of the flow will make its way into the existing detention facility to the southwest of the site, while the remainder of the flow will travel undetained into the existing channel to the east of the site) will be maintained with this development. Detention and water quality is not proposed as part of this development, as a detention facility exists to the southwest of the site, which was constructed to serve several lots in the Bent Grass Development (including residential subdivisions and other commercial properties) in their developed conditions.

Offsite Flow Patterns

No offsite flows are incorporated into the analysis of the development. Despite the existing flow patterns (flowing north to south), virtually no flows from the site to the north will make their way onto the Dunkin Bent Grass site due to the use of curb and gutter on the south end of the 7-Eleven development, which carries flow toward either the Meridian Park Drive flowline, or the existing channel to the east of the site.

Historic Drainage Patterns

The site generally slopes from north to south, with an existing drainage ditch that runs along the eastern edge of the property. Existing slopes on the majority site range from 1% to 3%, while the eastern edge that slopes down to the ditch slopes up to 25%. There is a ridgeline that effectively bisects the site, taking some of the flows to the east and into the existing channel, while the majority of runoff will flow onto the property to the south, and eventually into the existing detention facility (which also provides stormwater quality treatment).

The site, in its existing condition, has been divided into sub-basins and design points as described below:

- Basin EX1 (Q₅ = 0.04 cfs, Q₁₀₀ = 0.90 cfs) represents the existing flows for the western portion of the site. Sheet flows travel in a southwesterly direction, eventually discharging into the Meridian Park Drive flowline and eventually into the existing detention facility. **Design Point 1 (Q₁₀ = 0.04 cfs, Q₁₀₀ = 0.90 cfs)** represents the concentration of these flows from Basin EX1.
- Basin EX2 (Q₅ = 0.04 cfs, Q₁₀₀ = 0.81 cfs) represents the existing flows for the eastern portion of the site. Sheet flows travel in a southeasterly direction before eventually discharging into the existing channel along Meridian Road. **Design Point 2 (Q₁₀ = 0.04 cfs, Q₁₀₀ = 0.81 cfs)** represents the concentration of these flows from Basin EX2.

Basin	Total Area (sf)	% Impervious	C 5	C ₁₀₀	Q ₅ (cfs)	Q ₁₀₀ (cfs)
EX1	33,381	0.0%	0.01	0.13	0.04	0.90
EX2	30,085	0.0%	0.01	0.13	0.04	0.81
Total/Overall	63,480	0.0%	0.01	0.13	0.07	1.72

A summary of the existing flows can be found in the table below:

An Existing Drainage Plan (**Appendix D**) and runoff calculations (**Appendix E**) have been included with this report to better illustrate the pre-development hydrologic conditions.



Proposed Drainage Patterns

The proposed development aims to maintain the existing drainage patterns of the site, in that some of the flow will make its way into the existing detention facility to the southwest of the site, while the remainder of the flow will travel undetained into the existing channel to the east of the site.

The site, in its proposed condition, has been divided into sub-basins and design points as described below:

- Basin A1 (Q₅ = 1.51 cfs, Q₁₀₀ = 4.44 cfs) represents the developed flows for the western portion of the site. Sheet flows travel in a southwesterly direction, eventually discharging into the Meridian Park Drive flowline and eventually into the existing detention facility. **Design Point 1 (Q**₅ = 1.51 cfs, Q₁₀₀ = 4.44 cfs) represents the concentration of these flows from Basin A1.
- Basin U1 ($Q_5 = 0.01$ cfs, $Q_{100} = 0.20$ cfs) represents the developed flows for the eastern portion of the site. Sheet flows travel in a southeasterly direction before eventually discharging into the existing channel along Meridian Road. **Design Point 2 (Q_5 = 0.01 cfs, Q_{100} = 0.20 cfs)** represents the concentration of these flows from Basin U2.

Basin	Total Area (sf)	% Impervious	C 5	C100	Q5 (cfs)	Q100 (cfs)
A1	56,111	31.3%	0.23	0.38	1.51	4.44
U1	7,369	0.0%	0.01	0.13	0.01	0.20
Total/Overall	63,480	31.3%	0.20	0.35	1.52	4.64

A summary of the proposed flows can be found in the table below:

A Proposed Drainage Plan (**Appendix D**) and runoff calculations (**Appendix E**) have been included with this report to better illustrate the post-development hydrologic conditions.

A summary of the development's disturbed area and increase in overall runoff can be found in the table below:

	Total Area (sf)	% Impervious	Q10 (cfs)	Q100 (cfs)
Existing	63,480	0.0%	0.07	1.72
Proposed	63,480	31.3%	1.52	4.64
Change	-	+31.3%	+1.45	+2.92

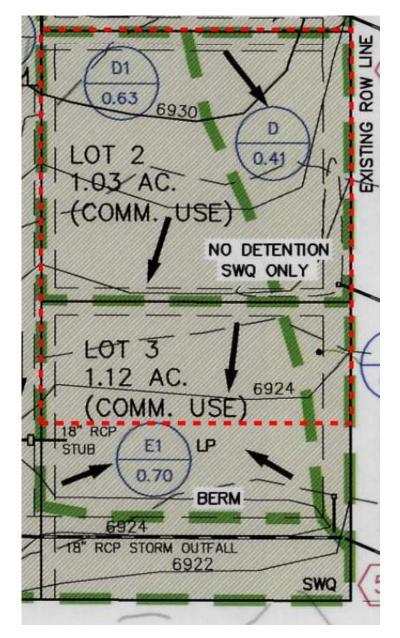
With this increase in runoff and impervious area, detention and water quality treatment are required to mitigate these impacts.

Detention Facility Capacity Analysis

As mentioned earlier in this report, the storage and water quality treatment have been provided for the site (and several others in the Bent Grass development) in their developed conditions in the existing detention facility located to the southwest of the site. As outlined in the Phase 1 PDR by Classic Consulting, the detention facility collects tributary flows from the surrounding Bent Grass sites before storing, treating, and eventually discharging them via controlled release into the existing channel to the east of the site. The existing detention facility appears to be in good condition and is functioning as intended.



More specifically, the Phase 1 PDR shows drainage basins drawn and sized to correspond to the existing lots at the time. As part of the Bent Grass East Commercial development, the drainage divides and hydrologic calculations were drawn prior to a lot line shift, which increased the size of the Dunkin Bent Grass lot. In the figure below, two lots in the Bent Grass East Commercial Development are shown prior to this shift. The southern lot line for the parcel described as "Lot 2" was shifted approximately 90 feet south, creating the newly re-platted Lot 1A (project, boundary approximately shown below in a dashed red line) and Lot 2A to the south (now an existing dental clinic office with parking lot).



A complete Developed Drainage Map from the Phase 1 PDR has been included with **Appendix F** of this report to better illustrate the hydrologic conditions of the design.



In the Phase 1 PDR, Lot 2's flows were quantified in terms of two conditions: the runoff eventually leading to the existing detention facility for storage and treatment **(Basin D1)**, and the flows leaving the site undetained and entering the existing channel to the east of the site **(Basin D)**. These runoff patterns are consistent with the analysis performed on the existing and proposed layouts of the site outlined earlier in this report, though it should be noted with updated topography and survey information, as well as the increased size of the parcel in question, the basin geometry has changed. Further analysis has been performed below to better compare the conceptual design proposed in the Phase 1 PDR and the existing and proposed drainage conditions as analyzed earlier in this report.

Lot 2 was previously designed as a 1.03-acre parcel with 95% imperviousness in a built-out condition. A summary of the conceptual flows per the Phase 1 PDR can be found in the table below:

Basin	Total Area (sf)	% Impervious	C 5	C ₁₀₀	Q ₅ (cfs)	Q ₁₀₀ (cfs)
D1	27,443	95.0%	0.54	0.86	1.74	4.90
D	17,860	95.0%	0.54	0.86	1.13	3.19
Total/Overall	45,303	95.0%	0.54	0.86	2.86	8.08

To more consistently compare the existing drainage analysis in the Phase 1 PDR with the proposed design, the flows were recalculated with runoff coefficients per Table 6-5 in the MHFD Drainage Criteria Manual, Volume 1. A summary of these recalculated conceptual flows can be found in the table below:

Basin	Total Area (sf)	% Impervious	C 5	C100	Q ₅ (cfs)	Q100 (cfs)
D1	27,443	95.0%	0.81	0.89	2.60	5.09
D	17,860	95.0%	0.81	0.89	1.69	3.31
Total/Overall	45,303	95.0%	0.81	0.89	4.30	8.40

With the Phase 1 PDR's **Basin D1** being the only area planned to discharge into the existing detention basin to the southwest of the site, even with the lot line shift making this a conservative estimate, the flow values can be compared to **Basin A1** of the proposed design.

Basin	Total Area (sf)	% Impervious	C 5	C ₁₀₀	Q ₅ (cfs)	Q ₁₀₀ (cfs)
D1	27,443	95.0%	0.81	0.89	2.60	5.09
A1	56,111	31.3%	0.23	0.38	1.51	4.44
Change	+28,668	(-63.7%)	(-0.58)	(-0.51)	(-1.09)	(-0.65)

The overall flow being routed to the detention facility for the proposed development (Basin A1) is less than the planned value for the site in the Phase 1 PDR (Basin D1), despite the area for Basin A1 being much larger after the lot line shift. If flows for future development on the lot exceed the planned values, on-site detention will be provided at that time.



Drainage and Bridge Fees

As part of a Site Development Plan, no fees are due with this submittal. The bridge fees have been previously paid, and the drainage basin fees were credited at the time of the plat (per BoCC Approval for Filing No. 2). Refer to PCD File No. #SF1411, Plat No. #13515.

Summary

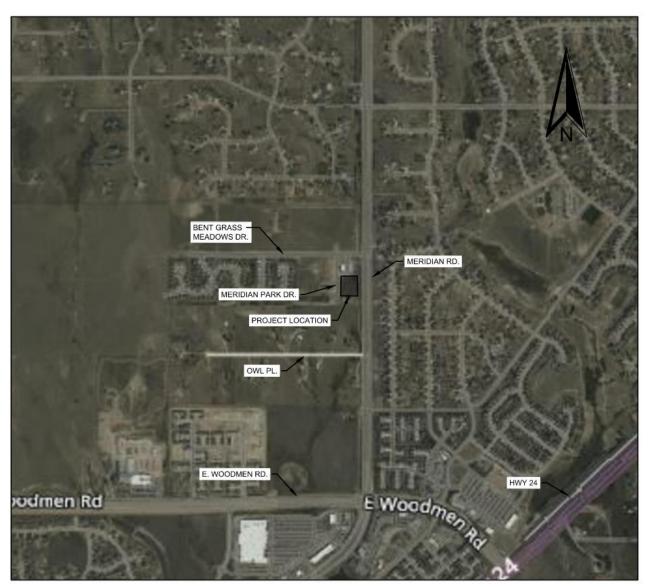
The Dunkin Bent Grass development will create a new drive-thru restaurant with associated parking lot and drive aisles, but only develops the southern portion of the site, leaving the northern portion undeveloped. Developed flows for the Dunkin Bent Grass development will not negatively impact downstream facilities. No on-site detention or water quality treatment is proposed as the existing detention facility to the southwest of the site has been sized to accept flows from future development, and the flows created by this development do not exceed planned flows in previous studies conducted on the Bent Grass East Commercial Development. In the event future development on the northern portion of the Dunkin Bent Grass site causes runoff routed to the detention facility to exceed capacity, on-site detention and water quality treatment will be provided at that point.

All erosion control measures will be handed on-site to minimize any downstream impacts on existing facilities. All drainage calculations were performed sing the current El Paso County Drainage Criteria Manual and will safely discharge stormwater runoff to existing facilities.



Appendix A Vicinity Map





Appendix A – Vicinity Map



Appendix B Soils Map (NRCS Soils Study)



Natural Resources Conservation Service Web Soil Survey National Cooperative Soil Survey

	MAP L	EGEND		MAP INFORMATION
Area of Inte Soils Colls Special F Colls Special F		GEND	Spoil Area Stony Spot Very Stony Spot Wet Spot Other Special Line Features atures Streams and Canals tation Rails	The soil surveys that comprise your AOI were mapped at 1:24,000. Warning: Soil Map may not be valid at this scale. Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale. Please rely on the bar scale on each map sheet for map measurements. Source of Map: Natural Resources Conservation Service Web Soil Survey URL:
◇☆☆☆◇●◇+☆☆☆	Gravel Pit Gravelly Spot Landfill Lava Flow Marsh or swamp Mine or Quarry Miscellaneous Water Perennial Water Rock Outcrop Saline Spot Sandy Spot Severely Eroded Spot Sinkhole Slide or Slip Sodic Spot	Backgrout	Interstate Highways US Routes Major Roads Local Roads Ind Aerial Photography	 Coordinate System: Web Mercator (EPSG:3857) Maps from the Web Soil Survey are based on the Web Mercato projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data a of the version date(s) listed below. Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 19, Aug 31, 2021 Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. Date(s) aerial images were photographed: Sep 11, 2018—Oct 20, 2018 The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	54.9	100.0%
Totals for Area of Interest		54.9	100.0%



El Paso County Area, Colorado

19—Columbine gravelly sandy loam, 0 to 3 percent slopes

Map Unit Setting

National map unit symbol: 367p Elevation: 6,500 to 7,300 feet Mean annual precipitation: 14 to 16 inches Mean annual air temperature: 46 to 50 degrees F Frost-free period: 125 to 145 days Farmland classification: Not prime farmland

Map Unit Composition

Columbine and similar soils: 97 percent Minor components: 3 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Columbine

Setting

Landform: Flood plains, fan terraces, fans Down-slope shape: Linear Across-slope shape: Linear Parent material: Alluvium

Typical profile

A - 0 to 14 inches: gravelly sandy loam C - 14 to 60 inches: very gravelly loamy sand

Properties and qualities

Slope: 0 to 3 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Very low
Capacity of the most limiting layer to transmit water (Ksat): High to very high (5.95 to 19.98 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Very low (about 2.5 inches)

Interpretive groups

Land capability classification (irrigated): 4e Land capability classification (nonirrigated): 6e Hydrologic Soil Group: A Ecological site: R049XY214CO - Gravelly Foothill Hydric soil rating: No

Minor Components

Fluvaquentic haplaquolls

Percent of map unit: 1 percent

USDA

Landform: Swales Hydric soil rating: Yes

Other soils

Percent of map unit: 1 percent Hydric soil rating: No

Pleasant

Percent of map unit: 1 percent Landform: Depressions Hydric soil rating: Yes

Data Source Information

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 19, Aug 31, 2021





Appendix C FEMA Flood Insurance Rate Map

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where Base Flood Elevations (BFEs and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The horizontal datum was NAD83, GRS80 spheroid Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988 (NAVD88). These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website a http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12

National Geodetic Survey SSMC-3, #9202 1315 East-West Highway

Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base Map information shown on this FIRM was provided in digital format by EI Paso County, Colorado Springs Utilities, City of Fountain, Bureau of Land Management National Oceanic and Atmospheric Administration, United States Geological Survey, and Anderson Consulting Engineers, Inc. These data are current as of 2006.

This map reflects more detailed and up-to-date stream channel configurations and floodplain delineations than those shown on the previous FIRM for this jurisdiction The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Stud Report (which contains authoritative hydraulic data) may reflect stream channe distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates fo each community as well as a listing of the panels on which each community is located.

Contact FEMA Map Service Center (MSC) via the FEMA Map Information eXchange (FMIX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website http://www.msc.fema.gov/.

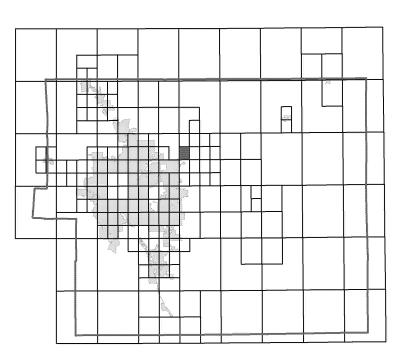
If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip.

> El Paso County Vertical Datum Offset Table **Vertical Datum**

Flooding Source

Offset (ft) REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION

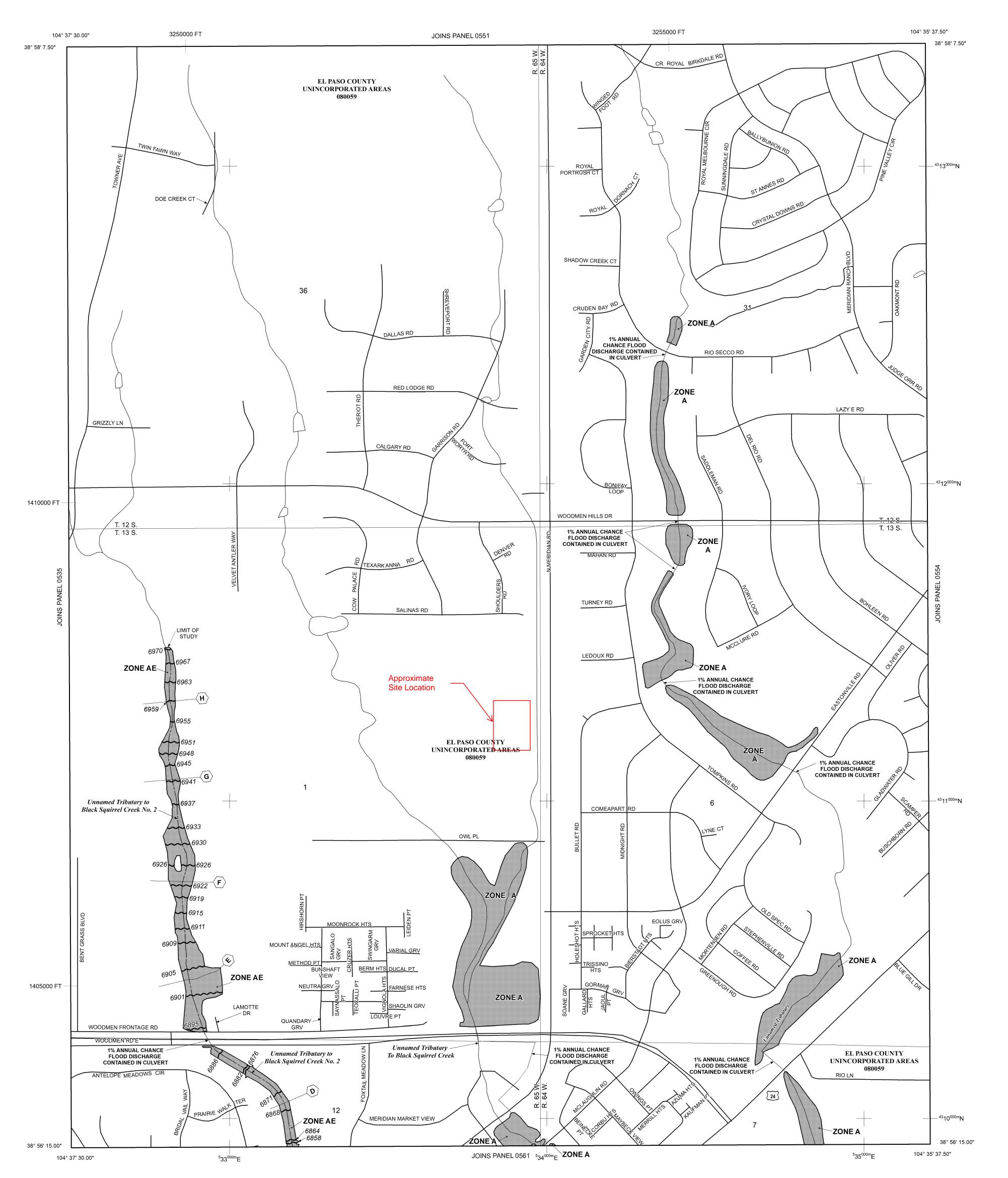
Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).



Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



	LEGEND FLOOD HAZARD AREAS (SFHAS) SUBJECT TO
	ON BY THE 1% ANNUAL CHANCE FLOOD
that has a 1% chance of be Hazard Area is the area sul	d (100-year flood), also known as the base flood, is the flood ing equaled or exceeded in any given year. The Special Flood oject to flooding by the 1% annual chance flood. Areas of
Elevation is the water-surfac	e Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood e elevation of the 1% annual chance flood.
ZONE AE Base Flood El	d Elevations determined. evations determined.
Elevations de	s of 1 to 3 feet (usually areas of ponding); Base Flood termined. of 1 to 3 feet (usually sheet flow on sloping terrain); average
depths deter determined.	mined. For areas of alluvial fan flooding, velocities also
flood by a flo	Hazard Area Formerly protected from the 1% annual chance bod control system that was subsequently decertified. Zone that the former flood control system is being restored to
<i>,</i> ,	ction from the 1% annual chance or greater flood. protected from 1% annual chance flood by a Federal flood
determined.	ystem under construction; no Base Flood Elevations
Elevations de	zone with velocity hazard (wave action); no Base Flood termined. d zone with velocity hazard (wave action); Base Flood
Elevations de	
The floodway is the channel	of a stream plus any adjacent floodplain areas that must be
substantial increases in flood	so that the 1% annual chance flood can be carried without I heights.
	DOD AREAS
average dept	6 annual chance flood; areas of 1% annual chance flood with ths of less than 1 foot or with drainage areas less than 1 and areas protected by levees from 1% annual chance flood.
OTHER AR	EAS
	ined to be outside the 0.2% annual chance floodplain. h flood hazards are undetermined, but possible.
	BARRIER RESOURCES SYSTEM (CBRS) AREAS
	SE PROTECTED AREAS (OPAs)
	rmally located within or adjacent to Special Flood Hazard Areas.
	Floodplain boundary Floodway boundary
:	Zone D Boundary
•	CBRS and OPA boundary Boundary dividing Special Flood Hazard Areas of different Base
	Flood Elevations, flood depths or flood velocities. Base Flood Elevation line and value; elevation in feet*
	Base Flood Elevation value where uniform within zone; elevation in feet*
	merican Vertical Datum of 1988 (NAVD 88)
	Cross section line
	Transect line Geographic coordinates referenced to the North American
32° 22' 30.00"	Datum of 1983 (NAD 83) 1000-meter Universal Transverse Mercator grid ticks,
	zone 13
	5000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0502), Lambert Conformal Conic Projection
	Bench mark (see explanation in Notes to Users section of this FIRM panel)
● M1.5	River Mile
	MAP REPOSITORIES
	efer to Map Repositories list on Map Index
FFF67	FLOOD INSURANCE RATE MAP MARCH 17, 1997
DECEMBER 7, 2018 - te Special Flood Hazard Are	IVE DATE(S) OF REVISION(S) TO THIS PANEL o update corporate limits, to change Base Flood Elevations and eas, to update map format, to add roads and road names, and to
	rate previously issued Letters of Map Revision. history prior to countywide mapping, refer to the Community
Map History Table located in	the Flood Insurance Study report for this jurisdiction.
	ance is available in this community, contact your insurance ood Insurance Program at 1-800-638-6620.
250	MAP SCALE 1" = 500' 500 1000
150	0 150 300
	PANEL 0553G
	FIRM
	FLOOD INSURANCE RATE MAP
	EL PASO COUNTY,
	COLORADO
	AND INCORPORATED AREAS
	PANEL 553 OF 1300
	(SEE MAP INDEX FOR FIRM PANEL LAYOUT)
	<u>CONTAINS:</u> <u>COMMUNITY</u> <u>NUMBER</u> <u>PANEL</u> <u>SUFFIX</u>
	EL PASO COUNTY 080059 0553 G
	Notice: This map was reissued on 05/15/2020 to make a correction. This version replaces any previous versions. See the Notice-to-User Letter that accompanied
	this correction for details.
	Notice to User: The Map Number shown below should be used when placing map orders: the Community Number shown above should be used on insurance applications for the
	shown above should be used on insurance applications for the subject community.
	08041C0553G
	MAP REVISED DECEMBER 7, 2018
	Federal Emergency Management Agency

National Flood Hazard Layer FIRMette



Legend

104°36'49"W 38°57'9"N SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT Without Base Flood Elevation (BFE) Zone A. V. A9 With BFE or Depth Zone AE, AO, AH, VE, AR SPECIAL FLOOD HAZARD AREAS **Regulatory Floodway** 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile Zone X Future Conditions 1% Annual Chance Flood Hazard Zone X Area with Reduced Flood Risk due to Levee. See Notes. Zone X OTHER AREAS OF FLOOD HAZARD Area with Flood Risk due to Levee Zone D NO SCREEN Area of Minimal Flood Hazard Zone X Effective LOMRs OTHER AREAS Area of Undetermined Flood Hazard Zone D - — – – Channel, Culvert, or Storm Sewer GENERAL STRUCTURES LIIII Levee, Dike, or Floodwall 20.2 Cross Sections with 1% Annual Chance AREA OF MINIMAL FLOOD HAZARD 17.5 Water Surface Elevation EL PASO COUNTY **Coastal Transect** Mase Flood Elevation Line (BFE) 080059 Limit of Study Jurisdiction Boundary **Coastal Transect Baseline** OTHER **Profile Baseline** 08041C0553G FEATURES Hydrographic Feature eff. 12/7/2018 **Digital Data Available** No Digital Data Available MAP PANELS Unmapped The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location. This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 3/10/2022 at 5:03 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time. Zone A This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for 104°36'12"W 38°56'42"N Feet

1:6.000

2.000

250

500

1,000

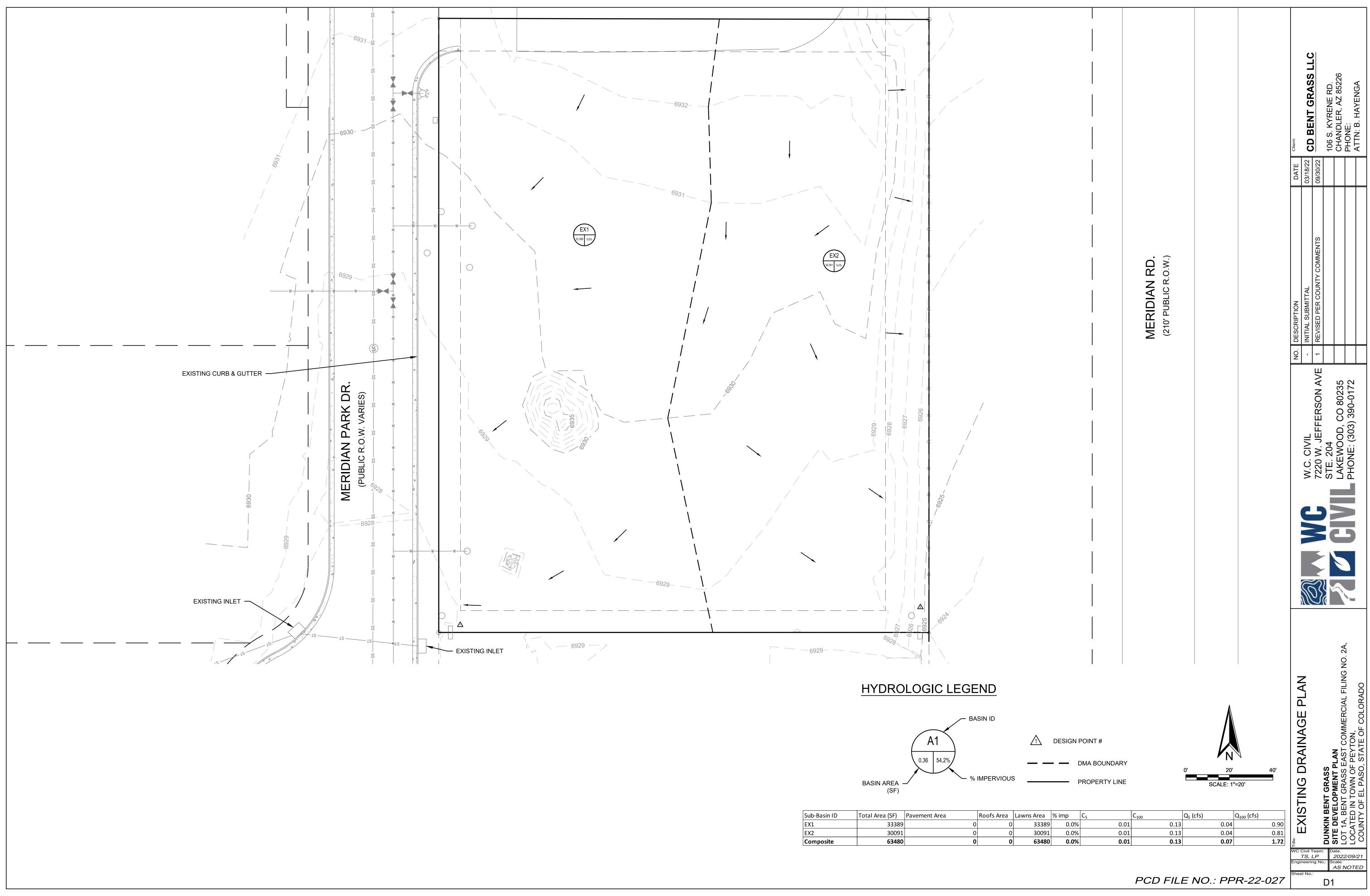
1,500

Basemap: USGS National Map: Orthoimagery: Data refreshed October, 2020

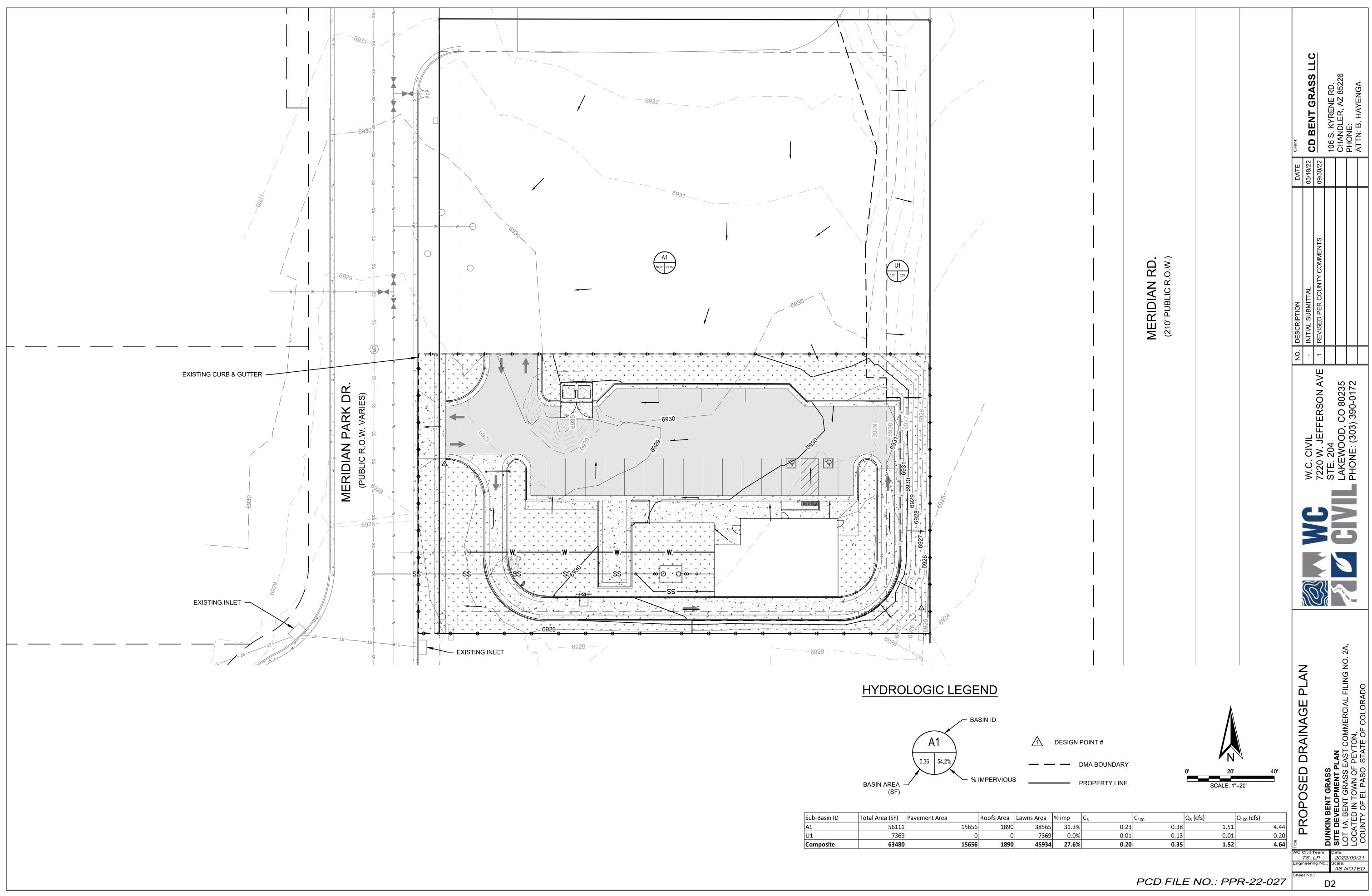
unmapped and unmodernized areas cannot be used for regulatory purposes.



Appendix D Drainage Maps



Composite	63480	0	
EX2	30091	0	
EX1	33389	0	
Sub-Basin ID	Total Area (SF)	Pavement Area	Roofs Area



<u> </u>		•		•
Composite	63480		15656	189
U1	7369		0	
A1	56111		15656	189
Sub-Basin ID	Total Area (SF)	Pavement Area		Roofs Area



Appendix E Hydrologic Calculations

Dunkin Bent Grass Hydrology Calcs

NRCS Soil Group

Runoff coefficients per Phase 1 PDR (Basins D and D1, 95% Impervious)

C-valu	es	C ₅	C ₁₀₀		
Weigh	ted C	0.54	().857	
PDR C-values used for comparative purposes ONLY.					

per Table 6-5, MHF	D Drainage Criteria Manual,	Vol. 1
C ₅	C ₁₀₀	
0.010	0.130	
0.230	0.380	
0.810	0.890	
	C ₅ 0.010 0.230	0.010 0.130 0.230 0.380

Rainfall (in/hr)	i ₅	i ₁₀₀
	5.10	9.07

Rainfall values per Bent Grass East Commercial Phase 1 PDR (Basins D and D1)

Existing Drainage Basin Analysis (C values taken from Bent Grass East Commercial Phase 1 PDR)

Sub-Basin ID	Total Area (SF)	Pavement Area	Roofs Area	Lawns Area	% imp	C ₅ (Weighted)	C ₁₀₀ (Weighted)	Q ₅ (cfs)	Q ₁₀₀ (cfs)
D1	27443	26071	0	1372	95.0%	0.54	0.86	1.74	4.90
D	17860	16966	0	894	95.0%	0.54	0.86	1.13	3.19
Composite	45303	43037	0	2266	95.0%	0.05	0.17	2.86	8.08

Existing Drainage Basin Analysis (C values per Table 6-5, MHFD DCM, Vol. 1)

Sub-Basin ID	Total Area (SF)	Pavement Area	Roofs Area	Lawns Area	% imp	C ₅	C ₁₀₀	Q ₅ (cfs)	Q ₁₀₀ (cfs)
D1	27443	26071	0	1372	95.0%	0.81	0.89	2.60	5.09
D	17860	16966	0	894	95.0%	0.81	0.89	1.69	3.31
Composite	45303	43037	0	2266	95.0%	0.81	0.89	4.30	8.40

Existing Drainage Basin Analysis (C values per Table 6-5, MHFD DCM, Vol. 1), 95% Imperviousness per Phase 1 PDR, Revised Lot Area

А

Sub-Basin ID	Total Area (SF)	Pavement Area	Roofs Area	Lawns Area	% imp	C ₅	C ₁₀₀	Q ₅ (cfs)	Q ₁₀₀ (cfs)
EX1	33389	31720	0	1669	95.0%	0.81	0.85	3.17	5.91
EX2	30091	28586	0	1505	95.0%	0.81	0.85	2.85	5.33
Composite	63480	60306	0	3174	95.0%	0.05	0.17	6.02	11.24

WCC Existing Drainage Plan

Sub-Basin ID	Total Area (SF)	Pavement Area	Roofs Area	Lawns Area	% imp	C ₅	C ₁₀₀	Q ₅ (cfs)	Q ₁₀₀ (cfs)
EX1	33389	0	0	33389	0.0%	0.01	0.13	0.04	0.90
EX2	30091	0	0	30091	0.0%	0.01	0.13	0.04	0.81
Composite	63480	0	0	63480	0.0%	0.01	0.13	0.07	1.72

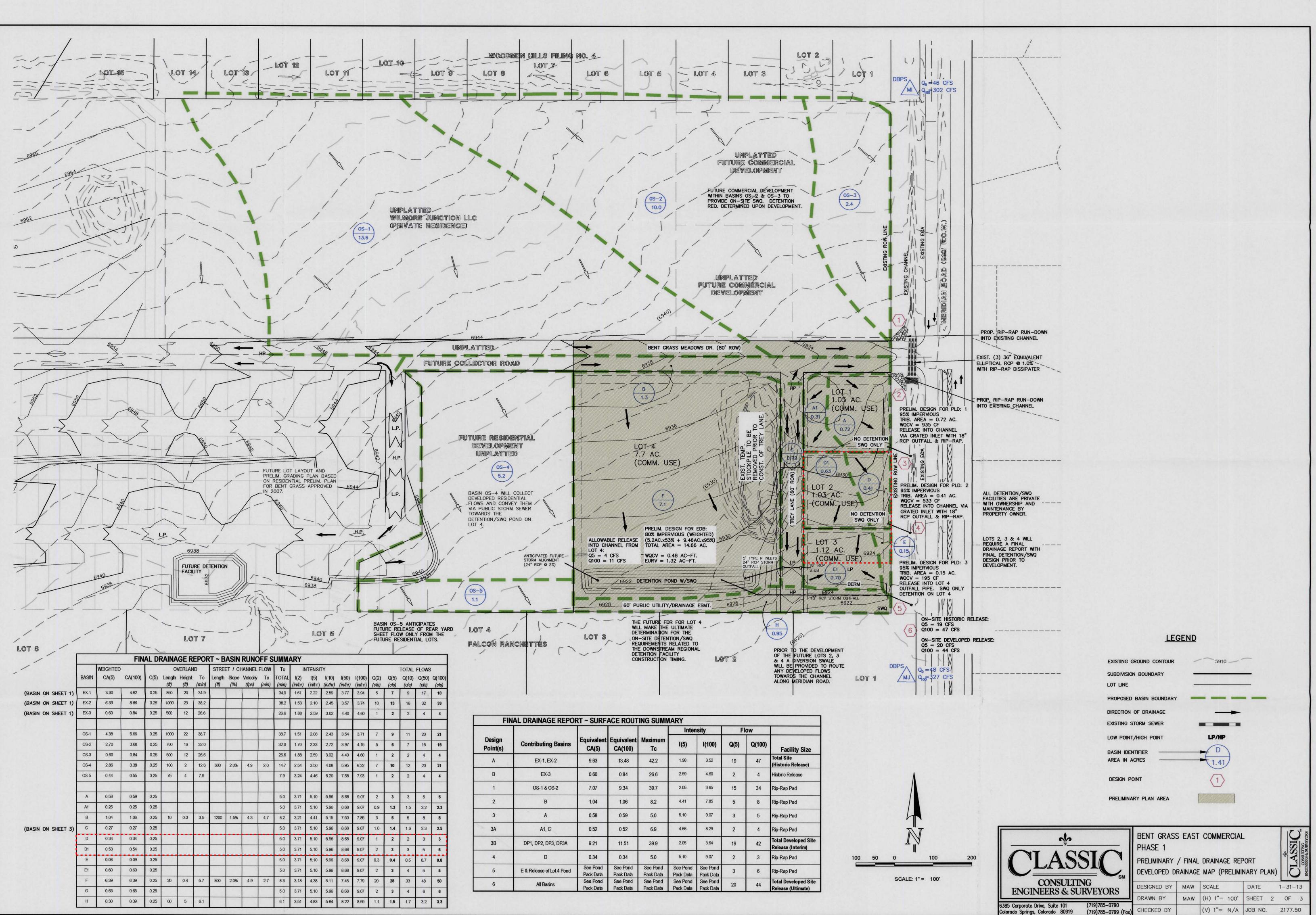
WCC Proposed Drainage Plan

Sub-Basin ID	Total Area (SF)	Pavement Area	Roofs Area	Lawns Area	% imp	C ₅	C ₁₀₀	Q ₅ (cfs)	Q ₁₀₀ (cfs)
A1	56111	15656	1890	38565	31.3%	0.23	0.38	1.51	4.44
U1	7369	0	0	7369	0.0%	0.01	0.13	0.01	0.20
Composite	63480	15656	1890	45934	27.6%	0.20	0.35	1.52	4.64



Appendix F

Phase 1 PDR Developed Drainage Map (Preliminary Plan) By Classic Consulting Engineers & Surveyors (1/31/2013)



400	50	
100	50	0