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**SOILS AND GEOLOGY STUDY
CALM BREEZE SUBDIVISION FILING NO. 1
PARCEL NO. 52110-00-013
EL PASO COUNTY, COLORADO**



Prepared for:

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1 SUMMARY

Project Location

The project site is located within the SE¼ of Section 11, Township 12 South, Range 65 West of the 6th Principal Meridian in El Paso County, Colorado. The site is located approximately 4 miles northeast of the Colorado Springs city limits at the eastern end of Shoup Road. The location of the site is as shown in the Vicinity Map (Figure 1).

Project Description

A four-lot rural residential subdivision is proposed for the 42.22-acre parcel. The new lots will be approximately 10 acres and will be serviced by individual water wells and on-site wastewater treatment systems (OWTS). The lots will be accessed by an existing private roadway, Calm Breeze Place.

Scope of Report

This report presents the results of our geologic evaluation and treatment of engineering geologic hazard study with respect to the proposed subdivision.

Land Use and Engineering Geology

This site was found to be suitable for the proposed development; however, geologic conditions in certain areas will impose some constraints on development and land use. These include areas of artificial fill, potentially expansive soils, downslope creep, potentially unstable slopes, potentially shallow and seasonally shallow groundwater areas, and the potential for elevated radon levels. These conditions are discussed in greater detail in this report.

In general, it is our opinion that the development can be achieved if the observed geologic conditions on site are either properly mitigated or avoided. All recommendations are subject to the limitations discussed in the report.

2 GENERAL SITE CONDITIONS AND PROJECT DESCRIPTION

The topography of the site is generally gradually to moderately sloping to the southwest, with moderate to steep slopes along the drainages in the western and southeast portions of the site. Minor drainages were observed on the site, generally sloping to the southwest towards Black Squirrel Creek. The site boundaries are indicated in the USGS Map (Figure 2). Previous land uses have included rural residential. Vegetation observed on the site includes field grasses,

weeds, and Ponderosa Pines. A portion of the site is located within the Black Forest burn scar. Site photographs are included in Appendix A.

The project consists of the subdividing of 42.22 acres into a 4-lot rural residential subdivision. The new lots will be approximately 10 acres and will be serviced by individual water wells and OTWS. The proposed lot configuration is shown in the Site and Exploration Plan (Figure 3).

3 SCOPE OF THE REPORT

The scope of this report includes a general geologic analysis utilizing published geologic data. Detailed site-specific mapping was conducted to obtain general information with respect to major geographic and geologic features, geologic descriptions, and their effects on the development of the property in accordance with the El Paso Land Development Code.

4 FIELD INVESTIGATION

Our field investigation consisted of the preparation of a geologic map of any bedrock features and significant surficial deposits. The site was also evaluated using the Web Soil Survey from the Natural Resource Conservation Service (NRCS), formerly known as the Soil Conservation Service (SCS). The position of mappable units within the subject property is shown on the Geologic Map. Our mapping procedures involved both field reconnaissance and measurements and air photo reconnaissance and interpretation. The same mapping procedures have also been utilized to produce the Geology/Engineering Geology Map, which identified pertinent geologic conditions affecting development. The field mapping was performed by personnel of Entech on April 29, 2026. Site photographs are included in Appendix A.

Four (4) test borings were drilled and four (4) tactile test pits were excavated on the site to determine the classification and engineering characteristics of the soils. The borings were drilled to depths of 20 feet using a truck-mounted, continuous flight auger drilling rig supplied and operated by Entech, and the test pits were excavated to depths of 5.5 to 8 feet.

The locations of the test boring and test pits are indicated on the Site and Exploration Plan (Figure 3). The Test Boring and Test Pit Logs are included in Appendix B, and Laboratory Test Results are included in Appendix C. The results of the testing will be discussed later in this report.

Laboratory testing was performed to classify and determine the engineering characteristics of the soils. Laboratory tests included moisture content testing (ASTM D2216), grain-size analysis (ASTM D6913), and Atterberg Limits testing (ASTM D4318). Results of the laboratory testing are included in Appendix C.

5 SOIL, GEOLOGY, AND ENGINEERING GEOLOGY

5.1 General Geology

The site lies in the western portion of the Great Plains Physiographic Province just south of the Palmer Divide. A major structural feature known as the Rampart Range Fault lies approximately 15 miles to the west. This fault marks the boundary between the Great Plains Physiographic Province and the Southern Rocky Mountain Province. The site exists within the southeastern edge of a large structural feature known as the Denver Basin. Bedrock in the area tends to be very gently dipping in a northeasterly direction (Reference 1). The rocks in the area of the site are sedimentary in nature and typically Tertiary to Cretaceous in age. The bedrock underlying the site consists of the Dawson Arkose Formation of Tertiary to Cretaceous Age. Overlying this formation are unconsolidated deposits of alluvium, colluvium, sheetwash, residual soils, and man-placed fill of Quaternary Age. The residual soils are produced by the in-situ action of weathering of the bedrock on site. Some colluvial soils deposited by gravity and sheetwash exist. The alluvial soils were deposited by water along Black Squirrel Creek and the minor drainages on the site. The site's stratigraphy will be discussed in more detail in Section 5.3.

5.2 Soil Conservation Survey

The Natural Resource Conservation Service (Reference 2), previously the Soil Conservation Service (Reference 3), has mapped two soil types on the site (Figure 4). In general, they are loamy sands. The soils are described in Exhibit 1 below.

Exhibit 1: Soil Survey Description

Type	Description
26	Elbeth sandy loam, 8 to 15% slopes
41	Kettle gravelly loamy sand, 8 to 40% slopes

Exhibit 2: Soil Map



Complete descriptions of each soil type are presented in Appendix D. The soils have generally been described as having moderately high to high permeabilities. Limitations on development include limited ability to support a load, shrink-swell potential, slopes, and frost action potential. Possible hazards with soil erosion are present on the site. The erosion potential can be controlled with vegetation. The soils have been described to have moderate erosion hazards.

5.3 Site Stratigraphy

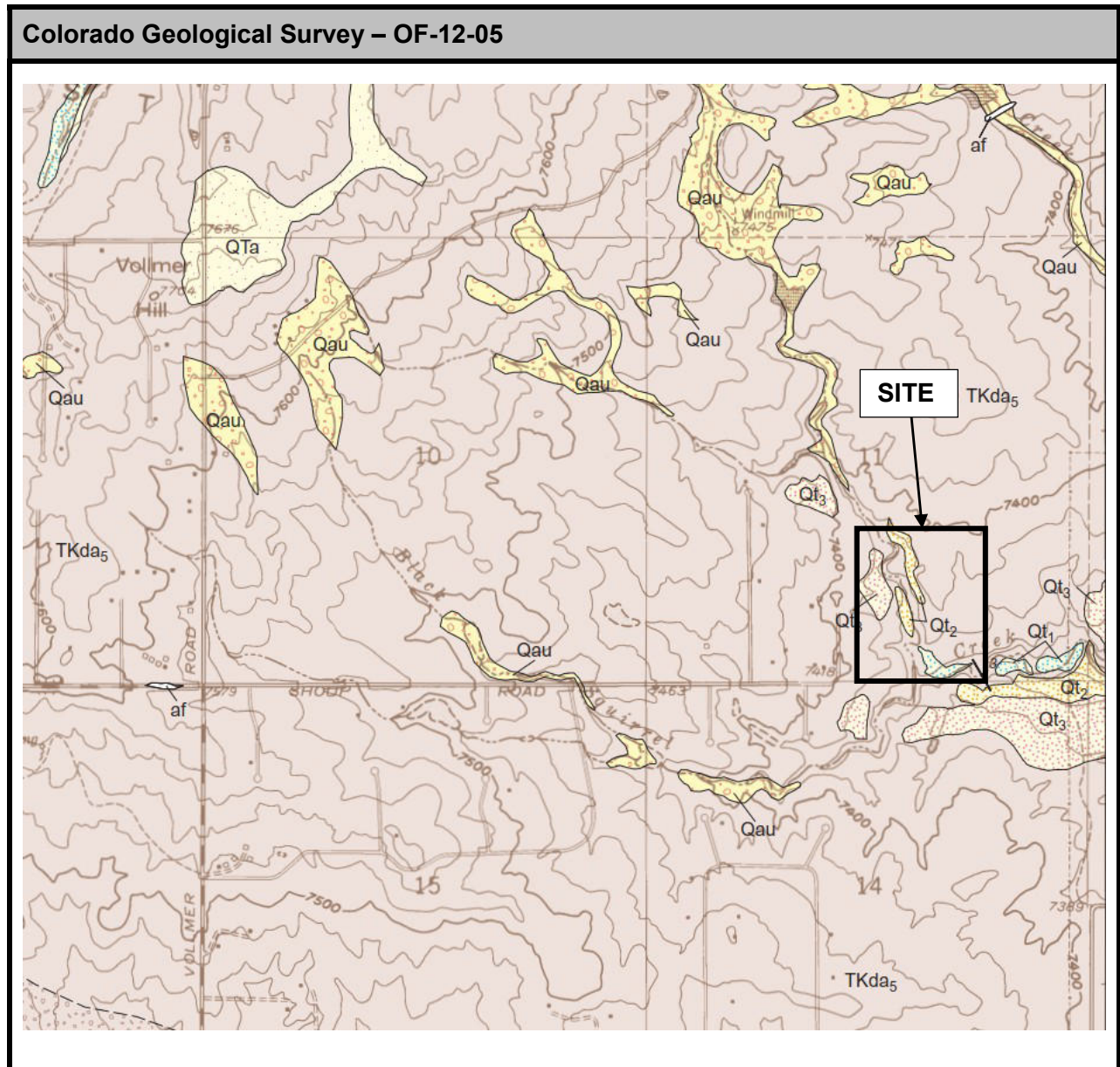
The Geologic Map of the Black Forest Quadrangle showing the site is presented in Figure 5 (Reference 4). The Geology/Engineering Geology Map prepared for the site is presented in Figure 6. Six mappable units were identified on this site that are described as follows:

- Qaf Artificial Fill of Holocene Age:** These are recent man-place fill associated with the existing private roadway and an earthen embankment on Lot 1.
- Qal Alluvium of Quaternary Age:** These are recent water-deposited materials associated with Black Squirrel Creek. These materials generally consist of silty to clayey sands with lenses of clays and gravels. These materials tend to have relatively high permeabilities and moderate densities
- Qt₁ Terrace Alluvium one of Quaternary Age:** These terrace deposited alluvium along the Black Squirrel Creek. The upper surface of the deposit is approximately 5 to 25 feet above current stream elevation. The materials typically consist of light-brown to gray-brown sand, silty fine sand, sandy silt, and clayey silt and gravel.
- Qt₂ Terrace Alluvium two of Quaternary Age:** These terrace deposited alluvium along the Black Squirrel Creek. The upper surface of the deposit is approximately 20 to 20 feet above current stream elevation. The materials typically consist of light-brown to light grayish-brown poorly sorted sand and gravel.
- Qt₃ Terrace Alluvium three of Quaternary Age:** These terrace deposited alluvium along the Black Squirrel Creek. The upper surface of the deposit is approximately 20 to 50 feet above current stream elevation. The materials typically consist of light-brown extremely poorly sorted sand and gravel.
- Qc/Tkd Colluvium of Quaternary Age overlying Dawson Formation of Tertiary to Cretaceous Age:** The Dawson Formation typically consists of arkosic sandstone with interbedded fine-grained sandstone, siltstone, and claystone. Overlying this formation is a variable layer of colluvium and residual soils. The residual soils were derived from the in-situ weathering of the bedrock materials on-site. These soils consist of clayey sands and sandy clays. The colluvial materials are derived from the bedrock materials and have been re-deposited by the action of sheetwash and gravity.

The soils listed above were mapped from site-specific mapping, the *Geologic Map of the Black Forest Quadrangle* distributed by the Colorado Geological Survey in 2003 (Reference 4), the

Geologic Map of the Colorado Springs-Castle Rock Area, distributed by the US Geological Survey in 1979 (Reference 5), and the Geologic Map of the Denver 1° x 2° Quadrangle, distributed by the US Geological Survey in 1981 (Reference 6). The Test Borings and Test Pit Logs used in evaluating the site are included in Appendix B. The Geology/Engineering Geology Map prepared for the site is presented in Figure 6.

Exhibit 3: Geologic Map of the Black Forest Quadrangle



5.4 Soil Conditions

The soils encountered in the test borings drilled on the site can be grouped into two general soil and rock types. The soils were classified using the Unified Soil Classification System (USCS).

Soil Type 1 classified as a silty to clayey sand (SM, SC). The sand was encountered in all test borings at the existing surface and extending to depths of 1 foot to 7 feet below the existing grade surface (bgs). The sand was encountered at medium dense to very dense states and moist conditions.

Soil Type 2 classified as highly weathered sandstone or clayey sand when classified as a soil (SM). The sandstone was encountered in all test borings at depths of 1 foot to 7 feet bgs and extending to the termination depth of the borings (20 feet). The sandstone was encountered at very dense to dense states and moist conditions.

Although clays, claystone, or siltstone were not encountered in the test borings, they are common in the area of the project site. Claystone and siltstone are commonly interbedded in the Dawson Formation. Clays, claystone, and siltstone found in the project area are typically expansive.

The test boring logs and laboratory test results pertaining to this investigation are included in Appendix B.

5.5 Groundwater

Groundwater was not encountered in the borings. Redoximorphic features were encountered in TP-3 at a depth of 4 feet during excavation. Areas of potentially shallow and seasonally shallow groundwater have been mapped in the drainages and some low-lying areas on the site. These areas are discussed in the following section. Fluctuation in groundwater conditions may occur due to variations in rainfall and other factors including development of the site and surrounding areas.

For the sandy materials on site, it should be noted that some groundwater conditions might be encountered due to the variability in the soil profile. Isolated sand and gravel layers within the soils, sometimes only a few feet in thickness and width, can carry water in the subsurface. Groundwater may also flow on top of the underlying bedrock or clays. Builders and planners should monitor potential occurrences of such subsurface water features during construction on-site and mitigate as necessary at the time of construction.

6 ENGINEERING GEOLOGY – IDENTIFICATION AND MITIGATION OF GEOLOGIC HAZARDS

Detailed mapping has been performed on this site to produce a Geology/Engineering Geology Map (Figure 6). This map shows the location of various geologic conditions the developers should monitor during the planning, design, and construction stages of the project. No significant geologic hazards were identified on the site; however, the following constraints have been identified on the site: artificial fill, potentially expansive soils, potentially shallow and seasonally shallow groundwater areas, drainage areas, downslope creep, potentially unstable slopes, and the potential for elevated radon levels. These constraints and the recommended mitigation techniques are discussed below.

Artificial Fill – Constraint

These are areas of man-placed fill associated with the existing road on the site and an earthen embankment in the northeastern portion of the site on Lot 1. Other areas of fill may exist on the site other than those mapped.

Mitigation: The fill on this site is not located within any potential building areas. Any uncontrolled fill encountered beneath foundations will require removal and recompaction at a minimum of 95% of the Modified Proctor (ASTM D1557) maximum dry density.

Potentially Expansive Soils – Constraint

The site is classified in an area of low swell potential according to the *Map of Potentially Swelling Soil and Rock in the Front Range Urban Corridor, Colorado* by Hart, 1974 (Reference 7). Expansive clays and claystone bedrock are commonly interbedded in the Dawson Formation. These occurrences are typically sporadic; therefore, none have been indicated on the maps. These clays, if encountered beneath foundations, can cause differential movement in the structure foundation. The sand and sandstone soils are anticipated to exhibit low expansion potentials. If potentially expansive materials are found during excavation, they will need to be mitigated. Expansive soils were not encountered in the test borings. Expansive soils should be identified and mitigated on an individual basis.

Mitigation: Should expansive soils be encountered beneath foundations; mitigation will be necessary. Overexcavation and replacement with non-expansive soils at a minimum of 95% of the Modified Proctor (ASTM D1557) maximum dry density is a suitable mitigation, which is common in the area. Overexcavation depths of 3 to 5 feet may be necessary depending on soil conditions. Another alternative in areas of highly expansive soils is the use of drilled pier

foundation systems. Typical minimum pier depths are on the order of 25 feet or more and require penetration into the bedrock material a minimum of 4 to 6 feet, depending upon building loads. Floor slabs on expansive soils should be expected to experience movement. Overexcavation and replacement have been successful in minimizing slab movements. The use of structural floors should be considered for basement construction on highly expansive clays. Final recommendations should be made after additional investigation of each building site.

Drainage and Floodplain Areas – Constraint

Part of the southeastern portion of the site is located within a 100-year floodplain associated with Black Squirrel Creek according to FEMA Map No. 08041C0320G (Reference 8) and is Shown in Exhibit 5 below. In addition to the mapped floodplain, the unnamed tributary of Black Squirrel Creek in the western and northwestern portions of the site have been mapped as a physiographic floodplain on the Geology and Engineering Geology Map, Figure 4. Construction is not anticipated within floodplain areas. Finished floor elevations should be at least one foot above the floodplain. Specific drainage studies and floodplain locations are beyond the scope of this report.

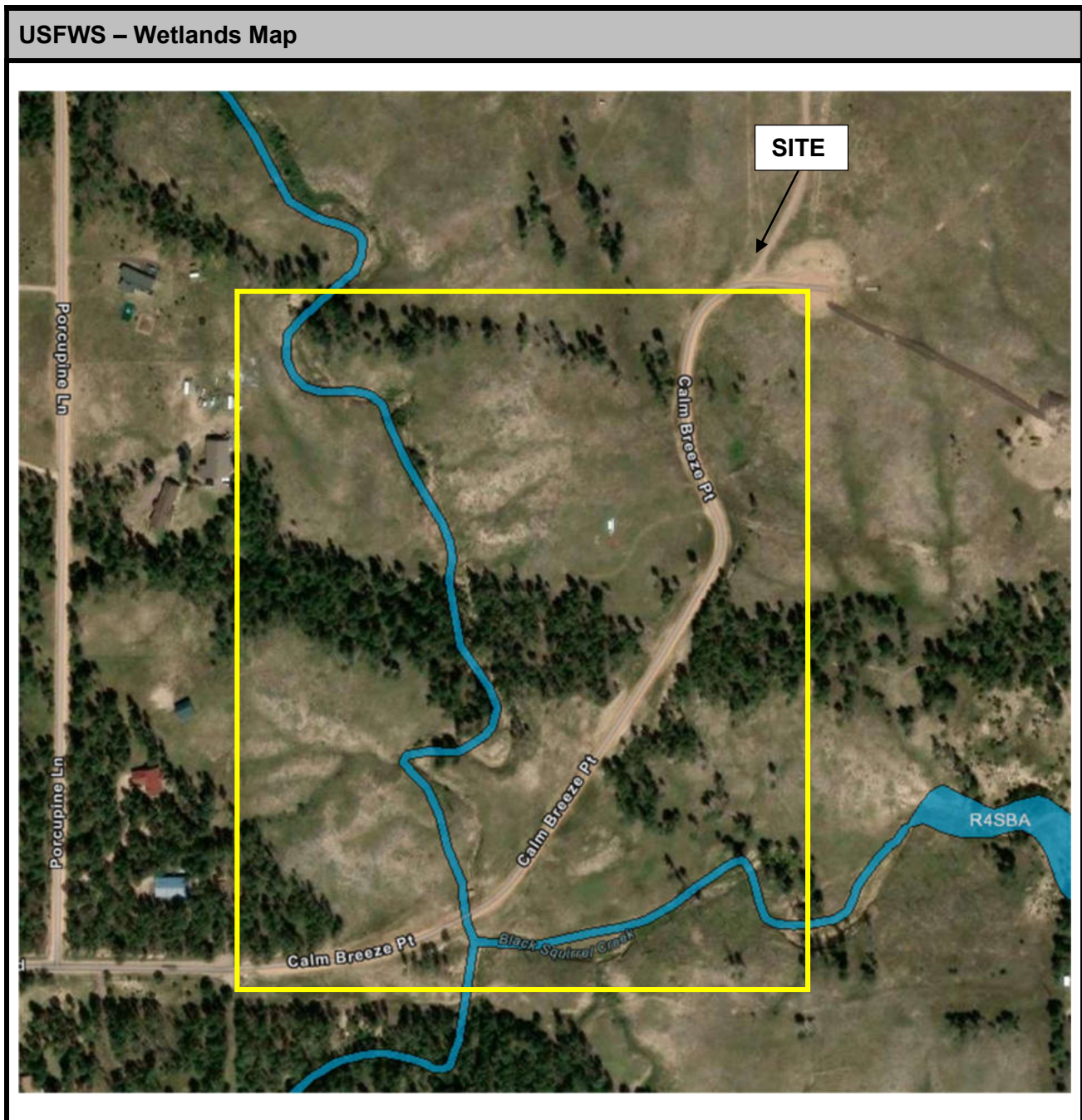
Exhibit 4: FEMA Flood Map

FEMA Map No. 08041C0320G, eff. 12/7/2018



Black Squirrel Creek and the unnamed tributary of Black Squirrel Creek located on the site have been included in the National Wetlands Inventory as a Freshwater Wetland habitat classified as R4SBA (R – Riverine, 4 – Intermittent, SB – Streambed, A – Temporary Flooded) (Reference 9) and is shown in Exhibit 6 below. These areas should be avoided by construction.

Exhibit 6: National Wetlands Inventory Map



Potentially Shallow and Seasonally Shallow Groundwater Areas – Constraint

In these areas, we would anticipate periodic high subsurface moisture conditions and frost heave potential on a seasonal basis. Redoximorphic features were observed in TP-3 at a depth of 4 feet. Any structures in or adjacent to potentially shallow groundwater areas should follow the mitigation discussed below.

Mitigation: Construction is not anticipated in these areas. Foundations on this site must have a minimum 30-inch depth for frost protection. Buildings should be a minimum of 3 feet above groundwater levels. Subsurface perimeter drains are recommended for any usable below-grade areas including crawlspaces. Should groundwater be encountered within 3 feet of foundation grade, additional drains that could include but are not limited to interceptor drains, underslab drains (capillary break), and overexcavation drains may be needed. Typical drain details are presented in Figures 5 through 7. Any grading in these areas should be done to direct surface flow around construction to avoid areas of ponded water. Structures should not block drainages. All organic material should be completely removed prior to any fill placement. Septic fields should not be placed in areas where there is the potential for shallow groundwater.

Slope Stability and Landslide Susceptibility – Hazard

The majority of the slopes on the site are gradually to moderately sloping and do not exhibit any past or potential unstable slopes or landslides. Moderate to steep slopes associated with Black Squirrel Creek were observed in the western and southeastern portions of the site. Signs of failures or instability were not observed during field mapping, however, signs of downslope creep were observed. It is recommended that any future grading or fill slopes be 3:1 or flatter. The steeper slopes along Black Squirrel Creek have been identified as potentially unstable. The areas impacted by the downslope creep and potentially unstable slopes are further discussed below.

Downslope Creep – Constraint

Areas of downslope creep associated with the moderate to steep slopes on the site have been identified. In these areas we would anticipate lateral and vertical movement of the near surface soils in the downslope direction. These areas are suitable as building sites with the following constraints on construction. If construction in these areas is considered, the following mitigation recommendations should be considered.

Mitigation: Building is possible in these areas if the following engineering and construction mitigation steps are taken: Downslope creep will increase lateral pressures against foundation components on the uphill side of structures. The design of foundations in these areas should account for this additional pressure. A lateral pressure detail is shown in Figure 8. Where possible in areas of downslope creep, structures should be designed to be as compact and rigid as possible. This will help them better tolerate the vertical and lateral movements to which the foundation system may be subjected. Long, rambling, irregular structures should be avoided in these areas as they are associated with a much greater potential for damaging differential movement. Tie walls and buttresses are often used to stiffen the foundation system.

Potentially Unstable Slope Areas - Hazard

The steep slopes along portions of the drainages on the site have been identified as potentially unstable. These slopes are considered stable in their present condition; however, considerable care must be exercised in these areas not to create a condition which would tend to activate instability. These areas are primarily located along the banks of Black Squirrel Creek and should be avoided by construction. A minimum setback of 30 feet from the crest of the potentially unstable slope is recommended. Depending on final development plans and building locations site specific slope stability analysis may be needed.

Debris Flow Susceptibility – Hazard

The site is not located within an area of debris flow susceptibility according to the *Debris Flow Susceptibility Map of El Paso County, Colorado* by McCoy et al. in 2018 (Reference 10).

Radon – Hazard

Radon is a colorless, tasteless radioactive gas with a United States Environmental Protection Agency (EPA) specified action level of 4.0 picocuries per liter (pCi/L) of air. Radon gas has a very short half-life of 3.8 days. Radon levels for the area have been reported by the Colorado Geologic Survey in the open file, Report No. 91-4 (Reference 11). The average radon level for the 80908 zip code is 3.40 pCi/l. The average radon levels are presented in Exhibit 2 below.

Exhibit 7: Average Radon Levels

Average Radon Levels for the 80908 Zip Code	
0 < 4 pCi/L	50.00%
4 < 10 pCi/L	50.00%
10 < 20 pCi/L	0.00%
> 20 pCi/L	0.00%

Mitigation: The potential for high radon levels is present for the site. Build-up of radon gas can usually be mitigated by providing increased ventilation of basement and crawlspace and sealing joints. **Specific requirements for mitigation should be based on site specific testing.**

6.1 Relevance of Geologic Conditions to Land Use Planning

We understand that the development will be rural residential. Four lots approximately 10 acres in size are proposed. It is our opinion that the existing geologic and engineering geologic conditions will impose some constraints on the proposed development. The most significant problems will be those associated with the potentially expansive soils, potentially unstable slopes, floodplains,

potentially shallow groundwater, and the potential for elevated radon levels. The constraints/hazards on the site may be satisfactorily mitigated through proper engineering design and construction practices.

The granular soils were generally encountered at medium dense to very dense states. The granular soils encountered in the upper soil profiles of the test boring should provide good support for foundations. Foundations anticipated for the site are standard spread footings on the medium dense granular soils or sandstone bedrock.

Expansive layers may also be encountered in the soil and bedrock on this site. Areas of expansive soils encountered on site are sporadic; therefore, none have been indicated on the maps. Expansive soils, if encountered, will require special foundation design and/or overexcavation. These soils will not prohibit development. Overexcavation and replacement with non-expansive soils is a suitable mitigation. Site-specific subsurface investigations will need to be conducted.

In summary, development of the site can be achieved if the items mentioned above are mitigated. These items can be mitigated through proper design and construction or through avoidance. Investigation of the individual lots and building sites is recommended prior to construction.

7 ECONOMIC MINERAL RESOURCES

Some of the sandy materials on site could be considered a low-grade sand resource. According to the *El Paso County Aggregate Resource Evaluation Map* (Reference 12), the site is not mapped within any aggregate resources. According to the *Atlas of Sand, Gravel and Quarry Aggregate Resources, Colorado Front Range Counties* distributed by the Colorado Geological Survey (Reference 13), areas of the site are not mapped with any resources. According to the *Evaluation of Mineral and Mineral Fuel Potential* (Reference 14), the area of the site has been mapped as “Little or No Potential” for industrial minerals. It is possible that sand materials on site could be an aggregate resource. However, considering the silty to clayey nature of many of these materials, the abundance of similar materials throughout the region, shallow bedrock, and the proximity to developed land, they would be considered to have little significance as an economic resource.

According to the *Evaluation of Mineral and Mineral Fuel Potential of El Paso County State Mineral Lands* (Reference 14), the site is mapped within the Denver Basin Coal Region. However, the area of the site has been mapped as “Poor” for coal resources. No active or inactive mines have been mapped near the site. No metallic mineral resources have been mapped on the site (Reference 14).

The site has been mapped as “Fair” for oil and gas resources (Reference 14). No oil or gas fields have been discovered near the site. The sedimentary rocks in the area may lack the geologic structure for trapping oil or gas; therefore, they may not be considered a significant resource. Hydraulic fracturing is a new method used to extract oil and gas from rocks. It utilizes pressurized fluid to extract oil and gas from rocks that would not normally be productive. The area of the site has not been explored to determine if the rocks underlying the site would be commercially viable utilizing hydraulic fracturing. The practice of hydraulic fracturing has come under review due to concerns about environmental impacts, health, and safety.

8 EROSION CONTROL

The soil types observed on the site are mildly to highly susceptible to wind erosion and moderately to highly susceptible to water erosion. A minor wind erosion and dust problem may be created for a short time during and immediately after construction. Should the problem be considered severe enough during this time, watering of the cut areas or the use of chemical palliative may be required to control dust. However, once construction has been completed and vegetation re-established, the potential for wind erosion should be considerably reduced.

Regarding water erosion, loosely compacted soils will be the most susceptible to water erosion; residually weathered soils and weathered bedrock materials become increasingly less susceptible to water erosion. For the typical soils observed on site, allowable velocities or unvegetated and unlined earth channels would be on the order of 3 to 4 feet/second, depending upon the sediment load carried by the water. Permissible velocities may be increased through the use of vegetation to something on the order of 4 to 7 feet/second, depending upon the type of vegetation established. Should the anticipated velocities exceed these values, some form of channel lining material may be required to reduce erosion potential. These might consist of synthetic channel lining materials available on the market or conventional riprap. In cases where ditch-lining materials are still insufficient to control erosion, small check dams or sediment traps may be required. The check dams will serve to reduce flow velocities as well as provide small traps for containing sediment. The determination of the amount, location, and placement of ditch linings, check dams, and special erosion control features should be performed by or in conjunction with the drainage engineer who is more familiar with the flow quantities and velocities.

Cut and fill slope areas will be subjected primarily to sheetwash and rill erosion. Unchecked rill erosion can eventually lead to concentrated flows of water and gully erosion. The best means to

combat this type of erosion is, where possible, the adequate re-vegetation of cut and fill slopes. Cut and fill slopes having gradients more than three (3) horizontal to one (1) vertical become increasingly more difficult to revegetate successfully. Therefore, recommendations pertaining to the vegetation of the cut and fill slopes may require input from a qualified landscape architect and/or the Soil Conservation Service.

9 PRIVATE DRIVEWAY AND ROADWAY CONSTRUCTION RECOMMENDATIONS

The site soils encountered in the test boring are suitable for the proposed driveways and roads. Any areas to receive fill should have all topsoil, organic material, or debris removed. Prior to fill placement, Entech should observe the subgrade. Fill must be properly benched and compacted to minimize potentially unstable conditions in slope areas. Fill slopes should be 3:1 or flatter. The subgrade should be scarified and moisture conditioned to within 2% of optimum moisture content and compacted to a minimum of 95% of the Modified Proctor (ASTM D1557) maximum dry density prior to placing new fill. Areas receiving fill may require stabilization with rock or fabric if shallow groundwater conditions are encountered.

New fill should be placed in thin lifts not to exceed 6 inches after compaction while maintaining at least 95% of the Modified Proctor (ASTM D1557) maximum dry density. These materials should be placed at a moisture content conducive to compaction, usually 0 to +/-2% of the Proctor optimum moisture content. The placement and compaction of fill should be observed and tested by Entech during construction. Entech should approve any imported materials prior to placing or hauling them to the site. Additional investigation will be required for pavement designs once roadway grading is completed and utilities are installed.

10 CLOSURE

It is our opinion that the existing geologic engineering and geologic conditions will impose some constraints on the proposed development and construction. The site conditions can be mitigated through proper engineering design and construction practices. The proposed development and use are consistent with anticipated geologic and engineering geologic conditions.

It should be pointed out that because of the nature of data obtained by random sampling of such variable and non-homogeneous materials as soil and rock, it is important that we be informed of

any differences observed between surface and subsurface conditions encountered in construction and those assumed in the body of this report. Construction and design personnel should be made familiar with the contents of this report. Additional investigation is recommended as plans are generated prior to construction on each lot.

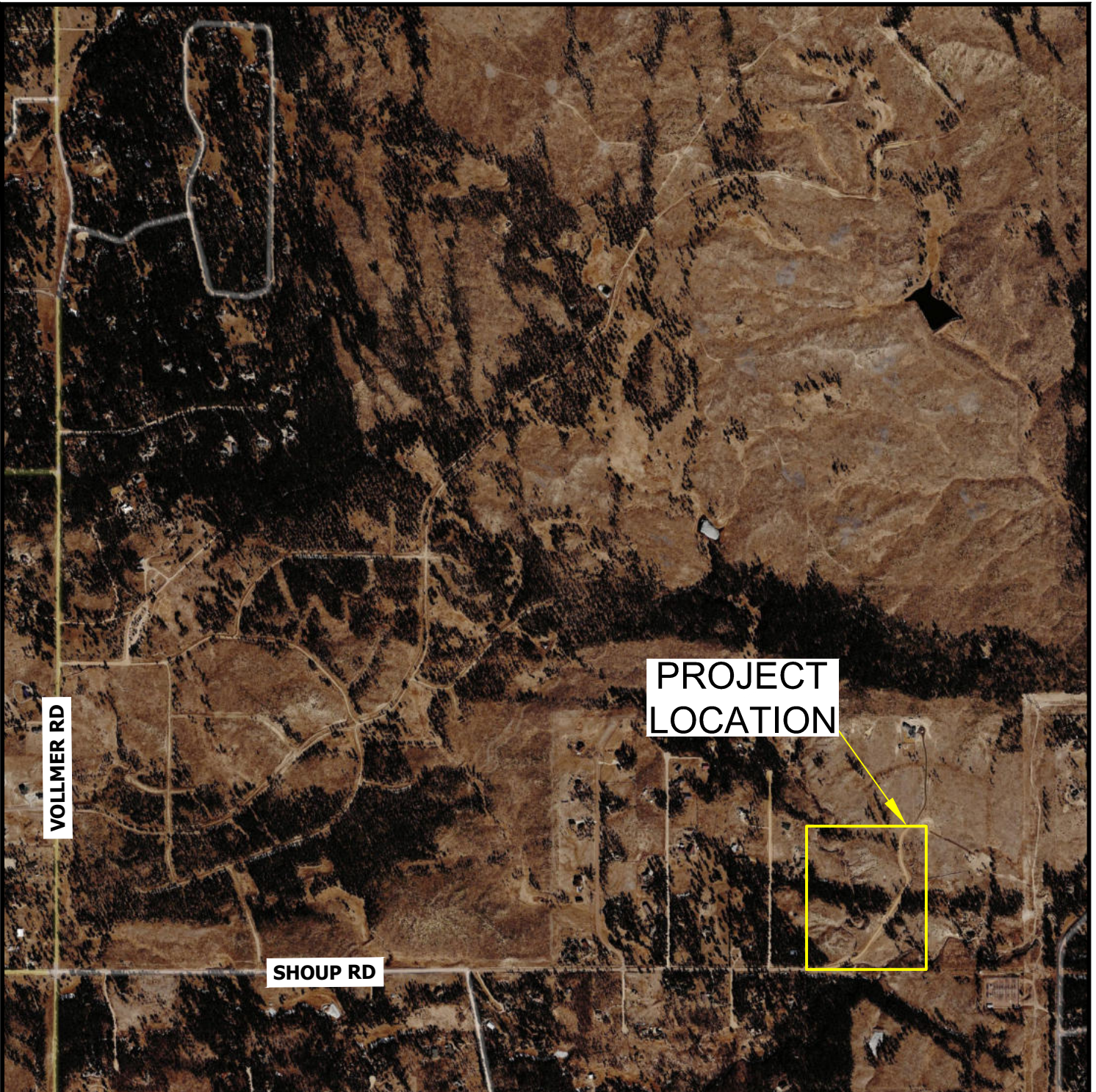
This report has been prepared for M&S Civil Consultants for application to the proposed project in accordance with generally accepted geologic soil and engineering practices. No other warranty, expressed or implied, is made.

We trust that this report has provided you with all the information that you require. Should you require additional information, please do not hesitate to contact Entech Engineering, Inc.

11 REFERENCES

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FIGURES



PROJECT
LOCATION

VOLLMER RD

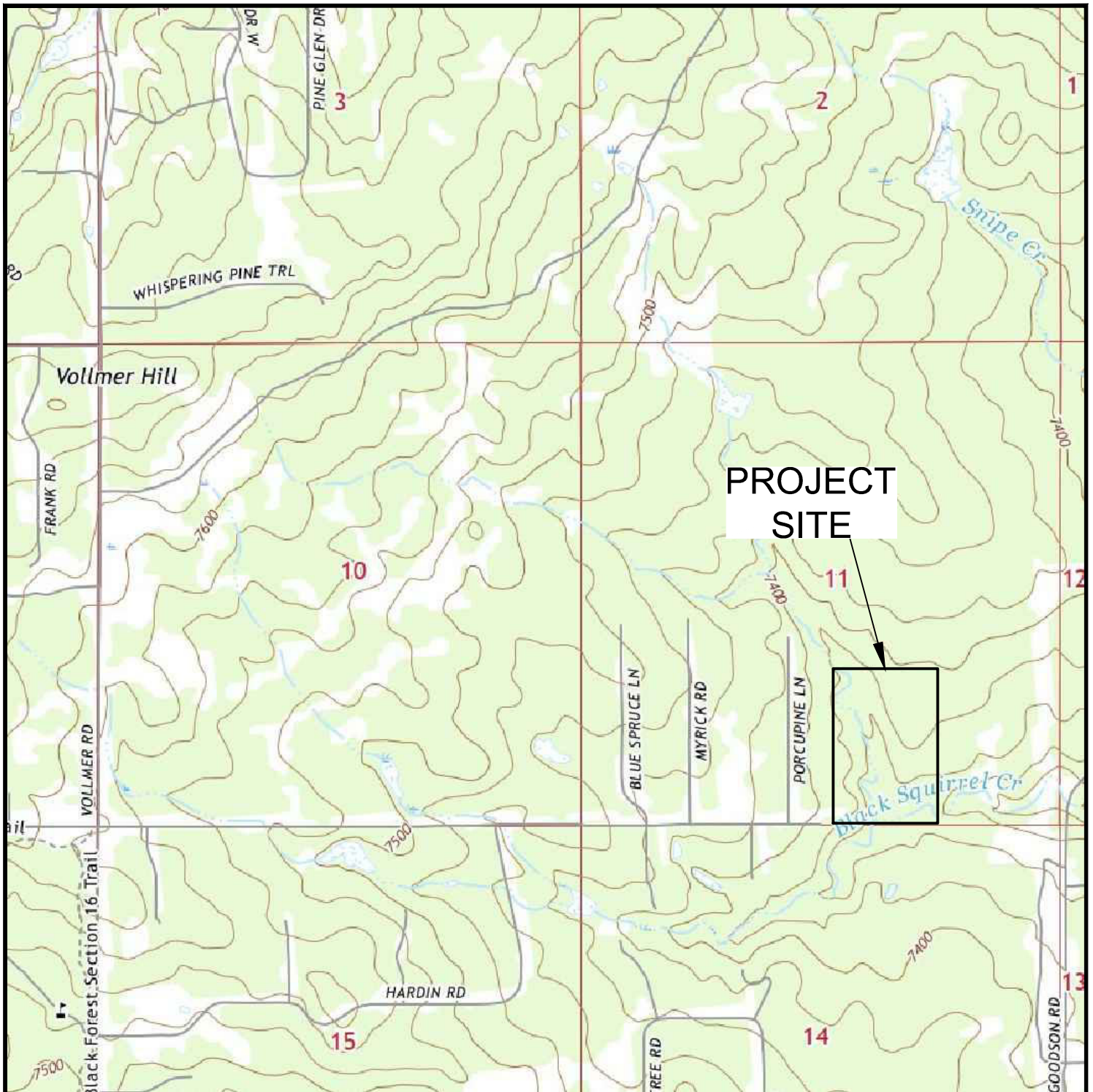
SHOUP RD



VICINITY MAP
CALM BREEZE FILING NO. 1
M&S CIVIL CONSULTANTS

JOB NO.
260651

FIG. 1



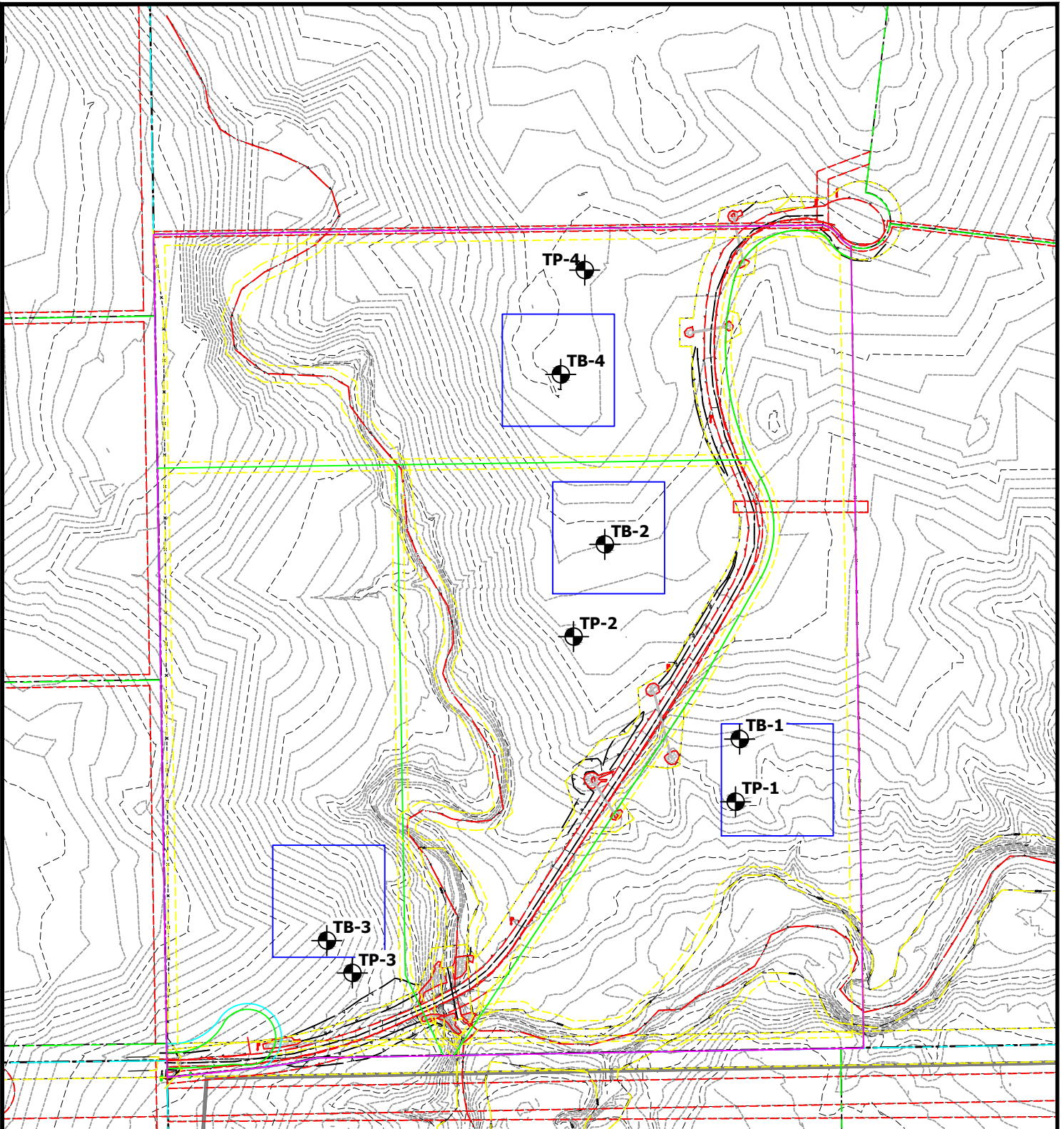
**PROJECT
SITE**





USGS TOPOGRAPHY MAP
 CALM BREEZE SUBDIVISION FILING NO. 1
 M&S CIVIL CONSULTANTS

JOB NO.
260651

FIG. 2



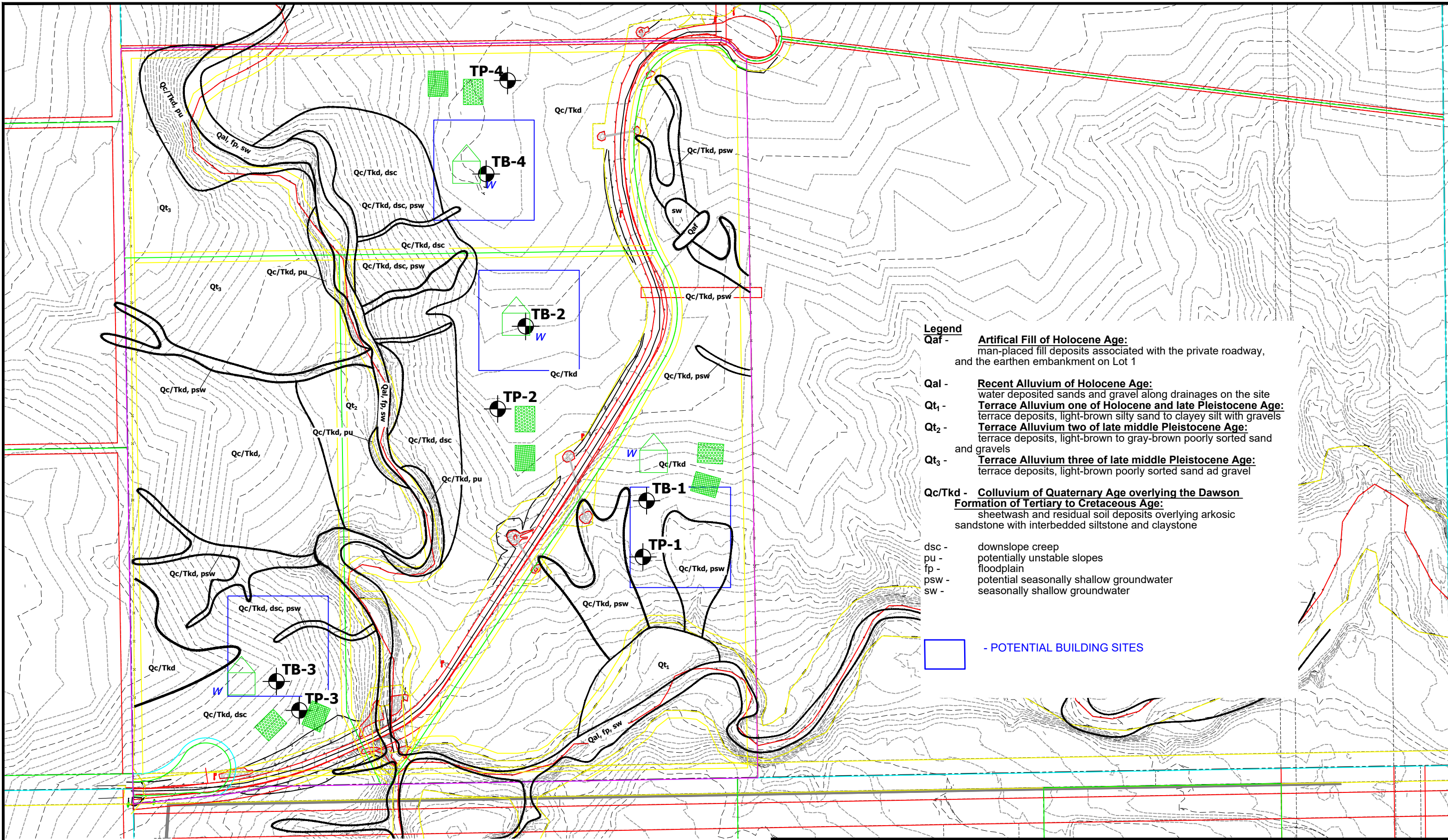
-  - APPROXIMATE TEST BORING LOCATION AND NUMBER
-  - APPROXIMATE TEST PIT LOCATION AND NUMBER

 - POTENTIAL BUILDING SITES



SITE AND EXPLORATION MAP
 CALM BREEZE SUBDIVISION FILING NO. 1
 M&S CIVIL CONSULTANTS

JOB NO.
 260298
FIG. 3



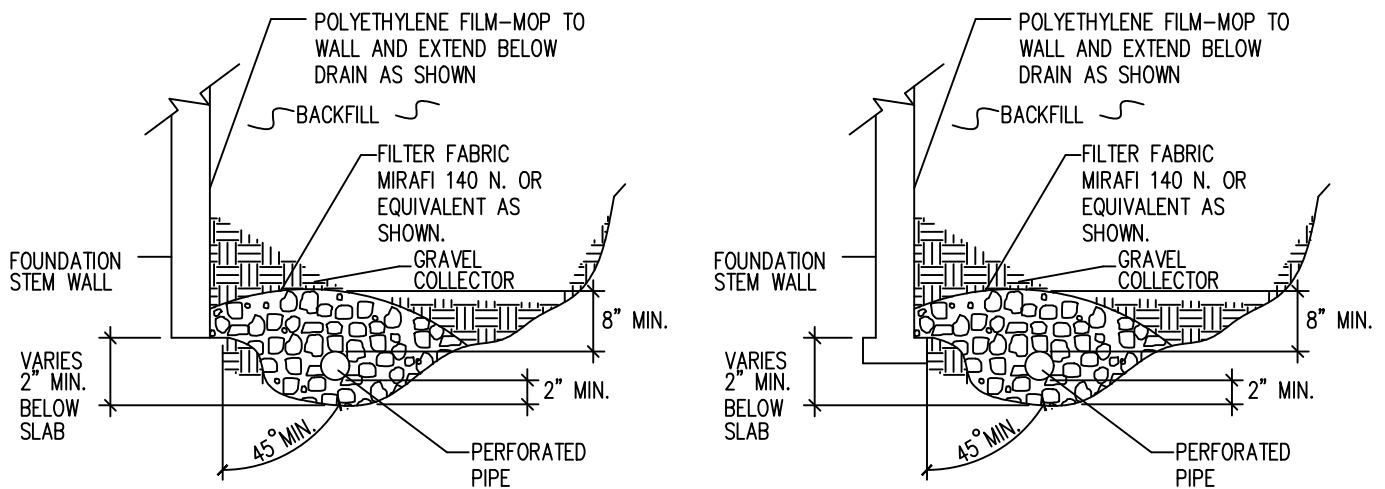
Legend

- Qaf - Artificial Fill of Holocene Age:**
man-placed fill deposits associated with the private roadway, and the earthen embankment on Lot 1
- Qal - Recent Alluvium of Holocene Age:**
water deposited sands and gravel along drainages on the site
- Qt₁ - Terrace Alluvium one of Holocene and late Pleistocene Age:**
terrace deposits, light-brown silty sand to clayey silt with gravels
- Qt₂ - Terrace Alluvium two of late middle Pleistocene Age:**
terrace deposits, light-brown to gray-brown poorly sorted sand and gravels
- Qt₃ - Terrace Alluvium three of late middle Pleistocene Age:**
terrace deposits, light-brown poorly sorted sand and gravel
- Qc/Tkd - Colluvium of Quaternary Age overlying the Dawson Formation of Tertiary to Cretaceous Age:**
sheetwash and residual soil deposits overlying arkosic sandstone with interbedded siltstone and claystone
- dsc - downslope creep
- pu - potentially unstable slopes
- fp - floodplain
- psw - potential seasonally shallow groundwater
- sw - seasonally shallow groundwater

- POTENTIAL BUILDING SITES

- APPROXIMATE TEST BORING LOCATION AND NUMBER
- APPROXIMATE TEST PIT LOCATION AND NUMBER





NOTES:

-GRAVEL SIZE IS RELATED TO DIAMETER OF PIPE PERFORATIONS-85% GRAVEL GREATER THAN 2x PERFORATION DIAMETER.

-PIPE DIAMETER DEPENDS UPON EXPECTED SEEPAGE. 4-INCH DIAMETER IS MOST OFTEN USED.

-ALL PIPE SHALL BE PERFORATED PLASTIC. THE DISCHARGE PORTION OF THE PIPE SHOULD BE NON-PERFORATED PIPE.

-FLEXIBLE PIPE MAY BE USED UP TO 8 FEET IN DEPTH, IF SUCH PIPE IS DESIGNED TO WITHSTAND THE PRESSURES. RIGID PLASTIC PIPE WOULD OTHERWISE BE REQUIRED.

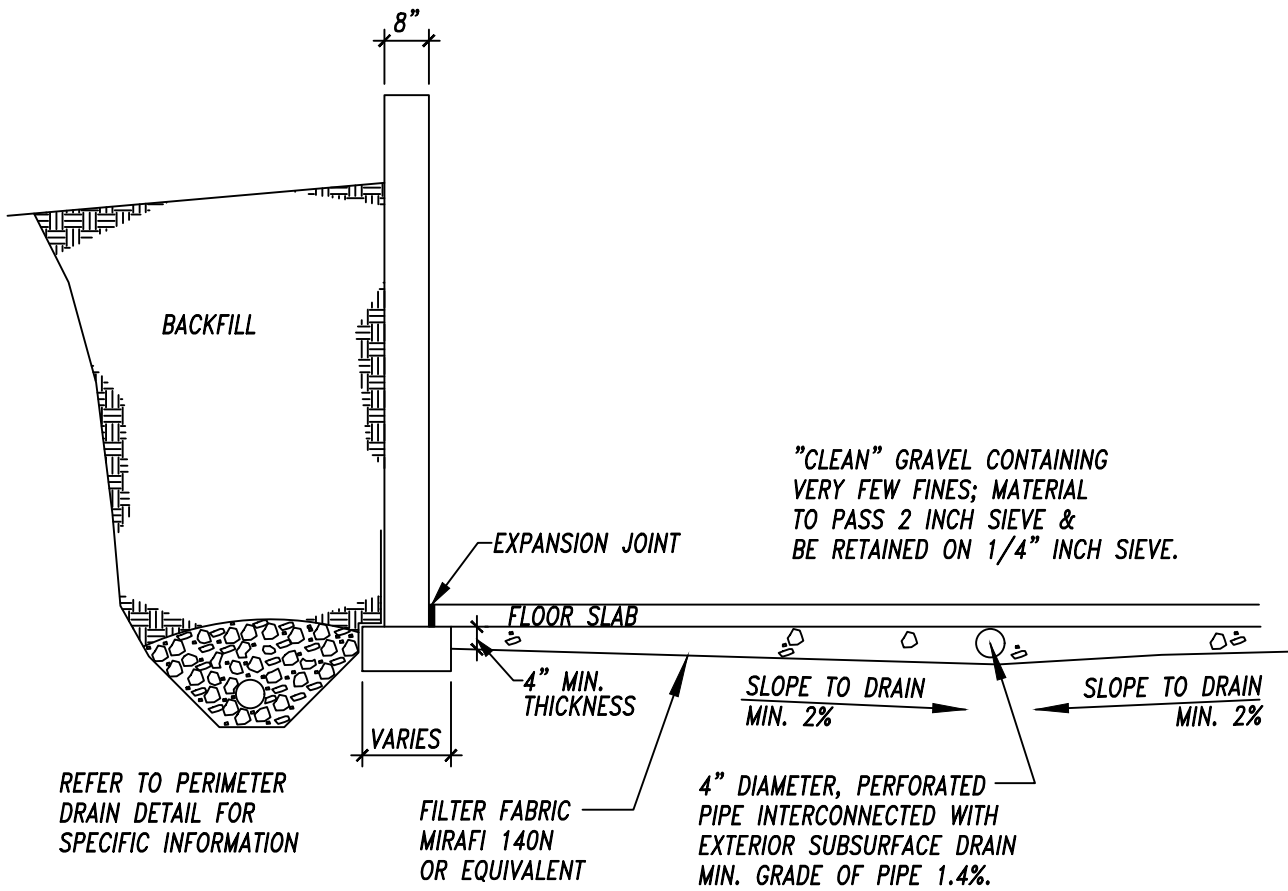
-MINIMUM GRADE FOR DRAIN PIPE TO BE 1% OR 3 INCHES OF FALL IN 25 FEET.

-DRAIN TO BE PROVIDED WITH A FREE GRAVITY OUTFALL, IF POSSIBLE. A SUMP AND PUMP MAY BE USED IF GRAVITY OUT FALL IS NOT AVAILABLE.



PERIMETER DRAIN DETAIL
 CALM BREEZE SUBDIVISION FILING NO. 1
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FIG. 5

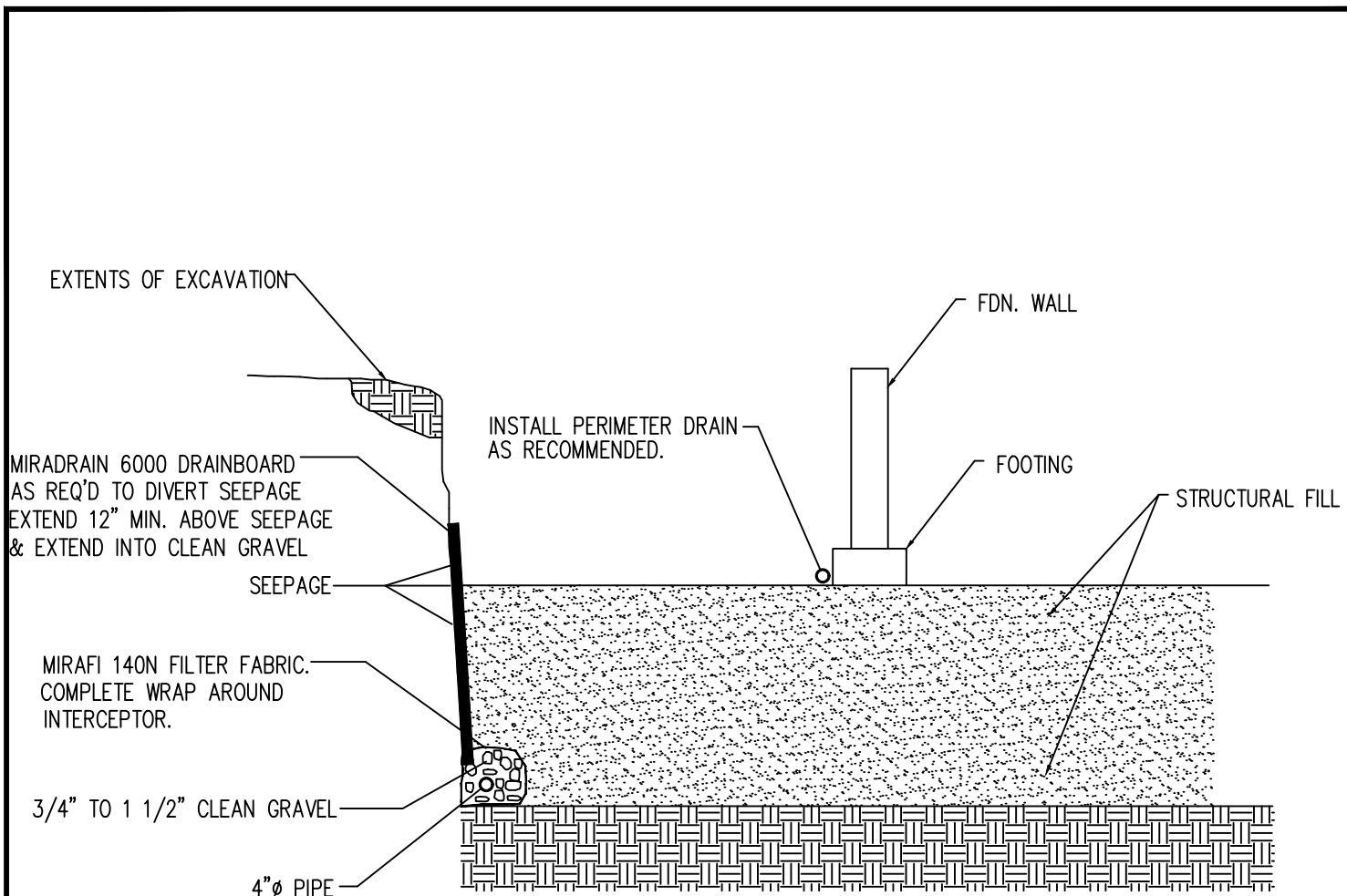


UNDERSLAB DRAINAGE LAYER

CALM BREEZE SUBDIVISION FILING NO. 1
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JOB NO.
260651

FIG. 6



NOTE:
 EXTEND INTERCEPTOR DRAIN TO UNDERDRAIN OR TO SUMP.
 BENCH DRAIN INTO NATIVE SOILS 12 INCHES MINIMUM.

INTERCEPTOR DRAIN DETAIL

N.T.S.

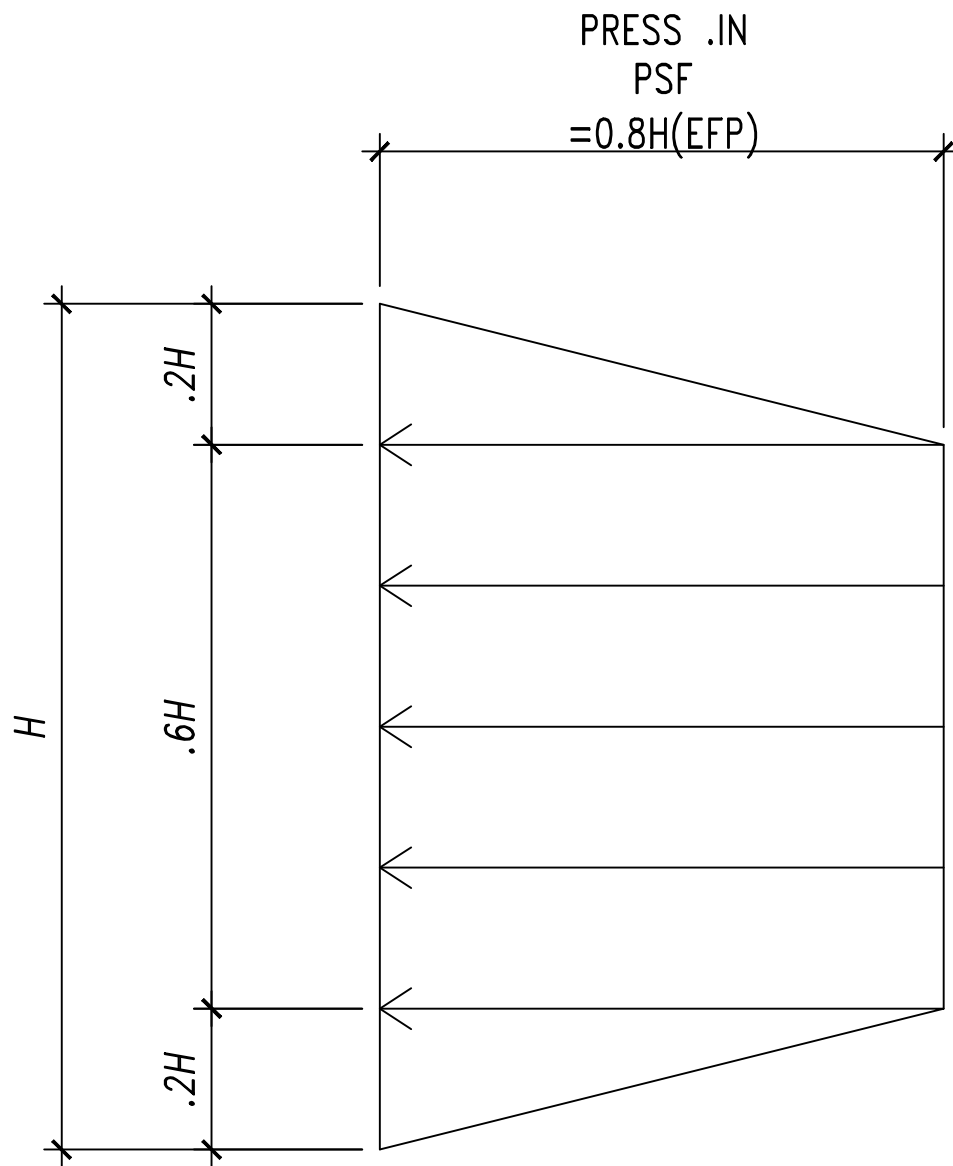


INTERCEPTOR DRAIN DETAIL

CALM BREEZE ESTATES FILING NO. 1
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 260651

FIG. 7



PRESSURE DISTRIBUTION



LATERAL PRESSURE DIAGRAM

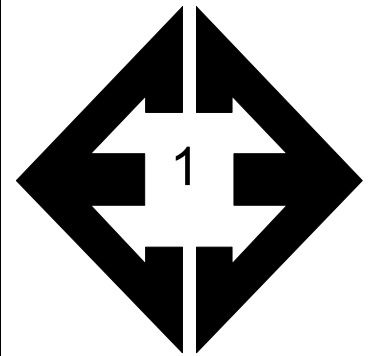
CALM BREEZE SUBDIVISION FILING NO. 1
M&S CIVIL CONSULTANTS

JOB NO.
260651

FIG. 8

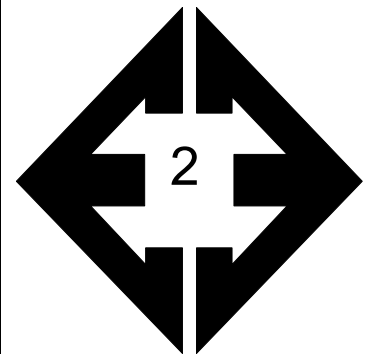


APPENDIX A: Site Photographs



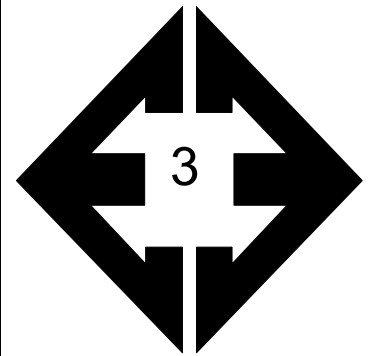
Looking north from the southern portion of proposed Lot 3.

April 29, 2026



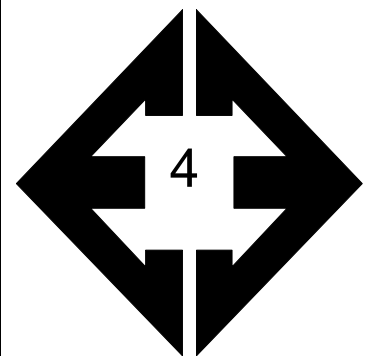
Looking east along Black Squirrel Creek from the southeastern portion of proposed lot 1.

April 29, 2026



**Looking northeast
from the northern
portion of proposed
Lot 3.**

April 29, 2026



**Looking east from the
central portion of
proposed Lot 1.**

April 29, 2026



APPENDIX B: Test Boring and Test Pit Logs

TEST BORING 1
DATE DRILLED 4/13/2026

TEST BORING 2
DATE DRILLED 4/13/2026

REMARKS

REMARKS

DRY TO 20', 4/13/26

DRY TO 20', 4/13/26

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
0-2	2" TOPSOIL				
2-5	SAND, CLAYEY, TAN, MEDIUM DENSE, MOIST		20	11.8	1
5-10	SANDSTONE, EXTREMELY WEAK, TAN, HIGHLY WEATHERED (SAND, SILTY, VERY DENSE, MOIST)		50	6.7	2
10-15			50 8"	6.6	2
15-20			50 7"	9.1	2
20			50 10"	15.0	2

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
0-5	SAND, SILTY, TAN, DENSE, MOIST		32	9.3	1
5-10	SANDSTONE, EXTREMELY WEAK, TAN, HIGHLY WEATHERED (SAND, SILTY, VERY DENSE, MOIST)		50 11"	10.6	2
10-15			50 10"	9.7	2
15-20			50 6"	13.1	2
20			50 8"	9.4	2



TEST BORING LOGS

SHOUP RD., PARCEL 52110-00-013
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JOB NO.
260651

FIG. B-1

TEST BORING 3
DATE DRILLED 4/13/2026

TEST BORING 4
DATE DRILLED 4/13/2026

REMARKS

REMARKS

DRY TO 20', 4/13/26

DRY TO 20', 4/13/26

2" TOPSOIL

2" TOPSOIL

SAND, SILTY, TAN, DENSE to VERY DENSE, MOIST

SAND, SILTY, TAN

SANDSTONE, EXTREMELY WEAK, TAN, HIGHLY WEATHERED (SAND, SILTY, VERY DENSE, MOIST)

SANDSTONE, EXTREMELY WEAK, TAN, HIGHLY WEATHERED (SAND, SILTY, VERY DENSE to DENSE, MOIST)

Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type	Depth (ft)	Symbol	Samples	Blows per foot	Watercontent %	Soil Type
0-2				1	0-2				1
2-5		43	9.8	1	2-5		50	3.9	2
5-10		50	10.7	1	5-10		10"	4.2	2
10-15		50	9.8	2	10-15		48	11.3	2
15-20		50	8.3	2	15-20		38	10.7	2
20-25		50	8.6	2	20-25		50	10.6	2
			10"						11"		
									10"		



TEST BORING LOGS

SHOUP RD., PARCEL 52110-00-013
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260651

FIG. B-2

TABLE B-1
DEPTH TO BEDROCK

TEST BORING	DEPTH TO BEDROCK (ft.)
1	4
2	3
3	7
4	1

TEST PIT 1
 DATE EXCAVATED 4/15/2026

TEST PIT 2
 DATE EXCAVATED 4/15/2026

REMARKS

REMARKS

REMARKS	Depth (ft.)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	Soil Type	REMARKS	Depth (ft.)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	Soil Type
topsoil, brown	1						topsoil, brown	1					
sandy loam with gravel, tan, moist	2			gr	w	2A	sandy loam with gravel, tan, moist	2			gr	w	2
	3						sandy clay loam with gravel, tan, moist	3			gr	m	3
	4						sandy loam with gravel, tan, moist	4					
loamy sand with gravel, tan, moist	5			gr	w	1		5			gr	s	2
	6							6					
	7						Refusal at 5'6" due to sandstone bedrock	7					
sandy clay loam, tan, moist	8			ma	sl	3A		8					
	9							9					
	10							10					

Soil Structure Shape

granular - gr
 platy - pl
 blocky - bl
 prismatic - pr
 single grain - sg
 massive - ma

Soil Structure Grade

weak - w
 moderate - m
 strong - s
 loose - l
 structureless - sl



TEST PIT LOGS

CALM BREEZE SUBDIVISION FILING NO. 1
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JOB NO.
 260651

FIG. B-3

TEST PIT 3
 DATE EXCAVATED 4/15/2026

TEST PIT 4
 DATE EXCAVATED 4/15/2026

REMARKS

REMARKS

Redoximorphic Features at 4'

sandy gravelly loam, tan to light brown, moist

sandy clay loam with gravel, olive, moist, redox at 4'

Refusal at 6' due to sandstone bedrock

Depth (ft.)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	Soil Type
1			gr	m	2
2			gr	m	2
3			gr	m	2
4			gr	s	3
5			gr	s	3
6					
7					
8					
9					
10					

topsoil, brown

sandy clay loam, tan, moist

loamy gravelly sand, tan, moist

sandy gravelly loam, tan, moist

Depth (ft.)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	Soil Type
1					
2			ma	sl	3A
3			gr	w	1
4			gr	m	2
5			gr	m	2
6					
7					
8					
9					
10					

Soil Structure Shape

- granular - gr
- platy - pl
- blocky - bl
- prismatic - pr
- single grain - sg
- massive - ma

Soil Structure Grade

- weak - w
- moderate - m
- strong - s
- loose - l
- structureless - sl



TEST PIT LOGS

CALM BREEZE SUBDIVISION FILING NO. 1
 M&S CIVIL CONSULTANTS

JOB NO.
 260651

FIG. B-4

APPENDIX C: Laboratory Test Results

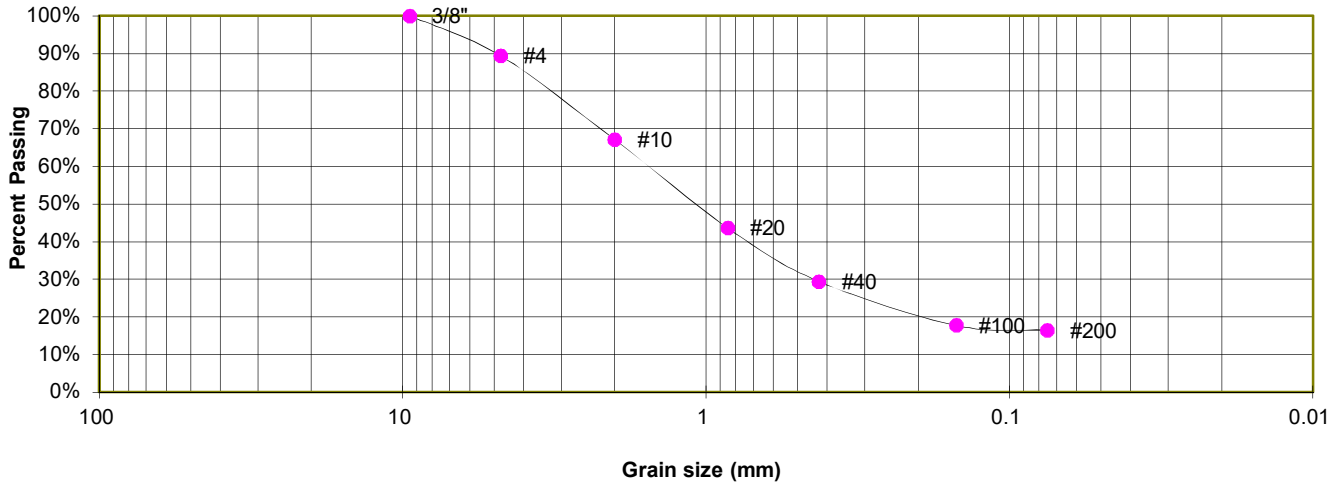
**TABLE C-1
SUMMARY OF LABORATORY TEST RESULTS**

SOIL TYPE	TEST BORING NO.	DEPTH (FT)	PASSING NO. 200 SIEVE (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTIC INDEX	SULFATE (WT %)	USCS	SOIL DESCRIPTION
1	1	2-3	16.5	29	21	8	0.00	SC	SAND, CLAYEY
1	3	5	13.6					SM	SAND, SILTY
2	2	10	19.9	30	23	7	<0.01	SM	SANDSTONE (SAND, SILTY)
2	4	5	10.6					SW-SM	SANDSTONE (SAND, WITH SILT)

TEST BORING 1
 DEPTH (FT) 2-3

SOIL DESCRIPTION SAND, CLAYEY
 SOIL TYPE 1

**Sieve Analysis
 Grain Size Distribution**



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	89.4%
10	67.1%
20	43.7%
40	29.4%
100	17.7%
200	16.5%

ATTERBERG LIMITS

Plastic Limit	21
Liquid Limit	29
Plastic Index	8

SOIL CLASSIFICATION

USCS CLASSIFICATION: SC



LABORATORY TEST RESULTS

SHOUP RD., PARCEL 52110-00-013
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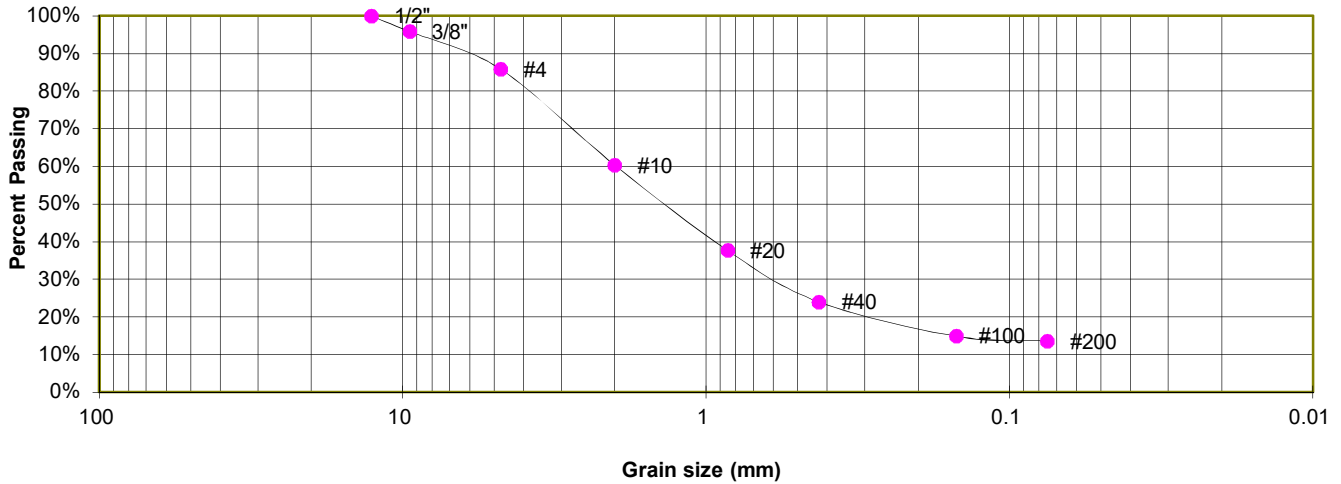
JOB NO.
 260651

FIG. C-1

TEST BORING 3
 DEPTH (FT) 5

SOIL DESCRIPTION SAND, SILTY
 SOIL TYPE 1

**Sieve Analysis
 Grain Size Distribution**



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	95.9%
4	85.8%
10	60.4%
20	37.7%
40	24.0%
100	14.9%
200	13.6%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SM



LABORATORY TEST RESULTS

SHOUP RD., PARCEL 52110-00-013
 M&S CIVIL CONSULTANTS

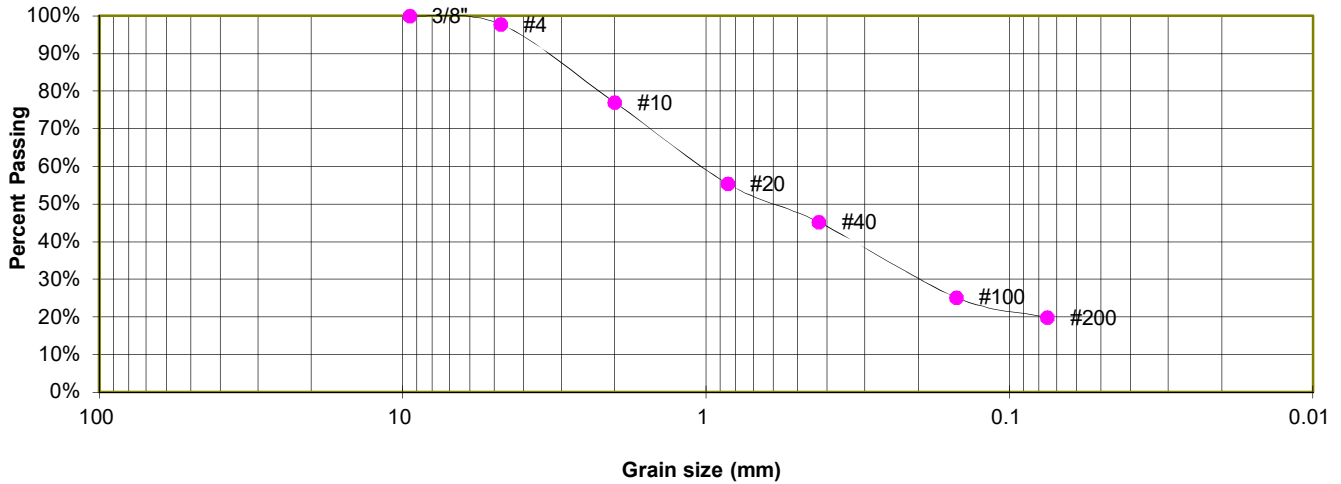
JOB NO.
 260651

FIG. C-2

TEST BORING 2
 DEPTH (FT) 10

SOIL DESCRIPTION SANDSTONE (SAND, SILTY)
 SOIL TYPE 2

**Sieve Analysis
 Grain Size Distribution**



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	97.8%
10	77.0%
20	55.4%
40	45.2%
100	25.2%
200	19.9%

ATTERBERG LIMITS

Plastic Limit	23
Liquid Limit	30
Plastic Index	7

SOIL CLASSIFICATION

USCS CLASSIFICATION: SM



LABORATORY TEST RESULTS

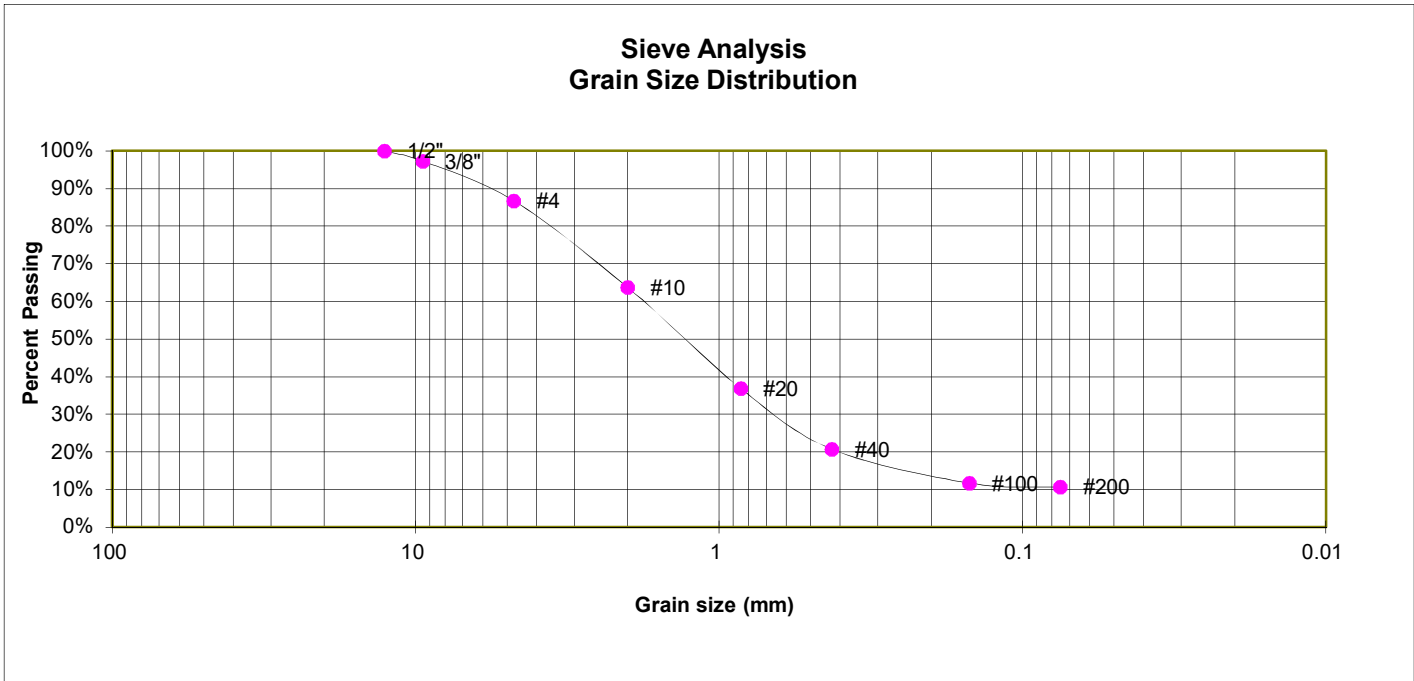
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 260651

FIG. C-3

TEST BORING 4
 DEPTH (FT) 5

SOIL DESCRIPTION SANDSTONE (SAND, WITH SILT)
 SOIL TYPE 2



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	97.3%
4	86.8%
10	63.7%
20	36.9%
40	20.7%
100	11.8%
200	10.6%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SW-SM



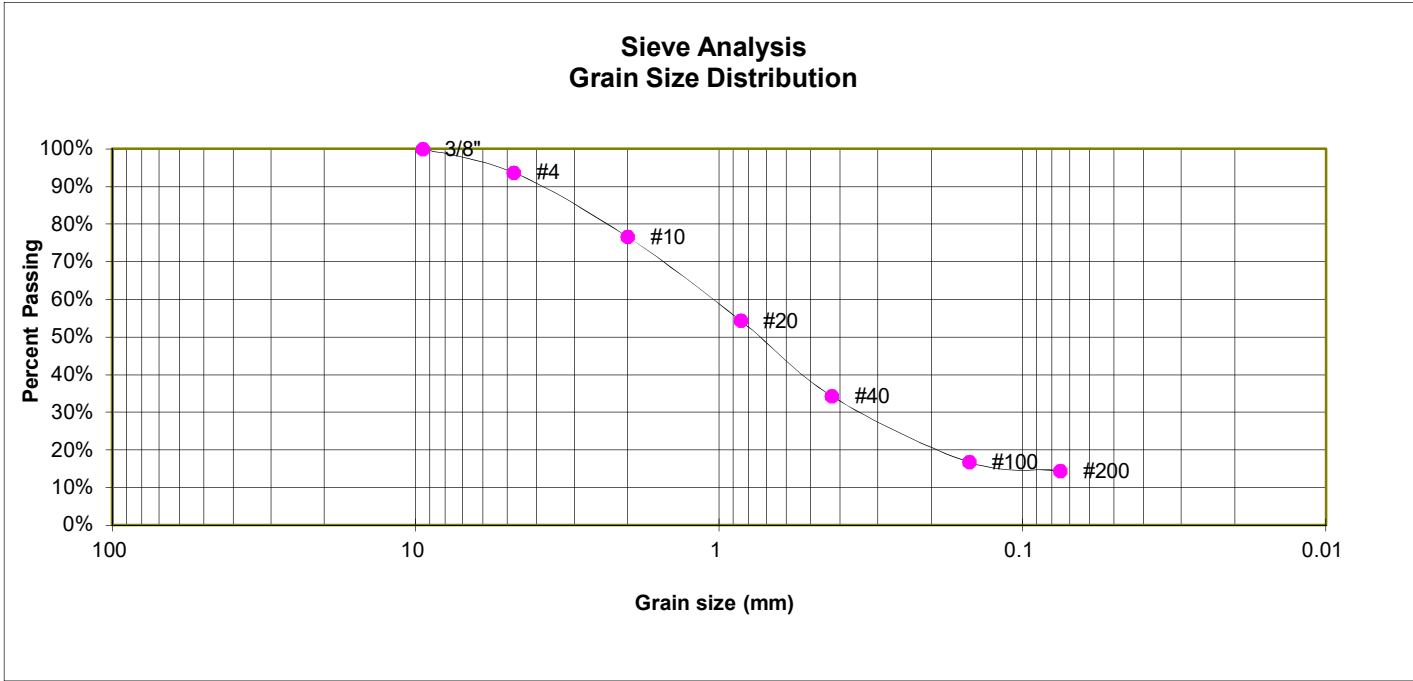
LABORATORY TEST RESULTS

SHOUP RD., PARCEL 52110-00-013
 M&S CIVIL CONSULTANTS

JOB NO.
 260651

FIG. C-4

TEST PIT	TP-1	SOIL DESCRIPTION SAND, SILTY
DEPTH (FT)	18-48"	SOIL TYPE # 1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	93.8%
10	76.6%
20	54.4%
40	34.3%
100	16.7%
200	14.5%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SM



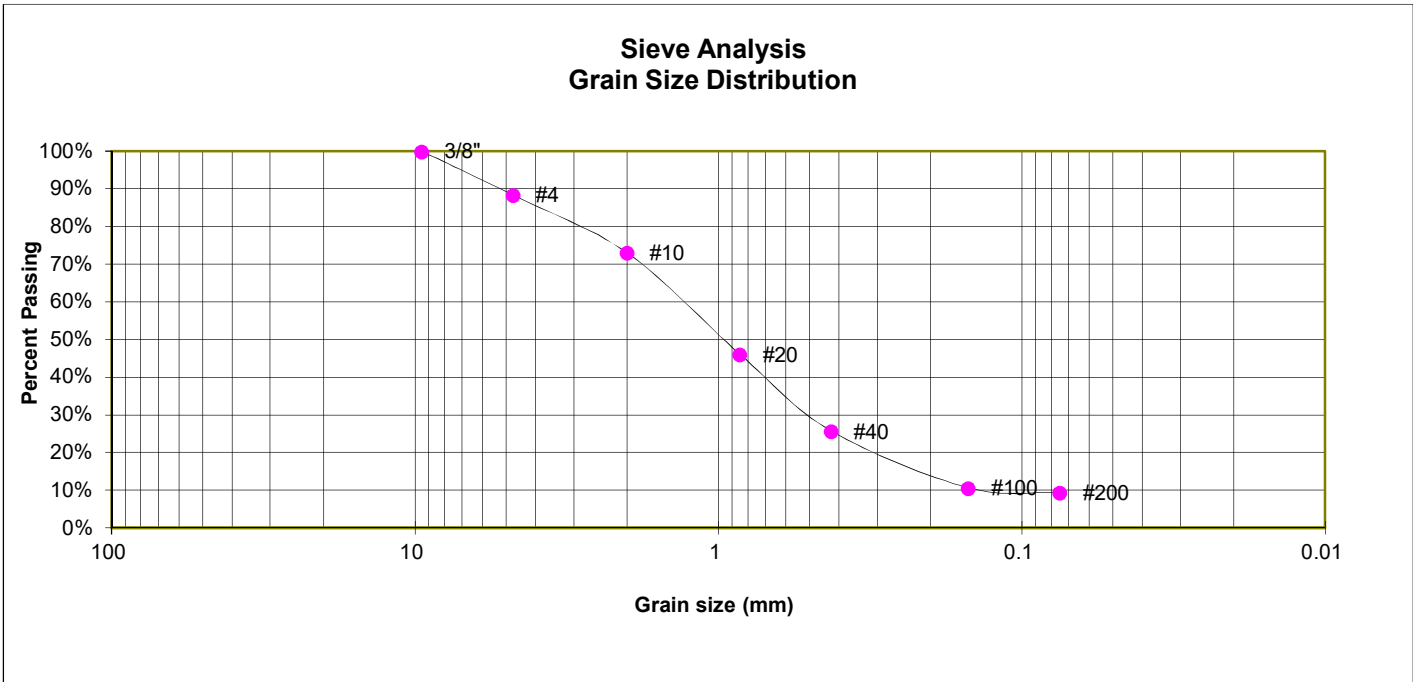
LABORATORY TEST RESULTS

SHOUP ROAD
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260651

FIG. C-5

TEST PIT	TP-1	SOIL DESCRIPTION SAND, GRAVELLY, WITH SILT
DEPTH (FT)	48-84"	SOIL TYPE # 1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	88.3%
10	73.1%
20	46.1%
40	25.7%
100	10.6%
200	9.3%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SW-SM



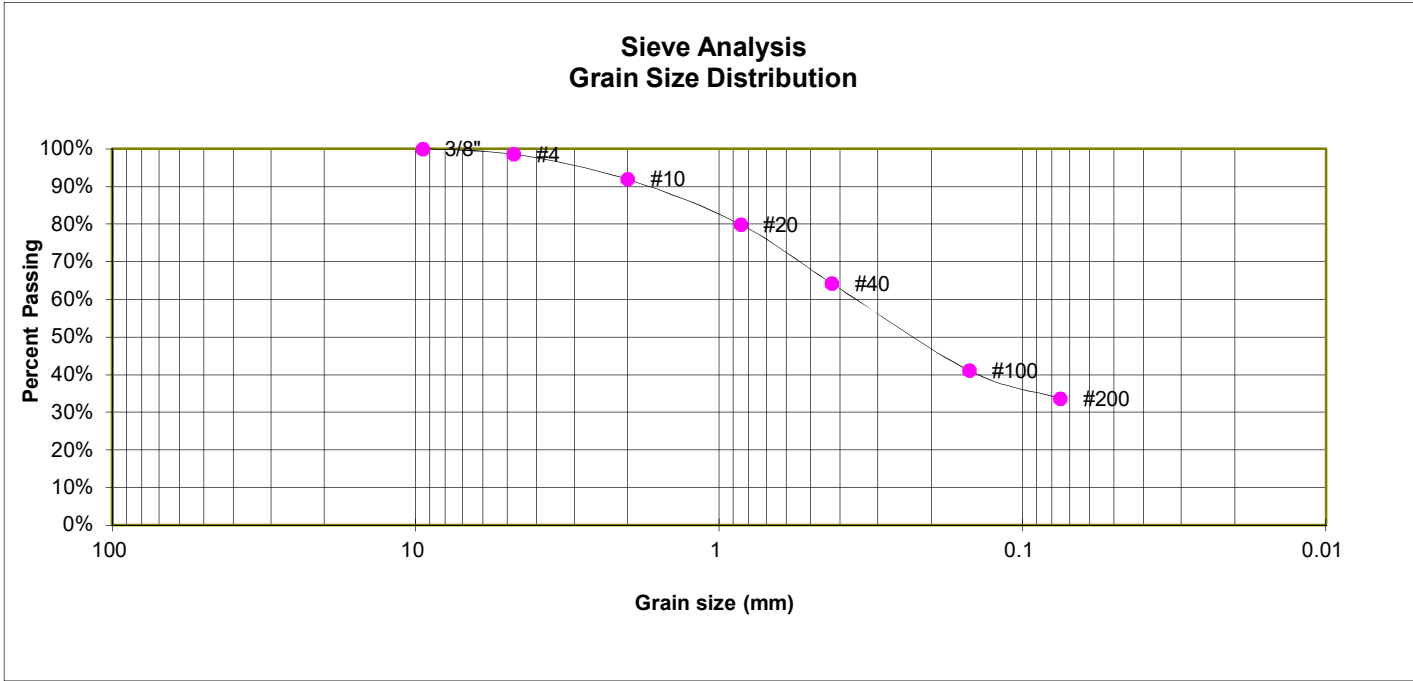
LABORATORY TEST RESULTS

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FIG. C-6

TEST PIT	TP-1	SOIL DESCRIPTION SAND, CLAYEY
DEPTH (FT)	84-96"	SOIL TYPE # 4



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	98.6%
10	91.9%
20	79.9%
40	64.3%
100	41.1%
200	33.7%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SC



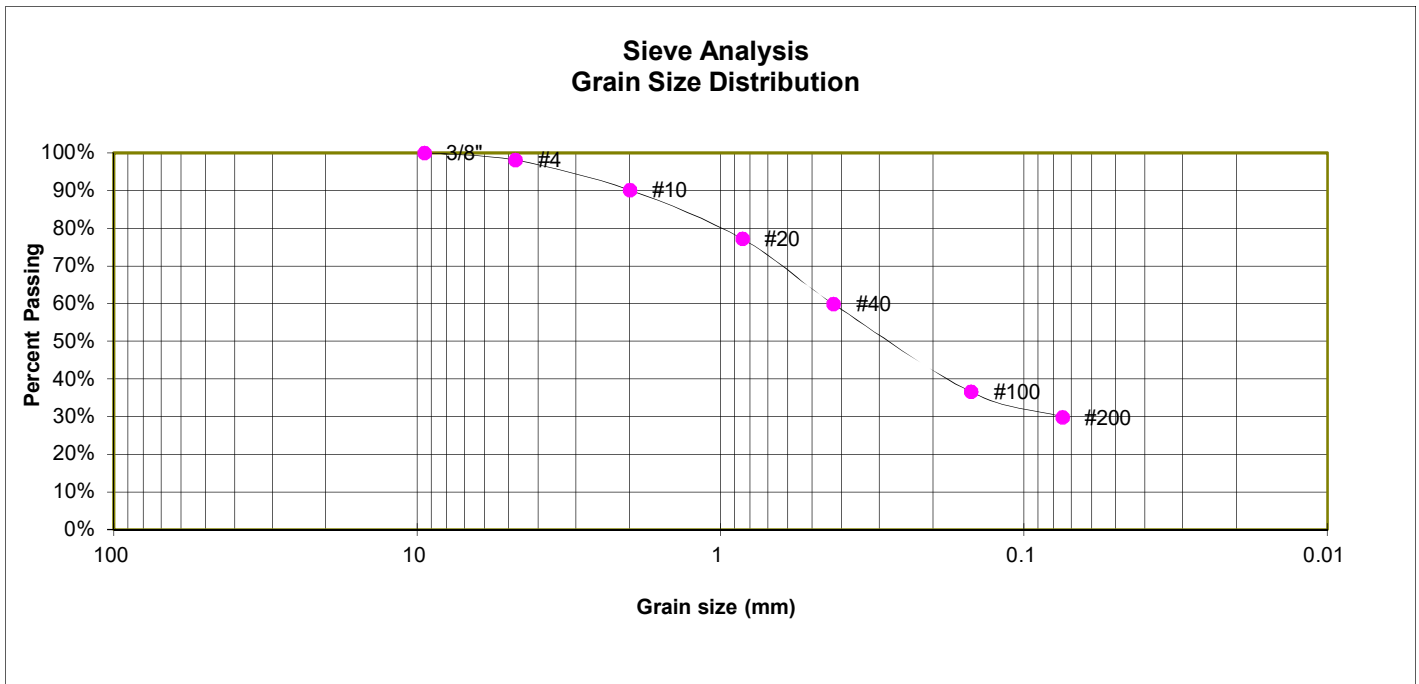
LABORATORY TEST RESULTS

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FIG. C-7

TEST PIT	TP-2	SOIL DESCRIPTION SAND, SILTY
DEPTH (FT)	6-18"	SOIL TYPE # 2



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	98.1%
10	90.1%
20	77.2%
40	59.9%
100	36.6%
200	29.9%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SM



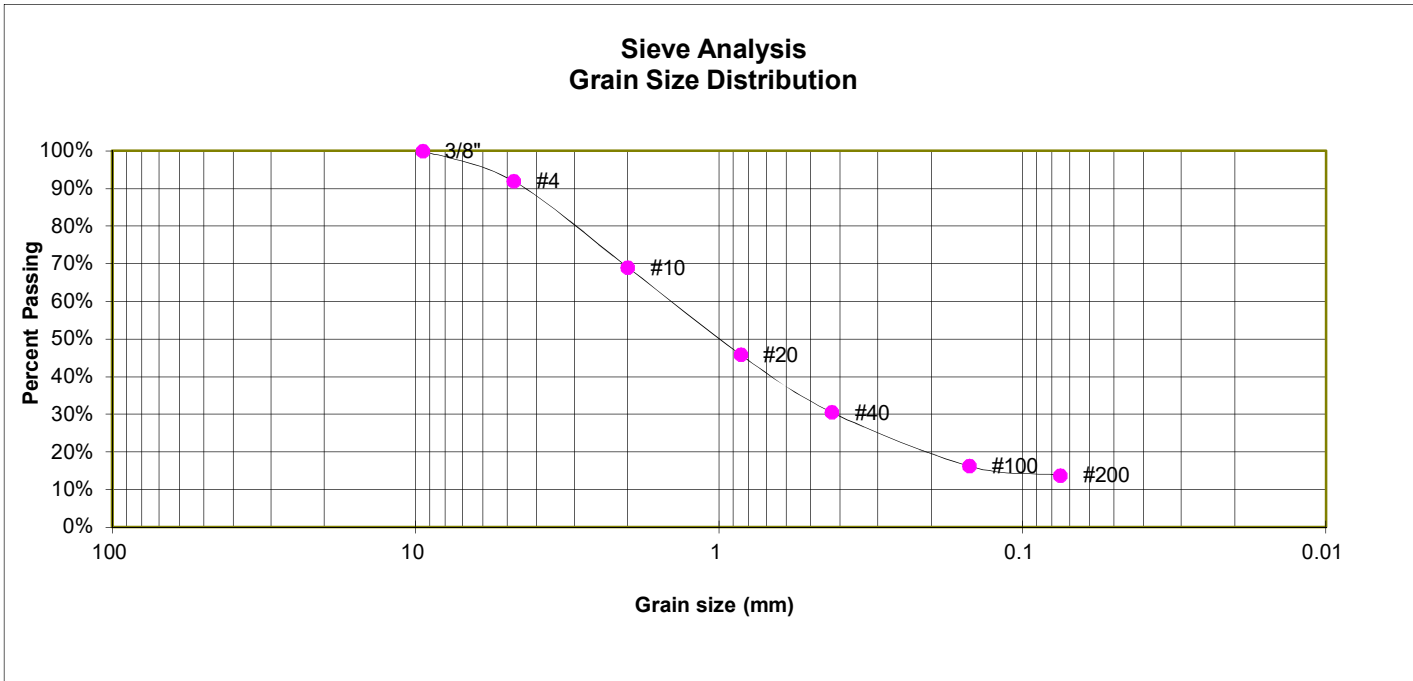
LABORATORY TEST RESULTS

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FIG. C-8

TEST PIT	TP-2	SOIL DESCRIPTION SANDSTONE (SAND, SILTY)
DEPTH (FT)	18-36"	SOIL TYPE # 1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	91.9%
10	69.1%
20	45.9%
40	30.5%
100	16.2%
200	13.8%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SM



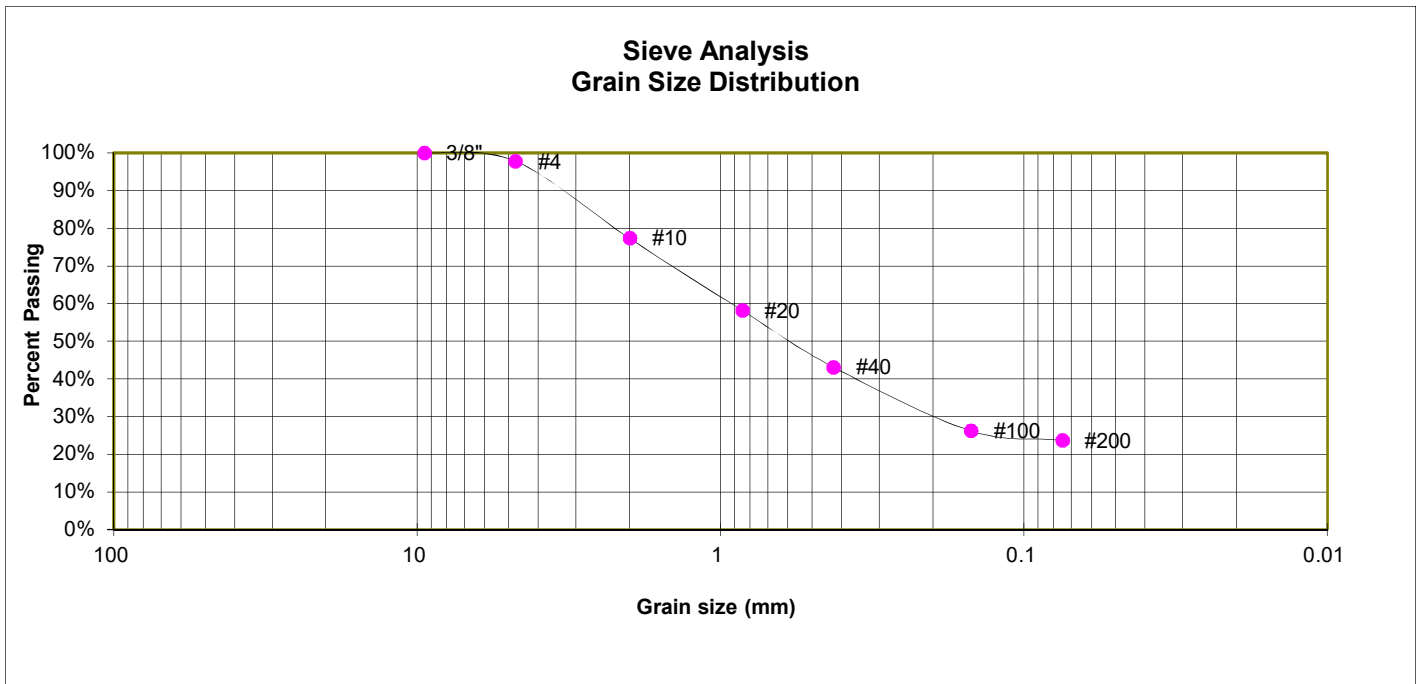
LABORATORY TEST RESULTS

SHOUP ROAD
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FIG. C-9

TEST PIT	TP-2	SOIL DESCRIPTION SANDSTONE (SAND, SILTY)
DEPTH (FT)	36-66"	SOIL TYPE # 1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	97.8%
10	77.3%
20	58.2%
40	43.1%
100	26.2%
200	23.7%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SM



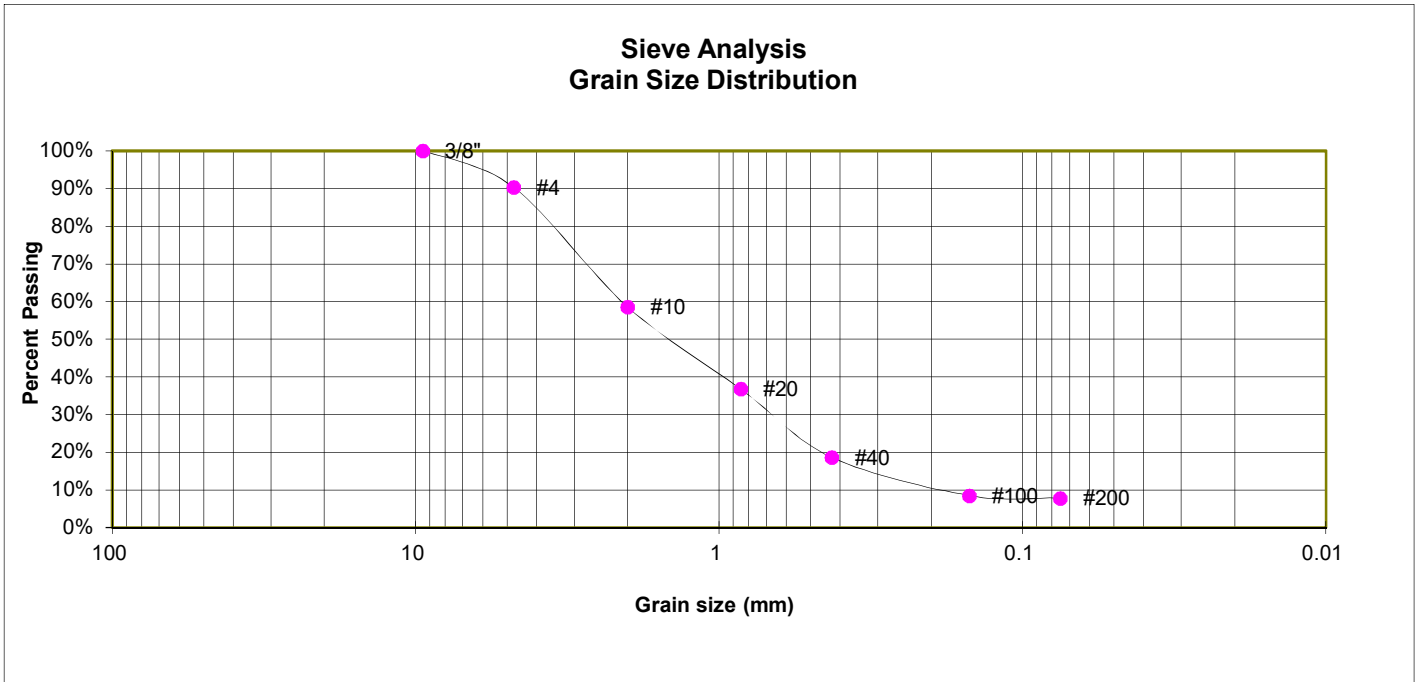
LABORATORY TEST RESULTS

SHOUP ROAD
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FIG. C-10

TEST PIT	TP-3	SOIL DESCRIPTION SANDSTONE (SAND, WITH SILT)
DEPTH (FT)	0-18"	SOIL TYPE # 1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	90.3%
10	58.5%
20	36.8%
40	18.6%
100	8.5%
200	7.8%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SW-SM



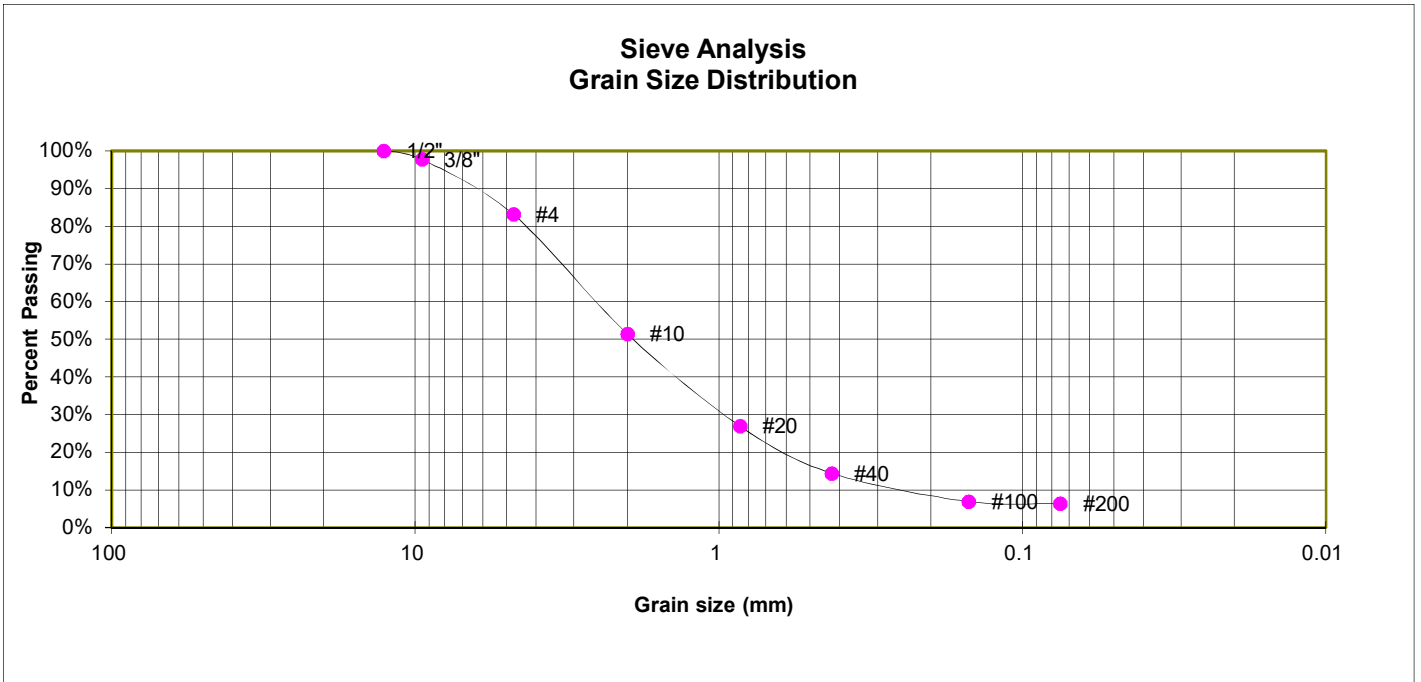
LABORATORY TEST RESULTS

SHOUP ROAD
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JOB NO.
260651

FIG. C-11

TEST PIT	TP-3	SOIL DESCRIPTION SANDSTONE (SAND, WITH SILT)
DEPTH (FT)	18-42"	SOIL TYPE # 1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	97.7%
4	83.1%
10	51.4%
20	26.9%
40	14.4%
100	6.9%
200	6.3%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SW-SM



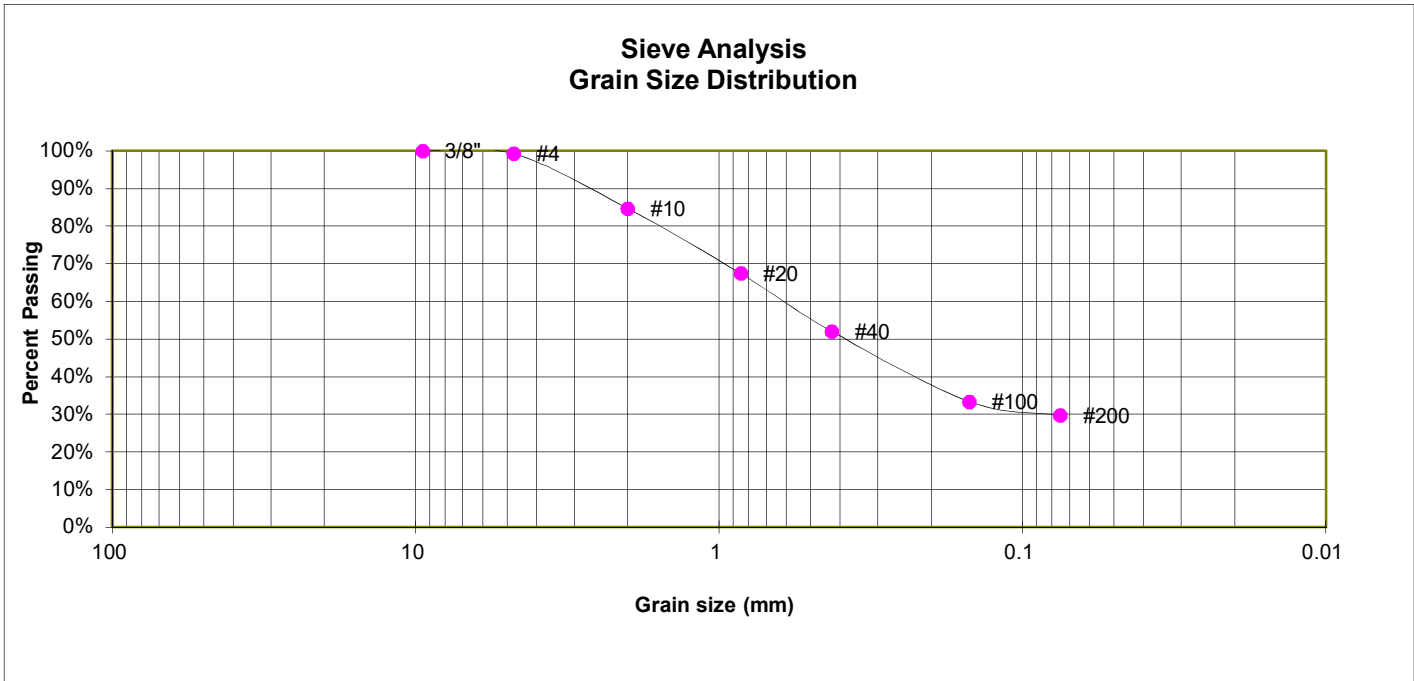
LABORATORY TEST RESULTS

SHOUP ROAD
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JOB NO.
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FIG. C-12

TEST PIT	TP-3	SOIL DESCRIPTION SANDSTONE (SAND, CLAYEY)
DEPTH (FT)	48-60"	SOIL TYPE # 2



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.2%
10	84.7%
20	67.4%
40	52.0%
100	33.4%
200	29.8%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SC



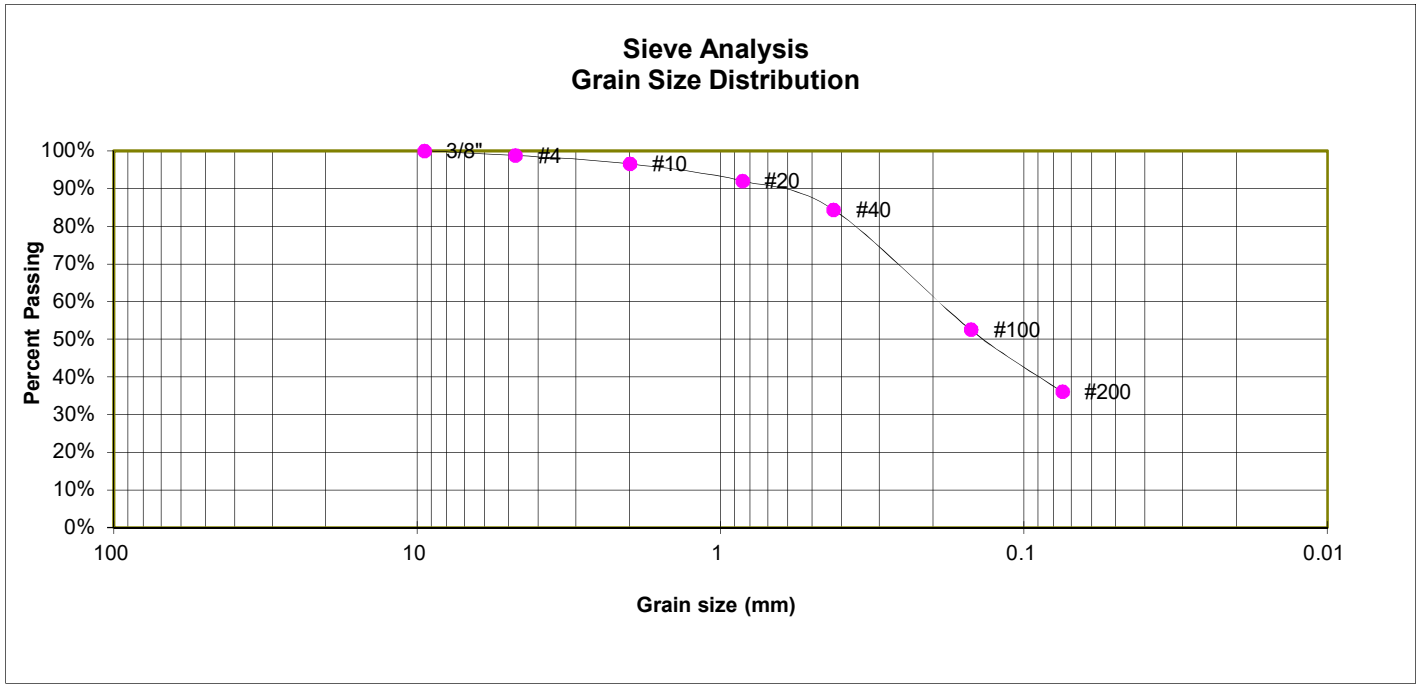
LABORATORY TEST RESULTS

SHOUP ROAD
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FIG. C-13

TEST PIT	TP-4	SOIL DESCRIPTION SAND, CLAYEY
DEPTH (FT)	12-36"	SOIL TYPE # 2



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	98.8%
10	96.6%
20	92.1%
40	84.4%
100	52.5%
200	36.1%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SC



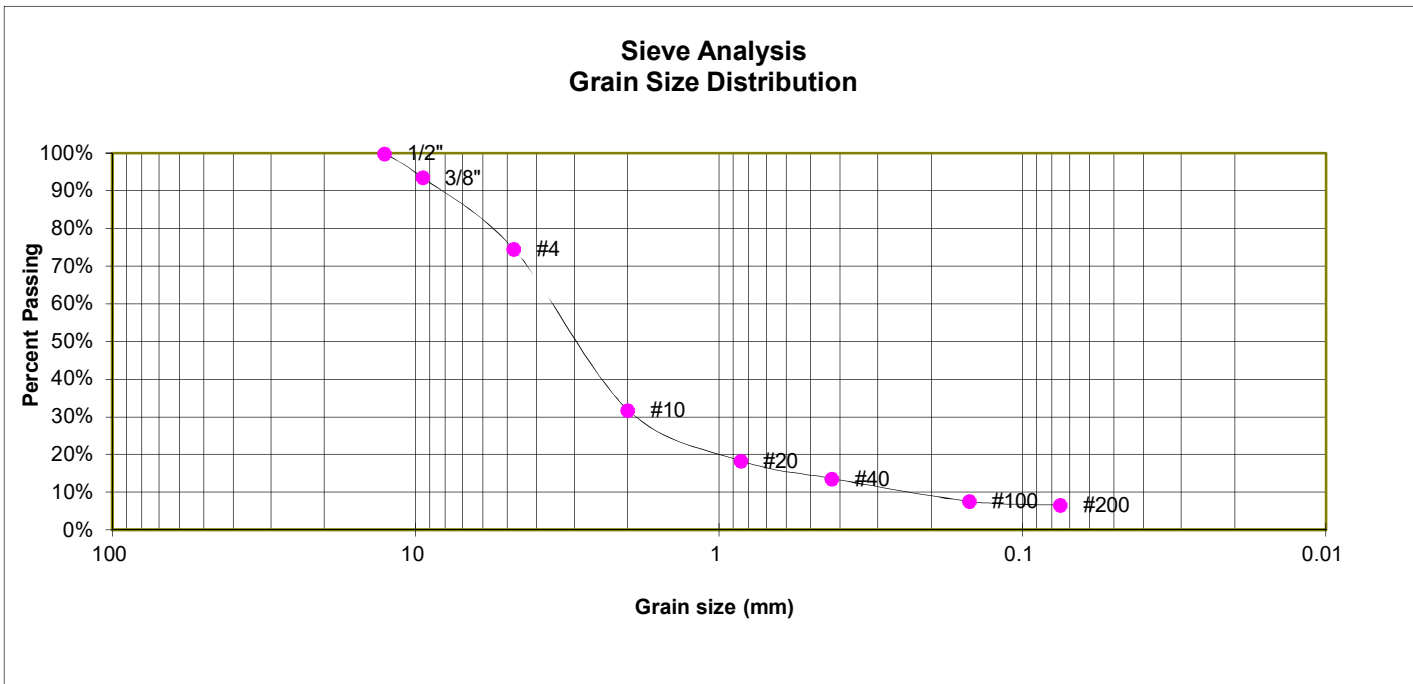
LABORATORY TEST RESULTS

SHOUP ROAD
M&S CIVIL CONSTRUCTION

JOB NO.
260651

FIG. C-14

TEST PIT	TP-4	SOIL DESCRIPTION SAND, WITH SILT
DEPTH (FT)	36-48"	SOIL TYPE # 1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	100.0%
3/8"	93.6%
4	74.5%
10	31.8%
20	18.4%
40	13.6%
100	7.6%
200	6.6%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SP-SM



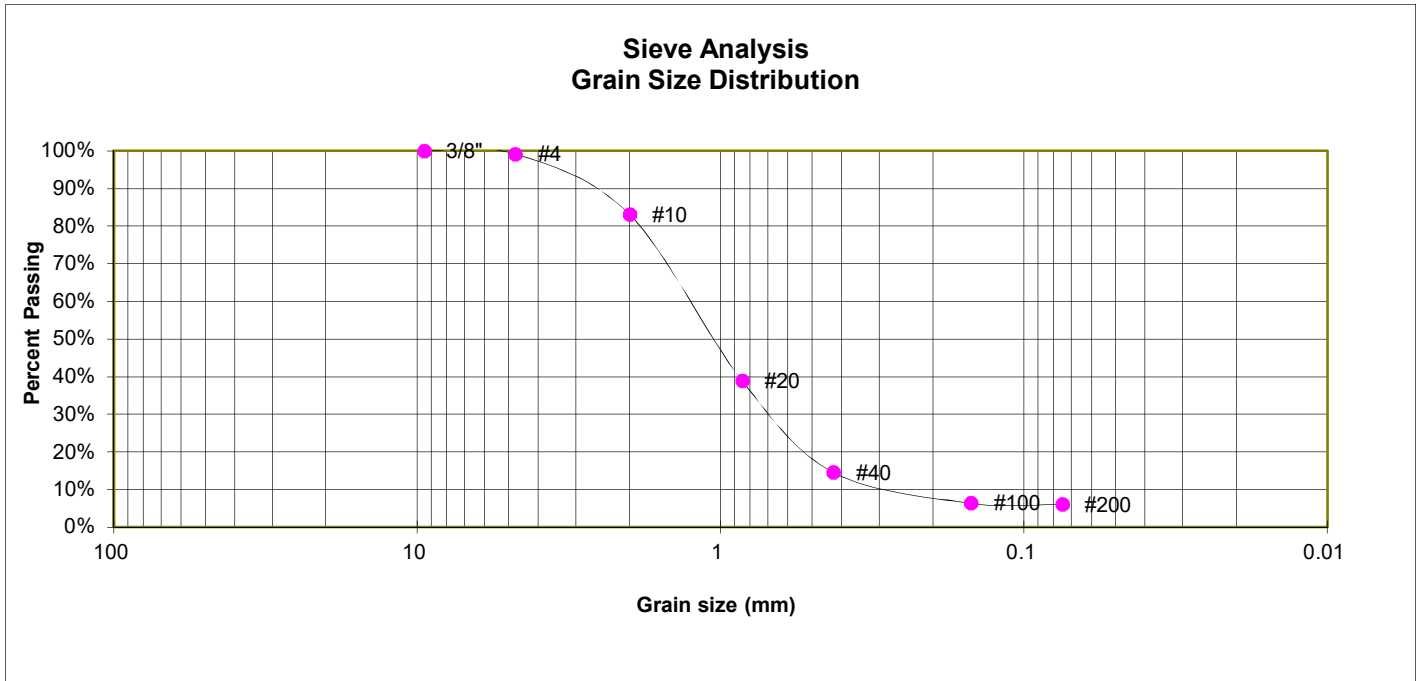
LABORATORY TEST RESULTS

SHOUP ROAD
M&S CIVIL CONSTRUCTION

JOB NO.
260651

FIG. C-15

TEST PIT	TP-4	SOIL DESCRIPTION SANDSTONE (SAND, WITH SILT)
DEPTH (FT)	48-96"	SOIL TYPE # 1



GRAIN SIZE ANALYSIS

U.S. Sieve #	Percent Finer
3"	
1 1/2"	
3/4"	
1/2"	
3/8"	100.0%
4	99.2%
10	83.2%
20	38.9%
40	14.6%
100	6.4%
200	6.1%

SOIL CLASSIFICATION

USCS CLASSIFICATION: SP-SM



LABORATORY TEST RESULTS

SHOUP ROAD
M&S CIVIL CONSTRUCTION

JOB NO.
260651

FIG. C-16



APPENDIX D: USDA Soil Survey Descriptions

El Paso County Area, Colorado

26—Elbeth sandy loam, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 367y
Landscape: Uplands
Elevation: 7,300 to 7,600 feet
Farmland classification: Not prime farmland

Map Unit Composition

Elbeth and similar soils: 85 percent
Minor components: 15 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Elbeth

Setting

Landscape: Uplands
Landform: Hills
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium derived from arkose

Typical profile

A - 0 to 3 inches: sandy loam
E - 3 to 23 inches: loamy sand
Bt - 23 to 68 inches: sandy clay loam
C - 68 to 74 inches: sandy clay loam

Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat):
Moderately high (0.20 to 0.60 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Moderate (about 7.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 4e
Hydrologic Soil Group: B
Ecological site: F048AY908CO - Mixed Conifer
Hydric soil rating: No



Minor Components

Other soils

Percent of map unit: 10 percent

Hydric soil rating: No

Pleasant

Percent of map unit: 5 percent

Landform: Depressions

Hydric soil rating: Yes

Data Source Information

Soil Survey Area: El Paso County Area, Colorado

Survey Area Data: Version 23, Aug 29, 2025



El Paso County Area, Colorado

41—Kettle gravelly loamy sand, 8 to 40 percent slopes

Map Unit Setting

National map unit symbol: 368h

Landscape: Uplands

Elevation: 7,000 to 7,700 feet

Farmland classification: Not prime farmland

Map Unit Composition

Kettle and similar soils: 85 percent

Minor components: 15 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Kettle

Setting

Landscape: Uplands

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear

Across-slope shape: Linear

Parent material: Sandy alluvium derived from arkose

Typical profile

E - 0 to 16 inches: gravelly loamy sand

Bt - 16 to 40 inches: gravelly sandy loam

C - 40 to 60 inches: extremely gravelly loamy sand

Properties and qualities

Slope: 8 to 40 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Somewhat excessively drained

Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): High
(2.00 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 3.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 7e

Hydrologic Soil Group: B

Ecological site: F048AY908CO - Mixed Conifer

Hydric soil rating: No

Minor Components

Other soils

Percent of map unit: 10 percent

Hydric soil rating: No

Pleasant

Percent of map unit: 5 percent

Landform: Depressions

Hydric soil rating: Yes

Data Source Information

Soil Survey Area: El Paso County Area, Colorado

Survey Area Data: Version 23, Aug 29, 2025