MONUMENT GLAMPING OWTS 18045 HWY 83, COLORADO SPRINGS, CO 80908

<u>OWNER:</u>

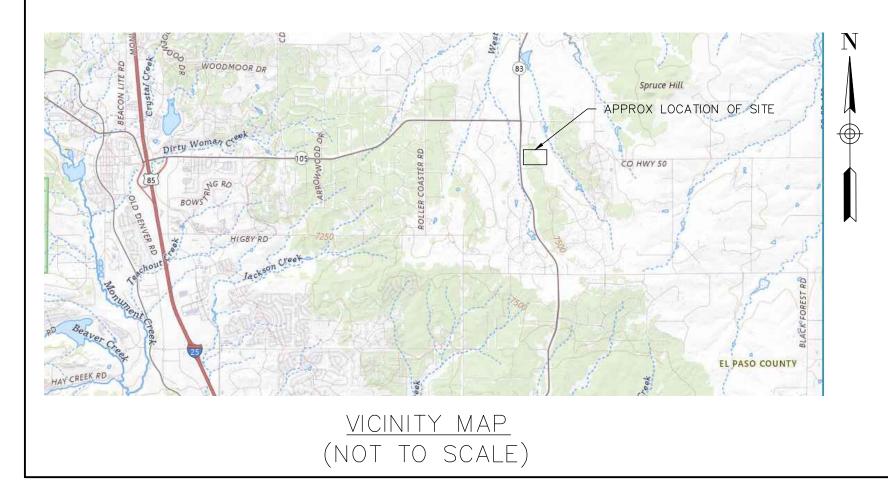
CHRIS JEUB MONUMENT RIDGE LTD, LLC (719) 660-5781

PROJECT ENGINEER: ALEX EWERS, P.E. 53341 (719) 430-5333 alexe@3rocksengineering.com

<u>SCOPE OF WORK:</u> The scope of work in these plans includes a design for an OWTS (On-Site Wastewater Treatment System) for an existing 3-bedroom residence and 24 glamping sites.

			SOIL (DBSERVATION PIT # 1				
DATE OF OBSERVATION	7/30/2024					NOTES		CONTENT BY VISUAL SERVATION
COORDINATES /LOCATION		SLOPE		LANDSCAPE POSITION		SOILS EVALUATOR	MORGAN HEGGIE, SEPTEMBER 2022 CPOW CERTIFIED	
DEPTH (IN)	SOIL TYPE, TEXTURE & LTAR (GAL PER DAY PER SQ FT)	PRESENCE OF R TYPE SOILS (ROCKY SOILS)	MATRIX COLOR(S)	MOTTLE COLOR(S)	REDOX KIND(S)	STRUCTURE SHAPE	STRUCTURE GRADE	CONSISTENCE
0-6	TOPSOIL	NONE						
6-48	2A, SANDY LOAM, 0.5	NONE			NONE	MASSIVE		LOOSE
48-96	2A, SANDY LOAM, 0.5	NONE			NONE	MASSIVE		LOOSE
		1	1			1		

			SOIL O	BSERVATION PIT # 2				
DATE OF OBSERVATION	7/30/2024					NOTES:	NO ROCK CONTEI OBSERV/	
COORDINATES /LOCATION		SLOPE		LANDSCAPE POSITION		SOILS EVALUATOR	MORGAN HEGGIE, SEPTEMBER 2022 CPOW CERTIFIED	
DEPTH (IN)	SOIL TYPE, TEXTURE & LTAR (GAL PER DAY PER SQ FT)	PRESENCE OF R TYPE SOILS (ROCKY SOILS)	MATRIX COLOR(S)	MOTTLE COLOR(S)	REDOX KIND(S)	STRUCTURE SHAPE	STRUCTURE GRADE	CONSISTENCE
0-6	TOPSOIL	NONE						
6-48	2A, SANDY LOAM, 0.5	NONE			NONE	MASSIVE		LOOSE
48-96	2A, SANDY LOAM, 0.5	NONE			NONE	MASSIVE		LOOSE



 $\frac{ABBRE VIATIONS:}{APPROX} = APPROXIMATE$ ASPH = ASPHALTBLDG = BUILDINGCONC = CONCRETEC&G= CURB AND GUTTER Ø=DIAMFTFR E = EASTINGFI = FI F VATIONEX = EXISTINGGPD= GALLONS PER DAY GPH= GALLONS PER HOUR IRR = IRRIGATIONN = NORTHINGNDDS= NON-PRESSURIZED DRIP DISPERSAL SYS O.C.=ON CENTER SPACING OWTS= ON-SITE WASTEWATER TREATMENT SYST PL = PROPERTY LINE PROP = PROPOSEDPVC = POLYVINYL CHLORIDE PIPEROW = RIGHT OF WAYSCH= SCHEDULE SDR= STANDARD DIMENSIONAL RATIO SQ.FT= SQUARE FEET TP=SOIL OBSERVATION TEST PIT TYP = TYPICALW/= WITH *NOT ALL ABBREVIATIONS

MAY BE PRESENT IN THIS DRAWING

100% CONSTRUCTION DOCUMENTS September 6, 2024

DESIGN CRITERIA

SERVICED LOCATIONS: 24 PROPOSED GLAMPING SITES AND EXISTING RESIDENCE TYPE: PRESSURIZED TRENCH SYSTEM

DESIGN FLOW AND TANKAGE:

SYSTEM - 1,650 GPD GLAMPING SITES ARE RATED AT 50 GPD PER GLAMPING TENT (50 GPD PER CAMPGROUND CAMPSITE) ACCORDING TO TABLE 6-2, AND A SINGLE FAMILY 3-BEDROOM RESIDENCE IS RATED AT 450 GPD. TANKAGE: (1) 2,000 GAL SEPTIC TANK, (1) 1,500 GAL SEPTIC TANK, AND (1) 1,000 GAL TANK AS A DISCHARGE TANK.

AN OPERATIONS AND MAINTENANCE IS REQUIRED BY EL PASO COUNTY TO MONITOR THAT

SEPTIC TANK EFFLUENT QUALITY MEETS THE REG 43 DEFINITION FOR TL1 WASTE WATER

<u>APPLIED_LTAR:</u> 0.5 GPD/SQ. FT. FOR TL1 WASTEWATER

SIZING ADJUSTMENTS: 0.7 FOR CHAMBERS 0.8 FOR PRESSURIZED TRENCH SYSTEM

TOTAL REQUIRED INFILTRATIVE SURFACE: 1848 SQ. FT. 160 QUICK 4 CHAMBERS

ADDITIONAL REMARKS:

CONSISTENCE

LOOSE	
LOOSE	

	SHEET INDEX
NUMBER	TITLE
C1	COVER
C2	EX CONDITIONS & DEMO PLAN
C3	SITE PLAN
C4	OWTS DESIGN AND DETAILS
C5	TANK DETAILS

LINE	TYPE	KEY:

SEPARATION REQUIREMENTS ACCORDING TO TABLE 7-1 OF EL PASO COUNTY OWTS REGULATIONS					
	WELL, OR POTABLE WATER CISTERN	POTABLE WATER LINES	STRUCTURE WITH BASEMENT, CRAWL SPACE, OR FOOTING DRAINS	STRUCTURE W/OUT BASEMENT, CRAWL SPACE, OR FOOTING DRAINS	PROPERTY LINES
STA TRENCH	100	5	20	10	10
SEWER OR EFFLUENT LINES	50	5	0	0	10
SEPTIC TANK	50	10	5	5	10

	SHEET INDEX
NUMBER	TITLE
C1	COVER
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1.6.



1.3. THESE PLANS ARE THE RESULT OF INTERPRETATION OF THE ENGINEER'S ANALYSIS OF THE EXISTING SOIL CONDITIONS AS FOUND ON SITE FROM THE OPEN HOLE INSPECTION AND SUBSEQUENT ANALYSIS OF THE SOIL SAMPLES. FAILURE TO THOROUGHLY FOLLOW PLANS, ENGINEER'S RECOMMMENDATIONS, MANUFACTURER'S GUIDELINES FOR TANK INSTALLATION, AND THE EL PASO COUNTY OWTS REGULATIONS CAN RESULT IN PREMATURE SOIL TREATMENT AREA FAILURE. POOR CONTRACTOR PRACTICES, ABUSE OF SYSTEM, OR FAILURE TO PERFORM SCHEDULED MAINTENANCE CAN CAUSE PREMATURE FAILURE OF SOIL TREATMENT AREA. 1.4. THESE PLANS ARE NOT A CONSTRUCTION MANUAL. FOR SUCCESSFUL SEPTIC SYSTEM INSTALLATION CONTRACTOR MUST REVIEW THE EL PASO COUNTY OWTS REGULATIONS, BE CERTIFIED FOR INSTALLATION, AND FOLLOW GENERAL CONTRACTOR BEST PRACTICES.

1.5. CONTACT ENGINEER IN CASE OF ANY UNCLEAR INFORMATION IN THESE PLANS. CONTRACTOR IS RESPONSIBLE FOR READING THE ENTIRETY OF THE PLANS. FAILURE TO UNDERSTAND THE SCOPE OF THE PROJECT WILL NOT RESULT IN SUFFICIENT GROUNDS FOR CUTTING CORNERS AND POOR INSTALLATION PRACTICES. CONSTRUCTION THAT DOES NOT MATCH THE DESIGN SHALL BE CORRECTED BY THE INSTALLER AT THE INSTALLER'S EXPENSE. 2. <u>CONSTRUCTION AND CONTRACTOR</u>

FOLLOW INFILTRATOR WATER TECHNOLOGIES DOCUMENTS ON INSTALLATION OF QUICK 4 STANDARD CHAMBERS 2.2. CONTRACTOR REQUIRED TO LOCATE ALL UTILITIES PRIOR TO EXCAVATION.

2.3. COMPACTION OF SOIL INFILTRATIVE AREA TO BE AVOIDED DURING CONSTRUCTION (AVOID TRAFFIC AND STORAGE OF MATERIALS). IT IS RECOMMENDED TO STAKE OFF SOIL TREATMENT AREA (STA) FOOTPRINT DURING CONSTRUCTION AND PLACE PERMANENT SIGNS AFTER CONSTRUCTION. CONTRACTOR SHALL NOT DRIVE WHEELED MACHINERY OVER BOTTOM OF THE INFILTRATIVE SURFACE. IT IS RECOMMENDED TO PLACE CHAMBERS AND THEN BACKFILL WITH TRACKED MACHINERY. CONSTRUCTION SHOULD PROCEED ONLY WHEN THE SOIL IS SUFFICIENTLY DRY TO RESIST COMPACTION AND SMEARING DURING EXCAVATION. (TAKE A SAMPLE OF SOIL FROM THE BOTTOM OF INFILTRATIVE SURFACE AND IF WHEN ROLLED BETWEEN THE FINGERS FORMS A WIRE INSTEAD OF CRUMBLING, THE SOIL IS TOO WET). TRAFFIC OVER THE STA WILL CAUSE AND SUBSEQUENT FAILURE. TOP SOIL TO BE EXCAVATED AND STOCKPILED FOR LATER USE. RESEED SOIL TREATMENT AREA WITH NATIVE VEGETATION (SHALLOW ROOT). 2.4. SLOPE FINISHED GRADE ABOVE LEACH FIELD 3-4% TO MAXIMIZE RUNOFF FROM PRECIPITATION. AVOID IRRIGATION ON STA. 2.5. QUICK 4 PLUS CHAMBERS PROPOSED FOR EFFLUENT DISTRIBUTION. PLACE CHAMBERS ACCORDING TO THE SECTION VIEW. 3. <u>SANITARY LINES</u>

3.1. GRAVITY SEWER LINES (4" PVC SCH 40 ASTM F891 DWV PIPE) SHALL BE INSTALLED WITH SLOPE NO LESS THAN ¹/₈" PER FOOT (~1%) AND NO GREATER THAN 1.8" PER FOOT (~15%). THERE SHALL BE A MINIMUM 36" OF COVER ON SEWER LINES. INSTALL CLEANOUTS AT ANY CHANGE OF DIRECTION OF 45° OR GREATER OR IF NO OTHER CLEANOUT EXISTS WITHIN 40' OF THE BEND. SWEEPING 90'S MUST BE USED OR BENDS LIMITED TO 45" MUST BE PROVIDED. CLEANOUTS MUST BE PROVIDED AT INTERVALS OF NO MORE THAN 100'. 10' LATERAL SEPARATION IS REQUIRED FROM POTABLE WATER LINES AND 2' VERTICAL SEPARATION WITH SANITARY CROSSING UNDER.

3.2. IF ANY TWO SEWER LINES CONNECT, THEY SHALL CONNECT THROUGH 45° Y FITTINGS AND A 2-WAY CLEANOUT MUST BE PROVIDED BEFORE THE CONNECTION 3.3. CONTACT ENGINEER IMMEDIATELY IF ANY VARIANCE IS SOUGHT FROM THIS PLAN.

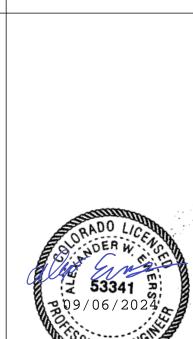
4. OFFSET REQUIREMENTS

4.1. HORIZONTAL INFLUENCE AREA (ACCORDING TO CDPHE WQSA POLICY 6) 4.1.1. THERE ARE NO OTHER EXISTING SYSTEMS KNOWN ON SITE THAT WILL INFLUENCE THE PROPOSED SYSTEM. 4.2. OFFSET REQUIREMENTS ACCORDING TO EL PASO COUNTY OWTS REGULATIONS ARE GIVEN IN THE TABLE (SHEET C1). 4.3. CONTRACTOR TO VERIFY ALL SETBACKS ARE MET BEFORE CONSTRUCTION.

5. <u>SEPTIC TANKS AND PUMP TANK</u> 5.1. THE TANKS ARE NOT TRAFFIC RATED UNLESS CLEARLY SPECIFIED. PLACE BOULDERS OR FENCE AROUND PERIMETER OF TANKS TO AVOID DAMAGE. INSTALL TANK ACCORDING TO MANUFACTURER'S SPECIFICATIONS.

6. PRESSURE DOSED SYSTEM 6.1. ELECTRICAL WIRING, SPLICE BOX SET UP, AND CONTROL PANEL SET UP ARE ALL OUTSIDE THE SCOPE OF THESE PLANS. ALL OF THESE SHALL BE DONE BY A CERTIFIED AND COMPETENT ELECTRICIAN. 7. OPERATION AND MAINTENANCE (O&M)

7.1. OPERATION AND MAINTENANCE IS OUTSIDE OF THE SCOPE AND RESPONSIBILITY OF 3 ROCKS ENGINEERING. IT IS HIGHLY RECOMMENDED TO INSPECT SLUDGE AND SCUM LEVELS ONCE A YEAR IN BOTH COMPARTMENTS OF SEPTIC TANK. DUE TO MOSTLY ANAEROBIC CONDITIONS IN A SEPTIC TANK, NOT ALL SOLIDS WILL BE DIGESTED BY THE BACTERIA AND THEREFORE SOLIDS WILL HAVE TO BE PUMPED OUT AS DETERMINED BY INSPECTION. A GOOD INTERVAL FOR PUMPING SLUDGE FROM TANK IS WHENEVER SCUM AND SLUDGE LEVELS SURPASS 25% OF WORKING CAPACITY OF TANK. 7.2. PULL ORENCO BIOTUBE FT SERIES FILTER AT LEAST ONCE A YEAR AND RINSE SOLIDS INTO PRIMARY TREATMENT TANK. IT IS RECOMMENDED TO PLACE A SIGN AT EACH SERVICED LOCATION WITH INSTRUCTIONS FOR USERS TO PREVENT FLUSHING OF CHEMICALS, MEDICINES, AND ANY OTHER MATERIAL/SUBSTANCE THAT COULD CAUSE PREMATURE FAILURE OF STA 7.3. O&M PROVIDER SHOULD CHECK ACTUATION OF THE AUTOMATIC DISTRIBUTION VALVE (THE CLEAR TUBING SHOULD SHOW ACTUATION BETWEEN ZONES). VALVES WILL BUILD UP SLIME OVER TIME AND WILL HAVE TO BE CLEANED AT AN INTERVAL TO BE DETERMINED BY INSPECTION DURING THE FIRST YEAR OF OPERATION. FAILURE TO CLEAN THE VALVE WILL RESULT IN THE CAM LOCKING INTO ONE ZONE AND CAN CAUSE SYSTEM FAILURE.



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3 ROCKS ENGINEERING & SURVEYING

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V1.0-30% SD-08/30/2024 V2.0-100% CD-09/06/2024 ____ ____

PROJECT ENG: AWE QA/QC:PBG Drawn by: MPH Prj # 24.144

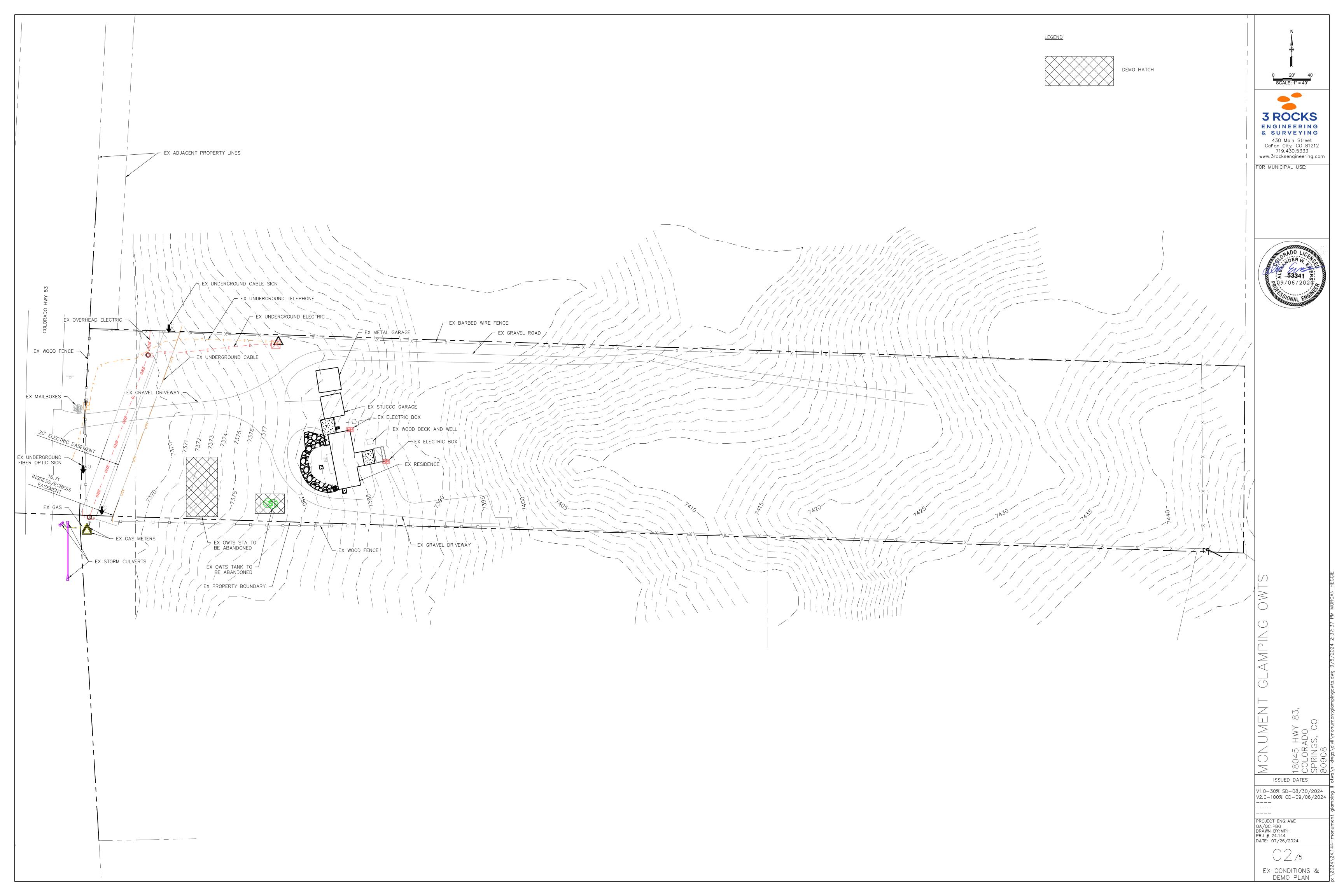
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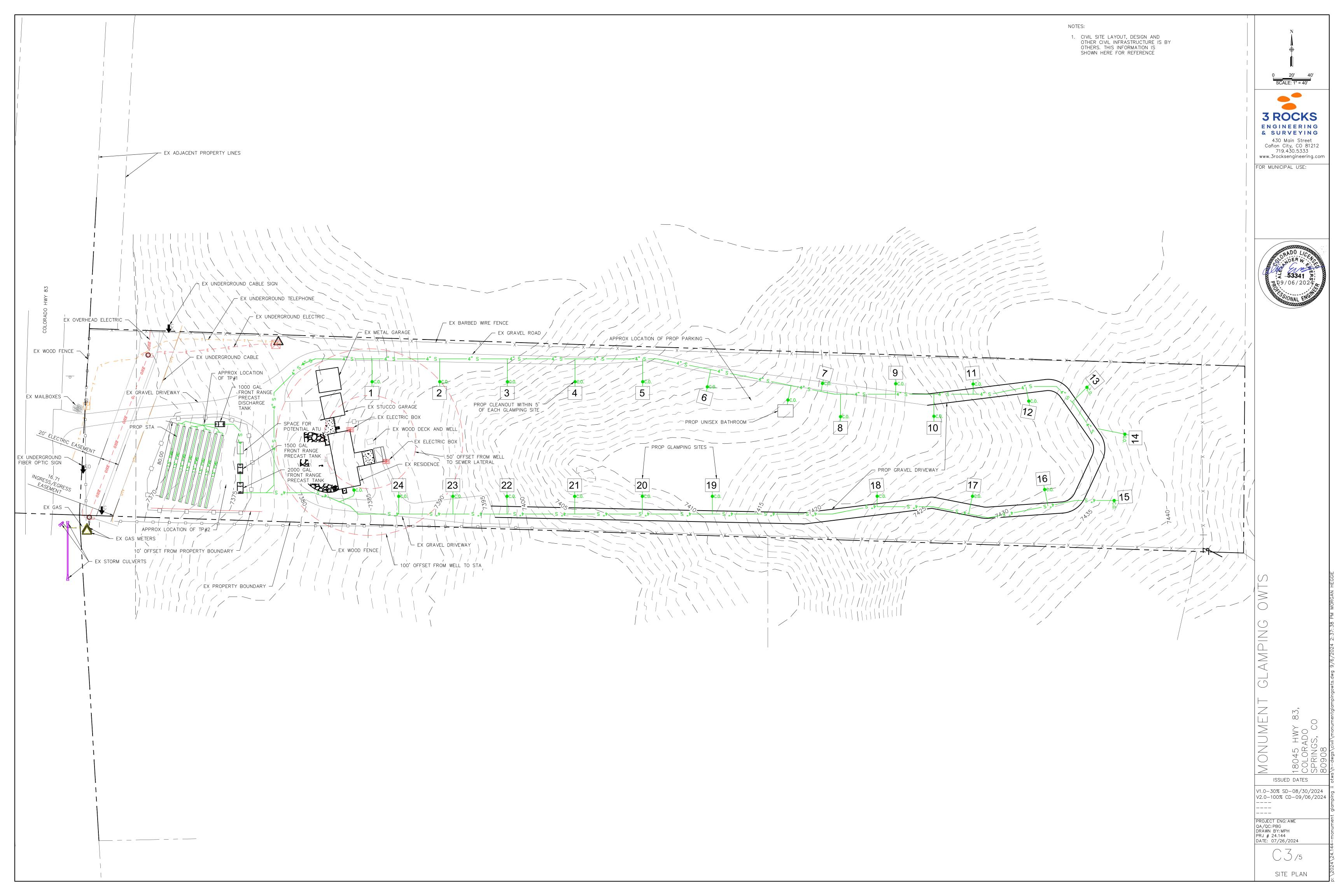
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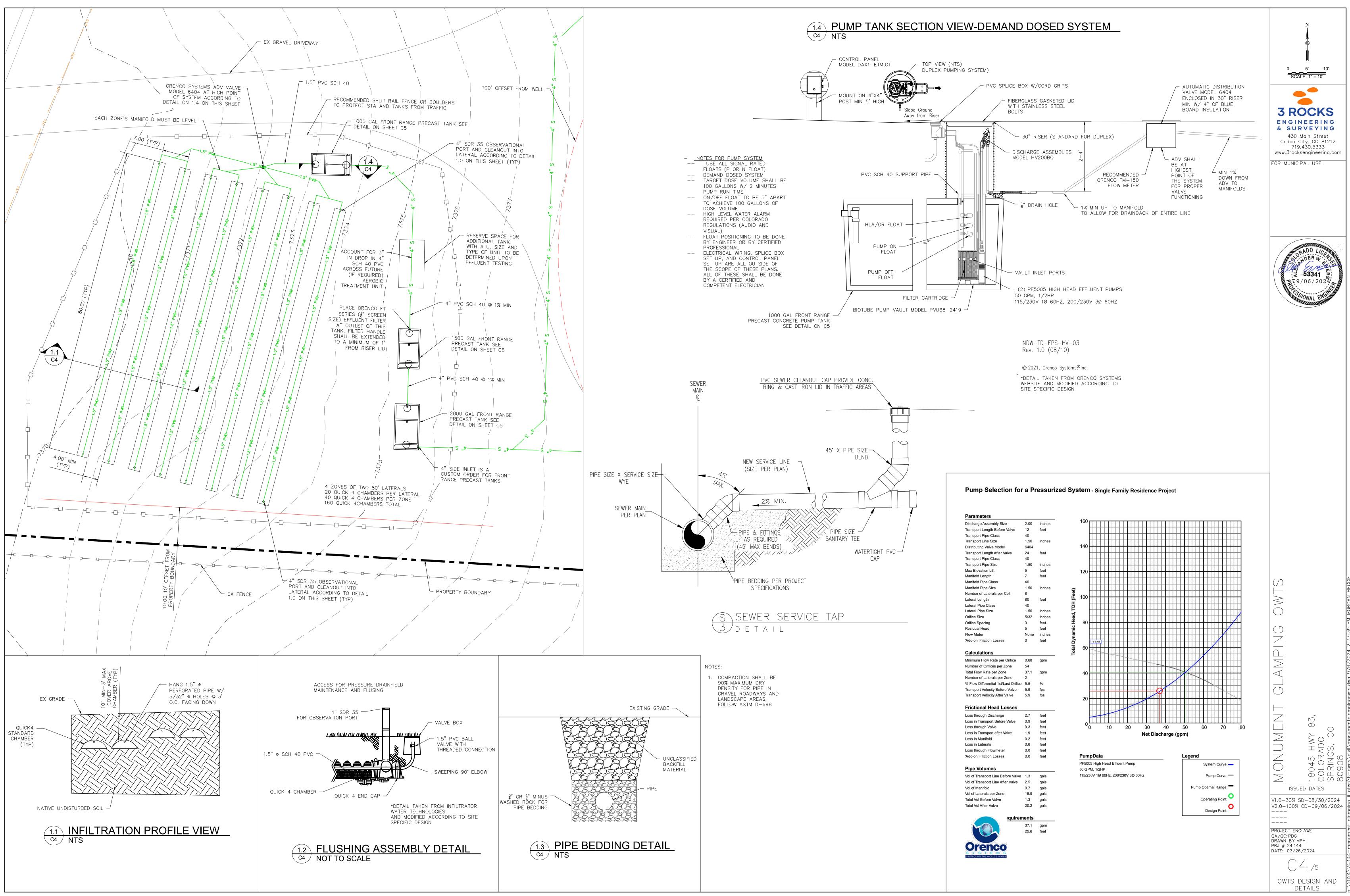


Know what's **below. Call** before you dig.

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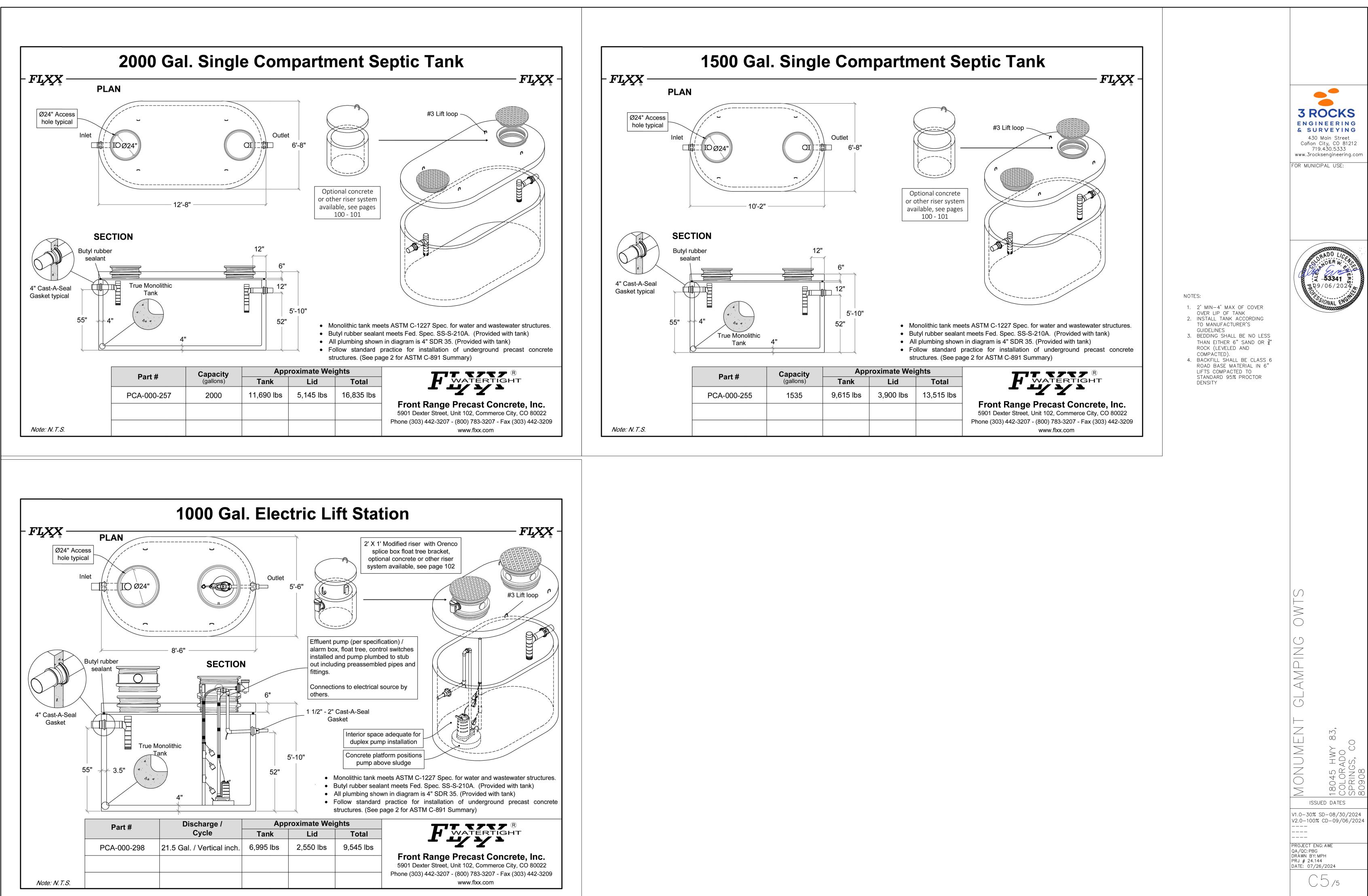






Paramete	rs

Discharge Assembly Size		
	2.00	inches
Transport Length Before Valve	12	feet
Transport Pipe Class	40	
Transport Line Size	1.50	inches
Distributing Valve Model	6404	
Transport Length After Valve	24	feet
Transport Pipe Class	40	
Transport Pipe Size	1.50	inches
Max Elevation Lift	5	feet
Manifold Length	7	feet
Manifold Pipe Class	40	
Manifold Pipe Size	1.50	inches
Number of Laterals per Cell	8	
Lateral Length	80	feet
Lateral Pipe Class	40	
Lateral Pipe Size	1.50	inches
Orifice Size	5/32	inches
Orifice Spacing	3	feet
Residual Head	5	feet
Flow Meter	None	inches
'Add-on' Friction Losses	0	feet
Calculations		
Minimum Flow Rate per Orifice	0.68	gpm
Number of Orifices per Zone	54	
Total Flow Rate per Zone	37.1	gpm
Number of Laterals per Zone	2	
% Flow Differential 1st/Last Orifice	5.5	%
	5.9	e
Transport Velocity Before Valve	5.9	fps
Transport Velocity Before Valve Transport Velocity After Valve	5.9 5.9	tps fps
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Transport Velocity After Valve		•
Transport Velocity After Valve Frictional Head Losses	5.9	fps
Transport Velocity After Valve Frictional Head Losses Loss through Discharge	5.9 2.7	fps feet
Transport Velocity After Valve Frictional Head Losses Loss through Discharge Loss in Transport Before Valve	5.9 2.7 0.9	fps feet feet
Transport Velocity After Valve Frictional Head Losses Loss through Discharge Loss in Transport Before Valve Loss through Valve	5.9 2.7 0.9 9.3	fps feet feet feet
Transport Velocity After Valve Frictional Head Losses Loss through Discharge Loss in Transport Before Valve Loss through Valve Loss in Transport after Valve	5.9 2.7 0.9 9.3 1.9	fps feet feet feet feet
Transport Velocity After Valve Frictional Head Losses Loss through Discharge Loss in Transport Before Valve Loss through Valve Loss in Transport after Valve Loss in Manifold	5.9 2.7 0.9 9.3 1.9 0.2	fps feet feet feet feet feet
Transport Velocity After Valve Frictional Head Losses Loss through Discharge Loss in Transport Before Valve Loss through Valve Loss in Transport after Valve Loss in Manifold Loss in Laterals	5.9 2.7 0.9 9.3 1.9 0.2 0.6	fps feet feet feet feet feet feet
Transport Velocity After Valve Frictional Head Losses Loss through Discharge Loss in Transport Before Valve Loss through Valve Loss in Transport after Valve Loss in Manifold Loss in Laterals Loss through Flowmeter	5.9 2.7 0.9 9.3 1.9 0.2 0.6 0.0	fps feet feet feet feet feet feet
Transport Velocity After Valve Frictional Head Losses Loss through Discharge Loss in Transport Before Valve Loss in Transport after Valve Loss in Transport after Valve Loss in Manifold Loss in Laterals Loss through Flowmeter 'Add-on' Friction Losses Pipe Volumes	5.9 2.7 0.9 9.3 1.9 0.2 0.6 0.0 0.0	fps feet feet feet feet feet feet feet
Transport Velocity After Valve Frictional Head Losses Loss through Discharge Loss in Transport Before Valve Loss in Transport after Valve Loss in Transport after Valve Loss in Manifold Loss in Laterals Loss through Flowmeter 'Add-on' Friction Losses Pipe Volumes Vol of Transport Line Before Valve	5.9 2.7 0.9 9.3 1.9 0.2 0.6 0.0 0.0 1.3	fps feet feet feet feet feet feet feet gals
Transport Velocity After Valve Frictional Head Losses Loss through Discharge Loss in Transport Before Valve Loss in Transport after Valve Loss in Transport after Valve Loss in Manifold Loss in Laterals Loss through Flowmeter 'Add-on' Friction Losses Pipe Volumes Vol of Transport Line Before Valve Vol of Transport Line After Valve	5.9 2.7 0.9 9.3 1.9 0.2 0.6 0.0 0.0 0.0 1.3 2.5	fps feet feet feet feet feet feet feet gals gals
Transport Velocity After Valve Frictional Head Losses Loss through Discharge Loss through Valve Loss in Transport After Valve Loss in Transport after Valve Loss in Manifold Loss in Laterals Loss through Flowmeter 'Add-on' Friction Losses Pipe Volumes Vol of Transport Line Before Valve Vol of Manifold	5.9 2.7 0.9 9.3 1.9 0.2 0.6 0.0 0.0 0.0 1.3 2.5 0.7	fps feet feet feet feet feet feet feet gals gals gals
Transport Velocity After Valve Frictional Head Losses Loss through Discharge Loss in Transport Before Valve Loss in Transport after Valve Loss in Transport after Valve Loss in Manifold Loss in Laterals Loss through Flowmeter 'Add-on' Friction Losses Pipe Volumes Vol of Transport Line Before Valve Vol of Transport Line After Valve	5.9 2.7 0.9 9.3 1.9 0.2 0.6 0.0 0.0 0.0 1.3 2.5	fps feet feet feet feet feet feet feet gals gals



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TANK DETAILS