



June 2, 2025

El Paso County
Planning and Community Development
2880 International Circle, Suite 110
Colorado Springs, CO 80910

Attn: Brad Walters, Inspections Supervisor

RE: **The Retreat at TimberRidge Filing No. 2**
Public Street Improvement Certification

The public street improvements for **The Retreat at TimberRidge Filing No. 2** consist of asphalt paving, shouldering, curb and gutter, signage and striping. The site and adjacent properties (as affected by work performed under the County permit) appear to be stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover and the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements.

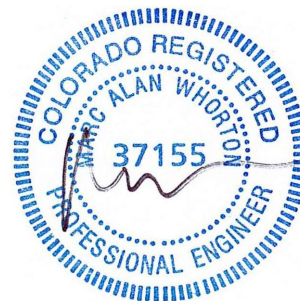
On behalf of the developer and based on County inspections of the site, Classic Consulting Engineers & Surveyors, LLC hereby requests **Final Acceptance** of the referenced public roadway improvements.

STATEMENT OF ENGINEER IN RESPONSIBLE CHARGE:

Based upon the information gathered during periodic site visits to the project during the construction and then after completion of construction of the street improvements, to the best of my knowledge, information and belief, the referenced public street improvements have been constructed in general compliance with the approved design plans and specifications as filed with El Paso County.

A handwritten signature in blue ink, appearing to read "Marc A. Whorton".

Marc A. Whorton, P.E.
Colorado No. 37155



6/2/2025

Seal & Signature of P.E.

maw/118520/letters/PE CERT STREET.doc



June 2, 2025

El Paso County
Planning and Community Development
2880 International Circle, Suite 110
Colorado Springs, CO 80910

Attn: Brad Walters, Inspections Supervisor

RE: **The Retreat at TimberRidge Filing No. 2**
Public Storm Improvement Certification

The public storm improvements for **The Retreat at TimberRidge Filing No. 2** consist of RCP culverts, rip-rap dissipators, curb inlets and RCP storm systems. Any channel improvements associated with this project will be handled on a separate certification. The site and adjacent properties (as affected by work performed under the County permit) appear to be stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover and the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements.

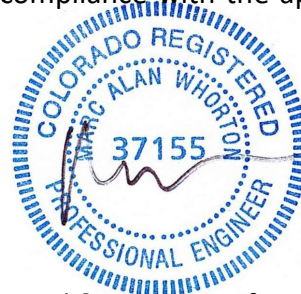
On behalf of the developer and based on County inspections of the site, Classic Consulting hereby requests **Final Acceptance** of the referenced public storm improvements.

STATEMENT OF ENGINEER IN RESPONSIBLE CHARGE:

Based upon the information gathered during periodic site visits to the project during the construction and then after completion of construction of the storm improvements, to the best of my knowledge, information and belief, the referenced public storm improvements, prepared by Classic Consulting, have been constructed in general compliance with the approved design plans and specifications as filed with El Paso County.

A handwritten signature in blue ink, appearing to read "Marc A. Whorton".

Marc A. Whorton, P.E.
Colorado No. 37155



6/2/2025

Seal & Signature of P.E.

maw/118520/letters/PE CERT STORM.doc



Civil Engineer
Stormwater Best Management Practice (permanent) Certification Letter

June 2, 2025

El Paso County
Planning and Community Development
2880 International Circle, Suite 110
Colorado Springs, CO 80910

Attn.: Brad Walters, Inspections Supervisor

RE: **Retreat at TimberRidge Filing No. 2**

The permanent stormwater Best Management Practices (BMPs) for the **Retreat at TimberRidge Filing No. 2** consists of a Private Full Spectrum Extended Detention Basin. This facility is described by the following:

Pond 3 within Tract A, Retreat at TimberRidge Filing No. 2

The site and adjacent properties (as affected by work performed under the County permit) appear to be stable with respect to settlement and subsidence, sloughing of cut and fill slopes, revegetation or other ground cover and the improvements (public improvements, common development improvements, site grading and paving) meet or exceed the minimum design requirements. The detention/stormwater quality facilities provide the required storage volume and will meet the required release rates, as documented by the attached UDFCD design form submitted with the original application, the stage areas, elevations and outlet dimensions.

Statement Of Engineer In Responsible Charge:

I, Marc A. Whorton, a registered Professional Engineer in the State of Colorado, in accordance with Sections 5.2 and 5.3 of the Bylaws and Rules of the State Board of Registration for Professional Engineers and Professional Land Surveyors, do hereby certify that I or a person under my responsible charge periodically observed the construction of the above-mentioned project. Based on the on-site

field observations and review of pertinent as-built documentation, it is my professional opinion that the required permanent BMPs have been installed and are in general compliance with the approved Construction Plans, and Specifications as filed with El Paso County. For BMPs with a Water Quality Capture Volume (WQCV), I have attached the post-construction As-Built drawings and calculations. The As-Built drawings accurately depict the final installation of the stormwater BMPs.



Marc A. Whorton, P.E.
Colorado No. 37155



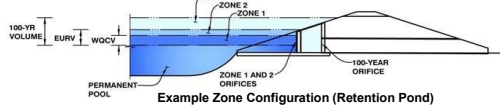
6/2/2025

Seal & Signature of P.E. Goes Here

MHFD-Detention, Version 4.05 (January 2022)

Project: REETREAT AT TIMBERRIDGE FILING NO. 2

Basin ID: POND 3 (AS-BUILT CHECK)



Example Zone Configuration (Retention Pond)

Selected BMP Type =	EDB	
Watershed Area =	58.30	acres
Watershed Length =	3,400	ft
Watershed Length to Centroid =	1,500	ft
Watershed Slope =	0.050	ft/ft
Watershed Imperviousness =	12.60%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Water Quality Capture Volume (WQCV) =	0.395	acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	0.703	acre-feet		acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.904	acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	1.876	acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	2.832	acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	4.494	acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	5.642	acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	7.279	acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 3.85 in.) =	13.819	acre-feet	3.85	inches
Approximate 2-yr Detention Volume =	0.460	acre-feet		
Approximate 5-yr Detention Volume =	0.708	acre-feet		
Approximate 10-yr Detention Volume =	1.336	acre-feet		
Approximate 25-yr Detention Volume =	1.794	acre-feet		
Approximate 50-yr Detention Volume =	1.887	acre-feet		
Approximate 100-yr Detention Volume =	2.404	acre-feet		

Zone 1 Volume (WQV_1)	= 0.395	acre-feet
Zone 2 Volume ($EURV - Zone 1$)	= 0.309	acre-feet
Zone 3 Volume ($100\text{-year} - Zones 1 \& 2$)	= 1.700	acre-feet
Total Detention Basin Volume	= 2.404	acre-feet
Initial Surcharge Volume (ISV)	= user	ft^3
Initial Surcharge Depth (ISD)	= user	ft
Total Available Detention Depth (H_{total})	= user	ft
Depth of Trickle Channel (H_{TC})	= user	ft
Slope of Trickle Channel (S_{TC})	= user	ft/ft
Slopes of Main Basin Sides (S_{main})	= user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$)	= user	

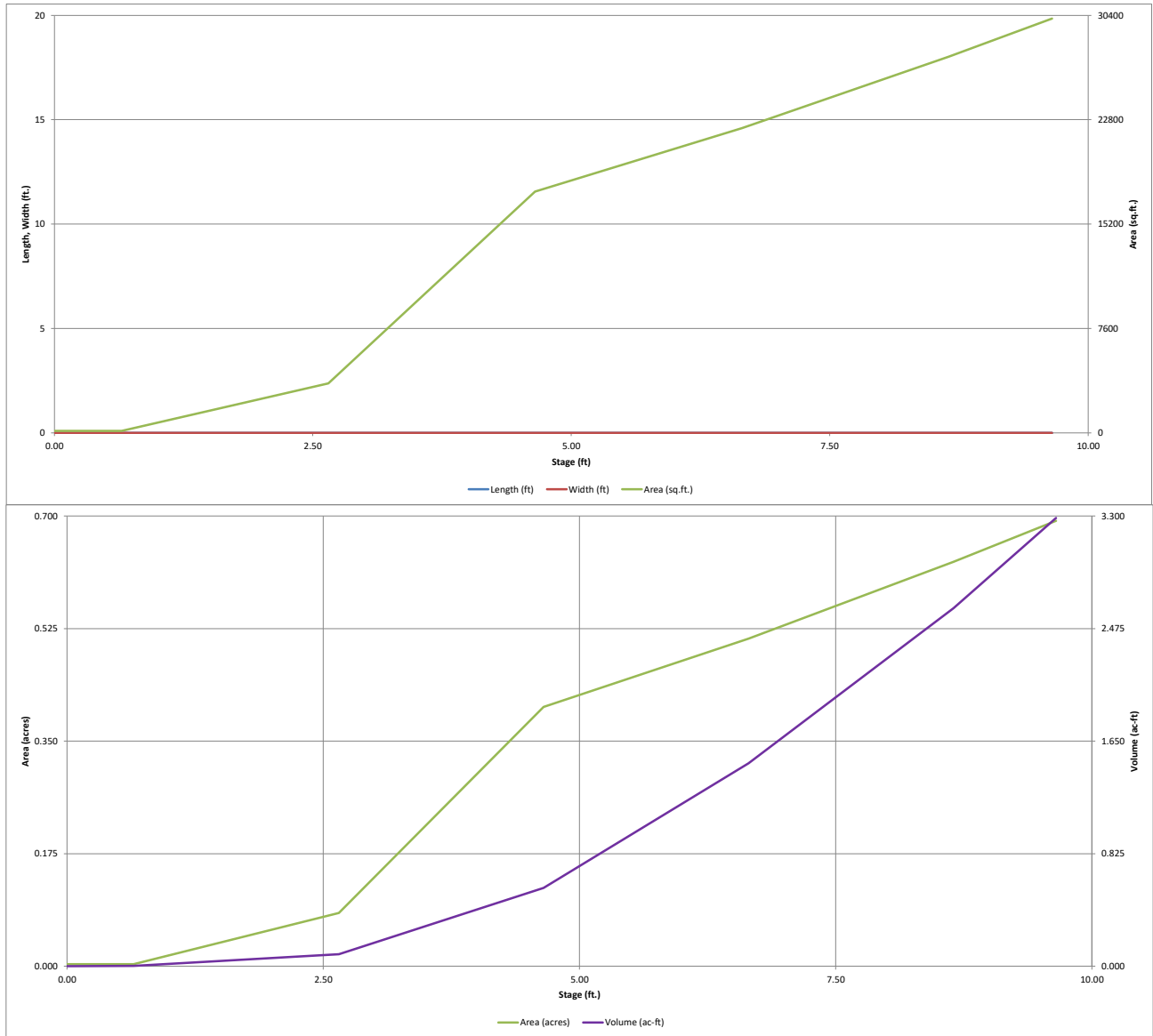
Initial Surchage Area (A_{ISV})	=	user	ft ²
Surchage Volume Length (L_{ISV})	=	user	ft
Surchage Volume Width (W_{ISV})	=	user	ft
Depth of Basin Floor (H_{FLOOR})	=	user	ft
Length of Basin Floor (L_{FLOOR})	=	user	ft
Width of Basin Floor (W_{FLOOR})	=	user	ft
Area of Basin Floor (A_{FLOOR})	=	user	ft ²
Volume of Basin Floor (V_{FLOOR})	=	user	ft ³
Depth of Main Basin (H_{MAIN})	=	user	ft
Length of Main Basin (L_{MAIN})	=	user	ft
Width of Main Basin (W_{MAIN})	=	user	ft
Area of Main Basin (A_{MAIN})	=	user	ft ²
Volume of Main Basin (V_{MAIN})	=	user	ft ³
Calculated Total Basin Volume (V_{OBS})	=	user	acre-feet

Depth Increment =	1.00	ft
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MHFD-Detention_v4-05 Pond 3 AS-BUILT, Basin

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.05 (January 2022)

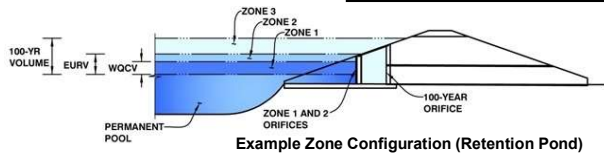


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.05 (January 2022)

Project: REETREAT AT TIMBERRIDGE FILING NO. 2

Basin ID: POND 3 (AS-BUILT CHECK)



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	4.16	0.395	Orifice Plate
Zone 2 (EURV)	4.97	0.309	Orifice Plate
Zone 3 (100-year)	8.30	1.700	Weir&Pipe (Restrict)
Total (all zones)		2.404	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = N/A ft (distance below the filtration media surface)
Underdrain Orifice Diameter = N/A inches

Calculated Parameters for Underdrain
Underdrain Orifice Area = N/A ft²
Underdrain Orifice Centroid = N/A feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice = 0.00 ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate = 5.03 ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing = 20.00 inches
Orifice Plate: Orifice Area per Row = N/A sq. inches

Calculated Parameters for Plate
WQ Orifice Area per Row = N/A ft²
Elliptical Half-Width = N/A feet
Elliptical Slot Centroid = N/A feet
Elliptical Slot Area = N/A ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.70	3.40					
Orifice Area (sq. inches)	1.23	1.29	1.29					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = N/A ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice = N/A ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter = N/A inches

Calculated Parameters for Vertical Orifice
Vertical Orifice Area = N/A ft²
Vertical Orifice Centroid = N/A feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

Overflow Weir Front Edge Height, H_o = 5.03 ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length = 10.00 feet
Overflow Weir Grate Slope = 4.00 H:V
Horiz. Length of Weir Sides = 4.00 feet
Overflow Grate Type = Close Mesh Grate
Debris Clogging % = 50%

Calculated Parameters for Overflow Weir
Height of Grate Upper Edge, H_u = 6.03 feet
Overflow Weir Slope Length = 4.12 feet
Grate Open Area / 100-yr Orifice Area = 6.64
Overflow Grate Open Area w/o Debris = 32.61 ft²
Overflow Grate Open Area w/ Debris = 16.31 ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe = 0.58 ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter = 30.00 inches
Restrictor Plate Height Above Pipe Invert = 30.00 inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate
Outlet Orifice Area = 4.91 ft²
Outlet Orifice Centroid = 1.25 feet
Half-Central Angle of Restrictor Plate on Pipe = 3.14 radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = 7.65 ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length = 35.00 feet
Spillway End Slopes = 4.00 H:V
Freeboard above Max Water Surface = 1.00 feet

Calculated Parameters for Spillway
Spillway Design Flow Depth = 0.76 feet
Stage at Top of Freeboard = 9.41 feet
Basin Area at Top of Freeboard = 0.68 acres
Basin Volume at Top of Freeboard = 3.12 acre-ft

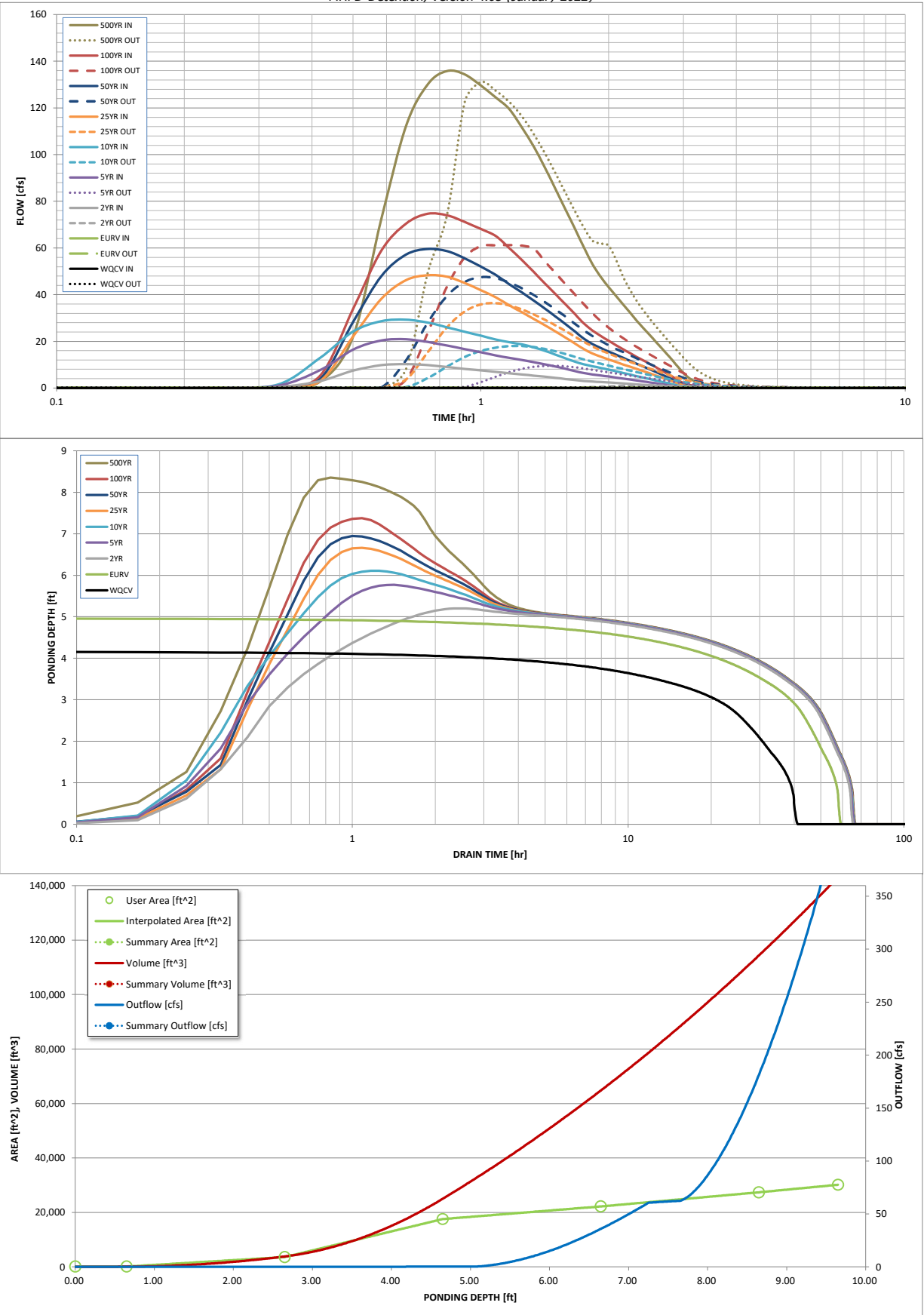
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.85
One-Hour Rainfall Depth (in) =	0.395	0.703	0.904	1.876	2.832	4.494	5.642	7.279	13.819
CUHP Runoff Volume (acre-ft) =	N/A	N/A	0.904	1.876	2.832	4.494	5.642	7.279	13.819
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	5.6	15.8	24.0	43.0	54.0	69.2	129.0
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A							
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.10	0.27	0.41	0.74	0.93	1.19	2.21
Peak Inflow Q (cfs) =	N/A	N/A	10.3	20.8	29.3	48.3	59.6	74.7	135.8
Peak Outflow Q (cfs) =	0.2	0.2	1.1	9.5	18.0	36.4	47.5	61.2	131.1
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.6	0.7	0.8	0.9	0.9	1.0
Structure Controlling Flow =	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	0.03	0.3	0.5	1.1	1.5	1.9	2.0
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	53	58	54	50	45	42	38	24
Time to Drain 99% of Inflow Volume (hours) =	40	56	62	60	58	56	54	52	45
Maximum Ponding Depth (ft) =	4.16	4.97	5.20	5.77	6.11	6.66	6.95	7.38	8.35
Area at Maximum Ponding Depth (acres) =	0.32	0.42	0.43	0.46	0.48	0.51	0.53	0.55	0.61
Maximum Volume Stored (acre-ft) =	0.396	0.706	0.804	1.055	1.215	1.493	1.638	1.870	2.440

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.05 (January 2022)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.08
	0:15:00	0.00	0.00	0.10	0.17	0.21	0.14	0.18	0.17	0.37
	0:20:00	0.00	0.00	0.41	1.01	1.58	0.43	0.50	0.67	2.55
	0:25:00	0.00	0.00	2.95	7.51	12.72	2.84	3.62	4.99	21.98
	0:30:00	0.00	0.00	7.42	16.44	24.17	22.06	28.28	33.86	72.66
	0:35:00	0.00	0.00	9.78	20.38	28.68	38.31	48.01	58.97	112.34
	0:40:00	0.00	0.00	10.28	20.85	29.25	46.22	57.17	70.48	130.16
	0:45:00	0.00	0.00	9.81	19.73	28.04	48.33	59.58	74.66	135.85
	0:50:00	0.00	0.00	8.97	18.17	26.01	47.63	58.62	74.00	134.31
	0:55:00	0.00	0.00	8.21	16.68	24.13	44.98	55.52	71.13	129.59
	1:00:00	0.00	0.00	7.56	15.30	22.42	41.90	51.98	68.13	124.56
	1:05:00	0.00	0.00	6.95	14.00	20.83	38.93	48.54	65.17	119.60
	1:10:00	0.00	0.00	6.35	12.93	19.65	35.47	44.47	60.02	111.61
	1:15:00	0.00	0.00	5.82	12.02	18.70	32.44	40.91	54.77	103.41
	1:20:00	0.00	0.00	5.32	11.08	17.43	29.61	37.42	49.70	94.43
	1:25:00	0.00	0.00	4.83	10.12	15.90	26.90	34.01	44.80	85.24
	1:30:00	0.00	0.00	4.35	9.17	14.33	24.23	30.65	40.22	76.53
	1:35:00	0.00	0.00	3.88	8.23	12.76	21.63	27.37	35.86	68.18
	1:40:00	0.00	0.00	3.42	7.24	11.26	19.07	24.16	31.62	60.17
	1:45:00	0.00	0.00	3.04	6.41	10.14	16.62	21.10	27.63	53.05
	1:50:00	0.00	0.00	2.79	5.83	9.33	14.81	18.88	24.65	47.66
	1:55:00	0.00	0.00	2.57	5.36	8.62	13.41	17.14	22.30	43.29
	2:00:00	0.00	0.00	2.38	4.92	7.90	12.24	15.67	20.27	39.46
	2:05:00	0.00	0.00	2.17	4.48	7.18	11.13	14.24	18.36	35.73
	2:10:00	0.00	0.00	1.96	4.04	6.46	10.09	12.89	16.56	32.17
	2:15:00	0.00	0.00	1.75	3.62	5.76	9.09	11.60	14.88	28.81
	2:20:00	0.00	0.00	1.56	3.20	5.09	8.13	10.36	13.29	25.66
	2:25:00	0.00	0.00	1.37	2.80	4.46	7.21	9.19	11.83	22.74
	2:30:00	0.00	0.00	1.18	2.41	3.84	6.31	8.04	10.39	19.91
	2:35:00	0.00	0.00	1.00	2.03	3.26	5.42	6.92	8.96	17.16
	2:40:00	0.00	0.00	0.82	1.66	2.69	4.54	5.81	7.55	14.42
	2:45:00	0.00	0.00	0.64	1.29	2.13	3.67	4.71	6.14	11.71
	2:50:00	0.00	0.00	0.47	0.93	1.58	2.81	3.61	4.74	9.03
	2:55:00	0.00	0.00	0.30	0.60	1.09	1.96	2.54	3.36	6.50
	3:00:00	0.00	0.00	0.19	0.40	0.80	1.23	1.63	2.20	4.51
	3:05:00	0.00	0.00	0.14	0.30	0.63	0.81	1.12	1.49	3.23
	3:10:00	0.00	0.00	0.11	0.24	0.51	0.55	0.78	1.03	2.34
	3:15:00	0.00	0.00	0.09	0.20	0.41	0.38	0.56	0.69	1.67
	3:20:00	0.00	0.00	0.07	0.16	0.33	0.26	0.40	0.46	1.17
	3:25:00	0.00	0.00	0.06	0.12	0.26	0.19	0.29	0.29	0.80
	3:30:00	0.00	0.00	0.04	0.10	0.20	0.13	0.21	0.18	0.53
	3:35:00	0.00	0.00	0.04	0.07	0.15	0.10	0.15	0.11	0.36
	3:40:00	0.00	0.00	0.03	0.06	0.11	0.07	0.11	0.09	0.26
	3:45:00	0.00	0.00	0.02	0.04	0.08	0.05	0.08	0.07	0.20
	3:50:00	0.00	0.00	0.02	0.03	0.06	0.04	0.07	0.05	0.16
	3:55:00	0.00	0.00	0.01	0.02	0.04	0.03	0.05	0.04	0.12
	4:00:00	0.00	0.00	0.01	0.01	0.03	0.02	0.04	0.03	0.09
	4:05:00	0.00	0.00	0.01	0.01	0.02	0.02	0.03	0.02	0.06
	4:10:00	0.00	0.00	0.00	0.01	0.01	0.01	0.02	0.01	0.04
	4:15:00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.02
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.05 (January 2022)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

[illegible]