

Drainage Report 8830 Mustang Place, Colorado Springs, CO 80908

PREPARED FOR: Mike Cartmell

PREPARED BY: WaterVation, PLLC

DATE: August 22nd, 2022

PCD File No. VR-224

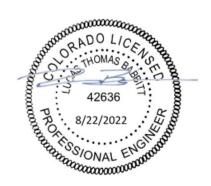




Table of Contents

Engineer's Statement	1
Developer's Statement	1
El Paso County	1
Project Description	
Purpose	3
Drainage Fee	3
Drainage Basin Characteristics	3
Proposed Drainage Characteristics	6
Detention & Water Quality	6
Regulatory Floodplains	7
References	8
Drainage Plan	9
Appendix A	10
Table of Tables	
Table 1. Peak Flows for Existing Conditions	
Table 2. Peak Flows for Proposed Conditions	
Table 3. Peak Flow Increase at Design Point	
Table 3. Hydrologic Calculations for Existing Conditions	
Table 4. Hydrologic Calculations for Proposed Conditions	11
Table of Figures	
Figure 1 Site Man	2

Engineer's Statement

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

- B.	8/22/2022
[Name, P.E. # 42636]	Date

Developer's Statement

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

Mike Cartmell (Aug 22, 2022 13:56 MDT)	Aug 22, 2022
Michael Cartmell, President	Date
MASON, LLC	
7702 Barnes Road #140-58	

El Paso County

Colorado Springs, CO 80922

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

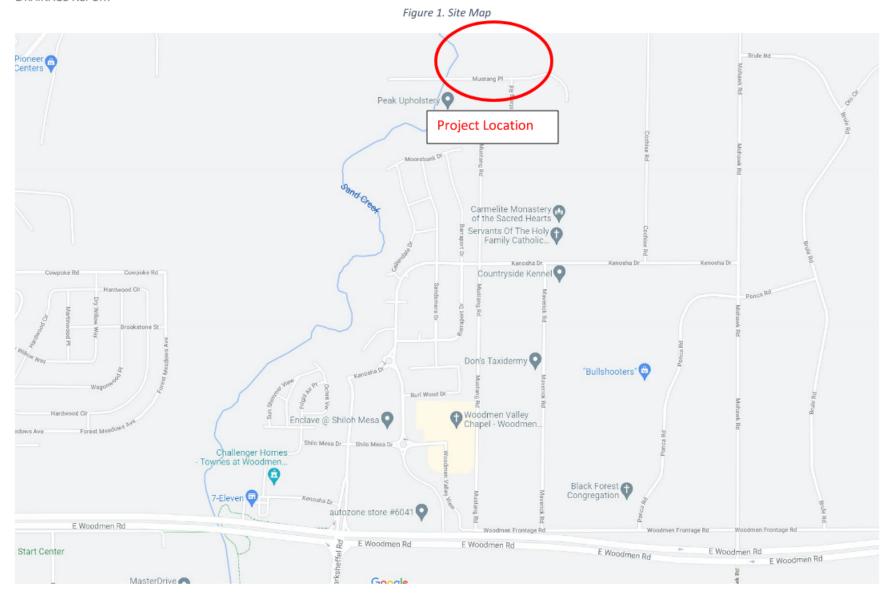
APPROVED

	Engineering Review
Joshua Palmer, P.E.	Date 09/26/2022 11:02:44 AM
Interim County Engineer / ECM Administrator	dsdrice
	JeffRice@elpasoco.com
Conditions:	(719) 520-7877
	EPC Planning & Community

Project Description

This Project is located approximately 0.75 miles to the north of the intersection of East Woodmen Road and Marksheffel Road (Figure 1). The Project area is comprised of one 5.37-acre lot and is proposed to be subdivided into two lots.

Development Department



Purpose

The purpose of this drainage report is to evaluate existing and proposed drainage characteristics for the proposed subdivision of 8830 Mustang Place, Colorado Springs, CO 80908 (Project). This report was prepared on September 24th, 2021.

Drainage Fee

The property is in Sand Creek Drainage basin, which is studied and has drainage/bridge fees associated with it. Per Resolution No. 21-468 the drainage fee is \$21,814/impervious acre and bridge fee is \$8,923/impervious acre. However, per ECM 3.10.2a, a 25% reduction in drainage fees is authorized for this Project because it is a low-density lot. Therefore, the drainage fee is \$16,360.50.

Impervious Area (AC)	2022 Drainage Fee	Total Drainage Fee for Project
0.31	\$16,360.50	\$5,071.76

Impervious Area (AC)	2022 Bridge Fee	Total Bridge Fee for Project
0.31	\$8,923	\$2,766.13

Drainage Basin Characteristics

Basin A

Basin A slopes to the south and southwest at slopes ranging from 0.005 ft/ft to 0.01 ft/ft. Existing impervious area was delineated using aerial imagery flown in July 2021 and was calculated to be 0%. Proposed impervious area was established by the property owner and is 0%.

The Natural Resources Conservation Service (NRCS) Web Soil Survey was referenced to identify hydrologic soil groups within the Project area. The Project area is comprised of Hydrologic Soil Group (HSG) A and HSG B soils with dominantly sandy-loam soil. Soil conditions for all basins will remain unchanged in proposed conditions.

Existing land cover is dominantly upland grasses.

Hydrologic soil groups are based on estimates of runoff potential. HSG A soils have a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission. HSG B soils have a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission. Soil maps for the Project area are provided in Appendix A.

Peak flow rates were calculated using the Rational Method with precipitation data from Figure 6-5 of El Paso County's DCM. Runoff coefficients were calculated as a function of impervious area storm frequency using impervious area. A summary of estimated peak flows for existing conditions is provided in Table 1.

Basin B

Basin B slopes to the south and southwest at slopes ranging from 0.005 ft/ft to 0.01 ft/ft. Existing impervious area was delineated using aerial imagery flown in July 2021 and was calculated to be 7.16%. Proposed impervious area was established by the property owner and is 15.28%.

The Natural Resources Conservation Service (NRCS) Web Soil Survey was referenced to identify hydrologic soil groups within the Project area. The Project area is comprised of Hydrologic Soil Group (HSG) A and HSG B soils with dominantly sandy-loam soil. Soil conditions for all basins will remain unchanged in proposed conditions.

Existing land cover is dominantly upland grasses.

Hydrologic soil groups are based on estimates of runoff potential. HSG A soils have a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission. HSG B soils have a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission. Soil maps for the Project area are provided in Appendix A.

Peak flow rates were calculated using the Rational Method with precipitation data from Figure 6-5 of El Paso County's DCM. Runoff coefficients were calculated as a function of impervious area storm frequency using impervious area. A summary of estimated peak flows for existing conditions is provided in Table 1.

Basin C

Basin C slopes to the south and southwest at slopes ranging from 0.005 ft/ft to 0.01 ft/ft. Existing impervious area was delineated using aerial imagery flown in July 2021 and was calculated to be 3.84%. Proposed impervious area was established by the property owner and is 3.84%.

The Natural Resources Conservation Service (NRCS) Web Soil Survey was referenced to identify hydrologic soil groups within the Project area. The Project area is comprised of Hydrologic Soil Group (HSG) A and HSG B soils with dominantly sandy-loam soil. Soil conditions for all basins will remain unchanged in proposed conditions.

Existing land cover is dominantly upland grasses.

Hydrologic soil groups are based on estimates of runoff potential. HSG A soils have a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission. HSG B soils have a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission. Soil maps for the Project area are provided in Appendix A.

Peak flow rates were calculated using the Rational Method with precipitation data from Figure 6-5 of El Paso County's DCM. Runoff coefficients were calculated as a function of impervious area storm frequency using impervious area. A summary of estimated peak flows for existing conditions is provided in Table 1.

Basin D

Basin D slopes to the south and southwest at slopes ranging from 0.005 ft/ft to 0.01 ft/ft. Existing impervious area was delineated using aerial imagery flown in July 2021 and was calculated to be 1.14%. Proposed impervious area was established by the property owner and is 1.14%.

The Natural Resources Conservation Service (NRCS) Web Soil Survey was referenced to identify hydrologic soil groups within the Project area. The Project area is comprised of Hydrologic Soil Group (HSG) A and HSG B soils with dominantly sandy-loam soil. Soil conditions for all basins will remain unchanged in proposed conditions.

Existing land cover is dominantly upland grasses.

Hydrologic soil groups are based on estimates of runoff potential. HSG A soils have a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission. HSG B soils have a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission. Soil maps for the Project area are provided in Appendix A.

Peak flow rates were calculated using the Rational Method with precipitation data from Figure 6-5 of El Paso County's DCM. Runoff coefficients were calculated as a function of impervious area storm frequency using impervious area. A summary of estimated peak flows for existing conditions is provided in Table 1.

Basin E

Basin E slopes to the south and southwest at slopes ranging from 0.005 ft/ft to 0.01 ft/ft. Existing impervious area was delineated using aerial imagery flown in July 2021 and was calculated to be 1.39%. Proposed impervious area was established by the property owner and is 1.39%.

The Natural Resources Conservation Service (NRCS) Web Soil Survey was referenced to identify hydrologic soil groups within the Project area. The Project area is comprised of Hydrologic Soil Group (HSG) A and HSG B soils with dominantly sandy-loam soil. Soil conditions for all basins will remain unchanged in proposed conditions.

Existing land cover is dominantly upland grasses.

Hydrologic soil groups are based on estimates of runoff potential. HSG A soils have a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission. HSG B soils have a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission. Soil maps for the Project area are provided in Appendix A.

Peak flow rates were calculated using the Rational Method with precipitation data from Figure 6-5 of El Paso County's DCM. Runoff coefficients were calculated as a function of impervious area storm frequency using impervious area. A summary of estimated peak flows for existing conditions is provided in Table 1.

Table 1. Peak Flows for Existing Conditions

EX	ISTING SUBB	ASIN CHARACTERIST	rics	EXISTING PEAK FLOW (CFS)					
SUBBASIN	AREA (AC)	NRCS HYDROLOGIC SOIL GROUP	PERCENT IMPERVIOUS	2-YR	5-YR	25-YR	100-YR		
Α	0.36	В	0.00%	0.00	0.00	0.40	0.90		
В	0.96	В	7.16%	0.09	0.15	1.21	2.46		
С	3.60	В	3.84%	0.15	0.24	3.59	7.59		
D	1.49	В	1.14%	0.02	0.03	1.74	3.81		
E	0.55	В	1.39%	0.01	0.01	0.56	1.23		

Proposed Drainage Characteristics

The only alteration to proposed conditions is the addition of 3,000 square-feet of impervious area in Subbasin B. A summary of estimated peak flows for proposed conditions is provided in Table 2.

Table 2. Peak Flows for Proposed Conditions

PRO	POSED SUBI	BASIN CHARACTERIS	TICS	PROPOSED PEAK FLOW (CFS)					
SUBBASIN	AREA (AC)	NRCS HYDROLOGIC SOIL GROUP	PERCENT IMPERVIOUS	2-YR	5-YR	25-YR	100-YR		
Α	0.36	В	0.00%	0.00	0.00	0.40	0.90		
В	0.96	В	15.28%	0.24	0.34	1.46	2.73		
С	3.60	В	3.84%	0.15	0.24	3.59	7.59		
D	1.49	В	1.14%	0.02	0.03	1.74	3.81		
E	0.55	В	1.39%	0.01	0.01	0.56	1.23		

Design Points

Design Point 1

Design Point 1 captures flow from Basin A. As shown in Table 3, there is no increase in flow between existing and proposed conditions at this design point during either the 5-year or 100-year storm.

Design Point 2

Design Point 1 captures flow from Basin B and C. As shown in Table 3, there is an increase in flow of 0.15 cfs and 0.22 cfs between existing and proposed conditions at this design point during either the 5-year or 100-year storm, respectively.

Design Point 3

Design Point 1 captures flow from Basin D. As shown in Table 3, there is no increase in flow between existing and proposed conditions at this design point during either the 5-year or 100-year storm.

Design Point 4

Design Point 1 captures flow from Basin D. As shown in Table 3, there is no increase in flow between existing and proposed conditions at this design point during either the 5-year or 100-year storm.

Table 3. Peak Flow Increase at Design Point

DESIGN POINT SUMMARY												
DESIGN POINT	EXI	STING PEA	K FLOW (C	CFS)	PROPOSED PEAK FLOW (CFS)				DIFFERENCE (CFS)			
	2-YR	5-YR	25-YR	100-YR	2-YR	5-YR	25-YR	100-YR	2-YR	5-YR	25-YR	100-YR
1	0.00	0.00	0.40	0.90	0.00	0.00	0.40	0.90	0.00	0.00	0.00	0.00
2	0.22	0.36	4.63	9.69	0.33	0.51	4.83	9.91	0.11	0.15	0.20	0.22
3	0.02	0.03	1.74	3.81	0.02	0.03	1.74	3.81	0.00	0.00	0.00	0.00
4	0.01	0.01	0.56	1.23	0.01	0.01	0.56	1.23	0.00	0.00	0.00	0.00

Detention & Water Quality

The only design point that experiences an increase in peak flow rate is Design Point 2 (Table 3). The increase in peak flow, compared to existing conditions, ranges between 0.11 cfs (2-year) to 0.22 cfs (100-year). Water quality for the increase in runoff will be provided by the large grass swale that exists in Basin C, which has a bottom width of approximately 150 feet.

Regulatory Floodplains

No regulatory floodplains exist on-site. The FEMA FIRM is 08041C0533G.

Conclusion

The project area is already developed. The proposed project contemplates adding one house. The impacts on stormwater runoff are negligible as shown in this report. Flow leaving the site does not exceed 15 cfs at any design point, therefore drainage easements are not required.

8830 MUSTANG PLACE DRAINAGE REPORT

References

City of Colorado Springs, Drainage Criteria Manual Volume 1, May 2014.

El Paso Engineering Criteria Manual, 2018

Mile High Flood Control District, UD Rational 2.00

Natural Resources Conservation Service (NRCS) Web Soil Survey

Drainage Plan

GENERAL NOTES & LEGEND

TBD

-SUBBASIN ID -IMPERVIOUS AREA (%) -SUBBASIN AREA (ACRES) —DESIGN POINT SUBBASIN BOUNDARY LONGEST FLOW PATH IMPERVIOUS AREA (EXISTING) IMPERVIOUS AREA (PROPOSED) EXISTING CONTOUR PROPOSED CONTOUR

> EXISTING CONTOUR ELEVATION (2000)2000 PROPOSED CONTOUR ELEVATION

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Horizontal Scale & Orientation

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MUSTANG PL. & CHAPARRAL LP. DRAINAGE

EL PASO COUNTY,

CO

Project Milestone

GENERAL NOTES & LEGEND

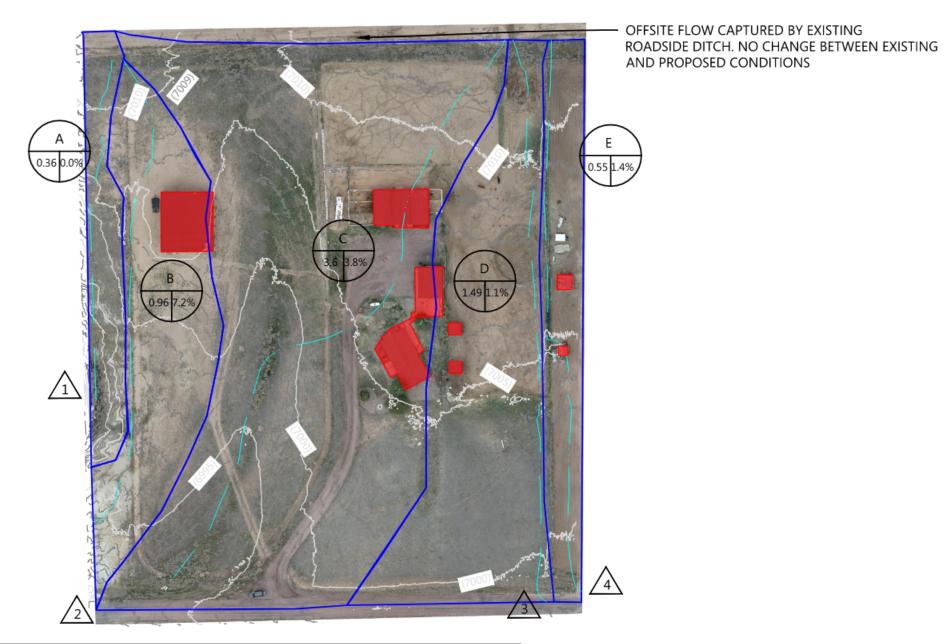
2021006027 NTS

SURVEY CONTROL

CONTROL POINT NUMBER XXXX NORTHING: XXX EASTING: XXX ELEVATION: XXX NAD 83 COLORADO STATE PLANES, U.S. FOOT, CODE CO83-CF NAVD 88

Sheet 1 of 7

EX	ISTING SUBB	ASIN CHARACTERIST	rics	EXISTING PEAK FLOW (CFS)				
SUBBASIN	AREA (AC)	NRCS HYDROLOGIC SOIL GROUP	PERCENT IMPERVIOUS	2-YR	5-YR	25-YR	100-YR	
Α	0.36	В	0.00%	0.00	0.00	0.40	0.90	
В	0.96	В	7.16%	0.09	0.15	1.21	2.46	
С	3.60	В	3.84%	0.15	0.24	3.59	7.59	
D	1.49	В	1.14%	0.02	0.03	1.74	3.81	
E	0.55	В	1.39%	0.01	0.01	0.56	1.23	



						DESIGN PO	IMUS TAIC	MARY					
	DESIGN POINT	EXISTING PEAK FLOW (CFS)				PROPOSED PEAK FLOW (CFS)				DIFFERENCE (CFS)			
		2-YR	5-YR	25-YR	100-YR	2-YR	5-YR	25-YR	100-YR	2-YR	5-YR	25-YR	100-YR
	1	0.00	0.00	0.40	0.90	0.00	0.00	0.40	0.90	0.00	0.00	0.00	0.00
Cacas Balls	2	0.22	0.36	4.63	9.69	0.33	0.51	4.83	9.91	0.11	0.15	0.20	0.22
S PM S	3	0.02	0.03	1.74	3.81	0.02	0.03	1.74	3.81	0.00	0.00	0.00	0.00
24000/12 12	4	0.01	0.01	0.56	1.23	0.01	0.01	0.56	1.23	0.00	0.00	0.00	0.00



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Issued	_	Ву	Appd.	YY.MM.DD
File Name:	 ITB Dwn.	Chkd.	LTB Dsgn.	21.09.20 YY.MM.DD
Permit-Seal			_	

Client/Project

MUSTANG PL. & CHAPARRAL LP. DRAINAGE

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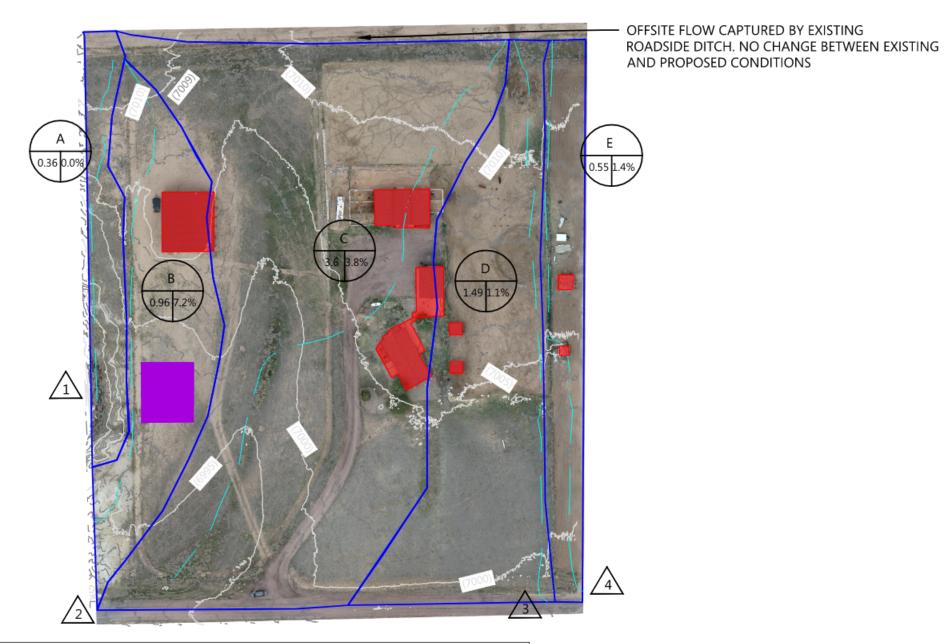
Project Milestone

EX DRAINAGE PLAN-MUSTANG PLACE

2021006027 1" = 100'

Sheet 2 of 7

PRO	POSED SUBI	BASIN CHARACTERIS	TICS		PROPOSED PE	AK FLOW (CFS)	
SUBBASIN	AREA (AC)	NRCS HYDROLOGIC SOIL GROUP	PERCENT IMPERVIOUS	2-YR	5-YR	25-YR	100-YR
Α	0.36	В	0.00%	0.00	0.00	0.40	0.90
В	0.96	В	15.28%	0.24	0.34	1.46	2.73
С	3.60	В	3.84%	0.15	0.24	3.59	7.59
D	1.49	В	1.14%	0.02	0.03	1.74	3.81
E	0.55	В	1.39%	0.01	0.01	0.56	1.23



					DESIGN PO	IMUS TAIC	MARY									
DESIGN POINT	EXI	STING PEA	K FLOW (C	CFS)	PRO	POSED PE	AK FLOW (CFS)	DIFFERENCE (CFS)							
	2-YR	5-YR	25-YR	100-YR	2-YR	5-YR	25-YR	100-YR	2-YR	5-YR	25-YR	100-YR				
1	0.00	0.00	0.40	0.90	0.00	0.00	0.40	0.90	0.00	0.00	0.00	0.00				
2	0.22	0.36	4.63	9.69	0.33	0.51	4.83	9.91	0.11	0.15	0.20	0.22				
3	0.02	0.03	1.74	3.81	0.02	0.03	1.74	3.81	0.00	0.00	0.00	0.00				
4	0.01	0.01	0.56	1.23	0.01	0.01	0.56	1.23	0.00	0.00	0.00	0.00				



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Horizontal Scale & Orientation

File Name:

Client/Project

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EL PASO COUNTY,

CO

Project Milestone

PR DRAINAGE PLAN-MUSTANG PLACE

2021006027 1" = 100'

Sheet 3 of 7

Appendix A

A summary of the hydrologic calculations for this Project are provided in Table 3 and Table 4. Rational calculations were performed using the UD-RATIONAL 2.00 software.

Table 4. Hydrologic Calculations for Existing Conditions

																Calculat	ion of Pea	ak Runoff	using Ra	itional M	ethod																
	Water 5/10/2	Vation 2022		. [Cells of the	his col	or are fo	r requ		r-inp	t _i =	0.395(1.1 - C ₅) S _i ^{0.33})√L _i	Compute	ed t _c = t _i +	te		t _{minimum} = t _{minimum} =	5 (urban) 10 (non-urban	1-hour rainfall depth, P1 (in) = 1.20 1.45 1.80 2.00 2.45 2.60												AA wet					
Project: Location:		Mustang so Count		- 1	Cells of the Cells of the						t _t =	$= \frac{L_t}{60K\sqrt{S_t}} = \frac{1}{60}$	OV _t	Regiona	al t _c = (26 –	$+17i) + {60(14)}$	L_t $\pm i + 9)\sqrt{S_t}$	Selected t _c	= max{t _{minimu}	_{um} , min (Comp	puted t _c , Regior	nal t _c)} Rain	ıfall Intensi	ty Equation	Coefficients			c 0.786	I(in/h	$=\frac{a}{(b)}$	Use Area	e Denver a Intensity		Q(cfs) = CIA			
Subcatch	Aroa	NRCS Hydrolo	Percent	Н	Ru	inoff (Coefficie	ent, C	; 	+	Overland		d (Initial) Flow T D/S Elevation	Overland	Overland	Channelized	U/S Elevation	Channeli D/S Elevation	ized (Travel) F		Channelized	Channelized	Tim	ne of Conce	entration		Rai	infall In	tensity,	l (in/hr)	\perp		<u> </u>	Peak Flow,	, Q (cfs)	
ment Name	(ac)	gic Soil Group		2-yr	5-yr 10-	yr 25	i-yr 50-y	/r 100	0-yr 500)-yr F	Flow Length L _i (ft)	(ft) (Optional)	(ft) (Optional)	Flow Slope S _i (ft/ft)		Flow Length	(ft) (Optional)	(ft) (Optional)	Flow Slope S _t (ft/ft)		Flow Velocity V _t (ft/sec)	Flow Time	Computed t _c (min)	Regional t _c (min)	Selected t _c (min)	2-yr	5-yr 1	10-yr 2	5-yr 50	yr 100)-yr 500)-yr 2-yr	5-yr 10	-yr 25-yr	50-yr	100-yr 500-y	
A (EX)	0.36	В	0.00	0.00	0.00 0.0	6 0.	.25 0.33	3 0.	.43 0.	54	126.00	7012.00	7006.00	0.048	13.32	236.00	7006.00	6986.00	0.085	7	2.04	1.93	15.25	27.50	15.25	2.70	3.27	4.05	.50 5.	52 5.8	86	0.00	0.00 0.	.08 0.40	0.65	0.90	
B (EX)	0.96	В	7.16	0.04	0.05 0.1	1 0.	.29 0.37	7 0.	.46 0.	56	141.00	7012.00	7005.00	0.050	13.28	369.00	7005.00	6994.00	0.030	10	1.73	3.56	16.85	28.34	16.85	2.58	3.11	3.86	.29 5.	26 5.5	58	0.09	0.15 0.	.43 1.21	1.86	2.46	
C (EX)	3.60	В	3.84	0.02	0.02 0.0	9 0.	.27 0.35	5 0.	.44 0.	55	71.00	7012.00	7011.00	0.014	14.61	627.00	7011.00	6991.00	0.032	7	1.25	8.36	22.97	31.48	22.97	2.19	2.65	3.29	3.65 4.	47 4.	75	0.15	0.24 1.	.04 3.59	5.63	7.59	
D (EX)	1.49	В	1.14	0.00	0.01 0.0	7 0.	.26 0.33	3 0.	.43 0.	54	36.00	7012.00	7011.00	0.028	8.45	560.00	7011.00	6999.00	0.021	10	1.46	6.38	14.83	32.77	14.83	2.74	3.31	4.11	.56 5.	59 5.9	93	0.02	0.03 0.	.41 1.74	2.79	3.81	
E (EX)	0.55	В	1.39	0.01	0.01 0.0	7 0.	.26 0.34	4 0.	.43 0.	54	86.00	7012.00	7010.00	0.023	13.84	506.00	7010.00	6999.00	0.022	10	1.47	5.72	19.56	31.98	19.56	2.39	2.89	3.58	3.98 4.	88 5.	17	0.01	0.01 0.	.13 0.56	0.90	1.23	
	Н					+		+	_																			_	_	-	士	_	#		口	\pm	
DP 1 (EX)	0.36	В	0.00	0.00	0.00 0.0	06 0.	.25 0.33	3 0.	.43 0.	54	126.00	7012.00	7006.00	0.048	13.32	236.00	7006.00	6986.00	0.08	7	2.04	1.93	15.25	27.50	15.25	2.70	3.27	4.05	.50 5.	52 5.8	86	0.00	0.00 0.	.08 0.40	0.65	0.90	
DP 2 (EX)	4.56	В	4.50	0.02	0.03 0.0	9 0.	.28 0.35	5 0.	.45 0.	55	71.00	7012.00	7011.00	0.014	14.55	627.00	7011.00	6991.00	0.032	7	1.25	8.36	22.91	31.31	22.91	2.20	2.65	3.29	3.66 4.	48 4.	76	0.22	0.36 1.	.40 4.63	7.22	9.69	
DP 3 (EX)	1.49	В	1.14	0.00	0.01 0.0	7 0.	.26 0.33	3 0.	.43 0.	54	36.00	7012.00	7011.00	0.028	8.45	560.00	7011.00	6999.00	0.021	10	1.46	6.38	14.83	32.77	14.83	2.74	3.31	4.11	.56 5.	59 5.9	93	0.02	0.03 0.	.41 1.74	2.79	3.81	
DP 4 (EX)	0.55	В	1.39	0.01	0.01 0.0	7 0.	.26 0.34	4 0.	.43 0.	54	86.00	7012.00	7010.00	0.023	13.84	506.00	7010.00	6999.00	0.022	10	1.47	5.72	19.56	31.98	19.56	2.39	2.89	3.58	3.98 4.	88 5.	17	0.01	0.01 0.	.13 0.56	0.90	1.23	
						#		Ŧ																					1	+	#					#	

															Ca	lculation	of Peak	Runoff us	ing Ratio	nal Meth	nod															
Designer: L. Babbitt fersion 2.00 released May 20 Clear Workshe Company: WaterVation Date: 5/10/2022 Cells of this color are for required user-ing							rksheet	$t_i = \frac{0}{1}$	S ₁ 0.33		Compute	$d t_c = t_i + t_i$	t _t	,	t _{minimum} = 5 (urban) t _{minimum} = 10 (non-urban)						1-hour rainfall depth, P1 (in) = 1.20 1.45 1.80 2.00 2.45 2.60 Senter your own depths obtained from the NOAA v										AA v					
Project: 8330 Mustang Place Cells of this color are for optional overrid Cells of this color are for calculated resu								lts	t _t =	$\frac{1}{60 \text{K} \sqrt{\text{S}_{\text{t}}}} = \frac{1}{60 \text{K}}$	0V _t		Regional $t_c = (26 - 17i) + \frac{L_t}{60(14i + 9)\sqrt{S_t}}$			$Selected \ t_c = max\{t_{minimum}, min(Computed \ t_c \ , Regional \ t_c)\} \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \$					ntensity Equ	uation Coe	efficients =	28.50 10	a b c $1(in/hr) = \frac{a}{(b+1)^2}$ Use Deriver Area Intensity						Q(cfs) = CIA					
				\vdash	Ru	noff Co	efficie T	ent, C	_	-		Overland	d (Initial) Flow 1	Time				Channeli	zed (Travel) F	low Time			Time o	f Concent	tration		Rainf	all Inter	nsity, I (in/hr)	ightarrow		Pea	ak Flow, C	Q (cfs)	
Subcatc ment Name	Area (ac)	NRCS Hydrologic Soil Group			r 10-	yr 25-yr	50-у	/r 100-yr	f 500-y	Overi r Flow L L; (ength	U/S Elevation (ft) (Optional)	D/S Elevation (ft) (Optional)	Overland Flow Slope S _i (ft/ft)		Channelized Flow Length L _t (ft)	U/S Elevation (ft) (Optional)	D/S Elevation (ft) (Optional)	Channelized Flow Slope S _t (ft/ft)	NRCS Conveyance Factor K	Channelized Flow Velocity V _t (ft/sec)	Channelized Flow Time t _t (min)	Computed	Regional t _c (min)		2-yr 5-	yr 10	-yr 25-	yr 50-y	r 100-yr	500-yr	2-yr 5-yr	10-yr	25-yr 5	0-yr 10	0-yr 500-yr
A (PR)	0.36	В	0.00	0.00 0.0	0.0	0.25	0.33	3 0.43	0.54	126.	00	7012.00	7006.00	0.048	13.32	236.00	7006.00	6986.00	0.085	7	2.04	1.93	15.25	27.50	15.25	2.70 3.	27 4.	05 4.5	0 5.52	5.86	\blacksquare	0.00 0.00	0.08	0.40 ().65 0.	.90
B (PR)	0.96	В	15.28	0.09 0.1	1 0.1	8 0.34	0.41	1 0.50	0.59	141.	00	7012.00	7005.00	0.050	12.50	369.00	7005.00	6994.00	0.030	10	1.73	3.56	16.06	26.60	16.06	2.64 3.	19 3.	96 4.3	9 5.38	5.71		0.24 0.34	0.68	1.46 2	2.14 2.	73
C (PR)	3.60	В	3.84	0.02 0.0	2 0.0	9 0.27	0.35	5 0.44	0.55	71.0	00	7012.00	7011.00	0.014	14.61	627.00	7011.00	6991.00	0.032	7	1.25	8.36	22.97	31.48	22.97	2.19 2.	65 3.	29 3.6	5 4.47	4.75		0.15 0.24	1.04	3.59 5	j.63 7.	.59
D (PR)	1.49	В	1.14	0.00 0.0	1 0.0	0.26	0.33	3 0.43	0.54	36.	00	7012.00	7011.00	0.028	8.45	560.00	7011.00	6999.00	0.021	10	1.46	6.38	14.83	32.77	14.83	2.74 3.	31 4.	11 4.5	6 5.59	5.93		0.02 0.03	0.41	1.74 2	2.79 3.	.81
E (PR)	0.55	В	1.39	0.01 0.0	1 0.0	0.26	0.34	4 0.43	0.54	86.	00	7012.00	7010.00	0.023	13.84	506.00	7010.00	6999.00	0.022	10	1.47	5.72	19.56	31.98	19.56	2.39 2.	89 3.	58 3.9	8 4.88	5.17	\Box	0.01 0.01	0.13	0.56).90 1.	23
																											Ŧ		\pm	=	\blacksquare				\mp	
DP 1 (PF	0.36	В	0.00	0.00 0.0	0.0	0.25	0.33	3 0.43	0.54	126.	00	7012.00	7006.00	0.048	13.32	236.00	7006.00	6986.00	0.08	7	2.04	1.93	15.25	27.50	15.25	2.70 3.	27 4.	05 4.5	0 5.52	5.86		0.00 0.00	0.08	0.40 0).65 0.	90
DP 2 (PF	4.56	В	6.25	0.03 0.0	4 0.1	1 0.29	0.36	6 0.46	0.56	71.0	00	7012.00	7011.00	0.014	14.38	627.00	7011.00	6991.00	0.032	7	1.25	8.36	22.73	30.86	22.73	2.20 2.	66 3.	31 3.6	7 4.50	4.78		0.33 0.51	1.62	4.83 7	.45 9.	.91
DP 3 (PF	1.49	В	1.14	0.00 0.0	1 0.0	0.26	0.33	3 0.43	0.54	36.	00	7012.00	7011.00	0.028	8.45	560.00	7011.00	6999.00	0.021	10	1.46	6.38	14.83	32.77	14.83	2.74 3.	31 4.	11 4.5	6 5.59	5.93		0.02 0.03	0.41	1.74 2	2.79 3.	81
DP 4 (PF	0.55	В	1.39	0.01 0.0	1 0.0	0.26	0.34	4 0.43	0.54	86.	00	7012.00	7010.00	0.023	13.84	506.00	7010.00	6999.00	0.022	10	1.47	5.72	19.56	31.98	19.56	2.39 2.	89 3.	58 3.9	8 4.88	5.17		0.01 0.01	0.13	0.56).90 1.	.23



MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:24,000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D Soil Rating Polygons Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil Water Features line placement. The maps do not show the small areas of A/D contrasting soils that could have been shown at a more detailed Streams and Canals В Transportation B/D Rails Please rely on the bar scale on each map sheet for map С measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service US Routes Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. B/D Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 18, Jun 5, 2020 Soil map units are labeled (as space allows) for map scales D 1:50,000 or larger. Not rated or not available Date(s) aerial images were photographed: Aug 19, 2018—Sep 23, 2018 Soil Rating Points The orthophoto or other base map on which the soil lines were Α compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. В B/D

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A	4.8	58.5%
71	Pring coarse sandy loam, 3 to 8 percent slopes	В	3.4	41.5%
Totals for Area of Intere	est		8.2	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified

Tie-break Rule: Higher

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or floodways have been determined, users are encouraged to consult the Flood Profies and Floodway Data and/or Summary of Stilwater Elevations tables contained within the Flood Insurance Study (FRS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, the delevation data presented in the EFS zeroet should be utilized a conjunction with flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be owere that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report

Certain areas not in Special Flood Hazard Areas may be protected by **flood contro structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transvers Mercator (UTM) zone 13. The horizontal datum was NAD83, GRS80 spheroid, Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1988 (NAVD88). These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.ncaa.gov/ or contact the National Geodetic Survey at the following

NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the Nationa Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, and Anderson Consulting Engineers, Inc. These data are current as of 2008.

This map reflects more detailed and up-to-date stream channel configurations and floodplain delineations than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study result, the Proce Profites and Procolary Data tables in the Proce Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

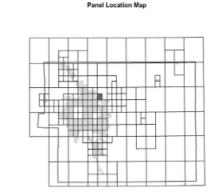
Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is

Contact FEMA Map Service Center (MSC) via the FEMA Map Information eXchang Contact Pelan and Service Center (NOSC) was the Pelan map information exchange (PMIX) 1-877-335-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at bits liveur may fem only.

If you have **questions about this map** or questions concerning the National Floor insurance Program in general, please call **1-877-FEMA MAP** (1-877-396-2627) or visit the FEMA website at http://www.fema.gov/business/nflp.

El Paso County Vertical Datum Offset Table Flooding Source

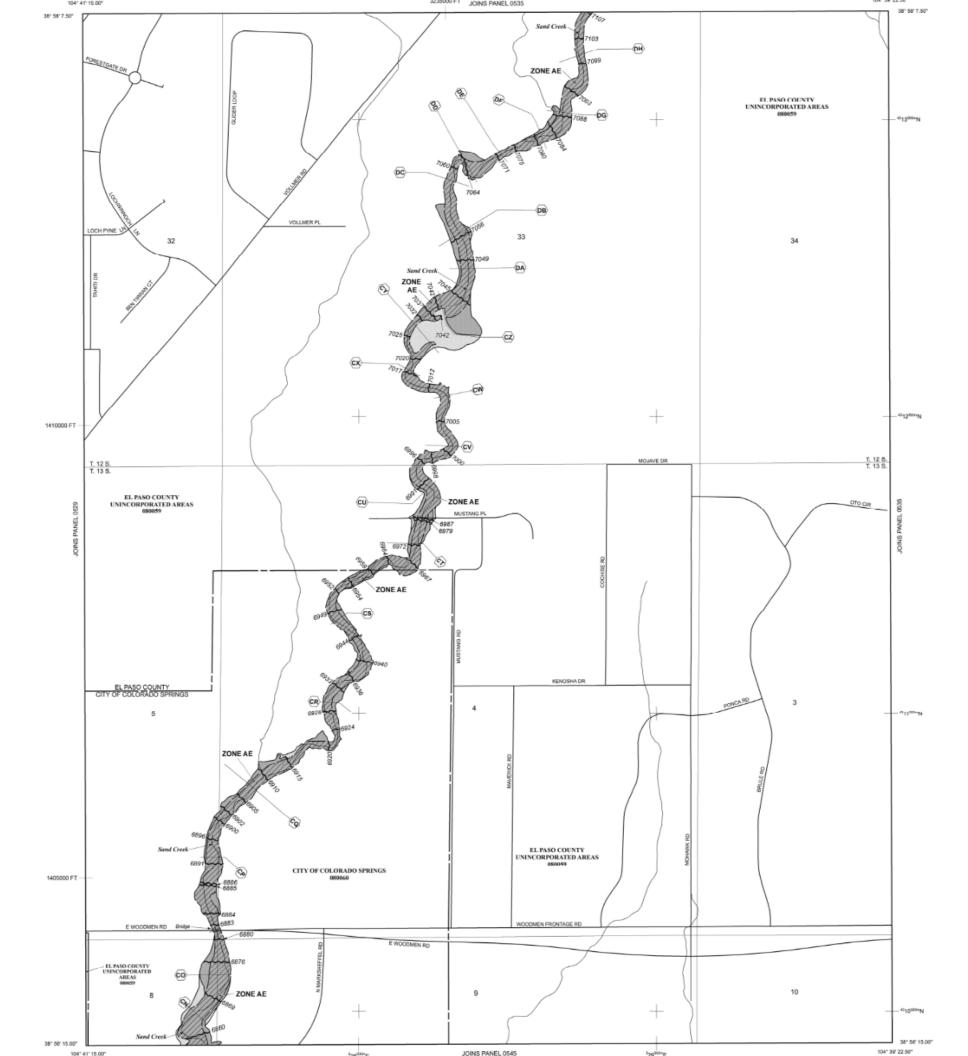
REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Assess



Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



The 1% annual chance tood (1904-year food), also known as the base flood, is the tood that has a 1% chance of being equaleted or exceeded in any given year. The Special Flood Haxard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Haxard include Zones A, AE, JAH, AO, AR, A99, V, of VE. The Base Flood Bevotion is the water-surface elevation of the 1% annual chance flood. ZONE A No Base Flood Elevations determined.

ZONE AE Bese Flood Elevations determined.

ZONE AH Flood deputs of 1 to 3 faut (usually areas of punding); Base Flood Elevations determined.

Special Flood Hazard Area Formenly protected from the 1% annual chance flood by a flood control system that was subsequently described. Zone AR indicates that the former flood control system is being rectived to provide protection from the 1% annual chance or greater flood.

Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined. Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be legst free of encroadment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER AREAS

ZONE X Areas determined to be outside the 0.2% annual chance floodplain. ZONE D Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

Floodplain boundary Floodway boundary CBRS and OPA boundar

Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities. Base Flood Elevation line and value; elevation in feet*

Base Flood Elevation value where uniform within zone; elevation in feet*

--(23) Transect line

Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)

5000-foot grid licks: Colorado State Plane coordinate system, central zone (PIPSZONE 0502), Lambert Conformal Conic Projection

DX5510 Sench mark (see explanation in Notes to Users section of his FIRM panel)

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP MARCH 17, 1997

EFFECTIVE DATE(5) OF REVISION(5) TO THIS PANEL DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update and fermat, to add reads and road names, and to incorporate previously issued Letters of Map Revision.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

MAP SCALE 1" = 500"



GRAAM

PANEL 0533G

FLOOD INSURANCE RATE MAP

EL PASO COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 533 OF 1300 (SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY NUMBER PANEL COLORADO SPRINOS, CITY OF 080000 6503



MAP NUMBER

8830 Mustang Place_Drainage Report_FINAL

Final Audit Report 2022-08-22

Created: 2022-08-22

By: Lucas Babbitt (lucas.babbitt@water-vation.com)

Status: Signed

Transaction ID: CBJCHBCAABAAzNWZBFLpoZ_9y78apxlJisDliaFISOrW

"8830 Mustang Place_Drainage Report_FINAL" History

Document created by Lucas Babbitt (lucas.babbitt@water-vation.com) 2022-08-22 - 7:37:03 PM GMT- IP address: 216.147.124.20

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