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OHE TV T	EXISTING PHONE OR FIBER OPTIC EXISTING STORM DRAIN PROPOSED STORM SEWER	AS-BUILT STREET-STM FEB 14, 2023	JENNIFER IR CONDITIONS: ENGINEE THESE DET DIRECTION PREPARED DETAILED R
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SF-19-003



CALL 2-BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES

	SHEET INDEX								
SHEET NO.	SHEET DESCRIPTION								
C1.1	COVER SHEET								
C1.2	NOTES								
C1.3	TYPICAL SECTIONS								
C2.1	STREET HORIZONTAL CONTROL								
C5.1	SIGNING/STRIPING PLANS								
C6.1-C6.6	STREET/STORM PLAN AND PROFILES								
C10.1-C10.3	DETAILS								

# **DEVELOPER'S STATEMENT**

THE UNDERSIGNED OWNER/DEVELOPER HAS READ AND WILL COMPLY WITH ALL THE REQUIREMENTS SPECIFIED IN THESE CONSTRUCTION PLANS AND THE ACCOMPANYING DRAINAGE REPORT.

BUSINESS NAME _ LORSON, LLC	
BY AN	DATE 2/28/19
TITLE Authorized Signing Agent	-
ADDRESS 212 N. WAHSATCH AVE. SUITE 301	
COLORADO SPRINGS, CO 80903	

The developer and/or Lorson Ranch Metropolitan District shall be responsible for maintenance of the storm drain system and sediment basin within Lamprey Drive, including removal of any sediment that enters the downstream storm drain system east of Yamhill Drive until construction of Lamprey Drive and the storm drain system is completed and accepted for County maintenance.

# TRUCTION APPROVAL

PLAN REVIEW IS PROVIDED ONLY FOR GENERAL CONFORMANCE WITH COUNTY DESIGN THE COUNTY IS NOT RESPONSIBLE FOR THE ACCURACY AND ADEQUACY OF THE DESIGN, ONS, AND/OR ELEVATIONS WHICH SHALL BE CONFIRMED AT THE JOB SITE. THE COUNTY THE APPROVAL OF THIS DOCUMENT ASSUMES NO RESPONSIBILITY FOR COMPLETENESS ACCURACY OF THIS DOCUMENT.

ACCORDANCE WITH THE REQUIREMENTS OF THE EL PASO COUNTY LAND DEVELOPMENT DRAINAGE CRITERIA MANUALS VOLUME 1 AND 2, AND ENGINEERING CRITERIA MANUAL AS D. IN ACCORDANCE WITH ECM SECTION 1.12, THESE CONSTRUCTION DOCUMENTS WILL BE OR CONSTRUCTION FOR A PERIOD OF 2 YEARS FROM THE DATE SIGNED BY THE EL PASO ENGINEER. IF CONSTRUCTION HAS NOT STARTED WITHIN THOSE TWO YEARS THE PLANS EED TO BE RESUBMITTED FOR APPROVAL, INCLUDING PAYMENT OF REVIEW FEES AT THE NG AND COMMUNITY DEVELOPMENT DIRECTOR'S DISCRETION

ND COMMUNITY DEVELOPMENT DIRECTOR'S DISCRETION  Approved by Jeff Rice by Jeff Rice	
Ef Paso County Planning and Community Development         EVINE, COUNTY ENGINEER/ECM ADMINISTRA Dolehalf of Elizabeth Nijkamp, Engineering Review Manager         See above.         05/06/2019       11:52:43 AM	
ER'S APPROVAL	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
TAILED PLANS AND SPECIFICATIONS WERE PREPARED UNDER MY AND SUPERVISION. SAID PLANS AND SPECIFICATIONS HAVE BEEN ACCORDING TO THE CRITERIA ESTABLISHED BY THE COUNTY FOR ROADWAY, DRAINAGE, GRADING AND EROSION CONTROL PLANS AND TONS, AND SAID PLANS AND SPECIFICATIONS ARE IN CONFORMITY WITH E MASTER DRAINAGE PLANS AND MASTER TRANSPORTATION PLANS. SAID O SPECIFICATIONS MEET THE PURPOSES FOR WHICH THE PARTICULAR AND DRAINAGE FACILITIES ARE DESIGNED AND ARE CORRECT TO THE	33997 3-6-2019
MY KNOWLEDGE AND BELIEF. I ACCEPT RESPONSIBILITY FOR ANY CAUSED BY ANY NEGLIGENT ACTS, ERRORS OR OMSSIONS ON MY PART CATION OF THESE DETAILED PLANS AND SPECIFICATIONS	date: MARCH 6, 2019
33997	project no. 100.049
. SCHINDLER, P.E. # 33997 ON BEHALF OF CORE ENGINEERING GROUP	SHEET NUMBER
	TOTAL SHEETS: 14

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## CONSTRUCTION NOTES

- 1. ALL WORK SHALL COMPLY WITH THE CODES AND POLICIES FOR EL PASO COUNTY.
- 2. EXISTING TOPOGRAPHIC INFORMATION SHOWN ON THIS GRADING PLAN WAS OBTAINED FROM DREXEL, BARRELL & CO., JULY, 2005. SUPPLEMENTAL SURVEY DATA WAS OBTAINED FOR MARKSHEFFEL ROAD FROM M&S CIVIL GROUP IN NOVEMBER, 2016. THE CONTRACTOR SHALL BE RESPONSIBLE TO EXAMINE THE SITE AND BE FAMILIAR WITH THE EXISTING CONDITIONS.
- 3. DEPTH OF MOISTURE-DENSITY CONTROL FOR THIS PROJECT SHALL BE AS FOLLOWS: BASE OF ALL CUTS AND FILLS – 12 INCHES, FULL DEPTH OF ALL EMBANKMENTS
- 4. THE CONTRACTOR IS RESPONSIBLE FOR THE RE-ESTABLISHMENT OF ALL SURVEY MONUMENTS DISTURBED WITHIN THE PROJECT LIMITS.
- 5. THE CONTRACTOR SHALL PROTECT ALL WORK AREAS AND FACILITIES FROM FLOODING AT ALL TIMES. AREAS AND FACILITIES SUBJECTED TO FLOODING, REGARDLESS OF THE SOURCE OF WATER, SHALL BE PROMPTLY DEWATERED AND RESTORED.
- 6. PRIOR TO PAVING OPERATIONS, THE ENTIRE SUBGRADE SHALL BE PROOF-ROLLED WITH A LOADED 988 FRONT-END LOADER OR SIMILAR HEAVY RUBBER TIRED VEHICLE (GVW OF 50,000 POUNDS WITH 18 KIP PER AXLE AT TIRE PRESSURES OF 90 PSI) TO DETECT ANY SOFT OR LOOSE AREAS. IN AREAS WHERE SOFT OR LOOSE SOILS, PUMPING OR EXCESSIVE MOVEMENT IS OBSERVED, THE EXPOSED MATERIALS SHALL BE OVER-EXCAVATED TO A MINIMUM DEPTH OF TWO FEET BELOW PROPOSED FINAL GRADE OR TO A DEPTH AT WHICH SOILS ARE STABLE. AFTER THIS HAS BEEN COMPLETED, THE EXPOSED MATERIALS SHALL BE SCARIFIED TO A DEPTH OF 12 INCHES AND MOISTURE CONDITIONED. THE SUBGRADE SHALL THEN BE UNIFORMLY COMPACTED TO A MINIMUM OF 95% OF STANDARD PROCTOR DENSITY (ASTMM D-698) AT 0 TO +4.0% OF OPTIMUM MOISTURE CONTENT FOR A-6 AND A-7-6 SOILS ENCOUNTERED. OTHER SUBGRADE TYPES SHALL BE UNIFORMLY COMPACTED TO A MINIMUM OF 95% OF MODIFIED PROCTOR DENSITY (ASTM D-1557) AT PLUS OR MINUS 2.0% OF OPTIMUM MOISTURE CONTENT. AREAS WHERE STABLE NATURAL SOILS ARE ENCOUNTERED AT PROPOSED SUBGRADE ELEVATION SHALL ALSO BE SCARIFIED (18 INCHES FOR A-7-6 SOILS BELOW FULL-DEPTH ASPHALT CONCRETE) AND COMPACTED AS OUTLINED ABOVE PRIOR TO PAVING OPERATIONS. SUBGRADE FILL SHALL BE PLACED IN SIX-INCH LIFTS AND UNIFORMLY COMPACTED, MEETING THE REQUIREMENTS AS PREVIOUSLY DESCRIBED.
- 7. SUBGRADE MATERIALS DEEMED UNSUITABLE BY THE ENGINEER SHALL BE EXCAVATED, DISPOSED OF AND REPLACED WITH APPROVED MATERIALS.
- 8. FILL SHALL BE PLACED IN 8-INCH MAXIMUM LOOSE LIFTS AND SHALL BE COMPACTED PRIOR TO SUCCESSIVE LIFTS.
- 9. THE CONTRACTOR IS RESPONSIBLE FOR PREVENTING AND CONTROLLING EROSION DURING CONSTRUCTION ACTIVITIES AT ALL TIMES DURING GRADING AND CONSTRUCTION. THE CONTRACTOR SHALL PROVIDE THE FOLLOWING EROSION AND SEDIMENT CONTROL MEASURES:
  - HAY BALE BARRIERS WHERE NEEDED AND/OR AS DIRECTED BY THE ENGINEER.
  - SILT FENCE WHERE NEEDED AND/OR AS DIRECTED BY THE ENGINEER.
  - TEMPORARY SEDIMENTATION BASINS WHERE NEEDED AND/OR AS DIRECTED BY THE ENGINEER.
  - MULCHING AND SEEDING OF EXCESSIVE SLOPED AREAS AS NEEDED OR AS DIRECTED BY THE ENGINEER.
  - TEMPORARY VEHICLE TRACKING CONTROL AS NEEDED AND/OR DIRECTED BY THE ENGINEER.
  - CONCRETE WASH AREAS.

INLET PROTECTION.

THESE AND ALL EROSION CONTROL BEST MANAGEMENT PRACTICES AS SHOWN IN THE GRADING AND EROSION CONTROL PLANS SHALL BE STRICTLY ADHERED TO.

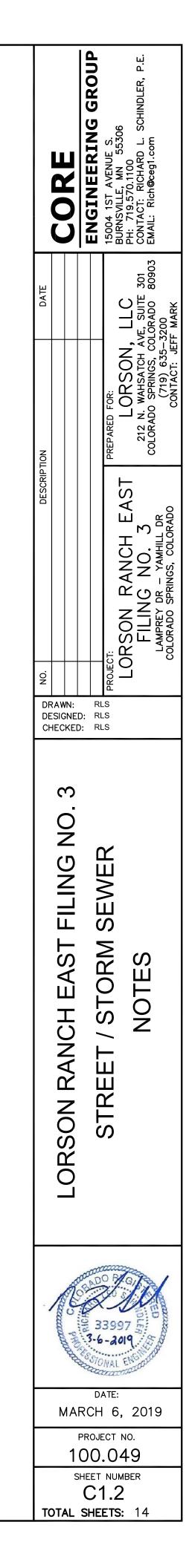
10. FINISHED CONTOURS/SPOT ELEVATIONS SHOWN HEREON REPRESENT FINISHED GRADES. ALL PAVEMENT SUBGRADES ARE BASED ON THE COMPOSITE ASPHALT PAVEMENT RECOMMENDATIONS MADE IN THE "GEOTECHNICAL STUDY" FOR LORSON RANCH.

## EL PASO COUNTY STANDARD CONSTRUCTION NOTES:

- 1. ALL DRAINAGE AND ROADWAY CONSTRUCTION SHALL MEET THE STANDARDS AND SPECIFICATIONS OF THE CITY OF COLORADO SPRINGS/EL PASO COUNTY DRAINAGE CRITERIA MANUAL, VOLUMES 1 AND 2, AND THE EL PASO COUNTY ENGINEERING CRITERIA MANUAL.
- 2. CONTRACTOR SHALL BE RESPONSIBLE FOR THE NOTIFICATION AND FIELD NOTIFICATION OF ALL EXISTING UTILITIES, WHETHER SHOWN ON THE PLANS OR NOT, BEFORE BEGINNING CONSTRUCTION. LOCATION OF EXISTING UTILITIES SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. CALL 811 TO CONTACT THE UTILITY NOTIFICATION CENTER OF COLORADO (UNCC).
- 3. CONTRACTOR SHALL KEEP A COPY OF THESE APPROVED PLANS, THE GRADING AND EROSION CONTROL PLAN, THE STORMWATER MANAGEMENT PLAN (SWMP), THE SOILS AND GEOTECHNICAL REPORT, AND THE APPROPRIATE DESIGN AND CONSTRUCTION STANDARDS AND SPECIFICATIONS AT THE JOB SITE AT ALL TIMES, INCLUDING THE FOLLOWING:
  - a. EL PASO COUNTY ENGINEERING CRITERIA MANUAL (ECM)
  - b. CITY OF COLORADO SPRINGS/EL PASO COUNTY DRAINAGE CRITERIA MANUAL, VOLUMES 1 AND 2 c. COLORADO DEPARTMENT OF TRANSPORTATION (CDOT) STANDARD SPECIFICATIONS FOR ROAD AND
- BRIDGE CONSTRUCTION d. CDOT M & S STANDARDS
- 4. NOTWITHSTANDING ANYTHING DEPICTED IN THESE PLANS IN WORDS OR GRAPHIC REPRESENTATION, ALL DESIGN AND CONSTRUCTION RELATED TO ROADS, STORM DRAINAGE AND EROSION CONTROL SHALL CONFORM TO THE STANDARDS AND REQUIREMENTS OF THE MOST RECENT VERSION OF THE RELEVANT ADOPTED EL PASO COUNTY STANDARDS, INCLUDING THE LAND DEVELOPMENT CODE, THE ENGINEERING CRITERIA MANUAL, THE DRAINAGE CRITERIA MANUAL, AND THE DRAINAGE CRITERIA MANUAL VOLUME 2. ANY DEVIATIONS FROM REGULATIONS AND STANDARDS MUST BE REQUESTED, AND APPROVED, IN WRITING. ANY MODIFICATIONS NECESSARY TO MEET CRITERIA AFTER-THE-FACT WILL BE ENTIRELY THE DEVELOPER'S RESPONSIBILITY TO RECTIFY.
- 5. IT IS THE DESIGN ENGINEER'S RESPONSIBILITY TO ACCURATELY SHOW EXISTING CONDITIONS, BOTH ONSITE AND OFFSITE, ON THE CONSTRUCTION PLANS. ANY MODIFICATIONS NECESSARY DUE TO CONFLICTS, OMISSIONS, OR CHANGED CONDITIONS WILL BE ENTIRELY THE DEVELOPER'S RESPONSIBILITY TO RECTIFY.
- 6. CONTRACTOR SHALL SCHEDULE A PRE-CONSTRUCTION MEETING WITH PLANNING AND COMMUNITY DEVELOPMENT (PCD) INSPECTIONS, PRIOR TO STARTING CONSTRUCTION.
- 7. IT IS THE CONTRACTOR'S RESPONSIBILITY TO UNDERSTAND THE REQUIREMENTS OF ALL JURISDICTIONAL AGENCIES AND TO OBTAIN ALL REQUIRED PERMITS, INCLUDING BUT NOT LIMITED TO EL PASO COUNTY EROSION AND STORMWATER QUALITY CONTROL PERMIT (ESQCP), REGIONAL BUILDING FLOODPLAIN DEVELOPMENT PERMIT, U.S. ARMY CORPS OF ENGINEERS-ISSUED 401 AND/OR 404 PERMITS, AND COUNTY AND STATE FUGITIVE DUST PERMITS.
- 8. CONTRACTOR SHALL NOT DEVIATE FROM THE PLANS WITHOUT FIRST OBTAINING WRITTEN APPROVAL FROM THE DESIGN ENGINEER AND PCD. CONTRACTOR SHALL NOTIFY THE DESIGN ENGINEER IMMEDIATELY UPON DISCOVERY OF ANY ERRORS OR INCONSISTENCIES.
- 9. ALL STORM DRAIN PIPE SHALL BE CLASS III RCP UNLESS OTHERWISE NOTED AND APPROVED BY PCD.
- 10. CONTRACTOR SHALL COORDINATE GEOTECHNICAL TESTING PER ECM STANDARDS. PAVEMENT DESIGN SHALL BE APPROVED BY EL PASO COUNTY PCD PRIOR TO PLACEMENT OF CURB AND GUTTER AND PAVEMENT.
- 11. ALL CONSTRUCTION TRAFFIC MUST ENTER/EXIT THE SITE AT APPROVED CONSTRUCTION ACCESS POINTS.
- 12. SIGHT VISIBILITY TRIANGLES AS IDENTIFIED IN THE PLANS SHALL BE PROVIDED AT ALL INTERSECTIONS. OBSTRUCTIONS GREATER THAN 18 INCHES ABOVE FLOWLINE ARE NOT ALLOWED WITHIN SIGHT TRIANGLES.
- 13. SIGNING AND STRIPING SHALL COMPLY WITH EL PASO COUNTY PUBLIC WORKS DEPARTMENT AND MUTCD CRITERIA.
- 14. CONTRACTOR SHALL OBTAIN ANY PERMITS REQUIRED BY EL PASO COUNTY PUBLIC WORKS DEPARTMENT, INCLUDING WORK WITHIN THE RIGHT-OF-WAY AND SPECIAL TRANSPORT PERMITS.
- 15. THE LIMITS OF CONSTRUCTION SHALL REMAIN WITHIN THE PROPERTY LINE UNLESS OTHERWISE NOTED. THE OWNER/DEVELOPER SHALL OBTAIN WRITTEN PERMISSION AND EASEMENTS, WHERE REQUIRED, FROM ADJOINING PROPERTY OWNER(S) PRIOR TO ANY OFF-SITE DISTURBANCE, GRADING, OR CONSTRUCTION.

## STORM SEWER NOTES:

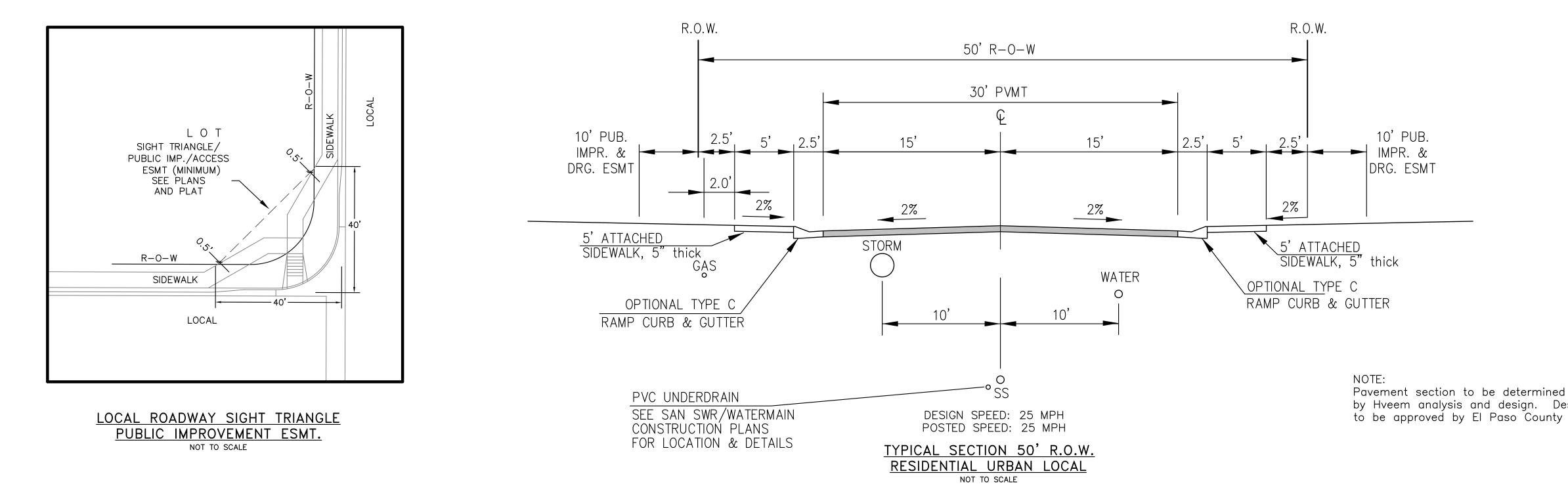
1. CONTRACTOR SHALL USE "TYLOX SUPER SEAL" OR APPROVED EQUL JOINT GASKET FOR ALL RCP STORM SEWER JOINTS



**AS-BUILT** 

STREET-STM

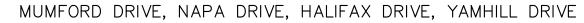
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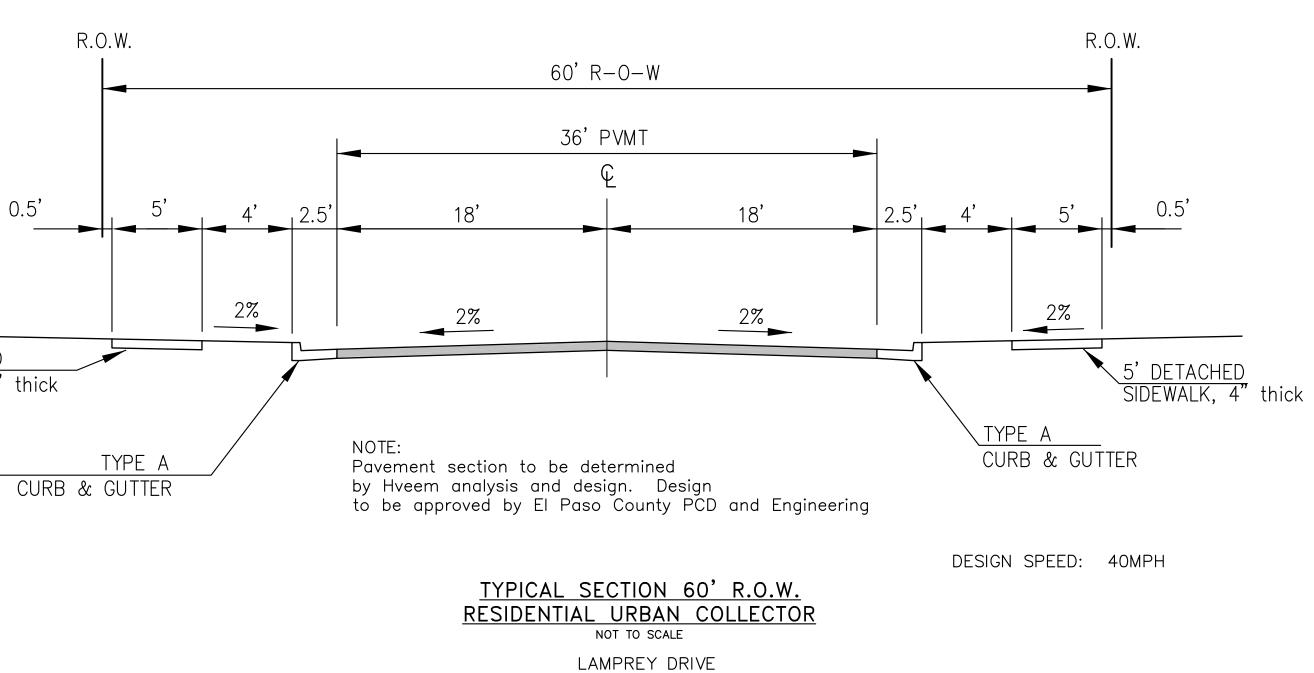


NOTE: ADDITIONAL PUBLIC IMPROVEMENT EASEMENTS ARE REQUIRED WHERE SIDEWALK ENCROACHES INTO THE PRIVATE LOTS. SEE CONSTRUCTION DRAWINGS AND THE FINAL PLAT. SEE CONSTRUCTION DRAWINGS AND PLAT FOR SIGHT TRIANGLES

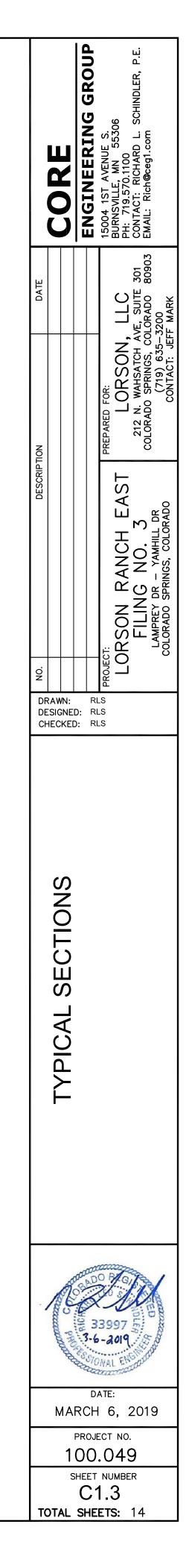
0.5'

5' DETACHED SIDEWALK, 4" thick

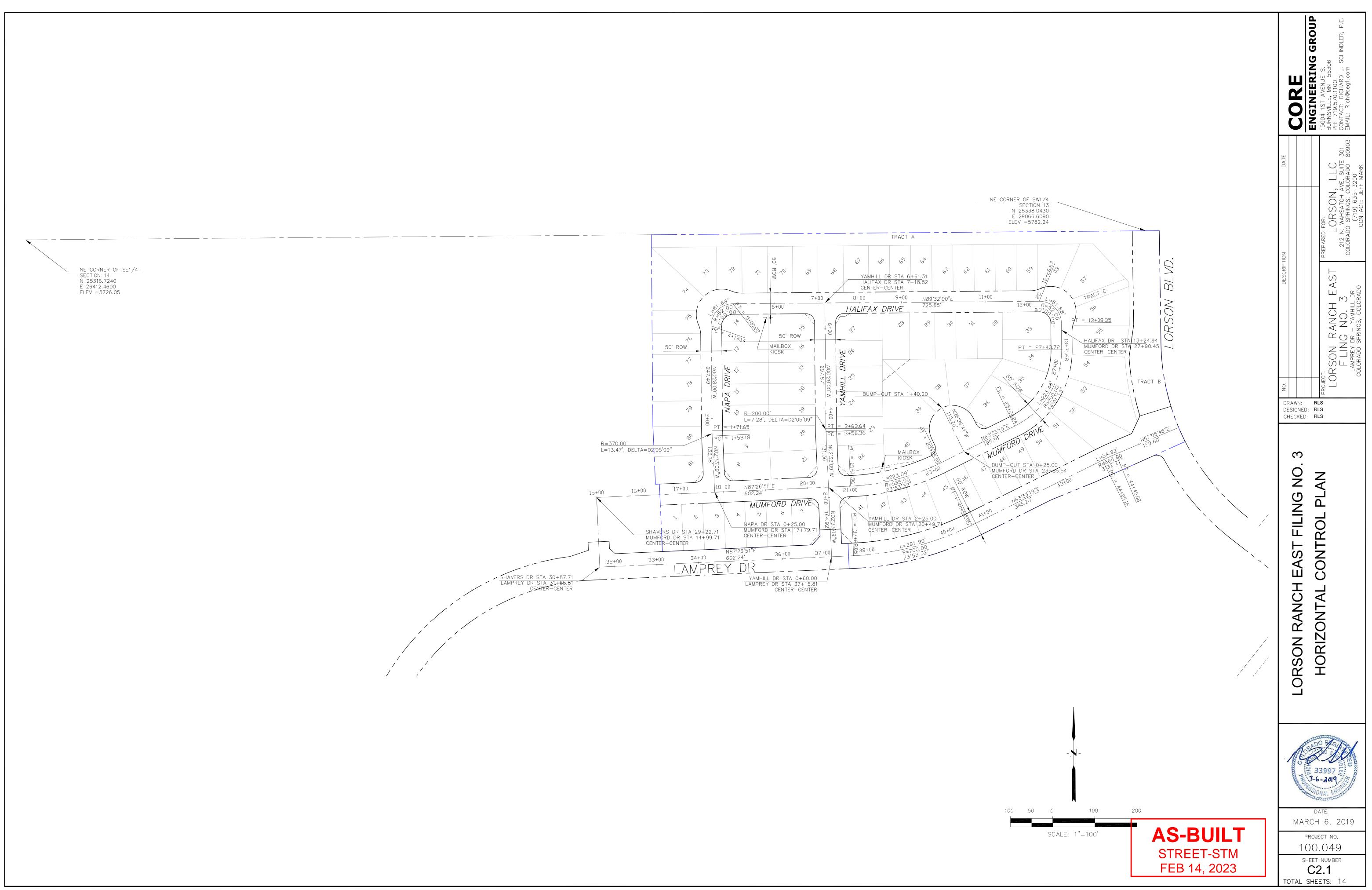




by Hveem analysis and design. Design to be approved by El Paso County PCD Engineering

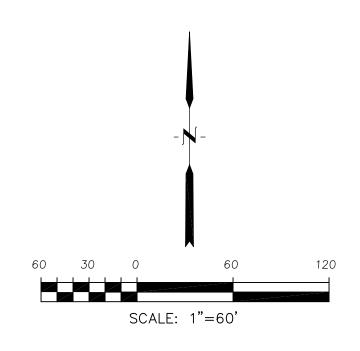






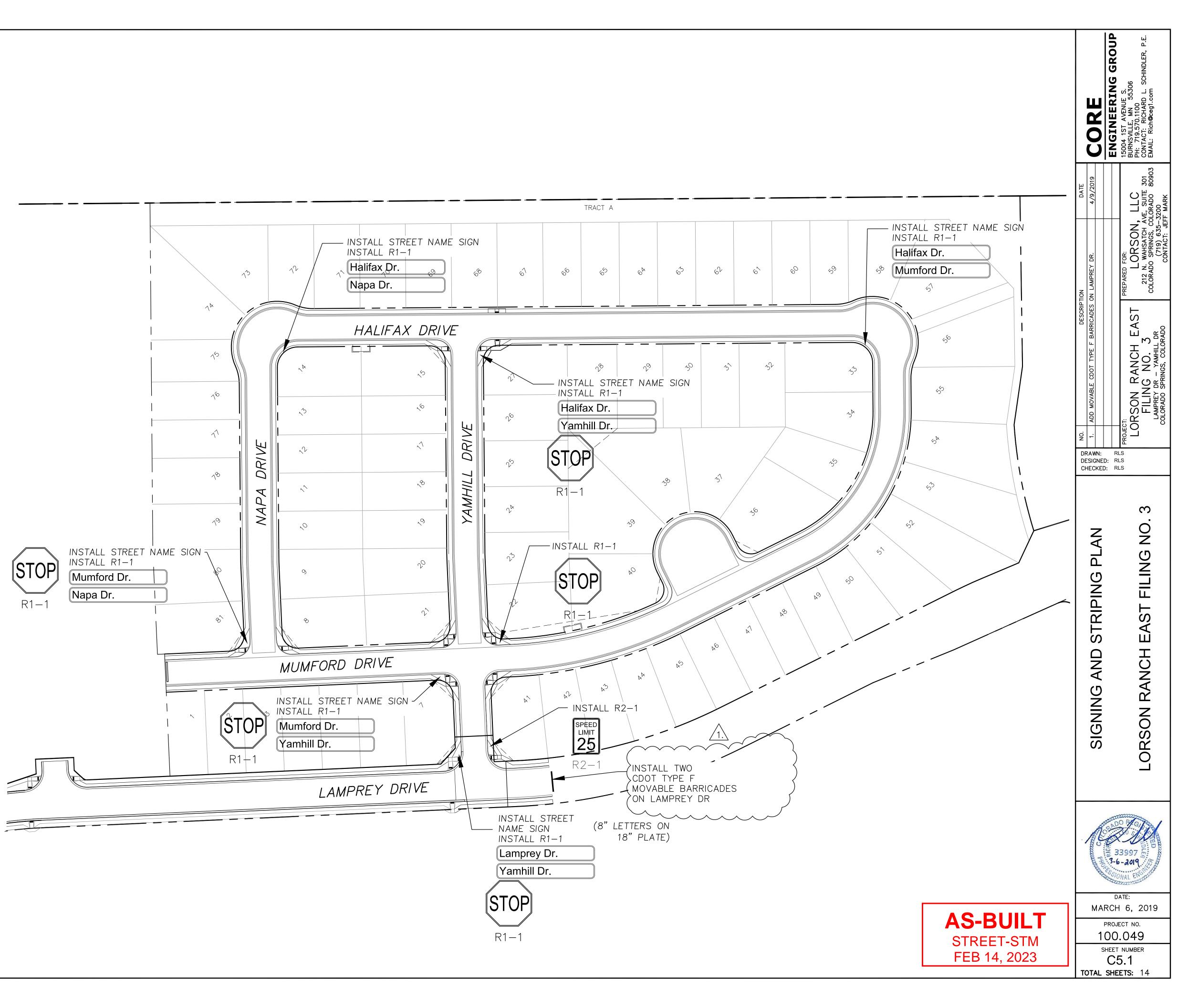
Notes:

1. Contractor must submit shop drawings to the engineer and to the county for approval prior to ordering signs

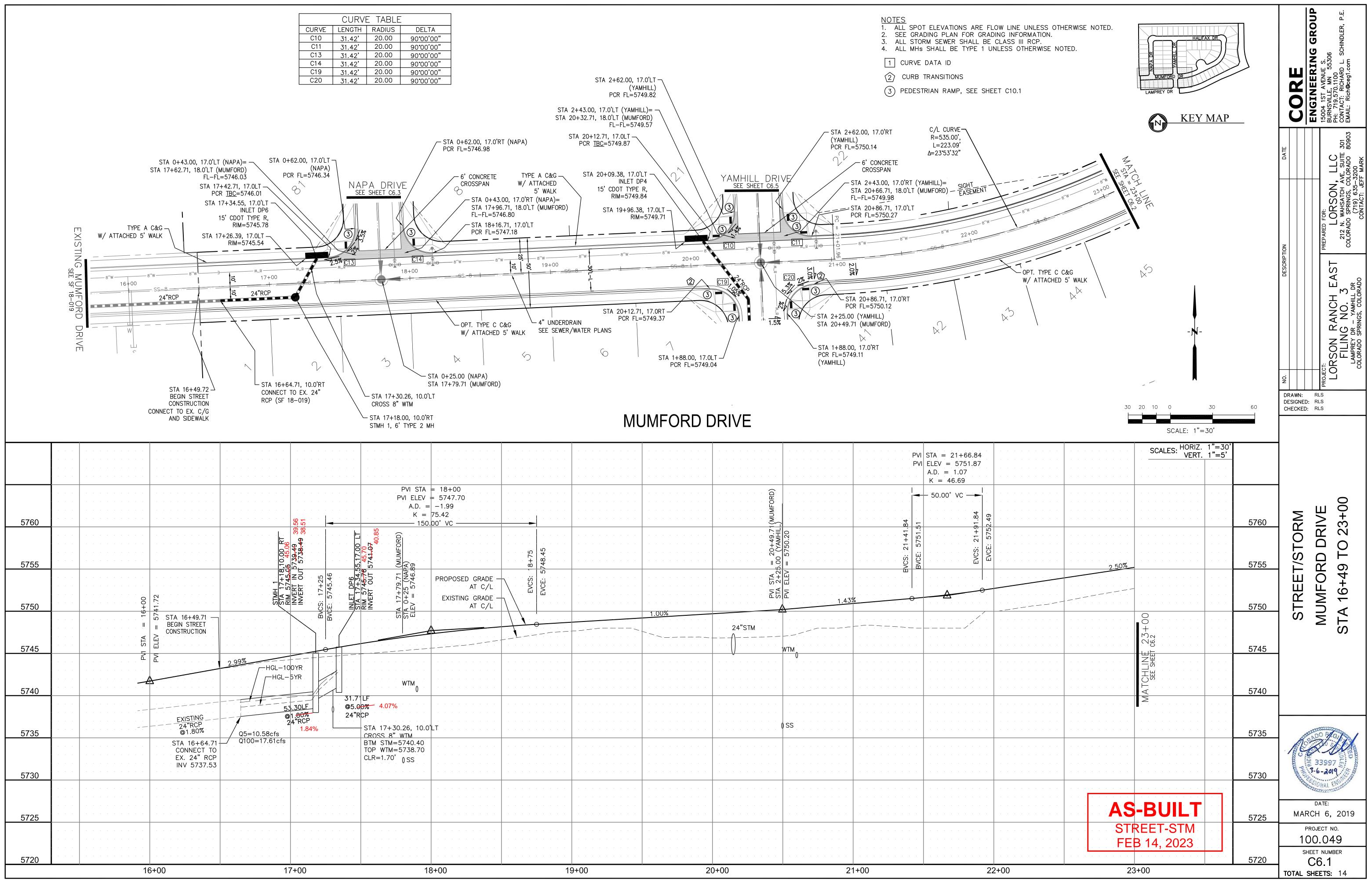


Signing and Striping Notes:

- 1. All signs and pavement markings shall be in compliance with the current Manual on Uniform Traffic Control Devices (MUTCD).
- 2. Removal of existing pavement markings shall be accomplished by a method that does not materially damage the pavement. The pavement markings shall be removed to the extent that they will not be visible under day or night conditions. At no time will it be acceptable to paint over existing pavement markings.
- 3. Any deviation from the striping and signing plan shall be approved by El Paso County Planning and Community Development. All signs shown on the signing and striping plan shall be new signs. Existing signs may remain or be reused if they meet current El Paso County Public Works Department and MUTCD standards.
- 4. Street name and regulatory stop signs shall be on the same post at intersections.
- 5. All removed signs shall be disposed of in a proper manner by the contractor.
- 6. All street name signs shall have "D" series letters, with local roadway signs being 4" upper-lower case lettering on 8" blank and non-local roadway signs being 6" lettering, upper-lower case on 12" blank, with a white border that is not recessed. Multi-lane roadways with speed limits of 40 mph or higher shall have 8" upper-lower case lettering on 18" blank with a white border that is not recessed. The width of the non-recessed white borders shall match page 255 of the 2012 MUTCD "Standard Highway Signs"
- 7. All traffic signs shall have a minimum High Intensity Prismatic grade sheeting.
- 8. All local residential street signs shall be mounted on a 1.75" x 1.75" square tube sign post and stub post base. For other applications, refer to the CDOT Standard S-614-8 regarding use of the P2 tubular steel post slipbase design.
- 9. All signs shall be single sheet aluminum with 0.100" minimum thickness.
  10. All limit lines/stop lines, crosswalk lines, pavement legends, and arrows shall be a minimum 125 mil thickness preformed thermoplastic pavement markings with tapered leading edges per CDOT Standard S-627-1. Word and symbol markings shall be the narrow type. Stop bars shall be 24" in width. Crosswalks lines shall be 12" wide and 8' long per CDOT S-627-1.
- All longitudinal lines shall be a minimum 15mil thickness epoxy paint. All non-local residential roadways shall include both right and left edge line striping and any additional striping as required by CDOT S-627-1.
   The contractor shall notify El Paso County Planning and Community
- Development (719) 520-6819 prior to and upon completion of signing and striping. 13. The contractor shall obtain a work in the right of way permit from
- the El Paso County Public Works Department prior to any signage or striping work within an existing El Paso County roadway.

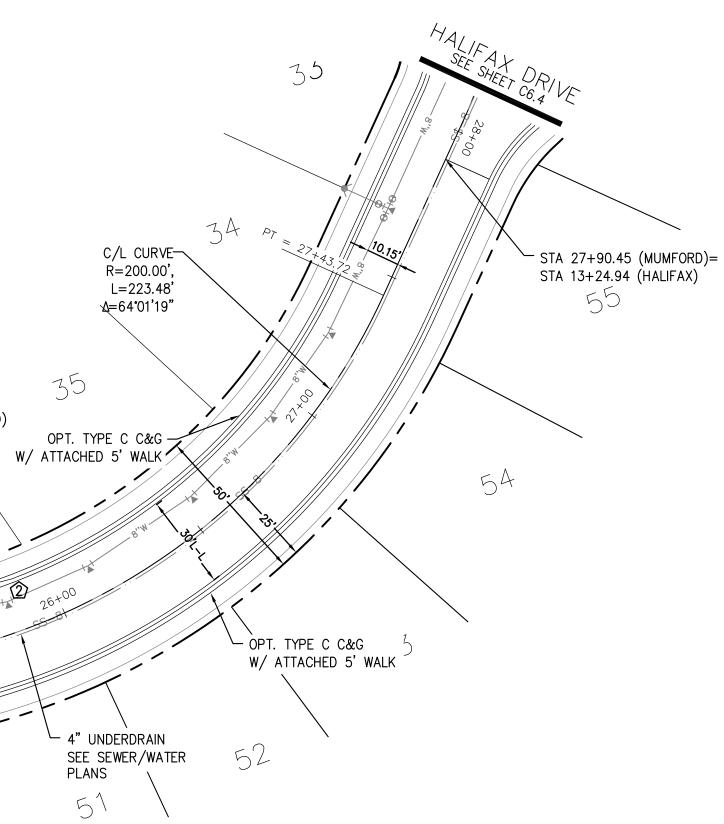


CURVE TABLE										
CURVE	LENGTH	RADIUS	DELTA							
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C11	31.42'	20.00	90°00'00"							
C13	31.42'	20.00	90°00'00"							
C14	31.42'	20.00	90 <b>°</b> 00'00"							
C19	31.42'	20.00	90°00'00"							
C20	31.42'	20.00	90 <b>°</b> 00'00"							

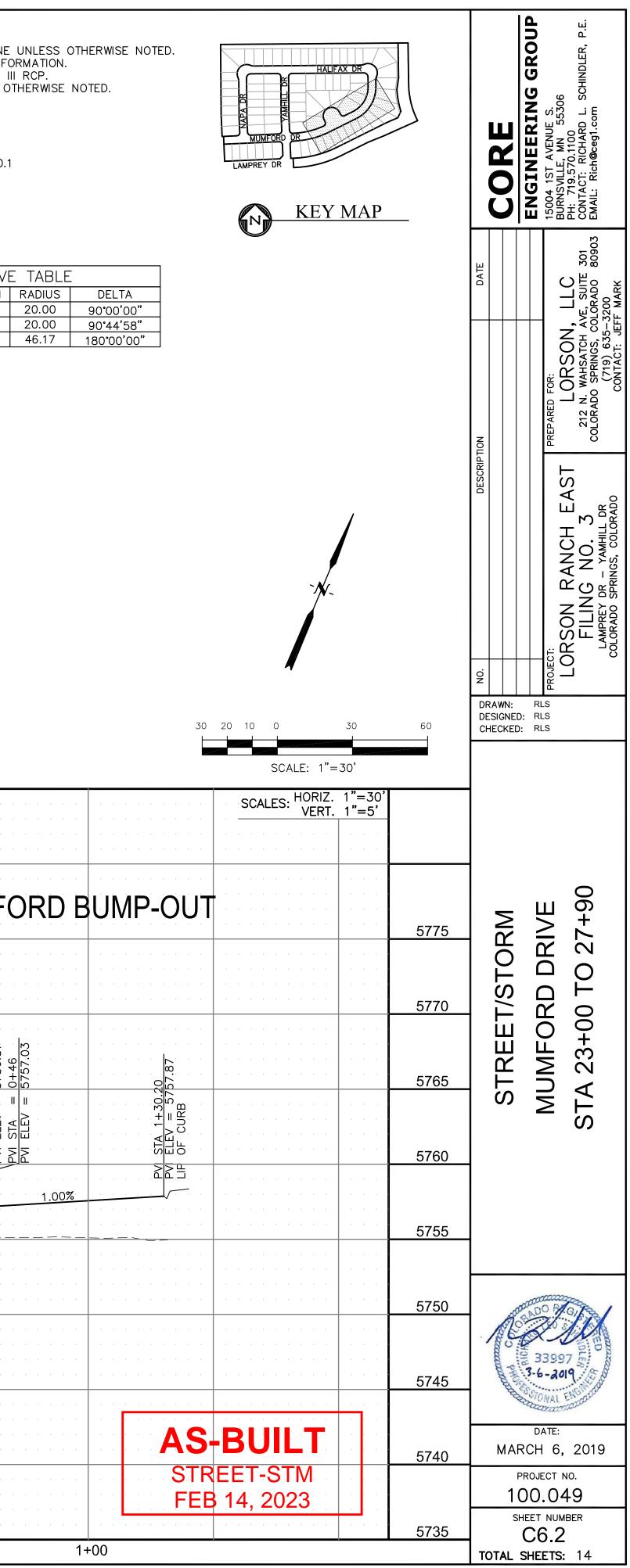


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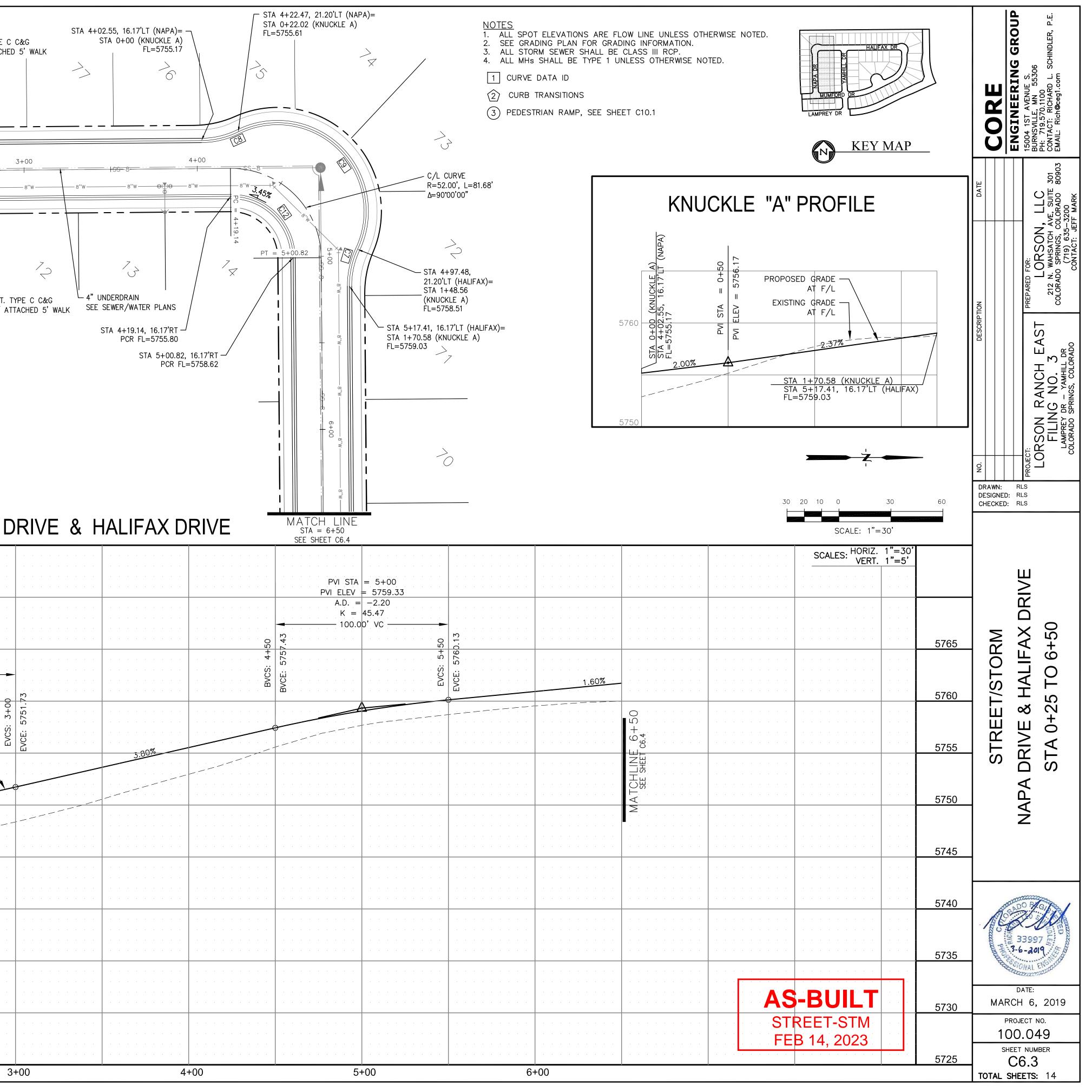
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<u>5775</u> 5770 5765		<th>.         .</th> <th>3+85.54 (MUMFORD) 10MP OUT) 757.33</th> <th>·         ·</th> <th>.         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .         .           .         .         .         .         .         .         .           .         .         .         .         .         .         .         .           .         .         .         .         .         .         .         .           .         .</th> <th>5759.12</th> <th></th> <th>PVI ELEV = 5761.12 A.D. = 3.21 K = 62.21 200.00' VC PROPOSED GF AT EXISTING GF</th> <th>ADE</th> <th>EVCE: 2766.33</th> <th>0.03 → 50.0 CE: 27+18 CE: 5768.58</th> <th>EVCS: 27+68 EVCE: 5770.75 A 27+90.45 (MU A 13+24.94 (HA EV = 5771.53</th> <th>.       .</th> <th>  .</th> <th>3+85.54 (MUMFORD) 10MP OUT) 757.33 0+40 5757.03 0+46 0+46 0+46</th>	.         .	3+85.54 (MUMFORD) 10MP OUT) 757.33	·         ·	.         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .           .         .         .         .         .         .           .         .         .         .         .         .         .           .         .         .         .         .         .         .         .           .         .         .         .         .         .         .         .           .         .	5759.12		PVI ELEV = 5761.12 A.D. = 3.21 K = 62.21 200.00' VC PROPOSED GF AT EXISTING GF	ADE	EVCE: 2766.33	0.03 → 50.0 CE: 27+18 CE: 5768.58	EVCS: 27+68 EVCE: 5770.75 A 27+90.45 (MU A 13+24.94 (HA EV = 5771.53	.       .	.	3+85.54 (MUMFORD) 10MP OUT) 757.33 0+40 5757.03 0+46 0+46 0+46
5770	TCHLINE 23+ See Sheet c6:1			$\begin{array}{rcl} PV & STA &=& 23+85.54 & (MUMFORD) \\ STA & 0+25 & (BUMP & OUT) \\ PV & ELEV &=& 5757.33 \\ \end{array}$			BVCS: 24+75 BVCE: 5759.12		PVI ELEV = 5761.12 A.D. = 3.21 K = 62.21 200.00' VC PROPOSED GF AT EXISTING GF	ADE	EVCE: 5766.	0.03 → 50.0 CE: 27+18 CE: 5768.58	EVCS: 27+68 EVCE: 5770.75 A 27+90.45 (MU A 13+24.94 (HA EV = 5771.53			PV STA = 23+85.54 (MUMFORD) STA 0+25 (BUMP OUT) PV ELEV = 5757.03 PVI STA = 0+40 PVI ELEV = 5757.03 PVI ELEV = 5756.87 PVI STA = 0+46 PVI STA = 0+46 PVI STA = 0+46 PVI STA = 0+46
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5770 5765 5760 5755 5750	ATCHLINE 23+ See Sheet c6:1			MIK       PVI STA       = 23+85.54 (MUMFORD)         MIK       STA       0+25 (BUMP OUT)         PVI ELEV       = 5757.33			BVCE: 5759.12		PVI ELEV = 5761.12 A.D. = 3.21 K = 62.21 200.00' VC PROPOSED GF AT EXISTING GF	ADE		0.03 → 50.0 CE: 27+18 CE: 5768.58	EVCS: 27+68 EVCE: 5770.75 A 27+90.45 (MU A 13+24.94 (HA EV = 5771.53		.       .	PVI STA = $23+85.54$ (MUMFORD) PVI STA = $23+85.54$ (MUMFORD) PVI STA 0+25 (BUMP OUT) PVI STA = $0+40$ PVI STA = $0+40$ PVI STA = $0+43$ PVI STA = $0+46$ PVI STA = $0+46$
5770 5765 5760 5755	MATCHLINE 23+			0       MIN         0       MIN         0       MIN         0       SIA         0       SIA					PVI ELEV = 5761.12         A.D. = 3.21         K = 62.21         200.00' VC         PROPOSED GF         AT         EXISTING GF         AT	ADE		0.03 → 50.0 CE: 27+18 CE: 5768.58	ELEV = 5771.53			State       State <td< th=""></td<>

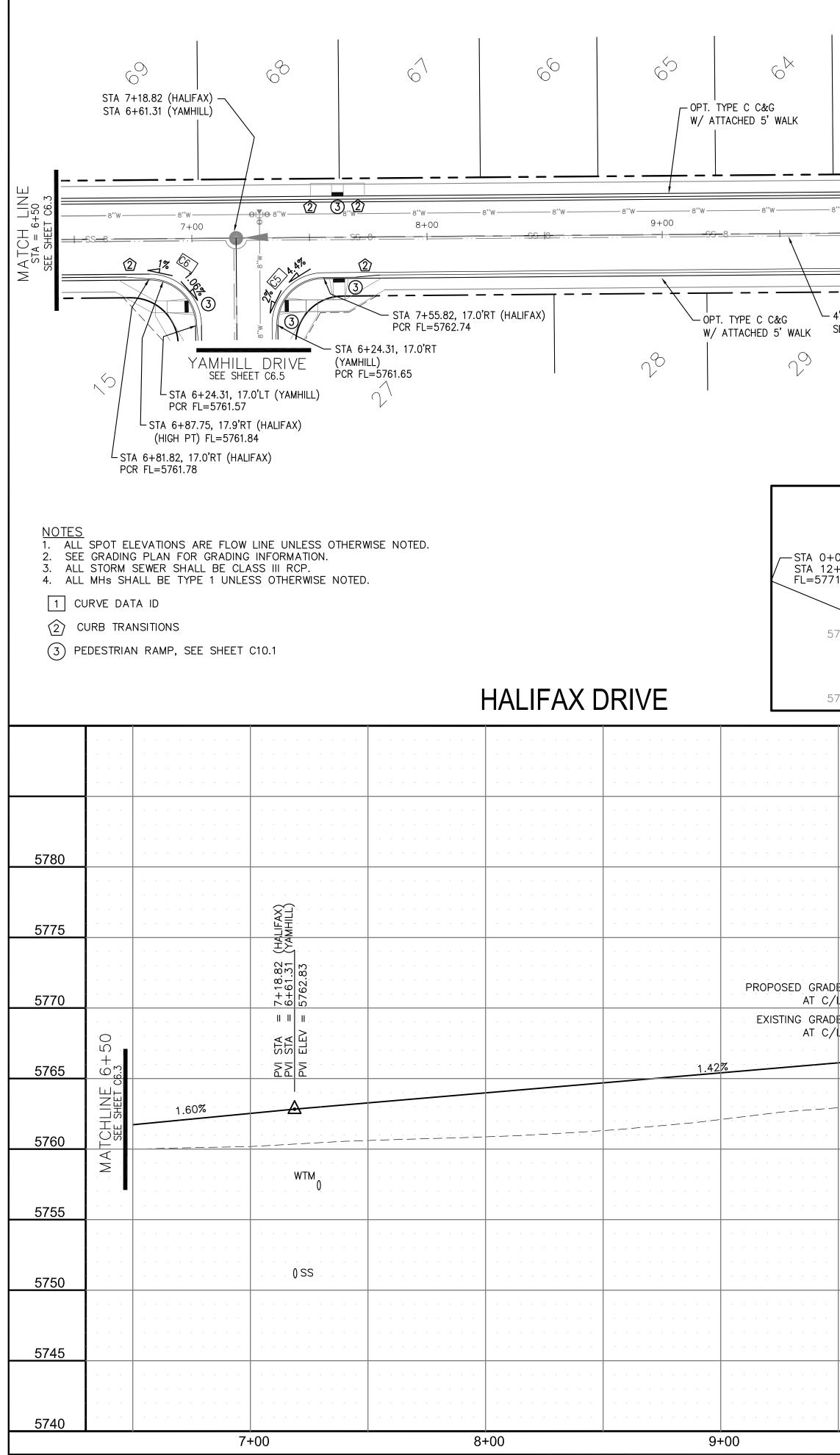


	CURV	-
CURVE	LENGTH	
C15	31.42'	
C16	31.68'	
C17	145.05'	

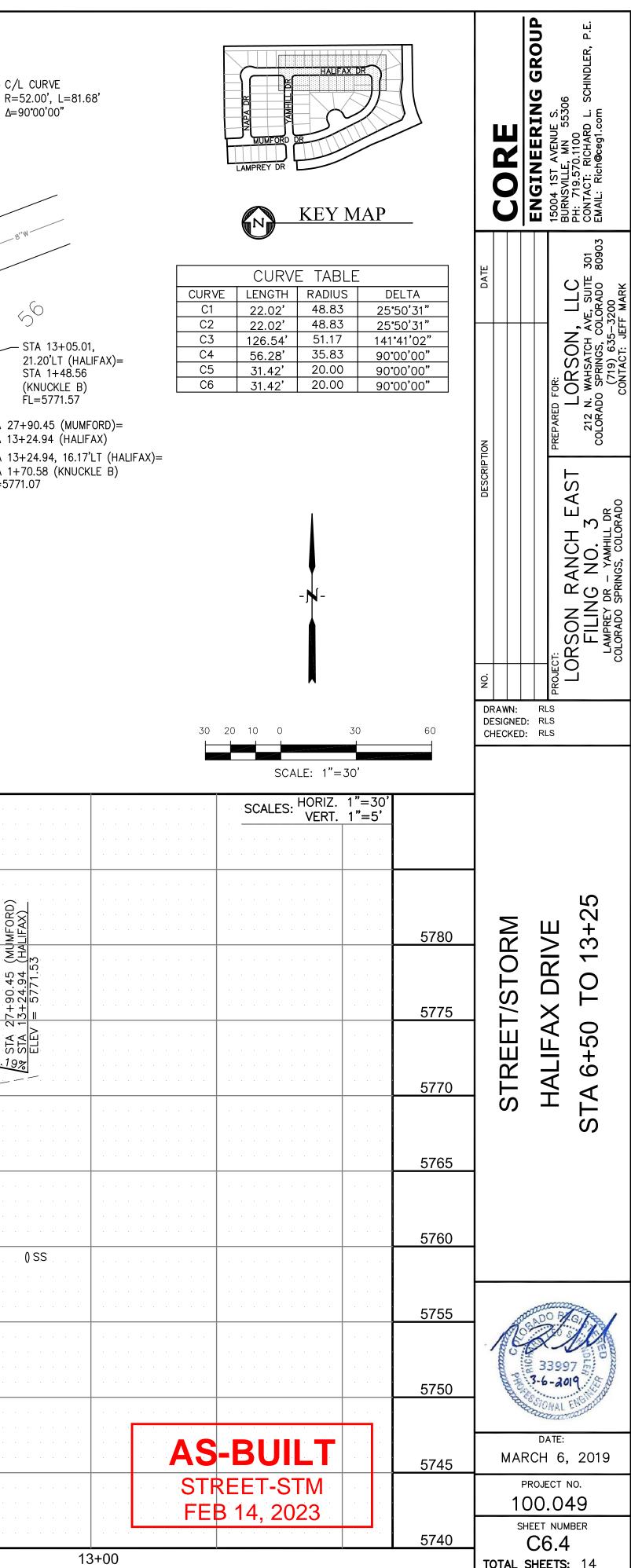


STA 0+2 STA 17+79.7	:5.00 (NAP	UMFOR D SEE SHEET CO I I I I I I I I I I I I I I I I I I I		STA FL- STA PCR RVE TABLE TH RADIUS 2' 48.83 2' 48.83 2' 48.83 54' 51.17 8' 35.83 2' 20.00	PCR IE - STA 0+ STA 17 FL-FL= STA (N PC - ST (N PC - ST (N PC - - - - - - - - - - - - -	TA 0+62.00, 17.0'LT IAPA) CR FL=5746.34 1+00 8'W 1+00 8'W 8'W 8'W 8'W 8'W 8'W 8'W 8'W 8'W 1+00 8'W 8'W 8'W 1+00 8'W 8'W 8'W 8'W 8'W 1+00 1+00 8'W 8'W 1+000 1+000 1+000 1		C/L CURVE R=370.00', L=13.47' Δ=02'05'09"	W B'W
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5765			<ul> <li></li> /ul>		<ul> <li></li> </ul>		.       .	PVI STA	
5765 5760				0+40 5746.59 0+43 5746.43 0+46	5746.59			PVI ELEV = A.D. = K = 4	= 2+50 5749.83 2.21
		ļ Ļ	· · ·	STA ELEV = ELEV = STA =	ELEV = ELEV		<b>5+</b> 00	PVI ELEV = A.D. = K = 4 100.0	= 2+50 = 5749.83 = 2.21 45.23
5760		PVI STA = 0+25 (NAPA) PVI STA = 17+79.71 (MUMFORD) PVI ELEV = 5746.89	· · ·	н. н. л. н. н.	►		BCS: BCS:	PVI ELEV = A.D. = K = 4 100.0 PROPOSEI	= 2+50 = 5749.83 2.21 45.23 00' VC
5760 5755		STA = 0+25 (NAPA) STA = 17+79.71 (MUMF ELEV = 5746.89		HAN STA -0+43.00		PROFILE=1.63% PROFILE=0.93%		PVI ELEV = A.D. = K = 4 100.0 PROPOSEI S EXISTING	= 2+50 = 5749.83 2.21 45.23 0' VC  0 GRADE AT C/L
5760 5755 5750		PVI STA = 0+25 (NAPA) PVI STA = 0+25 (NAPA) PVI STA = 17+79.71 (MUMF) PVI ELEV = 5746.89		PVI STA = PVI ELEV = PVI STA = PVI STA = PVI STA =		PROFILE=1.63% PROFILE=0.93%	1.59% STA 1+52.00	PVI ELEV = A.D. = K = 4 100.0 PROPOSEI S EXISTING	= 2+50 = 5749.83 2.21 45.23 0' VC  0 GRADE AT C/L
5760 5755 5750 5745 5740		00 PVI STA = 0+25 (NAPA) PVI STA = 0+25 (NAPA) PVI ELEV = 5746.89		= = = = = = = = = = = = = = = = = = =		PROFILE=1.63% PROFILE=0.93%	1.59% STA 1+52.00	PVI ELEV = A.D. = K = 4 100.0 PROPOSEI S EXISTING	= 2+50 = 5749.83 2.21 45.23 0' VC  0 GRADE AT C/L
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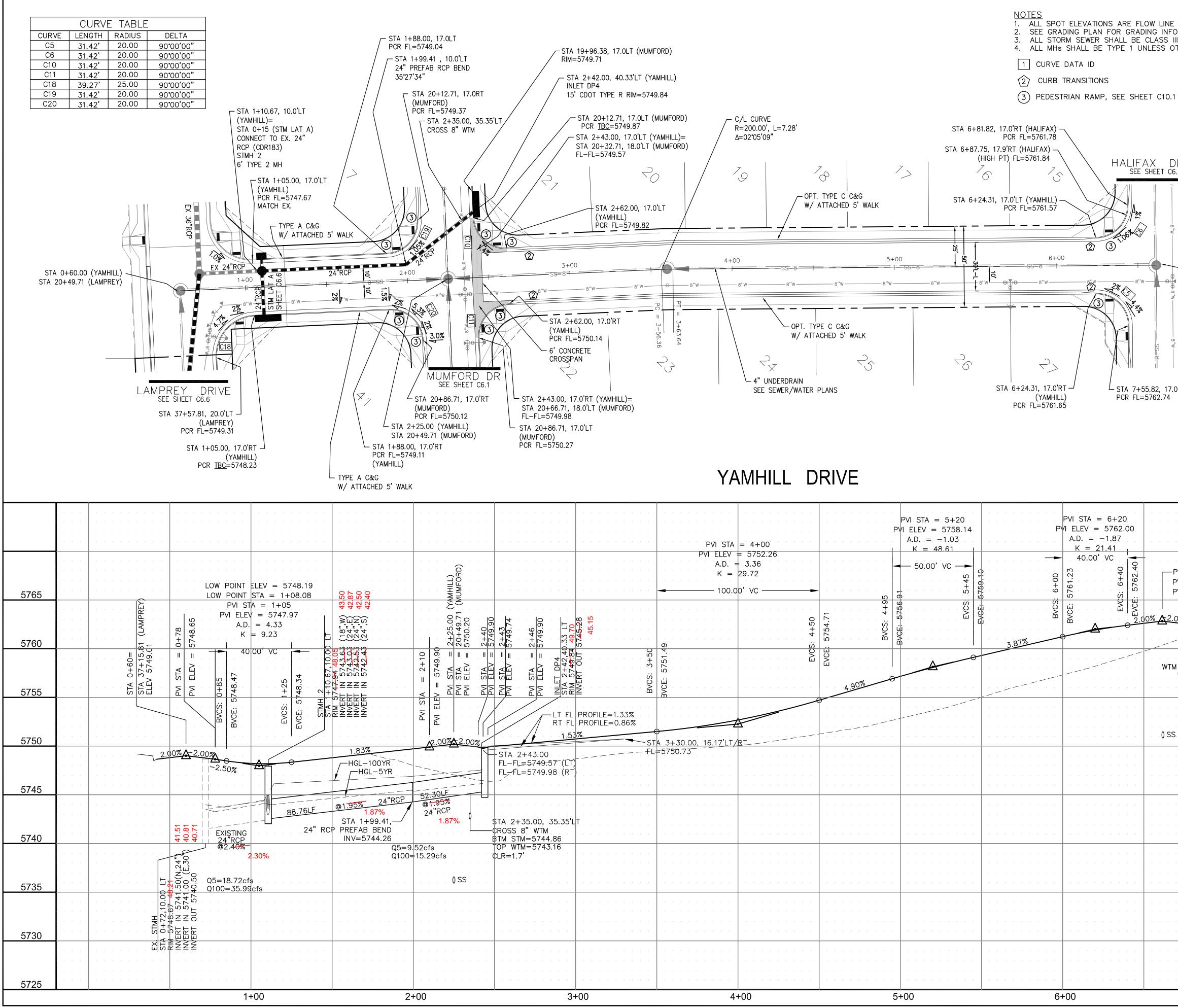




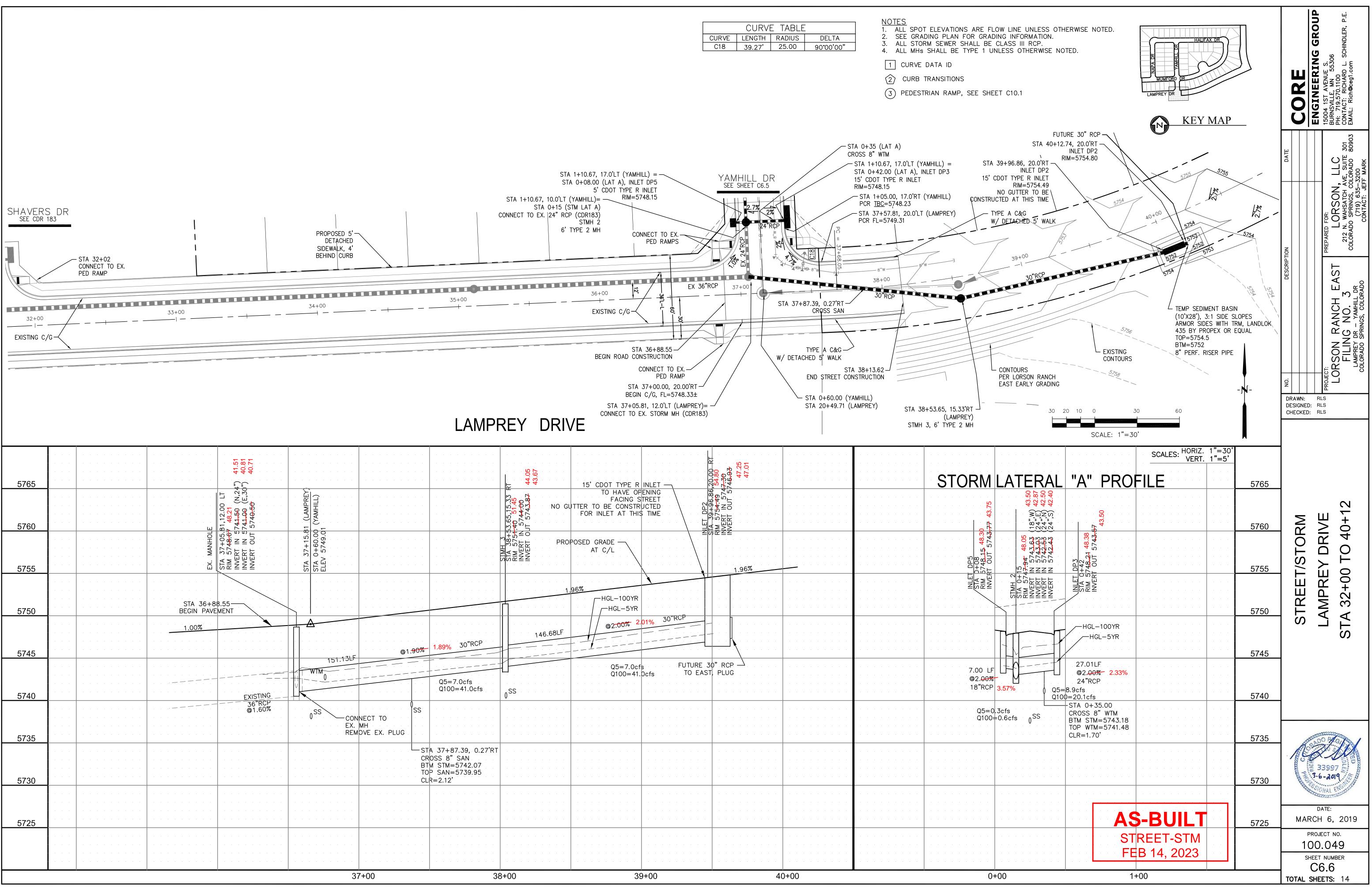
	63	62	6	STA 12+10.08, 16 STA 0	5.17'LT (HALIFAX)= +00 (KNUCKLE B) FL=5771.21	STA 12+30.0 STA 0+22.0 FL=5771.67	01, 21.20'LT (HALIFAX)= 2 (KNUCKLE B)	— C/L R=52
								R=52 Δ=90
— 8''W	8"W	€ + 8''W + 10' 30' + 25' 	-8"W	8"W8"W			BIN L	8''W 8''W
	UNDERDRAIN E SEWER/WATER PLANS	3	3V		STA 12+26.67,	PT = 13+08.35	ORD DRIVE SHEET C6.2	STA 27+ STA 27+ STA 13+ STA 13+ STA 13+
	K	40.00'				SEL	SHELI CO.2	FL=5771.
0+00 12+1 771.	) (KNUCKLE B) 10.08, 16.17'LT (HAL 21 <u>2.11%</u>	BVCE: 5772.58	- h A.D	T ELEV = $5772.78$ T STA = $0+84.31$ STA = $0+85$ EV = $5773.00$ $\therefore = -4.36$ $\therefore = 9.17$				
577 576		PROPOSED GRADE - AT F/L EXISTING GRADE - AT F/L		STA 1+ STA 13 STA 27 FL=577	<u>70.58 (KNUCKLE B)</u> +24.94, 16.17'LT (H/ +90.45, 16.17'RT (M 1.07	ALIFAX) UMFORD)		
						HIGH P' P	POINT ELEV = $5772$ POINT STA = $12+65$ VI STA = $12+66.08$ VI ELEV = $5773.41$ A.D. = $-6.30$ K = $12.69$	
· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · ·		PVI STA = PVI ELEV = A.D. = K = 1 SO.00	5768.24 1.69		2+25.08	80.00' VC	106.08
· · · ·	· · · · · · · · · · · ·		BVCS: 10+75 3VCE: 5767.89	EVCS: 11+25 EVCE: 5769.02		BVCS: 12 BVCS: 12		LEVCS: 13 EVCS: 13 EVCE: 5 STA 27+90.45 STA 13+24.94
RADE C/L RADE C/L					3.11%		· · · · · · · · · · · · · · · · · · ·	
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· · · · · · · · · · · · · · · · · · ·	<td< td=""><td></td><td></td><td>· · · · · · · · · · · ·</td><td></td><td></td><td>· · · · · · · · · · · · · · · · · · ·</td><td>· · · · · · · ·</td></td<>			· · · · · · · · · · · ·			· · · · · · · · · · · · · · · · · · ·	· · · · · · · ·
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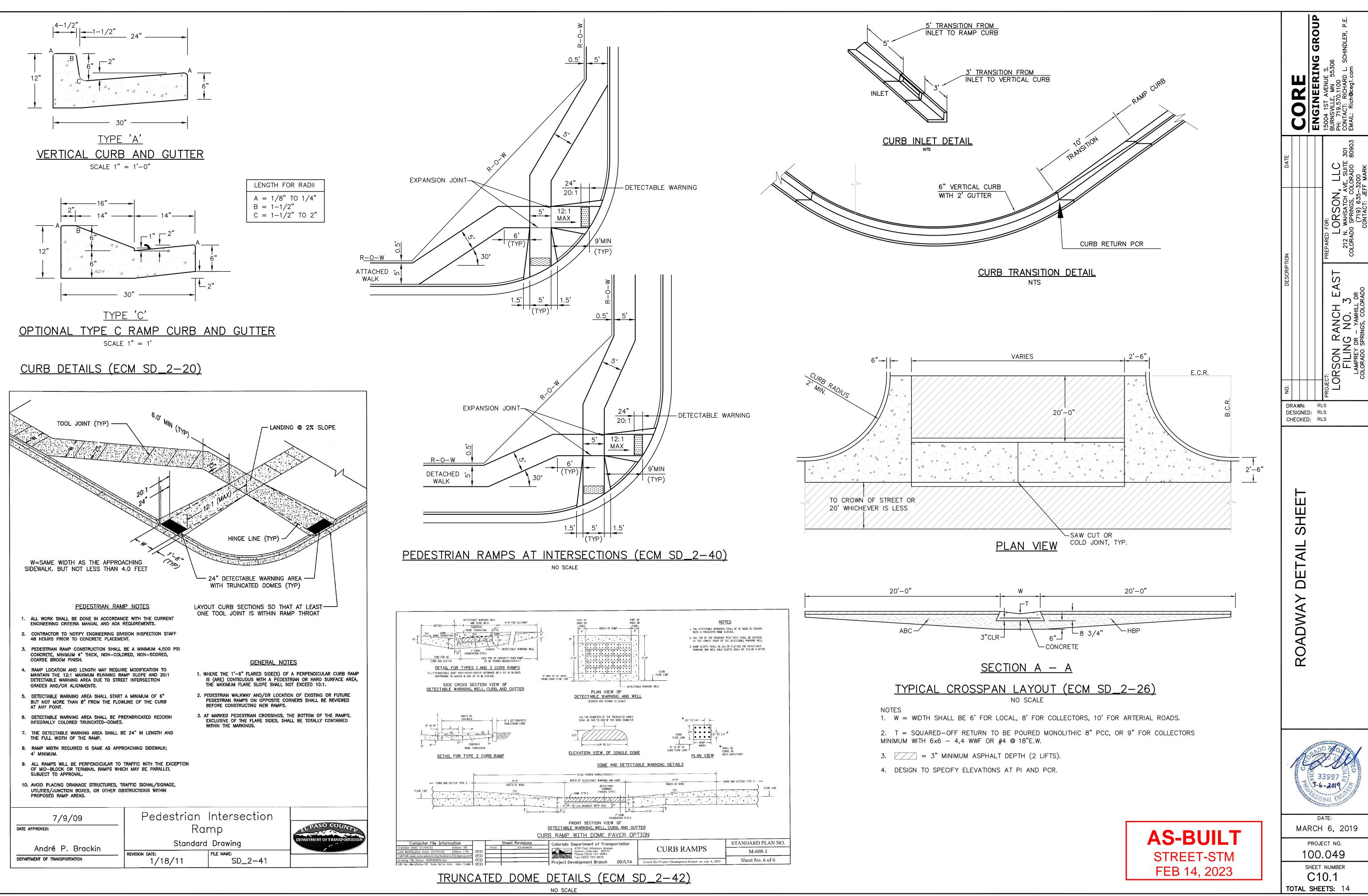
TOTAL SHEETS: 14



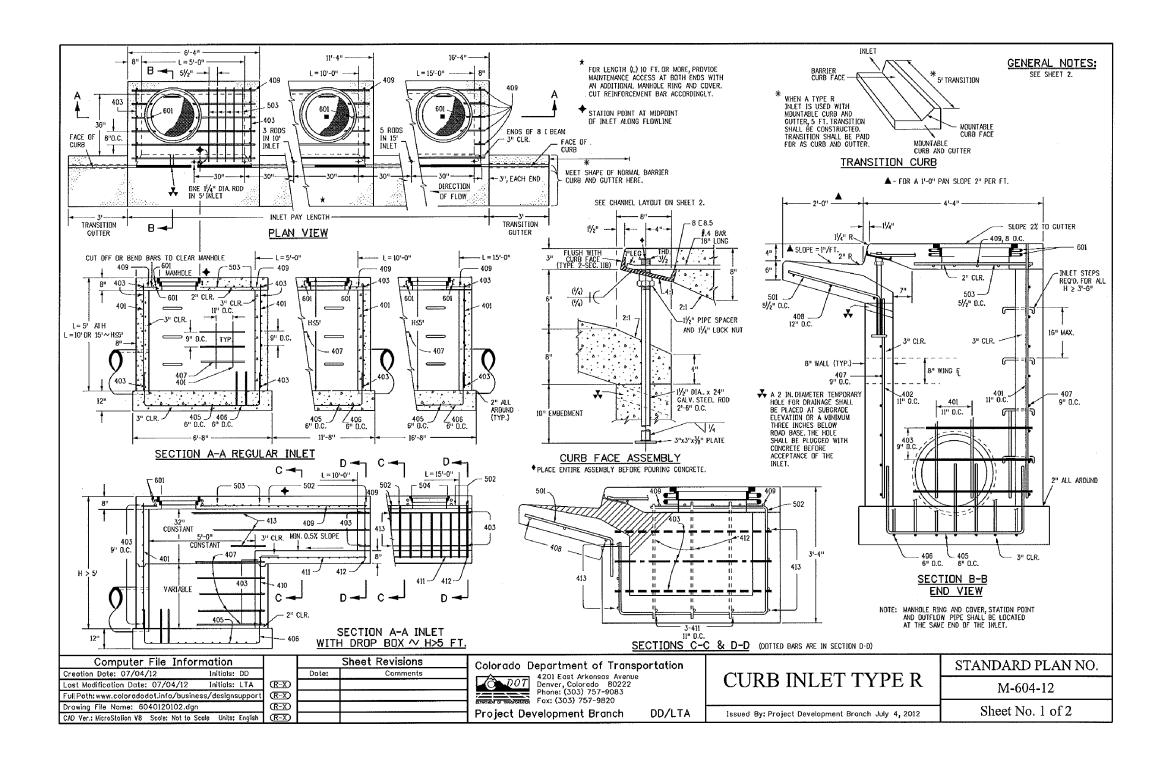
STA 19+96.38, 17.0LT (MUMFORD) RIM=5749.71 - STA 2+42.00, 40.33'LT (YAMHILI INLET DP4 15' CDOT TYPE R RIM=5749.84 STA 20+12.71, 17.0LT (MUMF PCR IBC=5749.87 - STA 2+43.00, 17.0'LT (YAMHI STA 20+32.71, 18.0'LT (MUMF FL-FL=5749.57 - STA 2+62.00, 17.0'LT (YAMHILL) PCR FL=5749.82 	L) FORD) $C/L C$ R=200 $\Delta=02^{\circ}$ FORD) $\zeta$ 4+00 4+00 8''W 8''W 8''W 8''W	CURVE 0.00', L=7.28' OPT. TYPE C W/ ATTACHEL SS 8 8'W 8'W OPT. TYPE C C&C W/ ATTACHED 5' C W/ ATTACHED 5' C W/ ATTACHED 5'	ST C&G D 5' WALK 	2. SEE 3. ALL 4. ALL 1 CU 2 CU 3 PE STA 6+81.82, 17.0'RT (HALIFA) (HIGH PT) FL=5761.8 6 STA 6+24.31, 17.0'LT (YAMP PCR FL=576 8''W 9 8''W 9 8''W 9 8''W 9 8'' STA 6+24.31, 17.0'LT (YAMP STA 6+24.31, 17.0'LT (YAMP STA 6+24.31, 17.0'LT (YAMP STA 6+24.31, 17.0'LT (YAMP STA 6+24.31, 17.0'LT (YAMP 1 CU 3 PE	A) HALIF SEE HALIF SEE HALIF SEE HALIF SEE HALIF SEE SEE SEE SEE SEE SEE SEE SE	DING INFORMATION. CLASS III RCP. JNLESS OTHERWISE NOTE EET C10.1	-	K)	P	NO.     DATE     DATE       NO.     DATE     DATE       ROLECT:     DATE     DATE       ROLECT:     ENCINERING GROUP       ROLECT:     PREPARED FOR:       ROLECT:     PREPARED FOR:       ROLECT:     PREPARED FOR:       ROLECT:     PREPARED FOR:       LORSON RANCH EAST     PREPARED RONCH FOR:       COLORADO SPRINCS, COLORADO BODOS     BURNSVILLE, MN 55306       LAMPREY DR - YAMHILL DR     COLORADO SPRINCS, COLORADO BOSOS       COLORADO SPRINCS, COLORADO BOSOS     EMALI: RICHARD L. SCHINDLER, P.E.       LAMPREY DR - YAMHILL DR     (719) 635-3200       COLORADO SPRINCS, COLORADO BOSOS     EMALI: RICHARD L. SCHINDLER, P.E.
D.27	YAM	IHILL DRIV	Έ	+20	PVI STA = 6+20		30 20	SCALE: 1"=30' SCALE: 1"=30' SCALES: HORIZ. 1"=3 VERT. 1"=5	60 ,	DRAWN: RLS DESIGNED: RLS CHECKED: RLS
	PVI STA = 4 PVI ELEV = 5 A.D. = 3.3 K = 29.7	752.26	PVI ELEV = 57 A.D. = -1.0 K = 48.6 	58.14	PVI ELEV = 5762.00 A.D. = -1.87 K = 21.41 40.00' VC					
L1 20 2.0 2.7 2.7 2.7 2.7 2.7 2.7 2.7 2.7	100.00' V		CS: 4+95 E: 5756.91	EVCS 5+4. EVCE: 5759.	BVCS: 6+00 BVCE: 5761.2 EVCS: 6+4 NEVCE: 5762.	PVI STA = 7+ PVI ELEV = 576	18.82 (HALIFAX) 52.83		5765	STORM DRIVE FO 6+61
STA 2+42,40.33 RIM 5749.84 49 INVERT OUT 574 BVCS: 3+50 BVCS: 3+50		EVCE: 21 EVCE:	m m m ↓.90%					.	5760	STREET/S YAMHILL STA 0+60 <sup>-</sup>
FL PROFILE=1.33% FL PROFILE=0.86% 1.53%STA_3- FL=57	+30.00, 16.17'LT/RT_ 50.73				.       .	· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · ·		5750	0, , , , , , , , , , , , , , , , , , ,
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35'LT	.     .     .     .     .     .     .       .     .     .     .     .     .     .       .     .     .     .     .     .     .	· · · · · · · · · · · · · · ·							5740	A Starter All
<td< td=""><td> </td><td>· · · · · · · · · · · · · · · · · · ·</td><td></td><td></td><td></td><td></td><td></td><td></td><td>5735</td><td>99 33997 55 3-6-2019 50 510 NAL ENGOLD</td></td<>		· · · · · · · · · · · · · · · · · · ·							5735	99 33997 55 3-6-2019 50 510 NAL ENGOLD
						· · · · · · · · · · ·		<b>BUILT</b> ET-STM 14, 2023	5730	DATE: MARCH 6, 2019 PROJECT NO. 100.049 SHEET NUMBER
3+00	4+00		5+00		6+00	7+0	0		5725	C6.5 TOTAL SHEETS: 14

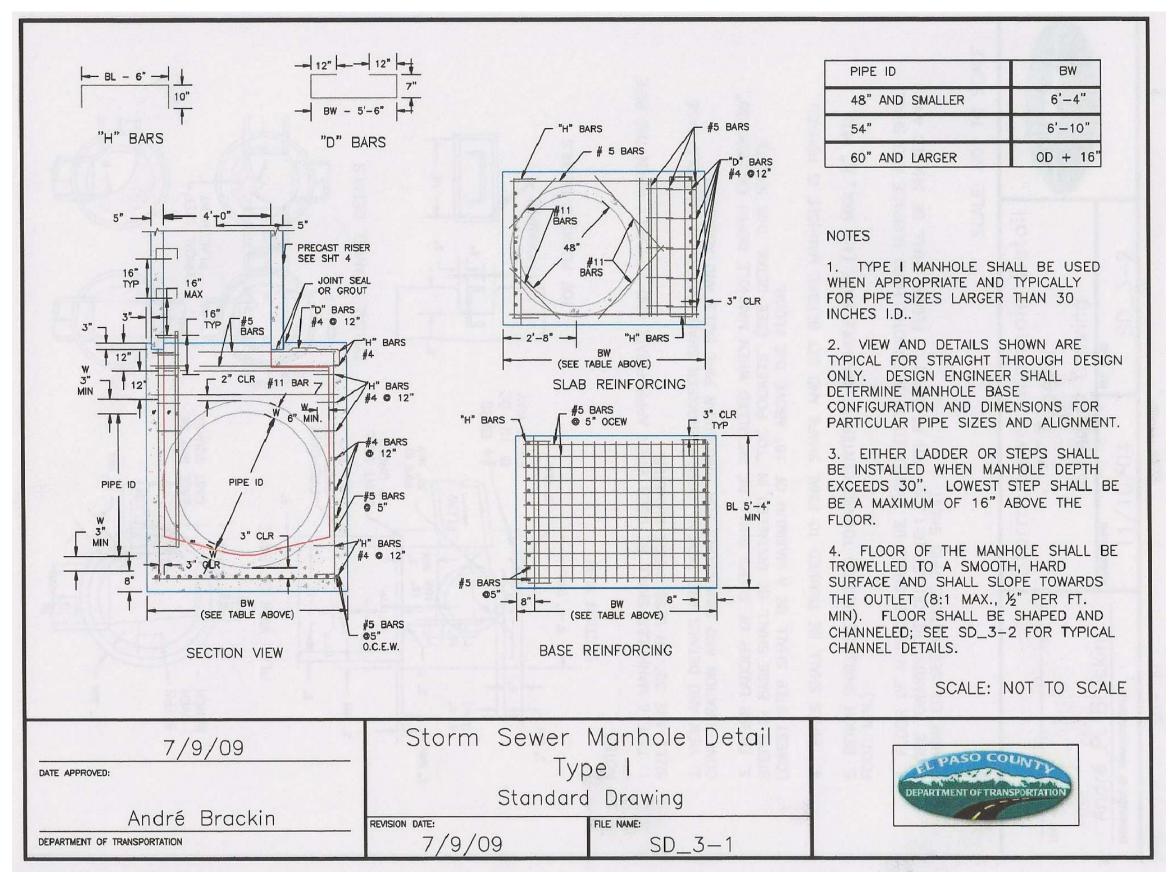


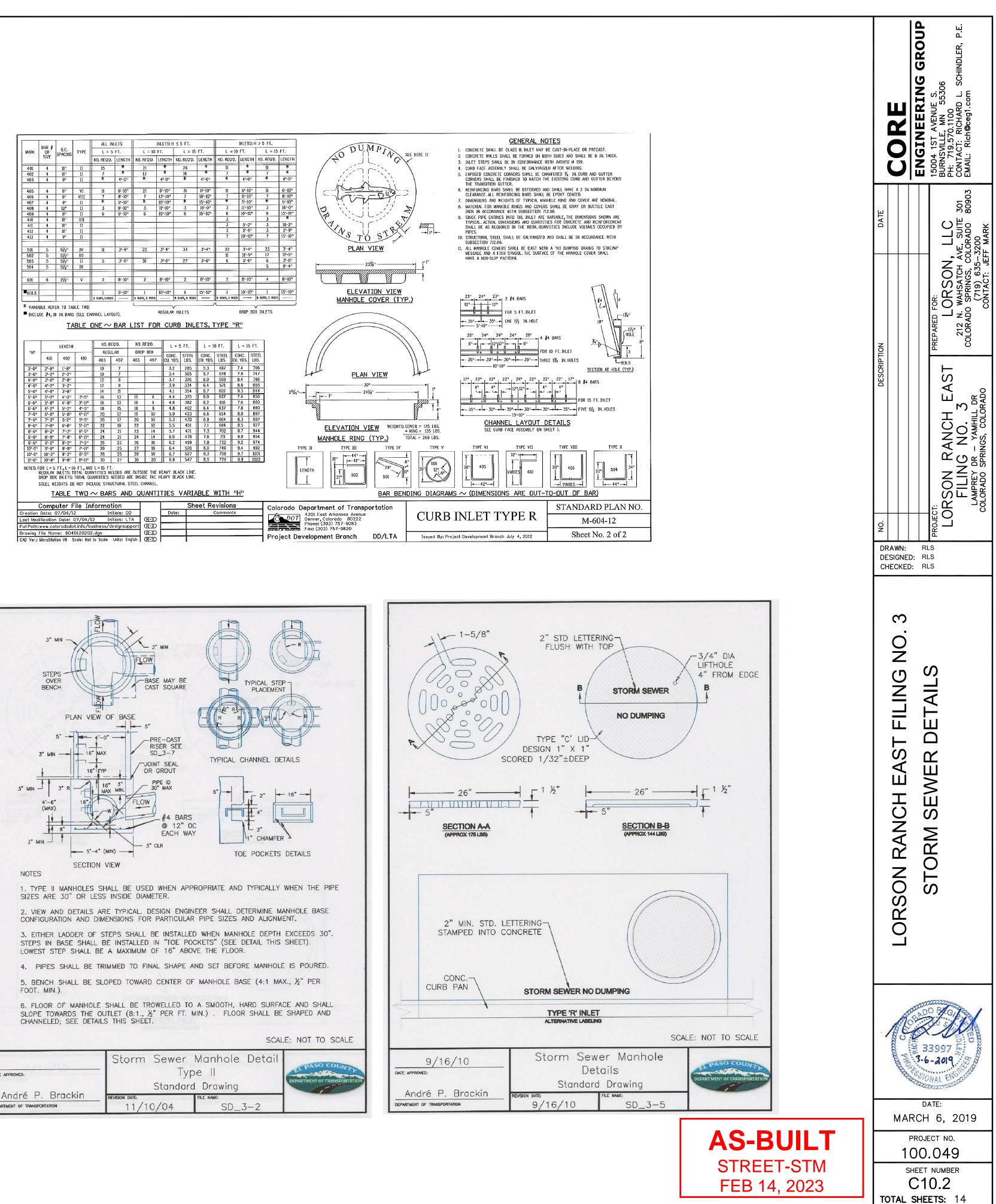
5725	 			· · · · · · ·						· · · · · · · · · ·
	 			· · · · · · ·						
5730	 								CL	P SAN=5739.95 R=2.12'
	 								BT	M STM=5742.07
	 									A 37+87.39, 0.27'R OSS 8" SAN
5735	 							OVE EX. PLUG		
	 				· · · ·	· · · · ·	e e eEX.	NECT TO MH		
	 				50%		s		SS	
5740	 			EXIST	ING					
	 				· · · · ·					Q5=7.0cfs Q100=41.0cfs
	 					- WT	· · · · · ·			
5745	 						151.13LF	@1	.90%	
	 									1.89% 30"RCP
	 		1.00%							
5750	 		STA 36+88 BEGIN PAVE	MENT	· · · · · · · ·					
5755	 		· · · · · ·	STA RIM INVE INVE		STA STA				
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	 		MANHOL	37+05. 5748.67 RT IN 5 RT IN 5 RT IN 5 RT OUT		37+15.81 0+60.00	/ 5749.01			
5760	 		 Lu	81,12 741,1 741,1 5741,0						
	 			2.00 5.00 0.50 0.50		(LAMPREY (YAMHILL)				
0100	 			) LT (N,24 (E,30		E SE	• • • • •			
5765	 									
	 			41.51 40.81 40.71						

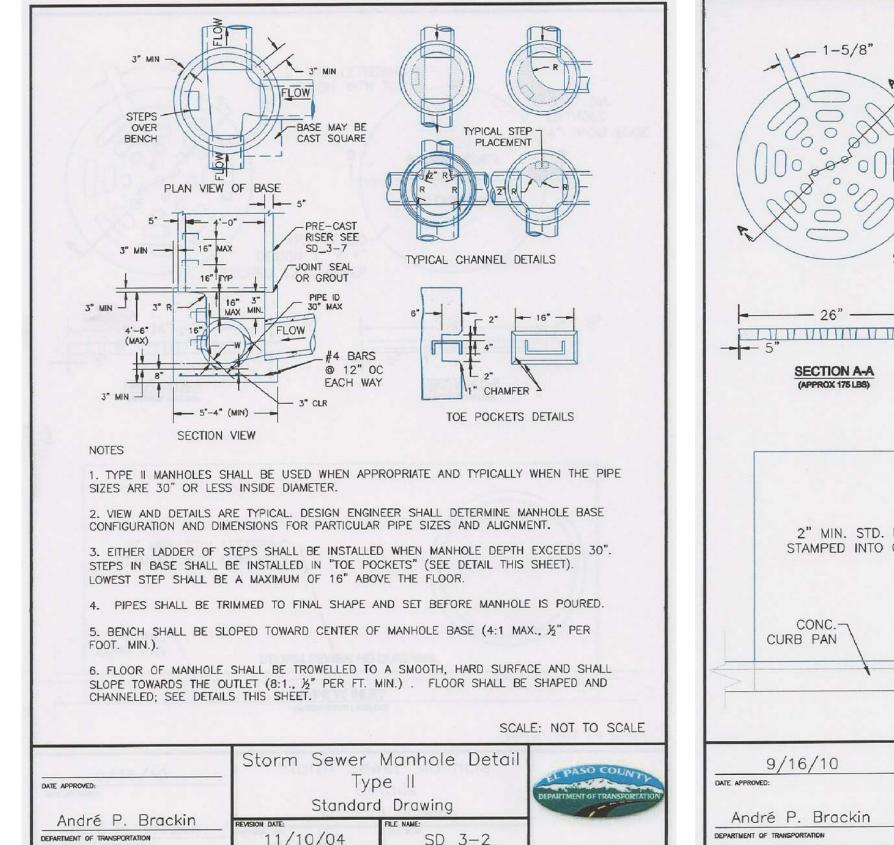


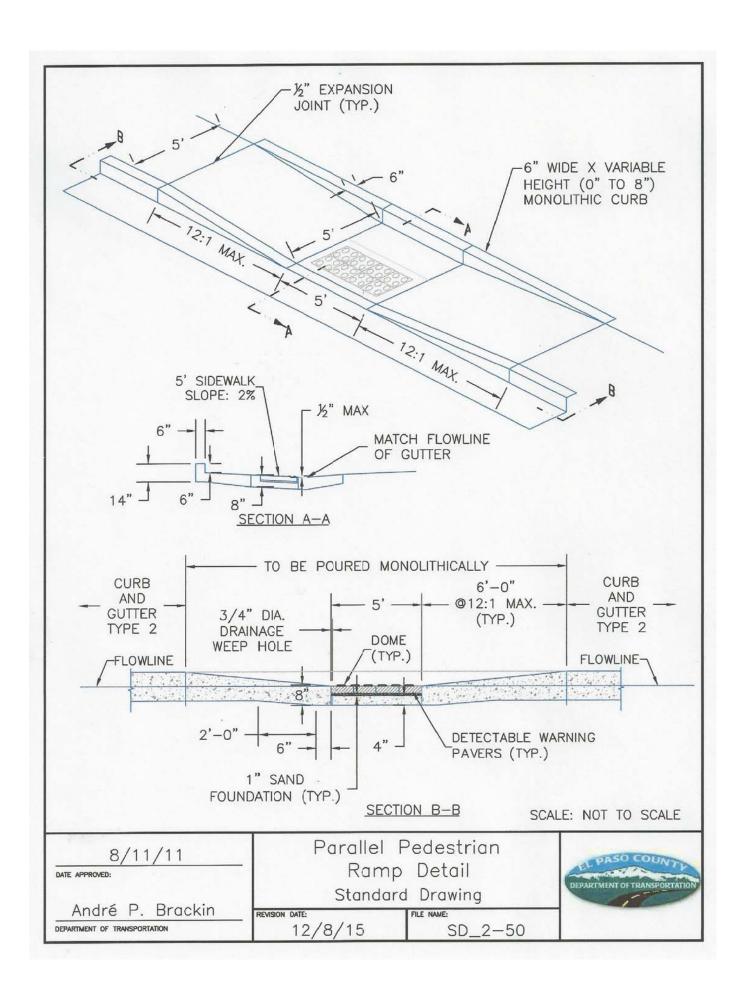
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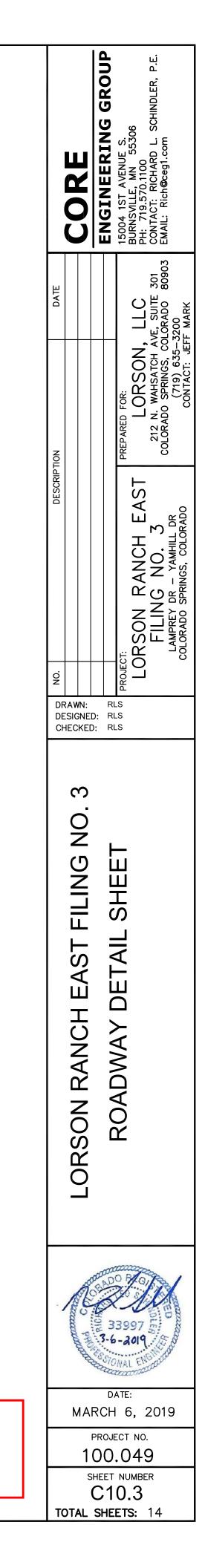












**AS-BUILT** 

STREET-STM

FEB 14, 2023