$\left.\begin{array}{lll}\text { TO：} & \begin{array}{l}\text { Loren Boreland } \\ \text { Classic Homes }\end{array} & \begin{array}{l}\text { MEMORANDUM }\end{array} \\ \text { FROM：} & \begin{array}{l}\text { Brandon Wilson，EIT } \\ \text { Zac Trotter，EIT } \\ \text { SM ROCHA，LLC }\end{array} \\ \text {（ight－in movement at the show } \\ \text { Vollmer access location }\end{array}\right\}$

## Analysis Objective

－Referencing the PrairieRidge Filing No． 1 Preliminary Plan Traffic Impact Study ${ }^{1}$ ，re－evaluate site distribution with consideration of the right－in／fight－out commercial access．
－Assess southbound vehicle queue lengths along Vollmer Road approaching Briargate Parkway，to verify if there is potential for vehicles to stack to，or beyond，the proposed right－ in／fight－out intersection with Vollmer Road．
－Analyze auxiliary lane requirements along Vollmer Road at the proposed right－in／fight－out access．
－Based on the objectives described above，a determination of access viability will be made．

## Site Development Background

Land for the development is vacant and surrounded predominantly by existing or proposed residential land uses．

As described and analyzed within the PrairieRidge Filing No． 1 Preliminary Plan Traffic Impact Study， the overall preliminary plan is understood to accommodate a maximum of 450 single－family dwelling units and approximately 4.5 acres of commercial land uses．

[^0][^1]Access to the commercial area of development is being proposed at the following locations:

- One previously analyzed right-in/right-out access onto the future Briargate Parkway (referred to as Commercial Access per the PrairieRidge Filing No. 1 Preliminary Plan Traffic Impact Study).
- One newly proposed right-in/fight-outaccess onto Vollmer Road (referred to as Vollmer Access).


## Existing and Committed Surface Transportation Network

As described within the PrairieRidge Filing No. 1 Preliminary Plan Traffic Impact Study, Vollmer Road and Briargate Parkway are the primary roadways that will accommodate traffic to and from the proposed development. A brief description of each roadway, based on the County's 2016 Major Transportation Corridors Plan (MTCP) ${ }^{2}$ and the County's Engineering Criteria Manual (ECM) ${ }^{3}$, as well as the City of Colorado Springs' Major Thoroughfare Plan ${ }^{4}$, is provided below:

Vollmer Road is a north-south, minor arterial roadway having two through lanes (one lane in each direction) with shared turn lanes at the intersections within the study area. Vollmer Road provides a posted speed limit of 45 MPH .

Pursuant to the County's 2040 MTCP Roadway Plan, Vollmer Road is envisioned to be widened from two to four through lanes from Briargate Parkway to Marksheffel Road, and remain as a two-lane roadway north of Briargate Parkway. Recently approved traffic studies ${ }^{5,6}$ for area development on the east side of Vollmer Road have proposed a modification to the MTCP Roadway Plan, recommending the widening of Vollmer Road to four through lanes between Briargate Parkway and Poco Road. The intermediate vision of Vollmer Road would remain as a two-lane roadway north of Poco Road and remain two lanes through the industrial segment south of Dines Boulevard and north of Sterling Ranch Filing 2.

Briargate Parkway is a future east-west, four-lane principal arterial roadway. Briargate Parkway design plans, for the portion east of Vollmer Road to Sterling Ranch Road, are understood to be under County review. The Briargate Parkway extension west of Vollmer Road to Black Forest Road, and ultimately to N Powers Boulevard, will be completed through various future private development or public improvement projects. Briargate Parkway is envisioned to provide a posted speed limit of 45 MPH .

[^2]The study intersections along Vollmer Road currently operate under stop-controlled conditions. A stop-controlled intersection is defined as a roadway intersection where vehicle rights-of-way are controlled by one or more "STOP" signs.

## Access Spacing - PrairieRidge Filing No. 1 Preliminary Plan

The PrairieRidge Filing No. 1 Preliminary Plan proposes approximately 630 feet of spacing along Vollmer Road from Briargate Parkway to proposed Vollmer Access (centerline-to-centerline) and approximately 760 feet of spacing along Vollmer Road from Sam Bass Drive to proposed Vollmer Access (centerline-to-centerline).

## Access Spacing - El Paso County Engineering Criteria Manual

Pursuant to Section 2.4 of the County's ECM, access drives shall be designed to accommodate various horizontal and vertical curve objectives, including but not limited to adequate spacing, clear sight distance requirements, and clearances from adjacent intersections.

Review of Table 2-35 of the County's ECM indicates a minimum access spacing distance of 900 feet is required along Vollmer Road based on entering sight distance requirements for access drives. However, Table 2-35 further states how, if auxiliary lanes are present, the entering posted speed for the deceleration lane and the posted speed at the end of the acceleration lane shall be used. As a result, access spacing requirements for the proposed Vollmer Access may be less than 700 feet.

In comparison of these access spacing requirements to the latest preliminary plan, it is believed that adequate access spacing is being provided along Vollmer Road.

## Trip Distribution \& Assignment

As referenced in the PrairieRidge Filing No. 1 Preliminary Plan Traffic Impact Study, the construction of the commercial development is assumed to occur within the build-out phase of development, to be completed by the end of Year 2040.

The overall directional distribution of site-generated traffic was re-evaluated in reference to the PrairieRidge Filing No. 1 Preliminary Plan Traffic Impact Study and with consideration for the newly proposed right-in/fight-out-commercial access on Vollmer Road.

Traffic assignment is how generated and distributed vehicle trips are expected to be loaded onto the available roadway network.

Applying updated trip distribution patterns to site-generated traffic for the build-out phase of development, as represented in the PrairieRidge Filing No. 1 Preliminary Plan Traffic Impact Study, provides the overall site-generated trip assignments shown in Figure 1.


Figure 1

## Future Traffic Conditions With Proposed Developments

Total traffic is the traffic projected to be on area roadways with consideration of the proposed development. Total traffic includes background traffic projections for Year 2040 with consideration of site-generated traffic.

Pursuant to area roadway improvement discussions provided in Section III of the PrairieRidge Filing No. 1 Preliminary Plan Traffic Impact Study, Year 2040 total traffic conditions assume no additional roadway improvements to accommodate regional transportation demands. Roadway improvements associated with site development are expected to be limited to site access and frontage as required by the governing agency.

Figures 2 and 3 show projected total traffic volumes and intersection geometry for Year 2040, respectively.


Figure 2
TOTAL TRAFFIC - YEAR 2040
Volumes
AM / PM Peak Hour
PrairieRidge Filing No. 1 Preliminary Plan
Traffic Memorandum
Hour

SM ROCHA, LLC
January 2024
Traffic and Transportation Consultants
Page 6


Figure 3
TOTAL TRAFFIC - YEAR 2040 Intersection Geometry

AM / PM Peak Hour
PrairieRidge Filing No. 1 Preliminary Plan
Traffic Memorandum
(ADT) : Average Daily Traffic

## Auxiliary Lane Analysis

Auxiliary lanes for the site development access are to be based on the County's ECM.
Considering development build-out, an evaluation of auxiliary lane requirements, pursuant to Section 2.3.7.D of the County's, reveals that a southbound right turn deceleration lane along Vollmer Road at the proposed right-in/right-out Vollmer Access is not required since the development's projected peak hour right turn ingress volume does not exceed the County's threshold of 50 VPH .

In further review of Section 2.3.7.D of the County's ECM, acceleration lanes onto minor arterial roadways are generally not required.

## Peak Hour Intersection Levels of Service - Total Traffic - Year 2040

The re-analyses and procedures described in this memorandum were performed in accordance with the Highway Capacity Manual (HCM) and are based upon the worst-case conditions that occur during a typical weekday upon build-out of site development and analyzed land uses. Therefore, study intersections are likely to operate with traffic conditions better than those described within this memorandum, which represent the peak hours of weekday operations only.

Upon re-analysis of long-term total traffic conditions, level of service (LOS) results for the intersection of Briargate Parkway and Vollmer Road remain unaffected by the newly proposed Vollmer Access, in comparison to LOS results illustrated within the PrairieRidge Filing No. 1 Preliminary Plan Traffic Impact Study (LOS C and D during the morning and afternoon peak traffic hour, respectively).

The newly proposed stop-controlled intersection of Vollmer Road and Vollmer Access is projected to have turning movement operations at LOS B during both the morning and afternoon peak traffic hours.

## Queue Length Analysis

Queue lengths for the study intersections were re-analyzed using Year 2040 total traffic conditions. The analysis yields estimate of $95^{\text {th }}$ percentile queue lengths, which only have a five percent probability of being exceeded during the analysis time period. An average vehicle length of 25 feet was assumed. Queue lengths were modeled and are included within the Synchro worksheets in Attachment A.

Table 1 summarizes the $95^{\text {th }}$ percentile queue results in comparison to the projected storage requirements for turn movements within the study area for Year 2040. Table 1 further provides recommended turn lane lengths based on minimum requirements from Section 2.3.7 of the County's ECM and projected $95^{\text {th }}$ percentile queue lengths.

Table 1 - Turn Lane Queues and Storage Requirements - Total Traffic - Year 2040

| Intersection | Turn Movement |  | Existing Turn Lane Length (feet) | AM Peak Hour | PM Peak Hour | Recommended <br> Turn Lane Length (feet) |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | 95th Percentile Queue Length (feet) | 95th Percentile Queue Length (feet) |  |
| Signalized Intersections |  |  |  |  |  |  |
| Briargate Parkway / Vollmer Road | EB | L |  | - | 46 ' | 258' | 260' |
|  |  | T | - | 354' | 822' | - |
|  |  | R | - | $0^{\prime}$ | 21' | 235' |
|  | WB | L | - | 207' | 185' | 235' |
|  |  | T | - | 612' | 712' | - |
|  |  | R | - | 9' | $0 '$ | 235' |
|  | NB | L | - | 141' | 297' | 300' |
|  |  | T | - | $90^{\prime}$ | 249' | - |
|  |  | R | - | 45' | 276 | 280' |
|  | SB | L | - | 99' | 107' | 235' |
|  |  | T | - | 208' | 232' | - |
|  |  | R | - | 55' | 9' | 235 |
| Stop-Controlled Intersections |  |  |  |  |  |  |
| Vollmer Road / Vollmer Access | EB | R | - | 3' | 8' | - |
|  | NB | T | - | $0 '$ | $0^{\prime}$ | - |
|  | SB | T,R | - | $0^{\prime}$ | $0^{\prime}$ | - |

Note: Turn Lane Length does not include taper length.

Table 1 shows that the maximum southbound queue length along Vollmer Road approaching Briargate Parkway is projected at 232 feet and occurs during the afternoon peak traffic hour.

Comparison of anticipated $95^{\text {th }}$ percentile queue lengths to the latest preliminary plan continues to indicate that adequate access spacing is being provided along Vollmer Road without negatively impacting the adjacent roadway network.

## Analysis Conclusion

This memorandum re-evaluated traffic impacts associated with the proposed PrairieRidge Filing No. 1 Preliminary Plan with consideration of a right-in/right-out access drive onto Vollmer Road, associated with the commercial area of development, and determined the proposed access drive's viability.

It is our professional opinion that the proposed right-in/right-outonto Vollmer Road is expected to create no negative impact to traffic operations for the surrounding roadway network and study intersections. Re-analysis of site-generated traffic concludes that changes to intersection operations and $95^{\text {th }}$ percentile queue lengths are minor in comparison to those analyzed within the PrairieRidge Filing No. 1 Preliminary Plan Traffic Impact Study, and all conclusions presented within the referenced traffic impact study remain valid. As a result, it is our determination that the proposed Vollmer Access functions efficiently and within the County's limits.

We trust that our findings will assist in the planning and approval of the proposed right-in/right-out Vollmer Access associated with the commercial area of development within the PrairieRidge Filing No. 1 Preliminary Plan. Please contact us should further assistance be needed.

Sincerely,
SM ROCHA, LLC
Traffic and Transportation Consultants


Zac Trotter, EIT
Traffic Engineer


## ATTACHMENT A

Capacity Worksheets

4: Vollmer Road \& Briargate Parkway

|  | 4 |  |  | 4 |  |  | $4$ | $\dagger$ |  |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 44 | F | ${ }^{7} 1$ | 44 | 「 | ${ }^{7}$ | 44 | F | ${ }^{7}$ | 44 | 7 |
| Traffic Volume (vph) | 69 | 855 | 71 | 373 | 1499 | 69 | 125 | 146 | 128 | 84 | 335 | 136 |
| Future Volume (vph) | 69 | 855 | 71 | 373 | 1499 | 69 | 125 | 146 | 128 | 84 | 335 | 136 |
| Satd. Flow (prot) | 1770 | 3539 | 1583 | 3433 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.091 |  |  | 0.950 |  |  | 0.261 |  |  | 0.651 |  |  |
| Satd. Flow (perm) | 170 | 3539 | 1583 | 3433 | 3539 | 1583 | 486 | 3539 | 1583 | 1213 | 3539 | 1583 |
| Satd. Flow (RTOR) |  |  | 155 |  |  | 109 |  |  | 155 |  |  | 155 |
| Lane Group Flow (vph) | 75 | 929 | 77 | 405 | 1629 | 75 | 136 | 159 | 139 | 91 | 364 | 148 |
| Turn Type | pm+pt | NA | Perm | Prot | NA | Perm | pm+pt | NA | Perm | pm+pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  | 2 |  |  | 6 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 5 | 2 | 2 | 1 | 6 | 6 | 3 | 8 | 8 | 7 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split (s) | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split (s) | 10.0 | 57.0 | 57.0 | 26.0 | 73.0 | 73.0 | 16.0 | 26.0 | 26.0 | 11.0 | 21.0 | 21.0 |
| Total Split (\%) | 8.3\% | 47.5\% | 47.5\% | 21.7\% | 60.8\% | 60.8\% | 13.3\% | 21.7\% | 21.7\% | 9.2\% | 17.5\% | 17.5\% |
| Yellow Time (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead/Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | None | None | None | None | None | None | Min | Min | None | Min | Min |
| Act Effct Green (s) | 47.8 | 42.5 | 42.5 | 17.4 | 57.8 | 57.8 | 29.8 | 21.8 | 21.8 | 21.0 | 14.7 | 14.7 |
| Actuated g/C Ratio | 0.45 | 0.40 | 0.40 | 0.16 | 0.55 | 0.55 | 0.28 | 0.21 | 0.21 | 0.20 | 0.14 | 0.14 |
| v/c Ratio | 0.48 | 0.65 | 0.11 | 0.72 | 0.84 | 0.08 | 0.53 | 0.22 | 0.31 | 0.33 | 0.74 | 0.42 |
| Control Delay | 24.5 | 27.8 | 0.3 | 51.5 | 25.4 | 1.0 | 40.9 | 40.8 | 7.2 | 37.9 | 56.0 | 10.7 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 24.5 | 27.8 | 0.3 | 51.5 | 25.4 | 1.0 | 40.9 | 40.8 | 7.2 | 37.9 | 56.0 | 10.7 |
| LOS | C | C | A | D | C | A | D | D | A | D | E | B |
| Approach Delay |  | 25.6 |  |  | 29.5 |  |  | 30.1 |  |  | 42.2 |  |
| Approach LOS |  | C |  |  | C |  |  | C |  |  | D |  |
| Queue Length 50th (ft) | 20 | 274 | 0 | 148 | 507 | 0 | 79 | 53 | 0 | 51 | 137 | 0 |
| Queue Length 95th (ft) | 46 | 354 | 0 | 207 | 612 | 9 | 141 | 90 | 45 | 99 | \#208 | 55 |
| Internal Link Dist (ft) |  | 412 |  |  | 884 |  |  | 915 |  |  | 549 |  |
| Turn Bay Length (ft) | 375 |  | 250 | 375 |  | 250 | 500 |  | 250 | 250 |  |  |
| Base Capacity (vph) | 155 | 1818 | 888 | 712 | 2377 | 1099 | 277 | 748 | 456 | 274 | 559 | 380 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.48 | 0.51 | 0.09 | 0.57 | 0.69 | 0.07 | 0.49 | 0.21 | 0.30 | 0.33 | 0.65 | 0.39 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 105.6 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 80 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Actuated-Uncoordinated |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 0.84 |  |  |  |  |  |  |  |  |  |  |  |  |

Intersection Signal Delay: 30.4 Intersection Capacity Utilization 78.5\% Analysis Period (min) 15
\# 95th percentile volume exceeds capacity, queue may be longer. Queue shown is maximum after two cycles.

Splits and Phases: 4: Vollmer Road \& Briargate Parkway




4: Vollmer Road \& Briargate Parkway

|  | 4 | $\rightarrow$ |  | 7 |  |  | $4$ | $\dagger$ |  |  | $\dagger$ | $\downarrow$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Lane Group | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ${ }^{7}$ | 44 | 7 | ${ }^{*} 1$ | 44 | 「 | ${ }^{7}$ | 44 | F | ${ }^{7}$ | 44 | 「 |
| Traffic Volume (vph) | 230 | 1460 | 114 | 346 | 1247 | 82 | 300 | 459 | 368 | 104 | 273 | 121 |
| Future Volume (vph) | 230 | 1460 | 114 | 346 | 1247 | 82 | 300 | 459 | 368 | 104 | 273 | 121 |
| Satd. Flow (prot) | 1770 | 3539 | 1583 | 3433 | 3539 | 1583 | 1770 | 3539 | 1583 | 1770 | 3539 | 1583 |
| Flt Permitted | 0.075 |  |  | 0.083 |  |  | 0.286 |  |  | 0.469 |  |  |
| Satd. Flow (perm) | 140 | 3539 | 1583 | 300 | 3539 | 1583 | 533 | 3539 | 1583 | 874 | 3539 | 1583 |
| Satd. Flow (RTOR) |  |  | 155 |  |  | 200 |  |  | 231 |  |  | 200 |
| Lane Group Flow (vph) | 250 | 1587 | 124 | 376 | 1355 | 89 | 326 | 499 | 400 | 113 | 297 | 132 |
| Turn Type | pm+pt | NA | Perm | pm+pt | NA | Perm | pm+pt | NA | Perm | pm+pt | NA | Perm |
| Protected Phases | 5 | 2 |  | 1 | 6 |  | 3 | 8 |  | 7 | 4 |  |
| Permitted Phases | 2 |  | 2 | 6 |  | 6 | 8 |  | 8 | 4 |  | 4 |
| Detector Phase | 5 | 2 | 2 | 1 | 6 | 6 | 3 | 8 | 8 | 7 | 4 | 4 |
| Switch Phase |  |  |  |  |  |  |  |  |  |  |  |  |
| Minimum Initial (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Minimum Split (s) | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 | 10.0 |
| Total Split (s) | 23.0 | 58.0 | 58.0 | 16.0 | 51.0 | 51.0 | 32.0 | 30.0 | 30.0 | 16.0 | 14.0 | 14.0 |
| Total Split (\%) | 19.2\% | 48.3\% | 48.3\% | 13.3\% | 42.5\% | 42.5\% | 26.7\% | 25.0\% | 25.0\% | 13.3\% | 11.7\% | 11.7\% |
| Yellow Time (s) | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 | 3.0 |
| All-Red Time (s) | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Lost Time Adjust (s) | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Lost Time (s) | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 | 5.0 |
| Lead/Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag | Lead | Lag | Lag |
| Lead-Lag Optimize? | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes | Yes |
| Recall Mode | None | Max | Max | Max | Max | Max | Max | Min | Min | Max | Min | Min |
| Act Effct Green (s) | 67.8 | 53.0 | 53.0 | 59.0 | 48.0 | 48.0 | 41.0 | 25.0 | 25.0 | 20.0 | 9.0 | 9.0 |
| Actuated g/C Ratio | 0.56 | 0.44 | 0.44 | 0.49 | 0.40 | 0.40 | 0.34 | 0.21 | 0.21 | 0.17 | 0.08 | 0.08 |
| v/c Ratio | 0.84 | 1.02 | 0.16 | 0.87 | 0.96 | 0.12 | 0.71 | 0.68 | 0.78 | 0.50 | 1.12 | 0.44 |
| Control Delay | 54.5 | 60.3 | 1.9 | 49.3 | 51.7 | 0.3 | 41.4 | 49.1 | 30.4 | 38.0 | 141.7 | 5.5 |
| Queue Delay | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Total Delay | 54.5 | 60.3 | 1.9 | 49.3 | 51.7 | 0.3 | 41.4 | 49.1 | 30.4 | 38.0 | 141.7 | 5.5 |
| LOS | D | E | A | D | D | A | D | D | C | D | F | A |
| Approach Delay |  | 55.8 |  |  | 48.7 |  |  | 40.9 |  |  | 86.9 |  |
| Approach LOS |  | E |  |  | D |  |  | D |  |  | F |  |
| Queue Length 50th (ft) | 136 | $\sim 660$ | 0 | 100 | 545 | 0 | 203 | 188 | 127 | 61 | ~139 | 0 |
| Queue Length 95th (ft) | \#258 | \#822 | 21 | \#185 | \#712 | 0 | 297 | 249 | \#276 | 107 | \#232 | 9 |
| Internal Link Dist (ft) |  | 412 |  |  | 884 |  |  | 915 |  |  | 549 |  |
| Turn Bay Length (ft) | 375 |  | 250 | 375 |  | 250 | 500 |  | 250 | 250 |  |  |
| Base Capacity (vph) | 325 | 1563 | 785 | 434 | 1414 | 752 | 460 | 737 | 512 | 227 | 265 | 303 |
| Starvation Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Spillback Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Storage Cap Reductn | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Reduced v/c Ratio | 0.77 | 1.02 | 0.16 | 0.87 | 0.96 | 0.12 | 0.71 | 0.68 | 0.78 | 0.50 | 1.12 | 0.44 |
| Intersection Summary |  |  |  |  |  |  |  |  |  |  |  |  |
| Cycle Length: 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Actuated Cycle Length: 120 |  |  |  |  |  |  |  |  |  |  |  |  |
| Natural Cycle: 90 |  |  |  |  |  |  |  |  |  |  |  |  |
| Control Type: Semi Act-Uncoord |  |  |  |  |  |  |  |  |  |  |  |  |
| Maximum v/c Ratio: 1.12 |  |  |  |  |  |  |  |  |  |  |  |  |

Intersection Signal Delay: 53.2
Intersection LOS: D
Intersection Capacity Utilization 91.1\% ICU Level of Service F

Analysis Period (min) 15
~ Volume exceeds capacity, queue is theoretically infinite.
Queue shown is maximum after two cycles.
\# 95th percentile volume exceeds capacity, queue may be longer.
Queue shown is maximum after two cycles.
Splits and Phases: 4: Vollmer Road \& Briargate Parkway


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |


| Major/Minor | Minor2 |  |  |  |  |  |  | Major1 |  | Major2 |  |
| :--- | ---: | ---: | ---: | :--- | :--- | :--- | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | - | 249 | - | 0 | - | 0 |  |  |  |  |  |
| $\quad$ Stage 1 | - | - | - | - | - | - |  |  |  |  |  |
| Stage 2 | - | - | - | - | - | - |  |  |  |  |  |
| Critical Hdwy | - | 6.94 | - | - | - | - |  |  |  |  |  |
| Critical Hdwy Stg 1 | - | - | - | - | - | - |  |  |  |  |  |
| Critical Hdwy Stg 2 | - | - | - | - | - | - |  |  |  |  |  |
| Follow-up Hdwy | - | 3.32 | - | - | - | - |  |  |  |  |  |
| Pot Cap-1 Maneuver | 0 | 751 | 0 | - | - | - |  |  |  |  |  |
| $\quad$ Stage 1 | 0 | - | 0 | - | - | - |  |  |  |  |  |
| Stage 2 | 0 | - | 0 | - | - | - |  |  |  |  |  |
| Platoon blocked, \% |  |  |  | - | - | - |  |  |  |  |  |
| Mov Cap-1 Maneuver | - | 751 | - | - | - | - |  |  |  |  |  |
| Mov Cap-2 Maneuver | - | - | - | - | - | - |  |  |  |  |  |
| Stage 1 | - | - | - | - | - | - |  |  |  |  |  |
| Stage 2 | - | - | - | - | - | - |  |  |  |  |  |


| Approach | EB | NB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 10.2 | 0 | 0 |


| Minor Lane/Major Mvmt | NBT EBLn1 | SBT | SBR |
| :--- | ---: | ---: | ---: |
| Capacity (veh/h) | -751 | - | - |
| HCM Lane V/C Ratio | -0.087 | - | - |
| HCM Control Delay (s) | -10.2 | - | - |
| HCM Lane LOS | - | $B$ | - |
| HCM 95th \%tile Q(veh) | - | - |  |
| (s) | - | - |  |

## V3_Traffic Memo-Vollmer Rd RIRO.pdf Markup Summary



Line (8)

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| minary Plan Tr |
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## Subject: Line

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| Parkway, to v $\epsilon$ in/right-out ints Analyze auxili | Subject: Line <br> Page Label: 1 <br> Author: CDurham <br> Date: 6/25/2024 9:12:48 AM <br> Status: <br> Color: <br> Layer: <br> Space: |
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| t-in/right-out | Subject: Line <br> Page Label: 1 <br> Author: CDurham <br> Date: 6/25/2024 9:13:03 AM <br> Status: <br> Color: <br> Layer: <br> Space: |
| :iling No. 1 Prelir t-in/rightoutcon $\qquad$ | Subject: Line <br> Page Label: 3 <br> Author: CDurham <br> Date: 6/25/2024 9:15:18 AM <br> Status: <br> Color: <br> Layer: <br> Space: |


| 's, reveals the in/right out V igress volume | Subject: Line <br> Page Label: 8 <br> Author: CDurham <br> Date: 6/25/2024 10:15:32 AM <br> Status: <br> Color: <br> Layer: <br> Space: |  |
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| ciated with th in/right-out ac and determin | Subject: Line <br> Page Label: 10 <br> Author: CDurham <br> Date: 6/25/2024 10:16:50 AM <br> Status: <br> Color: <br> Layer: <br> Space: |  |
| -in/right-out ont the surroundin! | Subject: Line <br> Page Label: 10 <br> Author: CDurham <br> Date: 6/25/2024 10:17:01 AM <br> Status: <br> Color: <br> Layer: <br> Space: |  |
| Text Box (3) |  |  |
| Update figure to match with figure 6B in TIS | Subject: Text Box <br> Page Label: 4 <br> Author: CDurham <br> Date: 6/25/2024 9:24:14 AM <br> Status: <br> Color: <br> Layer: <br> Space: | Update figure to match with figure 6B in TIS |
| Update figure to match with figure 8 in TIS | Subject: Text Box <br> Page Label: 6 <br> Author: CDurham <br> Date: 6/25/2024 9:50:27 AM <br> Status: <br> Color: <br> Layer: <br> Space: | Update figure to match with figure 8 in TIS |
| MEMORANDUM Update analysis to only show right-in movement at the Vollmer access location | Subject: Text Box <br> Page Label: 1 <br> Author: CDurham <br> Date: 6/25/2024 10:17:46 AM <br> Status: <br> Color: <br> Layer: <br> Space: | Update analysis to only show right-in movement at the Vollmer access location |


[^0]:    ${ }^{1}$ PrairieRidge Filing No． 1 Preliminary Plan Traffic Impact Study，SM ROCHA，LLC，November 30， 2023.

[^1]:    87ロロ TURNPIKE DRIVE，SUITE 24ロ WESTMINSTER，CロLロRADロ Bロロ31（3ロ3）458－9798
    6 SaUTH TEJロN STREET，SUITE 618 CロLロRADロ SPRINGS，LロLロRADロ Bロ9ロ3（719）2ロ3－6639 WWW．SMRロCHA．CロM

[^2]:    ${ }^{2}$ El Paso County 2016 Major Transportation Corridors Plan Update, Felsburg Holt \& Ullevig, December 2016.
    ${ }^{3}$ El Paso County Engineering Criteria Manual, El Paso County, October 2020.
    ${ }^{4}$ Major Thoroughfare Plan, City of Colorado Springs, August 2011.
    ${ }^{5}$ The Retreat at Timber Ridge Preliminary Plan Transportation Memorandum, LSC Transportation Consultants Inc., June 29, 2018.
    ${ }^{6}$ Homestead North Phase 1 Updated Traffic Impact Study, LSC Transportation Consultants Inc., January 11, 2022.

