# TRAFFIC IMPACT STUDY 

For<br>Urban Landing<br>El Paso County, Colorado

May 2024

Prepared for:
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Prepared by:


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## Traffic Engineer's Statement

The attached traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.


Fred Lantz, P.E. \#23410

05/08/2024
Date

## Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.

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## I. Introduction

## Project Overview

This traffic impact study is provided as a planning document and addresses the capacity, geometric, and control requirements associated with the development entitled Urban Landing.

This proposed residential development consists of a single-family housing community. The development is located on the southeast corner of Struthers Road and Spanish Bit Drive in El Paso County, Colorado.

## Study Area Boundaries

The study area to be examined in this analysis was coordinated with the County's Public Works Department and encompasses the Spanish Bit Drive intersections with Struthers Road and the Big R Stores access drive and includes the proposed site access drives.

Figure 1 illustrates location of the site and study intersections.

## Site Description

Land for the development is currently vacant and surrounded by open space and a mix of residential and commercial land uses.

The proposed development is understood to entail the new construction of 49 single-family detached homes.

Proposed access to the development is provided via two full-movement accesses onto Spanish Bit Drive (referred to as Access A and Access B).

For purposes of this study, it is anticipated that development construction would be completed by end of Year 2026.

General site and access locations are shown on Figure 1.
A preliminary plan, as prepared by Classic Consulting Engineers \& Surveyors, LLC, is shown on Figure 2. This plan is provided for illustrative purposes only.



## Existing and Committed Surface Transportation Network

Within the study area, Spanish Bit Drive is the primary roadway that will accommodate traffic to and from the proposed development. The secondary roadway includes Struthers Road. A brief description of each roadway, based on the County's 2016 Major Transportation Corridors Plan (MTCP) ${ }^{1}$ and Engineering Criteria Manual (ECM) ${ }^{2}$, is provided below:

Struthers Road is a north-south minor arterial roadway having four through lanes (two lanes in each direction) with a combination of shared and exclusive turn lanes at the intersection within the study area. Struthers Road provides a posted speed limit of 45 MPH.

Spanish Bit Drive is an east-west rural local roadway having two through lanes (one lane in each direction) with shared turn lanes at the intersection within the study area. Spanish Bit Drive provides a posted speed limit of 30 MPH . Spanish Bit Drive is a paved roadway at its intersection with Struthers Road but becomes a gravel roadway east of the Big R Stores access drive.

All study intersections operate under a stop-controlled condition. A stop-controlled intersection is defined as a roadway intersection where vehicle rights-of-way are controlled by one or more "STOP" signs.

Pursuant to the County's MTCP, no regional or specific improvements for the above-described roadways are known to be planned or committed at this time.

[^0]
## II. Existing Traffic Conditions

Morning (AM) and afternoon (PM) peak hour traffic counts were collected at the intersections of Spanish Bit Drive with Struthers Road and the Big R Stores access drive. Average daily traffic (ADT) volumes were collected over a 24 -hour period on Struthers Road and Spanish Bit Drive. Counts were collected on Wednesday, January 17, 2024, with AM peak hour counts being collected during the period of 7:00 a.m. to 9:00 a.m. and PM peak hour counts being collected during the period of 4:00 p.m. to 6:00 p.m.

Existing volumes and intersection geometry are shown on Figure 3. Traffic count data is included for reference in Appendix A.


Figure 3
EXISTING TRAFFIC
Volumes \& Intersection Geometry
AM / PM Peak Hour
URBAN LANDING
Traffic Impact Study
(ADT) : Average Daily Traffic

## Peak Hour Intersection Levels of Service - Existing Traffic

The Unsignalized Intersection Analysis technique, as published in the Highway Capacity Manual (HCM), $6^{\text {th }}$ Edition, by the Transportation Research Board and as incorporated into the SYNCHRO computer program, was used to analyze the study intersections for existing and future traffic conditions. This nationally accepted technique allows for the determination of intersection level of service (LOS) based on the congestion and delay of each traffic movement.

Pursuant to Section B.4.1.A of the County's ECM, the design objective for each scenario of this study shall be level of service " $D$ ". Level of service is a method of measurement used by transportation professionals to quantify a driver's perception of travel conditions that include travel time, number of stops, and total amount of stopped delay experienced on a roadway network. The HCM categorizes level of service into a range from "A" which indicates little, if any, vehicle delay, to "F" which indicates a level of operation considered unacceptable to most drivers. These levels of service grades with brief descriptions of the operating condition, for unsignalized and signalized intersections, are included for reference in Appendix $B$ and have been used throughout this study.

The level of service analyses results for existing conditions are summarized in Table 1.
Intersection capacity worksheets developed for this study are provided in Appendix C.

Table 1 - Intersection Capacity Analysis Summary - Existing Traffic

| INTERSECTION | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
| LANE GROUPS | AM PEAK HOUR | PM PEAK HOUR |
| Struthers Road / Spanish Bit Drive (Stop-Controlled) |  |  |
| Westbound Left and Right | B | B |
| Southbound Left | A | A |
| Spanish Bit Drive / Big R Stores Access (Stop-Controlled) |  |  |
| Eastbound Left and Through | A | A |
| Southbound Left and Right | A | A |

Key: Stop-Controlled Intersection: Level of Service

## Existing Traffic Analysis Results

Under existing conditions, operational analysis shows that the stop-controlled intersection of Struthers Road with Spanish Bit Drive has turn movement operations at or better than LOS B during the morning and afternoon peak traffic hours.

The stop-controlled intersection of Spanish Bit Drive with the Big R Stores access drive has turn movement operations at LOS A during both peak traffic hours.

## III. Future Traffic Conditions Without Proposed Development

Background traffic is the traffic projected to be on area roadways without consideration of the proposed development. Background traffic includes traffic generated by development of vacant parcels in the area.

To account for projected increases in background traffic for Years 2026 and 2044, a compounded annual growth rate was determined using population growth estimates provided by the Pikes Peak Area Council of Governments' (PPACG) 2045 Long Range Transportation Plan³, which anticipates a 20-year growth rate between one and two percent. Therefore, in order to provide for a conservative analysis, a growth rate of two percent was applied to existing traffic volumes. This annual growth rate is also consistent with assumptions used within traffic studies prepared for adjacent future developments and is considered to be consistent with regional growth projections and the level of infill development expected within the area.

To account for projected traffic from adjacent developments not yet built, trip generations from the following traffic studies were added to background traffic volumes:

- Struthers Ranch Subdivision Filing No. $5^{4}$
- Monument Ridge Lots 7 \& $8^{5}$

It is important to note that trip generations from the future Monument Ridge Apartments development and other vacant lots within Monument Ridge, as shown within the Monument Ridge Lots 7 \& 8 Transportation Memorandum, were also included in background traffic volumes.

Trip generations from the retail portion of the Cathedral Rock Commons traffic study ${ }^{6}$ were also included in background traffic volumes.

Pursuant to the non-committed area roadway improvements discussed in Section I, Year 2026 and Year 2044 background traffic conditions assume no roadway improvements to accommodate regional transportation demands. This assumption continues to provide for a conservative analysis.

Projected background traffic volumes and intersection geometry for Years 2026 and 2044 are shown on Figure 4 and Figure 5, respectively.

[^1]

Figure 4
BACKGROUND TRAFFIC - YEAR 2026
Volumes \& Intersection Geometry
AM / PM Peak Hour
URBAN LANDING
Traffic Impact Study
(ADT) : Average Daily Traffic


Figure 5
BACKGROUND TRAFFIC - YEAR 2044
Volumes \& Intersection Geometry
AM / PM Peak Hour
URBAN LANDING
Traffic Impact Study

## Peak Hour Intersection Levels of Service - Background Traffic

As with existing traffic conditions, the operations of study intersections were analyzed under background conditions, without the proposed development, using the SYNCHRO computer program.

Background traffic level of service analysis results for Year 2026 are listed in Table 2. Year 2044 operational results are summarized in Table 3.

Definitions of levels of service are given in Appendix B. Intersection capacity worksheets are provided in Appendix C.

Table 2 - Intersection Capacity Analysis Summary - Background Traffic - Year 2026

| INTERSECTION | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
| LANE GROUPS | AM PEAK HOUR | PM PEAK HOUR |
| Struthers Road / Spanish Bit Drive (Stop-Controlled) |  |  |
| Westbound Left and Right | B | C |
| Southbound Left | A | A |
| Spanish Bit Drive / Big R Stores Access (Stop-Controlled) |  |  |
| Eastbound Left and Through | A | A |
| Southbound Left and Right | A | A |

Key: Stop-Controlled Intersection: Level of Service

## Background Traffic Analysis Results - Year 2026

Year 2026 background traffic analysis indicates that the stop-controlled intersection of Struthers Road with Spanish Bit Drive has turn movement operations at or better than LOS B during the morning peak traffic hour and LOS C or better during the afternoon peak traffic hour.

The stop-controlled intersection of Spanish Bit Drive with the Big R Stores access drive projects turn movement operations at LOS A during both peak traffic hours.

Table 3 - Intersection Capacity Analysis Summary - Background Traffic - Year 2044

| INTERSECTION | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
| LANE GROUPS | AM PEAK HOUR | PM PEAK HOUR |
| Struthers Road / Spanish Bit Drive (Stop-Controlled) |  |  |
| Westbound Left and Right | B | E |
| Southbound Left | A | A |
| Spanish Bit Drive / Big R Stores Access (Stop-Controlled) |  |  |
| Eastbound Left and Through | A | A |
| Southbound Left and Right | A | A |

Key: Stop-Controlled Intersection: Level of Service

## Background Traffic Analysis Results - Year 2044

By Year 2044 and without the proposed development, the stop-controlled intersection of Struthers Road with Spanish Bit Drive expects turn movement operations at or better than LOS B during the morning peak traffic hour and LOS A during the afternoon peak traffic hour. Exceptions would include the westbound left and right turning movement which operates at LOS E during the afternoon peak traffic hour. The LOS E operation is attributed to the through traffic volume along Struthers Road and the stop-controlled nature of the intersection.

The stop-controlled intersection of Spanish Bit Drive with the Big R Stores access drive projects turn movement operations at LOS A during both peak traffic hours.

It is to be noted that it is not uncommon for unsignalized movements to or from an arterial roadway, in urban areas, to operate with noticeable delays during peak traffic hours. It is, however, likely that turn movements will operate better than the results obtained with this HCM Two-Way Stop-Control (TWSC) level of service analysis would indicate, as the HCM analysis may not accurately account for the effect of vehicle platooning and gaps caused by upstream signals. The upstream signal control on Struthers Road may tend to create additional gaps in the traffic stream for turning movements at Spanish Bit Drive and will most likely provide mitigation to the LOS E operation projected during the afternoon peak traffic hour.

## IV. Proposed Project Traffic

## Trip Generation

Standard traffic generation characteristics compiled by the Institute of Transportation Engineers (ITE) in their report entitled Trip Generation Manual, $11^{\text {th }}$ Edition, were applied to the proposed land use in order to estimate average daily traffic (ADT), AM Peak Hour, and PM Peak Hour vehicle trips. A vehicle trip is defined as a one-way vehicle movement from a point of origin to a point of destination.

The ITE land use code 210 (Single-Family Detached Housing) was used for estimating trip generation because of its conservative rates and best fit to the proposed land use description.

Trip generation rates used in this study are presented in Table 4.

Table 4 - Trip Generation Rates

| $\begin{gathered} \text { ITE } \\ \text { CODE } \end{gathered}$ | LAND USE | UNIT | TRIP GENERATION RATES |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $\begin{gathered} 24 \\ \text { HOUR } \end{gathered}$ | AM PEAK HOUR |  |  | PM PEAK HOUR |  |  |
|  |  |  |  | ENTER | EXIT | TOTAL | ENTER | EXIT | TOTAL |
| 210 | Single-Family Detached Housing | DU | 9.43 | 0.18 | 0.53 | 0.70 | 0.59 | 0.35 | 0.94 |

Key: $\quad$ DU $=$ Dwelling Units.
Note: All data and calculations above are subject to being rounded to nearest value.

Table 5 illustrates projected ADT, AM Peak Hour, and PM Peak Hour traffic volumes likely generated by the proposed development upon build-out and provides comparison to traffic volume estimates for the previously approved land within the Cathedral Rock Commons traffic study.

Table 5 - Trip Generation Summary


Key: $\quad \mathrm{DU}=$ Dwelling Units.

* = Trip generation summary from Cathedral Rock Commons: Traffic Impact Study, SM ROCHA, LLC, March 2023.

Note: All data and calculations above are subject to being rounded to nearest value.

Upon build-out, Table 5 illustrates that the proposed development has the potential to generate approximately 462 daily vehicle trips with 34 of those occurring during the morning peak hour and 46 during the afternoon peak hour. Compared to previously approved land use, this represents a potential increase in site traffic generation of approximately 124 daily trips with 12 of those occurring during the morning peak hour and 19 during the afternoon peak hour.

## Adjustments to Trip Generation Rates

It is probable that the proximity of the existing and proposed retail land uses on the north side of Spanish Bit Drive could provide a reduction in vehicle trip generation for the proposed residential development. In review of the previously approved Cathedral Rock Commons traffic study, it was determined that as much as 47 percent of site-generated trips could be captured internally. However, in order to continue providing for a conservative analysis, no internal capture trip reduction was taken in this study.

## Trip Distribution

The overall directional distribution of site-generated traffic was determined based on the location of the development site within the County, proposed and existing area land uses, allowed turning movements, available roadway network, and in reference to the Cathedral Rock Commons traffic study.

Overall trip distribution patterns for the development are shown on Figure 6.

## Trip Assignment

Traffic assignment is how generated and distributed vehicle trips are expected to be loaded onto the available roadway network.

Applying trip distribution patterns to site-generated traffic provides the overall site-generated trip assignments shown on Figure 6.


Figure 6
SITE DEVELOPMENT DISTRIBUTION
(\%): Overall

## V. Future Traffic Conditions With Proposed Developments

Total traffic is the traffic projected to be on area roadways with consideration of the proposed development. Total traffic includes background traffic projections for Years 2026 and 2044 with consideration of site-generated traffic. For analysis purposes, it was assumed that development construction would be completed by end of Year 2026.

Pursuant to area roadway improvement discussions provided in Section III, Year 2026 and Year 2044 total traffic conditions assume no roadway improvements to accommodate regional transportation demands. Roadway improvements associated with site development are expected to be limited to site access and frontage as required by the governing agency.

Projected Year 2026 total traffic volumes and intersection geometry are shown in Figure 7.
Figure 8 shows projected total traffic volumes and intersection geometry for Year 2044.


Figure 7
TOTAL TRAFFIC - YEAR 2026 Volumes \& Intersection Geometry

AM / PM Peak Hour
URBAN LANDING
Traffic Impact Study
(ADT) : Average Daily Traffic


Figure 8
TOTAL TRAFFIC - YEAR 2044 Volumes \& Intersection Geometry

AM / PM Peak Hour
URBAN LANDING
Traffic Impact Study
(ADT) : Average Daily Traffic

## VI. Project Impacts

The analyses and procedures described in this study were performed in accordance with the latest HCM and are based upon the worst-case conditions that occur during a typical weekday upon buildout of site development and analyzed land uses. Therefore, study intersections are likely to operate with traffic conditions better than those described within this study, which represent the peak hours of weekday operations only.

## Peak Hour Intersection Levels of Service - Total Traffic

As with background trafic, the operations of the study intersections were analyzed under projected total traffic conditions using the SYNCHRO computer program. Total trafic level of service analysis results for Years 2026 and 2044 are summarized in Table 6 and Table 7, respectively.

Definitions of levels of service are given in Appendix B. Intersection capacity worksheets are provided in Appendix C.

Table 6 - Intersection Capacity Analysis Summary - Total Traffic - Year 2026

| INTERSECTION | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
|  | AM PEAK HOUR | PM PEAK HOUR |
| Struthers Road / Spanish Bit Drive (Stop-Controlled) |  |  |
| Westbound Left and Right | B | C |
| Southbound Left | A | A |
| Spanish Bit Drive / Big R Stores Access (Stop-Controlled) |  |  |
| Eastbound Left and Through | A | A |
| Southbound Left and Right | A | A |
| Access A / Spanish Bit Drive (Stop-Controlled) | A | A |
| Westbound Left and Through | A | A |
| Northbound Left and Right |  |  |
| Access B / Spanish Bit Drive (Stop-Controlled) | A | A |
| Westbound Left and Through | A | B |
| Northbound Left and Right |  |  |

Key: Stop-Controlled Intersection: Level of Service

Table 7 - Intersection Capacity Analysis Summary - Total Traffic - Year 2044

| INTERSECTION | LEVEL OF SERVICE |  |
| :--- | :---: | :---: |
|  | AM PEAK HOUR | PM PEAK HOUR |
| Struthers Road / Spanish Bit Drive (Stop-Controlled) |  |  |
| Westbound Left and Right | B | F |
| Southbound Left | A | A |
| Spanish Bit Drive / Big R Stores Access (Stop-Controlled) |  |  |
| Eastbound Left and Through | A | A |
| Southbound Left and Right | A | A |
| Access A / Spanish Bit Drive (Stop-Controlled) | A | A |
| Westbound Left and Through | A | A |
| Northbound Left and Right |  | A |
| Access B / Spanish Bit Drive (Stop-Controlled) | A | B |
| Westbound Left and Through | A |  |
| Northbound Left and Right |  |  |

Key: Stop-Controlled Intersection: Level of Service

## Total Traffic Analysis Results Upon Development Build-Out

Table 7 illustrates how, by Year 2044 and upon development build-out, the stop-controlled intersection of Struthers Road with Spanish Bit Drive expects turn movement operations at or better than LOS B during the morning peak traffic hour and LOS A during the afternoon peak traffic hour. Exceptions still include the westbound left and right turning movement which operates at LOS F during the afternoon peak traffic hour. The LOS F operation is attributed to the through traffic volume along Struthers Road and the stop-controlled nature of the intersection. The construction of exclusive westbound left and right turn lanes is a potential solution to mitigate the projected LOS F operation.

The stop-controlled intersection of Spanish Bit Drive with the Big R Stores access drive projects turn movement operations at LOS A during both peak traffic hours.

The stop-controlled intersection of Access A with Spanish Bit Drive projects turn movement operations at LOS A during both peak traffic hours.

The stop-controlled intersection of Access B with Spanish Bit Drive expects turn movement operations at LOS A during the morning peak traffic hour and LOS B or better during the afternoon peak traffic hour.

It is again noted that it is not uncommon for unsignalized movements to or from an arterial roadway, in urban areas, to operate with noticeable delays during peak traffic hours. It is, however, likely that turn movements will operate better than the results obtained with this HCM Two-Way Stop-Control (TWSC) level of service analysis would indicate, as the HCM analysis may not accurately account for the effect of vehicle platooning and gaps caused by upstream signals. The upstream signal control on Struthers Road may tend to create additional gaps in the traffic stream for turning movements at Spanish Bit Drive and will most likely provide mitigation to the LOS F operation projected during the afternoon peak traffic hour.

Compared to the background traffic analysis, the traffic generated by the proposed development is not expected to significantly change the operations of the study intersections. These intersection operations are similar to background conditions.

## Total Traffic Auxiliary Lane Analysis

Auxiliary lanes for the study intersections are to be based on the County's ECM.
Considering development build-out, an evaluation of auxiliary lane requirements, pursuant to Section 2.3.7 of the County's ECM, reveals that a northbound right turn deceleration lane at Spanish Bit Drive along Struthers Road may be required since the projected peak hour right turn volume exceeds the County's threshold of 50 vehicles per hour.

An evaluation of auxiliary lane requirements reveals that exclusive westbound left and right turn deceleration lanes at Struthers Road along Spanish Bit Drive may be required since the projected peak hour left and right turn volumes exceed the County's threshold of 25 and 50 vehicles per hour, respectively.

Pursuant to the County's ECM, an eastbound left turn deceleration lane at the Big R Stores access drive along Spanish Bit Drive may also be required since the projected peak hour left turn volume exceeds the County's threshold of 25 vehicles per hour.

## Queue Length Analysis

Queue lengths for the study intersections were analyzed using Year 2044 total traffic conditions. The analysis yields estimate of $95^{\text {th }}$ percentile queue lengths, which have only a five percent probability of being exceeded during the analysis time period. Queue lengths were modeled and are included with the Synchro worksheets in Appendix C.

The greatest queue length anticipated at the Struthers Road and Spanish Bit Drive intersection occurs during the afternoon peak hour. The queue length is approximately six vehicles for the westbound left and right turn movement. No significant queues at the other study intersections were indicated.

## Potential Public Improvements

The existing Development Agreement for the adjacent Big R Stores defines off-site improvements and cost participation for potential Struthers Road and Spanish Bit Drive improvements. These off-site public improvements include the construction or modification of auxiliary lanes along Struthers Road (referred to as Struthers Off-Site Improvements) and the paving of Spanish Bit Drive from the Big R Stores east property line to proposed development's east property line.

In conjunction with improvements discussed within the County's PCD - Engineering Meeting Notes dated November 17, 2020, Table 8 illustrates potential public improvements associated with the proposed Urban Landing development.

Table 8 - Potential Public Improvements

| PUBLIC IMPROVEMENT | TYPE | TIMING |
| :--- | :--- | :--- |
| Construct northbound right turn lane on <br> Struthers Road at Spanish Bit Drive | Auxiliary Lane | When Warranted |
| Lengthen southbound left turn lane on <br> Struthers Road at Spanish Bit Drive | Auxiliary Lane | When 95th Percentile Queuing <br> Exceeds Existing Lane Length |
| Pave Spanish Bit Drive east to eastern edge of <br> property line | Roadway Segment | With Development North of <br> Spanish Bit Drive |

As defined within the existing Development Agreement, these off-site public improvements should be paid for through a cost sharing agreement or participate in a cost recovery with other adjacent developments or owners benefitting from the Struthers Off-Site Improvements.

## VII. Conclusion

This traffic impact study addressed the capacity, geometric, and control requirements associated with the development entitled Urban Landing. This proposed residential development consists of a singlefamily housing community. The development is located on the southeast corner of Struthers Road and Spanish Bit Drive in El Paso County, Colorado.

The study area examined in this analysis encompassed the Spanish Bit Drive intersections with Struthers Road and the Big R Stores access drive and included the proposed site access drives.

Analysis was conducted for critical AM Peak Hour and PM Peak Hour traffic operations for existing traffic conditions, Year 2026 and Year 2044 background traffic conditions, and Year 2026 and Year 2044 total traffic conditions.

Analysis of existing traffic conditions indicates that the stop-controlled intersection of Struthers Road with Spanish Bit Drive has turn movement operations at or better than LOS B during the morning and afternoon peak traffic hours. The stop-controlled intersection of Spanish Bit Drive with the Big R Stores access drive has turn movement operations at LOS A during both peak traffic hours.

Without the proposed development, Year 2026 background operational analysis shows that the stopcontrolled intersection of Struthers Road with Spanish Bit Drive has turn movement operations at or better than LOS B during the morning peak traffic hour and LOS C or better during the afternoon peak traffic hour. The stop-controlled intersection of Spanish Bit Drive with the Big R Stores access drive projects turn movement operations at LOS A during both peak traffic hours.

By Year 2044 and without the proposed development, the stop-controlled intersection of Struthers Road with Spanish Bit Drive expects turn movement operations at or better than LOS B during the morning peak traffic hour and LOS A during the afternoon peak traffic hour. Exceptions would include the westbound left and right turning movement which operates at LOS E during the afternoon peak traffic hour. The LOS E operation is attributed to the through traffic volume along Struthers Road and the stop-controlled nature of the intersection. The stop-controlled intersection of Spanish Bit Drive with the Big R Stores access drive projects turn movement operations at LOS A during both peak traffic hours.

Analysis of future traffic conditions indicates that the addition of site-generated traffic is expected to create no negative impact to traffic operations for the existing and surrounding roadway system upon roadway and intersection control improvements assumed within this analysis. With all conservative assumptions defined in this analysis, the study intersections are projected to operate at future levels of service comparable to Year 2044 background traffic conditions. Proposed site accesses have longterm operations at LOS B or better during peak traffic periods and upon build-out.

APPENDIX A

Traffic Count Data

Location: 1 STRUTHERS RD \& SPANISH BIT DR AM
Date: Wednesday, January 17, 2024
Peak Hour: 08:00 AM - 09:00 AM
Peak 15-Minutes: 08:30 AM - 08:45 AM


Note: Total study counts contained in parentheses.
Traffic Counts - Motorized Vehicles


Location: 1 STRUTHERS RD \& SPANISH BIT DR PM
Date: Wednesday, January 17, 2024
Peak Hour: 04:00 PM - 05:00 PM
Peak 15-Minutes: 04:15 PM - 04:30 PM


Peak Hour - Bicycles


Peak Hour - Pedestrians


Note: Total study counts contained in parentheses.
Traffic Counts - Motorized Vehicles

(303) 216-2439
www.alltrafficdata.net
Location: 2 BIG R STORES ACCESS \& SPANISH BIT DR AM
Date: Wednesday, January 17, 2024
Peak Hour: 07:15 AM - 08:15 AM
Peak 15-Minutes: 07:45 AM - 08:00 AM


Peak Hour - Bicycles


Peak Hour - Pedestrians


Note: Total study counts contained in parentheses.
Traffic Counts - Motorized Vehicles

| Interval | SPANISH BIT DR <br> Eastbound |  |  |  | SPANISH BIT DR Westbound |  |  |  |  | Northbound |  |  | BIG R STORES ACCESS <br> Southbound |  |  |  |  |  |  | Rolling Hour |  | Pedestrian Crossings |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Start Time | U-Turn | Left | Thru | Right | U-Turn | Left |  | Thru R | Right | U-Turn | Left | Thru Right | U-Turn | Left |  | Thru | Right |  |  |  |  | West | East | South North |
| 7:00 AM | 0 | 1 | 1 | 0 | 0 | 0 |  | 5 | 0 |  |  |  | 0 | 0 |  | 0 | 0 | 0 | 7 | 7 | 31 | 0 | 0 | 0 |
| 7:15 AM | 0 | 1 | 1 | 0 | 0 | 0 |  | 7 | 0 |  |  |  | 0 | 0 |  | 0 | 0 | 0 | 9 | 9 | 34 | 0 | 0 | 0 |
| 7:30 AM | 0 | 2 | 1 | 0 | 0 | 0 |  | 1 | 0 |  |  |  | 0 | 0 |  | 0 | 0 | 0 | 4 | 4 | 29 | 0 | 0 | 0 |
| 7:45 AM | 0 | 4 | 0 | 0 | 0 | 0 |  | 6 | 0 |  |  |  | 0 | 0 |  | 0 |  | 1 | 11 |  | 32 | 0 | 0 | 0 |
| 8:00 AM | 0 | 4 | 2 | 0 | 0 | 0 |  | 1 | 1 |  |  |  | 0 | 0 |  | 0 | 2 | 2 | 10 |  | 32 | 0 | 0 | 0 |
| 8:15 AM | 0 | 0 | 1 | 0 | 0 | 0 |  | 2 | 0 |  |  |  | 0 | 0 |  | 0 | 1 | 1 | 4 | 4 |  | 0 | 0 | 0 |
| 8:30 AM | 0 | 2 | 1 | 0 | 0 | 0 |  | 3 | 0 |  |  |  | 0 | 0 |  | 0 | 1 | 1 | 7 | 7 |  | 0 | 0 | 0 |
| 8:45 AM | 0 | 3 | 1 | 0 | 0 | 0 |  | 4 | 1 |  |  |  | 0 | 0 |  | 0 | 2 | 2 | 11 | 1 |  | 0 | 0 | 0 |
| Count Total | 0 | 17 | 8 | 0 | 0 | 0 |  | 29 | 2 |  |  |  | 0 | 0 | 0 | 0 | 7 | 7 | 63 | 63 |  | 0 | 0 | 0 |
| Peak Hour | 0 | 11 | 4 | 0 | 0 | 0 |  | 15 | 1 |  |  |  | 0 |  | 0 | 0 | 0 | 3 |  | 34 |  | 0 | 0 | 0 |

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Peak Hour - Bicycles


Peak Hour - Pedestrians


Note: Total study counts contained in parentheses.
Traffic Counts - Motorized Vehicles

| Interval Start Time | SPANISH BIT DR <br> Eastbound |  |  |  | SPANISH BIT DR <br> Westbound |  |  |  | Northbound |  |  | BIG R STORES ACCESS <br> Southbound |  |  |  | Total | Rolling Hour | Pedestrian Crossings |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | U-Turn | Left | Thru | Right | U-Turn |  | Thru R |  | U-Turn | Left | Thru Right | U-Turn | Left | Thru | Right |  |  | West | East | South North |
| 4:00 PM | 0 | 2 | 2 | 0 | 0 | 0 | 6 | 0 |  |  |  | 0 | 0 | 0 | 8 | 18 | 76 | 0 | 0 | 0 |
| 4:15 PM | 0 | 9 | 4 | 0 | 0 | 0 | 7 | 1 |  |  |  | 0 | 2 | 0 | 5 | 28 | 73 | 0 | 0 | 0 |
| 4:30 PM | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 |  |  |  | 0 | 0 | 0 | 8 | 13 | 61 | 0 | 0 | 0 |
| 4:45 PM | 0 | 6 | 2 | 0 | 0 | 0 | 3 | 0 |  |  |  | 0 | 1 | 0 | 5 | 17 | 63 | 0 | 0 | 0 |
| 5:00 PM | 0 | 1 | 4 | 0 | 0 | 0 | 1 | 0 |  |  |  | 0 | 0 | 0 | 9 | 15 | 60 | 0 | 0 | 0 |
| 5:15 PM | 0 | 5 | 4 | 0 | 0 | 0 | 3 | 0 |  |  |  | 0 | 0 | 0 | 4 | 16 |  | 0 | 0 | 0 |
| 5:30 PM | 0 | 3 | 3 | 0 | 0 | 0 | 4 | 0 |  |  |  | 0 | 0 | 0 | 5 | 15 |  | 0 | 0 | 0 |
| 5:45 PM | 0 | 3 | 2 | 0 | 0 | 0 | 5 | 0 |  |  |  | 0 | 0 | 0 | 4 | 14 |  | 0 | 0 | 0 |
| Count Total | 0 | 34 | 21 | 0 | 0 | 0 | 29 | 1 |  |  |  | 0 | 3 | 0 | 48 | 136 |  | 0 | 0 | 0 |
| Peak Hour | 0 | 22 | 8 | 0 | 0 | 0 | 16 | 1 |  |  |  | 0 | 3 | 0 | 026 |  | 6 | 0 | 0 | 0 |

All Traffic Data Services

3VOL - STRUTHERS RD N.O. SPANISH BIT DR

| Time | NB | SB | Total |
| :---: | :---: | :---: | :---: |
| 1/17/2024 | 3 | 0 | 3 |
| 1/17/2024 12:15:00 AM | 1 | 0 | 1 |
| 1/17/2024 12:30:00 AM | 0 | 0 | 0 |
| 1/17/2024 12:45:00 AM | 2 | 0 | 2 |
| 1/17/2024 1:00:00 AM | 1 | 0 | 1 |
| 1/17/2024 1:15:00 AM | 0 | 0 | 0 |
| 1/17/2024 1:30:00 AM | 0 | 0 | 0 |
| 1/17/2024 1:45:00 AM | 1 | 0 | 1 |
| 1/17/2024 2:00:00 AM | 0 | 0 | 0 |
| 1/17/2024 2:15:00 AM | 1 | 0 | 1 |
| 1/17/2024 2:30:00 AM | 0 | 0 | 0 |
| 1/17/2024 2:45:00 AM | 1 | 0 | 1 |
| 1/17/2024 3:00:00 AM | 0 | 0 | 0 |
| 1/17/2024 3:15:00 AM | 0 | 0 | 0 |
| 1/17/2024 3:30:00 AM | 1 | 0 | 1 |
| 1/17/2024 3:45:00 AM | 1 | 1 | 2 |
| 1/17/2024 4:00:00 AM | 3 | 0 | 3 |
| 1/17/2024 4:15:00 AM | 3 | 0 | 3 |
| 1/17/2024 4:30:00 AM | 4 | 1 | 5 |
| 1/17/2024 4:45:00 AM | 7 | 1 | 8 |
| 1/17/2024 5:00:00 AM | 4 | 0 | 4 |
| 1/17/2024 5:15:00 AM | 5 | 2 | 7 |
| 1/17/2024 5:30:00 AM | 6 | 1 | 7 |
| 1/17/2024 5:45:00 AM | 8 | 3 | 11 |
| 1/17/2024 6:00:00 AM | 12 | 7 | 19 |
| 1/17/2024 6:15:00 AM | 17 | 9 | 26 |
| 1/17/2024 6:30:00 AM | 22 | 13 | 35 |
| 1/17/2024 6:45:00 AM | 21 | 11 | 32 |
| 1/17/2024 7:00:00 AM | 32 | 20 | 52 |
| 1/17/2024 7:15:00 AM | 55 | 37 | 92 |
| 1/17/2024 7:30:00 AM | 46 | 43 | 89 |
| 1/17/2024 7:45:00 AM | 59 | 45 | 104 |
| 1/17/2024 8:00:00 AM | 51 | 35 | 86 |
| 1/17/2024 8:15:00 AM | 76 | 33 | 109 |
| 1/17/2024 8:30:00 AM | 54 | 59 | 113 |
| 1/17/2024 8:45:00 AM | 49 | 61 | 110 |
| 1/17/2024 9:00:00 AM | 51 | 46 | 97 |
| 1/17/2024 9:15:00 AM | 47 | 48 | 95 |
| 1/17/2024 9:30:00 AM | 41 | 38 | 79 |
| 1/17/2024 9:45:00 AM | 58 | 46 | 104 |
| 1/17/2024 10:00:00 AM | 68 | 41 | 109 |
| 1/17/2024 10:15:00 AM | 46 | 66 | 112 |
| 1/17/2024 10:30:00 AM | 52 | 64 | 116 |
| 1/17/2024 10:45:00 AM | 72 | 65 | 137 |
| 1/17/2024 11:00:00 AM | 55 | 66 | 121 |
| 1/17/2024 11:15:00 AM | 91 | 71 | 162 |
| 1/17/2024 11:30:00 AM | 81 | 62 | 143 |
| 1/17/2024 11:45:00 AM | 83 | 81 | 164 |
| Total | 1,291 | 1,076 | 2,367 |
| Percentage | 54.5\% | 45.5\% |  |
| Peak Hour | 11:00 AM | 11:00 AM | 11:00 AM |
| Volume | 310 | 280 | 590 |
| PHF | 0.852 | 0.864 | 0.899 |

All Traffic Data Services

3VOL - STRUTHERS RD N.O. SPANISH BIT DR

| Time | NB | SB | Total |
| :---: | :---: | :---: | :---: |
| 1/17/2024 12:00:00 PM | 76 | 69 | 145 |
| 1/17/2024 12:15:00 PM | 60 | 86 | 146 |
| 1/17/2024 12:30:00 PM | 72 | 74 | 146 |
| 1/17/2024 12:45:00 PM | 79 | 89 | 168 |
| 1/17/2024 1:00:00 PM | 62 | 78 | 140 |
| 1/17/2024 1:15:00 PM | 58 | 67 | 125 |
| 1/17/2024 1:30:00 PM | 64 | 57 | 121 |
| 1/17/2024 1:45:00 PM | 74 | 69 | 143 |
| 1/17/2024 2:00:00 PM | 74 | 84 | 158 |
| 1/17/2024 2:15:00 PM | 76 | 56 | 132 |
| 1/17/2024 2:30:00 PM | 66 | 69 | 135 |
| 1/17/2024 2:45:00 PM | 59 | 78 | 137 |
| 1/17/2024 3:00:00 PM | 83 | 86 | 169 |
| 1/17/2024 3:15:00 PM | 82 | 80 | 162 |
| 1/17/2024 3:30:00 PM | 84 | 73 | 157 |
| 1/17/2024 3:45:00 PM | 85 | 87 | 172 |
| 1/17/2024 4:00:00 PM | 100 | 106 | 206 |
| 1/17/2024 4:15:00 PM | 105 | 119 | 224 |
| 1/17/2024 4:30:00 PM | 87 | 92 | 179 |
| 1/17/2024 4:45:00 PM | 82 | 120 | 202 |
| 1/17/2024 5:00:00 PM | 76 | 102 | 178 |
| 1/17/2024 5:15:00 PM | 91 | 101 | 192 |
| 1/17/2024 5:30:00 PM | 87 | 93 | 180 |
| 1/17/2024 5:45:00 PM | 60 | 65 | 125 |
| 1/17/2024 6:00:00 PM | 68 | 95 | 163 |
| 1/17/2024 6:15:00 PM | 49 | 55 | 104 |
| 1/17/2024 6:30:00 PM | 49 | 52 | 101 |
| 1/17/2024 6:45:00 PM | 53 | 41 | 94 |
| 1/17/2024 7:00:00 PM | 39 | 45 | 84 |
| 1/17/2024 7:15:00 PM | 40 | 37 | 77 |
| 1/17/2024 7:30:00 PM | 34 | 40 | 74 |
| 1/17/2024 7:45:00 PM | 26 | 39 | 65 |
| 1/17/2024 8:00:00 PM | 16 | 30 | 46 |
| 1/17/2024 8:15:00 PM | 22 | 22 | 44 |
| 1/17/2024 8:30:00 PM | 21 | 27 | 48 |
| 1/17/2024 8:45:00 PM | 21 | 25 | 46 |
| 1/17/2024 9:00:00 PM | 13 | 21 | 34 |
| 1/17/2024 9:15:00 PM | 17 | 16 | 33 |
| 1/17/2024 9:30:00 PM | 23 | 18 | 41 |
| 1/17/2024 9:45:00 PM | 11 | 14 | 25 |
| 1/17/2024 10:00:00 PM | 7 | 5 | 12 |
| 1/17/2024 10:15:00 PM | 7 | 6 | 13 |
| 1/17/2024 10:30:00 PM | 5 | 7 | 12 |
| 1/17/2024 10:45:00 PM | 4 | 3 | 7 |
| 1/17/2024 11:00:00 PM | 5 | 3 | 8 |
| 1/17/2024 11:15:00 PM | 4 | 6 | 10 |
| 1/17/2024 11:30:00 PM | 1 | 2 | 3 |
| 1/17/2024 11:45:00 PM | 0 | 2 | 2 |
| Total | 2,377 | 2,611 | 4,988 |
| Percentage | 47.7\% | 52.3\% |  |
| Peak Hour | 3:45 PM | 4:00 PM | 4:00 PM |
| Volume | 378 | 437 | 812 |
| PHF | 0.900 | 0.910 | 0.906 |
| Grand Total | 3,668 | 3,687 | 7,355 |
| Percentage | 49.9\% | 50.1\% |  |



4VOL - SPANISH BIT DR E.O. STRUTHERS RD

| Time | EB | WB | Total |
| :---: | :---: | :---: | :---: |
| 1/17/2024 12:00:00 PM | 8 | 18 | 26 |
| 1/17/2024 12:15:00 PM | 10 | 8 | 18 |
| 1/17/2024 12:30:00 PM | 8 | 8 | 16 |
| 1/17/2024 12:45:00 PM | 10 | 6 | 16 |
| 1/17/2024 1:00:00 PM | 11 | 9 | 20 |
| 1/17/2024 1:15:00 PM | 8 | 8 | 16 |
| 1/17/2024 1:30:00 PM | 5 | 11 | 16 |
| 1/17/2024 1:45:00 PM | 13 | 8 | 21 |
| 1/17/2024 2:00:00 PM | 10 | 10 | 20 |
| 1/17/2024 2:15:00 PM | 6 | 14 | 20 |
| 1/17/2024 2:30:00 PM | 9 | 4 | 13 |
| 1/17/2024 2:45:00 PM | 6 | 3 | 9 |
| 1/17/2024 3:00:00 PM | 10 | 8 | 18 |
| 1/17/2024 3:15:00 PM | 6 | 6 | 12 |
| 1/17/2024 3:30:00 PM | 19 | 6 | 25 |
| 1/17/2024 3:45:00 PM | 10 | 7 | 17 |
| 1/17/2024 4:00:00 PM | 4 | 13 | 17 |
| 1/17/2024 4:15:00 PM | 13 | 13 | 26 |
| 1/17/2024 4:30:00 PM | 6 | 9 | 15 |
| 1/17/2024 4:45:00 PM | 9 | 9 | 18 |
| 1/17/2024 5:00:00 PM | 5 | 10 | 15 |
| 1/17/2024 5:15:00 PM | 9 | 7 | 16 |
| 1/17/2024 5:30:00 PM | 6 | 7 | 13 |
| 1/17/2024 5:45:00 PM | 5 | 10 | 15 |
| 1/17/2024 6:00:00 PM | 5 | 5 | 10 |
| 1/17/2024 6:15:00 PM | 14 | 4 | 18 |
| 1/17/2024 6:30:00 PM | 4 | 6 | 10 |
| 1/17/2024 6:45:00 PM | 6 | 3 | 9 |
| 1/17/2024 7:00:00 PM | 3 | 2 | 5 |
| 1/17/2024 7:15:00 PM | 6 | 1 | 7 |
| 1/17/2024 7:30:00 PM | 3 | 3 | 6 |
| 1/17/2024 7:45:00 PM | 1 | 4 | 5 |
| 1/17/2024 8:00:00 PM | 1 | 2 | 3 |
| 1/17/2024 8:15:00 PM | 1 | 8 | 9 |
| 1/17/2024 8:30:00 PM | 0 | 2 | 2 |
| 1/17/2024 8:45:00 PM | 1 | 0 | 1 |
| 1/17/2024 9:00:00 PM | 2 | 0 | 2 |
| 1/17/2024 9:15:00 PM | 3 | 0 | 3 |
| 1/17/2024 9:30:00 PM | 3 | 1 | 4 |
| 1/17/2024 9:45:00 PM | 0 | 0 | 0 |
| 1/17/2024 10:00:00 PM | 0 | 0 | 0 |
| 1/17/2024 10:15:00 PM | 0 | 0 | 0 |
| 1/17/2024 10:30:00 PM | 0 | 0 | 0 |
| 1/17/2024 10:45:00 PM | 0 | 0 | 0 |
| 1/17/2024 11:00:00 PM | 0 | 0 | 0 |
| 1/17/2024 11:15:00 PM | 0 | 0 | 0 |
| 1/17/2024 11:30:00 PM | 0 | 0 | 0 |
| 1/17/2024 11:45:00 PM | 0 | 0 | 0 |
| Total | 259 | 253 | 512 |
| Percentage | 50.6\% | 49.4\% |  |
| Peak Hour | 3:30 PM | 4:00 PM | 3:30 PM |
| Volume | 46 | 44 | 85 |
| PHF | 0.605 | 0.846 | 0.817 |
| Grand Total | 373 | 372 | 745 |
| Percentage | 50.1\% | 49.9\% |  |

## APPENDIX B

Level of Service Definitions

The following information is referenced from the Highway Capacity Manual: A Guide for Multimodal Mobility Analysis, $6^{\text {th }}$ Edition, Transportation Research Board, 2016: Chapter 19 - Signalized Intersections.

## Motorized Vehicle Level of Service (LOS) for Signalized Intersections

Levels of service are defined to represent reasonable ranges in control delay.
LOS A Describes operations with a control delay of $10 \mathrm{~s} / \mathrm{veh}$ or less and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is exceptionally favorable or the cycle length is very short. If it is due to favorable progression, most vehicles arrive during the green indication and travel through the intersection without stopping.

LOS B Describes operations with control delay between 10 and 20 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is low and either progression is highly favorable or the cycle length is short. More vehicles stop than with LOS A.

LOS C Describes operations with control delay between 20 and 35 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when progression is favorable or the cycle length is moderate. Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear at this level. The number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.

LOS D Describes operations with control delay between 35 and 55 s/veh and a volume-to-capacity ratio no greater than 1.0 . This level is typically assigned when the volume-to-capacity ratio is high and either progression is ineffective or the cycle length is long. Many vehicles stop and individual cycle failures are noticeable.

LOS E Describes operations with control delay between 55 and 80 s/veh and a volume-to-capacity ratio no greater than 1.0. This level is typically assigned when the volume-to-capacity ratio is high, progression is unfavorable, and the cycle length is long. Individual cycle failures are frequent.

LOS F Describes operations with control delay exceeding $80 \mathrm{~s} /$ veh or a volume-to-capacity ratio greater than 1.0 . This level is typically assigned when the volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.

| Control Delay <br> (s/veh) | LOS by Volume-to-Capacity Ratio |  |
| :---: | :---: | :---: |
| $\mathbf{a} \leq 1.0$ | $\mathbf{v} \boldsymbol{>}>1.0$ |  |
| $\leq 10$ | A | F |
| $>10-20$ | B | F |
| $>20-35$ | C | F |
| $>35-55$ | D | F |
| $>55-80$ | E | F |
| $>80$ | F | F |

Note: a For approach-based and intersectionwide assessments, LOS is defined solely by control delay.

The following information is referenced from the Highway Capacity Manual: A Guide for Multimodal Mobility Analysis, $6^{\text {th }}$ Edition, Transportation Research Board, 2016: Chapter 20 - Two-Way Stop-Controlled Intersections, Chapter 21 - All-Way Stop-Controlled Intersections, and Chapter 22 - Roundabouts.

## Motorized Vehicle Level of Service (LOS) for Unsignalized \& Roundabout Intersections

LOS is a quantitative stratification of performance measure(s) representing quality of service. Quality of service describes how well a transportation facility or service operates from a traveler's perspective. LOS is measured on an A - F scale, with LOS A representing the best operating conditions from a traveler's perspective.

| Control Delay <br> (s/veh) | LOS by Volume-to-Capacity Ratio ${ }^{\mathbf{a}}$ |  |
| :---: | :---: | :---: |
| $0-10$ | $\mathbf{v} \leq 1.0$ | $\mathbf{v} \boldsymbol{c}>1.0$ |
| $>10-15$ | A | F |
| $>15-25$ | B | F |
| $>25-35$ | C | F |
| $>35-50$ | D | F |
| $>50$ | E | F |

Note: The LOS criteria apply to each lane on a given approach and to each approach on the minor street. LOS is not calculated for major-street approaches or for the intersection as a whole.
a For approaches and intersectionwide assessment, LOS is defined solely by control delay.

## APPENDIX C

Capacity Worksheets

| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.5 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | Mr |  | 作 |  | 1 | 个4 |
| Traffic Vol, veh/h | 8 | 9 | 221 | 7 | 6 | 182 |
| Future Vol, veh/h | 8 | 9 | 221 | 7 | 6 | 182 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 250 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 9 | 10 | 240 | 8 | 7 | 198 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 357 | 124 | 0 | 0 | 248 | 0 |
| Stage 1 | 244 | - | - | - | - | - |
| Stage 2 | 113 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 | - | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 615 | 904 | - | - | 1315 | - |
| Stage 1 | 774 | - | - | - | - | - |
| Stage 2 | 899 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 612 | 904 | - | - | 1315 | - |
| Mov Cap-2 Maneuver | 612 | - | - | - | - | - |
| Stage 1 | 774 | - | - | - | - | - |
| Stage 2 | 895 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 10 |  | 0 |  | 0.2 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 738 | 1315 | - |
| HCM Lane V/C Ratio |  | - | - | 0.025 | 0.005 | - |
| HCM Control Delay (s) |  | - | - | 10 | 7.8 | - |
| HCM Lane LOS |  | - | - | B | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.1 | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3.1 |  |  |  |  |  |
| Movement | SEL | SER | NEL | NET | SWT | SWR |
| Lane Configurations | $\mathbf{r}$ |  |  | $\mathbf{A}$ | $\mathbf{F}$ |  |
| Traffic Vol, veh/h | 0 | 3 | 11 | 4 | 15 | 1 |
| Future Vol, veh/h | 0 | 3 | 11 | 4 | 15 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 3 | 12 | 4 | 16 | 1 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 0.9 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * |  | 中 ${ }^{\text {F }}$ |  | ${ }^{7}$ | 44 |
| Traffic Vol, veh/h | 26 | 18 | 355 | 11 | 22 | 415 |
| Future Vol, veh/h | 26 | 18 | 355 | 11 | 22 | 415 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 250 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 28 | 20 | 386 | 12 | 24 | 451 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 666 | 199 | 0 | 0 | 398 | 0 |
| Stage 1 | 392 | - | - | - | - | - |
| Stage 2 | 274 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 | - | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 393 | 809 | - | - | 1157 | - |
| Stage 1 | 652 | - | - | - | - | - |
| Stage 2 | 747 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 385 | 809 | - | - | 1157 | - |
| Mov Cap-2 Maneuver | 385 | - | - | - | - | - |
| Stage 1 | 652 | - | - | - | - | - |
| Stage 2 | 731 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 13.1 |  | 0 |  | 0.4 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 490 | 1157 | - |
| HCM Lane V/C Ratio |  | - | - | 0.098 | 0.021 | - |
| HCM Control Delay (s) |  | - | - | 13.1 | 8.2 | - |
| HCM Lane LOS |  | - | - | B | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.3 | 0.1 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.4 |  |  |  |  |  |
| Movement | SEL | SER | NEL | NET | SWT | SWR |
| Lane Configurations | $\mathbf{r}$ |  |  | $\mathbf{A}$ | $\mathbf{7}$ |  |
| Traffic Vol, veh/h | 3 | 26 | 22 | 8 | 16 | 1 |
| Future Vol, veh/h | 3 | 26 | 22 | 8 | 16 | 1 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 3 | 28 | 24 | 9 | 17 | 1 |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.9 |  |  |  |  |  |
| Movement | SEL | SER | NEL | NET | SWT | SWR |
| Lane Configurations | Kr |  |  | $\boldsymbol{\uparrow}$ | $\mathbf{7}$ |  |
| Traffic Vol, veh/h | 0 | 29 | 50 | 6 | 16 | 2 |
| Future Vol, veh/h | 0 | 29 | 50 | 6 | 16 | 2 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 32 | 54 | 7 | 17 | 2 |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7.1 |  |  |  |  |  |
| Movement | SEL | SER | NEL | NET | SWT | SWR |
| Lane Configurations | Mr |  |  | $\mathbf{T}$ | F |  |
| Traffic Vol, veh/h | 3 | 112 | 109 | 13 | 17 | 5 |
| Future Vol, veh/h | 3 | 112 | 109 | 13 | 17 | 5 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 3 | 122 | 118 | 14 | 18 | 5 |


| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 271 | 21 | 23 | 0 | - | 0 |
| Stage 1 | 21 | - | - | - | - | - |
| Stage 2 | 250 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |
| Pot Cap-1 Maneuver | 718 | 1056 | 1592 | - | - | - |
| Stage 1 | 1002 | - | - | - | - | - |
| Stage 2 | 792 | - | - | - | - | - |
| Platoon blocked, \% |  |  |  | - | - | - |
| Mov Cap-1 Maneuver | 664 | 1056 | 1592 | - | - | - |
| Mov Cap-2 Maneuver | 664 | - | - | - | - | - |
| Stage 1 | 927 | - | - | - | - | - |
| Stage 2 | 792 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | SE |  | NE |  | SW |  |
| HCM Control Delay, s | 8.9 |  | 6.6 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NEL | NET SELn1 |  | SWT | SWR |
| Capacity (veh/h) |  | 1592 | - | 1040 | - | - |
| HCM Lane V/C Ratio |  | 0.074 | - | 0.12 | - | - |
| HCM Control Delay (s) |  | 7.4 | 0 | 8.9 | - | - |
| HCM Lane LOS |  | A | A | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0.2 | - | 0.4 | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.1 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{4}$ |  | $\mathbf{4} \mathbf{F}$ |  | $\mathbf{1}$ | 个4 |
| Traffic Vol, veh/h | 24 | 29 | 366 | 29 | 32 | 349 |
| Future Vol, veh/h | 24 | 29 | 366 | 29 | 32 | 349 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 250 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 26 | 32 | 398 | 32 | 35 | 379 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 674 | 215 | 0 | 0 | 430 | 0 |
| Stage 1 | 414 | - | - | - | - | - |
| Stage 2 | 260 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 |  | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 388 | 790 | - | - | 1126 | - |
| Stage 1 | 635 | - | - | - | - | - |
| Stage 2 | 760 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 376 | 790 | - | - | 1126 | - |
| Mov Cap-2 Maneuver | 376 | - | - | - | - | - |
| Stage 1 | 635 | - | - | - | - | - |
| Stage 2 | 736 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 12.7 |  | 0 |  | 0.7 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT NBRWBLn1 |  |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 527 | 1126 | - |
| HCM Lane V/C Ratio |  | - | - | 0.109 | 0.031 | - |
| HCM Control Delay (s) |  | - | - | 12.7 | 8.3 | - |
| HCM Lane LOS |  | - | - | B | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.4 | 0.1 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5.6 |  |  |  |  |  |
| Movement | SEL | SER | NEL | NET | SWT | SWR |
| Lane Configurations | $\mathbf{r}$ |  |  | $\mathbf{A}$ | $\boldsymbol{F}$ |  |
| Traffic Vol, veh/h | 0 | 31 | 56 | 8 | 23 | 3 |
| Future Vol, veh/h | 0 | 31 | 56 | 8 | 23 | 3 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 34 | 61 | 9 | 25 | 3 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 5.2 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * |  | 中 ${ }^{\text {F }}$ |  | ${ }^{7}$ | 44 |
| Traffic Vol, veh/h | 79 | 76 | 597 | 58 | 83 | 666 |
| Future Vol, veh/h | 79 | 76 | 597 | 58 | 83 | 666 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 250 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 86 | 83 | 649 | 63 | 90 | 724 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 1223 | 356 | 0 | 0 | 712 | 0 |
| Stage 1 | 681 | - | - | - | - | - |
| Stage 2 | 542 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 | - | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 172 | 640 | - | - | 884 | - |
| Stage 1 | 464 | - | - | - | - | - |
| Stage 2 | 547 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 154 | 640 | - | - | 884 | - |
| Mov Cap-2 Maneuver | 154 | - | - | - | - | - |
| Stage 1 | 464 | - | - | - | - | - |
| Stage 2 | 491 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 46.8 |  | 0 |  | 1.1 |  |
| HCM LOS | E |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 245 | 884 | - |
| HCM Lane V/C Ratio |  | - | - | 0.688 | 0.102 | - |
| HCM Control Delay (s) |  | - | - | 46.8 | 9.5 | - |
| HCM Lane LOS |  | - | - | E | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 4.5 | 0.3 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 7 |  |  |  |  |  |
| Movement | SEL | SER | NEL | NET | SWT | SWR |
| Lane Configurations | Y |  |  | $\mathbf{A}$ | $\mathbf{F}$ |  |
| Traffic Vol, veh/h | 5 | 124 | 119 | 17 | 24 | 6 |
| Future Vol, veh/h | 5 | 124 | 119 | 17 | 24 | 6 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 5 | 135 | 129 | 18 | 26 | 7 |


| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 306 | 30 | 33 | 0 | - | 0 |
| Stage 1 | 30 | - | - | - | - | - |
| Stage 2 | 276 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |
| Pot Cap-1 Maneuver | 686 | 1044 | 1579 | - | - | - |
| Stage 1 | 993 | - | - | - | - | - |
| Stage 2 | 771 | - | - | - | - | - |
| Platoon blocked, \% |  |  |  | - | - | - |
| Mov Cap-1 Maneuver | 629 | 1044 | 1579 | - | - | - |
| Mov Cap-2 Maneuver | 629 | - | - | - | - | - |
| Stage 1 | 911 | - | - | - | - | - |
| Stage 2 | 771 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | SE |  | NE |  | SW |  |
| HCM Control Delay, s | 9.1 |  | 6.5 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NEL | NET SELn1 |  | SWT | SWR |
| Capacity (veh/h) |  | 1579 | - | 1018 | - | - |
| HCM Lane V/C Ratio |  | 0.082 | - | 0.138 | - | - |
| HCM Control Delay (s) |  | 7.5 | 0 | 9.1 | - | - |
| HCM Lane LOS |  | A | A | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0.3 | - | 0.5 | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | M |  | 个F |  | F | 个4 |
| Traffic Vol, veh/h | 32 | 38 | 264 | 29 | 34 | 253 |
| Future Vol, veh/h | 32 | 38 | 264 | 29 | 34 | 253 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 250 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 35 | 41 | 287 | 32 | 37 | 275 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 515 | 160 | 0 | 0 | 319 | 0 |
| Stage 1 | 303 | - | - | - | - | - |
| Stage 2 | 212 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 | - | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 489 | 857 | - | - | 1238 | - |
| Stage 1 | 723 | - | - | - | - | - |
| Stage 2 | 803 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 474 | 857 | - | - | 1238 | - |
| Mov Cap-2 Maneuver | 474 | - | - | - | - | - |
| Stage 1 | 723 | - | - | - | - | - |
| Stage 2 | 779 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 11.5 |  | 0 |  | 0.9 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 626 | 1238 | - |
| HCM Lane V/C Ratio |  | - | - | 0.122 | 0.03 | - |
| HCM Control Delay (s) |  | - | - | 11.5 | 8 | - |
| HCM Lane LOS |  | - | - | B | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.4 | 0.1 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.3 |  |  |  |  |  |
| Movement | NWL | NWR | NET | NER | SWL | SWT |
| Lane Configurations | 1 |  | $\mathbf{T}$ |  |  | -1 |
| Traffic Vol, veh/h | 10 | 0 | 6 | 4 | 0 | 18 |
| Future Vol, veh/h | 10 | 0 | 6 | 4 | 0 | 18 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 11 | 0 | 7 | 4 | 0 | 20 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 29 | 9 | 0 | 0 | 11 | 0 |
| Stage 1 | 9 | - | - | - | - | - |
| Stage 2 | 20 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 |  | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 986 | 1073 | - | - | 1608 | - |
| Stage 1 | 1014 | - | - | - | - | - |
| Stage 2 | 1003 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 986 | 1073 | - | - | 1608 | - |
| Mov Cap-2 Maneuver | 986 | - | - | - | - | - |
| Stage 1 | 1014 | - | - | - | - | - |
| Stage 2 | 1003 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | NW |  | NE |  | SW |  |
| HCM Control Delay, s | 8.7 |  | 0 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NET | NERNWLn1 |  | SWL | SWT |
| Capacity (veh/h) |  | - | - | 986 | 1608 | - |
| HCM Lane V/C Ratio |  | - | - | 0.011 | - | - |
| HCM Control Delay (s) |  | - | - | 8.7 | 0 | - |
| HCM Lane LOS |  | - | - | A | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0 | 0 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |




| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 952 | 271 | 0 | 0 | 542 | 0 |
| Stage 1 | 507 | - | - | - | - | - |
| Stage 2 | 445 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 | - | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 257 | 727 | - | - | 1023 | - |
| Stage 1 | 570 | - | - | - | - | - |
| Stage 2 | 613 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 233 | 727 | - | - | 1023 | - |
| Mov Cap-2 Maneuver | 233 | - | - | - | - | - |
| Stage 1 | 570 | - | - | - | - | - |
| Stage 2 | 555 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 23.7 |  | 0 |  | 1.4 |  |
| HCM LOS | C |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT | NBRWBLn1 |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 355 | 1023 | - |
| HCM Lane V/C Ratio |  | - | - | 0.465 | 0.095 | - |
| HCM Control Delay (s) |  | - | - | 23.7 | 8.9 | - |
| HCM Lane LOS |  | - | - | C | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 2.4 | 0.3 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 6.7 |  |  |  |  |  |
| Movement | SEL | SER | NEL | NET | SWT | SWR |
| Lane Configurations | MF |  |  | $\mathbf{4}$ | $\mathbf{7}$ |  |
| Traffic Vol, veh/h | 3 | 112 | 109 | 25 | 24 | 5 |
| Future Vol, veh/h | 3 | 112 | 109 | 25 | 24 | 5 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 3 | 122 | 118 | 27 | 26 | 5 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.1 |  |  |  |  |  |
| Movement | NWL | NWR | NET | NER | SWL | SWT |
| Lane Configurations | $\mathbf{r}$ |  | $\uparrow$ |  |  | - |
| Traffic Vol, veh/h | 7 | 0 | 16 | 12 | 0 | 22 |
| Future Vol, veh/h | 7 | 0 | 16 | 12 | 0 | 22 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 8 | 0 | 17 | 13 | 0 | 24 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 48 | 24 | 0 | 0 | 30 | 0 |
| Stage 1 | 24 | - | - | - | - | - |
| Stage 2 | 24 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |
| Pot Cap-1 Maneuver | 962 | 1052 | - | - | 1583 | - |
| Stage 1 | 999 | - | - | - | - | - |
| Stage 2 | 999 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 962 | 1052 | - | - | 1583 | - |
| Mov Cap-2 Maneuver | 962 | - | - | - | - | - |
| Stage 1 | 999 | - | - | - | - | - |
| Stage 2 | 999 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | NW |  | NE |  | SW |  |
| HCM Control Delay, s | 8.8 |  | 0 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | N | NERNWLn1 |  | SWL SWT |  |
| Capacity (veh/h) |  | - | - | 962 | 1583 | - |
| HCM Lane V/C Ratio |  | - | - | 0.008 | - | - |
| HCM Control Delay (s) |  | - | - | 8.8 | 0 | - |
| HCM Lane LOS |  | - | - | A | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0 | 0 | - |



| Major/Minor | Minor1 | Major1 |  |  | Major2 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 310 | 158 | 0 | 0 | 167 | 0 |  |
| Stage 1 | 158 | - | - | - | - | - |  |
| Stage 2 | 152 | - | - | - | - | - |  |
| Critical Hdwy | 6.42 | 6.22 | - | - | 4.12 | - |  |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |  |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |  |
| Follow-up Hdwy | 3.518 | 3.318 | - | - | 2.218 | - |  |
| Pot Cap-1 Maneuver | 682 | 887 | - | - | 1411 | - |  |
| Stage 1 | 871 | - | - | - | - | - |  |
| Stage 2 | 876 | - | - | - | - | - |  |
| Platoon blocked, \% |  |  | - | - |  | - |  |
| Mov Cap-1 Maneuver | 682 | 887 | - | - | 1411 | - |  |
| Mov Cap-2 Maneuver | 682 | - | - | - | - | - |  |
| Stage 1 | 871 | - | - | - | - | - |  |
| Stage 2 | 876 | - | - | - | - | - |  |
|  |  |  |  |  |  |  |  |
| Approach | NW |  | NE |  | SW |  |  |
| HCM Control Delay, s | 10.4 |  | 0 |  | 0 |  |  |
| HCM LOS | B |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |
| Minor Lane/Major Mvm |  | NET | NER | WLn1 | SWL | SWT |  |
| Capacity (veh/h) |  | - | - | 682 | 1411 | - |  |
| HCM Lane V/C Ratio |  | - | - | 0.016 | - | - |  |
| HCM Control Delay (s) |  | - | - | 10.4 | 0 | - |  |
| HCM Lane LOS |  | - | - | B | A | - |  |
| HCM 95th \%tile Q(veh) |  | - | - | 0 | 0 | - |  |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | $\mathbf{4}$ |  | $\mathbf{4} \mathbf{F}$ |  | $\mathbf{1}$ | 个4 |
| Traffic Vol, veh/h | 36 | 43 | 366 | 33 | 37 | 349 |
| Future Vol, veh/h | 36 | 43 | 366 | 33 | 37 | 349 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 250 | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 39 | 47 | 398 | 36 | 40 | 379 |


| Major/Minor M | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 686 | 217 | 0 | 0 | 434 | 0 |
| Stage 1 | 416 | - | - | - | - | - |
| Stage 2 | 270 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 |  | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 |  | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 |  | - | 2.22 | - |
| Pot Cap-1 Maneuver | 381 | 787 | - | - | 1122 | - |
| Stage 1 | 634 | - | - | - | - | - |
| Stage 2 | 751 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 367 | 787 | - | - | 1122 | - |
| Mov Cap-2 Maneuver | 367 | - | - | - | - | - |
| Stage 1 | 634 | - | - | - | - | - |
| Stage 2 | 724 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 13.3 |  | 0 |  | 0.8 |  |
| HCM LOS | B |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT NBRWBLn1 |  |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 517 | 1122 | - |
| HCM Lane V/C Ratio |  | - | - | 0.166 | 0.036 | - |
| HCM Control Delay (s) |  | - | - | 13.3 | 8.3 | - |
| HCM Lane LOS |  | - | - | B | A | - |
| HCM 95th \%tile Q(veh) |  | - | - | 0.6 | 0.1 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 5 |  |  |  |  |  |
| Movement | SEL | SER | NEL | NET | SWT | SWR |
| Lane Configurations | $\mathbf{r}$ |  |  | $\mathbf{A}$ | $\boldsymbol{F}$ |  |
| Traffic Vol, veh/h | 0 | 31 | 56 | 12 | 33 | 3 |
| Future Vol, veh/h | 0 | 31 | 56 | 12 | 33 | 3 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 0 | 34 | 61 | 13 | 36 | 3 |





| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1 |  |  |  |  |  |
| Movement | NWL | NWR | NET | NER | SWL | SWT |
| Lane Configurations | 1 |  | $\uparrow$ |  |  | - |
| Traffic Vol, veh/h | 16 | 0 | 65 | 5 | 0 | 65 |
| Future Vol, veh/h | 16 | 0 | 65 | 5 | 0 | 65 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 17 | 0 | 71 | 5 | 0 | 71 |



| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh | 7.7 |  |  |  |  |  |
| Movement | WBL | WBR | NBT | NBR | SBL | SBT |
| Lane Configurations | * |  | 中 ${ }^{\text {a }}$ |  | ${ }^{*}$ | 44 |
| Traffic Vol, veh/h | 87 | 85 | 597 | 71 | 99 | 666 |
| Future Vol, veh/h | 87 | 85 | 597 | 71 | 99 | 666 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | 250 | - |
| Veh in Median Storage, \# | \# 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 95 | 92 | 649 | 77 | 108 | 724 |


| Major/Minor | Minor1 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 1266 | 363 | 0 | 0 | 726 | 0 |
| Stage 1 | 688 | - | - | - | - | - |
| Stage 2 | 578 | - | - | - | - | - |
| Critical Hdwy | 6.84 | 6.94 | - | - | 4.14 | - |
| Critical Hdwy Stg 1 | 5.84 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.84 | - | - | - | - | - |
| Follow-up Hdwy | 3.52 | 3.32 | - | - | 2.22 | - |
| Pot Cap-1 Maneuver | 161 | 634 | - | - | 873 | - |
| Stage 1 | 460 | - | - | - | - | - |
| Stage 2 | 524 | - | - | - | - | - |
| Platoon blocked, \% |  |  | - | - |  | - |
| Mov Cap-1 Maneuver | 141 | 634 | - | - | 873 | - |
| Mov Cap-2 Maneuver | 141 | - | - | - | - | - |
| Stage 1 | 460 | - | - | - | - | - |
| Stage 2 | 459 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | WB |  | NB |  | SB |  |
| HCM Control Delay, s | 65.9 |  | 0 |  | 1.3 |  |
| HCM LOS | F |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NBT NBRWBLn1 |  |  | SBL | SBT |
| Capacity (veh/h) |  | - | - | 229 | 873 | - |
| HCM Lane V/C Ratio |  | - | - | 0.816 | 0.123 | - |
| HCM Control Delay (s) |  | - | - | 65.9 | 9.7 | - |
| HCM Lane LOS |  | - | - | F | A | - |
| HCM 95th \%tile Q(veh) |  | - |  | 6.1 | 0.4 | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 6.6 |  |  |  |  |  |
| Movement | SEL | SER | NEL | NET | SWT | SWR |
| Lane Configurations | M |  |  | $\mathbf{T}$ | $\mathbf{7}$ |  |
| Traffic Vol, veh/h | 5 | 124 | 119 | 29 | 31 | 6 |
| Future Vol, veh/h | 5 | 124 | 119 | 29 | 31 | 6 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | - | 0 | 0 | - |
| Grade, \% | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 5 | 135 | 129 | 32 | 34 | 7 |


| Major/Minor | Minor2 |  | Major1 |  | Major2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 328 | 38 | 41 | 0 | - | 0 |
| Stage 1 | 38 | - | - | - | - | - |
| Stage 2 | 290 | - | - | - | - | - |
| Critical Hdwy | 6.42 | 6.22 | 4.12 | - | - | - |
| Critical Hdwy Stg 1 | 5.42 | - | - | - | - | - |
| Critical Hdwy Stg 2 | 5.42 | - | - | - | - | - |
| Follow-up Hdwy | 3.518 | 3.318 | 2.218 | - | - | - |
| Pot Cap-1 Maneuver | 666 | 1034 | 1568 | - | - | - |
| Stage 1 | 984 | - | - | - | - | - |
| Stage 2 | 759 | - | - | - | - | - |
| Platoon blocked, \% |  |  |  | - | - | - |
| Mov Cap-1 Maneuver | 610 | 1034 | 1568 | - | - | - |
| Mov Cap-2 Maneuver | 610 | - | - | - | - | - |
| Stage 1 | 901 | - | - | - | - | - |
| Stage 2 | 759 | - | - | - | - | - |
|  |  |  |  |  |  |  |
| Approach | SE |  | NE |  | SW |  |
| HCM Control Delay, s | 9.2 |  | 6 |  | 0 |  |
| HCM LOS | A |  |  |  |  |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | NEL | NET SELn1 |  | SWT | SWR |
| Capacity (veh/h) |  | 1568 | - | 1007 | - | - |
| HCM Lane V/C Ratio |  | 0.082 | - | 0.139 | - | - |
| HCM Control Delay (s) |  | 7.5 | 0 | 9.2 | - | - |
| HCM Lane LOS |  | A | A | A | - | - |
| HCM 95th \%tile Q(veh) |  | 0.3 | - | 0.5 | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 0.9 |  |  |  |  |  |
| Movement | NWL | NWR | NET | NER | SWL | SWT |
| Lane Configurations | $\mathbf{r}$ |  | $\uparrow$ |  |  | - |
| Traffic Vol, veh/h | 7 | 0 | 20 | 12 | 0 | 30 |
| Future Vol, veh/h | 7 | 0 | 20 | 12 | 0 | 30 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Stop | Stop | Free | Free | Free | Free |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | 0 | - | - | - | - | - |
| Veh in Median Storage, \# | 0 | - | 0 | - | - | 0 |
| Grade, \% | 0 | - | 0 | - | - | 0 |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 8 | 0 | 22 | 13 | 0 | 33 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |




[^0]:    ${ }^{1}$ El Paso County 2016 Major Transportation Corridors Plan Update, Felsburg Holt \& Ullevig, December 2016.
    ${ }^{2}$ El Paso County Engineering Criteria Manual, El Paso County, July 2023.

[^1]:    ${ }^{3}$ Moving Forward 2045: Pikes Peak Area Regional Transportation Plan, PPACG, January 2020.
    ${ }^{4}$ Struthers Ranch Subdivision Filing No. 5: Traffic Impact Study, LSC Transportation Consultants, Inc., May 14, 2021.
    ${ }^{5}$ Monument Ridge Lots 7 \& 8: Transportation Memorandum, LSC Transportation Consultants, Inc., December 20, 2019.
    ${ }^{6}$ Cathedral Rock Commons: Traffic Impact Study, SM ROCHA, LLC, March 2023.

