

GENERAL NOTES

1.1 Fabrication shall be in accordance with A.S.C. standard practices in compliance with the applicable sections, relating to design requirements and allowable stresses of the latest edition of the "AWS Structural Welding Code D1.1 and D1.3".

MATERIALS	ASTM DESIGNATION	MIN. YIELD STRENGTH
Hot Rolled Steel Shapes (W & C)	A572	Fy = 50 KSI
Hot Rolled Steel Angles (L)	A36	Fy = 36 KSI
Steel Pipes	A500	Fy = 42 KSI
Structural Tubing	A500	Fy = 42 KSI
Structural Steel Web Plate	A572/A1011	Fy = 50 KSI
Structural Steel Flange Plates/Bars	A529/A572	Fy = 55 KSI
Cold Formed Light Gage	A653/A1011	Fy = 55 KSI
Roof and Wall Sheets	A792/A653	Fy = 50, 80 KSI
Cable Brace	A475 - TYPE 1	Extra High Strength
Rod Brace	A529	Fy = 50 KSI
		MIN. TENSILE STRENGTH
Machine Bolts & Nuts	A307	Fu = 60 KSI
High Strength Bolts (1" and less)	A325-TYPE 1	Fu = 120 KSI
High Strength Bolts (>1" to 1 1/2")	A325-TYPE 1	Fu = 105 KSI
Anchor Bolts (Not supplied by A.S.C.)	A36/A307/F1554	Fu = 60 KSI

1.3 **PRIMER**
Shop primer paint is a rust inhibitive primer which meets the end performance of Federal Specification SSPC No. 15 and is A.S.C. Gray Oxide color. This paint is not intended for long term exposure to the elements. A.S.C. is not responsible for any deterioration of the shop primer paint as a result of improper handling and/or jobsite storage. A.S.C. shall not be responsible for any field applied paint and/or coatings. (AISC Code of Standard Practice, Latest Edition). Nominal thickness of primer will be 1 mil unless otherwise specified in contract documents.

1.4 **GALVANIZED OR SPECIAL COATINGS:**
See Contract Documents

1.5 **ALL BOLTS ARE 1/2" x 0'-1 1/4" A307 EXCEPT:**
a) Endwall rafter splice - 5/8" x 0'-1 3/4" A325-N
b) Endwall column to rafter connection - 1/2" x 0'-1 1/4" A325 MIN.(SEE WALL ELEVATION)
c) Main frame connections - SEE CROSS SECTION
d) Flange Brace connections - 1/2" x 0'-1 1/4" A325
NOTE: Washers are not supplied unless noted otherwise on drawing

1.6 **A325 BOLT TIGHTENING REQUIREMENTS**
All high strength bolts are A325-N unless specifically noted otherwise. Holes are not slotted and design is bearing connection. Structural bolts shall be tightened by the turn-of-the-nut method in accordance with the Latest Edition AISC "Specification For Structural Joints" using ASTM A325 or A490 Bolts, when specifically required. A325-N bolts are supplied without washer unless otherwise noted on the drawings.
All bolted connections unless noted are designed as bearing type connections with bolt threads not excluded from the shear plane.

1.7 **CLOSURE STRIPS ARE FURNISHED (IF ORDERED) FOR APPLICATION:**
INSIDE - Under roof panels & base of wall panels
OUTSIDE - Between roof panels & ridge cap
- Between wall panels & eave/gable trim

1.8 **ERECTION NOTE:**
All bracing, strapping, & bridging shown and provided by A.S.C. for this building is required and shall be installed by the erector as a permanent part of the structure. If additional bracing is required for stability during erection, it shall be the erector's responsibility to determine the amount of such bracing and to procure and install as needed.

1.9 **ERECTION AND UNLOADING NOT BY A.S.C.**

1.10 **SHORTAGES**
Any claims or shortages by buyer must be made to A.S.C. within five (5) working days after delivery, or such claims will be considered to have been waived by the customer and disallowed.

1.11 **CORRECTIONS OF ERRORS AND REPAIRS (MBMA 6.10)**
Claims for correction of alleged misfits will be disallowed unless A.S.C. shall have received prior notice thereof and allowed reasonable inspection of such misfits. The correction of minor misfits by the use of drift pins to draw the components into line, moderate amounts of reaming, chipping and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. No part of the Building may be returned for alleged misfits without the prior approval of A.S.C.

BUYER/END USE CUSTOMER RESPONSIBILITIES

- 2.1 It is the responsibility of the BUYER/END USE CUSTOMER to obtain appropriate approvals and secure necessary permits from City, County, State, or Federal Agencies as required, and to advise/release A.S.C. to fabricate upon receiving such.
- 2.2 Armstrong Steel Corp (hereafter referred to as A.S.C.) standard specifications apply unless stipulated otherwise in the Contract Documents. A.S.C. design, fabrication, quality criteria, standards, practice, methods and tolerances shall govern the work with any other interpretations to the contrary notwithstanding. It is understood by both Parties that the BUYER/END USE CUSTOMER is responsible for clarification of inclusions or exclusions from the architectural plans and/or specifications.
- 2.3 In case of discrepancies between A.S.C. structural steel plans and plans for other trades, A.S.C. plans shall govern. (Section 3 AISC Code of Standard Practices, Latest Edition)
- 2.4 Approval of A.S.C. drawings and calculations indicates that A.S.C. has correctly interpreted and applied the Contract Documents. This approval constitutes the contractor/owners acceptance of the A.S.C. design concepts, assumptions, and loading. (Section 4 AISC Code and MBMA 3.3.3)
- 2.5 Once the BUYER/END USE CUSTOMER has signed A.S.C. Approval Package and the project is released for fabrication, changes shall be billed to the BUYER/END USE CUSTOMER including material, engineering and other costs. An additional fee may be charged if the project must be moved from the fabrication and shipping schedule.
- 2.6 The BUYER/END USE CUSTOMER is responsible for overall project coordination. All interface, compatibility, and design considerations concerning any materials not furnished by A.S.C. and A.S.C. steel system are to be considered and coordinated by the BUYER/END USE CUSTOMER. Specific design criteria concerning this interface between materials must be furnished before release for fabrication or A.S.C. assumptions will govern (AISC Code of Standard Practice, Latest Edition)



PHONE: 800-345-4610
www.armstrongsteel.com

JOB NO. : 56633

CUSTOMER : JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER : JAMY WHITEMAN
END USE : RESIDENCE
LOCATION : 7445 MEADOWPINE DR
: BLACK FOREST, CO 80908
: EL PASO COUNTY
PH. NO. : 719-330-6681 EMAIL: JAMY@TRUSTFORTIFIED.COM

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE FOLLOWING AS INDICATED:

DESIGN LOADS:	56633A	56633B	BUILDING DESCRIPTION:	56633A	56633B	56633A
Design Code / Wind Code	:IBC-15	:IBC-15	Width (ft)	:40	:8.17	Insulation
Building Risk Category	:II - Normal	:II - Normal	Length (ft)	:60	:60	Roof Insulation : R-49
Enclosure	:Closed	:Partially Enclosed	Eave Ht. at BSW (ft)	:19.5	:9.08	Wall Insulation : R-25
Dead Load (psf)	:4.00	:4.25	Eave Ht. at FSW (ft)	:19.5	:10.44	
Collateral Load (psf)	:12.00	:2.00	Roof Slope at BSW	:5.0:12	:2.0:12	
Wind Load			Roof Slope at FSW	:5.0:12	:Single Slope	
Ultimate Wind Speed, (Vult) (mph)	:130.00	:130.00	Bay Spacing (ft)	:1 at 19, 1 at 22	:1 at 19, 1 at 22	
Wind Exposure	:C	:C				
Internal Pressure Coefficient, GCpi	:0.18/-0.18	:0.55/-0.55				
Wall Panel Design Wind Pressure (psf)	:40.3/-43.7	:45.3/-48.1				
Live Load			COVERING AND TRIMS:			
Primary Framing (psf)	:20.00		ALL ROOF AND WALL COVERING AND TRIM			
Trib. Area Reduction	:No		TO BE DESIGNED AND SUPPLIED "BY			
Secondary Framing (psf)	:20.00		OTHERS" UNLESS NOTED OTHERWISE			
Snow Load						
Ground Snow Load, Pg (psf)	:40.00					
Roof Snow Load, Pf (psf)	:32.00					
Sloped Roof Snow Load, Ps (psf)	:32.00					
Snow Exposure Factor, Ce	:1.000					
Snow Importance Factor, Is	:1.00					
Thermal Factor, Ct	:1.10					
Sloped Factor, Cs	:0.79					
Seismic Load						
Seismic Importance Factor, Ie	:1.00					
Site Class	:D					
Mapped Spectral Response Acceleration	:Ss = 0.185	:S1 = 0.059				
Spectral Response Coefficients	:Sds = 0.197	:Sd1 = 0.094				
Seismic Design Category	:B					
Basic Force Resisting Systems Used	:Steel System Not Specifically					
	:Detailed For Resistance					
	:Rigid Frames (OMF)					
	:Braced Frames (OCBF/OMF)					
Total Design Base Shear, V (kips)	:Longitudinal = 7.03	= 0.00				
	:Transverse = 6.48	= 0.68				
Response Modification Factors, R	:Rigid Frames = 3.00	Ω = 3.00				
	:SW X-Bracing = 3.00	Ω = 2.00				
	:SW Wind Bent = 3.00	Ω = 3.00				
Seismic Response Coefficient, Cs	:Rigid Frames = 0.0658					
	:SW X-Bracing = 0.0658					
	:SW Wind Bent = 0.0658					
Analysis Procedure Used	:Equivalent Lateral Force Procedure					
Other Loads/Requirements						

Drawing Index	
Drawing Name	Page(s)
Drawing Cover	COVER
3D Reference	3D
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Anchor Bolt Details	3
Anchor Bolt Reactions	4-5
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Sidewall/Endwall	10-14
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SEALING OF THIS DRAWING DOES NOT IMPLY OR CONSTITUTE THAT ARMSTRONG STEEL ENGINEER IS THE ENGINEER OF RECORD OR THE DESIGN PROFESSIONAL FOR THIS PROJECT. ONLY THE DESIGN OF THE METAL BUILDING SYSTEM AS FURNISHED BY A.S.C. IS INCLUDED. FOUNDATION ANALYSIS, ELECTRICAL, AND MECHANICAL SYSTEMS, AND/OR OTHER PARTS SUPPLIED BY ANYONE OTHER THAN ARMSTRONG ARE SPECIFICALLY EXCLUDED. NO INSPECTION OR SUPERVISION IS IMPLIED.

BUYER/END USE CUSTOMER RESPONSIBILITIES CONTINUED

- 2.7 It is the responsibility of the BUYER/END USE CUSTOMER to insure that A.S.C. plans comply with the applicable requirements of any governing building authorities. The supplying of sealed engineering data and drawings for the metal building system does not imply or constitute an agreement that A.S.C. or its design engineers are acting as the engineer of record or design professional for a construction project. These drawings are sealed only to certify the design of the structural components furnished by A.S.C.
- 2.8 The BUYER/END USE CUSTOMER is responsible for setting of anchor bolts and erection of steel in accordance with A.S.C. "For Construction" drawings only. Temporary supports such as guys, braces, falsework, cribbing or other elements required for the erection operation shall be determined, furnished and installed by the erector. No items should be purchased from a preliminary set of drawings, including anchor bolts. Use only final "FOR CONSTRUCTION DRAWINGS" for this use. (AISC Code of Standard Practice, Latest Edition.)
- 2.9 Armstrong Steel Corp is responsible for the design of the anchor bolt to permit the transfer of forces between the base plate and the anchor bolt in shear, bearing and tension, but is not responsible for the transfer of anchor bolt forces to the concrete or the adequacy of the anchor bolt in relation to the concrete. Unless otherwise provided in the Order Documents, A.S.C. does not design and is not responsible for the design, material and construction of the foundation or foundation embedments. The END USE CUSTOMER should assure himself that adequate provisions are made in the foundation design for loads imposed by column reactions of the building, other imposed loads, and bearing capacity of the soil and other conditions of the building site. It is recommended that the anchorage and foundation of the building be designed by a Registered Professional Engineer experienced in the design of such structures. (Latest MBMA Low Rise Building Systems Manual)
- 2.10 Normal erection operations include the corrections of minor misfits by moderate amounts of reaming, chipping, welding or cutting, and the drawing of elements into line through the use of drift pins. Errors which cannot be corrected by the foregoing means or which require major changes in member configuration are to be reported immediately to A.S.C. by the BUYER/END USE CUSTOMER, to enable whoever is responsible either to correct the error or to approve the most efficient and economic method of correction to be used by others. (AISC Code of Standard Practice Latest Edition)
- 2.11 Neither the fabricator nor the BUYER/END USE CUSTOMER will cut, drill or otherwise alter his work, or the work of other trades, to accommodate other trades, unless such work is clearly specified in the contract documents. Whenever such work is specified, the BUYER/END USE CUSTOMER is responsible for furnishing complete information as to materials, size, location and number of alterations prior to preparation of shop drawings. (AISC Code of Standard Practice Latest Edition)
- 2.12 **WARNING:** In no case should Galvalume steel panels be used in conjunction with lead or copper. Both lead and copper have harmful corrosive effects on the Galvalume alloy coating when they are in contact with Galvalume steel panels. Even run-off from copper flashing, wiring, or tubing onto Galvalume should be avoided.
- 2.13 **SAFETY COMMITMENT:** Armstrong Steel Corp has a commitment to manufacture quality building components that can be safely erected. However, the safety commitment and job site practices of the erector are beyond the control of A.S.C. It is strongly recommended that safe working conditions and accident prevention practices be the top priority of any job site. Local, State, and Federal safety and health standards should always be followed to help insure workers safety. Make certain all employees know the safest and most productive way of erecting a building. Emergency procedures should be known to all employees. Daily meetings highlighting safety procedures are also recommended. The use of hard hats, rubber sole shoes for roof work, proper equipment for handling material, and safety nets where applicable, are recommended.
- 2.14 Roof drainage systems (gutter, downspouts, etc.) must be free of any obstruction to ensure smooth operation at any given time.
- 2.15 It is recommended by Factory Mutual (Reference: B2.44) that roofs be cleared of snow when half of the maximum snow depth is reached. The maximum snow depth can be estimated based on the design snow load and the density of snow and/or ice buildup. See Chart below.

ROOF SNOW LOAD (IN PSF)	EQUIVALENT SNOW HEIGHT AT ROOF (IN INCHES)	RECOMMENDED SNOW HEIGHT WHEN SNOW REMOVAL SHOULD START (IN INCHES)
20	16.60	8.30
25	17.25	8.62
30	17.90	8.95
35	18.55	9.28
40	19.20	9.60
45	19.85	9.92
50	20.50	10.25
55	21.15	10.58
60	21.80	10.90
65	22.45	11.22
70	23.10	11.55
75	23.75	11.88
80	24.40	12.20

NOTE:
For Snow/Ice Removal Procedure, Refer to Metal Building System Manual 2002 Edition, Section A8.4, Page XI-A8-2.

Drawing Status

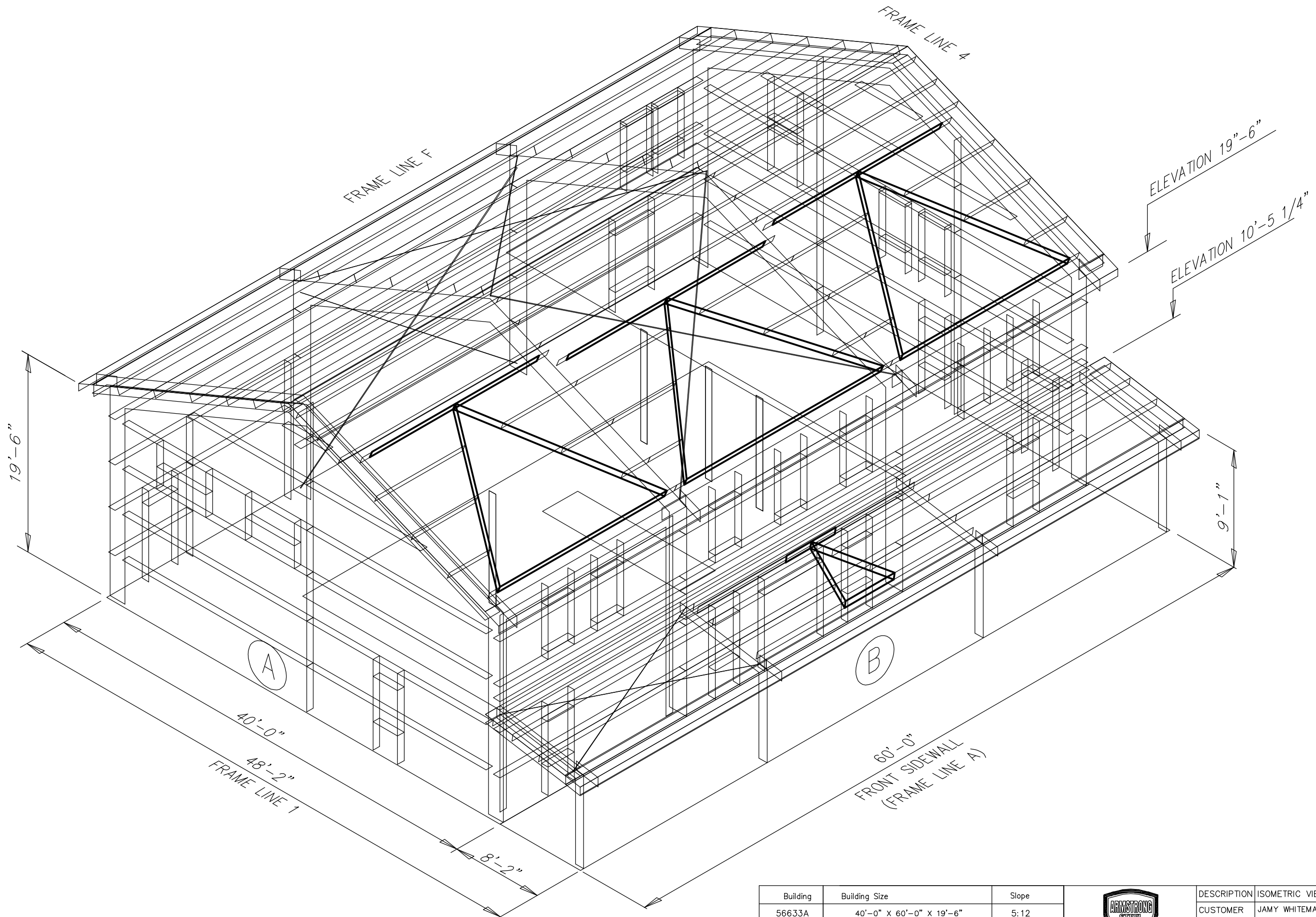
APPROVAL: These drawings, being for approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued "Construction" can be considered as complete.

REVISED APPROVAL:

PERMIT: These drawings, being for permit, are by definition not final. Only drawings issued "Construction" can be considered as complete.


REVISED PERMIT:

CONSTRUCTION: Final drawings to be used in the erection of the building.



NOTE:
 3D IS A GENERAL REPRESENTATION OF BUILDING.
 SOME MEMBERS MAY CHANGE IN FINAL ERECTION DRAWINGS

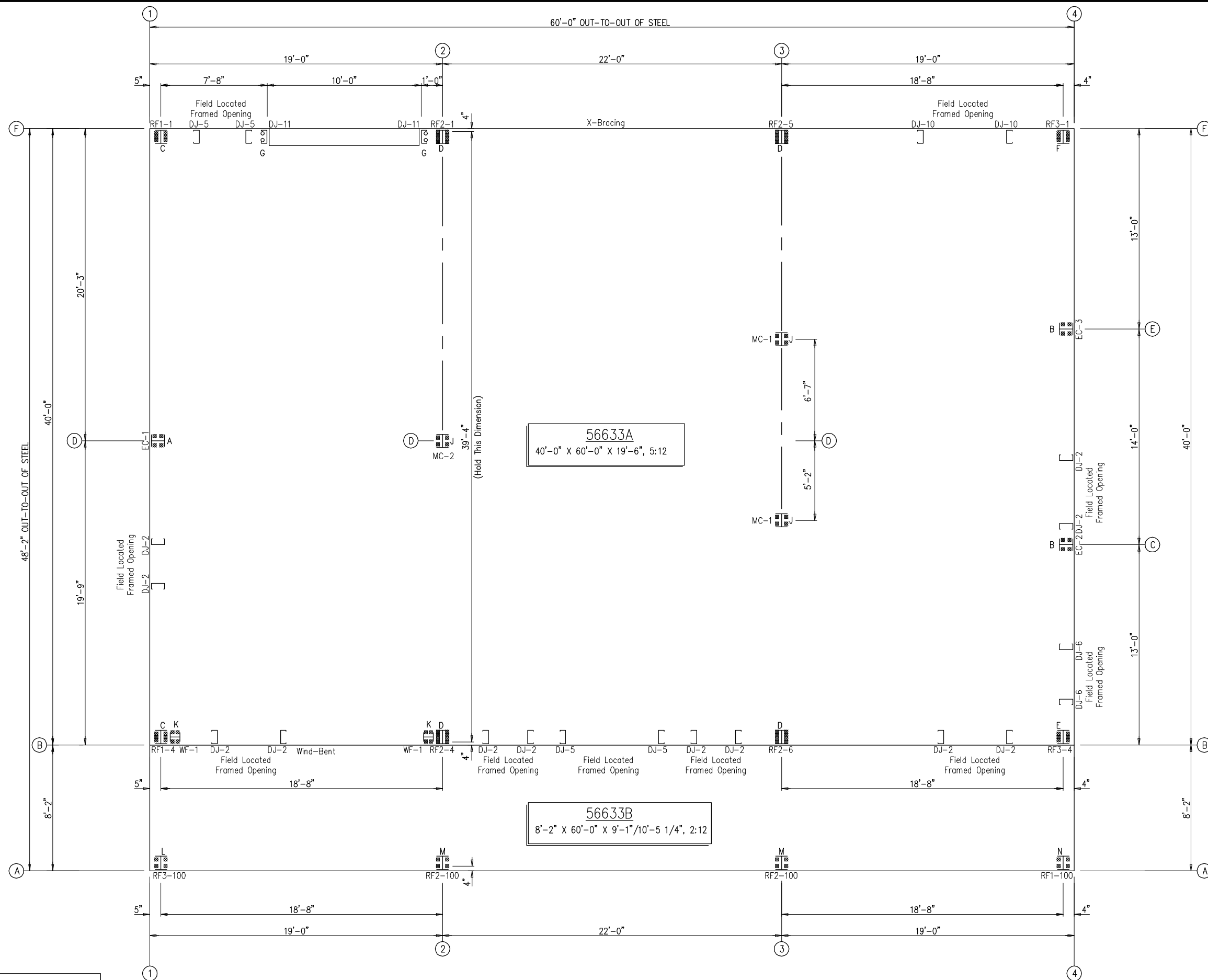
Building	Building Size	Slope
56633A	40'-0" X 60'-0" X 19'-6"	5:12
56633B	8'-2" X 60'-0" X 9'-1"/10'-5 1/4"	2:12


 2 Inverness Drive East, Ste#200
 Englewood, Colorado 80112
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DESCRIPTION	ISOMETRIC VIEW
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.:	56633
ENG. BY:	RA
DATE:	4/15/21
DWG. NO.:	3D REFERENCE
ISSUE:	P

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
4	Jamb	5/8"	A307
12	Endwall	3/4"	A307
72	Frame	3/4"	A307
8	WindCol	3/4"	A307
12	Floor	3/4"	A307



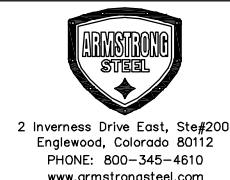
SOILS REPORT AND FOUNDATION DESIGN BY LICENSED COLORADO ENGINEER OR ARCHITECT SHALL BE ON HAND AT TIME OF FIRST INSPECTION.



NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ANCHOR BOLT PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



DESCRIPTION	ANCHOR BOLT PLAN		
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS		
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS		
SCALE	NOT TO SCALE		
JOB NO.:	56633	ENG. BY:	RA
DATE:	4/15/21	ISSUE:	P
DWG. NO.:	1 OF 21		

PERIMETER BEAM BOLT & PLATE TABLE						
ID	QUAN	TYPE	DIA	LENGTH	MARK/PART	
P1	6	A325	5/8"	1 1/2"	MC-02	
P2	6	A325	5/8"	1 1/2"	MC-02	

SUPPORT BEAM BOLT & PLATE TABLE						
ID	QUAN	TYPE	DIA	LENGTH	MARK/PART	
S1	6	A325	5/8"	1 1/2"	MC-02	
S2	6	A325	5/8"	1 1/2"	MC-02	
S3	12	A325	5/8"	1 1/2"	MC-04	
S4	12	A325	5/8"	1 1/2"	MC-04	

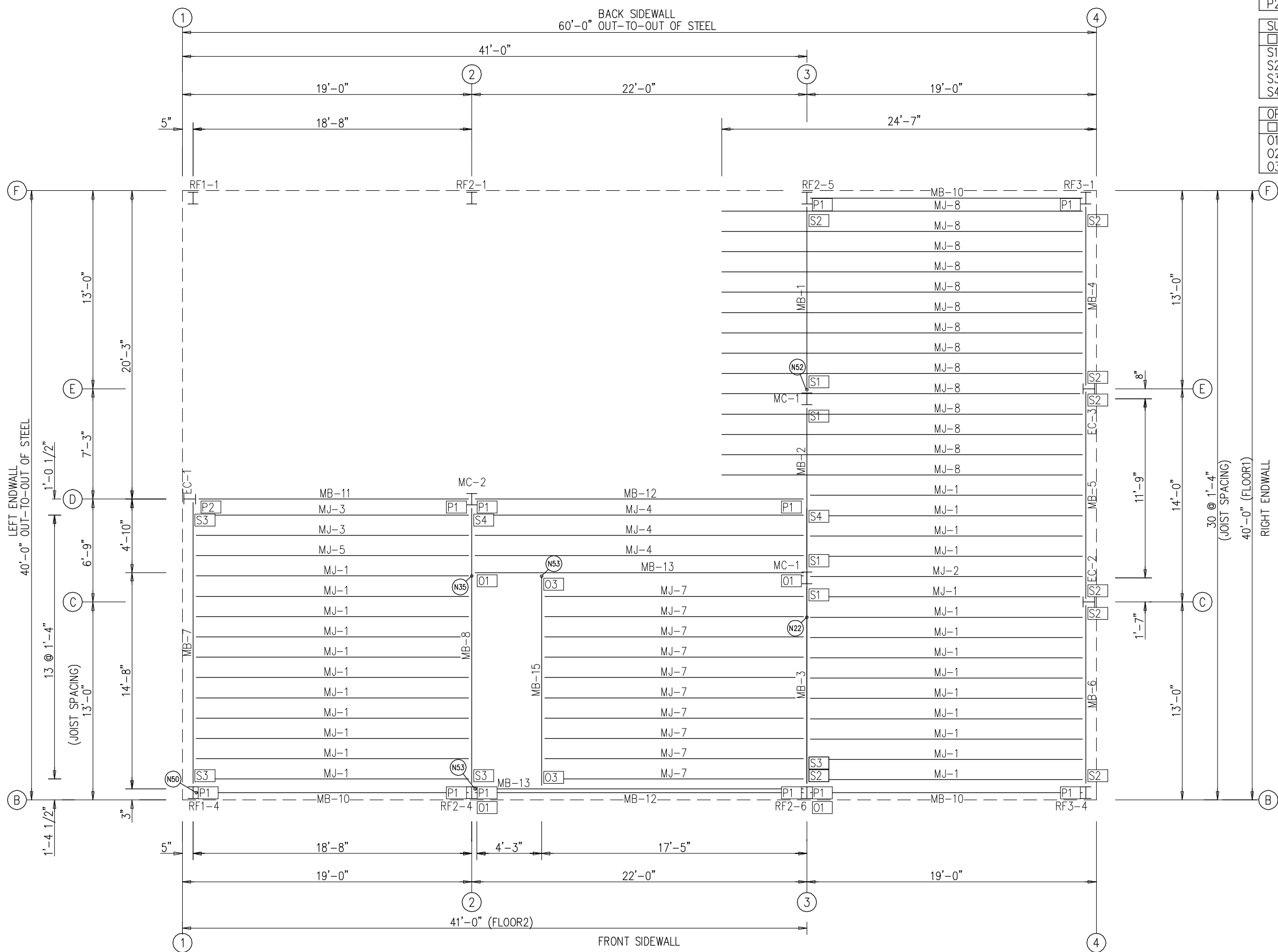
OPENING BOLT & PLATE TABLE						
ID	QUAN	TYPE	DIA	LENGTH	MARK/PART	
O1	6	A325	5/8"	1 1/2"	m7	
O2	6	A325	5/8"	1 1/2"	m9	
O3	6	A325	5/8"	1 1/2"	m8	

MEMBER TABLE				
QUAN	MARK	PART	LENGTH	
1	MB-1	W10531	11'-5 1/4"	
1	MB-2	W10531	11'-0"	
1	MB-3	W10531	12'-4 1/4"	
1	MB-4	W08531	11'-4 1/2"	
1	MB-5	W08531	13'-11"	
1	MB-6	W08531	11'-4 3/8"	
1	MB-7	W14531	18'-1 1/4"	
1	MB-8	W16543	17'-6"	
3	MB-10	W08531	18'-7"	
1	MB-11	W08531	18'-1"	
2	MB-12	W08531	21'-11"	
2	MB-13	W12X14	21'-8 1/2"	
1	MB-15	W12X14	14'-4 1/2"	
2	MC-1	W08531	9'-10 1/2"	
2	MC-2	W08531	10'-1"	
24	MJ-1	12K1	19'-3 1/2"	
2	MJ-2	12K1	18'-7 1/4"	
2	MJ-3	12K1	18'-11 1/2"	
3	MJ-4	12K1	22'-3 1/2"	
1	MJ-5	12K1	18'-11 3/4"	
11	MJ-7	12K1	18'-0 1/2"	
14	MJ-8	12K1	24'-10 1/2"	

MEZZANINE LOADS:
 LL = 40PSF
 DL = 40PSF

DEFLECTION LIMITS:
 DL + LL = 240
 LL = 360

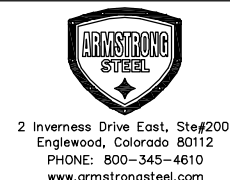
JOIST HEIGHT
 10'-1" @ TOP OF MEZZANINE JOIST/PERIMETER BEAM/
 BOTTOM OF FLOOR DECKING. DECKING IS NOT BY ASC.
 8'-5" MINIMUM CLEARANCE BELOW MEZZANINE BEAMS.



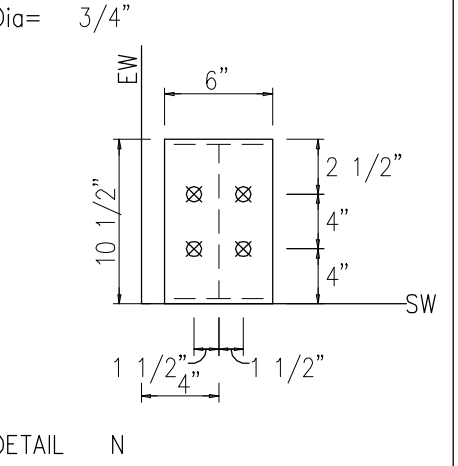
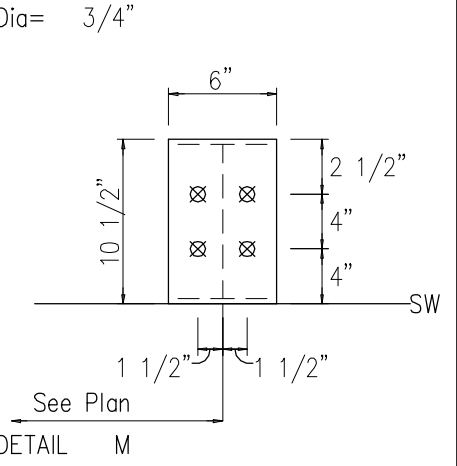
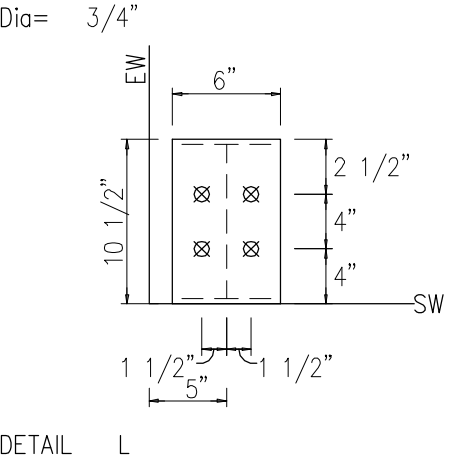
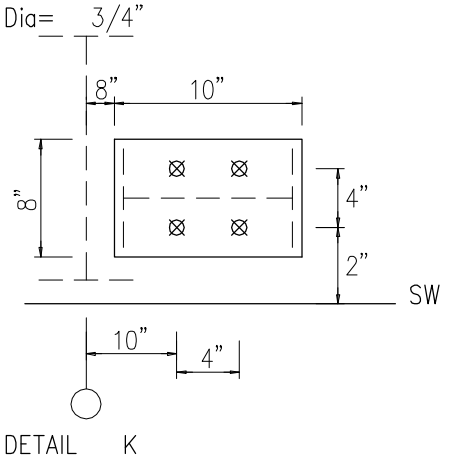
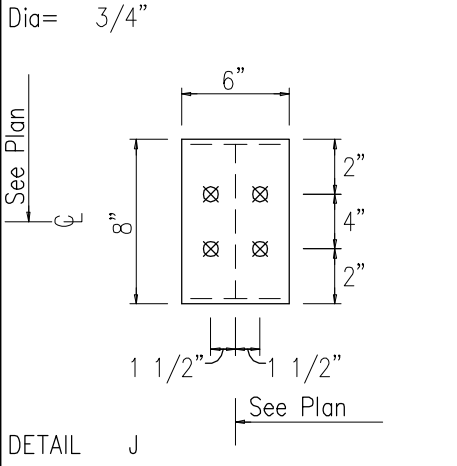
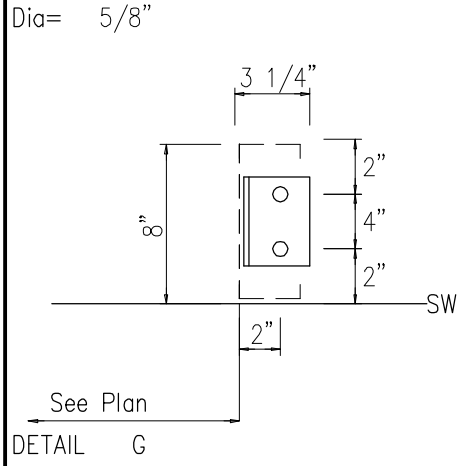
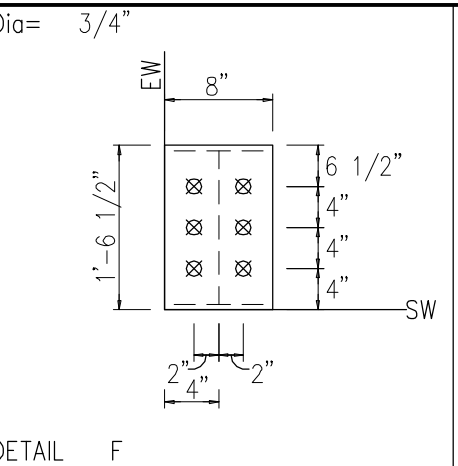
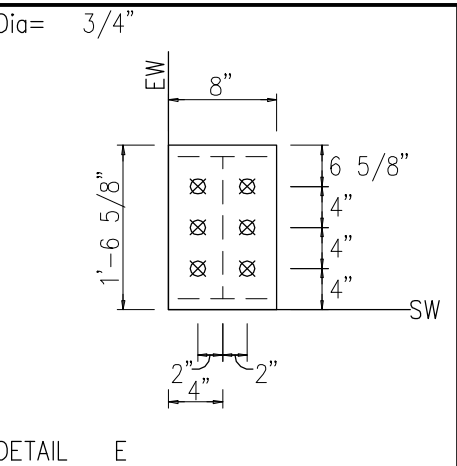
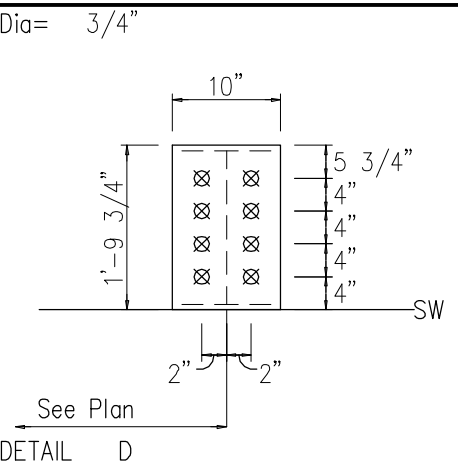
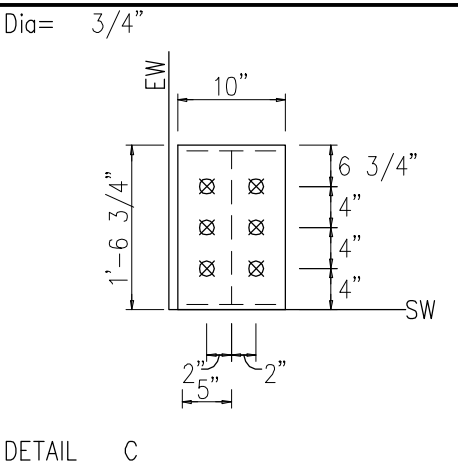
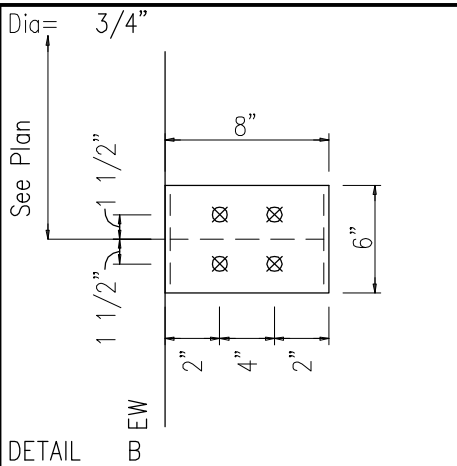
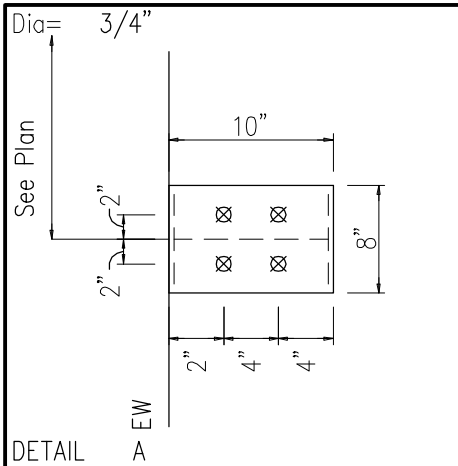
NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

MEZZANINE FRAMING & JOISTS

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



DESCRIPTION	MEZZANINE FRAMING & JOISTS
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.:	56633
ENG. BY:	RA
DATE:	4/15/21
DWG. NO.:	2 OF 21
ISSUE:	P



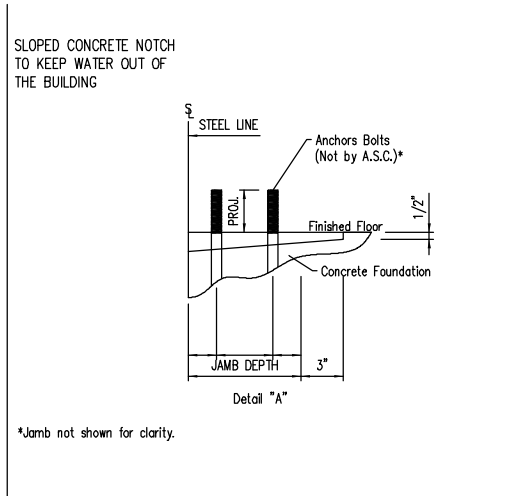
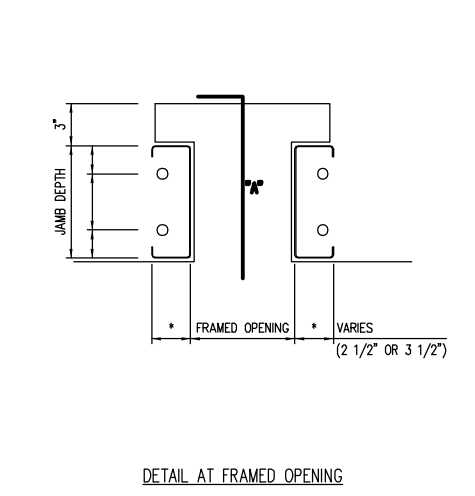
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ANCHOR BOLT DIAMETERS HAVE BEEN DESIGNED BY THE METAL BUILDING MANUFACTURER BASED ON AISC METHOD WITH COMBINED SHEAR AND TENSION.
DEVELOPMENT, EMBEDMENT AND HOOK LENGTH OF ANCHOR BOLTS IN THE CONCRETE ARE DESIGN RESPONSIBILITY OF OTHERS. ALSO DESIGN OF SHEAR ANGLES, TENSION PLATES, HAIRPINS, AND ANY OTHER EMBEDDED MATERIAL IN THE CONCRETE SHALL BE DESIGNED AND PROVIDED BY OTHERS.

NOTE: ANCHOR BOLT PROJECTION IS FROM BOTTOM OF BASE PLATE.

Anchor Bolt Diameter	Projection
1/2"	1 1/2"
5/8"	2"
3/4"	2 1/2"
7/8"	3 1/2"
1"	3 1/2"
1 1/8"	3 1/2"
1 1/4"	3 1/2"

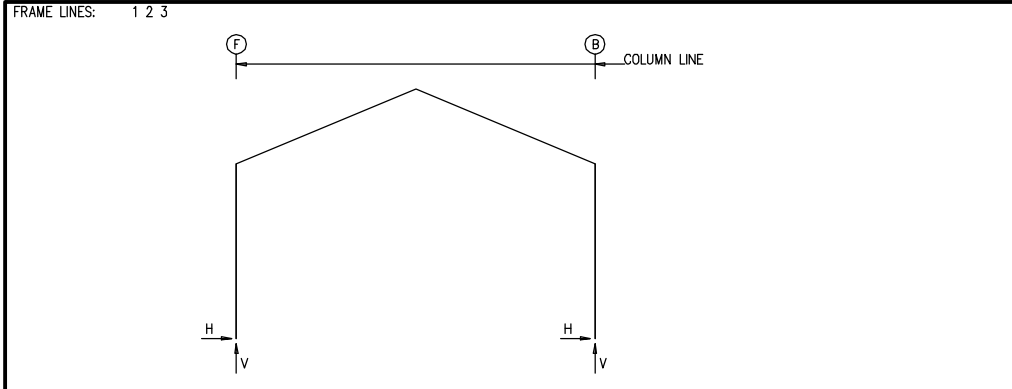
ANCHOR BOLT PROJECTION



ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA

ARMSTRONG STEEL
2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	ANCHOR BOLT DETAILS
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.: 56633	ENG. BY: RA
DATE: 4/15/21	ISSUE: P
DWG. NO.: 3 OF 21	



RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	---Dead---		---Collateral---		---Live---		---Floor---		---Snow---		---Snow_Drift---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	F	0.5	2.1	1.0	3.6	1.3	5.0	0.1	0.1	2.0	8.5	0.0	0.0
1	B	-0.5	6.5	-1.0	4.7	-1.3	5.9	-0.1	4.0	-2.1	10.4	0.0	1.3

Frame Line	Column Line	---Slide_Snow---		---Wind_Left1---		---Wind_Right1---		---Wind_Left2---		---Wind_Right2---		---Wind_Long1---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	F	0.0	0.0	-6.3	-10.2	3.3	-4.5	-6.5	-6.6	3.2	-1.0	1.1	-6.3
1	B	0.0	0.7	-5.3	-4.8	5.1	-11.3	-2.5	-1.5	7.8	-8.0	-2.6	-6.3

Frame Line	Column Line	---Wind_Long2---		---Seismic_Left---		---Seismic_Right---		---Seismic_Long---		F1UNB_SL_L---		F1UNB_SL_R---	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	F	0.0	-6.3	-0.9	-0.8	0.9	0.8	0.1	0.1	1.7	7.0	1.7	4.3
1	B	-3.6	-6.3	-1.2	0.9	1.2	-0.8	0.1	-0.6	-1.7	4.3	-1.7	7.0

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
4	Jamb	5/8"	A307
12	Endwall	3/4"	A307
56	Frame	3/4"	A307
8	WindCol	3/4"	A307
12	Floor	3/4"	A307

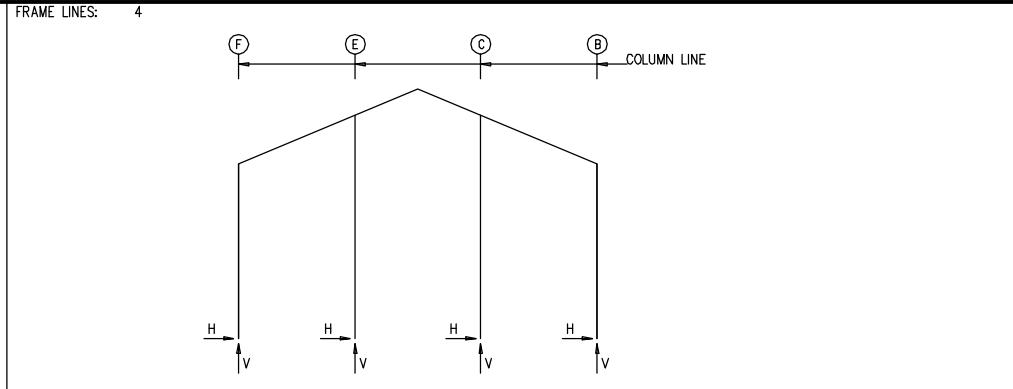
BUILDING BRACING REACTIONS

Wall Loc	Col Line	± Reactions(k)				Panel_Shear (lb/ft)		Note
Line	Line	Wind	Seismic	Wind	Seismic	Wind	Seis	
L_EW	1							(h)
F_SW	B	1.2						(a)
R_EW	4							(h)
B_SW	F	3.2	6.6	5.1	3.1	2.4		

(a) Wind bent in bay
(h) Rigid frame at endwall

WIND BENT REACTIONS

Wall Loc	Col Line	± Reactions(k)				Bolt(in)		Base_Plate(in)		Thick
Line	Line	Wind	Seismic	Wind	Seismic	Qty	Dia	Width	Length	
F_SW	B	3.9	8.3	2.0	4.2	4	0.750	8.000	10.000	0.500
F_SW	B	3.9	8.3	2.0	4.2	4	0.750	8.000	10.000	0.500



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	---Column_Reactions(k)---				Bolt(in) Qty	Dia	---Base_Plate(in)---			Grout (in)	
			Hmax	V	Vmax	Hmin			Width	Length	Thick		
1	F	8	4.5	10.1	13	-3.6	-2.7	6	0.750	10.00	18.75	0.500	0.0
1	B	14	4.4	-0.9	6	-5.5	19.9	6	0.750	10.00	18.75	0.500	0.0
3		3	-3.1	23.0	12	2.8	-2.9						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	---Column_Reactions(k)---				Bolt(in) Qty	Dia	---Base_Plate(in)---			Grout (in)	
			Hmax	V	Vmax	Hmin			Width	Length	Thick		
2*	F	7	7.5	22.9	13	-4.8	-1.1	8	0.750	10.00	21.75	0.500	0.0
2*	B	14	6.3	1.4	6	-9.1	38.3	8	0.750	10.00	21.75	0.500	0.0
3		3	-5.4	43.5	12	3.4	-2.1						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	---Column_Reactions(k)---				Bolt(in) Qty	Dia	---Base_Plate(in)---			Grout (in)	
			Hmax	V	Vmax	Hmin			Width	Length	Thick		
4	F	5	3.0	6.4	11	-3.2	-3.3	6	0.750	8.000	18.50	0.500	0.0
4	B	14	3.5	-2.4	4	-3.6	5.0	6	0.750	8.000	18.63	0.500	0.0
4	E	14	0.1	-2.0	11	-0.1	6.4	4	0.750	6.000	8.000	0.500	0.0
4	C	14	0.1	6.8	11	-0.1	-2.8	4	0.750	6.000	8.000	0.500	0.0

ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Floor Live Vert	Wind Press Horiz	Wind Suct Horiz	Seis Long Horiz	Seis Long Vert	Slide Snow Vert	Wind Left1 Vert	Wind Right1 Vert	Wind Left2 Vert	Wind Right2 Vert	Wind Press Horiz
1	D	4.6	3.9	-8.0	8.9	-1.0	-0.6						
4	C	6.3	3.4	3.4	5.2	5.6	0.0	0.0	-1.6	-4.0	-4.0	-5.1	-4.9
4	E	6.3	2.1	3.4	5.2	5.5	0.0	0.0	-4.1	-5.1	-3.4	-3.2	-4.9

ENDWALL COLUMN: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Load Id	---Column_Reactions(k)---				Bolt(in) Qty	Dia	---Base_Plate(in)---			Grout (in)	
			Hmax	V	Vmax	Hmin			Width	Length	Thick		
1	D	17	5.3	2.8	18	-4.8	2.8	4	0.750	8.000	10.00	0.375	0.0
4	C *	19	3.2	0.5	20	-2.9	0.5						
4	E *	22	3.2	0.5	18	-2.9	0.5						

*See Rigid Frame Interior Column Reactions

NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	= 40.0
Length (ft)	= 60.0
Eave Height (ft)	= 19.5/19.5
Roof Slope (rise/12)	= 5.0/5.0
Dead Load (psf)	= 4.0
Collateral Load (psf)	= 12.0
Live Load (psf)	= 20.0
Snow Load (psf)	= 32.0
Wind Speed (mph) (VULT)	= 130.0
Wind Code	= IBC-15
Exposure	= C
Closed/Open	= C
Importance Wind	= 1.00
Importance Seismic	= 1.00
Seismic Zone	= B
Seismic Coeff (Fa/Ss)	= 0.30

Notes for Reactions Table

ID	Description
1	Dead+Collateral+Floor_Live
2	Dead+Collateral+Snow
3	Dead+Collateral+0.75Snow_Drift+0.75Floor_Live
4	Dead+0.6Wind_Left1
5	Dead+0.6Wind_Right1
6	Dead+Collateral+0.75Snow+0.45Wind_Left1+0.75Floor_Live
7	Dead+Collateral+0.75Snow+0.45Wind_Right1+0.75Floor_Live
8	Dead+Collateral+0.75Snow+0.45Wind_Right1+0.75Slide_Snow+0.75Floor_Live
9	Dead+Collateral+0.75Snow+0.45Wind_Left1+0.75Snow_Drift+0.75Floor_Live
10	Dead+Collateral+0.75Snow+0.45Wind_Right1+0.75Floor_Live
11	0.6Dead+0.6Wind_Left1
12	0.6Dead+0.6Wind_Right1
13	0.6Dead+0.6Wind_Left2
14	0.6Dead+0.6Wind_Right2
15	0.6Dead+0.6Wind_LongL
16	1.02Dead+1.02Collateral+0.52Seismic_Left+0.75Floor_Live+0.75F3UNB_SL_L
17	0.6Dead+0.6Wind_Right2+0.6Wind_Suction
18	0.6Dead+0.6Wind_Pressure+0.6Wind_Long2L
19	0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
20	0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L
21	1.02Dead+1.02Collateral+0.52Seismic_LongR+0.75Floor_Live+0.75E2UNB_SL_L
22	0.6Dead+0.6Wind_Suction+0.6Wind_Long2L
23	1.02Dead+1.02Collateral+0.52Seismic_LongR+0.75Floor_Live+0.75E2UNB_SL_R

FLOOR COLUMN REACTIONS

Frame Line	Col Line	Max_Vert Ld (k)	Dead Vert (k)	Coll Vert (k)	Live Vert (k)	Anc_Bolt Qty	Dia	Base Plate (in) Width	Length	Thick	Grout (in)	
3	①3.7	1	9.8	5.0	0.0	4	0.750	6.000	8.000	0.500	0.0	
3	②5.4	1	10.2	5.2	0.0	4	0.750	6.000	8.000	0.500	0.0	
2	D	1	16.0	8.1	0.0	7.9	4	0.750	6.000	8.000	0.500	0.0
3	D	1	8.9	4.5	0.0	4.4	4	0.750	6.000	8.000	0.500	0.0

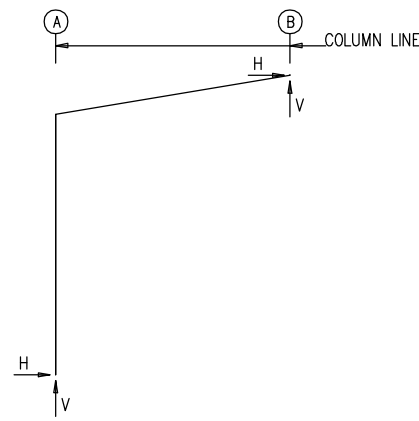


56633A

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA

DESCRIPTION	ANCHOR BOLT REACTIONS
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO: 56633	ENG. BY: RA DATE: 4/15/21
DWG. NO: 4 OF 21	ISSUE: P

2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com



RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Snow Horiz	Snow Vert	Snow_Drift Horiz	Snow_Drift Vert	Slide_Snow Horiz	Slide_Snow Vert
4	A	0.0	0.4	0.0	0.1	0.0	1.3	-0.1	3.2	0.0	0.7	0.0	0.7
4	B	0.0	0.2	0.0	0.1	0.0	0.9	0.1	1.8	0.0	1.2	0.0	0.7
4	A	0.0	-3.1	1.3	-2.2	-1.4	-1.1	-0.1	-0.1	1.3	-2.2	1.3	-1.7
4	B	0.8	-1.9	2.3	-1.1	-1.8	-0.8	-0.3	0.0	2.5	-1.3	2.3	-0.9
4	A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
4	B	-0.1	0.0	0.1	0.0	0.0	0.9	0.0	0.0	0.0	0.0	0.0	0.0
3*	A	0.0	0.6	0.0	0.8	0.0	2.3	-0.2	5.9	0.1	1.4	0.1	1.6
3*	B	0.0	0.4	0.0	0.6	0.0	1.6	0.2	3.2	-0.1	2.4	-0.1	1.4
3*	A	0.3	-4.9	2.1	-3.5	-2.3	-1.2	-0.5	0.2	2.3	-4.0	2.3	-3.2
3*	B	1.6	-2.9	3.9	-1.8	-3.1	-0.9	-0.9	0.2	4.3	-2.4	4.5	-1.6
3*	A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.1	1.7	0.0	0.0
3*	B	-0.2	0.0	0.2	0.0	-0.1	0.0	-0.1	0.0	-0.1	0.0	0.0	0.0
1	A	0.0	0.4	0.0	0.1	0.0	1.3	-0.1	3.2	0.0	0.7	0.0	0.7
1	B	0.0	0.2	0.0	0.1	0.0	0.9	0.1	1.8	0.0	1.2	0.0	0.7
1	A	0.0	-3.1	1.3	-2.2	-1.4	-1.1	-0.1	-0.1	1.3	-2.2	1.3	-1.7
1	B	0.8	-1.9	2.3	-1.1	-1.8	-0.8	-0.3	0.0	2.2	-1.3	2.6	-0.9
1	A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1	B	-0.1	0.0	0.1	0.0	-0.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)				Hmin	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id					Width	Length	Thick	
4	A	6	0.8	-0.8	4	-0.8	-0.4	4	0.750	6.000	10.50	0.500	0.0
		2	-0.1	4.5	3	0.0	-1.6						
4	B	5	1.5	-0.6	4	-1.1	-0.4	0	0.000	0.000	0.000	0.000	0.0
		1	0.1	3.2	3	0.5	-1.0						

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)				Hmin	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id					Width	Length	Thick	
3*	A	6	1.4	-1.5	4	-1.4	-0.4	4	0.750	6.000	10.50	0.500	0.0
		2	-0.1	8.8	3	0.2	-2.6						
3*	B	6	2.7	-0.7	4	-1.9	-0.3	0	0.000	0.000	0.000	0.000	0.0
		1	0.1	6.7	3	1.0	-1.5						

3* Frame lines: 3 2

RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)				Hmin	V Vmin	Bolt(in) Qty	Dia	Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id					Width	Length	Thick	
1	A	6	0.8	-0.8	4	-0.8	-0.4	4	0.750	6.000	10.50	0.500	0.0
		2	-0.1	4.5	3	0.0	-1.6						
1	B	6	1.5	-0.4	4	-1.1	-0.4	0	0.000	0.000	0.000	0.000	0.0
		1	0.1	3.2	3	0.5	-1.0						

NOTES FOR REACTIONS

- Building reactions are based on the following building data:
- Width (ft) = 8.2
 - Length (ft) = 60.0
 - Eave Height (ft) = 9.1/10.4
 - Roof Slope (rise/12) = 2.0
 - Dead Load (psf) = 4.3
 - Collateral Load (psf) = 2.0
 - Live Load (psf) = 20.0
 - Snow Load (psf) = 40.0
 - Wind Speed (mph) (VULT) = 130.0
 - Wind Code = IBC-15
 - Exposure = C
 - Closed/Open = P
 - Importance Wind = 1.00
 - Importance Seismic = 1.00
 - Seismic Zone = B
 - Seismic Coeff (Fa*Ss) = 0.30

ID	Description
1	Dead+Collateral+Snow+Snow_Drift
2	Dead+Collateral+Snow+Slide_Snow
3	0.6Dead+0.6Wind_Left1
4	0.6Dead+0.6Wind_Left2
5	0.6Dead+0.6Wind_Long1R
6	0.6Dead+0.6Wind_Long2R

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
16	Frame	3/4"	A307

BUILDING BRACING REACTIONS

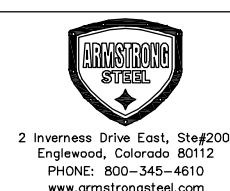
Wall Loc	Col Line	± Reactions(k)				Panel_Shear (lb/ft)		Note
		Wind Horiz	Wind Vert	Seismic Horiz	Seismic Vert	Wind	Seis	
L_EW	4							(h)
F_SW	B							(e)
R_EW	1							(h)
B_SW	A	Torsional Bracing Used						

(e)Bracing loads must be applied to supporting building
 (h)Rigid frame at endwall

56633B



ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



DESCRIPTION	ANCHOR BOLT REACTIONS
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.: 56633	ENG. BY: RA DATE: 4/15/21
	DWG. NO.: 5 OF 21 ISSUE: P

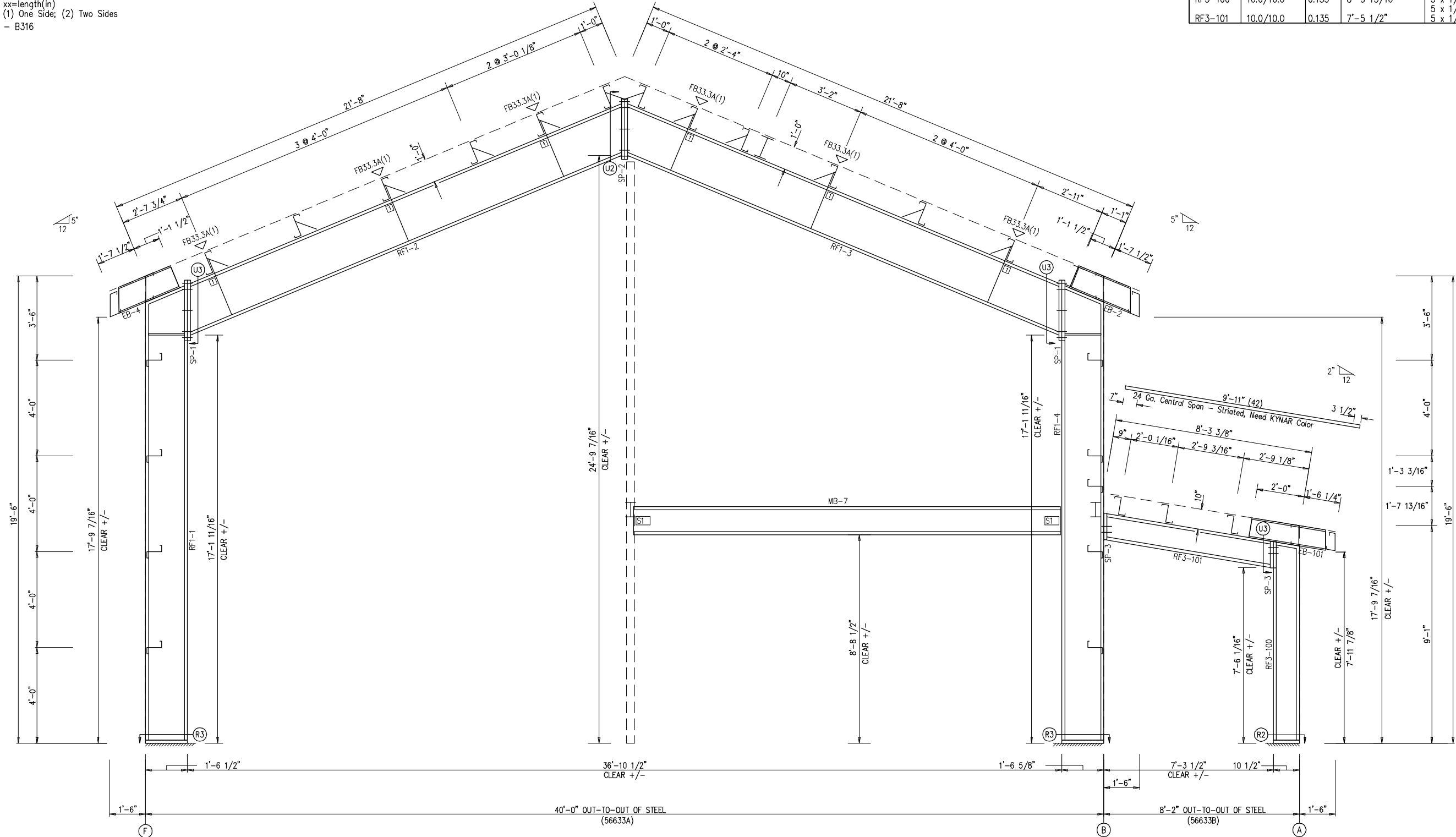
SPlice Bolt Table						
Mark	Qty	Top	Bot	Int	Type	Length
SP-1	4	4	2		A325	2.00
SP-2	4	4	2		A325	1.75
SP-3	4	0	0		A325	1.75

SUPPORT BEAM Bolt Table				
ID	Qty	Type	Dia	Length
S1	6	A325	0.625	1.50

FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 A - B316

Mark	Web Depth		Web Plate		Outside Flange	Inside Flange
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length
RF1-1	18.0	18.0	0.135	16'-6 1/4"	10 x 3/8" x 18'-4 1/8"	10 x 3/8" x 16'-9 1/4"
RF1-2	18.0	18.0	0.150	2'-5 1/2"	10 x 3/8" x 1'-7 3/4"	6 x 1/4" x 19'-10 1/2"
	21.0	21.0	0.150	10'-8 3/8"	6 x 1/4" x 19'-10 1/2"	
RF1-3	21.0	21.0	0.135	9'-11"	6 x 1/4" x 19'-10 1/2"	6 x 1/4" x 19'-10 1/2"
	21.0	21.0	0.150	10'-8 3/8"		
RF1-4	18.0	18.0	0.150	2'-5 1/2"	10 x 3/8" x 1'-7 3/4"	10 x 3/8" x 16'-9 1/4"
	18.0	18.0	0.135	16'-6 1/4"	10 x 3/8" x 18'-4 1/8"	
EB-2						
EB-4						
RF3-100	10.0	10.0	0.135	8'-3 13/16"	5 x 1/4" x 8'-2 1/8"	5 x 1/4" x 7'-5 7/16"
RF3-101	10.0	10.0	0.135	7'-5 1/2"	5 x 1/4" x 10 3/8"	5 x 1/4" x 7'-3 3/4"
					5 x 1/4" x 7'-3 3/4"	

CONNECTION PLATES		
ID	Mark/Part	
1	BC-46	

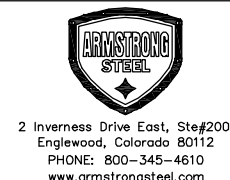


RIGID FRAME ELEVATION: FRAME LINE 1



NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



DESCRIPTION	RIGID FRAME ELEVATION
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.:	56633
ENG. BY:	RA
DATE:	4/15/21
DWG. NO.:	6 OF 21
ISSUE:	P

SPLICE BOLT TABLE						
Mark	Qty	Top	Bot	Int	Type	Length
SP-1	4	4	2		A325	0.750 2.25
SP-2	4	4	2		A325	0.625 1.75
SP-3	4	0	0		A325	0.625 1.75

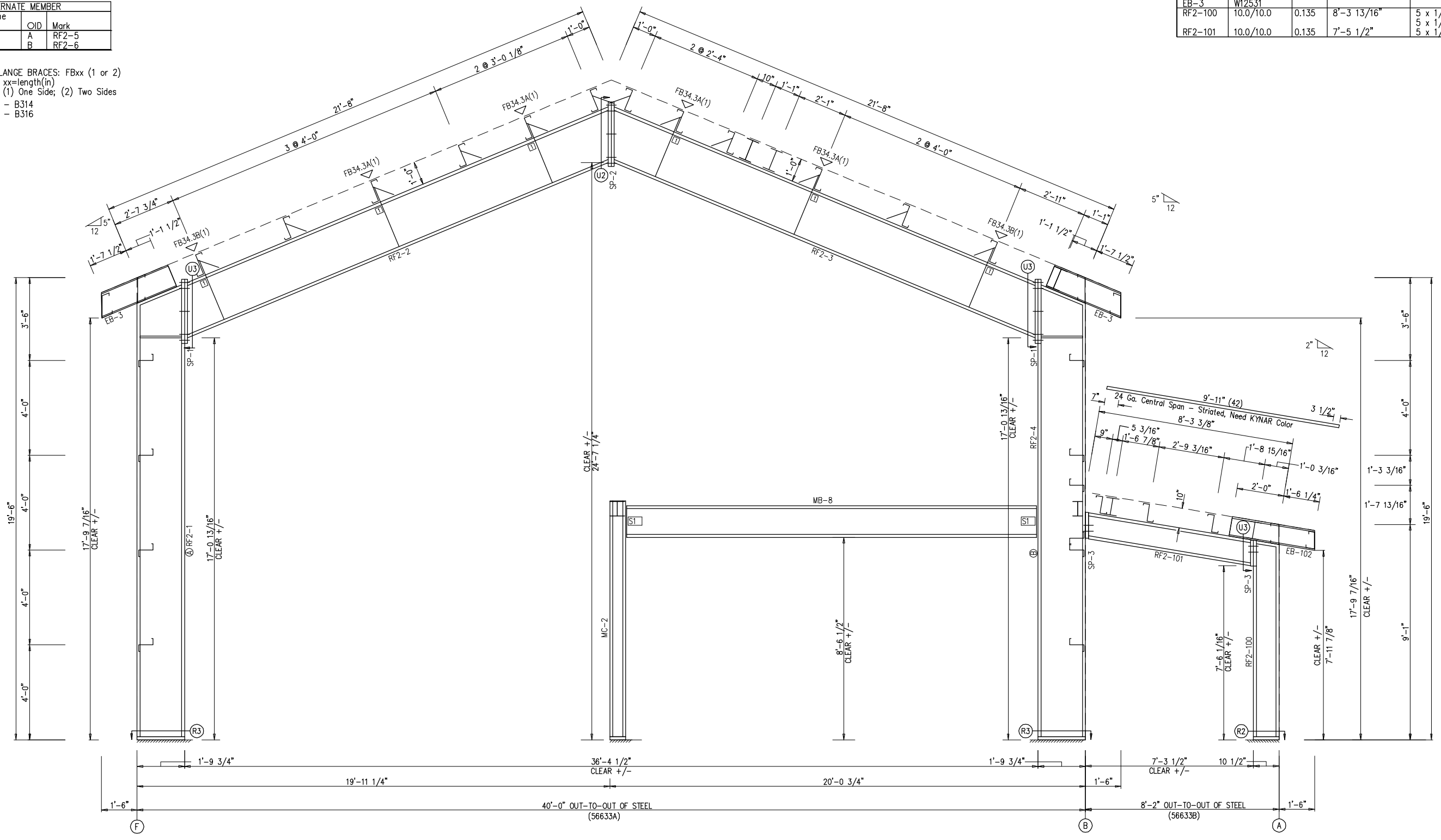
SUPPORT BEAM BOLT TABLE				
ID	Qty	Type	Dia	Length
S1	6	A325	0.625	1.50

ALTERNATE MEMBER		
Frame Line	OID	Mark
3	A	RF2-5
	B	RF2-6

FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 B - B314
 A - B316

MEMBER TABLE						
Mark	Web Depth		Web Plate		Outside Flange	Inside Flange
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length
RF2-1	21.0/21.0	0.135	16'-5 3/16"		10 x 3/8" x 18'-4 1/8"	10 x 3/8" x 16'-8 3/16"
RF2-2	21.0/21.0	0.188	2'-7 13/16"		10 x 3/8" x 1'-11"	
	23.0/23.0	0.188	10'-5 11/16"		8 x 1/4" x 19'-7"	8 x 5/16" x 10'-11 11/16"
	23.0/23.0	0.135	9'-11"			8 x 1/4" x 8'-7 5/16"
RF2-3	23.0/23.0	0.135	9'-11"		8 x 1/4" x 19'-7"	8 x 5/16" x 10'-11 11/16"
	23.0/23.0	0.188	10'-5 11/16"			8 x 1/4" x 8'-7 5/16"
RF2-4	21.0/21.0	0.188	2'-7 13/16"		10 x 3/8" x 1'-11"	10 x 3/8" x 16'-8 3/16"
	21.0/21.0	0.135	16'-5 3/16"		10 x 3/8" x 18'-4 1/8"	
EB-3	W12531					
RF2-100	10.0/10.0	0.135	8'-3 13/16"		5 x 1/4" x 8'-2 1/8"	5 x 1/4" x 7'-5 7/16"
RF2-101	10.0/10.0	0.135	7'-5 1/2"		5 x 1/4" x 10 3/8"	5 x 1/4" x 7'-3 3/4"

CONNECTION PLATES		
ID	Mark/Part	
2	BC-46	



RIGID FRAME ELEVATION: FRAME LINE 2



NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



2 Inverness Drive East, Ste#200
 Englewood, Colorado 80112
 PHONE: 800-345-4610
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DESCRIPTION	RIGID FRAME ELEVATION
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO:	56633
ENG. BY:	RA
DATE:	4/15/21
DWG. NO.:	7 OF 21
ISSUE:	P

SPLICE BOLT TABLE						
Mark	Qty	Top	Bot	Int	Type	Length
SP-1	4	4	2		A325	0.750 2.25
SP-2	4	4	2		A325	0.625 1.75
SP-3	4	0	0		A325	0.625 1.75

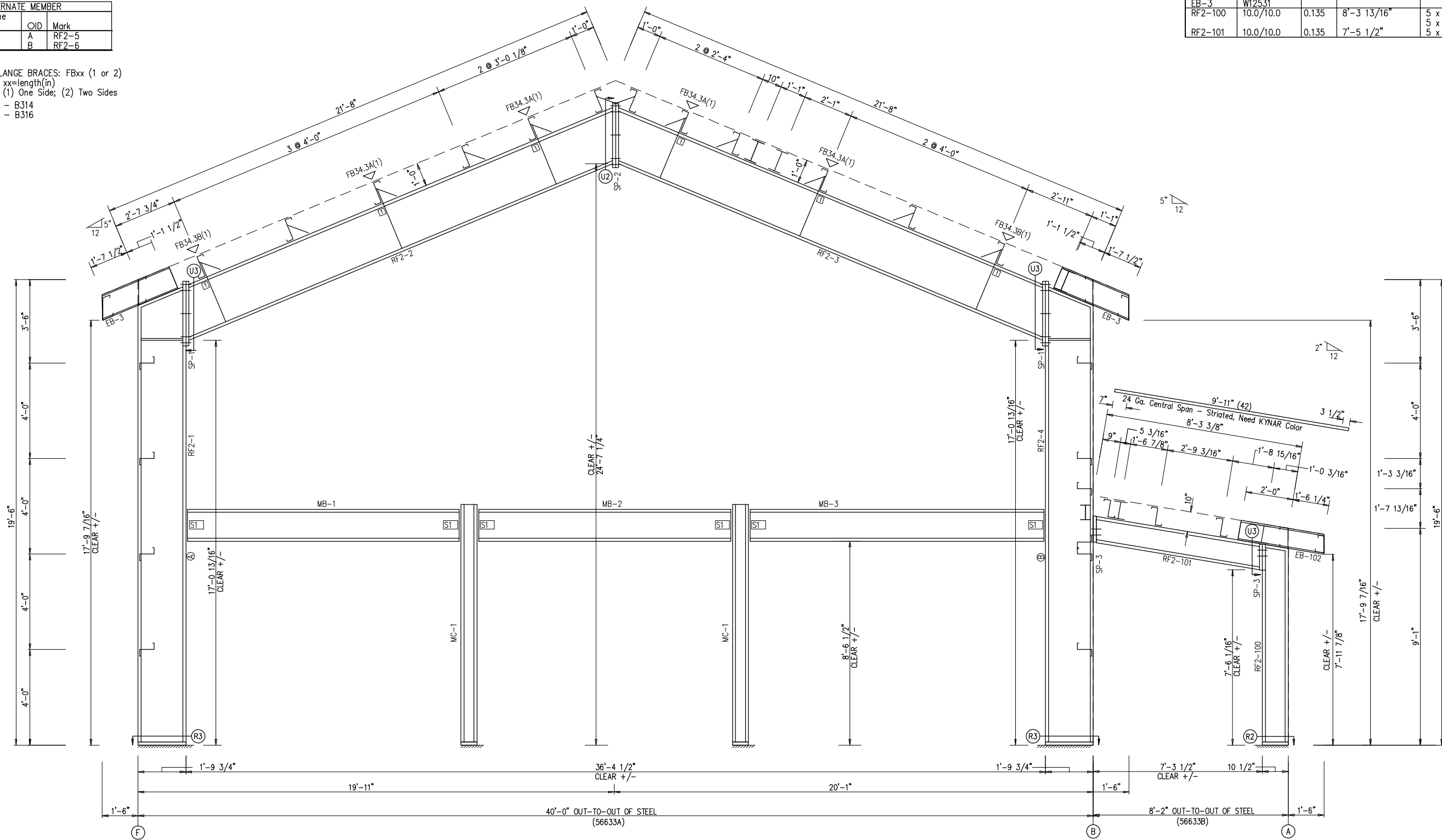
SUPPORT BEAM BOLT TABLE				
ID	Qty	Type	Dia	Length
S1	6	A325	0.625	1.50

ALTERNATE MEMBER		
Frame Line	OID	Mark
3	A	RF2-5
	B	RF2-6

FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 B - B314
 A - B316

MEMBER TABLE						
Mark	Web Depth		Web Plate		Outside Flange	Inside Flange
	Start	End	Thick	Length	W x Thk x Length	W x Thk x Length
RF2-1	21.0/21.0	0.135	16'-5 3/16"		10 x 3/8" x 18'-4 1/8"	10 x 3/8" x 16'-8 3/16"
RF2-2	21.0/21.0	0.188	2'-7 13/16"		10 x 3/8" x 1'-11"	
	23.0/23.0	0.188	10'-5 11/16"		8 x 1/4" x 19'-7"	8 x 5/16" x 10'-11 11/16"
RF2-3	23.0/23.0	0.135	9'-11"			8 x 1/4" x 8'-7 5/16"
	23.0/23.0	0.188	10'-5 11/16"		8 x 1/4" x 19'-7"	8 x 5/16" x 10'-11 11/16"
RF2-4	21.0/21.0	0.188	2'-7 13/16"		10 x 3/8" x 1'-11"	10 x 3/8" x 16'-8 3/16"
EB-3	W12531		16'-5 3/16"			
RF2-100	10.0/10.0	0.135	8'-3 13/16"		5 x 1/4" x 8'-2 1/8"	5 x 1/4" x 7'-5 7/16"
RF2-101	10.0/10.0	0.135	7'-5 1/2"		5 x 1/4" x 10 3/8"	5 x 1/4" x 7'-3 3/4"

CONNECTION PLATES	
ID	Mark/Part
2	BC-46

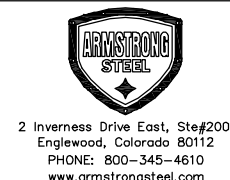


RIGID FRAME ELEVATION: FRAME LINE 2



NOTE:
 MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



DESCRIPTION	RIGID FRAME ELEVATION
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO. 56633	ENG. BY: RA DATE: 4/15/21
	DWG. NO.: 8 OF 21 ISSUE: P

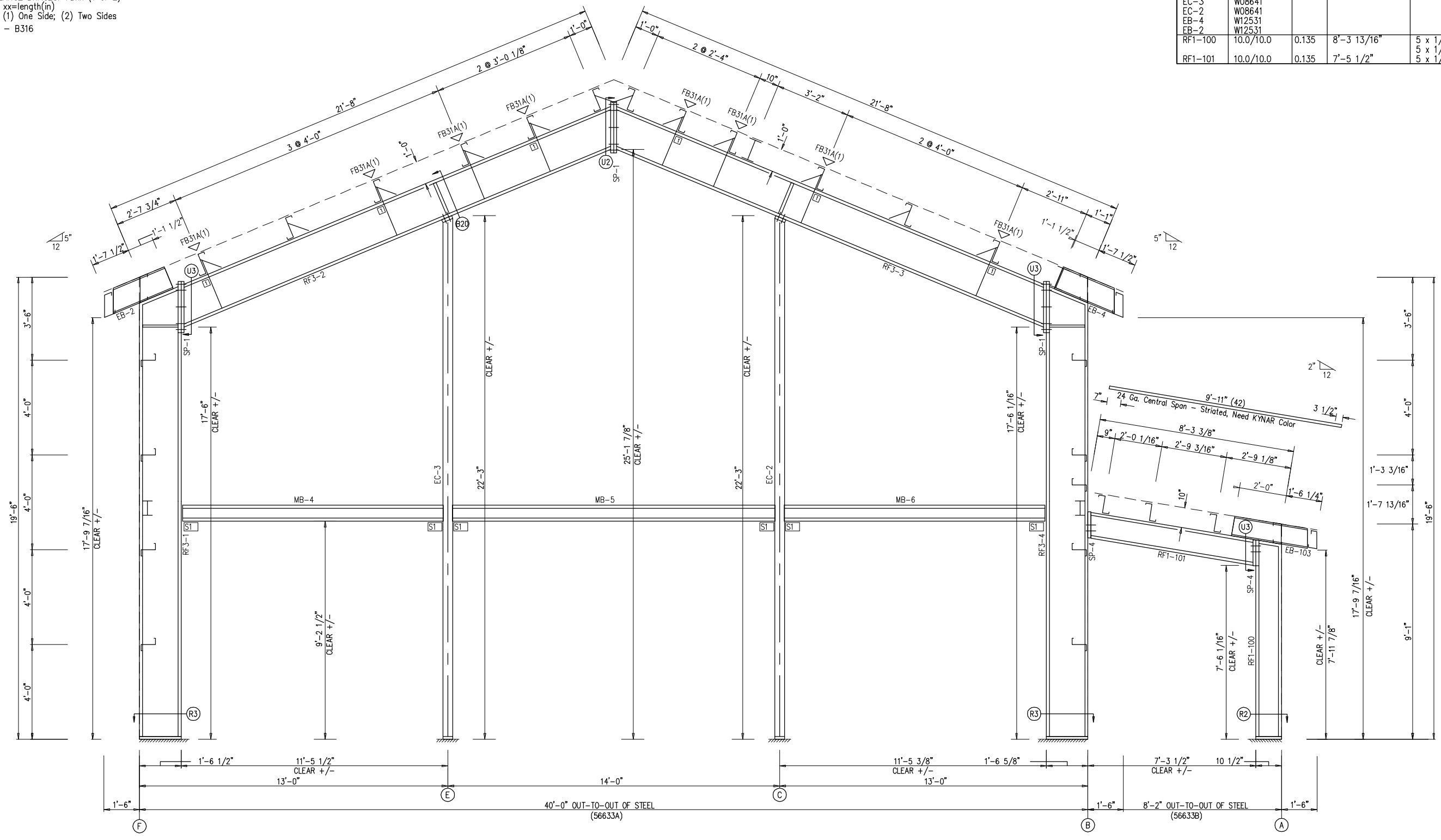
SPlice Bolt Table						CAP PLATE BOLTS						
Mark	Qty	Top	Bot	Int	Type	Dia	Length	Mark	Qty	Type	Dia	Length
SP-1	4	4	2		A325	0.625	1.75	EC-3	4	A325	0.625	1.50
SP-4	4	0	0		A325	0.625	1.75	EC-2	4	A325	0.625	1.50

SUPPORT BEAM BOLT TABLE				
ID	Qty	Type	Dia	Length
S1	6	A325	0.625	1.50

FLANGE BRACES: FBxx (1 or 2)
 xx=length(in)
 (1) One Side; (2) Two Sides
 A - B316

MEMBER TABLE	Web Depth		Web Plate		Outside Flange		Inside Flange	
	Start	End	Thick	Length	W x Thk	Length	W x Thk	Length
RF3-1	18.0	18.0	0.135	18'-11 13/16"	8 x 1/4"	18'-4 1/4"	8 x 1/4"	17'-1 1/2"
RF3-2	17.0	17.0	0.135	18'-6 1/8"	6 x 1/4"	19'-10 7/8"	6 x 1/4"	19'-10 7/8"
RF3-3	17.0	17.0	0.135	2'-0"	6 x 1/4"	19'-10 3/4"	6 x 1/4"	19'-10 3/4"
RF3-4	17.0	17.0	0.135	18'-5 15/16"	8 x 1/4"	17'-13 1/16"	8 x 1/4"	17'-13 1/16"
EC-3	W08641				8 x 1/4"	8'-2 1/8"	5 x 1/4"	7'-5 7/16"
EC-2	W08641				5 x 1/4"	10 3/8"	5 x 1/4"	10 3/8"
EB-4	W12531				8 x 1/4"	8'-7 15/16"	5 x 1/4"	7'-3 3/4"
EB-2	W12531				8 x 1/4"	8'-7 15/16"	5 x 1/4"	7'-3 3/4"
RF1-100	10.0	10.0	0.135	8'-3 13/16"	5 x 1/4"	7'-5 7/16"	5 x 1/4"	7'-5 7/16"
RF1-101	10.0	10.0	0.135	7'-5 1/2"	5 x 1/4"	7'-3 3/4"	5 x 1/4"	7'-3 3/4"

CONNECTION PLATES		
ID	Mark/Part	
2	BC-46	



RIGID FRAME ELEVATION: FRAME LINE 4



NOTE:
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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA

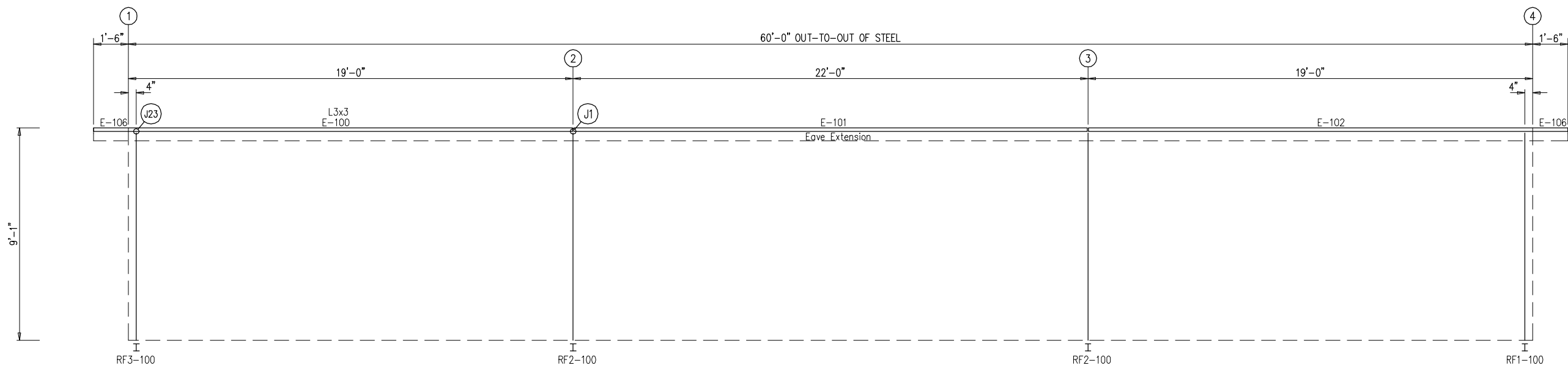


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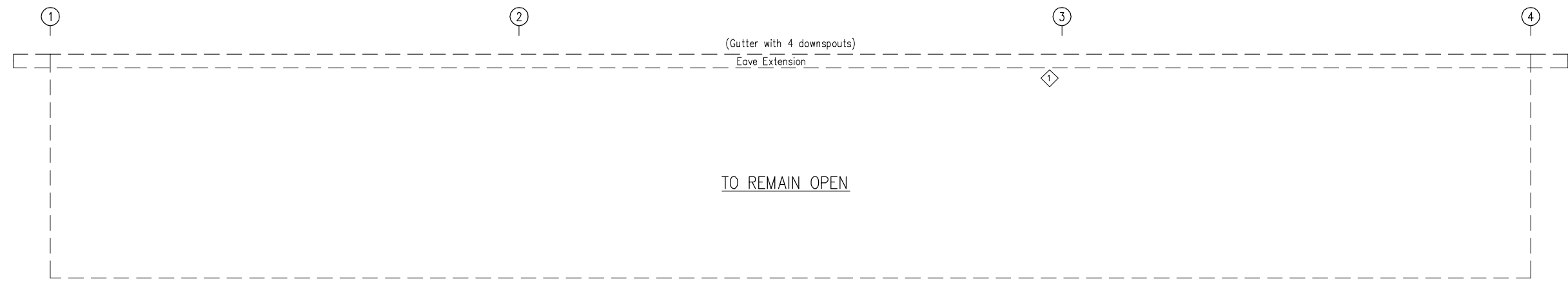
DESCRIPTION	RIGID FRAME ELEVATION
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO:	56633
ENG. BY:	RA
DATE:	4/15/21
DWG. NO.:	9 OF 21
ISSUE:	P

TRIM TABLE				
FRAME LINE A				
ID	QUAN	PART	LENGTH	DETAIL
1	3	AR192204	20'-4"	TRIM_324

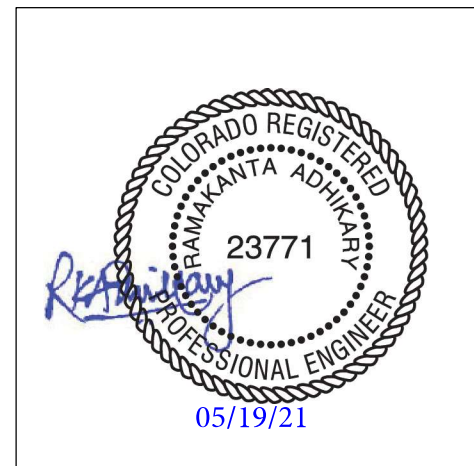
MEMBER TABLE				
FRAME LINE A				
QUAN	MARK	PART	LENGTH	
1	E-100	10354DU2	18'-0"	
1	E-101	10362DU2	21'-4"	
1	E-102	10354DU2	18'-0"	
2	E-106	10354DU2	1'-5 1/2"	



SIDEWALL FRAMING: FRAME LINE A

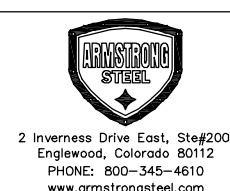


SIDEWALL TRIM: FRAME LINE A

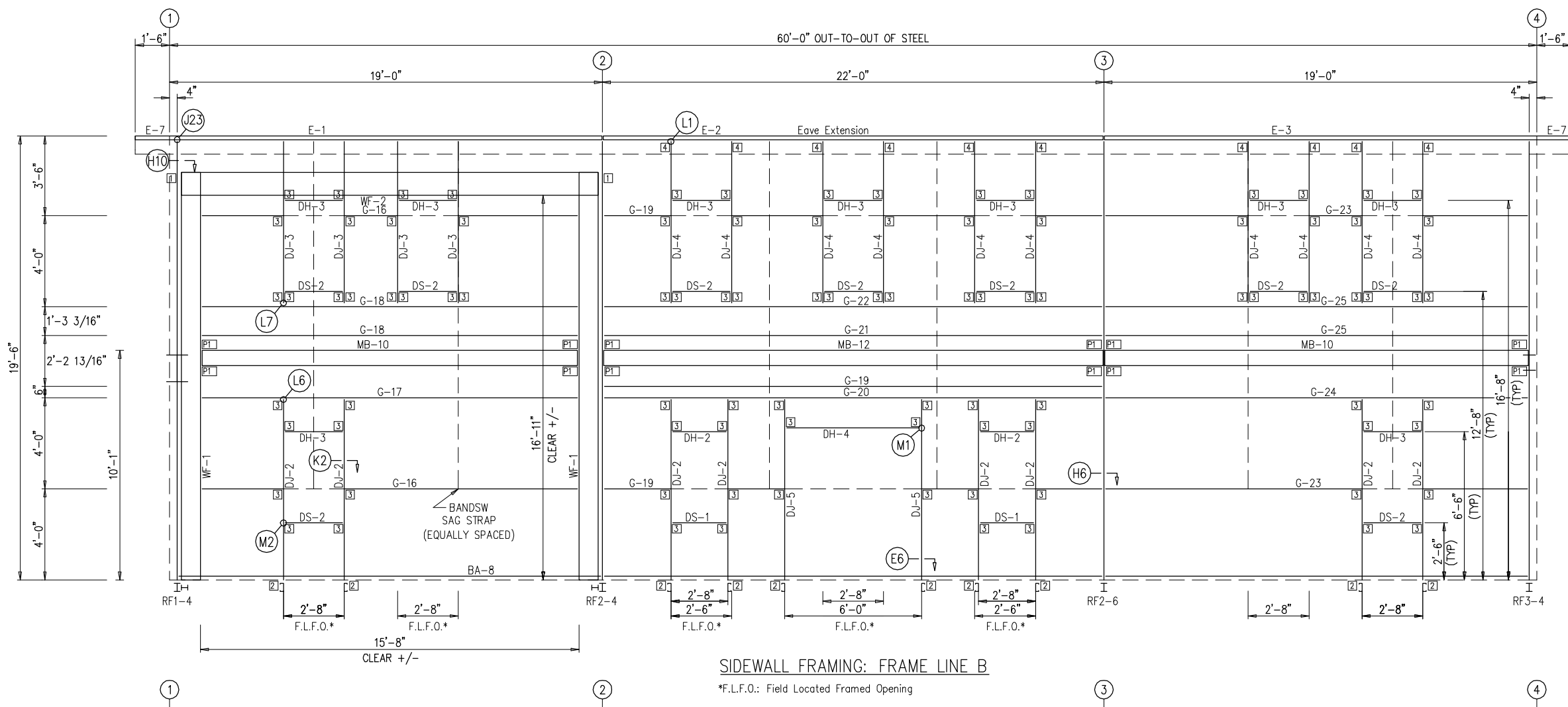


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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



DESCRIPTION	ENDWALL FRAMING & TRIM
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.: 56633	ENG. BY: RA DATE: 4/15/21
	DWG. NO.: 10 OF 21 ISSUE: P

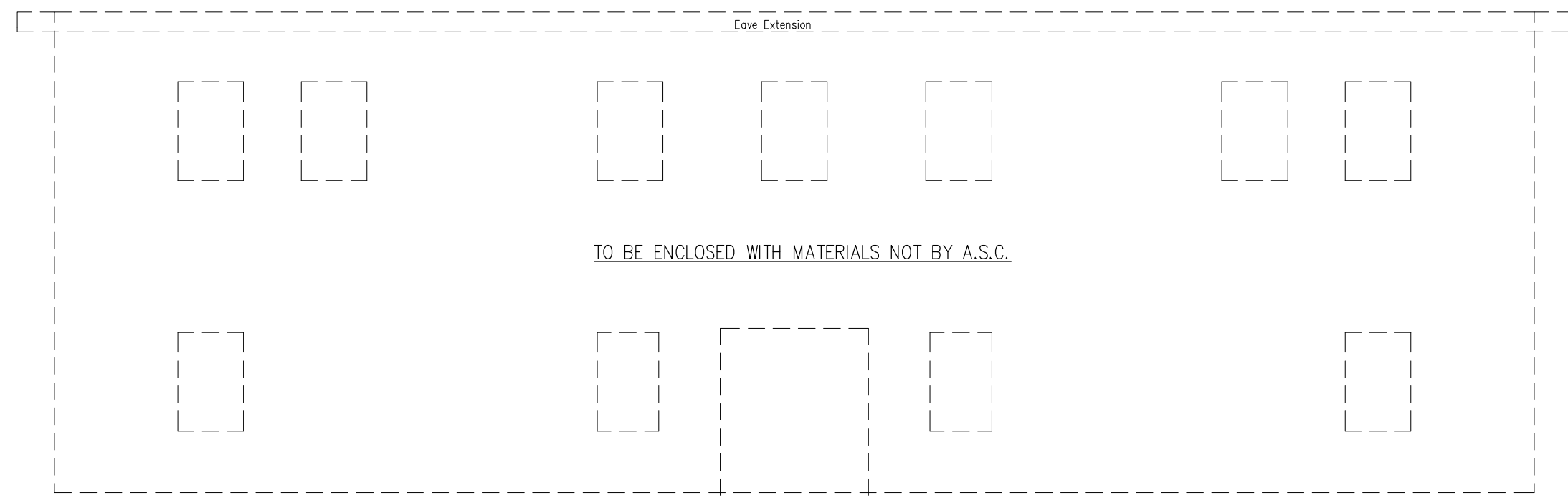


BOLT TABLE				
FRAME LINE B				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	1"	2 1/2"
WF-1 - RF1-4	8	A325	5/8"	1 1/2"
WF-1 - RF2-4	8	A325	5/8"	1 1/2"

MEMBER TABLE				
FRAME LINE B				
QUAN	MARK	PART	LENGTH	
2	WF-1	W10841	18'-1"	
1	WF-2	W12641	15'-7 1/4"	
8	DJ-2	8X25C16	7'-8"	
4	DJ-3	8X25C16	4'-8 1/2"	
10	DJ-4	8X25C16	6'-0 3/4"	
2	DJ-5	8X25C16	7'-8"	
2	DH-2	8X25C16	2'-5"	
9	DH-3	8X25C16	2'-7"	
1	DH-4	8X25C16	5'-11"	
2	DS-1	8X25C16	2'-5"	
9	DS-2	8X25C16	2'-7"	
1	E-1	12566DU5	18'-0"	
1	E-2	12566DU5	21'-4"	
1	E-3	12566DU5	18'-0"	
2	E-7	12566DU5	1'-5 1/2"	
2	G-16	8X25Z16	16'-8"	
1	G-17	8X25Z14	15'-7"	
2	G-18	8X35Z16	15'-7"	
3	G-19	8X25Z16	21'-1"	
1	G-20	8X35Z12	21'-1"	
1	G-21	8X35Z16	21'-1"	
1	G-22	8X25Z12	21'-1"	
2	G-23	8X25Z16	17'-10"	
1	G-24	8X25Z14	17'-10"	
2	G-25	8X35Z16	17'-10"	
2	MB-10	W08531	18'-7"	
1	MB-12	W08531	21'-11"	

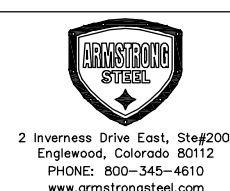
PERIMETER BEAM BOLT TABLE					
FRAME LINE B					
ID	QUAN	TYPE	DIA	LENGTH	QUAN
P1	6	A325	5/8"	1 1/2"	12

CONNECTION PLATES			
FRAME LINE B			
ID	QUAN	MARK/PART	
1	2	BC-49	
2	10	BC-05	
3	94	BC-01	
4	10	BC-37F	



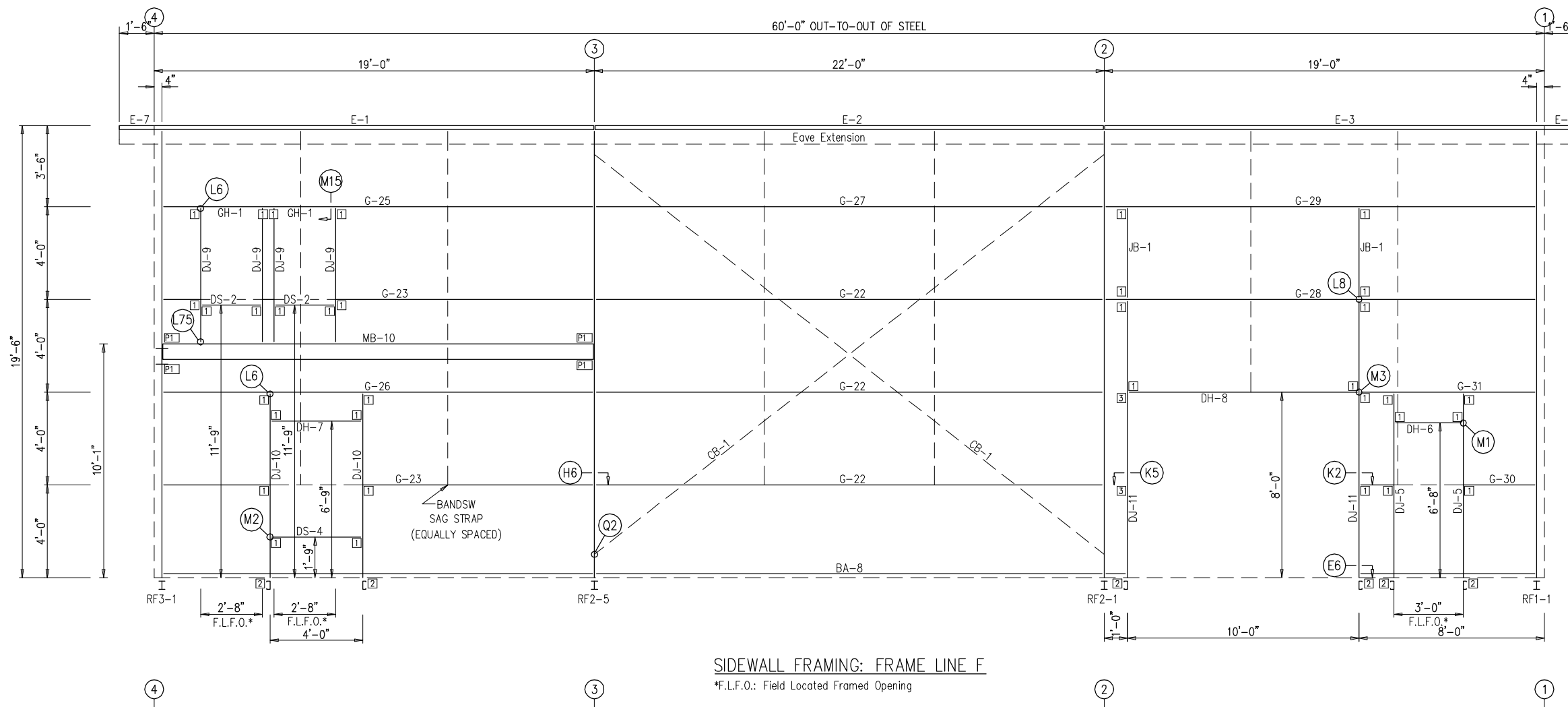
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ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



05/19/21

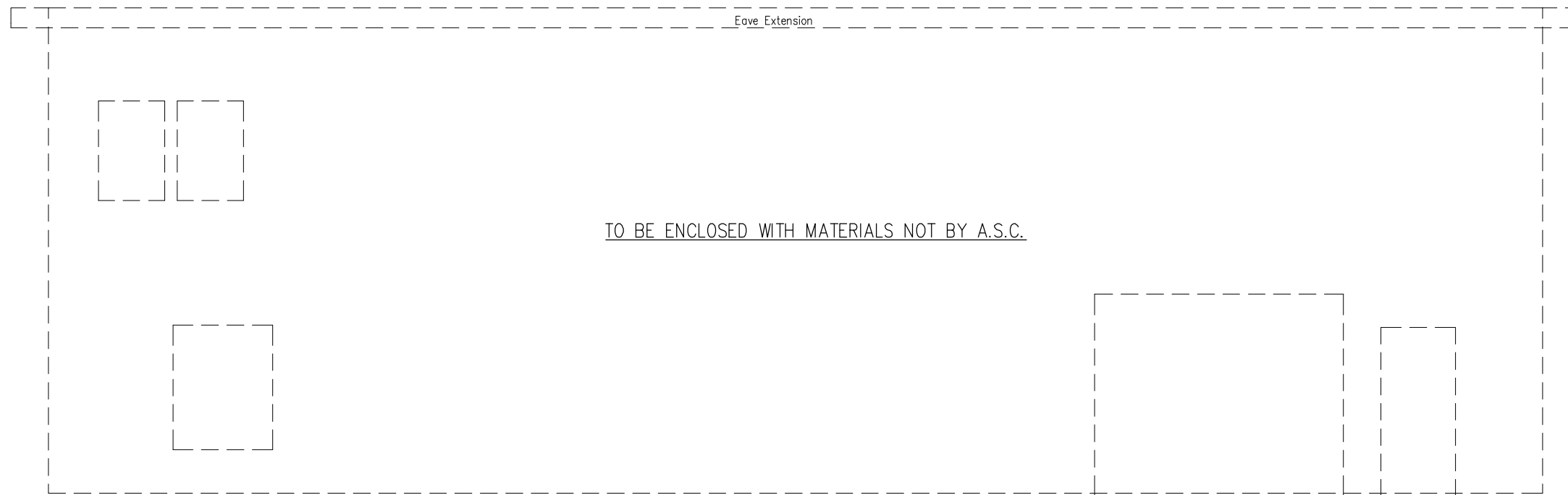
DESCRIPTION	ENDWALL FRAMING & TRIM
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.: 56633	ENG. BY: RA DATE: 4/15/21
	DWG. NO.: 11 OF 21 ISSUE: P



MEMBER TABLE			
QUAN	MARK	PART	LENGTH
2	DJ-5	8X25C16	7'-8"
4	DJ-9	8X25C16	5'-6 1/2"
2	DJ-10	8X25C16	7'-8"
2	DJ-11	8X25C16	11'-8"
1	DH-6	8X25C16	2'-11"
1	DH-7	8X25C16	3'-11"
1	DH-8	8X25C16	9'-11"
2	DS-2	8X25C16	2'-7"
1	DS-4	8X25C16	3'-11"
1	E-1	12566DU5	18'-0"
1	E-2	12566DU5	21'-4"
1	E-3	12566DU5	18'-0"
2	E-7	12566DU5	1'-5 1/2"
3	G-22	8X25Z12	21'-1"
2	G-23	8X25Z16	17'-10"
1	G-25	8X35Z16	17'-10"
1	G-26	8X25Z12	17'-10"
1	G-27	8X35Z14	21'-1"
1	G-28	8X35Z12	17'-9"
1	G-29	8X25Z16	17'-9"
1	G-30	8X25Z16	7'-0"
1	G-31	8X25Z16	6'-10 1/2"
2	CB-1	GS1720	28'-3 3/4"
1	MB-10	W08531	18'-7"
2	JB-1	8X25C16	3'-4"

PERIMETER BEAM BOLT TABLE					
CID	QUAN	TYPE	DIA	LENGTH	QUAN
P1	6	A325	5/8"	1 1/2"	4

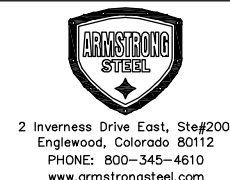
CONNECTION PLATES		
CID	QUAN	MARK/PART
1	38	BC-01
2	6	BC-05
3	2	b4



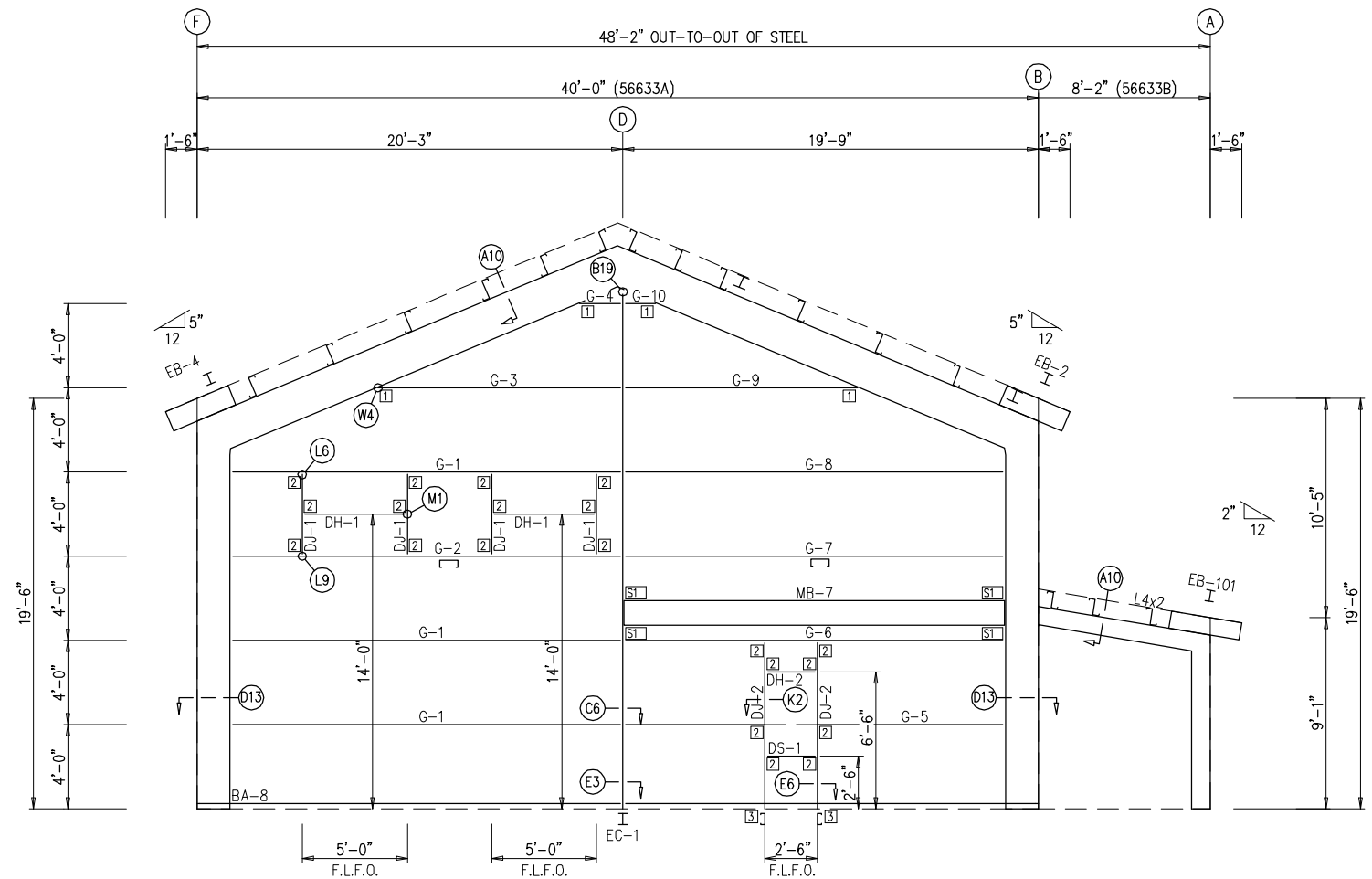
NOTE:
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SIDEWALL: FRAME LINE F

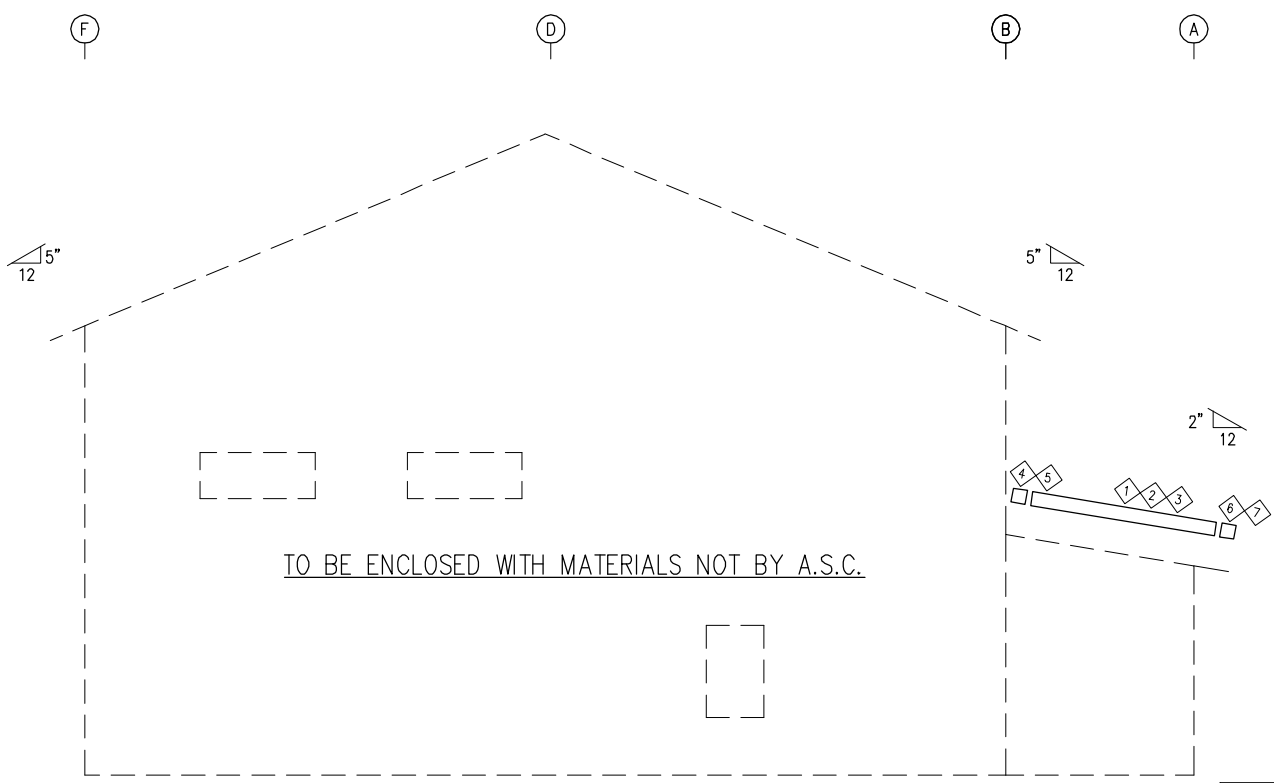
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



DESCRIPTION	ENDWALL FRAMING
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.: 56633	ENG. BY: RA DATE: 4/15/21
	DWG. NO.: 12 OF 21 ISSUE: P



ENDWALL FRAMING: FRAME LINE 1



ENDWALL TRIM: FRAME LINE 1

TRIM TABLE				
FRAME LINE 1				
ID	QUAN	PART	LENGTH	DETAIL
1	2	Q940052	4'-3"	TRIM_423
2	1	SS115	10'-2"	TRIM_404
3	1	IA3102	10'-2"	TRIM_326
4	1	SS134L	9'	
5	1	SS230R	1'-6"	
6	1	SS134R	9'	
7	1	SS230L	1'-6"	

BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
56633A				
Columns/Raf	2	A325	1/2"	1 1/4"

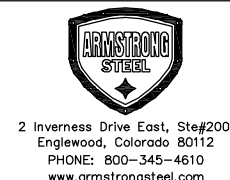
MEMBER TABLE				
FRAME LINE 1				
QUAN	MARK	PART	LENGTH	
56633A				
1	EB-2	W12531	3'-2"	
1	EB-4	W12531	3'-2"	
1	EC-1	W10841	24'-7 13/16"	
4	DJ-1	8X25C16	3'-7 1/2"	
2	DJ-2	8X25C16	7'-8"	
2	DH-1	8X25C16	4'-11"	
1	DH-2	8X25C16	2'-5"	
1	DS-1	8X25C16	2'-5"	
3	G-1	8X25Z12	18'-3 1/4"	
1	G-2	8X35C14	18'-3 1/4"	
1	G-3	8X25Z16	11'-4 3/16"	
1	G-4	8X25Z16	1'-9"	
1	G-5	8X25Z16	17'-9 1/4"	
1	G-6	8X35Z12	17'-9 1/4"	
1	G-7	8X25C14	17'-9 1/4"	
1	G-8	8X35Z14	17'-9 1/4"	
1	G-9	8X25Z16	10'-10 3/16"	
1	G-10	8X25Z16	1'-3"	
1	MB-7	W14531	18'-1 1/4"	
56633B				
1	EB-101	W10531	3'-7 15/16"	

SUPPORT BEAM BOLT TABLE					
FRAME LINE 1					
ID	QUAN	TYPE	DIA	LENGTH	QUAN
SI	6	A325	5/8"	1 1/2"	4

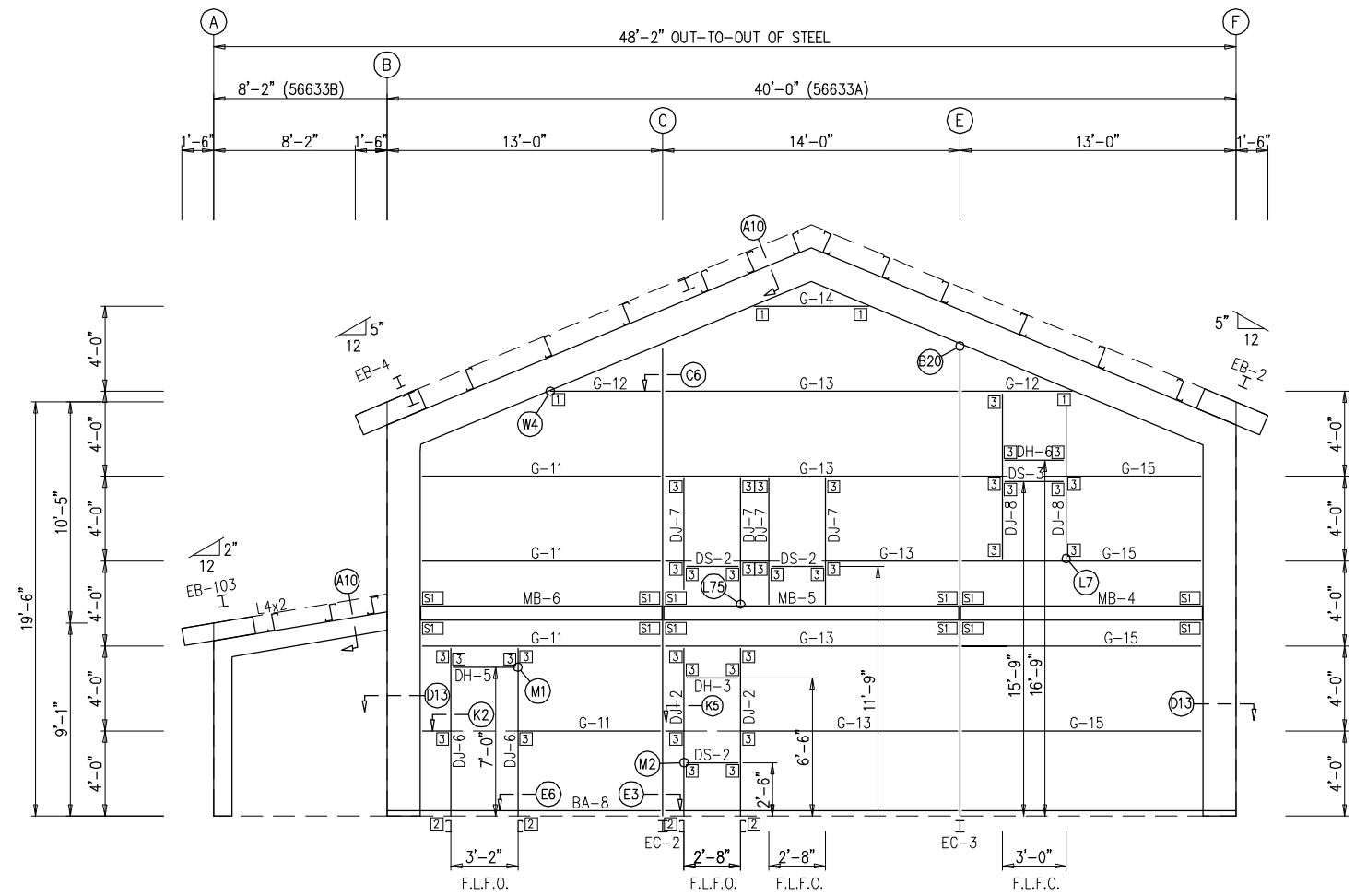
CONNECTION PLATES		
FRAME LINE 1		
ID	QUAN	MARK/PART
1	4	b2
2	20	BC-01
3	2	BC-05

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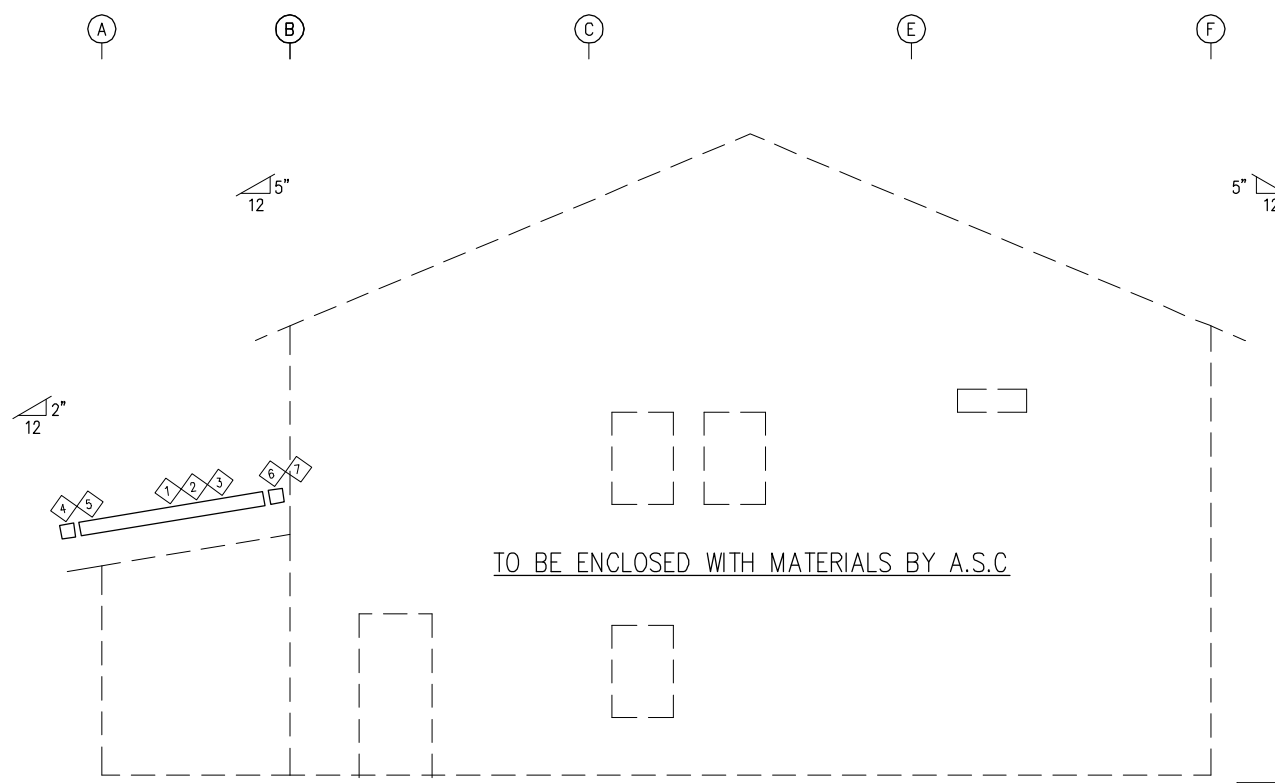
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



DESCRIPTION	ENDWALL FRAMING & TRIM
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.: 56633	ENG. BY: RA DATE: 4/15/21
	DWG. NO.: 13 OF 21 ISSUE: P



ENDWALL FRAMING: FRAME LINE 4



ENDWALL TRIM: FRAME LINE 4

TRIM TABLE				
FRAME LINE 4				
ID	QUAN	PART	LENGTH	DETAIL
1	2	Q940052	4'-3"	TRIM_423
2	1	SS115	10'-2"	TRIM_404
3	1	IA3102	10'-2"	TRIM_326
4	1	SS134L	9"	
5	1	SS230R	1'-6"	
6	1	SS134R	9"	
7	1	SS230L	1'-6"	

BOLT TABLE				
FRAME LINE 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
56633A				
Columns/Raf	4	A325	5/8"	1 1/2"

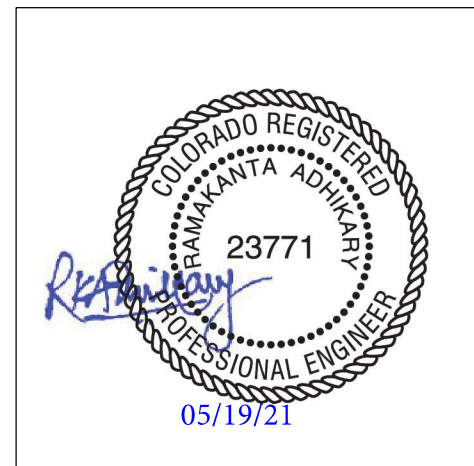
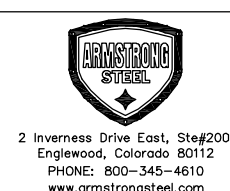
MEMBER TABLE				
FRAME LINE 4				
QUAN	MARK	PART	LENGTH	
56633A				
1	EB-2	W12531	3'-2"	
1	EB-4	W12531	3'-2"	
1	EC-2	W08641	22'-4 5/16"	
1	EC-3	W08641	22'-4 5/16"	
2	DJ-2	8X25C16	7'-8"	
2	DJ-6	8X25C16	7'-8"	
4	DJ-7	8X25C16	15'-7 1/2"	
2	DJ-8	8X25C16	7'-4"	
1	DH-3	8X25C16	2'-7"	
1	DH-5	8X25C16	3'-1"	
1	DH-6	8X25C16	2'-11"	
3	DS-2	8X25C16	2'-7"	
1	DS-3	8X25C16	2'-11"	
4	G-11	8X25Z16	11'-0 7/8"	
2	G-12	8X25Z16	5'-0 3/8"	
5	G-13	8X25Z16	13'-4"	
1	G-14	8X25Z16	5'-6 3/8"	
4	G-15	8X25Z16	11'-1"	
1	MB-4	W08531	11'-4 1/2"	
1	MB-5	W08531	13'-11"	
1	MB-6	W08531	11'-4 3/8"	
56633B				
1	EB-103	W10531	3'-7 15/16"	

SUPPORT BEAM BOLT TABLE					
FRAME LINE 4					
ID	QUAN	TYPE	DIA	LENGTH	QUAN
S1	6	A325	5/8"	1 1/2"	12

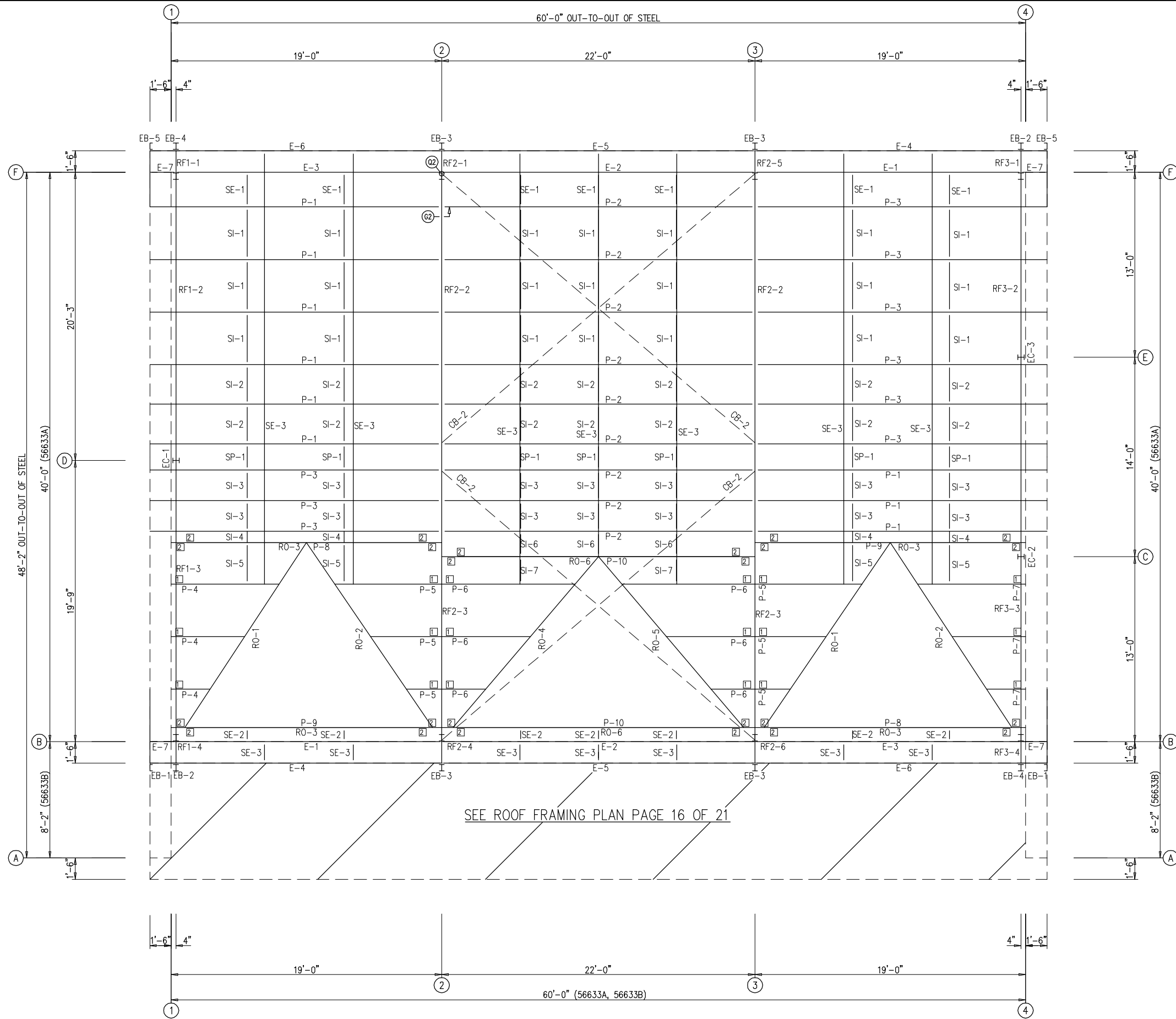
CONNECTION PLATES		
FRAME LINE 4		
ID	QUAN	MARK/PART
1	4	b2
2	4	BC-05
3	35	BC-01

NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY AND PANEL/TRIM ITEMS MAY BE NECESSARY TO ENSURE PROPER FIT. SUCH WORK IS CONSIDERED A NORMAL PART OF METAL BUILDING ERECTION. A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



DESCRIPTION	ENDWALL FRAMING & TRIM
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.: 56633	ENG. BY: RA DATE: 4/15/21
	DWG. NO.: 14 OF 21 ISSUE: P



SEE ROOF FRAMING PLAN PAGE 16 OF 21

ROOF FRAMING PLAN

EXTENSION/CANOPY BOLTS				
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	3/4"	1 3/4"
EB-3	4	A325	3/4"	1 3/4"
EB-4	4	A325	3/4"	1 3/4"
EB-101	4	A325	3/4"	1 3/4"
EB-102	4	A325	3/4"	1 3/4"
EB-103	4	A325	3/4"	1 3/4"

MEMBER TABLE			
ROOF PLAN			
QUAN	MARK	PART	LENGTH
56633A			
2	EB-1	12X25U16	5'-10 3/8"
2	EB-2	W12531	3'-2"
4	EB-3	W12531	3'-5 1/4"
2	EB-4	W12531	3'-2"
2	EB-5	12X25U16	4'-6 1/8"
2	RO-1	W12531	13'-5"
2	RO-2	W12531	13'-5"
4	RO-3	W12541	16'-11"
1	RO-4	W12531	12'-4"
1	RO-5	W12531	12'-4"
2	RO-6	W12561	19'-11"
9	P-1	12X25Z14	20'-4 3/4"
9	P-2	12X25Z12	21'-10 1/2"
9	P-3	12X25Z14	20'-4 3/4"
3	P-4	12X25Z14	2'-1 1/2"
3	P-5	12X25Z14	7 1/4"
6	P-6	12X25Z12	7 1/4"
3	P-7	12X25Z14	2'-1 1/2"
2	P-8	W08531	20'-5 1/4"
2	P-9	W08531	20'-5 1/4"
2	P-10	W08531	21'-11 1/2"
2	E-1	12566DU5	18'-0"
2	E-2	12566DU5	21'-4"
2	E-3	12566DU5	18'-0"
2	E-4	12566DU5	20'-1 1/2"
2	E-5	12566DU5	21'-4"
2	E-6	12566DU5	20'-1 1/2"
4	E-7	12566DU5	1'-5 1/2"
4	CB-2	GS1716	29'-11 3/4"
7	SP-1	6X25Z16	1'-4 1/8"
35	SI-1	6X25Z16	3'-11"
14	SI-2	6X25Z16	2'-11 1/8"
14	SI-3	6X25Z16	2'-3"
4	SI-4	6X25Z16	9"
4	SI-5	6X25Z16	3'-1"
3	SI-6	6X25Z16	1'-10"
3	SI-7	6X25Z16	2'-0"
7	SE-1	6X25Z16	2'-7 3/16"
7	SE-2	6X25Z16	1'-0 7/16"
7	SE-3	6X25Z16	2'-10"
14	SE-3	6X25Z16	1'-6 15/16"

CONNECTION PLATES		
ROOF PLAN		
ID	QUAN	MARK/PART
1	18	b1
2	32	BC-01
3	4	b100

NOTE:
FRAMED OPENINGS PROVIDED FOR
DORMERS TO BE DESIGNED &
SUPPLIED BY OTHERS



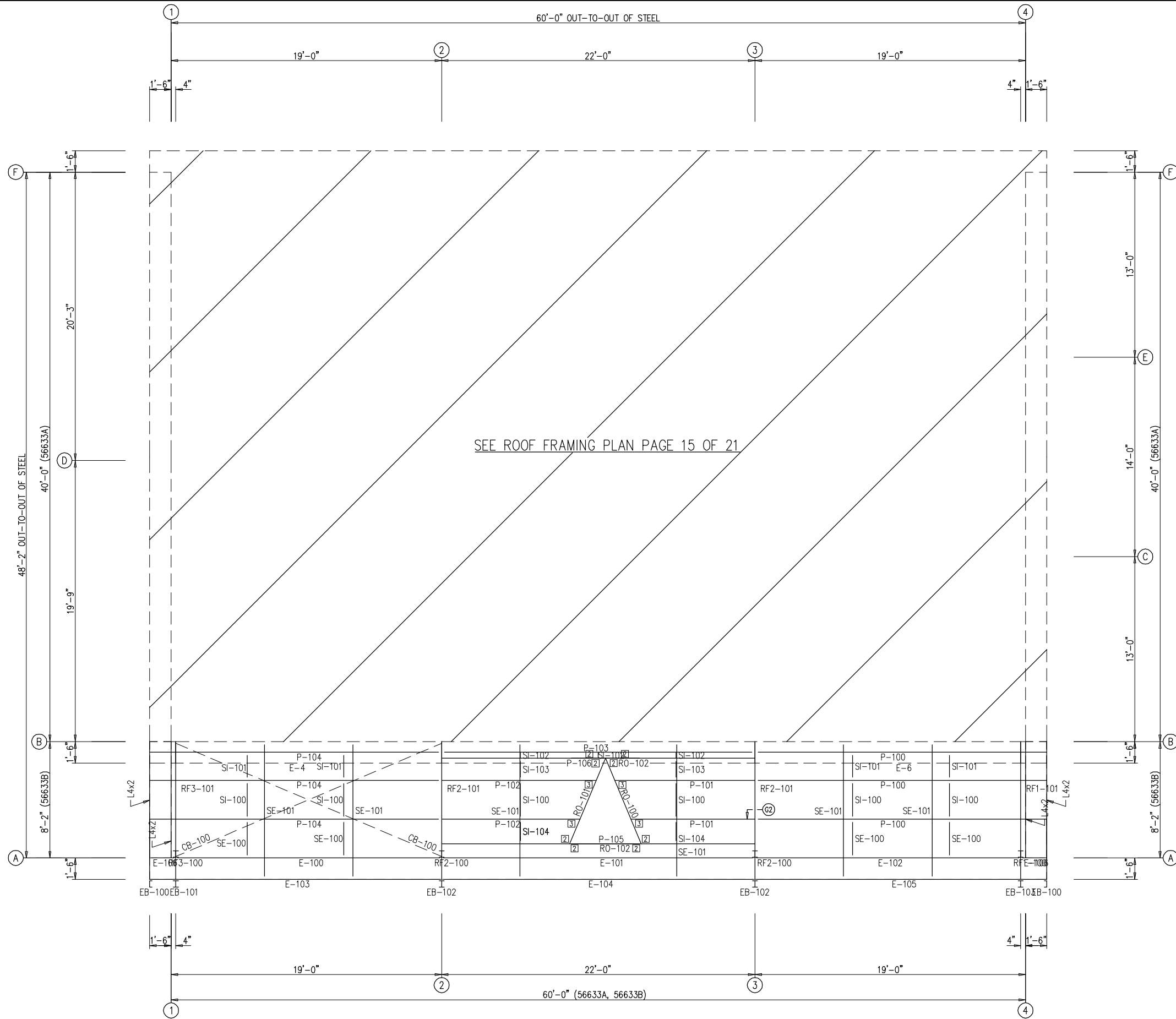
NOTE:
MINOR FIELD WORK OF STRUCTURAL, SECONDARY
AND PANEL/TRIM ITEMS MAY BE NECESSARY TO
ENSURE PROPER FIT. SUCH WORK IS CONSIDERED
A NORMAL PART OF METAL BUILDING ERECTION.
A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR
FIELD WORK.

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com

DESCRIPTION	ROOF FRAMING
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO:	56633
ENG. BY:	RA
DATE:	4/15/21
DWG. NO.:	15 OF 21
ISSUE:	P



EXTENSION/CANOPY BOLTS				
ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	3/4"	1 3/4"
EB-3	4	A325	3/4"	1 3/4"
EB-4	4	A325	3/4"	1 3/4"
EB-101	4	A325	3/4"	1 3/4"
EB-102	4	A325	3/4"	1 3/4"
EB-103	4	A325	3/4"	1 3/4"

MEMBER TABLE				
ROOF PLAN				
QUAN	MARK	PART	LENGTH	
56633B				
2	EB-100	10X25U16	4'-6 5/16"	
1	EB-101	W10531	3'-7 15/16"	
2	EB-102	W10531	3'-7 15/16"	
1	EB-103	W10531	3'-7 15/16"	
1	RO-100	W10531	5'-5"	
1	RO-101	W10531	5'-5"	
2	RO-102	W10531	4'-11"	
3	P-100	10x25Z12	20'-4 3/4"	
2	P-101	10x25Z12	7'-7 1/4"	
2	P-102	10x25Z12	8'-7 1/4"	
1	P-103	10x25Z12	21'-10 1/2"	
3	P-104	10x25Z12	20'-4 3/4"	
1	P-105	W10531	21'-11 1/2"	
1	P-106	W10531	21'-11 1/2"	
1	E-100	10354DU2	18'-0"	
1	E-101	10362DU2	21'-4"	
1	E-102	10354DU2	18'-0"	
1	E-103	10346DU2	20'-1 1/2"	
1	E-104	10354DU2	21'-4"	
1	E-105	10346DU2	20'-1 1/2"	
2	E-106	10354DU2	1'-5 1/2"	
2	CB-100	GS1716	20'-0 1/4"	
7	SI-100	6X25Z16	2'-8 3/16"	
4	SI-101	6X25Z16	1'-11"	
3	SI-102	6X25Z16	4 3/16"	
4	SI-103	6X25Z16	1'-5 7/8"	
4	SE-100	6X25Z16	2'-8 5/16"	
4	SI-104	6X25Z16	1'-8"	
7	SE-101	6X25Z16	1'-5 7/16"	

CONNECTION PLATES			
ROOF PLAN			
ID	QUAN	MARK/PART	
1	18	b1	
2	32	BC-01	
3	4	b100	

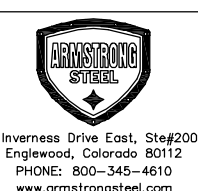
NOTE:
FRAMED OPENINGS PROVIDED FOR
DORMERS TO BE DESIGNED &
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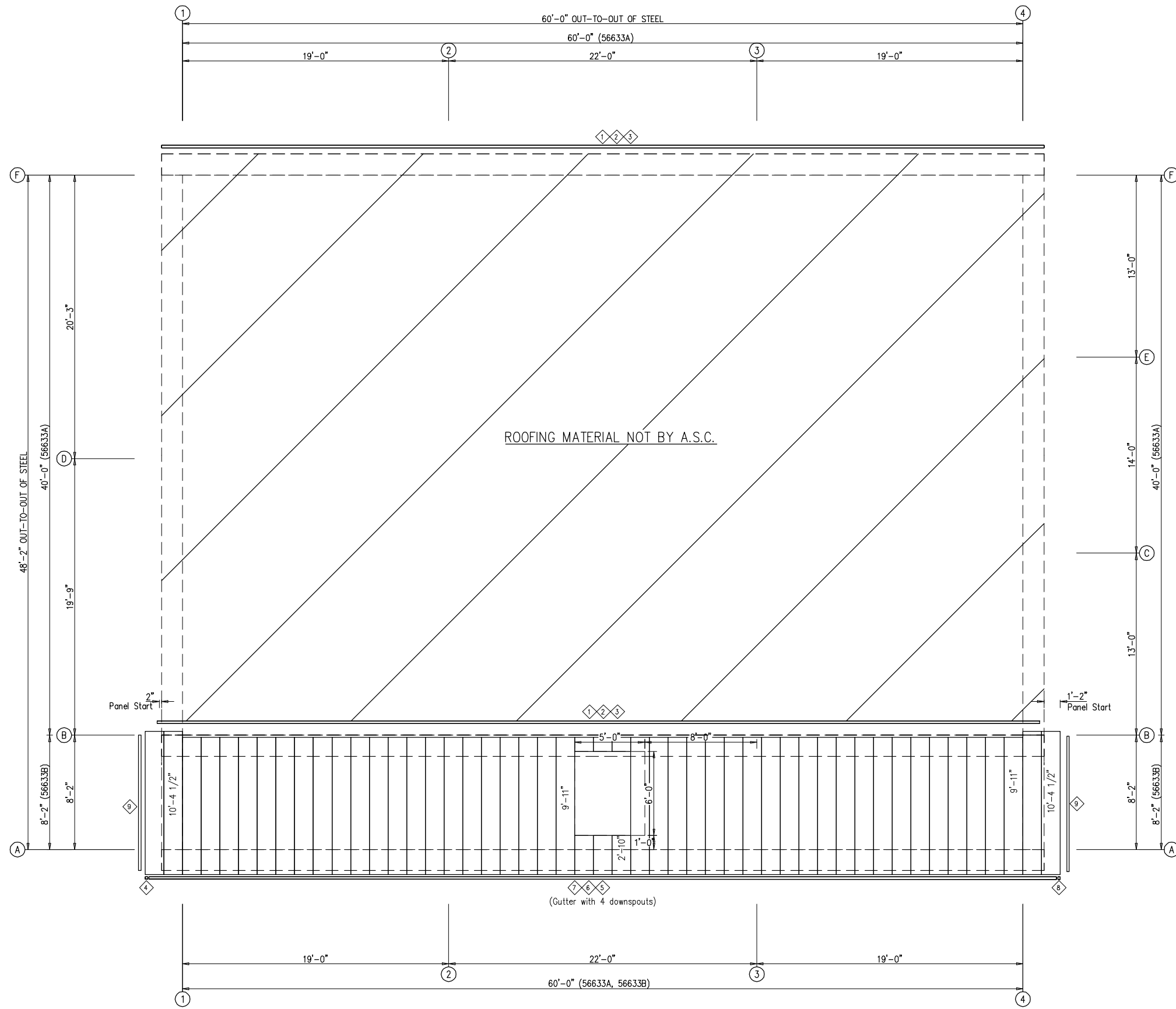
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A.S.C. WILL NOT HONOR BACKCHARGES FOR MINOR
FIELD WORK.

ROOF FRAMING PLAN

ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



DESCRIPTION	ROOF FRAMING
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO:	56633
ENG. BY:	RA
DATE:	4/15/21
DWG. NO.:	16 OF 21
ISSUE:	P



TRIM TABLE				
ID	QUAN	PART	LENGTH	DETAIL
1	2	SS250	10'-2"	TRIM_414
2	6	SS251	20'-4"	TRIM_414
3	8	Q190162	16'-2"	
4	1	SS245L	6 1/2"	
5	1	VSRGU102	10'-2"	TRIM_408
6	3	VSRGU204	20'-4"	TRIM_408
7	4	Q190162	16'-2"	TRIM_72
8	1	SS245R	6 1/2"	
9	2	IA3102	10'-2"	TRIM_326

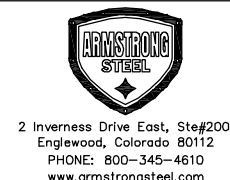
EXTENSION/CANOPY BOLTS				
ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	3/4"	1 3/4"
EB-3	4	A325	3/4"	1 3/4"
EB-4	4	A325	3/4"	1 3/4"
EB-101	4	A325	3/4"	1 3/4"
EB-102	4	A325	3/4"	1 3/4"
EB-103	4	A325	3/4"	1 3/4"

ROOF SHEETING PLAN
 PANELS: 24 Ga. Central Span - Striated - Need KYNAR Color

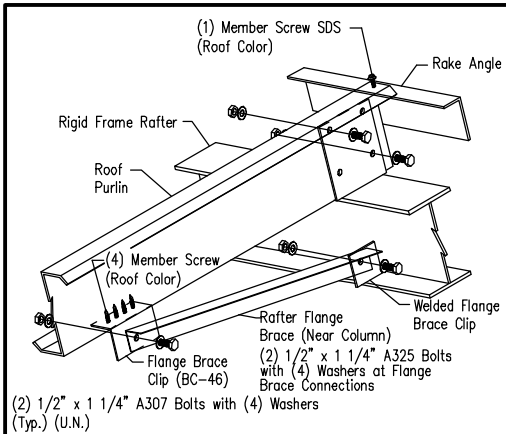


NOTE:
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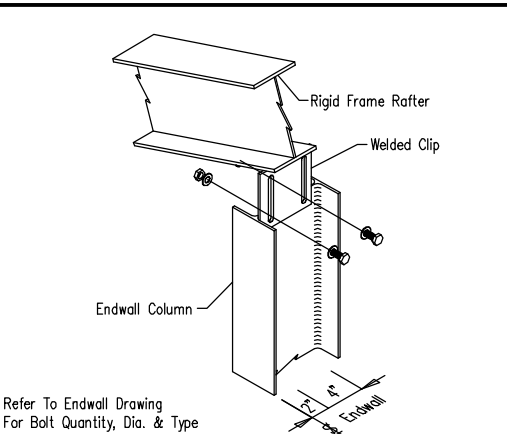
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



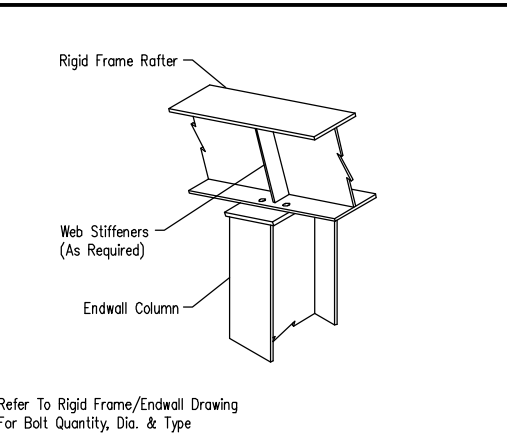
DESCRIPTION	ROOF SHEETING PLAN		
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS		
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS		
SCALE	NOT TO SCALE		
JOB NO.:	56633	ENG. BY:	RA
DATE:	4/15/21	ISSUE:	P
DWG. NO.:	17 OF 21		



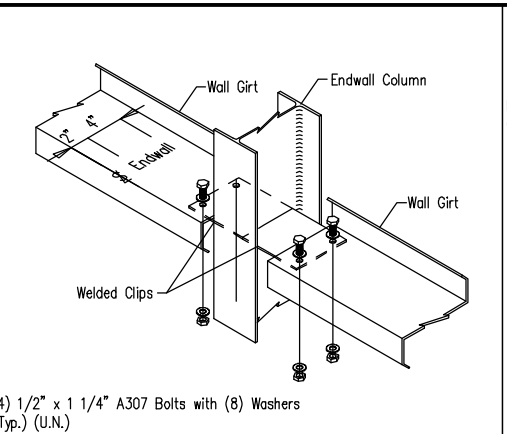
A10 ROOF PURLIN TO RIGID FRAME RAFTER W/ LINER PANEL OR LINER SYSTEM INSULATION



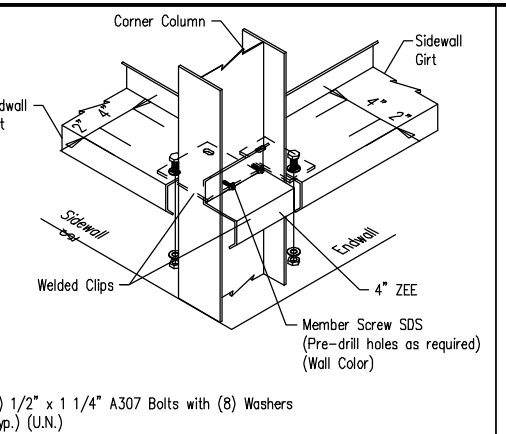
B19 BUILT-UP ENDWALL COLUMN TO RIGID FRAME RAFTER



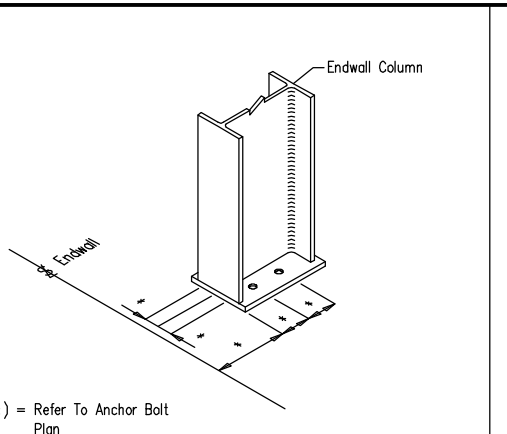
B20 ENDWALL COLUMN TO RIGID FRAME RAFTER



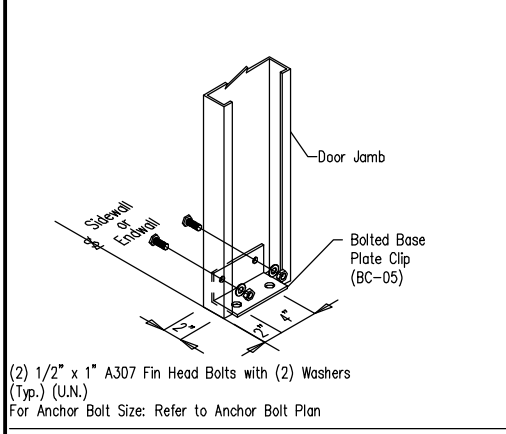
C6 WALL GIRTS TO ENDWALL COLUMN



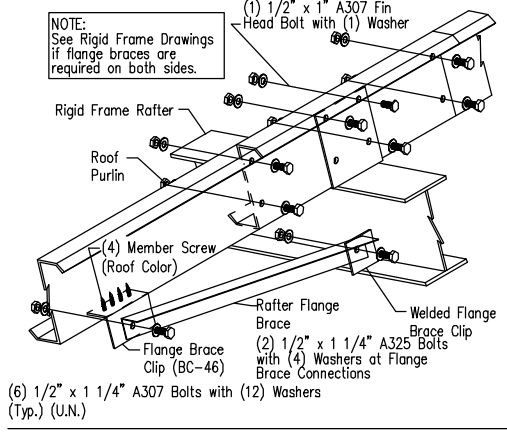
D13 WALL GIRTS TO ENDWALL CORNER COLUMN



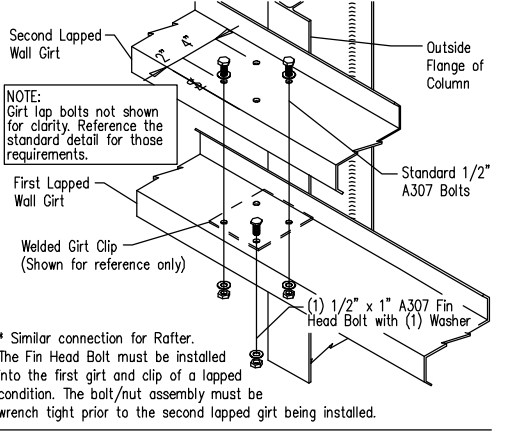
E3 ANCHOR BOLTS AT ENDWALL COLUMNS



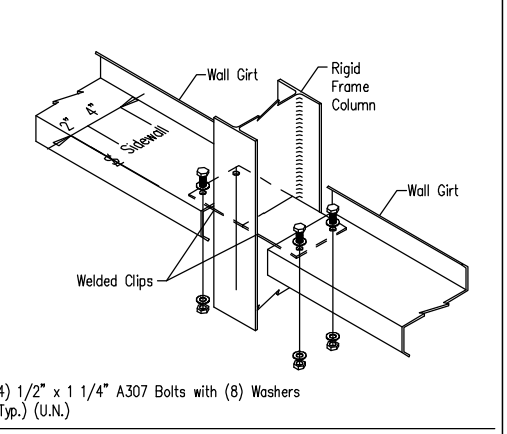
E6 BASE PLATE FOR DOOR JAMB



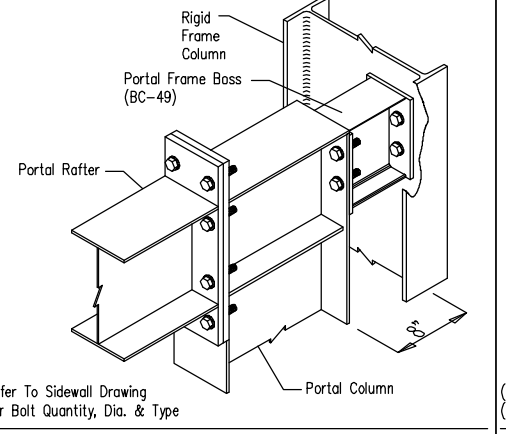
G2 ROOF PURLIN TO INTERIOR RIGID FRAME W/ LINER PANEL OR LINER SYSTEM INSULATION



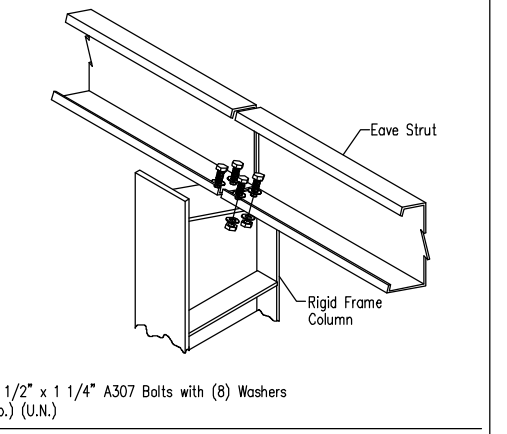
H0 BYPASS LAPPED WALL GIRTS



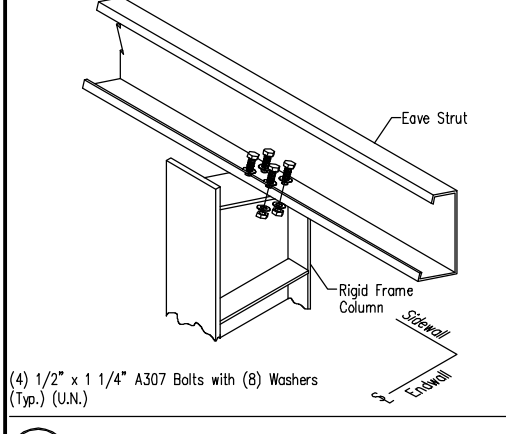
H6 WALL GIRTS TO RIGID FRAME COLUMN



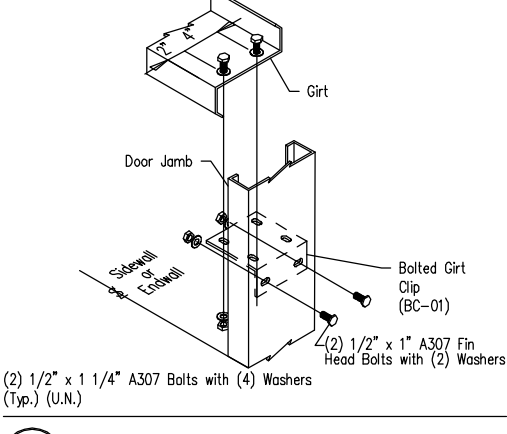
H10 PORTAL FRAME TO RIGID FRAME COLUMN



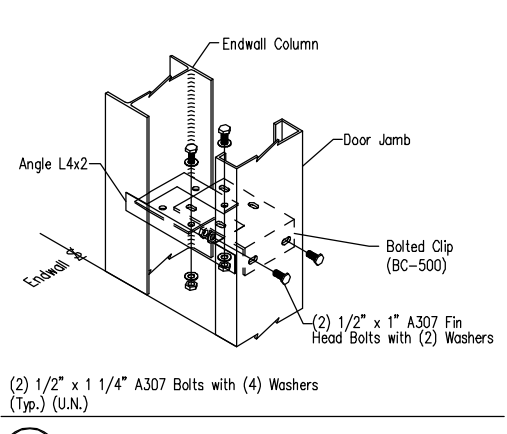
J1 LOWSIDE EAVE STRUT TO FLUSH RIGID FRAME



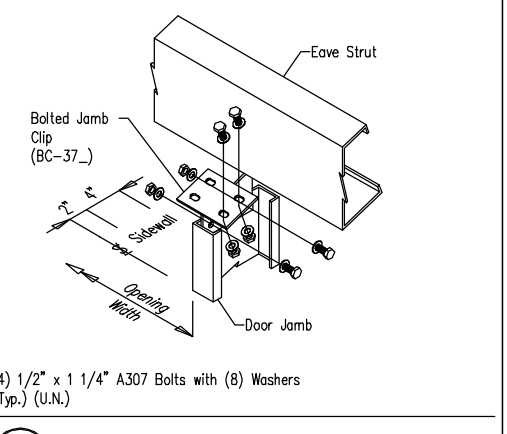
J23 LOWSIDE EAVE STRUT TO FLUSH RIGID FRAME



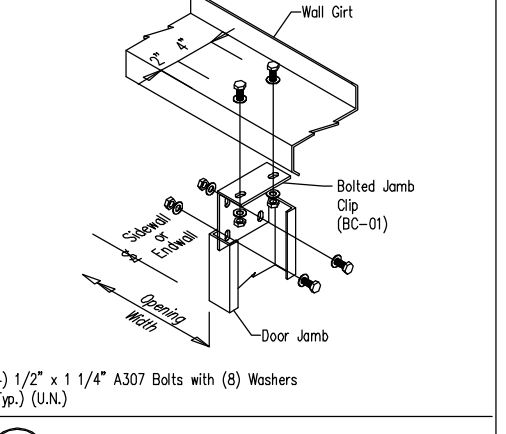
K2 WALL GIRTS TO DOOR JAMB



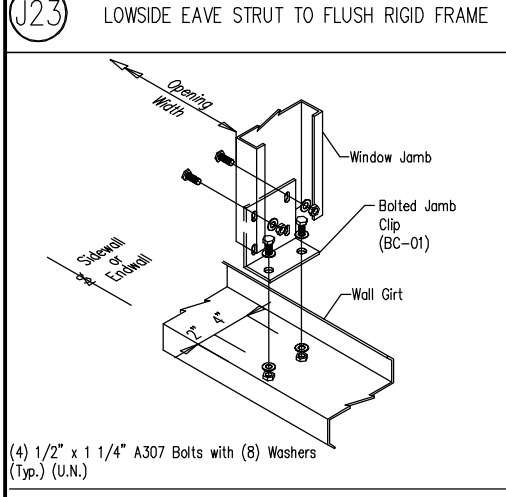
K5 ENDWALL COLUMN AND DOOR JAMB CONNECTION



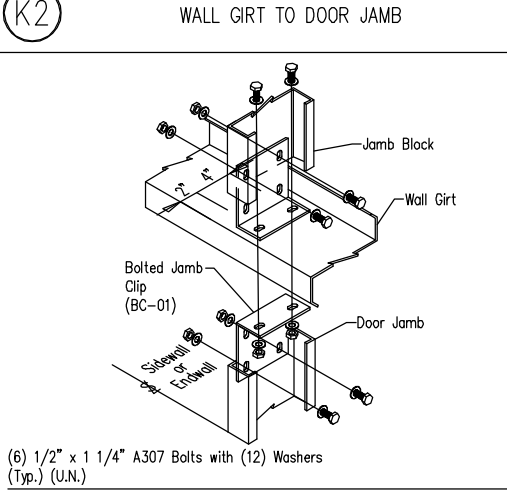
L1 DOOR JAMB TO EAVE STRUT



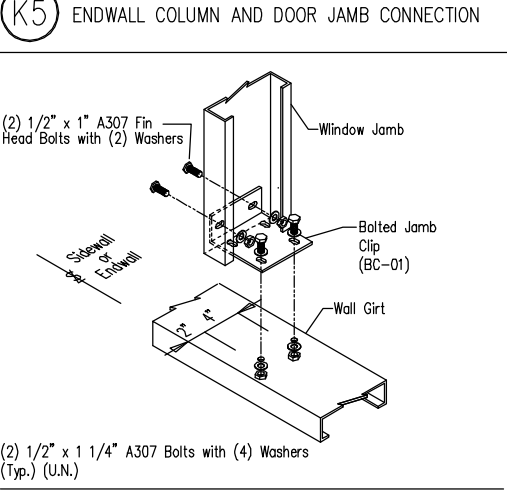
L6 DOOR JAMB TO WALL GIRTS



L7 WINDOW JAMB TO WALL GIRTS



L8 DOOR JAMB/JAMB BLOCK TO WALL GIRTS



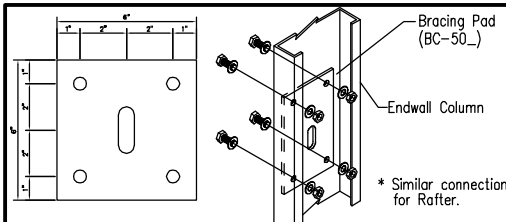
L9 DOOR JAMB TO WALL GIRTS



ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



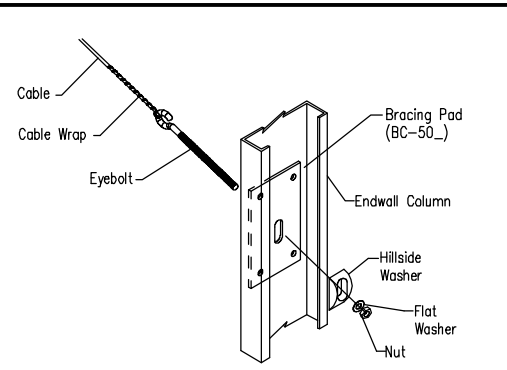
DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.: 56633	ENG. BY: RA DATE: 4/15/21
	DWG. NO.: 18 OF 21 ISSUE: P



DIAGONAL BRACE PAD INSTALLATION INSTRUCTIONS

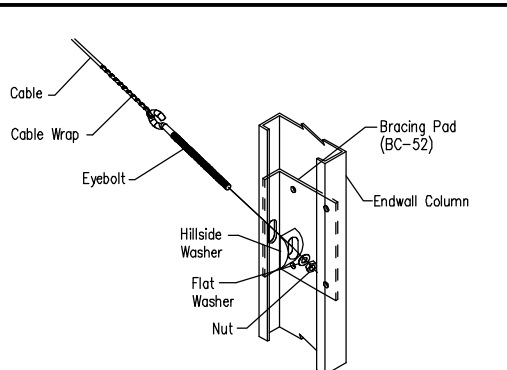
- STEP 1** Line up brace pad with pre existing hole punches in the member.
- STEP 2** Bolt the brace pad down using (4) 1/2" A307 bolts.
- STEP 3** Field cut out the slot, using the brace pad slot as a template.
- STEP 4** Install cable brace as normal, still leaving the brace pad installed along with the 1/2" A307 bolts.

Q2 DIAGONAL BRACE PAD TO WEB OF CEE COLUMN



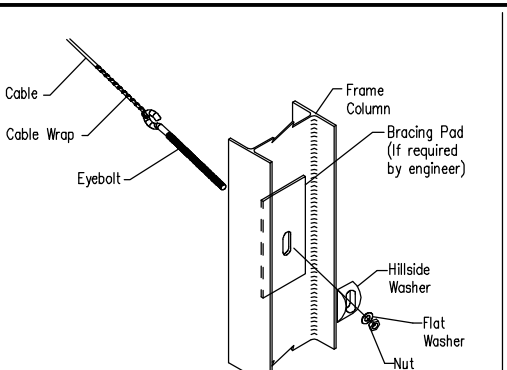
* Similar connection for Rafter. Insert Eyebolt through slot in web. Then assemble Hillside Washer, Flat Washer, and Nut.

DIAGONAL CABLE BRACE TO WEB OF CEE COLUMN



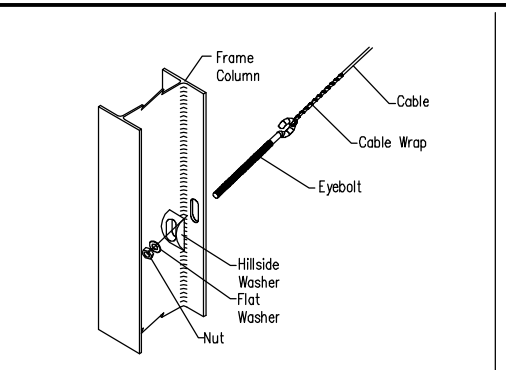
* Similar connection for Rafter. Insert Eyebolt through slot in flange. Then assemble Hillside Washer, Flat Washer, and Nut.

DIAGONAL CABLE BRACE TO FLANGE OF CEE COLUMN



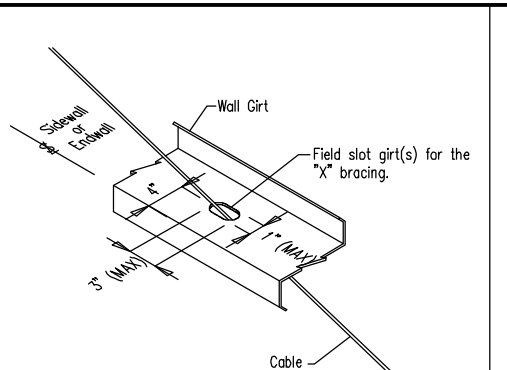
* Similar connection for Rafter. Insert Eyebolt through slot in web. Then assemble Hillside Washer, Flat Washer, and Nut.

DIAGONAL CABLE BRACE TO WEB OF FRAME COLUMN

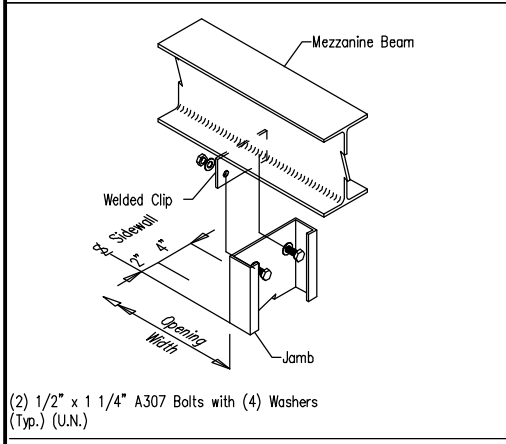


Insert Eyebolt through slot in flange. Then assemble Hillside Washer, Flat Washer, and Nut.

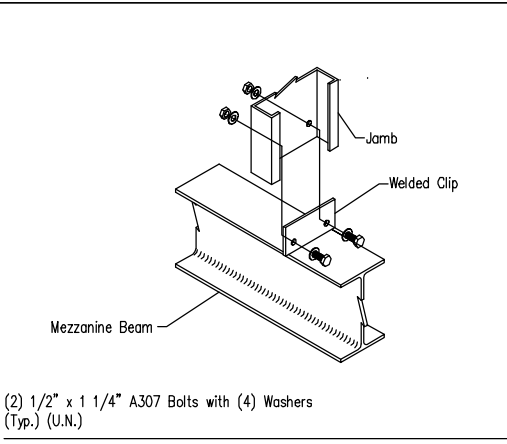
DIAGONAL CABLE BRACE TO FLANGE OF FRAME COLUMN



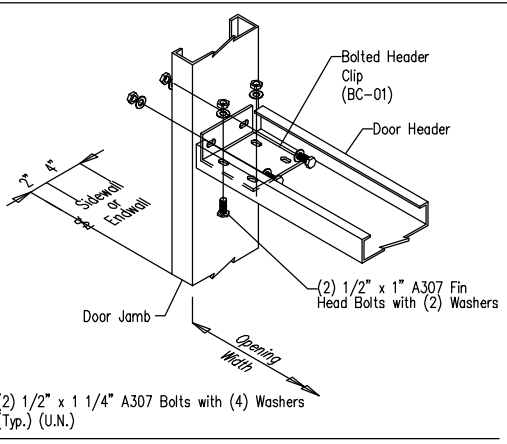
DIAGONAL CABLE BRACE AT FLUSH WALL GIRTS



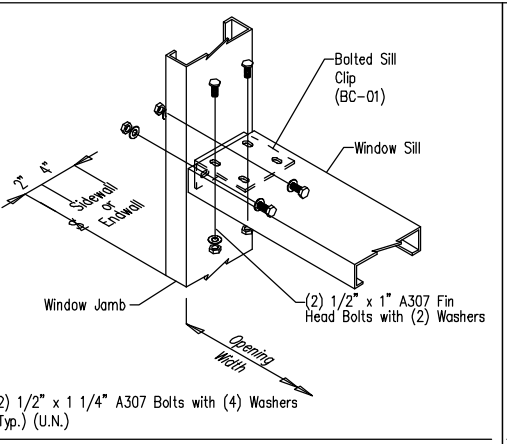
L71 JAMB TO MEZZANINE BEAM



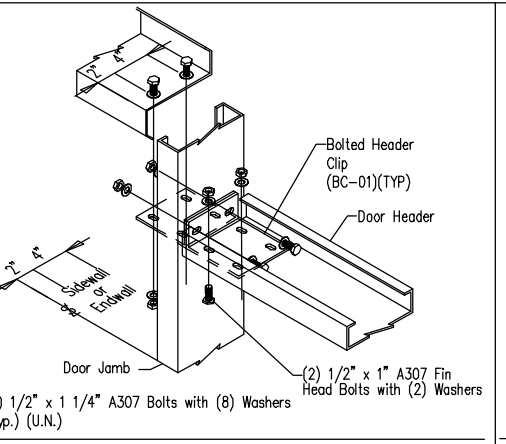
L75 JAMB TO MEZZANINE BEAM



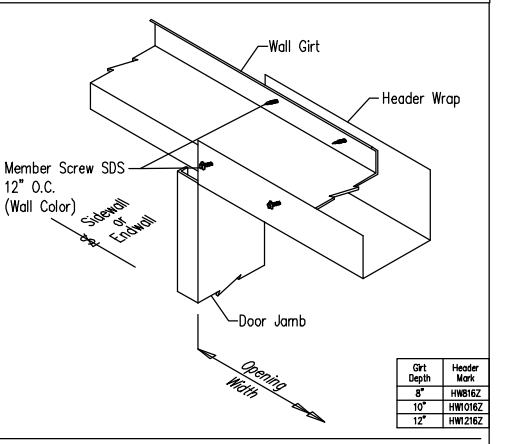
M1 HEADER TO JAMB



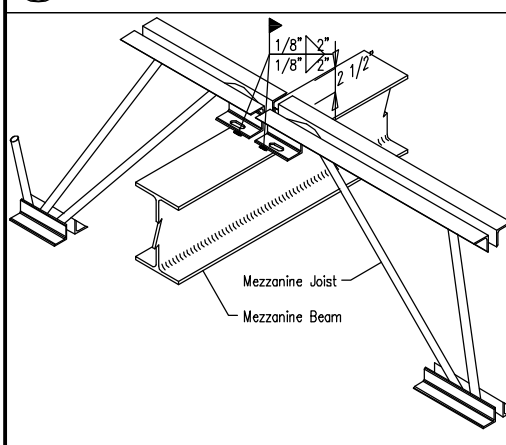
M2 SILL TO JAMB



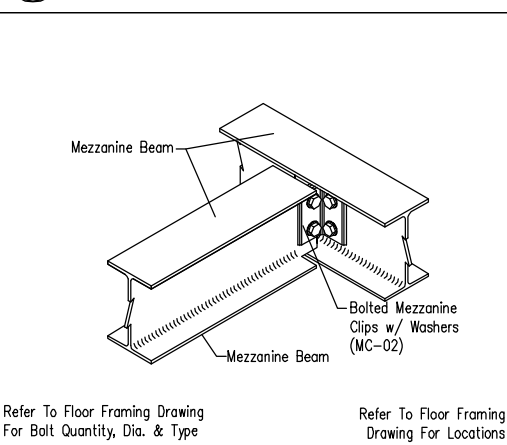
M3 DOOR HEADER & GIRTS TO DOOR JAMB



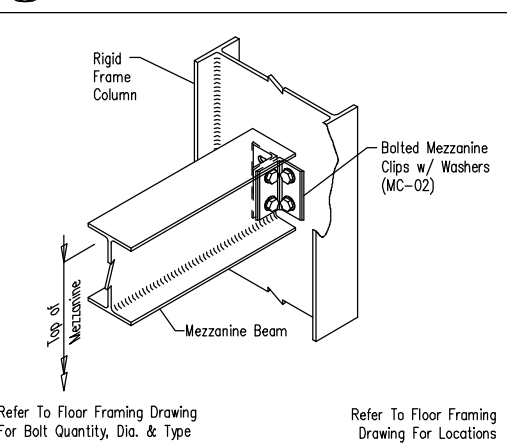
M15 HEADER WRAP TO WALL GIRTS



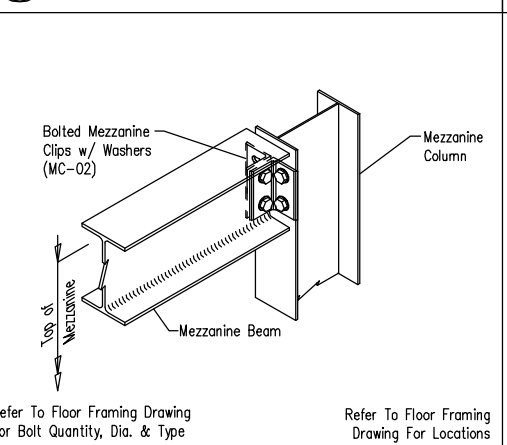
N22 MEZZANINE JOIST TO MEZZANINE BEAM



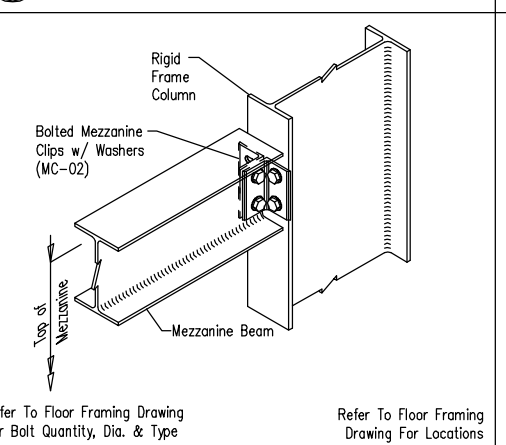
N35 MEZZANINE BEAM TO MEZZANINE COLUMN



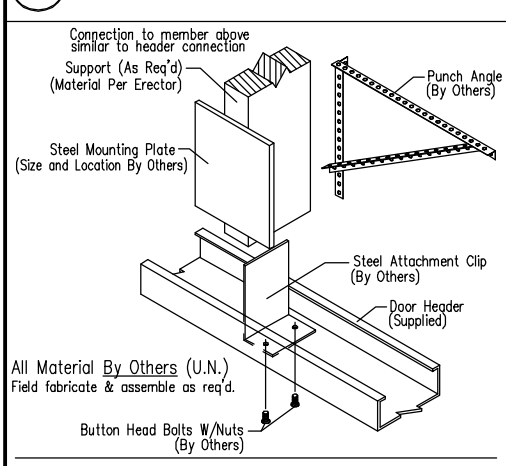
N50 MEZZANINE BEAM TO RIGID FRAME COLUMN WEB



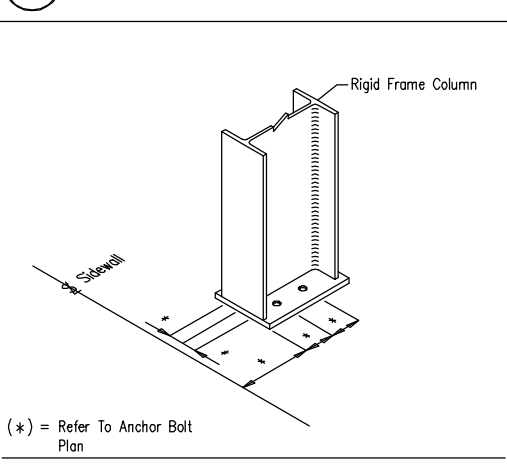
N52 MEZZANINE BEAM TO MEZZANINE COLUMN



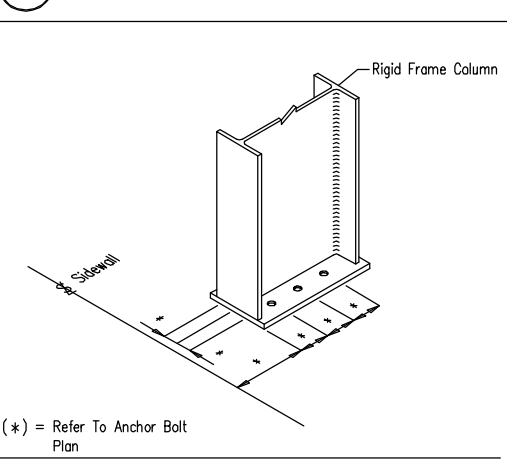
N53 MEZZANINE BEAM TO RIGID FRAME COLUMN FLANGE



OH O. H. DOOR TORSION BAR BEARING SUPPORT



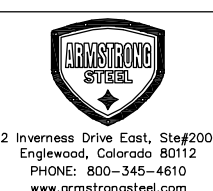
R2 ANCHOR BOLTS AT SIDEWALL COLUMNS



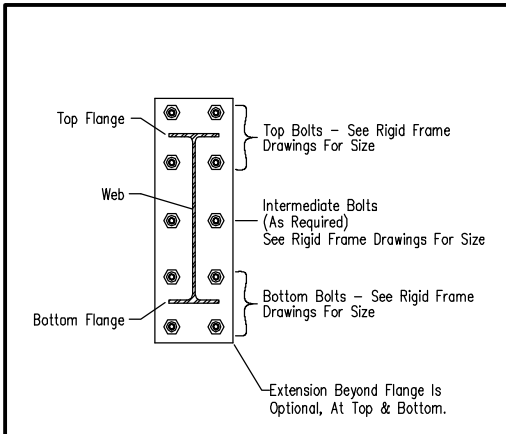
R3 ANCHOR BOLTS AT SIDEWALL COLUMNS



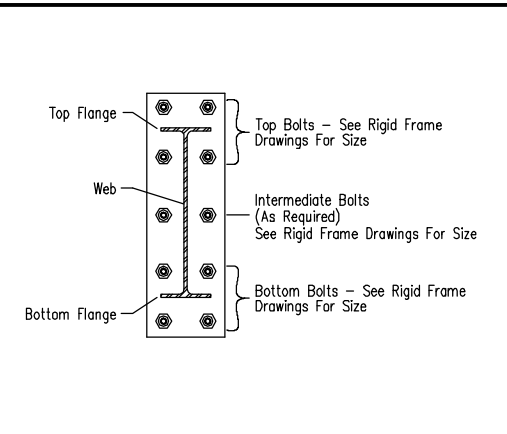
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



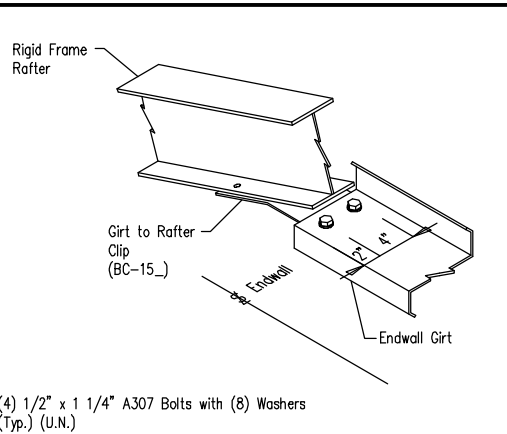
DESCRIPTION	DETAIL DRAWINGS
CUSTOMER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO.: 56633	ENG. BY: RA DATE: 4/15/21
	DWG. NO.: 19 OF 21 ISSUE: P



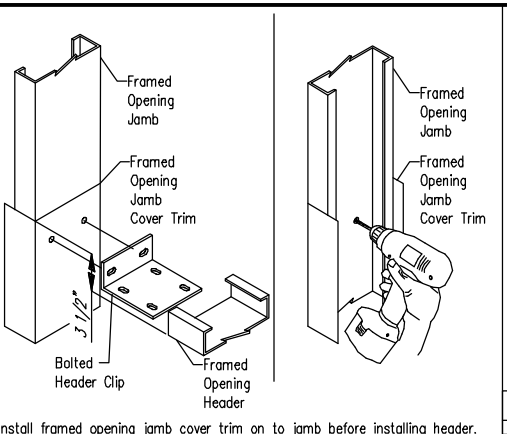
U2 BOLTS FOR RIGID FRAME RAFTER AT BUILDING PEAK



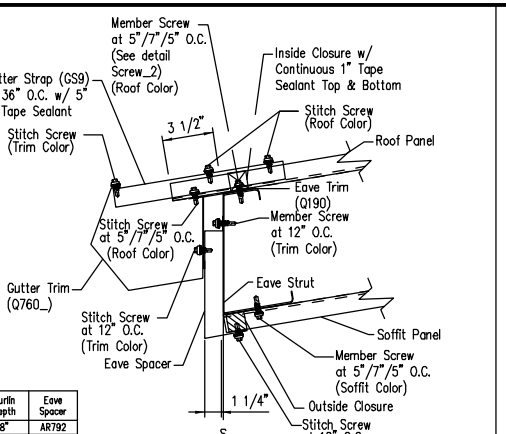
U3 BOLTS FOR RIGID FRAME RAFTER TO COLUMN CONNECTION



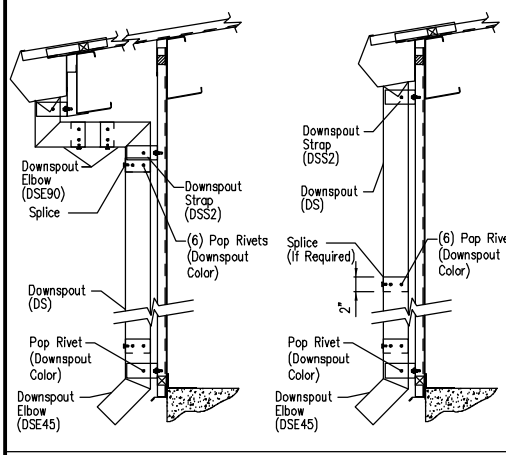
W4 ENDWALL GIRT TO RIGID FRAME RAFTER



TRIM_52 COVER TRIM INSTALLATION INSTRUCTIONS

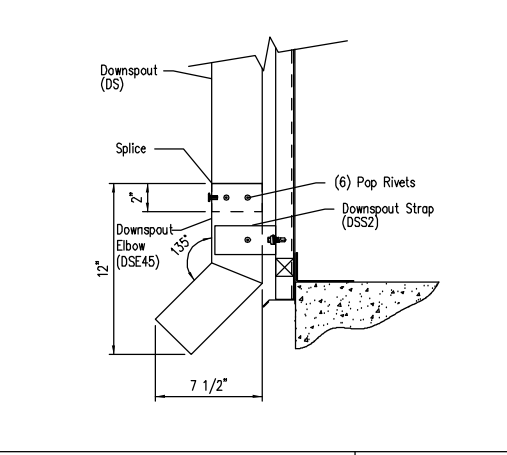


TRIM_72 EAVE DETAIL WITH GUTTER AT OPEN WALL (WITH SOFFIT/SIMILAR AT EAVE EXTENSION/CANOPY)

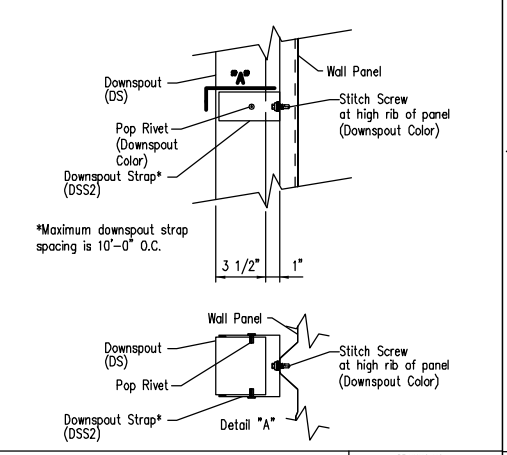


1. Refer to the building erection drawings for the location and spacing of the downspouts. If the spacing is not known, call customer service.
2. Locate all downspouts over a major rib if possible.
3. Make a cardboard template of the downspout shape. Place the template on the bottom of the gutter and trace the outline. Remove the template and draw a line from corner to corner, forming an "X" pattern.
4. Drill a hole at the center of the "X". Using tin snips, cut along the lines of the X only. Do not cut along the outside lines of the downspout square.
5. Bend each triangular tab down toward the ground, 90 degrees to the bottom of the gutter.
6. Position the top of the downspout under the gutter. Make sure all four gutter tabs are on the inside of the downspout.
7. Install (1) pop rivet through the downspout into the gutter tabs. Only the two sides and the front of the downspout will receive the pop rivets.
8. Place a bead of tube caulk around the top of the downspout for a tight seal.

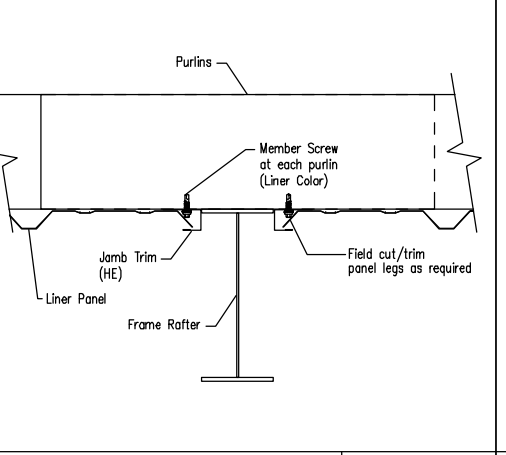
TRIM_90 STANDARD DOWNSPOUT TO GUTTER ATTACHMENT DETAIL



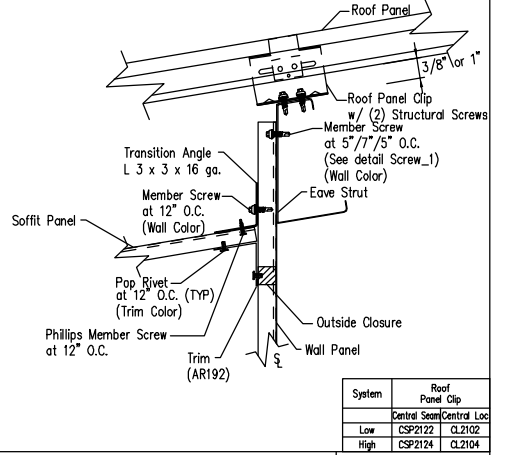
TRIM_91 3 1/2" x 4" DOWNSPOUT ELBOW DETAIL



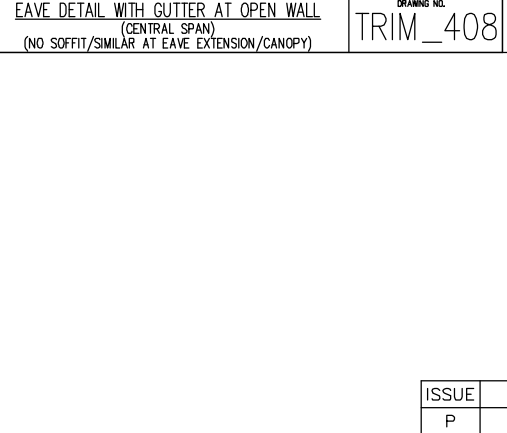
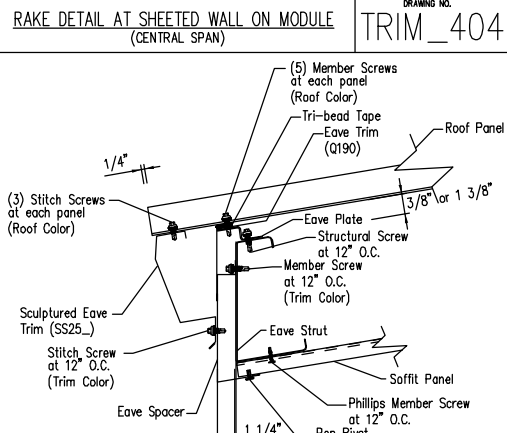
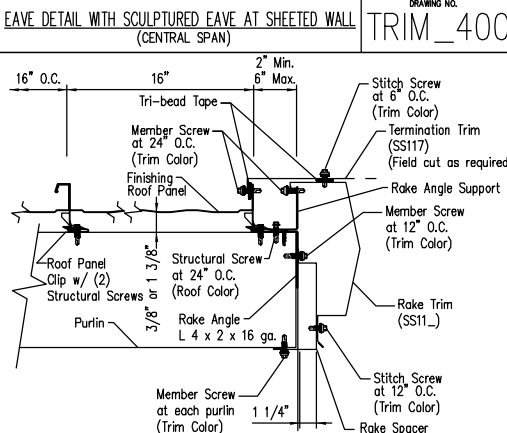
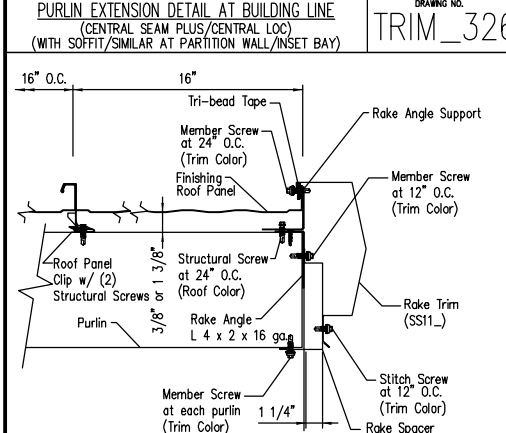
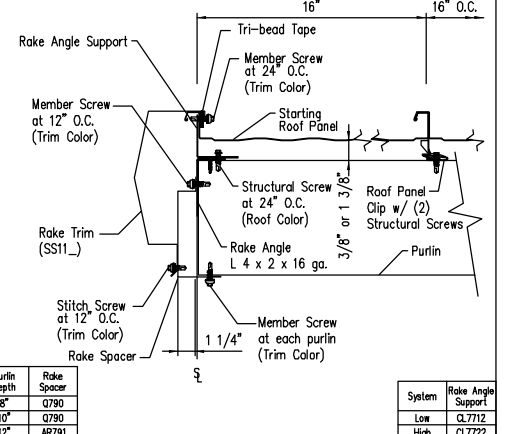
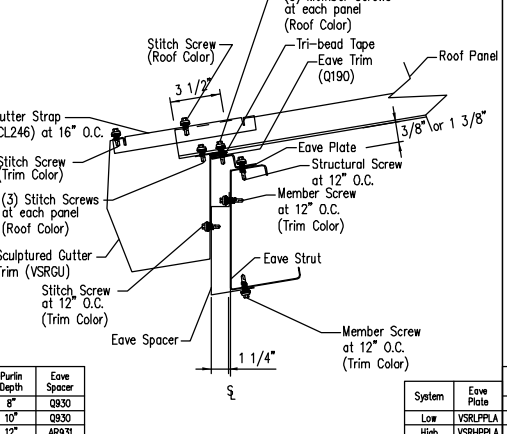
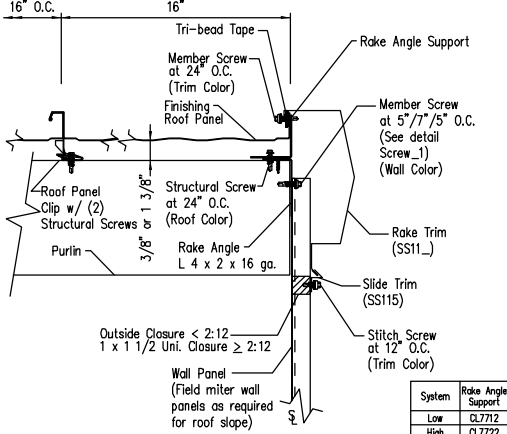
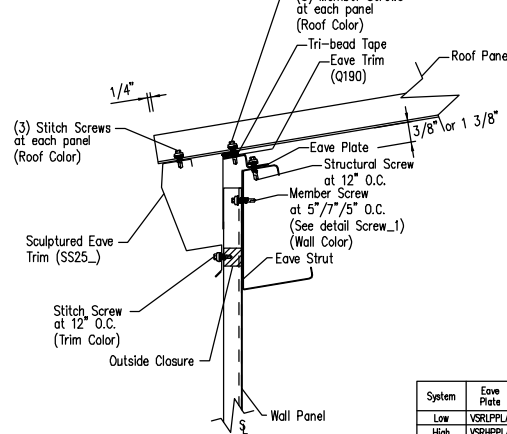
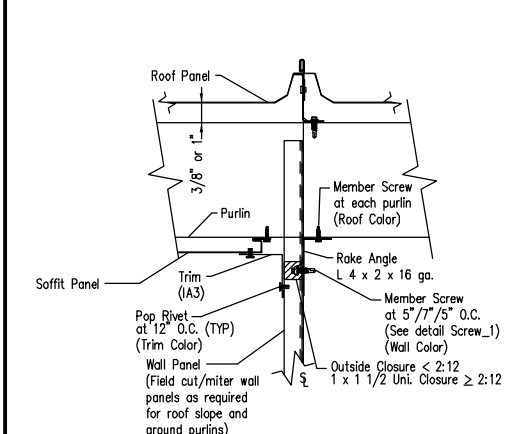
TRIM_92 3 1/2" x 4" DOWNSPOUT STRAP DETAIL



TRIM_200 LINER PANEL AT BYPASS FRAME COLUMNS



TRIM_324 EAVE EXTENSION DETAIL AT BUILDING LINE (CENTRAL SEAM PLUS/CENTRAL LOC) (WITH SOFFIT/SIMILAR AT EAVE EXTENSION/INSET BAY)



TRIM_411 RAKE DETAIL AT OPEN WALL ON MODULE (NO SOFFIT/SIMILAR AT PURLIN EXTENSION/END OF CANOPY)

TRIM_412 RAKE DETAIL AT OPEN WALL OFF MODULE (1) (CENTRAL SPAN) (NO SOFFIT/SIMILAR AT PURLIN EXTENSION/END OF CANOPY)

TRIM_414 EAVE DETAIL WITH SCULPTURED EAVE AT OPEN WALL (CENTRAL SPAN) (WITH SOFFIT/SIMILAR AT EAVE EXTENSION/CANOPY)

TRIM_414 EAVE DETAIL WITH GUTTER AT OPEN WALL (CENTRAL SPAN) (NO SOFFIT/SIMILAR AT PURLIN EXTENSION/END OF CANOPY)

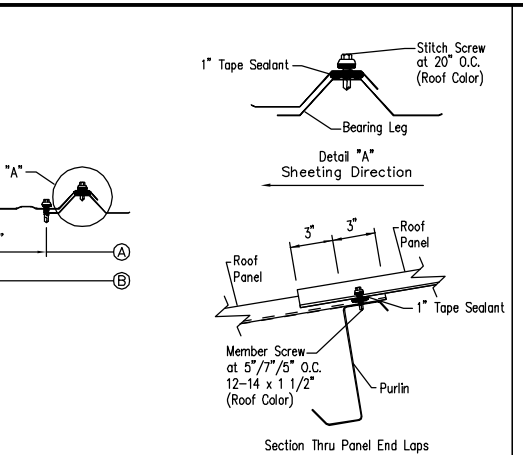
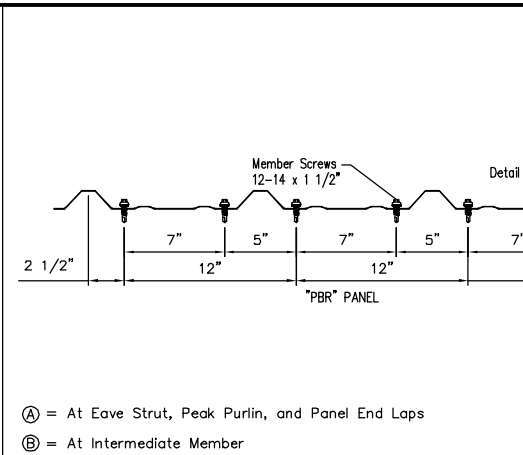
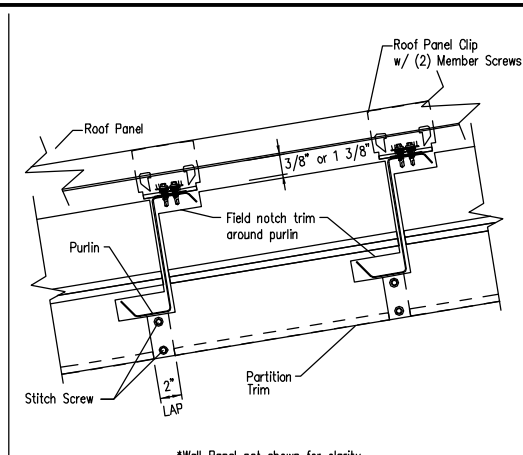
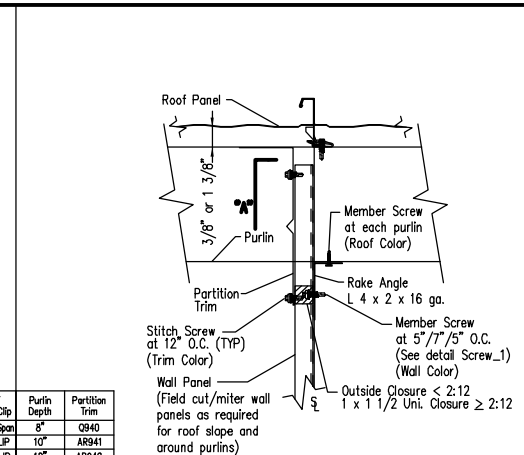
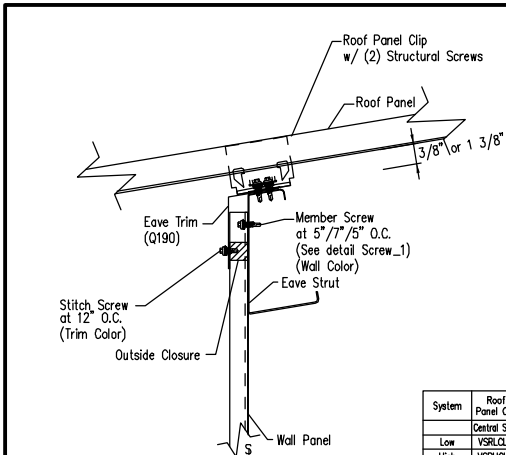
ISSUE	DESCRIPTION	DATE	DRN.	CHK.	DES.
P	PERMIT	4/28/21	DMC	SW	RA



2 Inverness Drive East, Ste#200
Englewood, Colorado 80112
PHONE: 800-345-4610
www.armstrongsteel.com



DESCRIPTION	DETAIL DRAWINGS
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END USER	JAMY WHITEMAN/FORTIFIED SOLUTIONS
SCALE	NOT TO SCALE
JOB NO:	56633
ENG. BY:	RA
DATE:	4/15/21
DWG. NO.:	20 OF 21
ISSUE:	P



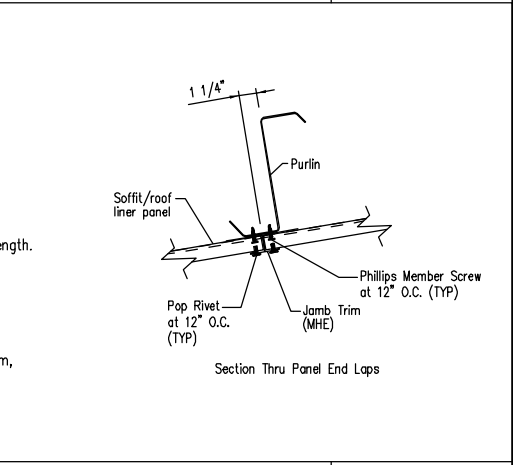
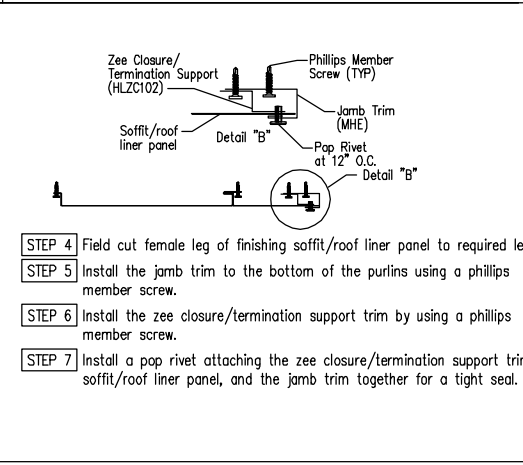
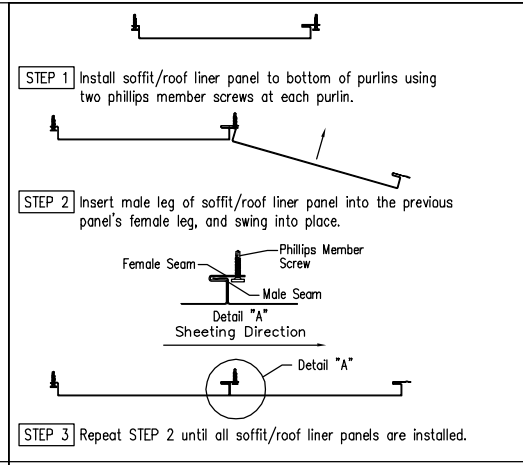
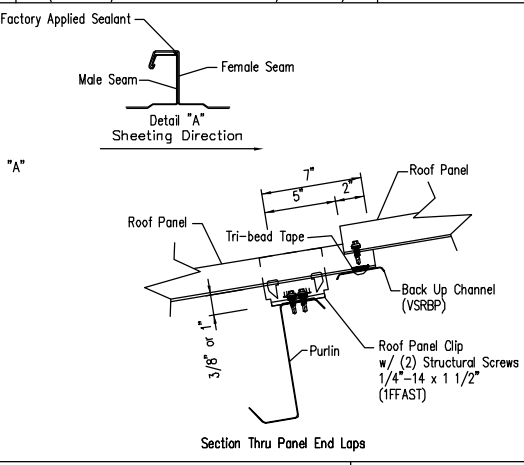
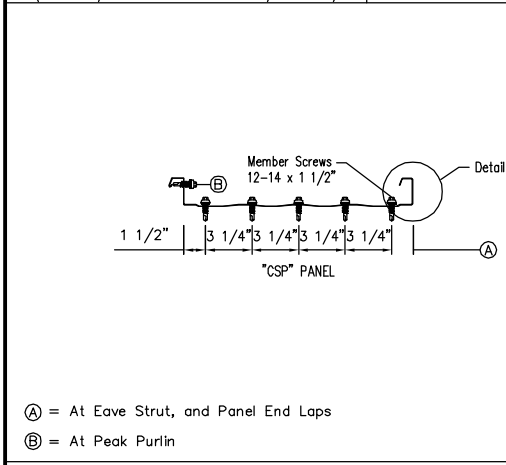
EAVE EXTENSION DETAIL AT BUILDING LINE (CENTRAL SPAN) (NO SOFFIT/SIMILAR AT EAVE EXTENSION/INSET BAY) TRIM_421

PURLIN EXTENSION DETAIL AT BUILDING LINE (CENTRAL SPAN) (NO SOFFIT/SIMILAR AT PARTITION WALL/INSET BAY) TRIM_423

FASTENER LOCATION FOR ROOF PANELS

FASTENER LOCATION FOR ROOF PANELS

SCREW_2



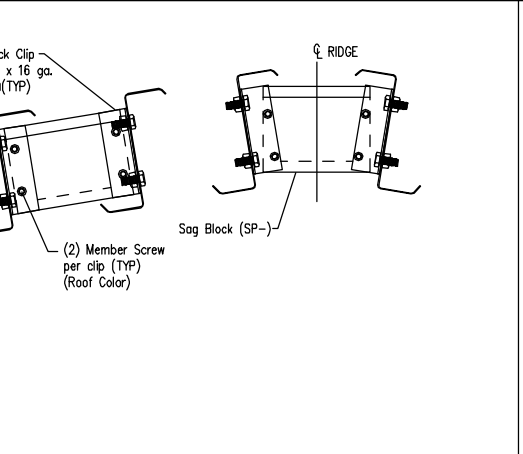
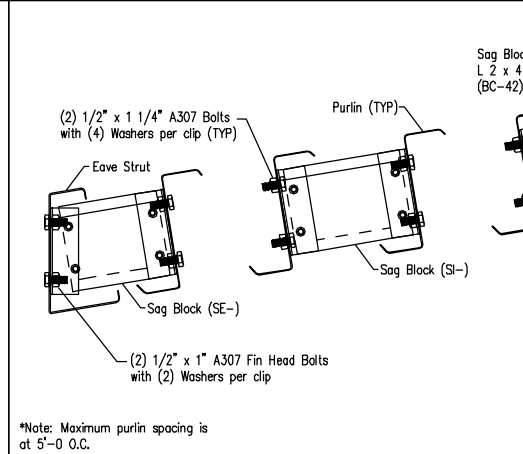
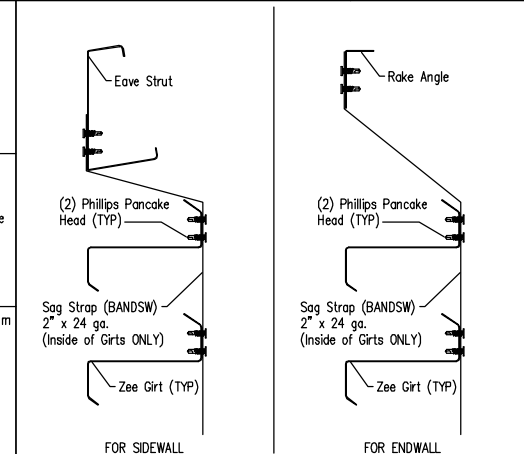
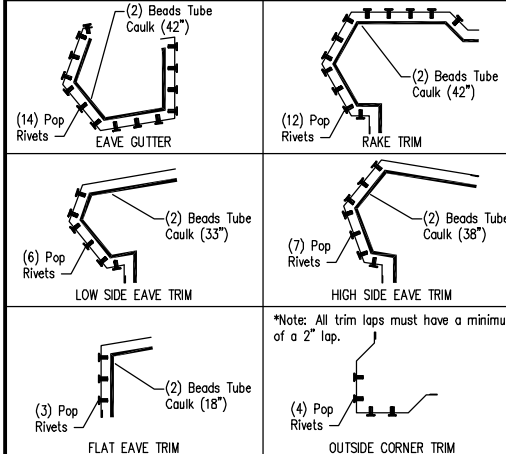
FASTENER LOCATION FOR ROOF PANELS

SCREW_4

FASTENER LOCATION FOR SOFFIT/ROOF LINER PANELS

FASTENER LOCATION FOR SOFFIT/ROOF LINER PANELS

SCREW_5



TRIM LAPS SCREW_10

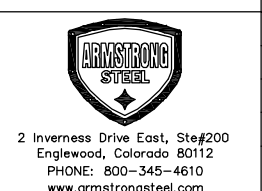
TYPICAL SAG STRAP AT WALLS SCREW_17

TYPICAL SAG BLOCK AT GABLED ROOF < 2:12

TYPICAL SAG BLOCK AT GABLED ROOF < 2:12 SCREW_18



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JOB NO.: 56633	ENG. BY: RA DATE: 4/15/21
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