



Flying Horse North Filing No. 8 Final Drainage Report

April 2026

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Table of Contents

Engineer's Statement.....	1
Developer's Statement.....	1
El Paso County:	1
Final Drainage Report – Flying Horse North.....	2
I. General Purpose, Location and Description	2
a. Purpose and Scope	2
b. DBPS Investigations	2
c. Stakeholder Process.....	3
d. Agency Jurisdictions	3
e. General Project Description.....	3
f. Data Sources	4
g. Applicable Criteria and Standards	4
II. Project Characteristics	5
a. Location in Drainage Basin, Offsite Flows, Size.....	5
b. Compliance with DBPS.....	6
c. Site Characteristics	6
d. Major Drainage Ways and Structures.....	6
e. Existing and proposed land uses.....	7
III. Hydrologic Analysis.....	7
a. Major Basins and Sub-basins	7
b. Major Basin Description	7
c. Existing Subbasin Description	7
d. Proposed Subbasin Description	9
e. Water Quality and Detention Facilities.....	13
f. Methodology	14
IV. Hydraulic Analysis.....	14
a. Major Drainageways	14
b. Storm Sewer Infrastructure and Culvert Pipes	14
V. Environmental Evaluations.....	14
a. Significant Existing or Potential Wetland and Riparian Areas Impacts	14
b. Stormwater Quality Considerations and Proposed Practices.....	14



c.	Permitting Requirements	15
d.	4-Step Process	15
VI.	Drawings	16
VII.	Drainage and Bridge Fees	16
VIII.	Permanent Control Measure Cost Estimates	17
IX.	Summary	17
X.	References	18

Appendices

- A. Maps & Exhibits
- B. Hydrology Calculations
- C. Hydraulic Calculations
- D. Referenced Report Excerpts
- E. Site Drainage Maps



Engineer's Statement

This report and plan for the drainage design of the development, Flying Horse North Filing No. 8, was prepared by me (or under my direct supervision) and is correct to the best of my knowledge and belief. Said report and plan has been prepared in accordance with the *El Paso County Drainage Criteria* Manual and is in conformity with the master plan of the drainage basin. I understand that El Paso County does not and will not assume liability for drainage facilities designed by others. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Richie Lyon, PE Date

State of Colorado No. 53921

For and on behalf of HR Green Development, LLC

Developer's Statement

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Flying Horse Development, LLC

Drew Balsick Date

Vice President

Flying Horse Development, LLC

2138 Flying Horse Club Drive

Colorado Springs, CO 80921

El Paso County:

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volumes 1 and 2 and the Engineering Criteria Manual, as amended.

Joshua J. Palmer, P.E.

Date

County Engineer/ECM Administrator

Final Drainage Report – Flying Horse North

I. General Purpose, Location and Description

a. Purpose and Scope

The Purpose of this Final Drainage Report (FDR) is to identify specific solutions to drainage concerns for onsite and offsite tributary areas resulting from the development of the subdivision to be platted. The FDR is to describe the onsite and offsite drainage patterns, existing and proposed storm infrastructure as it relates to water quality and stormwater detention for any proposed or existing facilities, the planned storm water management for Flying Horse North Filing No. 8.

The Preliminary Drainage Report for Flying Horse North Preliminary Plan and Final Drainage Report for Flying Horse North Filing No. 1 is a combined Preliminary Drainage Report (PDR) and Final Drainage Report (FDR) that was developed by Classic Consulting, latest revision June 2018. The combined PDR/FDR was approved by the County in September of 2018.

A more recent Master Development Drainage Plan (MDDP) was prepared by HR Green Development, LLC. and was approved by the County in September of 2022, entitled *Flying Horse North Master Development Drainage Plan* latest revision date of September 9, 2022. This MDDP also referenced the Classic Consulting report from 2018 for master drainage design of the proposed Filing No. 5 area.

Additionally, a PDR for a Major PUD Amendment for Flying Horse North Subdivision Filing Nos. 6, 7, & 8 (PUDSP252) has been prepared that addresses the Filing No. 8 area for stormwater drainage and infrastructure planning.

The items discussed in this FDR include final plat layout, land uses, and drainage patterns for Flying Horse North Filing No. 8. Included in this report are final hydrologic and hydraulic drainage calculations and design as required for the final design of the development of the public roadway extensions and the fifty (50) single-family residential estate lots included in this filing. This report references the aforementioned reports to compare and contrast findings in the final design to ensure that existing infrastructure and facilities are not negatively impacted by this development.

b. DBPS Investigations

Flying Horse North is split by the Arkansas River Basin and South Platte Basin. Within the South Platte Basin, a majority of the site is within the East Cherry Creek Drainage Basin. 10.23 acres of the site is within the Black Squirrel Creek Drainage Basin, which is a subbasin of the Arkansas River Drainage Basin. A Drainage Basin Planning Study (DBPS) does not currently exist for the East Cherry Creek Drainage Basin. This FDR is consistent with the 2022 MDDP prepared by HR Green Development, LLC. and a recent PDR Amendment for the Major PUD Amendment for Filings 6, 7, and 8 which complies with standard El Paso County regulations regarding drainage within this corridor.

A majority of the Filing No. 8 area, which consists of fifty (50) single-family residential estate lot of 2.5-acres minimum area within the total filing boundary area of 155.58 acres falls within the East Cherry Creek Basin. Three (3) tracts primarily used for drainage will also be platted in this area. The remaining area of Filing No. 8 site, consisting of a Tract B and portions of Lots 38, 39, and 40, is within the Black Squirrel Creek Drainage Basin. The filing primarily consists of the extension of 60' public rights-of-way for Sandbagger Drive from the existing Filing No. 6 termination point of this public roadway, and the extension of public roadway Holmes Road from the temporary roadway termination cul-de-sac of Filing 6 to the southern boundary of

Flying Horse North where a connection to the existing Holmes Road is to be made. Par Court and Wedge Trail are public rural roadways that are proposed to extend off of Holmes Road. Proposed developed areas are provided with water quality and full spectrum detention (FSD) prior to release offsite from the subdivision, except for portions of Lots 38, 39, and 40 (to be discussed in later sections). Much of the developed Filing area is planned to be treated by proposed, private FSD pond, Pond J. This pond will be maintained by the Flying Horse North Metropolitan District. The remaining developed area within this filing, except for a small area that drains directly offsite, is tributary to the existing downstream Irrigation Reservoir (Pond 13) within Filing No. 1.

The basins and sub-basins assessed as a part of this FDR are consistent with the basins represented in the Major PUD Amendment PDR (PUDSP252) in order to show compliance with the previous study and the subdivision as a whole. The Filing No. 8 area is shown in the 2025 PDR proposed hydrology maps within Basins I and J. Intermediate conveyance infrastructure such as natural channels and proposed culverts were preliminarily analyzed in that report, and shall be finalized in the following sections of this report.

c. Stakeholder Process

There are no amendments to the current DBPS.

d. Agency Jurisdictions

Listed below are the jurisdictions that this project will conform to:

El Paso County

Federal Emergency Management Agency

e. General Project Description

Flying Horse North Filing No. 8 is in El Paso County jurisdiction and is located within the larger Flying Horse North subdivision. The overall Flying Horse North development is bordered by Highway 83 to the west, Black Forest Road to the east, Cathedral Pines to the south, and High Forest Ranch to the north. The greater Flying Horse North area contains approximately 1,459 acres within the whole Section 36, Township 11 South, Range 66 West of the Sixth Principal Meridian, and a portion of Section 30 and 31, Township 11 South, and Range 65 West of the Sixth Principal Meridian. Filing No. 8 is within Section 31 and Section 36, Township 11 South, and Range 65 West of the Sixth Principal Meridian and is located south of Filing No. 5 and west of Filing No. 6.

The Filing No. 8 area totals approximately 159.44 acres for the development of fifty (50) single-family residential estate lots and three (3) tracts primarily for drainage use. The remaining areas are 60' public right-of-way for public roadways of Sandbagger Drive, Holmes Road, Par Court, and Wedge Trail. The rights-of-way within this filing are typical of the Flying Horse North right-of-way sections which include the 28' width asphalt paved rural local roadways with 2' gravel shoulders, roadside ditches where applicable, 5' public improvement easements, 10' public utility easements, and any applicable trail easements as needed. There is a 25' regional trail easement to the east of Holmes Road that is to connect to the regional trail system throughout Flying Horse North, specifically connecting to the platted Filing No. 5 regional trail easement.

Filing No. 8 was previously assessed in the 2018 Classic Consulting report with a similar land use plan that included 2-acre single-family residential estate lots and roadways. This report assesses the lots as 2.5-acre

lots. The layout shown in the developed conditions hydrology map of this report and the corresponding construction drawings differs slightly from the approved FDR/PDR with adjusted roadway alignments and lot lines. However, the drainage patterns, typical roadway section, and land use densities are similar. A more recent 2025 PDR for the Major PUD Amendment to amend future Filings 6, 7, & 8 (PUDSP252) was prepared by HR Green Development, LLC. and is referenced in this FDR to demonstrate compliance with criteria and future planning for stormwater infrastructure.

The existing vegetative cover is approximately 90 percent as evidenced by a field survey and aerial imagery. The existing vegetation includes native grasses and weeds, shrubs, and pinyon pine trees. Previous clearing of future planned roadways was done several years ago, and native grass and weeds have covered those areas.

f. Data Sources

Listed Below are the technical resources reviewed in the preparation of this FDR:

El Paso County Drainage Criteria Manual (DCM)

Mile High Flood District

NOAA Atlas 14

NRCS Soil Survey for El Paso County Area, Colorado

FEMA FIRM 08041C0305G and FIRM 08041C0315G (eff. 12/7/2018)

El Paso County Assessor Property Records

Preliminary Drainage Report for Flying Horse North Preliminary Plan and Final Drainage Report for Flying Horse North Filing No. 1 prepared by Classic Consulting – June 2018

Flying Horse North Master Development Drainage Report prepared by HR Green Development, LLC. – latest revision September 9, 2022

Flying Horse North Major PUD Amendment Filings 6, 7, & 8 - Preliminary Drainage Report prepared by HR Green Development, LLC. – latest revision September 2025

Amendment to the Final Drainage Report for Cathedral Pines prepared by Stillwater Engineering – dated August 28th, 2006

g. Applicable Criteria and Standards

Per El Paso County Criteria Manual, flows from the proposed site will be limited to historic flows to maintain the stability of the existing channels within the drainage basins. The final drainage plan follows the Drainage Criteria Manual for El Paso County which refers to the City of Colorado Springs Drainage Criteria Manuals as amended. Criteria within the County and City manuals refer to the Mile High Flood District manuals, particularly for extended detention basin design.

A distinct difference in the 2018 FDR/PDR and the PDR Amendment report (PUDSP252) are the hydrologic methodologies utilized to compute peak runoff values. The 2018 Classic Consulting report utilized the NRCS Curve Number method in order to be consistent with their previous MDDP for the greater Flying Horse North master development. The NRCS Curve Number method was used for Filing No. 1 and the future development of Filing No. 5 for sub-basins that did not exceed 100 acres. Typically, the Rational

Method is used for hydrologic computations when basin analysis is under 100 acres due to the NRCS Curve Number method yielding smaller minor and major storm event peak runoff values. The resultant hydraulics in this report are similar to that of the approved 2018 FDR/PDR on a basin-by-basin basis, however, any differences in calculated stormwater runoff will be discussed. The difference in methodologies between the 2018 report and this report result in larger cumulative stormwater runoff values reported for the minor and major storm events. Due to the more conservative nature of the Rational Method, cumulative peak flow rates are greater than that of the 2018 FDR/PDR for the minor and major storm events for downstream design points.

HR Green has discussed this discrepancy in hydrologic methodology with El Paso County engineering staff and it has been expressed that the chosen method for hydrologic computations is the Rational Method for this report to ensure sound design of the storm infrastructure for Filing No. 6 including swales, channels, culvert pipes, inlets, and roadway capacities. Due to the use of the NRCS Curve Number method in the 2018 FDR/PDR, the peak runoff values in this report are larger than that of the approved 2018 FDR/PDR. To complete a fair assessment of the impacts downstream of the site, existing hydrology calculations have been completed and included in Appendix B. There are no anticipated negative impacts to downstream offsite infrastructure because of this development as all other drainage parameters remain consistent with the 2018 FDR/PDR.

II. Project Characteristics

a. Location in Drainage Basin, Offsite Flows, Size

The majority of the Flying Horse North Filing No. 8 area is located within the East Cherry Creek Basin. There is not a current planning study of the drainage basin, but generally it slopes from southwest to northeast. The basin eventually flows into the South Platte River.

Within the portion of the East Cherry Creek Basin investigated with this Report, two major drainage basins have been designated within the 2025 PDR for the Major PUD Amendment (PUDSP252), Basins I and J. Basin I is tributary to the Filing No. 1 Irrigation Reservoir referred to as Pond 13 and Design Point I9. Basin J is tributary to Pond J in the Major PUD Amendment.

The remaining portion of the site that drains within the Black Squirrel Creek Basin will be discussed in the sections below.

This Filing No. 8 FDR utilizes similar tributary areas, runoff coefficients (when comparing the NRCS Curve Number method and the Rational Method), and percent imperviousness for respective sub-basins and downstream detention facilities compared to 2018 Classic Consulting FDR/PDR and the PDR Amendment (PUDSP252). Any deviation in the sub-basin area, coefficient, or percent imperviousness is due to slight roadway alignment adjustments for the final design as compared to the preliminary layout in the 2018 report. Any change in the peak runoff numbers as compared to the 2018 report is due to the change in hydrologic computation methodology as discussed in a previous section of this report. Due to these differences in the computational methodology between the previously approved 2018 FDR and the values being reported in this report, additional analysis of existing conditions has been completed on the entire site. The existing conditions flow values have been added to Appendix B.

The runoff within the "I" Basins of the Filing No. 8 site will be captured and treated in Pond 13 in Filing No. 1. The runoff within the "J" Basins is to be captured and treated within proposed private Pond J.

It is important to note that while there is an increase in peak runoff for these basins as compared to the 2018 report, there is a discrepancy in methodology. The difference in flow rates provided in the table may have significant differences as the basin area being analyzed in one report may be significantly bigger than in the other report. The proposed improvements will be designed using the updated values and pond sizing, which relies on a historic model, in the CUHP / SWMM to ensure that the developed flow rates are less than the historical flow rates. It is shown in the PDR for the Major PUD Amendment that the total flowrates being released off-site into Cherry Creek basin are being reduced overall based on preliminary stormwater planning, and this FDR complies with that preliminary report.

b. Compliance with DBPS

This FDR is in general conformance with the current drainage flows of the East Cherry Creek and Black Squirrel Creek Basin. Flying Horse North will construct multiple full spectrum detention facilities to limit the effects of development and mimic natural flow patterns. Flying Horse North Filing No. 8 development will follow historic drainage patterns and utilize the existing natural swales throughout the area for conveyance of stormwater runoff toward respective detention facilities, including a proposed private FSD pond (Pond J) and the existing Irrigation Reservoir (Pond 13).

c. Site Characteristics

Per the NRCS web soil survey, the site is made up entirely of Type B soils. Filing No. 8 is within the East Cherry Creek Basin which consists of Peyton sandy loam and Peyton-Pring complex. See Appendix A for the NRCS soil map.

The current ground cover in Filing No. 8 is short to mid-grass prairie grasslands and former farmland which consists of non-native weeds and grasses. This portion of the site has very few, if any, trees and a minimal number of shrubs are found on the site.

d. Major Drainage Ways and Structures

No major drainage ways exist within the development; however, small tertiary tributaries are adjacent to the site currently and function to convey flows to unnamed tributaries. These informal drainage ways are assessed within this report for stormwater runoff capacity and water surface elevations during the 100-year event. Roadside swales are included as a part of the typical roadway section for cut conditions and are assessed within sub-basins to ensure that swale and culvert pipe capacities are met and do not result in excessive pooling in the roadway sections per code.

The existing minor drainage channels within the site are planned to be maintained to the maximum extent possible. These will continue to be used for conveyance of storm drainage flows. The limits of construction and disturbance plan for no significant earthwork alterations to the existing minor drainage channels that would affect the drainage patterns or capacity of the sections throughout the filing as they are proven to have sufficient capacities for their respective tributary areas and to maintain the natural features of the site including existing trees and vegetation.

Drainageways of note including roadside swales are described within this report with parameters to demonstrate compliance with swale design criteria and capacities. Culvert pipes are sized to convey upstream flow under proposed roadways and maintain historic drainage patterns.

e. Existing and proposed land uses

The existing Filing No. 8 area is open rangeland within a forested area consisting of sparse native grasses, weeds, and pinyon pine trees as well as barren pervious soil. The 2018 Classic Report assumed 2-acre single-family residential estate lot development with the same percent imperviousness within the filing area. This report includes the final design layout of fifty (50) 2.5-acre lots and two (2) tracts primarily used for drainage with rural roadway sections, which is in accordance with the PDR Amendment (PUDSP252). Any deviations in basin areas, land use acreages, and resultant composite coefficients are shown within this report and demonstrated to meet downstream stormwater runoff and volume capacities for proposed and existing facilities.

III. Hydrologic Analysis

a. Major Basins and Sub-basins

b. Major Basin Description

Per FEMA FIRM 08041C0315G (eff. 12/7/2018), there are no FEMA Floodplains within this Filing.

The site has been divided into several major drainage basins where each basin is tributary to a full spectrum detention pond facility (except for Basin OS-1 and OS-3 to the southwest, located in the Black Squirrel Creek Drainage Basin). These basins and associated sub-basins are described in more detail in the next section of this report.

c. Existing Subbasin Description

The existing conditions for Filing No. 8 have been analyzed and the delineated basins presented in the Existing Conditions Map in Appendix F are described as follows:

Existing Basin I3: 9.51 acres, 2.5 acre residential ($Q_5 = 6.3$ cfs, $Q_{100} = 26.0$ cfs)

Runoff from 2.5 acre residential lot areas within this basin initially sheet flows southwest over existing topography through Filing 5 and eventually concentrates within the existing channel that conveys flows from the southeast to the northwest, culminating at design point I9. The runoff is released to Pond 13 as concentrated flow into a riprap apron. Slopes in this basin average between 2% and 3:1.

Existing Basin I4: 2.34 acres, undeveloped, roadway ($Q_5 = 1.5$ cfs, $Q_{100} = 6.0$ cfs)

Runoff generated in this basin sheet flows from the south to north and is collected in existing roadside ditches (West side of Street Section I4) on the south side of Sandbagger Drive to design point I4 where drainage is directed east through a proposed public 18-inch RCP culvert to Basin I5.

The street channel sections are lined with North American Green P300 erosion control blanketing (or approved equal). Calculations for the channel lining and culvert hydraulics can be found in Appendix C.

Existing Basin I5: 20.40 acres, undeveloped, roadway ($Q_5 = 15.0$ cfs, $Q_{100} = 51.9$ cfs)

Basin I5 is defined by the Flying Horse North Foundation project, reference final drainage report (PPR258). Runoff generated in this basin will sheet from southeast to northwest and is collected in roadside ditches (east side of Street Section I4 and south side of Street Section I6) that culminate with flows from design point I4 at rates of $Q_5=16.4$, $Q_{100}=57.4$ cfs at design point I5. Culminated flows are conveyed north via proposed 42" public RCP culvert which outfalls onto a proposed rip rap pad and subsequently an existing tertiary channel (Channel Section I1).

The street channel sections are lined with North American Green P300 erosion control blanketing (or approved equal). Calculations for the channel lining and culvert hydraulics can be found in Appendix C.

Existing Basin I6: 1.69 acres, undeveloped ($Q_5 = 0.5$ cfs, $Q_{100} = 3.8$ cfs)

Runoff generated in this basin sheet flows from southeast to northwest over existing topography out of the Filing 8 boundary. The flows are funneled through design point I6 before continuing through the existing golf course. The runoff flows through an existing swale on the golf course to the northwest, culminating at design point I9, and ultimately outfalls to the existing Irrigation Reservoir, Pond 13 via existing rip rap run down.

Existing Basin I7: 23.02 acres, undeveloped ($Q_5 = 6.0$ cfs, $Q_{100} = 44.2$ cfs)

Basin I7's runoff sheet flows from south to north over existing topography and crosses the Filing 8 boundary and into the existing golf course swale, ultimately draining to the irrigation reservoir.

Existing Basin I8: 6.38 acres, undeveloped ($Q_5 = 1.8$ cfs, $Q_{100} = 13.6$ cfs)

Runoff generated in this basin sheet flows over existing topography to the west out of the filing boundary and onto the golf course through design point I8. The runoff flows through the golf course to the northwest and outfalls to the existing detention pond, Pond 13.

Existing Basin I9: 0.54 acres, undeveloped ($Q_5 = 0.2$ cfs, $Q_{100} = 1.4$ cfs)

Runoff generated in this basin sheet flows into the channel within the basin, which flows northwest and ends at a riprap outfall into Irrigation Reservoir Pond 13. The basin accepts flows from Basins I4, I5, I6, and I7, totaling $Q_5=27.9$ and $Q_{100}=126.5$ at the design point.

Existing Basin J3: 30.10 acres, undeveloped, 5.0 acre residential, 2.5 acre residential ($Q_5 = 11.5$ cfs, $Q_{100} = 59.1$ cfs)

Runoff generated in this basin includes offsite flow from the southeast to the northwest onto the site. Drainage sheet flows across existing topography through native grass and ultimately concentrates in an existing natural tertiary swale at design point J3. The swale continues northwest into Basin J5. Slopes in this basin average between 5% and 9%. There is an existing stock pond at the design point that is being removed in the proposed condition.

Existing Basin J4: 92.88 acres, undeveloped / 5.0 acre single family residential ($Q_5 = 19.5$ cfs, $Q_{100} = 142.8$ cfs)

Runoff generated in this basin includes offsite flow from the south of the property flowing north onto the site. Drainage sheet flows across existing topography through native grass and ultimately concentrates in an existing natural tertiary swale at design point J4. Slopes in this basin average between 2% and 33%. There is an existing stock pond within this basin that is being removed in the proposed condition.

Existing Basin J5: 42.29 acres, undeveloped ($Q_5 = 10.1$ cfs, $Q_{100} = 71.9$ cfs)

Runoff generated in this basin sheet flows across existing topography to the center of the basin, through native grass until concentrating within the existing natural drainage channel that traverses the center of the basin and flows to the northwest. Slopes in this basin average between 3% and 16%. Flows from this basin combine with runoff of basin J3 at rates of 11.5 and 59.1 cfs in the 5 and 100 year events, respectively.

Design Point J5 was established to allow for comparison of flow rates at the proposed Pond J Location. This design point represents the culmination of flows from Design Points J4 and J5 at the offsite outfall onto the golf course. Combined flows at this design point are 38.5 and 259.0 cfs in the 5 and 100 year events, respectively.

Existing Basin OS-1. 23.50 acres, undeveloped, golf course ($Q_5 = 7.4$ cfs, $Q_{100} = 48.2$ cfs)

Runoff generated in this basin sheet flows over undeveloped land and golf course area and flows directly to the design point at the southern edge of the site. Basin OS-1 is a portion of a larger basin, OS-B12, previously studied with the Amendment to the Final Drainage Report for Cathedral Pines Filing 2. This is further discussed in the Proposed Basin OS-1 description.

Existing Basin OS-2. 1.55 acres, undeveloped, golf course ($Q_5 = 0.5$ cfs, $Q_{100} = 3.4$ cfs)

Runoff generated in this basin sheet flows over undeveloped land and golf course area to the design point at the north edge of the westernmost portion of the site. Runoff continues north from the design point and flows through channels within the existing golf course and ultimately drains to the existing Irrigation Reservoir, Pond 13.

d. Proposed Subbasin Description

The following design points are presented on the Developed Conditions Drainage Map and are described as follows:

Proposed Basin I3: 9.51 acres, residential (2.5 acre lots) ($Q_5 = 6.3$ cfs, $Q_{100} = 26.0$ cfs)

Runoff from 2.5 acre residential lot areas within this basin initially sheet flows southwest over existing topography through Filing 5 and eventually concentrates within the existing channel that conveys flows from the southeast to the northwest, culminating at design point I7. The runoff is released to Pond 13 as concentrated flow into a riprap apron. Slopes in this basin average between 2% and 3:1.

Proposed Basin I4: 2.94 acres, residential (2.5 acre lots) ($Q_5 = 1.8$ cfs, $Q_{100} = 7.4$ cfs)

Runoff generated in this basin will sheet flow from the south to north and is collected in existing roadside ditches (West side of Street Section I4) on the south side of Sandbagger Drive to design point I4 where drainage will be directed east through a proposed public 18-inch RCP culvert to Basin I5.

Proposed Basin I5: 20.40 acres, residential (2.5 acre lots) ($Q_5 = 15.0$ cfs, $Q_{100} = 51.9$ cfs)

Basin I5 is defined by the Flying Horse North Foundation project, reference final drainage report (PPR258). Runoff generated in this basin will sheet from SE to NW and is collected in roadside ditches (East side of Street Section I4 and south side of Street Section I6) that culminate with flows from design point I4 at rates of $Q_5=16.7$, $Q_{100}=58.7$ cfs at design point I5. These street sections are lined with North American Green TMAX erosion control blanketing (or approved equal). Culminated flows are conveyed north via proposed 42" public RCP culvert which outfalls onto a proposed rip rap pad and subsequently an existing tertiary channel (Channel Section I1).

Proposed Basin I6: 1.69 acres, residential (2.5 acre lots) ($Q_5 = 1.1$ cfs, $Q_{100} = 4.6$ cfs)

Runoff generated in this basin will sheet flow over existing topography from SE to NW out of the filing 6 boundary and eventually into an existing golf course pond. The flows are funneled through design point I6 before it flows to the NW and onto the golf course. The runoff will flow through the golf course to the

northwest, culminating at design point I7, and ultimately outfalls to the existing Irrigation Reservoir, Pond 13 via existing rip rap run down.

Proposed Basin I7: 18.90 acres, residential (2.5 acre lots) ($Q_5 = 10.5$ cfs, $Q_{100} = 43.2$ cfs)

Basin I7's runoff sheet flows from south to north over 2.5 acre residential lot area out of the Filing 8 boundary into the existing golf course. The runoff flows through the existing golf course to the northwest and outfall to the existing Irrigation reservoir, Pond 13, via existing rip rap rundown.

Proposed Basin I8: 5.82 acres, residential (2.5 acre lots) ($Q_5 = 3.6$ cfs, $Q_{100} = 14.8$ cfs)

Runoff generated in this basin sheet flows over existing topography to the west out of the filing boundary and onto the golf course through design point I8. The runoff will flow through the golf course to the northwest and outfall to the existing detention pond, Pond 13.

Flows have decreased in the proposed condition compared to the PDR Amendment (PUDSP252) due to variance in the basin geometry delineation, but this basin was previously analyzed under the same hydrologic characteristics.

Proposed Basin I9: 5.82 acres, residential (2.5 acre lots) ($Q_5 = 3.6$ cfs, $Q_{100} = 14.8$ cfs)

Runoff generated in this basin sheet flows over existing topography to the west out of the filing boundary and onto the golf course through design point I8. The runoff will flow through the golf course to the northwest and outfall to the existing detention pond, Pond 13.

Basins I3 through I7 are all tributaries to the I7 design point and contribute to its cumulative Q_5 and Q_{100} calculation of 33.2 and 127.5 cfs, respectively. The runoff will flow through the golf course to the northwest and outfall to the existing Irrigation reservoir, Pond 13, via existing rip rap rundown.

Flows are marginally increased from the PDR Amendment's reported 33.1 and 126.2 cfs at the equivalent design point I7 in the 5 and 100 year events, respectively, due to the addition of Basin I9. Therefore, the assumptions from said report regarding the capacity of Pond 13 and any intermediate infrastructure are maintained.

Proposed Basin J1: 25.63 acres, residential (2.5 & 5 acre lots) ($Q_5 = 13.7$ cfs, $Q_{100} = 61.6$ cfs)

Runoff generated in this basin includes offsite flow from the south of the property flowing north onto the site. Drainage ultimately sheet flows across existing topography and in proposed roadside ditches and collects at design point J1 where it is conveyed under Holmes Road through a public 36-inch RCP culvert to Basin J6.

Culvert hydraulics are represented in Appendix C.

Proposed Basin J2: 25.08 acres, residential (2.5 acre & 5.0 acre lots) ($Q_5 = 13.2$ cfs, $Q_{100} = 58.4$ cfs)

Runoff generated in this basin includes offsite flow from the south and east of the property that flows northwest onto the site. Drainage sheet flows across existing topography and proposed roadway until concentrating in the proposed roadside ditch on the south side of Holmes Road. The roadside ditch conveys runoff to the low point at design point J2 where the flows are routed under Holmes Road through a proposed public 42-inch RCP culvert to Basin J6.

Culvert hydraulics are represented in Appendix C.

Proposed Basin J3: 34.37 acres, undeveloped, residential (2.5 & 5 acre lots) residential ($Q_5 = 15.2$ cfs, $Q_{100} = 70.2$ cfs)

Runoff generated in this basin includes offsite flow from the southeast of the property flowing northwest onto the site. Drainage sheet flows across existing topography and proposed roadways until concentrating within the proposed roadside ditch. Flows within the roadside ditch are conveyed to the low point at design point J3 where they are conveyed under Holmes Road via a proposed public 42-inch RCP culvert to Basin J9.

Section I-I of the channel directly downstream of the culvert was analyzed and has been determined to need P300 erosion control blanketing. Channel section and culvert hydraulics can be found in Appendix C.

Proposed Basin J4: 1.22 acres, residential (2.5 acre lots) ($Q_5 = 0.9$ cfs, $Q_{100} = 3.5$ cfs)

Runoff generated in this basin sheet flows over 2.5 acre residential lot area from southwest to northeast and collects in roadside ditches ultimately funneling to design point J4. From this design point, it is conveyed to Basin J6 via proposed 18" public RCP.

Proposed Basin J5: 2.53 acres, residential (2.5 acre lots) ($Q_5 = 1.6$ cfs, $Q_{100} = 6.4$ cfs)

Runoff generated in this basin sheet flows over 2.5 acre residential lot area from southwest to northeast and collects in roadside ditches along Wedge Trail, ultimately funneling to design point J5. From this design point, it is conveyed to Basin J6 via proposed 18" public RCP.

Proposed Basin J6: 33.02 acres, undeveloped, ($Q_5 = 18.5$ cfs, $Q_{100} = 76.2$ cfs)

Runoff generated in this basin sheet flows over 2.5 acre residential lot area from southwest to northeast and collects in roadside ditches ultimately funneling to design point J5. From this design point, it is conveyed to Basin J6 via proposed swale that ties into the existing downstream channel.

Design Point J6 represents the culmination of flows from design points J1, J2, J4, J5, J6, and J7 at rates of 46.9 and 201.5 cfs in the 5 and 100 year events, respectively. Section K-K was analyzed with this combination of flows, and it was determined that P300 channel lining was necessary throughout this section. Channel lining and culvert hydraulics calcs can be found in Appendix C.

Proposed Basin J7: 1.83 acres, residential (2.5 acre lots) ($Q_5 = 1.1$ cfs, $Q_{100} = 4.7$ cfs)

Runoff generated in this basin sheet flows over roadway and concentrates in roadside ditches flowing northwest along Par Court, ultimately funneling into a proposed swale that redirects flows west to basin J6. The channel segment between J6 and J7 shall be lined with P300 erosion control blanketing per the results of Section K-K.

Proposed Basin J8: 2.05 acres, residential (2.5 acre lots) ($Q_5 = 1.3$ cfs, $Q_{100} = 5.5$ cfs)

Runoff generated in this basin sheet flows from east to west over 2.5 acre residential lot area directly into proposed Pond J.

Proposed Basin J9: 26.36 acres, residential (2.5 acre lots) ($Q_5 = 13.8$ cfs, $Q_{100} = 56.9$ cfs)

Runoff generated in this basin sheet flows over 2.5 acre residential lot area to the center of the basin, ultimately funneling into a ditch that redirects flows west towards the design point, where flows are conveyed into Pond J via a district culvert pipe.

Channel Section J5 directly downstream of Section I-I was analyzed and has been determined to need P300 erosion control blanketing. Channel section hydraulics can be found in Appendix C.

Proposed Basin J10. 2.25 acres, residential (2.5 acre lots) ($Q_5 = 1.4$ cfs, $Q_{100} = 5.9$ cfs)

Runoff generated in this basin sheet flows southeast to northwest over 2.5 acre residential lot area to the roadside ditch alongside Sandbagger Drive at the design point, ultimately being conveyed west through basin J11.

Channel Section J1 was analyzed and has been determined to need P300 erosion control blanketing. Channel section hydraulics can be found in Appendix C.

Proposed Basin J11. 3.25 acres, residential (2.5 acre lots) ($Q_5 = 1.7$ cfs, $Q_{100} = 7.1$ cfs)

Runoff generated in this basin sheet flows over roadway before concentrating in the roadside ditches along Sandbagger Drive which flow northwest towards the design point. From the design point, flows are directed southwest in an earthen channel (Section H-H), where a culvert conveys runoff into the forebay of proposed Pond J. Pond access runs parallel to this channel.

This Channel Section was analyzed and has been determined to need P300 erosion control blanketing. Channel section hydraulics can be found in Appendix C.

Proposed Basin J12. 4.94 acres, residential (2.5 acre lots) ($Q_5 = 3.3$ cfs, $Q_{100} = 13.4$ cfs)

Runoff generated in this basin sheet flows over 2.5 acre residential lot area from east to west. Approximately 50% of this basin is tributary to Section H-H, while the remaining portion sheet flows directly into proposed Pond J.

Proposed Basin J13. 6.79 acres, pond ($Q_5 = 1.9$ cfs, $Q_{100} = 14.2$ cfs)

Runoff generated in this basin sheet flows over pond area to the center of the basin, ultimately funneling culminated flows from design points J6, J8, J9, J11, J12, and basin J13. Combined flows at this design point are 64.9 and 280.4 cfs in the 5 and 100 year events, respectively.

Proposed Basin OS-1. 23.46 acres, residential (2.5 acre lots), golf course ($Q_5 = 10.7$ cfs, $Q_{100} = 53.7$ cfs)

Runoff generated in this basin sheet flows over 2.5 acre lot residential and golf course area to the design point at the southern edge of the site. Basin OS-1 is a portion of a larger basin, OS-B12, previously studied with the Amendment to the Final Drainage Report for Cathedral Pines Filing 2. Basin OS-B12 was analyzed with runoff coefficients corresponding to one-third acre lots with 30% imperviousness, while the proposed Basin OS-1 has an overall imperviousness of 6.9%. Therefore, it has been determined that existing downstream drainage ways and detention ponds within the Cathedral Pines Subdivision are not negatively impacted by the development because the proposed development within Basin OS-1 follows existing drainage patterns and is a lower imperviousness than anticipated by the previous study. Lots immediately south of Basin OS-1 are not anticipated to experience any negative impacts due to any minimal increase in flows due to future large lot home site development. Development of homesites is excluded from water quality treatment requirement per ECM App I.7.1.B.5.

Proposed Basin OS-2. 1.55 acres, undeveloped, golf course ($Q_5 = 0.5$ cfs, $Q_{100} = 3.4$ cfs)

Runoff generated in this basin sheet flows over undeveloped land and golf course area to the design point at the north edge of the westernmost portion of the site. Runoff continues north from the design point and

flows through channels within the existing golf course and ultimately drains to the existing Irrigation Reservoir, Pond 13.

e. Water Quality and Detention Facilities

There is an existing stormwater facility, Pond 13, located downstream north of this filing and is west of Filing No. 5. Pond 13 is designed with Flying Horse North Filing No. 1 (SF181) as a retention pond using the Mile High Flood District (MHFD) Detention workbook. The reservoir facility provides detention and water quality through a separate stormwater detention and water quality component in the form of a private concrete outlet box with an orifice plate and 30-inch RCP outlet pipe. The stormwater outlet box is constructed outside of the reservoirs dam embankment. The total tributary area contributing to Pond 13 as identified in *The Irrigation Reservoir Embankment Design Report* by Classic Consulting is 366.80 acres and of that area, 59.15 acres comes from within the Filing No. 8 boundary. The contributing area to Pond 13 in the Classic Consulting design report (corresponding to basins delineated in this report) included 2-acre residential land use, so an overall imperviousness of 14% was calculated (using linear interpolation between a 1 acre and 2.5 acre lot at 20% and 11% imperviousness, respectively). The area contributing to the pond from Filing No. 8 contains residential land use for 2.5 acre lots from this filing, 2.5 acre lots from Filing 6, and the existing Foundation site making the calculated imperviousness 13.41%. While the Filing No. 8 area is an addition to the 2016 PUD and its drainage planning, the imperviousness and its generated stormwater is minimal and falls within the capacity of downstream Pond 13.

Pond J, owned and maintained by the Flying Horse North Metropolitan District, provides full spectrum detention for 169.32 acres of the site within the J basins at an overall imperviousness of 9.39%. The proposed pond provides the required volume stages for Water Quality Capture Volume (WQCV), Excess Urban Runoff Volume (EURV), and the 100-year stage (flood control volume). The pond includes the required infrastructure such as concrete forebays, riprap armored emergency spillway, concrete trickle channels, and a 2.5-foot depth micro-pool attached to the outlet structure. The pond includes 15'-20' width maintenance paths with vehicular access to the bottom of pond to access forebays and outlet structures for continued maintenance. The pathways have access from the public right-of-way and proper turning radii and longitudinal and cross slopes for a maintenance vehicle. The pond includes 1.0-foot of freeboard to the emergency spillway berm of the pond with the crest elevation at or above the 100-year water surface elevation. The spillways are sized with a trapezoidal weir for the 100-year inflow with rip-rap prescribed for the outflow velocity as energy dissipation. The peak outflow from the district 60" RCP outlet pipe is 198.9 cfs in the 100-year storm event. The emergency outfall and outlet pipe from Pond J are armored for erosion protection and outflows follow existing drainage patterns, running through natural tertiary swales and through the golf course to the north. The downstream outfall location experiences decreased 100-year flows in the proposed condition (198.9 cfs) compared to the existing condition (259.2 cfs). Therefore, Pond J has a suitable outfall.

Basin OS-1 consists of 23.46 acres that is tributary to an offsite pond that was constructed with the Cathedral Pines Subdivision. Detention is not required for the basin as the proposed impervious area tributary to the off-site pond is less than the impervious area assumed for design of the Cathedral Pines Subdivision storm infrastructure. Future home site development within this basin is excluded from water quality via the ECM large lot exclusion, and

Pond imperviousness calculations are shown in the Proposed Rational Calcs in Appendix B. Pond hydraulics for proposed Pond J are provided in Appendix D.

f. Methodology

Design rainfall was determined utilizing Table 6-2 from the City of Colorado Springs Drainage Criteria Manual to determine the 5-year and 100-year rainfall values for the 1-hour events. The 1-hour rainfall depths are 1.5 and 2.52 in/hr respectively.

The proposed development will consist of fifty (50) 2.5-acre single-family residential estate lots which are assumed at a percent imperviousness of 11% per the County ECM Table 3-1 Typical Values of Percent Impervious within Appendix L of the ECM which provides guidance for larger rural lot developments. Existing golf course areas are to remain undisturbed and utilize a land use category of “lawn” with a percent imperviousness of 2% per the County ECM Table 6-6 land use table. Composite coefficients, rainfall intensities, and runoff flow rates are calculated on a Rational Method spreadsheet and provided within the Appendix. As discussed previously, the Rational Method used in this report will result in higher peak flow rates for the minor and major storm events as compared to the 2018 Classic Consulting FDR/PDR which utilized the NRCS Curve Number Method. Design points within Filing No. 5 are designed per the findings of this report and existing Filing No. 1 storm infrastructure and design points are to remain as-is.

Reference to the PDR Amendment set of calculations and spreadsheets is included to demonstrate compliance and consistency with the previously approved report which anticipated similar land uses and basin acreages tributary to existing and proposed stormwater facilities.

IV. Hydraulic Analysis

a. Major Drainageways

There are no major drainageways that exist within the development of Filing No. 8; however, small tertiary tributaries are within the site currently and function to convey flows to unnamed tributaries. These tertiary drainage ways are analyzed within this report to assess the water surface elevation within the swales during the 100-year storm event and determine buildability of lots adjacent to these sections. Roadside swales are to be constructed at a minimum to meet the typical roadway section (4:1 for 10' and 3:1 for 9' resulting in a total swale depth of 2.5'). The roadside swales are assessed along the roadways that capture sub-basins and result in cumulative flow.

b. Storm Sewer Infrastructure and Culvert Pipes

The Filing No. 8 development consists of rural development with fifty (50) 2.5-acre single family residential estate lots and rural roadway sections with roadside swales. The storm infrastructure within these areas consist of public culvert pipes for roadway crossings and consideration for future public culvert pipes for future driveways for each lot. Culvert calculations and graphics are provided within the Appendix of this report to demonstrate culvert capacities and show any roadway/driveway overtopping as a result of peak flows. The culverts are designed to have full capacity of the minor (5-year) storm event and a maximum of 4" of roadway or driveway pooling during the major (100-year) storm event.

V. Environmental Evaluations

a. Significant Existing or Potential Wetland and Riparian Areas Impacts

There are no significant impacts to potential wetland and riparian areas with this report.

b. Stormwater Quality Considerations and Proposed Practices

There is one proposed full spectrum stormwater pond associated with this filing. The vast majority of Filing No. 8 is tributary this pond, Pond J, while the remaining majority is tributary to the Filing No. 1 Irrigation Reservoir Pond 13 which provides water quality and detention. A small portion of the site at the southwest drains directly offsite following existing drainage patterns through natural drainage ways. This area has a proposed imperviousness less than what was planned for the area in the Cathedral Pines Subdivision.

On site practices for the estate home includes direct discharge of roof and hardscape runoff to the surrounding landscaped areas. This would include discharge of the gutters onto landscape areas vs. directly connecting to storm sewer and as discussed above as well using natural ditches and swales where it is logical and makes sense to convey stormwater in lieu of storm sewer piping.

c. Permitting Requirements

When work infringes upon the wetlands or floodplain a 404 Permit will be required. If the work within the waterways is minimal, it will likely be covered under a nationwide 404 permit; it is however possible that an individual permit will be required.

The Colorado Department of Public Health and Environment will require permits for any disturbance that exceeds 1 acre of land. Should groundwater be encountered, a dewatering permit will also be required.

El Paso County will require an Erosion and Stormwater Quality Control Permit, and any other construction permits required to complete the construction of the site.

Should development occur which affects the floodplain, FEMA will require a permit for work within the floodplain prior to the commencement of any construction or development within any special flood hazard area (SFHA). If the infrastructure is to be installed within the channel the designer shall route the design through the proper FEMA channels whether that be with a no rise certification or via the CLOMR/LOMR process should a more major improvement within the floodplain be proposed. At this time the project does not propose any direct development within the floodplain.

d. 4-Step Process

In accordance with the Engineering Criteria Manual I.7.2.A and DCM V2, this site has implemented the four-step process to minimize adverse impacts of urbanization. The four-step process includes reducing runoff volumes, stabilizing drainageways, treating the water quality capture volume, and considering the need for Industrial Commercial BMPs.

Step 1 – Reducing Runoff Volumes: This filing includes the land use categories of 2.5-acre single-family residential and lawn (golf course area). Both land uses have relatively minor imperviousness and runoff coefficients. The developed areas for the homes as designated by pad areas on the plans are disbursed with open land areas of vegetation and trees between which provide runoff reduction into the pervious soil.

Step 2 – Stabilize Drainageways: The existing tertiary drainage ways are assessed for stormwater runoff capacity, velocity, and shear stress. Any altered drainage ways will be designed in a manner that provides water quality benefits through infiltration and the removal of pollutants via phytoremediation. Vegetation and/or matting will also be selected to stabilize the drainage ways by reducing the velocity of flows and decreasing any scour. These improvements help stabilize drainageways and minimize erosion and sediment runoff. Roadside ditches are stabilized swales by way of compaction per the roadway typical section and are also prescribed any required seeding, erosion control blanketing, and/or matting.

Step 3 – Provide WQCV: Runoff from this development is treated through capture and slow release of the WQCV via detention and sediment ponds that are designed per current El Paso County DCM V2 and the MHFD. Existing Pond 13 provides WQCV for its tributary basins, including the “I” Basins in this filing. Disturbed areas will be routed through existing and proposed detention ponds and outfall to the East Cherry Creek Basin. Areas that fall under the “large lot” exclusion I.7.1.B.5 of the El Paso County ECM have been proven to be excluded from WQCV requirements. While water quality is not required for these areas, it is understood that runoff from these areas will still be routed through the existing detention ponds. The areas that fall under the exclusion under I.7.1.B.5 of the El Paso County ECM may not exceed 10 percent imperviousness unless a study specific to the watershed and/or MS4 shows that expected soil and vegetation conditions are suitable for infiltration/filtration of the WQCV for a typical site, and the permittee accepts such study as applicable with the MS4 boundaries. The maximum total lot impervious covered under this exclusion shall be 20 percent. This exclusion does not include grading related to roadway construction.

Step 4 – Consider the need for Industrial and Commercial BMP’s: A site specific storm water quality and erosion control plan and narrative will be prepared with subsequent land use approvals prepared in conjunction with the report prior to any construction. Site specific temporary source control BMPs as well as permanent BMPs are detailed in this plan and narrative. Guidelines detailed in the El Paso DCM V2 4.2 pertaining to the covering and storage handline and spill containment and control shall be followed as necessary. This filing does not contain any commercial or industrial land use.

VI. Drawings

Please refer to the appendices for the Vicinity Map, FEMA Floodplain Map, NRCS Soils Map, hydrology and hydraulic calculations, and drainage basin maps. Reference materials from previously approved reports are included in the Appendix.

VII. Drainage and Bridge Fees

The East Cherry Creek Basin does not currently have a Drainage Basin Fee.

The following fees for the Filing No. 8 platted area within the Black Squirrel Creek Basin are due prior to platting:

The fees are calculated using the following impervious acreage method approved by El Paso County. The acreage for Flying Horse Filing No. 8 within the Black Squirrel Creek Basin is 13.50 acres. This area is made up of 2.5 acre lots, undeveloped area, and golf course area, which has a combined imperviousness of 6.9%. The acreage of imperviousness is calculated below:

2.5 ac. Lots (including roads and tracts)

13.50 ac. @ 6.9% imperviousness = 0.93 Impervious ac.

The following calculations are based on the 2026 drainage/bridge fees for the Black Squirrel Creek Drainage Basin:

Filing 8 Fees:

Bridge Fees

\$780.00 x 0.93 ac. = \$725.40

Drainage Fees

$\$12,387.00 \times 0.93 \text{ ac.} = \$11,519.91$

The development qualifies for a 25% reduction in the drainage fee per EPC Land Development Code 8.5.5.C.3.c.i as the area of the subdivision within the Black Squirrel Creek Basin consists entirely of 2.5 acre or larger lots.

$\$11,519.91 \times 0.75 = \$8,639.93$

Filing 8 Fees Total:

Bridge Fees (\$725.40) + Drainage Fees (\$8,639.93) = **\$9,365.33**

VIII. Permanent Control Measure Cost Estimates

There is one (1) private full-spectrum detention pond within Filing No. 8 referred to as Pond J. A cost estimate of the construction and infrastructure for each respective pond is provided below and reflected in the Financial Assurances Estimate for the filing.

FHN F8 Pond J Cost Estimate				
Item	Unit	Qty.	Unit Estimate	Total Estimate
Outlet Structure & Micropool	EA	1	\$37,500.00	\$37,500.00
Forebay	EA	3	\$22,500.00	\$67,500.00
Concrete Trickle Channel	LF	920	\$105.00	\$96,600.00
Overflow Rip Rap Spillway	CY	400	\$44.00	\$17,600.00
Forebay Rip Rap	CY	60	\$47.00	\$2,820.00
Overflow Spillway Type II Bedding	TN	50	\$37.50	\$1,875.00
Handrail	LF	74	\$100.50	\$7,437.00
Stabilization Bedding	TN	150	\$37.50	\$5,625.00
Concrete Pump Truck	EA	6	\$875.00	\$5,250.00
Road Base	CY	816	\$71.00	\$57,936.00
24" RCP	LF	60	\$161.00	\$9,660.00
48" RCP	LF	36	\$349.00	\$12,564.00
60" RCP	LF	118	\$476.00	\$56,168.00
			Total	\$378,535.00

IX. Summary

Flying Horse North Filing No. 8 is a 159.44 acre filing within Flying Horse North that consists of fifty (50) single-family residential estate lots, two tracts primarily for drainage use, and right-of-way for Sandbagger Drive, Holmes Road, Par Court, and Wedge Trail.

Pond J (district owned and maintained) is located on the western boundary of the filing, adjacent to the golf course, and serves as full spectrum detention for a majority of the filing area that flows north. Pond 13 (district owned and maintained) is located outside of the Filing No. 8 boundary and is the downstream

permanent control measure for the remaining filing that drains north. Imperviousness is reduced for the tributary area to this pond from the 2018 Classic Report, therefore, this pond has sufficient capacity for full spectrum detention and no improvements are proposed.

Flows at the southwest of the site flowing directly offsite are not anticipated to have any negative impacts on downstream drainage infrastructure as the proposed development falls within the assumptions of the FDR for Cathedral Pines Subdivision Filing No. 2 and does not change existing drainage patterns.

The Filing No. 8 final design is assessed for stormwater capacity of roadway sections, roadside swales and the existing tertiary drainage ways to ensure that development of the 2.5-acre single-family residential estate lots will not be negatively impacted by drainage conditions, including existing and proposed altered areas for the roadway and lot construction phases.

All County and MHFD drainage design standards are met. It is anticipated that there will be no negative impacts to downstream and surrounding developments and facilities due to the development of Filing No. 8.

X. References

El Paso County – Drainage Criteria Manual, 2014

City of Colorado Springs – Drainage Criteria Manual, May 2014

Urban Storm Drainage Criteria Manual, Urban Drainage Flood Control District, January 2018

Mile High Flood District Urban Storm Drainage Criteria Manual Volumes 1, 2, and 3; latest revisions

Mile High Flood District Software Resources and Tools (MHFD-Detention, UD-Inlet, UD-BMP)

United States Department of Agriculture National Resources Conservation Service Rock Chute Design Data Spreadsheet

Preliminary Drainage Report for Flying Horse North Preliminary Plan and Final Drainage Report for Flying Horse North Filing No. 1, Classic Consulting Engineers and Surveyors, November 2017

Flying Horse North Master Development Drainage Plan, HR Green Development, LLC., September 2022

Flying Horse North Irrigation Reservoir Embankment Design Report, Classic Consulting Engineers and Surveyors, latest revision June 2018, County approved on September 25, 2018

Flying Horse North Filing No. 4 Final Drainage Report, HR Green Development, LLC., September 2024.

Flying Horse North Filing No. 5 Final Drainage Report, HR Green Development, LLC., December 2024.

Flying Horse North Major PUD Amendment Preliminary Drainage Report, HR Green Development, LLC., September 2025.

Amendment to the Final Drainage Report for Cathedral Pines prepared by Stillwater Engineering – dated August 28th, 2006



APPENDIX A

VICINITY MAP

NRCS SOILS MAP

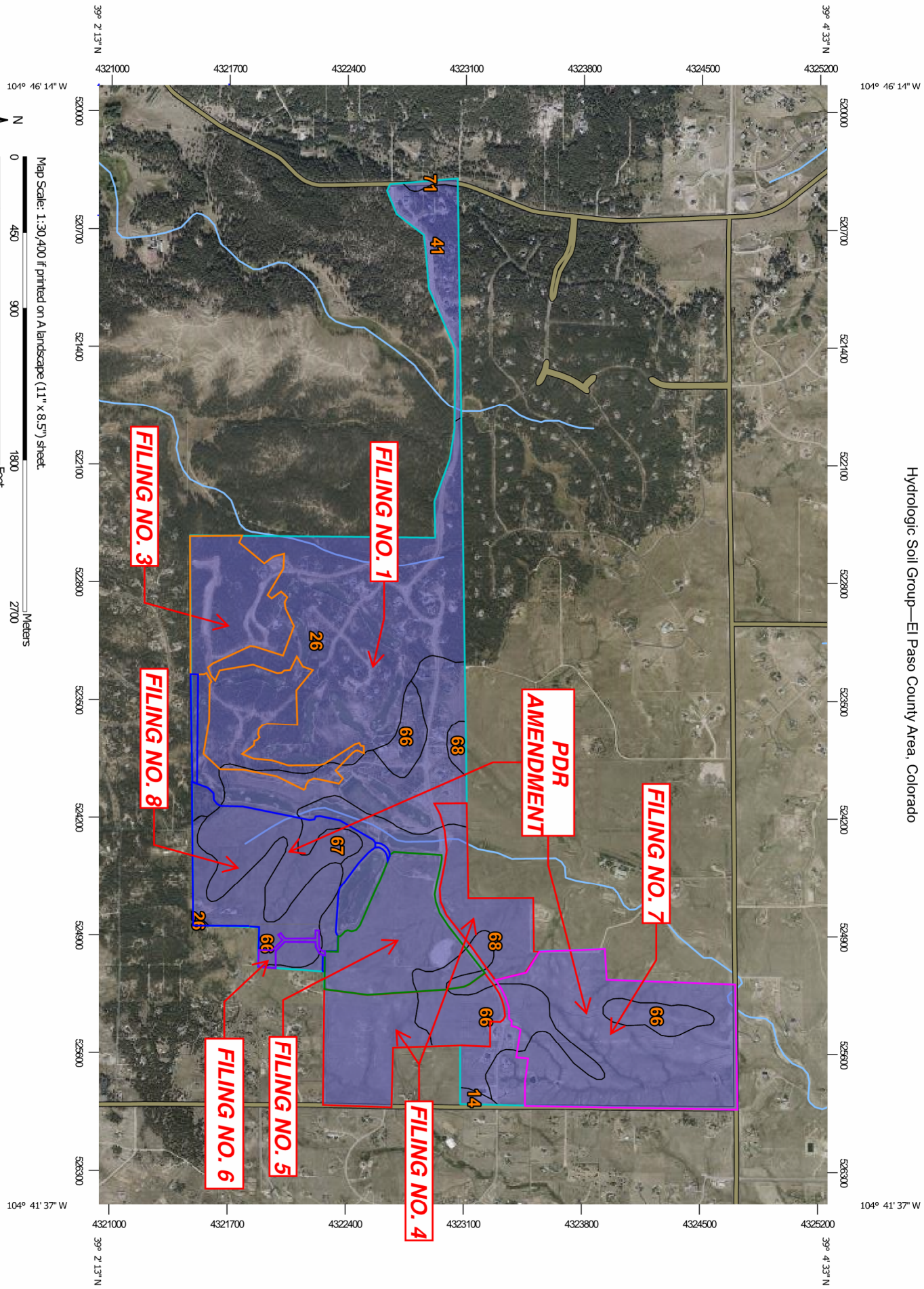
FEMA FLOODPLAIN MAP

EL PASO COUNTY MAJOR DRAINAGE BASINS MAP

VICINITY MAP

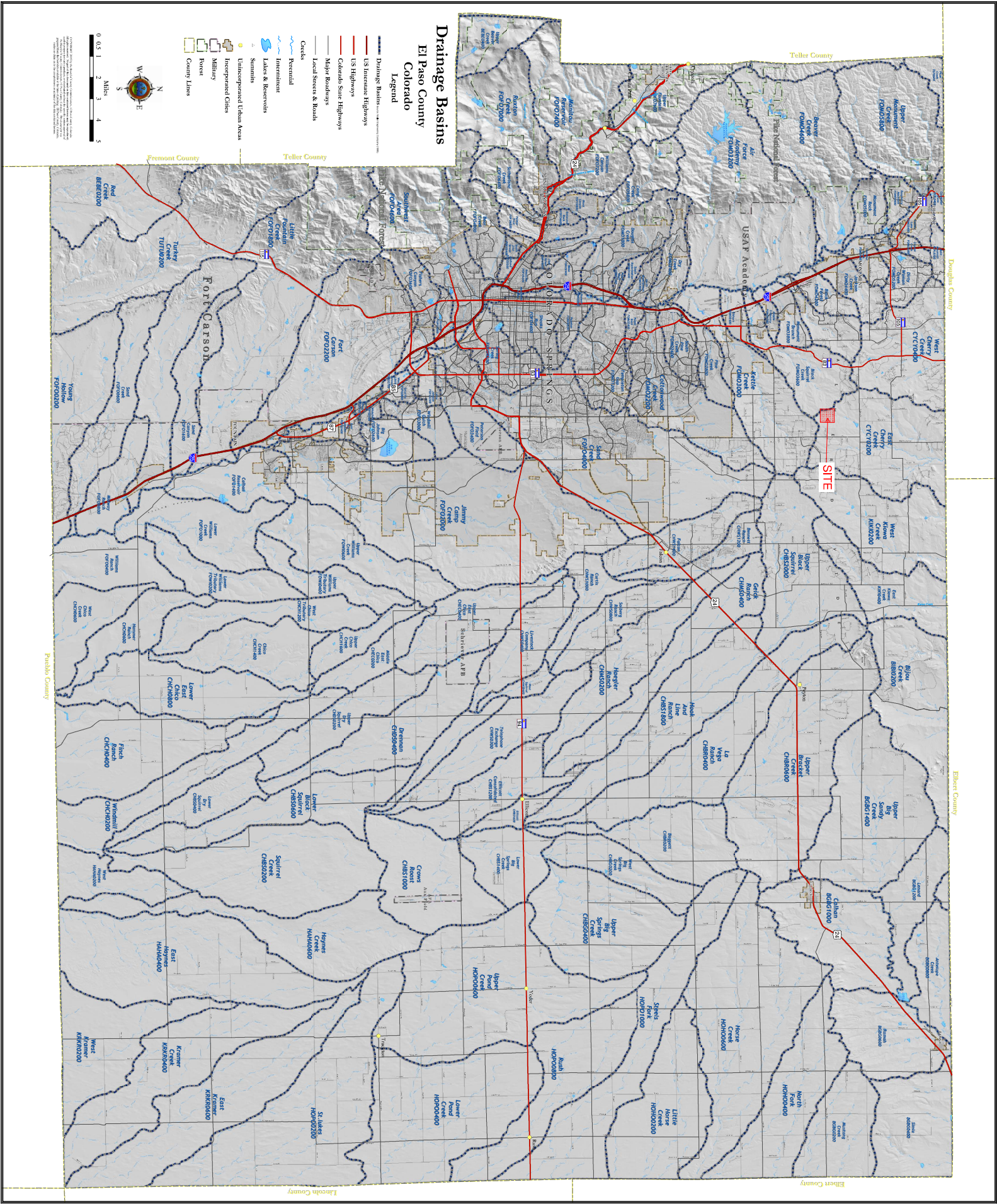


Hydrologic Soil Group—El Paso County Area, Colorado



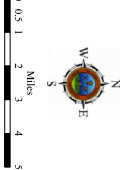
Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
14	Brussett loam, 1 to 3 percent slopes	B	3.6	0.2%
26	Elbeth sandy loam, 8 to 15 percent slopes	B	521.0	32.8%
41	Kettle gravelly loamy sand, 8 to 40 percent slopes	B	53.1	3.3%
66	Peyton sandy loam, 1 to 5 percent slopes	B	224.0	14.1%
67	Peyton sandy loam, 5 to 9 percent slopes	B	216.2	13.6%
68	Peyton-Pring complex, 3 to 8 percent slopes	B	567.2	35.7%
71	Pring coarse sandy loam, 3 to 8 percent slopes	B	3.0	0.2%
Totals for Area of Interest			1,588.3	100.0%



Drainage Basins El Paso County Colorado

- Legend**
- Drainage Basins
 - US Interstate Highways
 - US Highways
 - Colorado State Highways
 - Major Roadways
 - Local Streets & Roads
 - Creeks
 - Intermittent
 - Perennial
 - Lakes & Reservoirs
 - Summits
 - Unincorporated Urban Areas
 - Military
 - Forest
 - County Lines



0 0.5 1 2 3 4 5
Miles



APPENDIX B

HYDROLOGY CALCULATIONS



FLYING HORSE NORTH FILING 8
EXISTING CONDITIONS
EL PASO COUNTY, COLORADO

Calc'd by:	CVW
Checked by:	RDL
Date:	4/1/2026

SUMMARY RUNOFF TABLE

BASIN	AREA (ac)	% IMP.	C _s	C ₁₀₀	Q _s (cfs)	Q ₁₀₀ (cfs)
I3	9.51	11.0	0.17	0.42	6.3	26.0
I4	2.34	13.3	0.17	0.42	1.5	6.0
I5	20.40	18.9	0.22	0.45	15.0	51.9
I6	1.69	2.0	0.08	0.35	0.5	3.8
I7	23.02	2.0	0.08	0.35	6.0	44.2
I8	6.38	2.0	0.08	0.35	1.8	13.6
I9	0.54	2.0	0.08	0.35	0.2	1.4
J3	30.10	6.0	0.12	0.38	11.5	59.1
J4	92.88	2.0	0.08	0.35	19.5	142.8
J5	42.29	2.4	0.08	0.35	10.1	71.9
OS-1	23.50	3.3	0.09	0.36	7.4	48.2
OS-2	1.55	2.0	0.08	0.35	0.5	3.4

CUMULATIVE DESIGN POINT SUMMARY TABLE

DESIGN POINT	CONTRIBUTING BASINS	ΣQ _s (cfs)	ΣQ ₁₀₀ (cfs)	Tributary Area (ac.)	Weighted % Impervious
I3	I3	6.3	26.0	9.5	11.0
I4	I4	1.5	6.0	2.3	13.3
I5	I4, I5	16.4	57.4	22.7	18.3
I6	I6	0.5	3.8	1.7	2.0
I7	I7	6.0	44.2	23.0	2.0
I8	I8	1.8	13.6	6.4	2.0
I9	I3, I4, I5, I6, I7, I9	27.9	126.5	57.5	9.9
J3	J3	11.5	59.1	30.1	6.0
J4	J4	19.5	142.8	92.9	2.0
J5	J3, J5	38.5	259.0	72.4	3.9
OS-1	OS-1	7.4	48.2	23.5	3.3
OS-2	OS-2	7.4	48.2	23.5	3.3



FLYING HORSE NORTH FILING 8

CVW

EXISTING CONDITIONS

RDL

EL PASO COUNTY, COLORADO

Calcd by:

Checked by:

Date:

4/1/2026

COMPOSITE 'C' FACTORS

BASIN	GOLF COURSE / UNDEVELOPED	ROADWAY	ROOF	RESIDENTIAL		TOTAL	SOIL TYPE	GOLF COURSE / UNDEVELOPED		ROADWAY		ROOF		RESIDENTIAL		RESIDENTIAL		COMPOSITE							
	ACRES			(2.5 AC LOT)	(5.0 AC LOT)			%I	C _g	C ₁₀₀	%I	C _g	C ₁₀₀	%I	C _g	C ₁₀₀	%I	C _g *	C ₁₀₀ *	%I	C _g	C ₁₀₀			
13	0.00	0.00	0.00	9.51	0.00	9.51	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
14	2.07	0.27	0.00	0.00	0.00	2.34	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	13.3	0.17	0.42
15	16.89	3.51	0.00	0.00	0.00	20.40	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	18.9	0.22	0.45
16	1.69	0.00	0.00	0.00	0.00	1.69	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	2.0	0.08	0.35
17	23.02	0.00	0.00	0.00	0.00	23.02	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	2.0	0.08	0.35
18	6.38	0.00	0.00	0.00	0.00	6.38	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	2.0	0.08	0.35
19	0.54	0.00	0.00	0.00	0.00	0.54	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	2.0	0.08	0.35
J3	8.51	0.00	0.00	3.15	18.44	30.10	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	6.0	0.12	0.38
J4	92.88	0.00	0.00	0.00	0.00	92.88	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	2.0	0.08	0.35
J5	42.11	0.18	0.00	0.00	0.00	42.29	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	2.4	0.08	0.35
OS-1	20.03	0.00	0.00	3.47	0.00	23.50	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	3.3	0.09	0.36
OS-2	1.55	0.00	0.00	0.00	0.00	1.55	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	2.0	0.08	0.35
GRAND TOTAL	215.67	3.96	0.00	16.13	18.44	254.20																	4.46%	0.10	0.37
POND SUMMARY																									
POND J LOCATION	143.50	0.18	0.00	3.15	18.44	165.27	B																2.84%	0.09	0.36
POND 13	50.59	3.78	0.00	9.51	0.00	63.88	B																9.14%	0.14	0.40

	FLYING HORSE NORTH FILING 8		Calc'd by:	CVW
	EXISTING CONDITIONS		Checked by:	RDL
	EL PASO COUNTY, COLORADO		Date:	4/1/2026

TIME OF CONCENTRATION												
BASIN DATA			OVERLAND TIME (T _f)		TRAVEL TIME (T _f)					TOTAL		Design tc
DESIGNATION	C _s	AREA (ac)	LENGTH (ft)	SLOPE %	t _f (min)	C _v	LENGTH (ft)	SLOPE %	V (ft/s)	t _f (min)	t _c (min)	tc=(L/180)+10 tc max
I3	0.17	9.51	250	6.0	14.8	10	50	6.0	2.4	0.3	15.2	11.7
I4	0.17	2.34	300	4.2	18.2	10	430	2.9	1.7	4.2	22.4	14.1
I5	0.22	20.40	300	3.7	18.0	10	970	5.5	2.3	6.9	24.9	17.1
I6	0.08	1.69	173	6.9	12.9	10	197	3.5	1.9	1.8	14.7	12.1
I7	0.08	23.02	300	5.7	18.1	10	1100	8.4	2.9	6.3	24.5	17.8
I8	0.08	6.38	300	4.2	20.1	10	420	2.9	1.7	4.1	24.2	14.0
I9	0.08	0.54	105	14.0	8.0	10	78	6.0	2.4	0.5	8.5	11.0
J3	0.12	30.10	300	5.2	17.9	10	1586	4.6	2.1	12.3	30.2	20.5
J4	0.08	92.88	300	6.0	17.8	10	2830	1.1	1.0	45.0	62.8	27.4
J5	0.08	42.29	300	9.3	15.3	10	2060	2.1	1.4	23.7	39.0	23.1
OS-1	0.09	23.50	300	7.7	16.2	10	841	7.7	2.8	5.1	21.3	16.3
OS-2	0.08	1.55	300	7.7	16.4	10	268	7.7	2.8	1.6	18.0	13.2

		FLYING HORSE NORTH FILING 8				CVM						
		EXISTING CONDITIONS				RDL						
		DESIGN STORM: 5-YEAR				4/1/2026						
HRGreen												
		DIRECT RUNOFF					TOTAL RUNOFF		REMARKS			
DESIGN POINT	BASIN ID	AREA (ac)	C _s	t _c (min)	C _s *A (ac)	I (in./ hr.)	Q (cfs)	t _c (min)	C _s *A (ac)	I (in./ hr.)	Q (cfs)	
13	13	9.51	0.17	11.7	1.62	3.90	6.3					BASIN FLOW TO DP 13
14	14	2.34	0.17	14.1	0.41	3.62	1.5					BASIN FLOW TO DP 14
15	15	20.40	0.22	17.1	4.51	3.33	15.0					BASIN FLOW TO DP 15
16	16	1.69	0.08	12.1	0.14	3.85	0.5	17.1	4.92	3.33	16.4	COMBINED BASIN FLOW FROM BASINS 14 - 15
17	17	23.02	0.08	17.8	1.84	3.27	6.0					BASIN FLOW TO DP 16
18	18	6.38	0.08	14.0	0.51	3.62	1.8					BASIN FLOW TO DP 18
19	19	0.54	0.08	8.5	0.04	4.38	0.2					BASIN FLOW TO DP 19
J3	J3	30.10	0.12	20.5	3.75	3.05	11.5	17.8	8.56	3.27	27.9	COMBINED BASIN FLOW FROM BASINS 13 - 17, 19
J4	J4	92.88	0.08	27.4	7.43	2.62	19.5					BASIN FLOW TO DP J4
J5	J5	42.29	0.08	23.1	3.53	2.87	10.1					BASIN FLOW TO DP J5
OS-1	OS-1	23.50	0.09	16.3	2.19	3.39	7.4	27.4	14.71	2.62	38.5	COMBINED DP FLOW FROM DPS J3-J5 AT PR. POND J LOCATION
OS-2	OS-2	1.55	0.08	13.2	0.12	3.72	0.5					BASIN FLOW TO DP OS-1
												BASIN FLOW OFFSITE TO OS-2



FLYING HORSE NORTH FILING 8
EXISTING CONDITIONS
DESIGN STORM: 100-YEAR

Calcd by:
Checked by:
Date:

CVW
RDL
4/1/2026

HRGreen

		DIRECT RUNOFF						TOTAL RUNOFF				REMARKS
DESIGN PONT	BASIN ID	AREA (ac)	C ₁₀₀	t _c (min)	C ₁₀₀ *A (ac)	I (in./ hr.)	Q (cfs)	t _c (min)	C ₁₀₀ *A (ac)	I (in./ hr.)	Q (cfs)	
13	13	9.51	0.42	11.7	3.97	6.54	26.0					BASIN FLOW TO DP 13
14	14	2.34	0.42	14.1	0.98	6.07	6.0					BASIN FLOW TO DP 14
15	15	20.40	0.45	17.1	9.28	5.59	51.9					BASIN FLOW TO DP 15
												COMBINED BASIN FLOW FROM BASINS 14 - 15
16	16	1.69	0.35	12.1	0.59	6.46	3.8	17.1	10.26	5.59	57.4	BASIN FLOW TO DP 16
17	17	23.02	0.35	17.8	8.06	5.48	44.2					BASIN FLOW TO DP 16
18	18	6.38	0.35	14.0	2.23	6.08	13.6					BASIN FLOW TO DP 18
19	19	0.54	0.35	8.5	0.19	7.35	1.4					BASIN FLOW TO DP 19
												COMBINED BASIN FLOW FROM BASINS 13 - 17, 19
J3	J3	30.10	0.38	20.5	11.53	5.13	59.1	17.8	23.07	5.48	126.5	BASIN FLOW TO DP J3
J4	J4	92.88	0.35	27.4	32.51	4.39	142.8					BASIN FLOW TO DP J4
J5	J5	42.29	0.35	23.1	14.91	4.82	71.9					BASIN FLOW TO DP J5
								27.4	58.95	4.39	259.0	COMBINED DP FLOW FROM DP'S J3-J5 AT PR. POND J LOCATION
OS-1	OS-1	23.50	0.36	16.3	8.46	5.70	48.2					BASIN FLOW TO DP OS-1
OS-2	OS-2	1.55	0.35	13.2	0.54	6.24	3.4					BASIN FLOW OFFSITE TO OS-2



FLYING HORSE NORTH FILING 8
PROPOSED CONDITIONS
EL PASO COUNTY, COLORADO

Calcd by:	CVW
Checked by:	RDL
Date:	4/1/2026

SUMMARY RUNOFF TABLE

BASIN	AREA (ac)	% IMP.	C _s	C ₁₀₀	Q _s (cfs)	Q ₁₀₀ (cfs)
I3	9.51	11.0	0.17	0.42	6.3	26.0
I4	2.94	11.0	0.17	0.42	1.8	7.4
I5	20.40	18.9	0.22	0.45	15.0	51.9
I6	1.69	11.0	0.17	0.42	1.1	4.6
I7	18.90	11.0	0.17	0.42	10.5	43.2
I8	5.82	11.0	0.17	0.42	3.6	14.8
I9	0.54	2.0	0.08	0.35	0.2	1.4
J1	25.63	8.6	0.15	0.40	13.7	61.6
J2	25.08	9.1	0.15	0.41	13.2	58.4
J3	34.37	8.1	0.14	0.40	15.2	70.2
J4	1.22	11.0	0.17	0.42	0.9	3.5
J5	2.53	11.0	0.17	0.42	1.6	6.4
J6	33.02	11.0	0.17	0.42	18.5	76.2
J7	1.83	11.0	0.17	0.42	1.1	4.7
J8	2.05	11.0	0.17	0.42	1.3	5.5
J9	26.36	11.0	0.17	0.42	13.8	56.9
J10	2.25	11.0	0.17	0.42	1.4	5.9
J11	3.25	11.0	0.17	0.42	1.7	7.1
J12	4.94	11.0	0.17	0.42	3.3	13.4
J13	6.79	2.0	0.08	0.35	1.9	14.2
OS-1	23.46	6.9	0.13	0.39	10.7	53.7
OS-2	1.55	2.0	0.08	0.35	0.5	3.4
GRAND TOTAL	254.1	10.1%	0.16	0.41	137.4	590.2

POND SUMMARY

POND ID	TRIBUTARY AREA (ac)	EFFECTIVE % IMPERVIOUS	C _s	C ₁₀₀	PREDEV. PEAK INFLOW (UD-DET.) (cfs)	PEAK OUTFLOW (UD-DET.) (cfs)
POND J	169.3	9.39%	0.16	0.41	254.3	200.4
POND I3	60.8	13.41%	0.18	0.43		

CUMULATIVE DESIGN POINT SUMMARY TABLE

DESIGN POINT	CONTRIBUTING BASINS	ΣQ _s (cfs)	ΣQ ₁₀₀ (cfs)	Tributary Area (ac.)	Weighted % ImperVIOUS
I3	I3	6.3	26.0	9.5	11.0
I4	I4	1.8	7.4	2.9	11.0
I5	I4, I5	16.7	58.7	23.3	17.9
I6	I6	1.1	4.6	1.7	11.0
I7	I7	10.5	43.2	18.9	11.0
I8	I8	3.6	14.8	5.8	11.0
I9	I3, I4, I5, I6, I7, I9	33.2	127.5	0.5	2.0
J1	J1	13.7	61.6	25.6	8.6
J2	J2	13.2	58.4	25.1	9.1
J3	J3	15.2	70.2	34.4	8.1
J4	J4	0.9	3.5	1.2	11.0
J5	J5	1.6	6.4	2.5	11.0
J6	J1, J2, J4, J5, J6, J7	47.1	202.3	89.3	9.8
J7	J7	1.1	4.7	1.8	11.0
J8	J8	1.3	5.5	2.1	11.0
J9	J3, J9	28.9	126.5	60.7	9.3
J10	J10	1.4	5.9	2.3	11.0
J11	J10, J11	2.9	12.1	5.5	11.0
J12	J12	3.3	13.4	4.9	11.0
J13	J6, J8, J9, J11, J12, J13	64.9	280.4	76.4	10.2
OS-1	OS-1	10.7	53.7	23.5	6.9
OS-2	OS-2	0.5	3.4	1.6	2.0



FLYING HORSE NORTH FILING 8

PROPOSED CONDITIONS

EL PASO COUNTY, COLORADO

Calcd. by: CVW
Checked by: RDL
Date: 4/1/2026

COMPOSITE 'C' FACTORS

BASIN	GOLF COURSE / UNDEVELOPED	ROADWAY	ROOF	RESIDENTIAL (2.5 AC LOT)	RESIDENTIAL (5.0 AC LOT)	TOTAL	SOIL TYPE	GOLF COURSE / UNDEVELOPED		ROADWAY		ROOF		RESIDENTIAL (2.5 AC LOT)	RESIDENTIAL (5.0 AC LOT)	COMPOSITE IMPERVIOUSNESS & C									
								%I	C _g	%I	C _g	%I	C _g	%I	C _g	%I	C _g	C _{imp}							
13	0.00	0.00	0.00	9.51	0.00	9.51	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
14	0.00	0.00	0.00	2.94	0.00	2.94	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
15	16.89	3.51	0.00	0.00	0.00	20.40	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	18.9	0.22	0.45
16	0.00	0.00	0.00	1.69	0.00	1.69	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
17	0.00	0.00	0.00	18.90	0.00	18.90	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
18	0.00	0.00	0.00	5.82	0.00	5.82	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
19	0.54	0.00	0.00	0.00	0.00	0.54	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	2.0	0.08	0.35
J1	0.00	0.00	0.00	10.01	0.00	10.01	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	8.6	0.15	0.40
J2	0.00	0.00	0.00	12.98	0.00	12.98	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	9.1	0.15	0.41
J3	2.97	0.00	0.00	12.96	0.00	18.44	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	8.1	0.14	0.40
J4	0.00	0.00	0.00	1.22	0.00	1.22	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
J5	0.00	0.00	0.00	2.53	0.00	2.53	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
J6	0.00	0.00	0.00	33.02	0.00	33.02	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
J7	0.00	0.00	0.00	1.83	0.00	1.83	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
J8	0.00	0.00	0.00	2.05	0.00	2.05	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
J9	0.00	0.00	0.00	26.36	0.00	26.36	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
J10	0.00	0.00	0.00	2.25	0.00	2.25	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
J11	0.00	0.00	0.00	3.25	0.00	3.25	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
J12	0.00	0.00	0.00	4.94	0.00	4.94	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	11.0	0.17	0.42
J13	6.79	0.00	0.00	0.00	0.00	6.79	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	2.0	0.08	0.35
OS-1	10.66	0.00	0.00	12.80	0.00	23.46	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	6.9	0.13	0.39
OS-2	1.55	0.00	0.00	0.00	0.00	1.55	B	2	0.08	0.35	100	0.90	0.96	90.00	0.73	0.81	11	0.17	0.42	7	0.14	0.39	2.0	0.08	0.35
GRAND TOTAL	39.40	3.51	0.00	165.07	46.15	254.13	POND SUMMARY													10.11%	0.16	0.41			
POND J	9.76	0.00	0.00	113.41	46.15	169.32	B													9.39%	0.16	0.41			
POND 13	18.44	3.51	0.00	38.86	0.00	60.81	B													13.41%	0.18	0.43			



FLYING HORSE NORTH FILING 8			Calc'd by:	CVW
PROPOSED CONDITIONS			Checked by:	RDL
EL PASO COUNTY, COLORADO			Date:	4/1/2026

BASIN DATA		OVERLAND TIME (T _o)		TIME OF CONCENTRATION						TOTAL		Design tc	
DESIGNATION	C _s	AREA (ac)	LENGTH (ft)	SLOPE %	t ₁ (min)	C _v	LENGTH (ft)	SLOPE %	V (ft/s)	t ₁ (min)	t ₂ (min)	t _c =(L/180)+10	t _{c design (min)}
I3	0.17	9.51	250	6.0	14.8	10	50	6.0	2.4	0.3	15.2	11.7	11.7
I4	0.17	2.94	300	4.2	18.3	10	430	2.9	1.7	4.2	22.5	14.1	14.1
I5	0.22	20.40	300	3.7	18.0	10	970	5.5	2.3	6.9	24.9	17.1	17.1
I6	0.17	1.69	173	6.9	11.8	10	197	3.5	1.9	1.8	13.5	12.1	12.1
I7	0.17	18.90	300	5.7	16.5	10	1100	8.4	2.9	6.3	22.9	17.8	17.8
I8	0.17	5.82	300	8.4	14.5	10	420	7.8	2.8	2.5	17.0	14.0	14.0
I9	0.08	0.54	105	14.0	8.0	10	78	6.0	2.4	0.5	8.5	8.5	8.5
J1	0.15	25.63	300	5.6	17.0	10	530	3.5	1.9	4.7	21.7	14.6	14.6
J2	0.15	25.08	300	5.7	16.8	10	788	5.3	2.3	5.7	22.5	16.0	16.0
J3	0.14	34.37	300	5.2	17.5	10	1586	4.6	2.1	12.3	29.8	20.5	20.5
J4	0.17	1.22	100	7.3	8.8	10	125	2.6	1.6	1.3	10.1	11.3	10.1
J5	0.17	2.53	100	7.3	8.8	10	701	4.9	2.2	5.3	14.1	14.5	14.1
J6	0.17	33.02	100	7.3	8.8	10	1240	4.5	2.1	9.7	18.5	17.4	17.4
J7	0.17	1.83	95	3.3	11.2	10	580	3.3	1.8	5.3	16.5	13.8	13.8
J8	0.17	2.05	100	3.6	11.1	10	320	3.6	1.9	2.8	13.9	12.3	12.3
J9	0.17	26.36	100	6.7	9.0	10	1720	4.1	2.0	14.2	23.3	20.1	20.1
J10	0.17	2.25	100	3.3	11.5	10	465	3.3	1.8	4.3	15.7	13.1	13.1
J11	0.17	3.25	100	0.8	18.4	10	1581	2.0	1.4	18.8	37.2	19.3	19.3
J12	0.17	4.94	100	6.7	9.0	10	440	6.7	2.6	2.8	11.9	13.0	11.9
J13	0.08	6.79	50	25.0	4.5	10	770	0.5	0.7	18.1	22.7	14.6	14.6
OS-1	0.13	23.46	300	7.7	15.6	10	564	7.7	2.8	3.5	19.1	14.9	14.9
OS-2	0.08	1.55	300	15.0	13.1	10	268	7.7	2.8	1.6	14.7	13.2	13.2

		FLYING HORSE NORTH FILING 8		CVM
PROPOSED CONDITIONS		Calcd by:		RDL
DESIGN STORM: 5-YEAR		Checked by:		4/1/2026
		Date:		
HRGreen				

		DIRECT RUNOFF					TOTAL RUNOFF			REMARKS		
DESIGN POINT	BASIN ID	AREA (ac)	C ₅	t _c (min)	C ₅ *A (ac)	I (in./ hr.)	Q (cfs)	t _c (min)	C ₅ *A (ac)	I (in./ hr.)	Q (cfs)	
13	13	9.51	0.17	11.7	1.62	3.90	6.3					BASIN FLOW TO DP 13 NO CHANGE FROM PDR AMENDMENT (PUDSP252)
												BASIN FLOW TO DP 14 NO CHANGE FROM PDR AMENDMENT (PUDSP252)
14	14	2.94	0.17	14.1	0.50	3.62	1.8					BASIN FLOW TO DP 15, NO CHANGE FROM PDR AMENDMENT (PUDSP252) COMBINED BASIN FLOW FROM BASINS 14 - 15 BASIN FLOW TO DP 16 NO CHANGE FROM PDR AMENDMENT (PUDSP252)
15	15	20.40	0.22	17.1	4.51	3.33	15.0					BASIN FLOW TO DP 16, NO CHANGE FROM PDR AMENDMENT (PUDSP252)
16	16	1.69	0.17	12.1	0.29	3.85	1.1	17.1	5.01	3.33	16.7	
												BASIN FLOW TO DP 18 SLIGHT DECREASE FROM PDR AMENDMENT (PUDSP252)
17	17	18.90	0.17	17.8	3.22	3.27	10.5					
18	18	5.82	0.17	14.0	0.99	3.62	3.6					
19	19	0.54	0.08	8.5	0.04	4.38	0.2					COMBINED FLOW BASINS 13-17, 19, SLIGHT INCREASE FROM PDR AMENDMENT (PUDSP252)
								17.8	10.18	3.27	33.2	
J1	J1	25.63	0.15	14.6	3.85	3.56	13.7					BASIN FLOW TO DP J1 SLIGHT INCREASE FROM PDR AMENDMENT (PUDSP252)
J2	J2	25.08	0.15	16.0	3.87	3.42	13.2					BASIN FLOW TO DP J2 NO CHANGE FROM PDR AMENDMENT (PUDSP252)
J3	J3	34.37	0.14	20.5	4.98	3.05	15.2					BASIN FLOW TO DP J3 NO CHANGE FROM PDR AMENDMENT (PUDSP252)
J4	J4	1.22	0.17	10.1	0.21	4.12	0.9					BASIN FLOW TO DP J4 BASIN GEOMETRY CHANGE FROM PDR AMENDMENT (PUDSP252)
J5	J5	2.53	0.17	14.1	0.43	3.62	1.6					BASIN FLOW TO DP J5
J6	J6	33.02	0.17	17.4	5.62	3.29	18.5	17.4	14.29	3.29	47.1	BASIN FLOW TO DP J6 COMBINED FLOW BASINS J1, J2, J4, J5, J6
J7	J7	1.83	0.17	13.8	0.31	3.65	1.1					BASIN FLOW TO DP J7
J8	J8	2.05	0.17	12.3	0.35	3.81	1.3					BASIN FLOW TO DP J8

1+33 HRGreen		FLYING HORSE NORTH FILING 8				CVW						
		PROPOSED CONDITIONS				RDL						
		DESIGN STORM: 5-YEAR				4/1/2026						
		DIRECT RUNOFF				TOTAL RUNOFF		REMARKS				
DESIGN POINT	BASIN ID	AREA (ac)	C _s	t _c (min)	C _s *A (ac)	I (in./ hr.)	Q (cfs)					
J9	J9	26.36	0.17	20.1	4.49	3.08	13.8	20.5	9.46	3.05	28.9	BASIN FLOW TO DP J9
												COMBINED BASIN FLOW FROM BASINS J3 AND J9
J10	J10	2.25	0.17	13.1	0.38	3.72	1.4					BASIN FLOW TO DP J10
J11	J11	3.25	0.17	19.3	0.55	3.14	1.7					BASIN FLOW TO DP J11
								19.3	0.94	3.14	2.9	COMBINED BASIN FLOW FROM BASINS J10 AND J11
J12	J12	4.94	0.17	11.9	0.84	3.87	3.3					BASIN FLOW TO DP J12
J13	J13	6.79	0.08	14.6	0.54	3.57	1.9					BASIN FLOW TO PR. FSD POND J
								20.1	21.06	3.08	64.9	COMBINED FLOW DPs J6, J8, J9, J11, J12, J13
OS-1	OS-1	23.46	0.13	14.9	3.03	3.53	10.7					BASIN FLOW TO OS-1
OS-2	OS-2	1.55	0.08	13.2	0.12	3.72	0.5					BASIN FLOW OFFSITE TO OS-2



FLYING HORSE NORTH FILING 8

PROPOSED CONDITIONS

DESIGN STORM: 100-YEAR

CVW

Calcd. by:
Checked by:
Date:

HRGreen

DIRECT RUNOFF				TOTAL RUNOFF				REMARKS
DESIGN PONT	BASIN ID	AREA (ac)	C ₁₀₀	t _c (min)	C ₁₀₀ *A (ac)	I (in./ hr.)	Q (cfs)	
13	13	9.51	0.42	11.7	3.97	6.54	26.0	BASIN FLOW TO DP 13
								NO CHANGE FROM PDR AMENDMENT (PUDSP252)
14	14	2.94	0.42	14.1	1.23	6.07	7.4	BASIN FLOW TO DP 14
								NO CHANGE FROM PDR AMENDMENT (PUDSP252)
15	15	20.40	0.45	17.1	9.28	5.59	51.9	BASIN FLOW TO DP 15, NO CHANGE FROM PDR AMENDMENT (PUDSP252)
								COMBINED BASIN FLOW FROM BASINS 14 - 15
16	16	1.69	0.42	12.1	0.70	6.46	4.6	BASIN FLOW TO DP 16
								NO CHANGE FROM PDR AMENDMENT (PUDSP252)
17	17	18.90	0.42	17.8	7.88	5.48	43.2	BASIN FLOW TO DP 17, NO CHANGE FROM PDR AMENDMENT (PUDSP252)
18	18	5.82	0.42	14.0	2.43	6.08	14.8	BASIN FLOW TO DP 18
								SLIGHT DECREASE FROM PDR AMENDMENT (PUDSP252)
19	19	0.54	0.35	8.5	0.19	7.35	1.4	BASIN FLOW TO DP 19
								COMBINED FLOW BASINS 13-17, 19, SLIGHT INCREASE FROM PDR AMENDMENT (PUDSP252)
1	1	25.63	0.40	14.6	10.31	5.98	61.6	BASIN FLOW TO DP 1
								SLIGHT INCREASE FROM PDR AMENDMENT (PUDSP252)
2	2	25.08	0.41	16.0	10.17	5.74	58.4	BASIN FLOW TO DP 2
								NO CHANGE FROM PDR AMENDMENT (PUDSP252)
3	3	34.37	0.40	20.5	13.69	5.13	70.2	BASIN FLOW TO DP 3
								NO CHANGE FROM PDR AMENDMENT (PUDSP252)
4	4	1.22	0.42	10.1	0.51	6.91	3.5	BASIN FLOW TO DP 4
								BASIN GEOMETRY CHANGE FROM PDR AMENDMENT (PUDSP252)
5	5	2.53	0.42	14.1	1.06	6.07	6.4	BASIN FLOW TO DP 5
6	6	33.02	0.42	17.4	13.77	5.53	76.2	BASIN FLOW TO DP 6
								COMBINED FLOW BASINS 1, 2, 4, 5, 6
7	7	1.83	0.42	13.8	0.76	6.13	4.7	BASIN FLOW TO DP 7
8	8	2.05	0.42	12.3	0.86	6.40	5.5	BASIN FLOW TO DP 8



FLYING HORSE NORTH FILING 8		Calcd. by:	CVW
PROPOSED CONDITIONS		Checked by:	RDL
DESIGN STORM: 100-YEAR		Date:	4/1/2026

H+RGreen

DIRECT RUNOFF						TOTAL RUNOFF			REMARKS
DESIGN PONT	BASIN ID	AREA (ac)	C ₁₀₀	t _c (min)	C ₁₀₀ *A (ac)	I (in./ hr.)	Q (cfs)	t _c (min)	
J9	J9	26.36	0.42	20.1	10.99	5.17	56.9	20.5	BASIN FLOW TO DP J9
J10	J10	2.25	0.42	13.1	0.94	6.24	5.9	24.68	COMBINED BASIN FLOW FROM BASINS J3 AND J9
									BASIN FLOW TO DP J10
J11	J11	3.25	0.42	19.3	1.36	5.27	7.1	19.3	BASIN FLOW TO DP J11
J12	J12	4.94	0.42	11.9	2.06	6.50	13.4	2.29	COMBINED BASIN FLOW FROM BASINS J10 AND J11
									BASIN FLOW TO DP J12
J13	J13	6.79	0.35	14.6	2.38	5.99	14.2		BASIN FLOW TO PR. FSD POND J
								20.1	COMBINED FLOW DPs J6, J8, J9, J11, J12, J13
OS-1	OS-1	23.46	0.39	14.9	9.07	5.93	53.7		BASIN FLOW TO OS-1
OS-2	OS-2	1.55	0.35	13.2	0.54	6.24	3.4		BASIN FLOW OFFSITE TO OS-2



APPENDIX C

HYDRAULIC CALCULATIONS

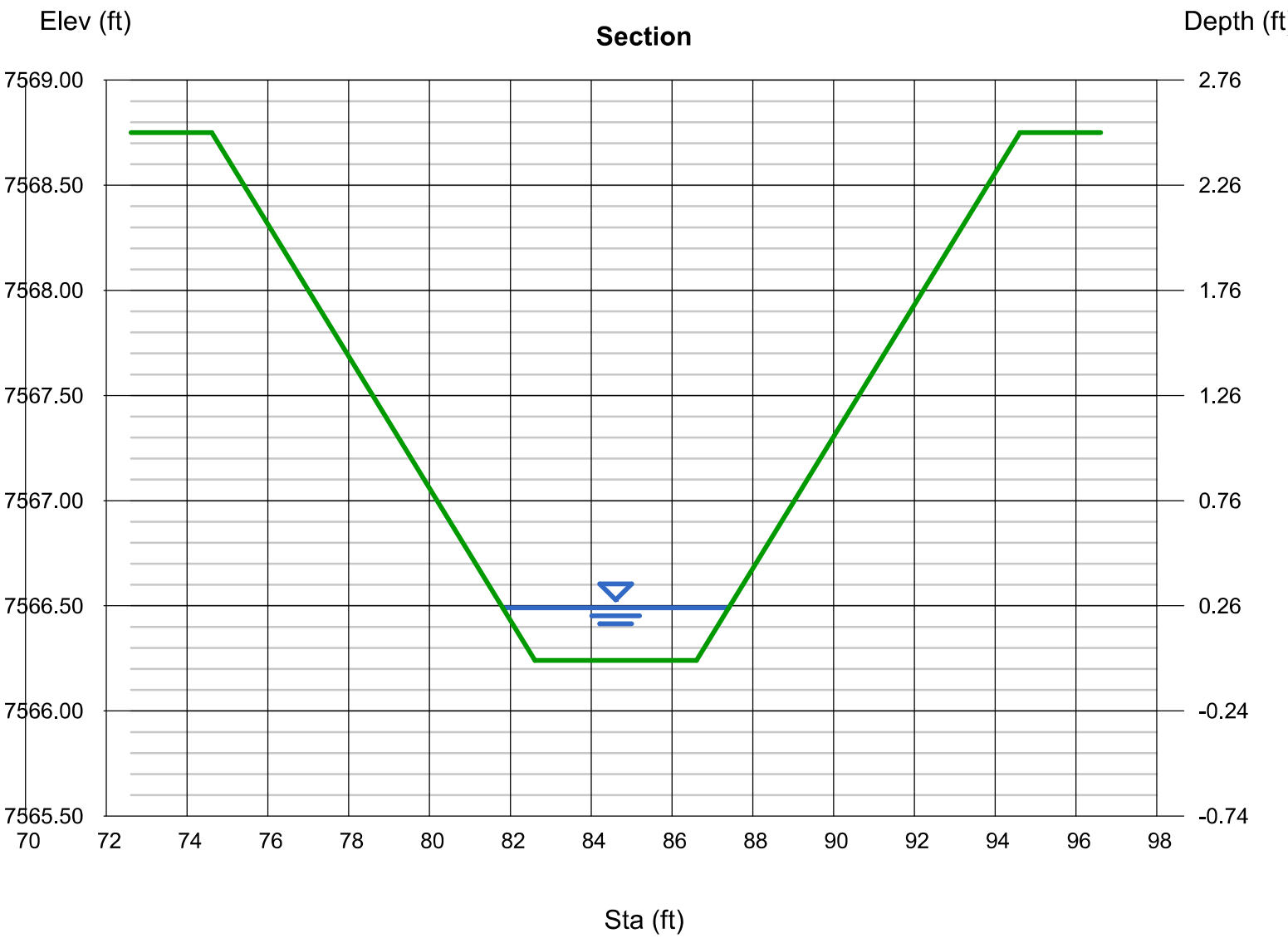
Channel Report

Channel Section HH - 5Yr Storm

User-defined		Highlighted	
Invert Elev (ft)	= 7566.24	Depth (ft)	= 0.25
Slope (%)	= 4.90	Q (cfs)	= 4.550
N-Value	= 0.030	Area (sqft)	= 1.20
		Velocity (ft/s)	= 3.79
		Wetted Perim (ft)	= 5.67
		Crit Depth, Yc (ft)	= 0.32
		Top Width (ft)	= 5.59
		EGL (ft)	= 0.47

(Sta, El, n)-(Sta, El, n)...

(74.61, 7568.75)-(82.61, 7566.24, 0.030)-(86.61, 7566.24, 0.030)-(94.61, 7568.75, 0.030)



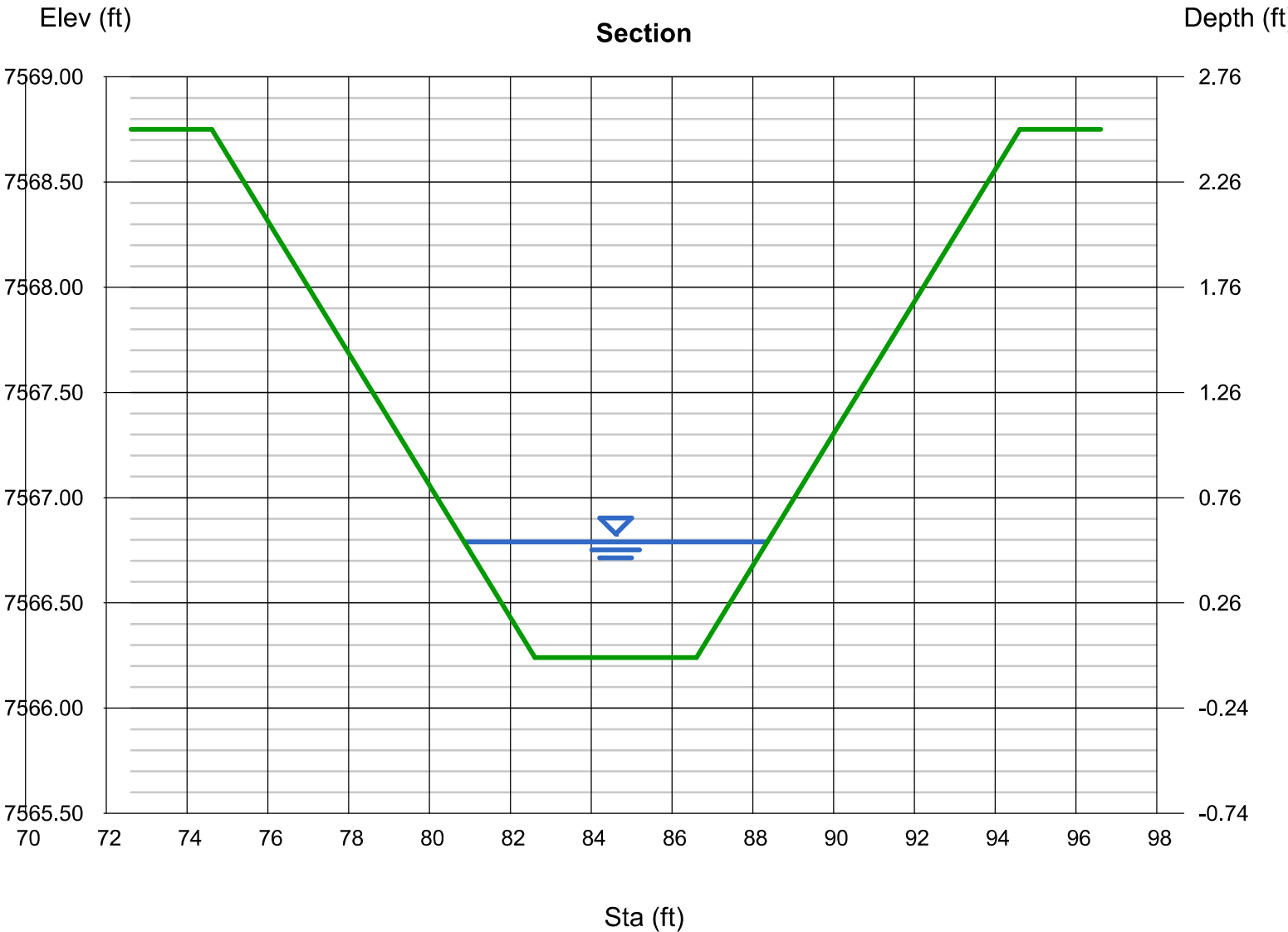
Channel Report

Channel Section HH - 100Yr Storm

User-defined		Highlighted	
Invert Elev (ft)	= 7566.24	Depth (ft)	= 0.55
Slope (%)	= 4.90	Q (cfs)	= 18.80
N-Value	= 0.030	Area (sqft)	= 3.16
		Velocity (ft/s)	= 5.94
		Wetted Perim (ft)	= 7.67
		Crit Depth, Yc (ft)	= 0.73
		Top Width (ft)	= 7.51
		EGL (ft)	= 1.10

(Sta, El, n)-(Sta, El, n)...

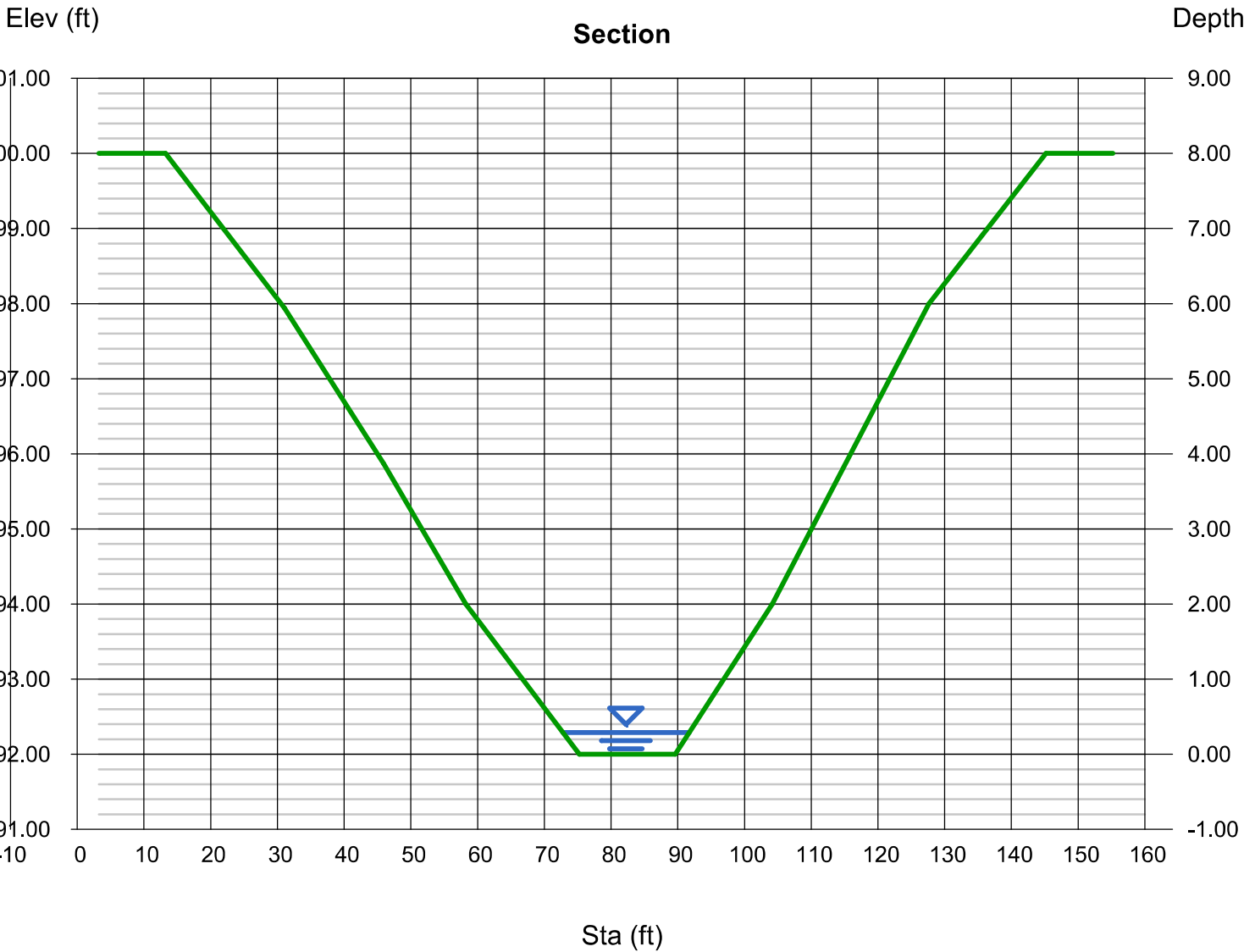
(74.61, 7568.75)-(82.61, 7566.24, 0.030)-(86.61, 7566.24, 0.030)-(94.61, 7568.75, 0.030)



Channel Report

Channel Section II - 5Yr Storm

User-defined		Highlighted	
Invert Elev (ft)	= 7592.00	Depth (ft)	= 0.29
Slope (%)	= 2.70	Q (cfs)	= 15.20
N-Value	= 0.030	Area (sqft)	= 4.84
Calculations		Velocity (ft/s)	= 3.14
Compute by:	Known Q	Wetted Perim (ft)	= 19.00
Known Q (cfs)	= 15.20	Crit Depth, Yc (ft)	= 0.31
		Top Width (ft)	= 18.96
		EGL (ft)	= 0.44
(Sta, El, n)-(Sta, El, n)...			
(13.24, 7600.00)-(31.05, 7597.94, 0.030)-(45.93, 7595.87, 0.030)-(58.22, 7594.00, 0.030)-(75.23, 7592.00, 0.030)-(89.62, 7592.00, 0.030)-(104.15, 7594.00, 0.030)-			
-(127.62, 7598.00, 0.030)-(145.20, 7600.00, 0.030)			



Channel Report

Channel Section II - 100Yr Storm

User-defined

Invert Elev (ft) = 7592.00
Slope (%) = 2.70
N-Value = 0.030

Calculations

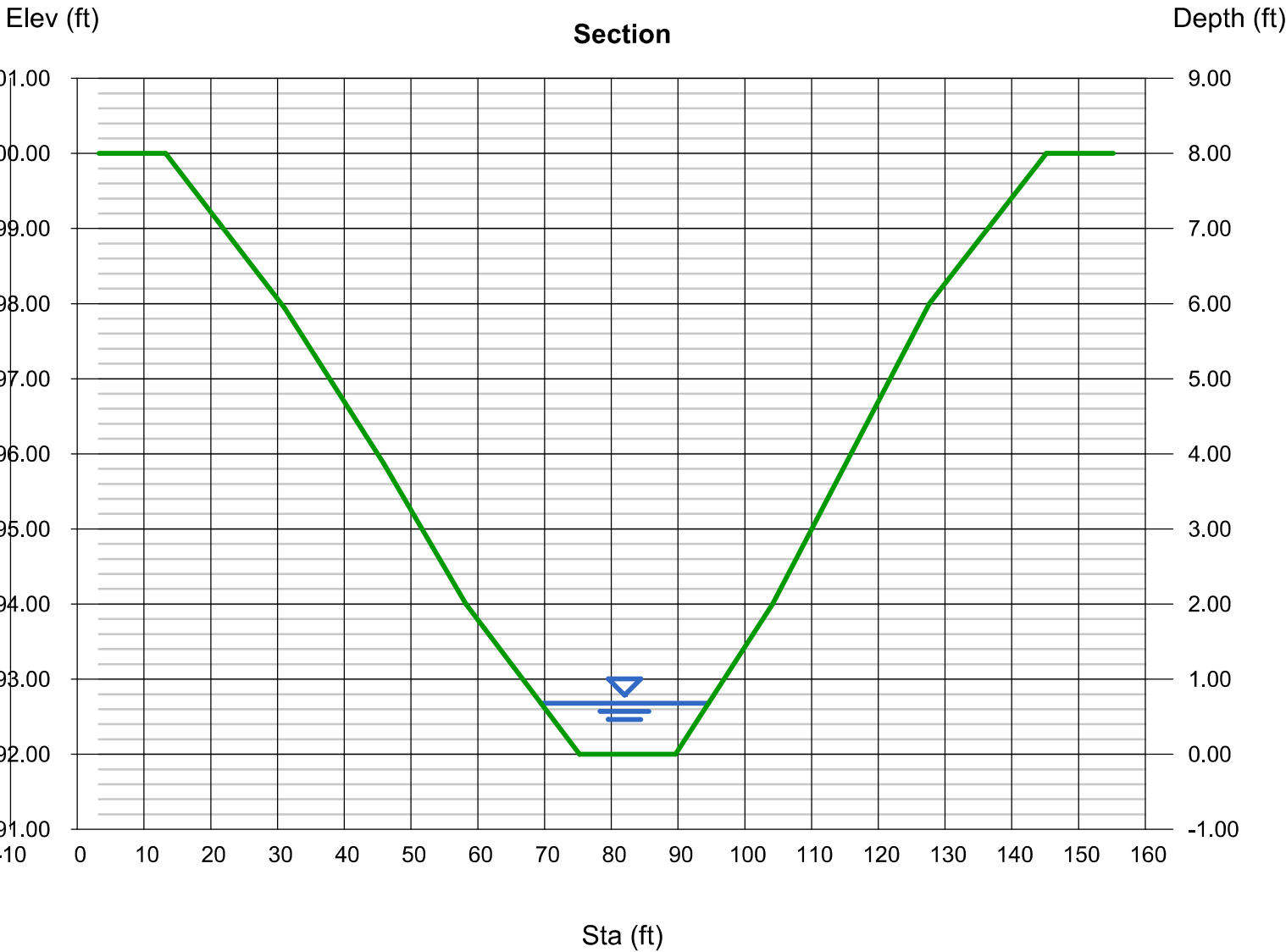
Compute by: Known Q
Known Q (cfs) = 70.20

Highlighted

Depth (ft) = 0.68
Q (cfs) = 70.20
Area (sqft) = 13.44
Velocity (ft/s) = 5.22
Wetted Perim (ft) = 25.20
Crit Depth, Yc (ft) = 0.78
Top Width (ft) = 25.12
EGL (ft) = 1.10

(Sta, El, n)-(Sta, El, n)...

(13.24, 7600.00)-(31.05, 7597.94, 0.030)-(45.93, 7595.87, 0.030)-(58.22, 7594.00, 0.030)-(75.23, 7592.00, 0.030)-(89.62, 7592.00, 0.030)-(104.15, 7594.00, 0.030)-
-(127.62, 7598.00, 0.030)-(145.20, 7600.00, 0.030)



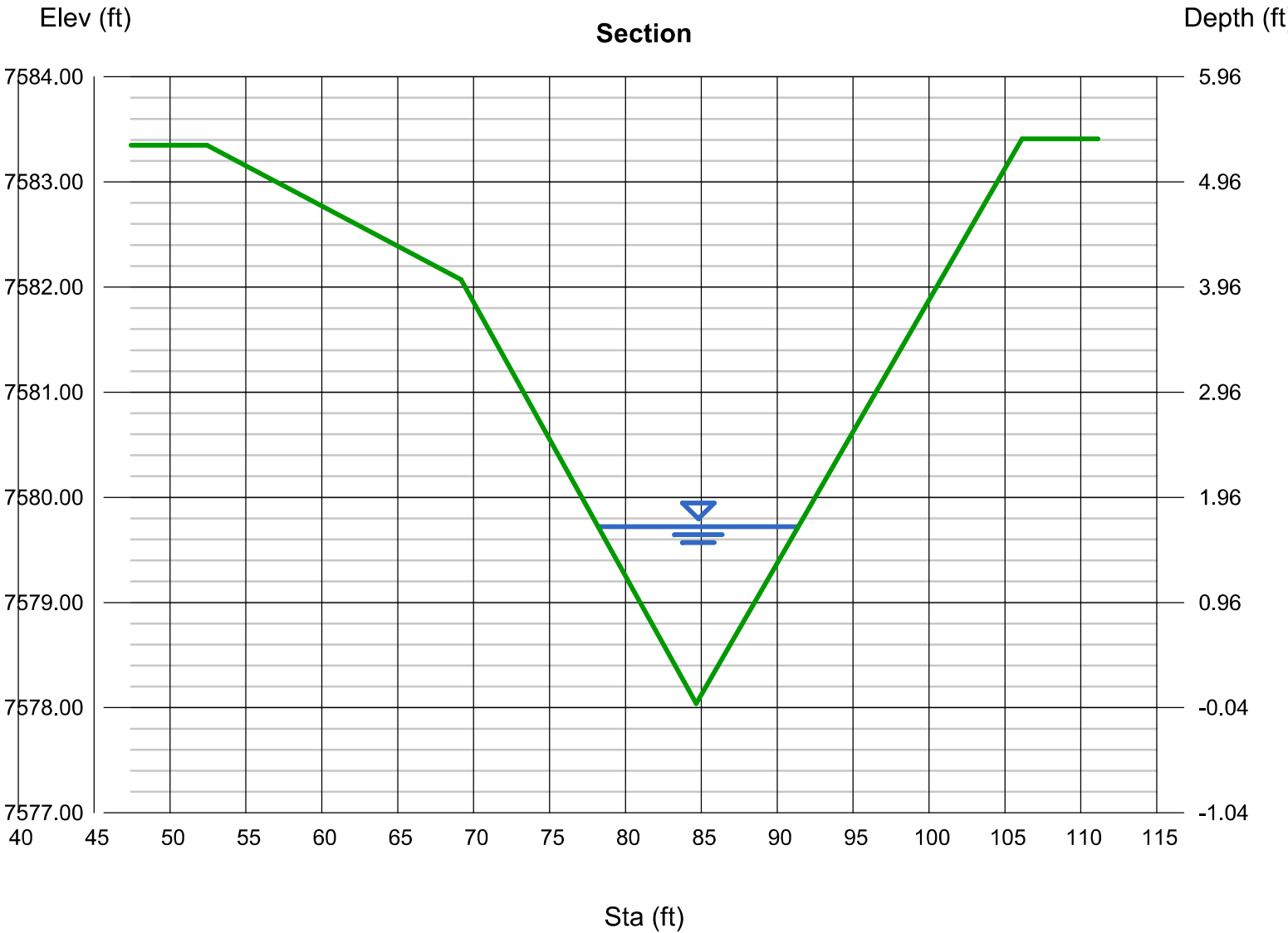
Channel Report

Channel Section KK - 5Yr Storm

User-defined		Highlighted	
Invert Elev (ft)	= 7578.04	Depth (ft)	= 1.68
Slope (%)	= 1.00	Q (cfs)	= 47.10
N-Value	= 0.030	Area (sqft)	= 11.07
		Velocity (ft/s)	= 4.26
		Wetted Perim (ft)	= 13.59
		Crit Depth, Yc (ft)	= 1.56
		Top Width (ft)	= 13.17
		EGL (ft)	= 1.96

(Sta, El, n)-(Sta, El, n)...

(52.43, 7583.35)-(69.18, 7582.07, 0.030)-(84.67, 7578.04, 0.030)-(106.13, 7583.41, 0.030)



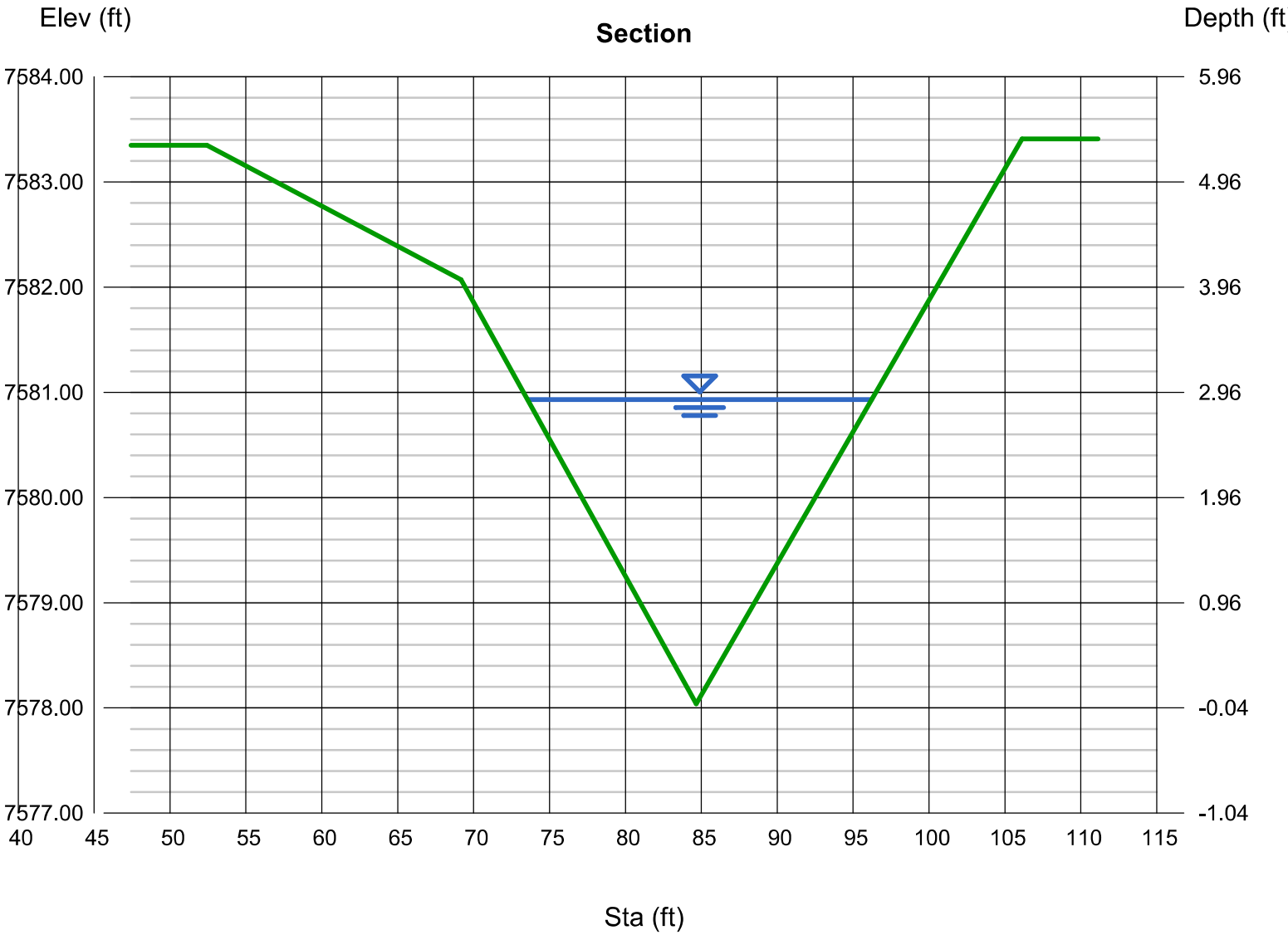
Channel Report

Channel Section KK - 100Yr Storm

User-defined		Highlighted	
Invert Elev (ft)	= 7578.04	Depth (ft)	= 2.89
Slope (%)	= 1.00	Q (cfs)	= 202.30
N-Value	= 0.030	Area (sqft)	= 32.74
		Velocity (ft/s)	= 6.18
		Wetted Perim (ft)	= 23.38
		Crit Depth, Yc (ft)	= 2.78
		Top Width (ft)	= 22.66
		EGL (ft)	= 3.48

(Sta, El, n)-(Sta, El, n)...

(52.43, 7583.35)-(69.18, 7582.07, 0.030)-(84.67, 7578.04, 0.030)-(106.13, 7583.41, 0.030)



Channel Report

Channel Section I1 - Proposed 5-Yr

User-defined

Invert Elev (ft)

= 7586.00

Slope (%)

= 4.00

N-Value

= 0.030

Highlighted

Depth (ft)

= 0.19

Q (cfs)

= 16.70

Area (sqft)

= 5.39

Velocity (ft/s)

= 3.10

Wetted Perim (ft)

= 29.47

Crit Depth, Yc (ft)

= 0.23

Top Width (ft)

= 29.43

EGL (ft)

= 0.34

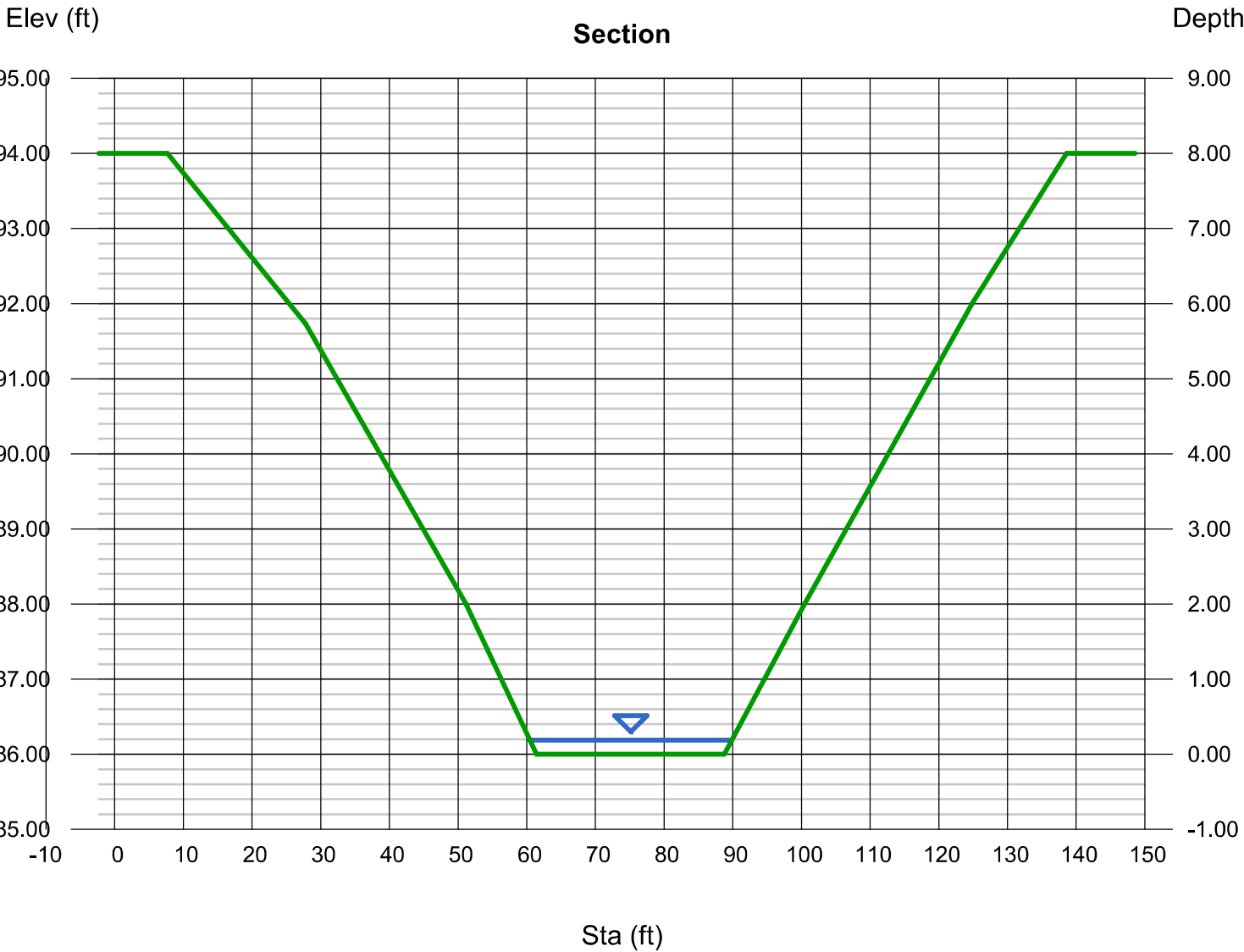
Calculations

Compute by: Known Q

Known Q (cfs) = 16.70

(Sta, El, n)-(Sta, El, n)...

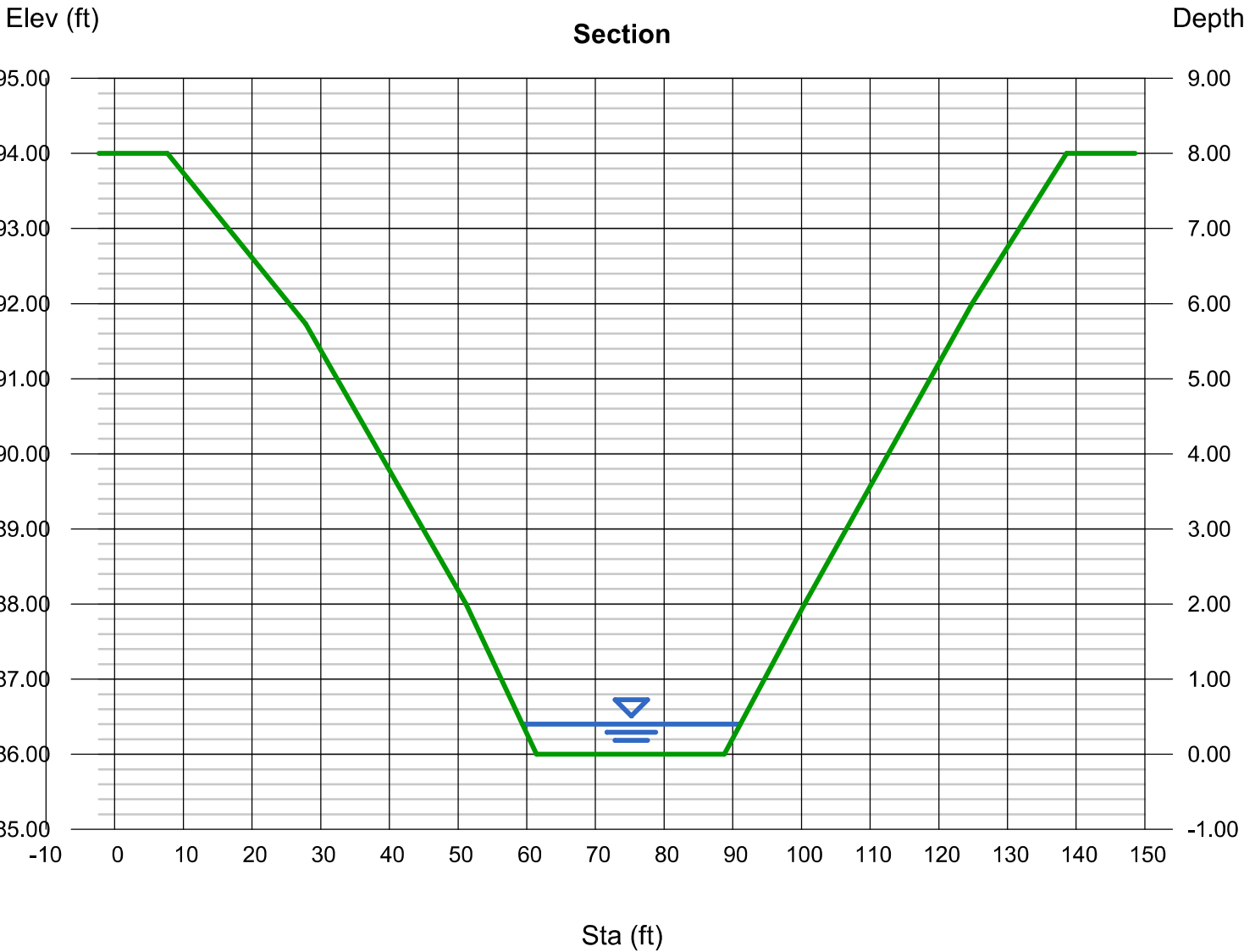
(7.70, 7594.00)-(27.80, 7591.73, 0.030)-(51.15, 7588.00, 0.030)-(61.43, 7586.00, 0.030)-(88.78, 7586.00, 0.030)-(100.44, 7588.00, 0.030)-(124.84, 7592.00, 0.030)-(-138.60, 7594.00, 0.030)



Channel Report

Channel Section I1 - Proposed 100-Yr

User-defined		Highlighted	
Invert Elev (ft)	= 7586.00	Depth (ft)	= 0.40
Slope (%)	= 4.00	Q (cfs)	= 58.70
N-Value	= 0.030	Area (sqft)	= 11.81
Calculations		Velocity (ft/s)	= 4.97
Compute by:	Known Q	Wetted Perim (ft)	= 31.81
Known Q (cfs)	= 58.70	Crit Depth, Yc (ft)	= 0.51
		Top Width (ft)	= 31.74
		EGL (ft)	= 0.78
(Sta, El, n)-(Sta, El, n)...			
(7.70, 7594.00)-(27.80, 7591.73, 0.030)-(51.15, 7588.00, 0.030)-(61.43, 7586.00, 0.030)-(88.78, 7586.00, 0.030)-(100.44, 7588.00, 0.030)-(124.84, 7592.00, 0.030)-			
-(138.60, 7594.00, 0.030)			



Channel Report

Channel Section J1 5Yr - Left Side (Basin J11)

Triangular

Side Slopes (z:1) = 3.00, 4.00
Total Depth (ft) = 2.50

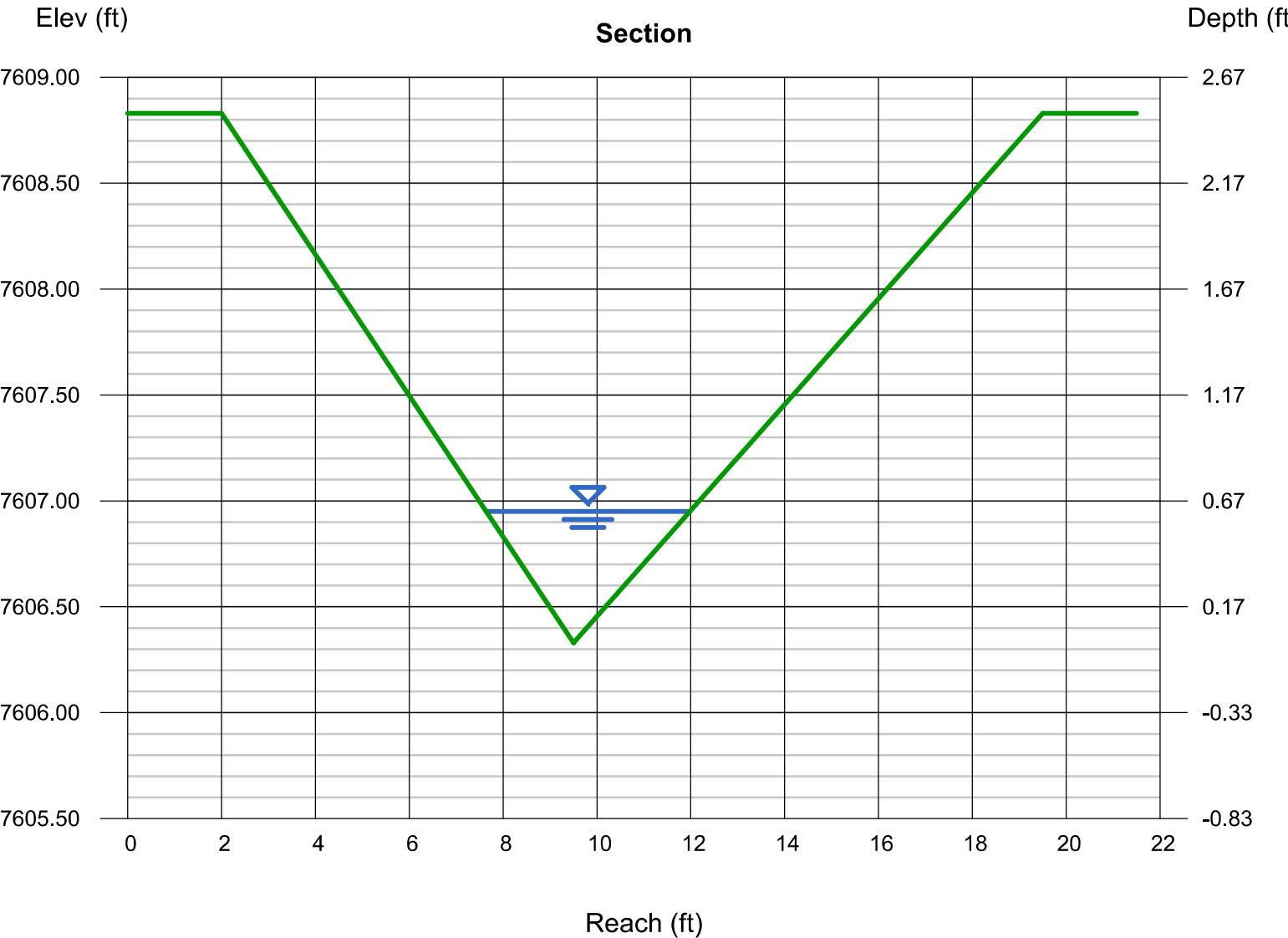
Invert Elev (ft) = 7606.33
Slope (%) = 1.00
N-Value = 0.030

Calculations

Compute by: Known Q
Known Q (cfs) = 2.90

Highlighted

Depth (ft) = 0.62
Q (cfs) = 2.900
Area (sqft) = 1.35
Velocity (ft/s) = 2.16
Wetted Perim (ft) = 4.52
Crit Depth, Yc (ft) = 0.54
Top Width (ft) = 4.34
EGL (ft) = 0.69



Channel Report

Channel Section J1 100Yr - Left Side (Basin J11)

Triangular

Side Slopes (z:1) = 3.00, 4.00
Total Depth (ft) = 2.50

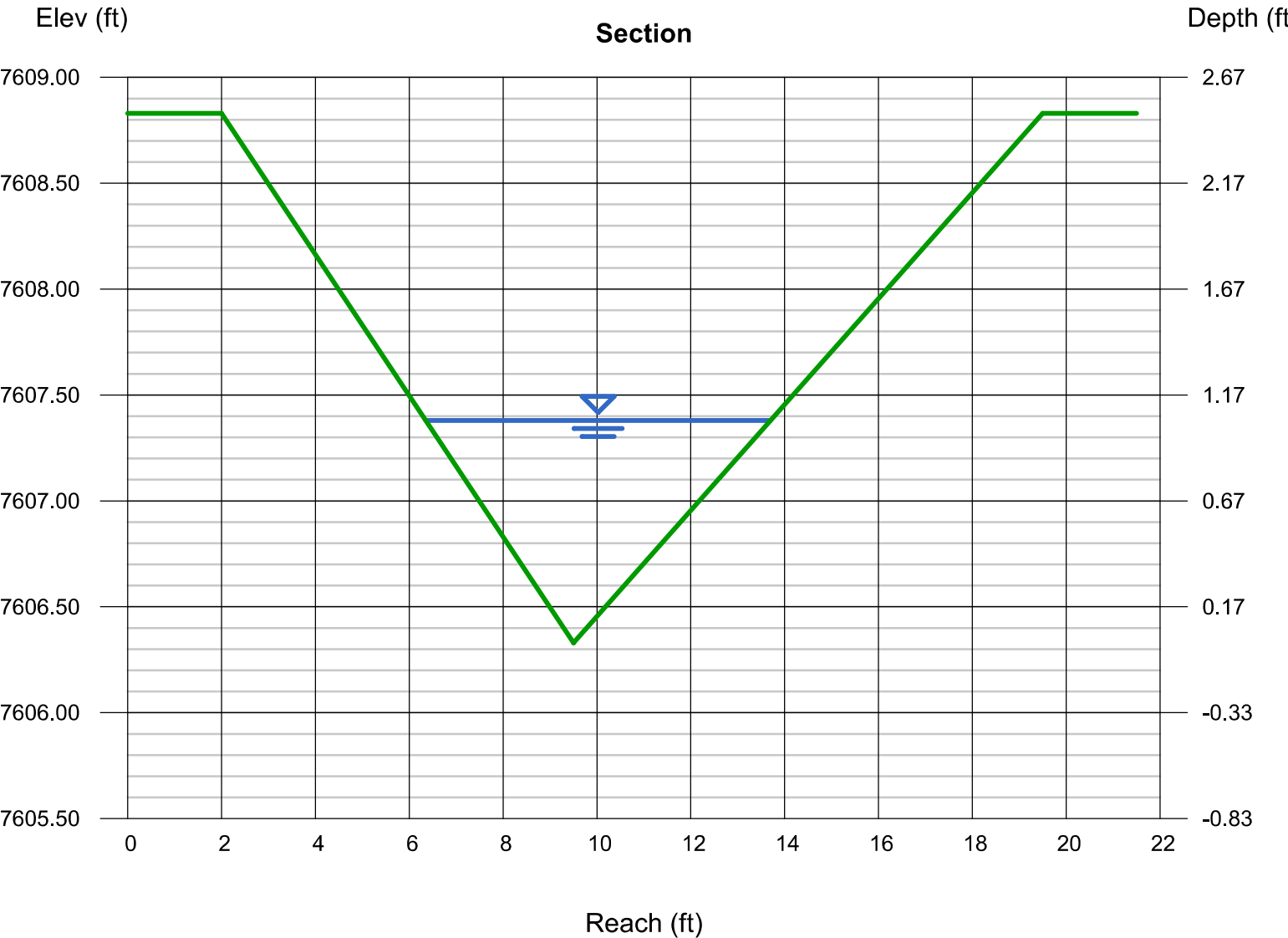
Invert Elev (ft) = 7606.33
Slope (%) = 1.00
N-Value = 0.030

Calculations

Compute by: Known Q
Known Q (cfs) = 12.10

Highlighted

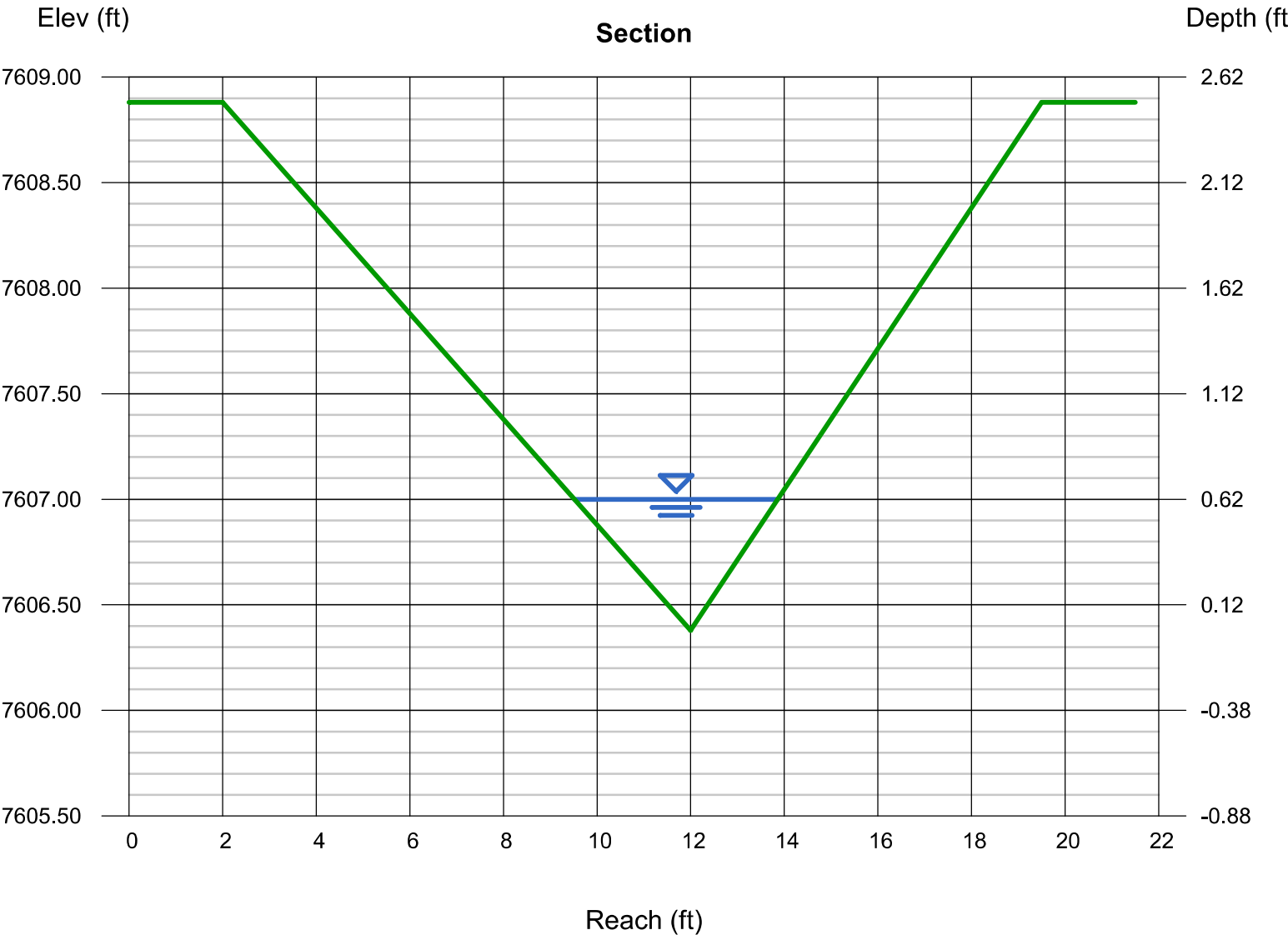
Depth (ft) = 1.05
Q (cfs) = 12.10
Area (sqft) = 3.86
Velocity (ft/s) = 3.14
Wetted Perim (ft) = 7.65
Crit Depth, Yc (ft) = 0.95
Top Width (ft) = 7.35
EGL (ft) = 1.20



Channel Report

Channel Section J1 5Yr - Right Side (Basin J11)

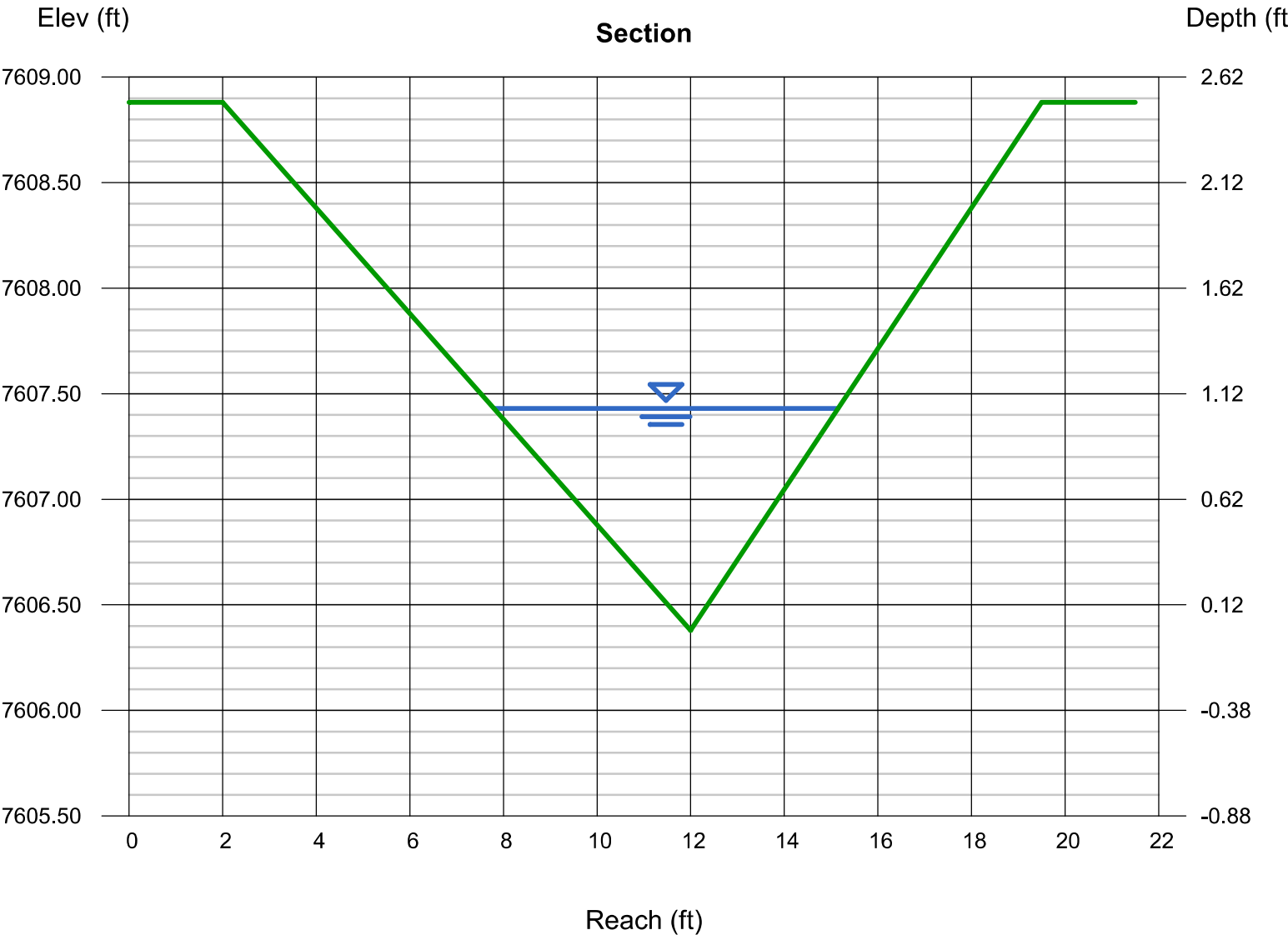
Triangular		Highlighted	
Side Slopes (z:1)	= 4.00, 3.00	Depth (ft)	= 0.62
Total Depth (ft)	= 2.50	Q (cfs)	= 2.900
		Area (sqft)	= 1.35
Invert Elev (ft)	= 7606.38	Velocity (ft/s)	= 2.16
Slope (%)	= 1.00	Wetted Perim (ft)	= 4.52
N-Value	= 0.030	Crit Depth, Yc (ft)	= 0.54
		Top Width (ft)	= 4.34
		EGL (ft)	= 0.69
Calculations			
Compute by:	Known Q		
Known Q (cfs)	= 2.90		



Channel Report

Channel Section J1 100Yr - Right Side (Basin J11)

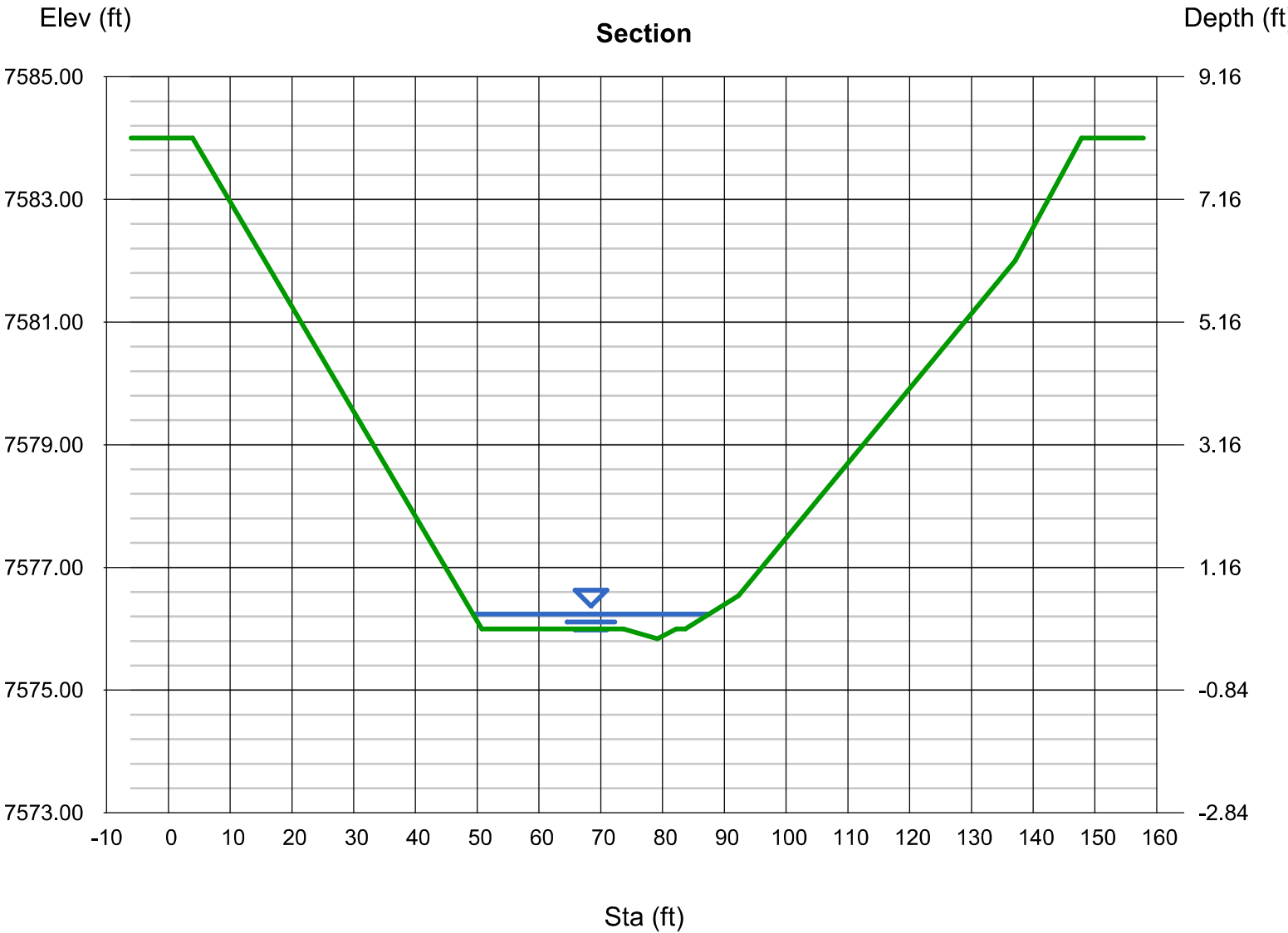
Triangular		Highlighted	
Side Slopes (z:1)	= 4.00, 3.00	Depth (ft)	= 1.05
Total Depth (ft)	= 2.50	Q (cfs)	= 12.10
		Area (sqft)	= 3.86
Invert Elev (ft)	= 7606.38	Velocity (ft/s)	= 3.14
Slope (%)	= 1.00	Wetted Perim (ft)	= 7.65
N-Value	= 0.030	Crit Depth, Yc (ft)	= 0.95
		Top Width (ft)	= 7.35
		EGL (ft)	= 1.20
Calculations			
Compute by:	Known Q		
Known Q (cfs)	= 12.10		



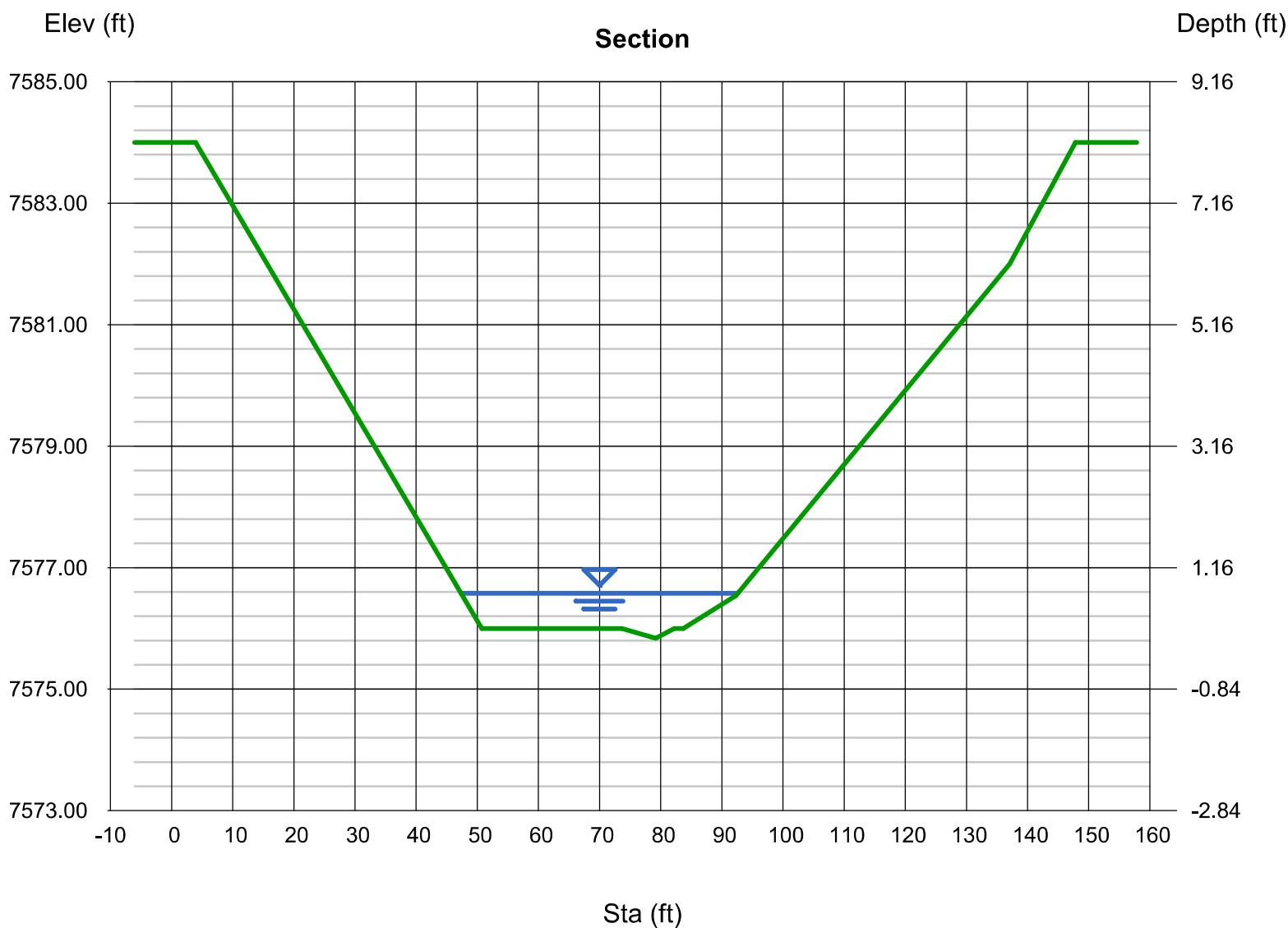
Channel Report

Channel Section J5 - 5Yr Storm

User-defined		Highlighted	
Invert Elev (ft)	= 7575.84	Depth (ft)	= 0.40
Slope (%)	= 3.00	Q (cfs)	= 28.90
N-Value	= 0.030	Area (sqft)	= 9.21
Calculations Compute by: Known Q Known Q (cfs) = 28.90		Velocity (ft/s)	= 3.14
		Wetted Perim (ft)	= 38.21
		Crit Depth, Yc (ft)	= 0.43
		Top Width (ft)	= 38.18
		EGL (ft)	= 0.55
(Sta, El, n)-(Sta, El, n)... (3.95, 7584.00)-(50.74, 7576.00, 0.030)-(73.66, 7576.00, 0.030)-(79.15, 7575.84, 0.030)-(82.24, 7576.00, 0.030)-(83.70, 7576.00, 0.030)-(92.30, 7576.54, 0.030) -(137.09, 7582.00, 0.030)-(147.84, 7584.00, 0.030)			



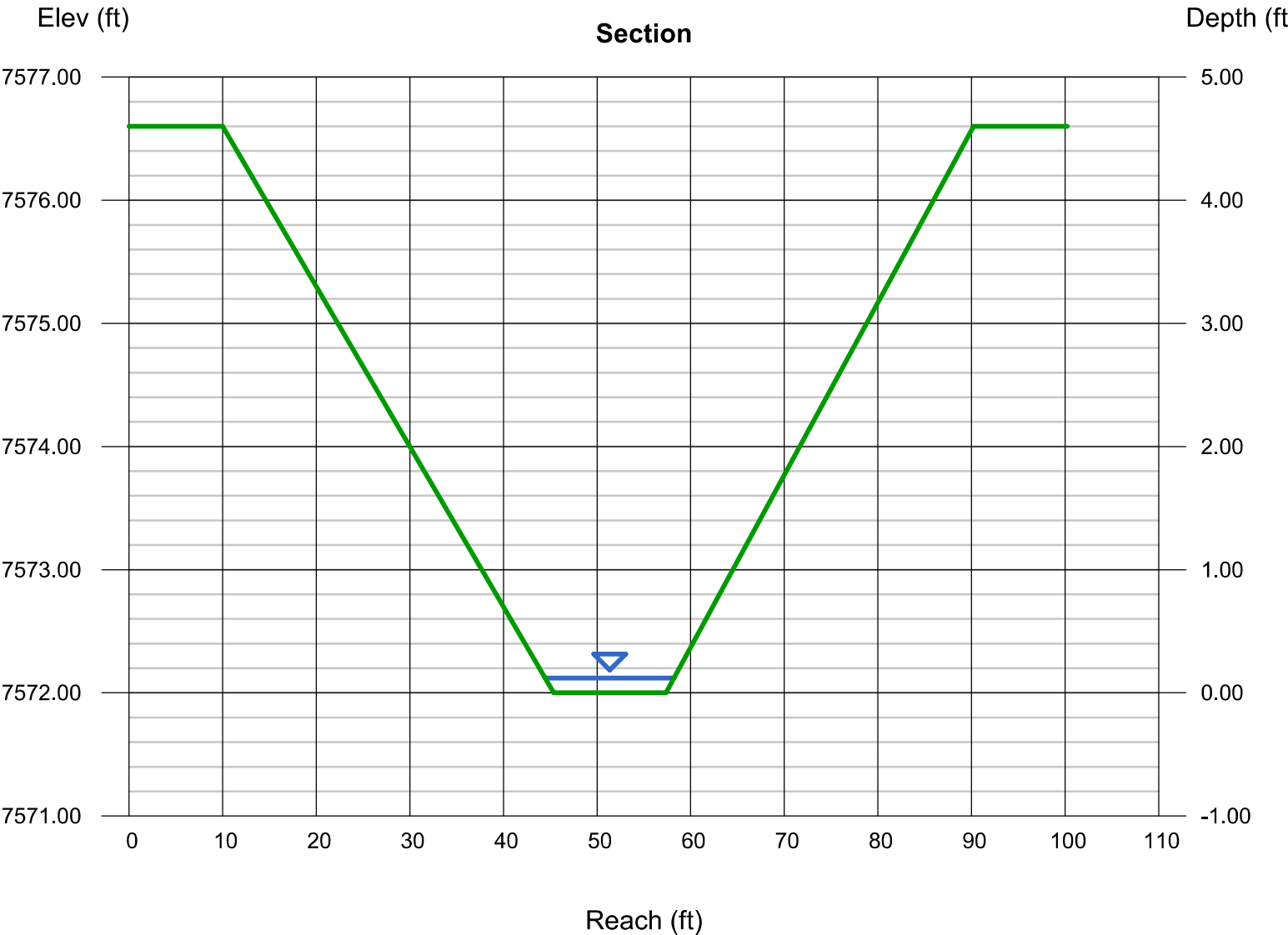
Friday, Mar 27 2026



Channel Report

Channel Section S1 - 5Yr Storm

Trapezoidal		Highlighted	
Bottom Width (ft)	= 12.00	Depth (ft)	= 0.12
Side Slopes (z:1)	= 7.69, 7.14	Q (cfs)	= 4.060
Total Depth (ft)	= 4.60	Area (sqft)	= 1.55
Invert Elev (ft)	= 7572.00	Velocity (ft/s)	= 2.62
Slope (%)	= 8.00	Wetted Perim (ft)	= 13.80
N-Value	= 0.035	Crit Depth, Yc (ft)	= 0.15
Calculations		Top Width (ft)	= 13.78
Compute by:	Known Q	EGL (ft)	= 0.23
Known Q (cfs)	= 4.06		



Channel Report

Channel Section S1 - 100Yr Storm

Trapezoidal

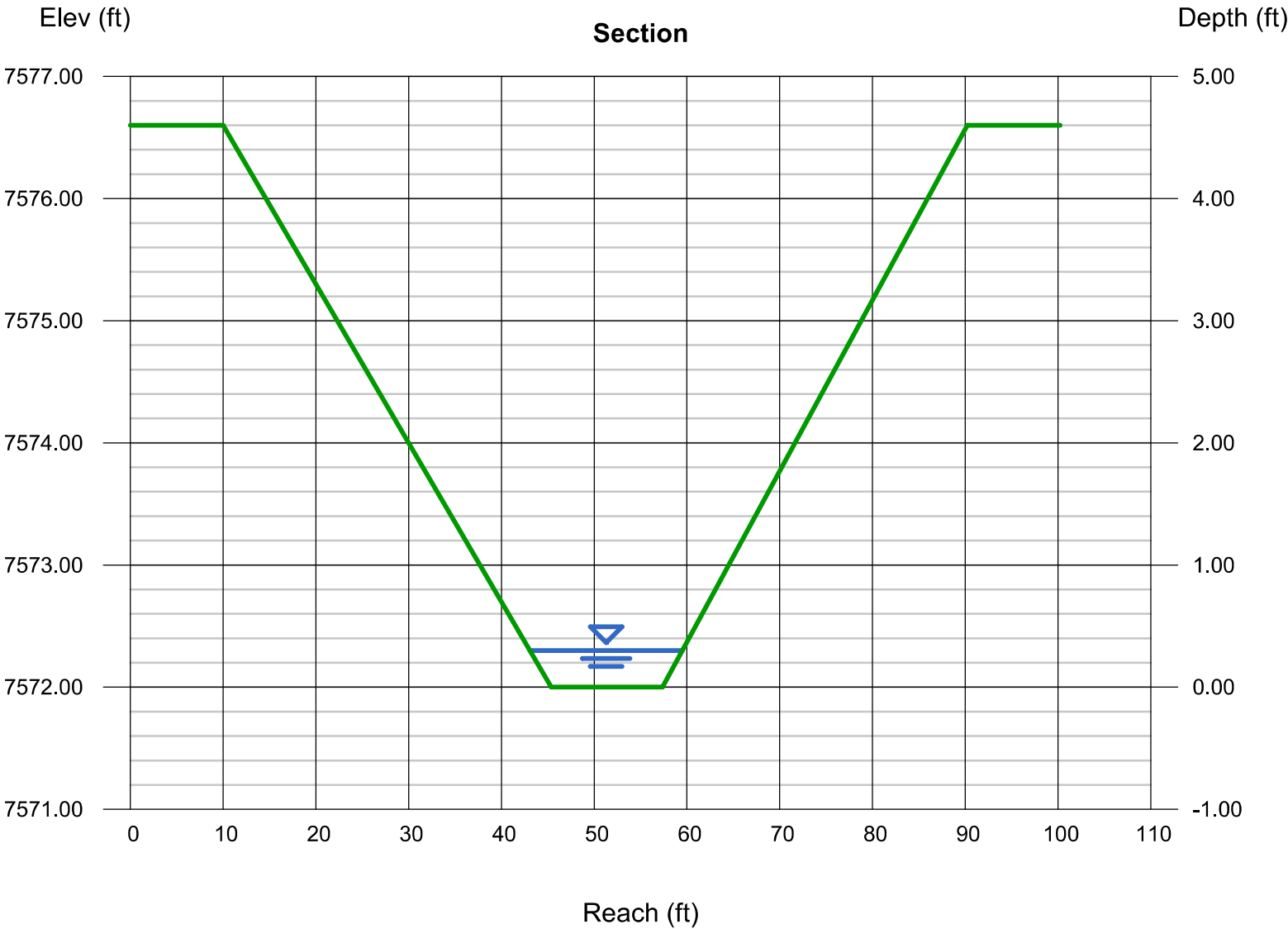
Bottom Width (ft)	= 12.00
Side Slopes (z:1)	= 7.69, 7.14
Total Depth (ft)	= 4.60
Invert Elev (ft)	= 7572.00
Slope (%)	= 8.00
N-Value	= 0.035

Highlighted

Depth (ft)	= 0.30
Q (cfs)	= 20.37
Area (sqft)	= 4.27
Velocity (ft/s)	= 4.77
Wetted Perim (ft)	= 16.49
Crit Depth, Yc (ft)	= 0.41
Top Width (ft)	= 16.45
EGL (ft)	= 0.65

Calculations

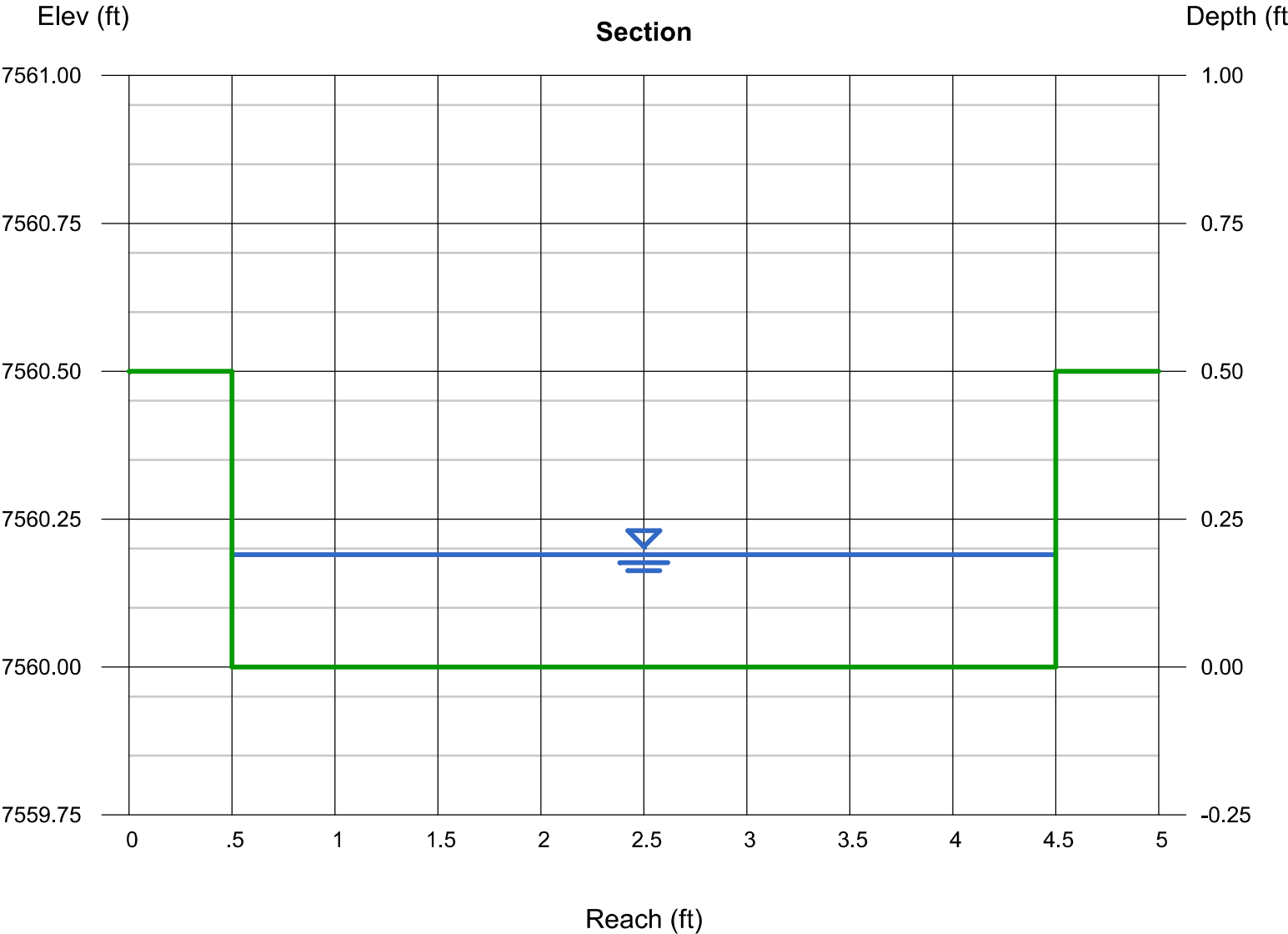
Compute by:	Known Q
Known Q (cfs)	= 20.37



Channel Report

Side Trickle Channel Sizing - 2Yr Flow

Rectangular		Highlighted	
Bottom Width (ft)	= 4.00	Depth (ft)	= 0.19
Total Depth (ft)	= 0.50	Q (cfs)	= 2.126
		Area (sqft)	= 0.76
Invert Elev (ft)	= 7560.00	Velocity (ft/s)	= 2.80
Slope (%)	= 0.50	Wetted Perim (ft)	= 4.38
N-Value	= 0.011	Crit Depth, Yc (ft)	= 0.21
		Top Width (ft)	= 4.00
		EGL (ft)	= 0.31
Calculations			
Compute by:	Known Q		
Known Q (cfs)	= 2.13		



Channel Report

Main Trickle Channel Sizing - 2Yr Flow

Rectangular

Bottom Width (ft) = 6.00
Total Depth (ft) = 0.50

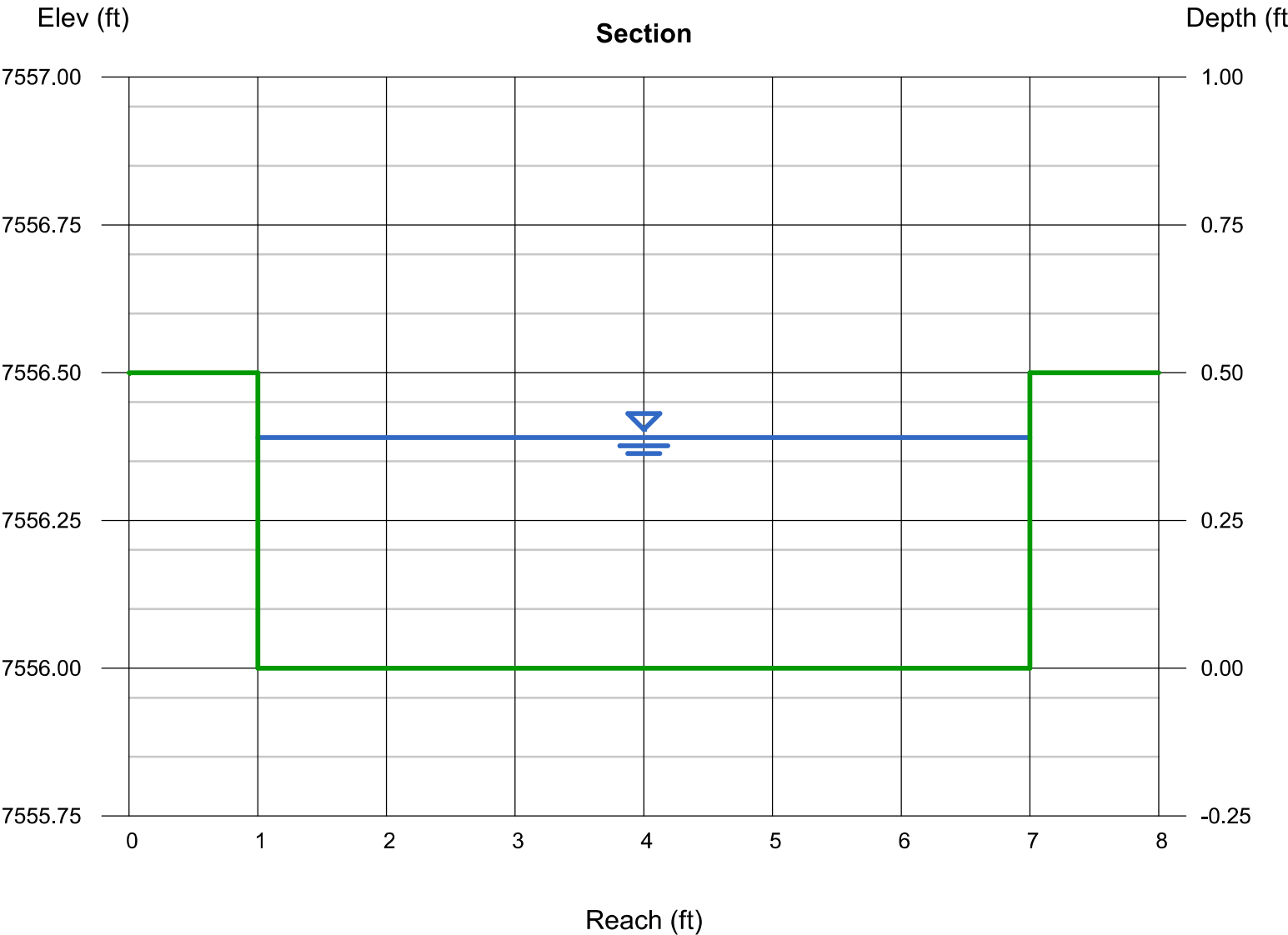
Invert Elev (ft) = 7556.00
Slope (%) = 0.50
N-Value = 0.011

Calculations

Compute by: Known Q
Known Q (cfs) = 10.90

Highlighted

Depth (ft) = 0.39
Q (cfs) = 10.90
Area (sqft) = 2.34
Velocity (ft/s) = 4.66
Wetted Perim (ft) = 6.78
Crit Depth, Yc (ft) = 0.47
Top Width (ft) = 6.00
EGL (ft) = 0.73



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Wednesday, Oct 8 2025

CULVERT 1 5 YR

Invert Elev Dn (ft) = 7579.62
Pipe Length (ft) = 60.38
Slope (%) = 2.73
Invert Elev Up (ft) = 7581.27
Rise (in) = 18.0
Shape = Circular
Span (in) = 18.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

Top Elevation (ft) = 7588.00
Top Width (ft) = 32.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 2.90
Qmax (cfs) = 2.90
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 2.90
Qpipe (cfs) = 2.90
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 2.14
Veloc Up (ft/s) = 3.98
HGL Dn (ft) = 7580.69
HGL Up (ft) = 7581.92
Hw Elev (ft) = 7582.17
Hw/D (ft) = 0.60
Flow Regime = Inlet Control



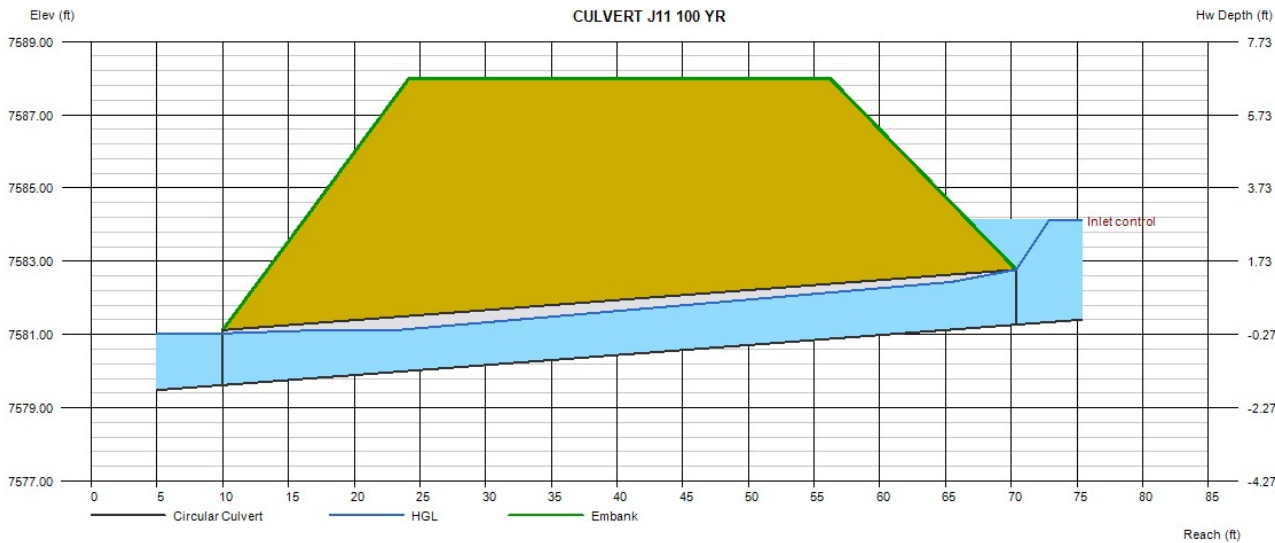
Culvert Report

CULVERT 1 100 YR

Invert Elev Dn (ft)	= 7579.62
Pipe Length (ft)	= 60.38
Slope (%)	= 2.73
Invert Elev Up (ft)	= 7581.27
Rise (in)	= 18.0
Shape	= Circular
Span (in)	= 18.0
No. Barrels	= 1
n-Value	= 0.013
Culvert Type	= Circular Concrete
Culvert Entrance	= Square edge w/headwall (C)
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5

Embankment	
Top Elevation (ft)	= 7588.00
Top Width (ft)	= 32.00
Crest Width (ft)	= 0.00

Calculations	
Qmin (cfs)	= 12.10
Qmax (cfs)	= 12.10
Tailwater Elev (ft)	= (dc+D)/2
Highlighted	
Qtotal (cfs)	= 12.10
Qpipe (cfs)	= 12.10
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 7.02
Veloc Up (ft/s)	= 7.36
HGL Dn (ft)	= 7581.03
HGL Up (ft)	= 7582.59
Hw Elev (ft)	= 7584.12
Hw/D (ft)	= 1.90
Flow Regime	= Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Wednesday, Apr 1 2026

CULVERT 2 5 YR

Invert Elev Dn (ft) = 7596.00
Pipe Length (ft) = 180.51
Slope (%) = 2.22
Invert Elev Up (ft) = 7600.00
Rise (in) = 42.0
Shape = Circular
Span (in) = 42.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

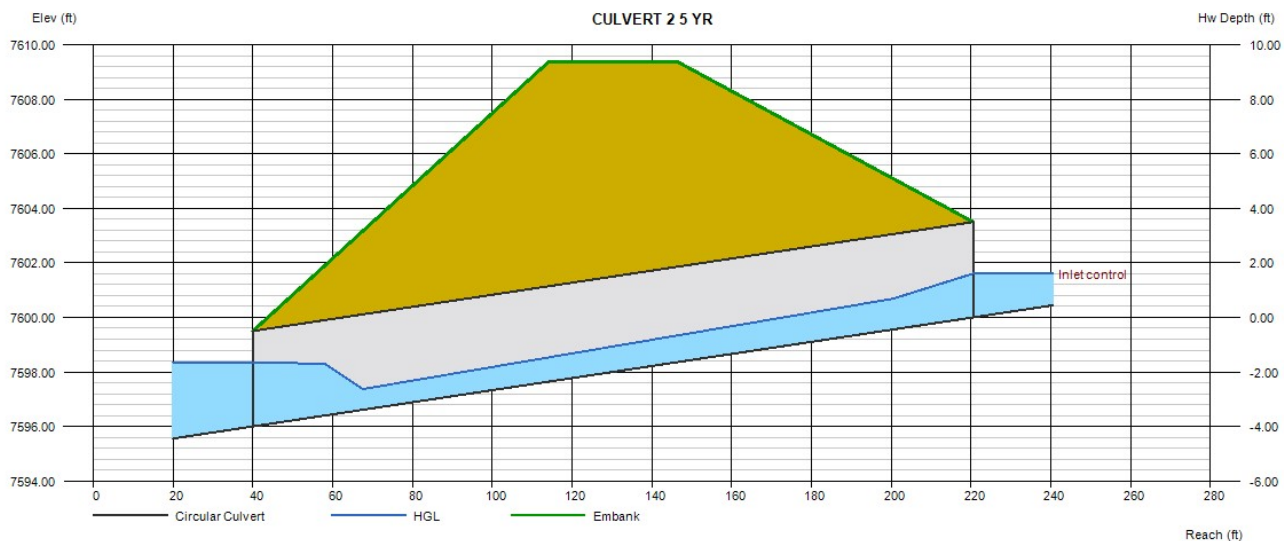
Top Elevation (ft) = 7609.39
Top Width (ft) = 32.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 15.20
Qmax (cfs) = 15.20
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 15.20
Qpipe (cfs) = 15.20
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 2.22
Veloc Up (ft/s) = 5.29
HGL Dn (ft) = 7598.34
HGL Up (ft) = 7601.19
Hw Elev (ft) = 7601.61
Hw/D (ft) = 0.46
Flow Regime = Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Wednesday, Apr 1 2026

CULVERT 2 100 YR

Invert Elev Dn (ft) = 7596.00
Pipe Length (ft) = 180.51
Slope (%) = 2.22
Invert Elev Up (ft) = 7600.00
Rise (in) = 42.0
Shape = Circular
Span (in) = 42.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

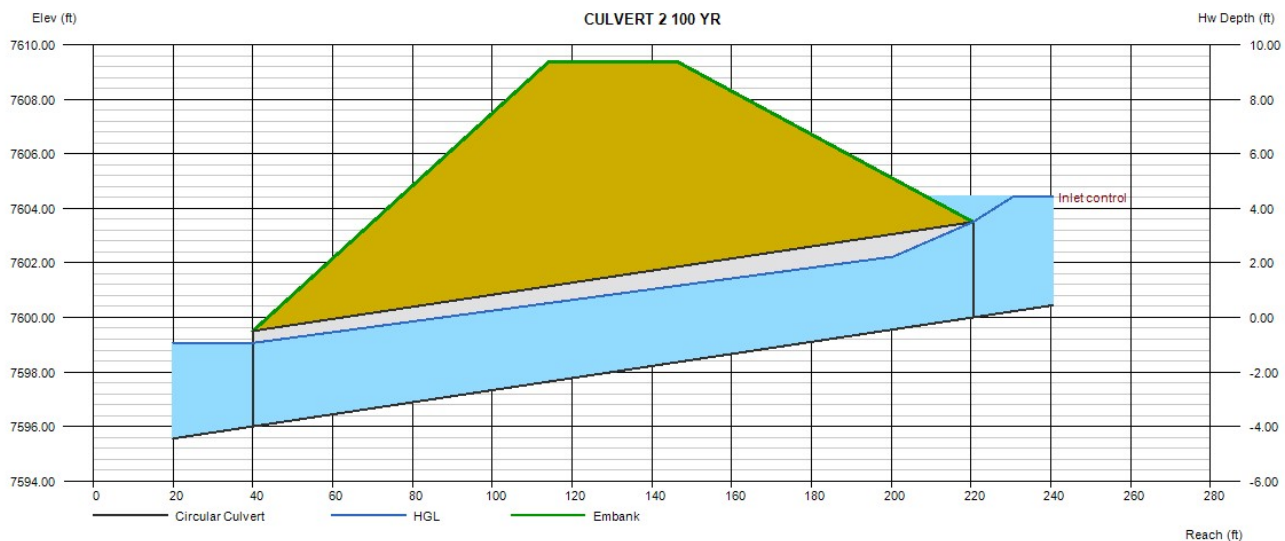
Top Elevation (ft) = 7609.39
Top Width (ft) = 32.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 70.20
Qmax (cfs) = 70.20
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 70.20
Qpipe (cfs) = 70.20
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 7.86
Veloc Up (ft/s) = 9.07
HGL Dn (ft) = 7599.06
HGL Up (ft) = 7602.62
Hw Elev (ft) = 7604.43
Hw/D (ft) = 1.26
Flow Regime = Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Monday, Oct 6 2025

CULVERT 3 5 YR

Invert Elev Dn (ft) = 7598.00
Pipe Length (ft) = 177.92
Slope (%) = 2.31
Invert Elev Up (ft) = 7602.11
Rise (in) = 42.0
Shape = Circular
Span (in) = 42.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

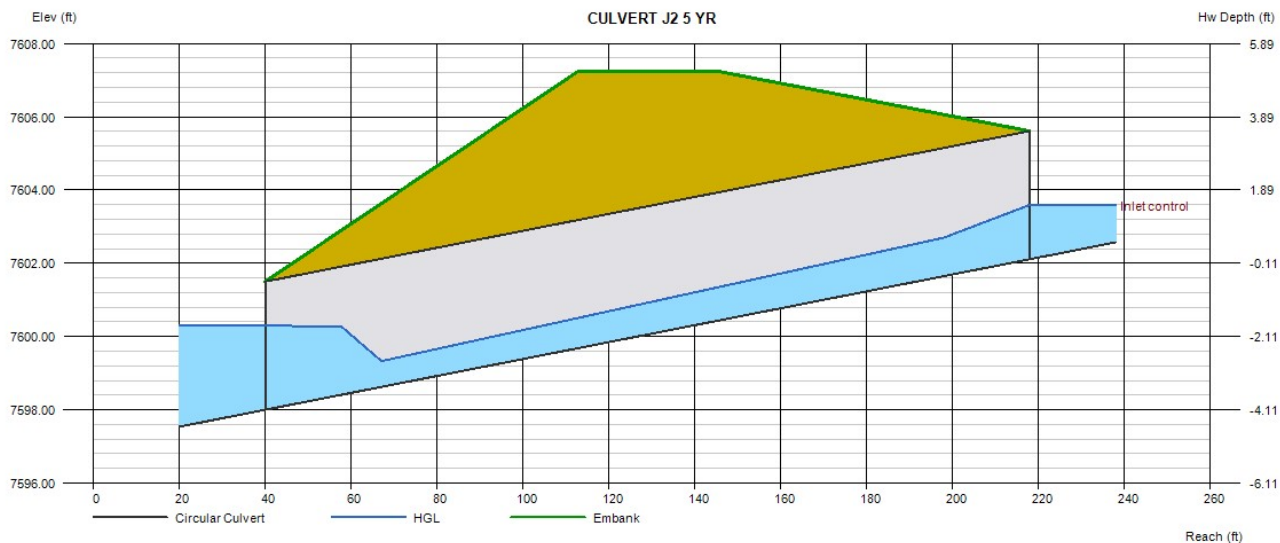
Top Elevation (ft) = 7607.25
Top Width (ft) = 32.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 13.20
Qmax (cfs) = 13.20
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 13.20
Qpipe (cfs) = 13.20
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 1.97
Veloc Up (ft/s) = 5.08
HGL Dn (ft) = 7600.30
HGL Up (ft) = 7603.21
Hw Elev (ft) = 7603.59
Hw/D (ft) = 0.42
Flow Regime = Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Monday, Oct 6 2025

CULVERT 3 100 YR

Invert Elev Dn (ft) = 7598.00
Pipe Length (ft) = 177.92
Slope (%) = 2.31
Invert Elev Up (ft) = 7602.11
Rise (in) = 42.0
Shape = Circular
Span (in) = 42.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

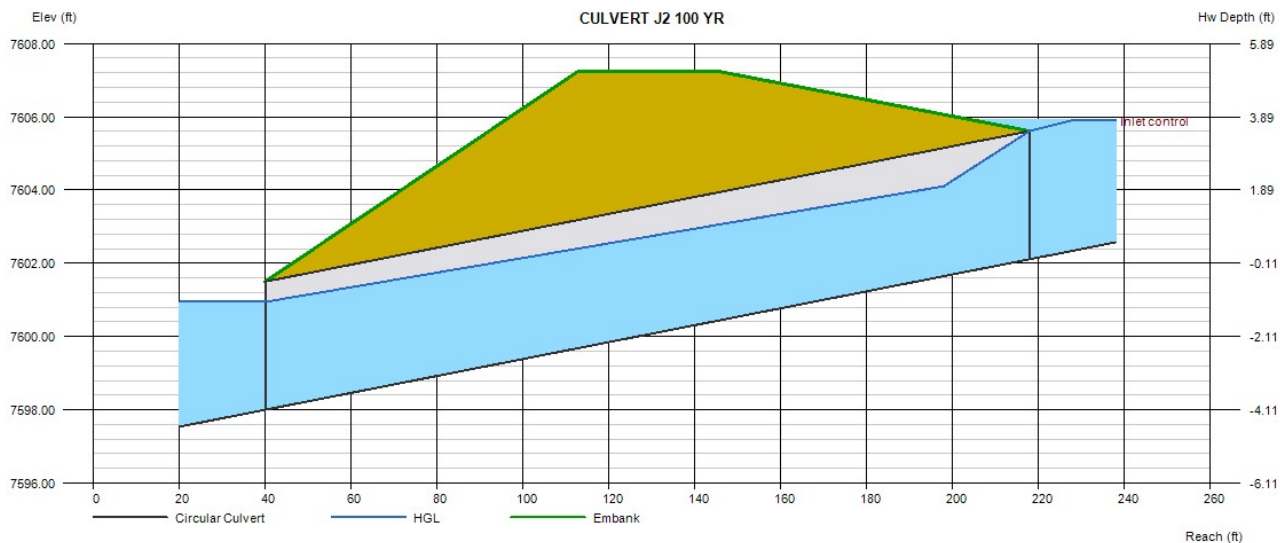
Top Elevation (ft) = 7607.25
Top Width (ft) = 32.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 58.40
Qmax (cfs) = 58.40
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 58.40
Qpipe (cfs) = 58.40
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 6.76
Veloc Up (ft/s) = 8.34
HGL Dn (ft) = 7600.95
HGL Up (ft) = 7604.50
Hw Elev (ft) = 7605.90
Hw/D (ft) = 1.08
Flow Regime = Inlet Control



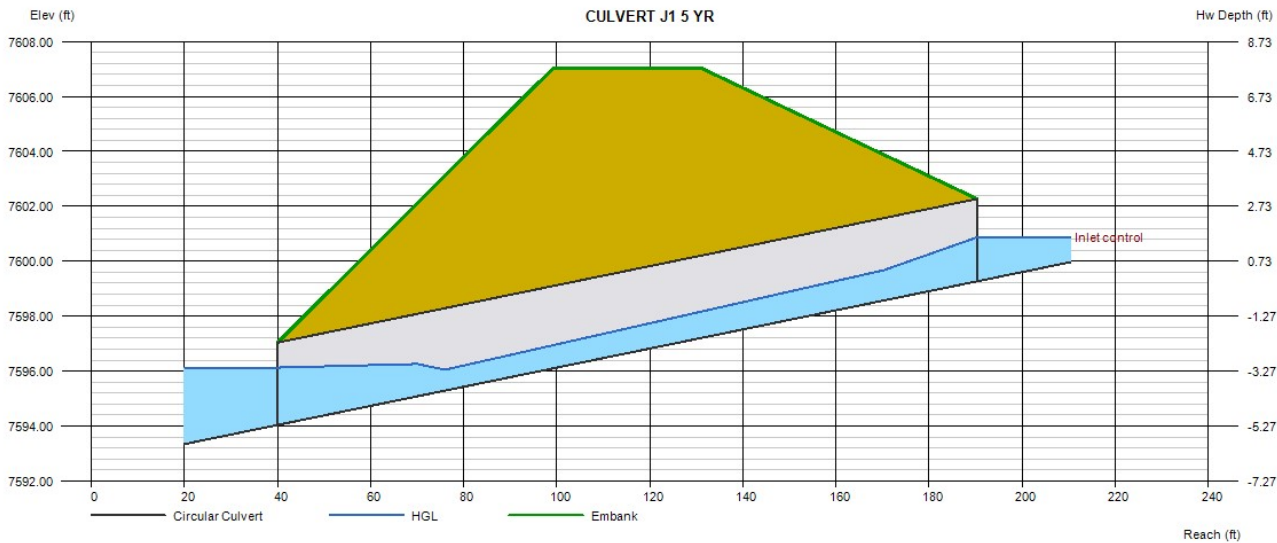
Culvert Report

CULVERT 4 5 YR

Invert Elev Dn (ft)	= 7594.04
Pipe Length (ft)	= 150.30
Slope (%)	= 3.48
Invert Elev Up (ft)	= 7599.27
Rise (in)	= 36.0
Shape	= Circular
Span (in)	= 36.0
No. Barrels	= 1
n-Value	= 0.013
Culvert Type	= Circular Concrete
Culvert Entrance	= Square edge w/headwall (C)
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5

Embankment	
Top Elevation (ft)	= 7607.02
Top Width (ft)	= 32.00
Crest Width (ft)	= 0.00

Calculations	
Qmin (cfs)	= 13.70
Qmax (cfs)	= 13.70
Tailwater Elev (ft)	= (dc+D)/2
Highlighted	
Qtotal (cfs)	= 13.70
Qpipe (cfs)	= 13.70
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 2.61
Veloc Up (ft/s)	= 5.32
HGL Dn (ft)	= 7596.13
HGL Up (ft)	= 7600.45
Hw Elev (ft)	= 7600.87
Hw/D (ft)	= 0.53
Flow Regime	= Inlet Control



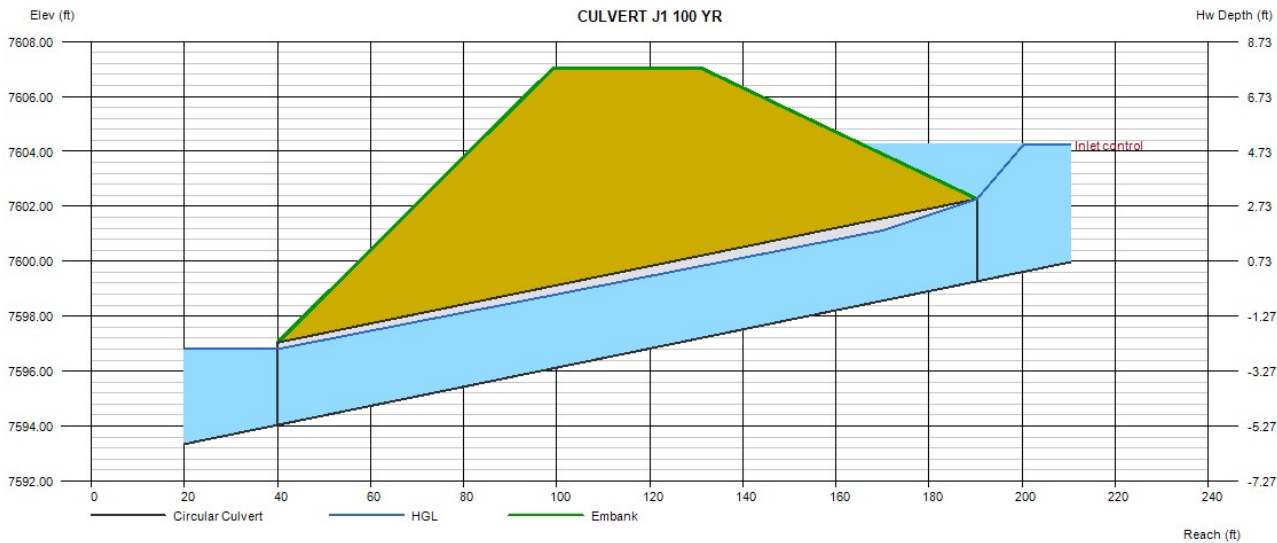
Culvert Report

CULVERT 4 100 YR

Invert Elev Dn (ft)	= 7594.04
Pipe Length (ft)	= 150.30
Slope (%)	= 3.48
Invert Elev Up (ft)	= 7599.27
Rise (in)	= 36.0
Shape	= Circular
Span (in)	= 36.0
No. Barrels	= 1
n-Value	= 0.013
Culvert Type	= Circular Concrete
Culvert Entrance	= Square edge w/headwall (C)
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5

Embankment	
Top Elevation (ft)	= 7607.02
Top Width (ft)	= 32.00
Crest Width (ft)	= 0.00

Calculations	
Qmin (cfs)	= 61.60
Qmax (cfs)	= 61.60
Tailwater Elev (ft)	= (dc+D)/2
Highlighted	
Qtotal (cfs)	= 61.60
Qpipe (cfs)	= 61.60
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 9.05
Veloc Up (ft/s)	= 9.69
HGL Dn (ft)	= 7596.80
HGL Up (ft)	= 7601.80
Hw Elev (ft)	= 7604.25
Hw/D (ft)	= 1.66
Flow Regime	= Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Mar 31 2026

CULVERT 5 5 YR

Invert Elev Dn (ft) = 7613.78
Pipe Length (ft) = 85.92
Slope (%) = 2.96
Invert Elev Up (ft) = 7616.32
Rise (in) = 18.0
Shape = Circular
Span (in) = 18.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

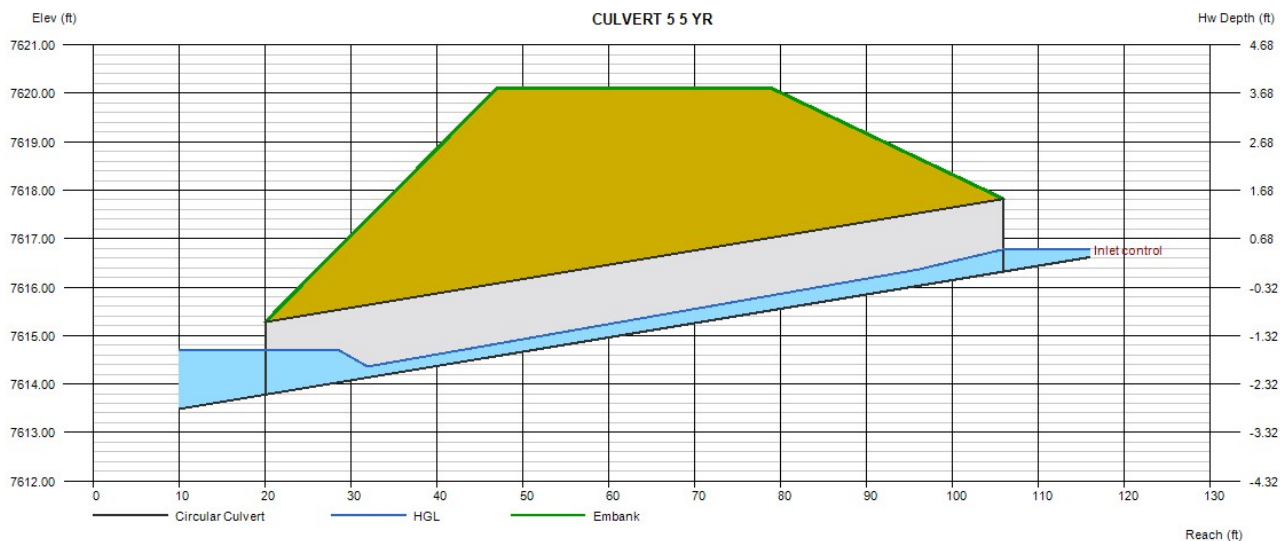
Top Elevation (ft) = 7620.10
Top Width (ft) = 32.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 0.90
Qmax (cfs) = 0.90
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 0.90
Qpipe (cfs) = 0.90
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 0.79
Veloc Up (ft/s) = 2.84
HGL Dn (ft) = 7614.71
HGL Up (ft) = 7616.67
Hw Elev (ft) = 7616.78
Hw/D (ft) = 0.31
Flow Regime = Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Tuesday, Mar 31 2026

CULVERT 5 100 YR

Invert Elev Dn (ft) = 7613.78
Pipe Length (ft) = 85.92
Slope (%) = 2.96
Invert Elev Up (ft) = 7616.32
Rise (in) = 18.0
Shape = Circular
Span (in) = 18.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

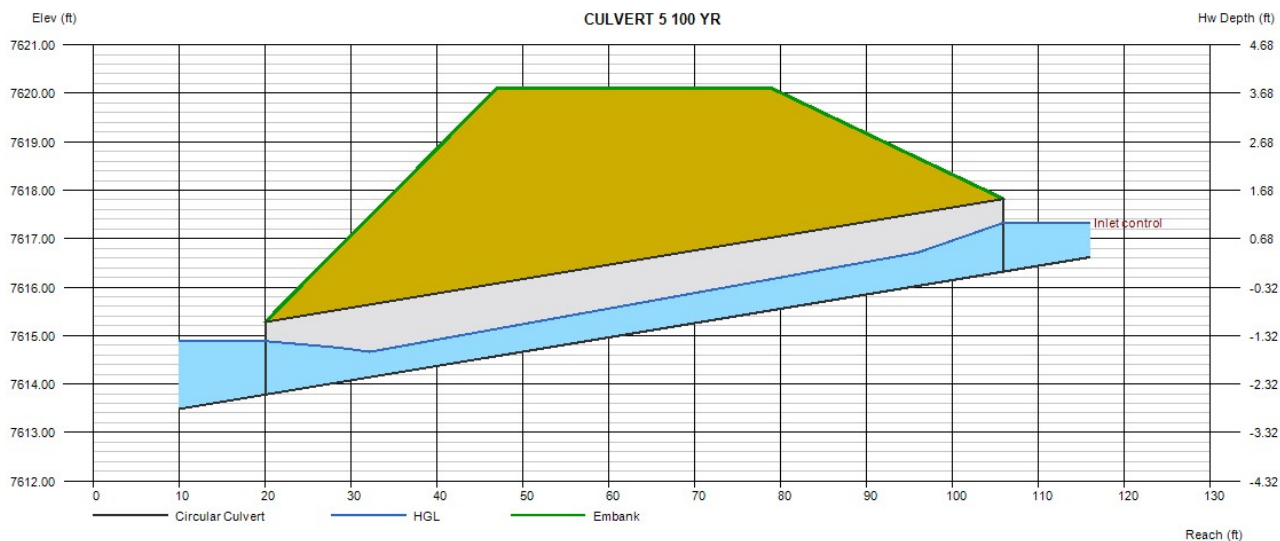
Top Elevation (ft) = 7620.10
Top Width (ft) = 32.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 3.50
Qmax (cfs) = 3.50
Tailwater Elev (ft) = (dc+D)/2

Highlighted

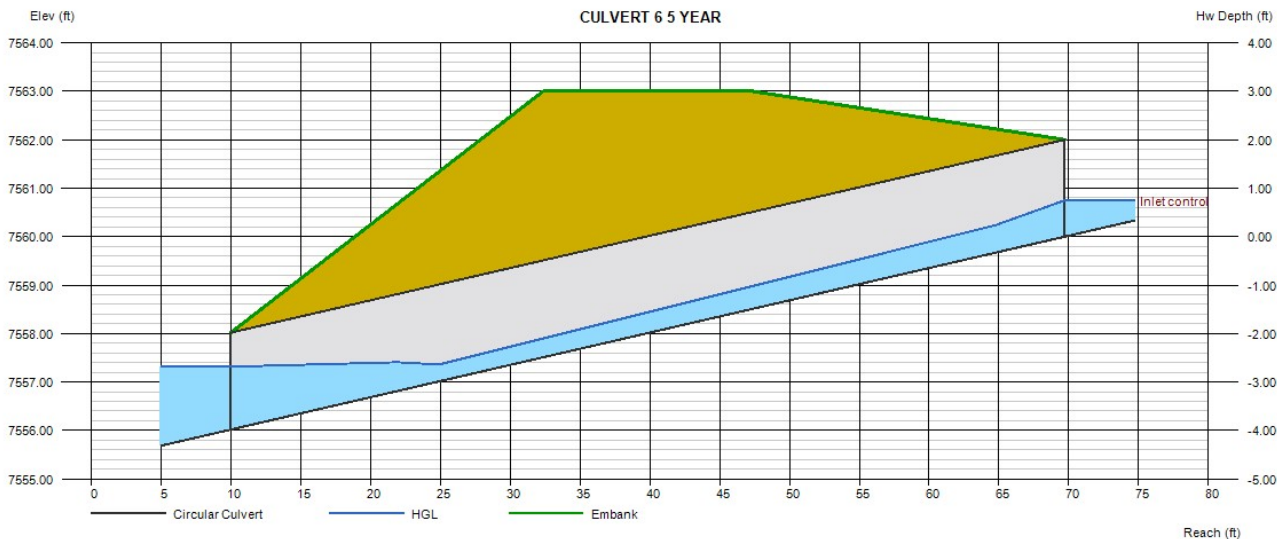
Qtotal (cfs) = 3.50
Qpipe (cfs) = 3.50
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 2.50
Veloc Up (ft/s) = 4.23
HGL Dn (ft) = 7614.89
HGL Up (ft) = 7617.03
Hw Elev (ft) = 7617.33
Hw/D (ft) = 0.67
Flow Regime = Inlet Control



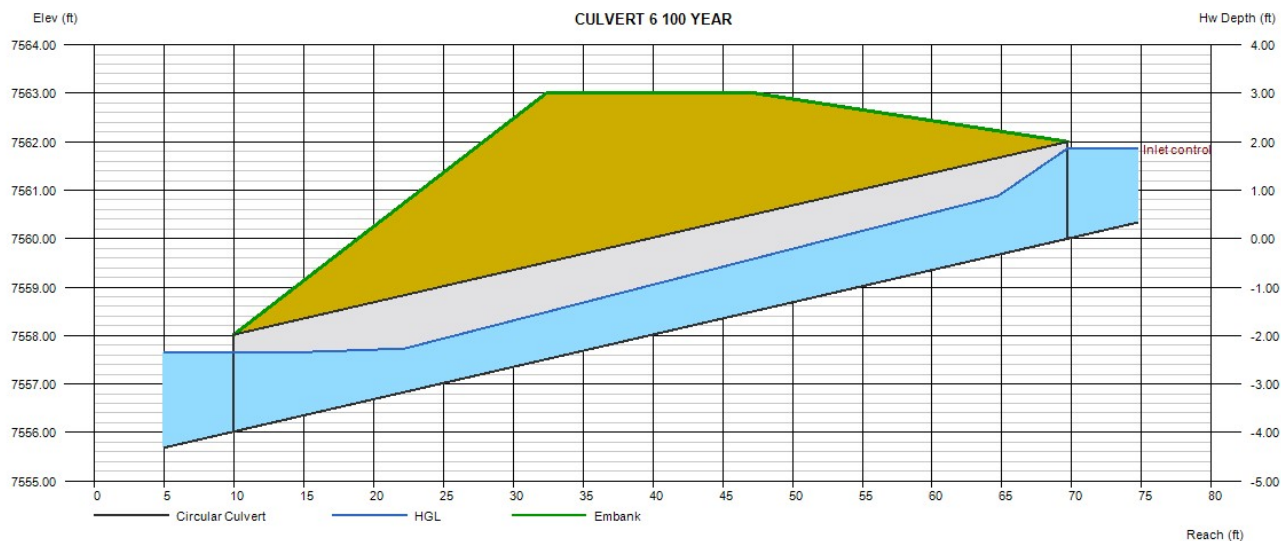
Culvert Report

CULVERT 6 5 YEAR

Invert Elev Dn (ft)	= 7556.02	Calculations	
Pipe Length (ft)	= 59.71	Qmin (cfs)	= 2.90
Slope (%)	= 6.67	Qmax (cfs)	= 2.90
Invert Elev Up (ft)	= 7560.00	Tailwater Elev (ft)	= (dc+D)/2
Rise (in)	= 24.0		
Shape	= Circular	Highlighted	
Span (in)	= 24.0	Qtotat (cfs)	= 2.90
No. Barrels	= 1	Qpipe (cfs)	= 2.90
n-Value	= 0.013	Qovertop (cfs)	= 0.00
Culvert Type	= Circular Concrete	Veloc Dn (ft/s)	= 1.35
Culvert Entrance	= Square edge w/headwall (C)	Veloc Up (ft/s)	= 3.72
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 7557.32
		HGL Up (ft)	= 7560.59
		Hw Elev (ft)	= 7560.75
		Hw/D (ft)	= 0.37
		Flow Regime	= Inlet Control
Embankment			
Top Elevation (ft)	= 7563.00		
Top Width (ft)	= 15.00		
Crest Width (ft)	= 0.00		



Thursday, Dec 11 2025



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Wednesday, Apr 1 2026

CULVERT 7 5 YEAR

Invert Elev Dn (ft) = 7559.18
Pipe Length (ft) = 39.91
Slope (%) = 0.50
Invert Elev Up (ft) = 7559.38
Rise (in) = 60.0
Shape = Circular
Span (in) = 60.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

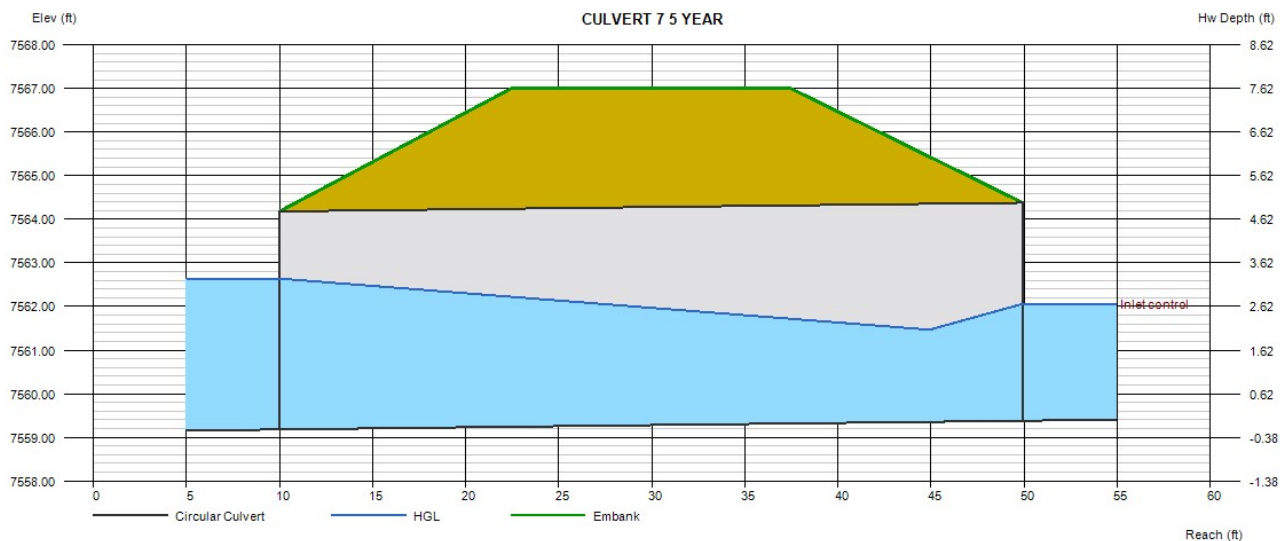
Top Elevation (ft) = 7567.00
Top Width (ft) = 15.00
Crest Width (ft) = 100.00

Calculations

Qmin (cfs) = 47.10
Qmax (cfs) = 47.10
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 47.10
Qpipe (cfs) = 47.10
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 3.25
Veloc Up (ft/s) = 6.79
HGL Dn (ft) = 7562.64
HGL Up (ft) = 7561.30
Hw Elev (ft) = 7562.06
Hw/D (ft) = 0.54
Flow Regime = Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Wednesday, Apr 1 2026

CULVERT 7 100 YEAR

Invert Elev Dn (ft) = 7559.18
Pipe Length (ft) = 39.91
Slope (%) = 0.50
Invert Elev Up (ft) = 7559.38
Rise (in) = 60.0
Shape = Circular
Span (in) = 60.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

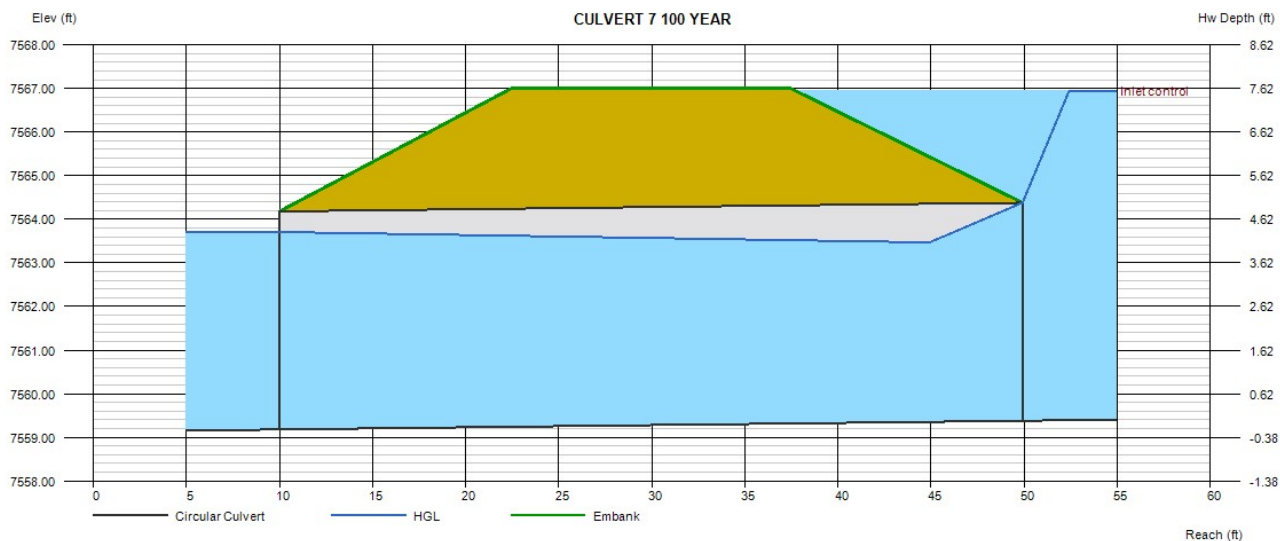
Top Elevation (ft) = 7567.00
Top Width (ft) = 15.00
Crest Width (ft) = 100.00

Calculations

Qmin (cfs) = 202.30
Qmax (cfs) = 202.30
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 202.30
Qpipe (cfs) = 202.30
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 10.82
Veloc Up (ft/s) = 11.86
HGL Dn (ft) = 7563.71
HGL Up (ft) = 7563.44
Hw Elev (ft) = 7566.94
Hw/D (ft) = 1.51
Flow Regime = Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Wednesday, Apr 1 2026

CULVERT 8 5 YEAR

Invert Elev Dn (ft) = 7558.32
Pipe Length (ft) = 36.30
Slope (%) = 8.07
Invert Elev Up (ft) = 7561.25
Rise (in) = 48.0
Shape = Circular
Span (in) = 48.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

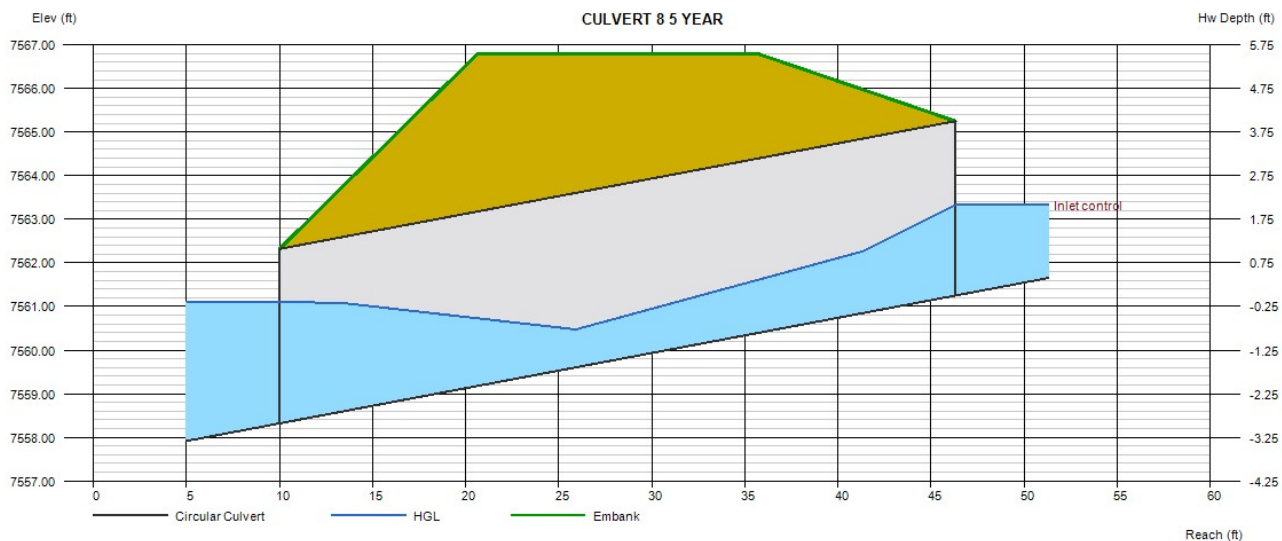
Top Elevation (ft) = 7566.80
Top Width (ft) = 15.00
Crest Width (ft) = 100.00

Calculations

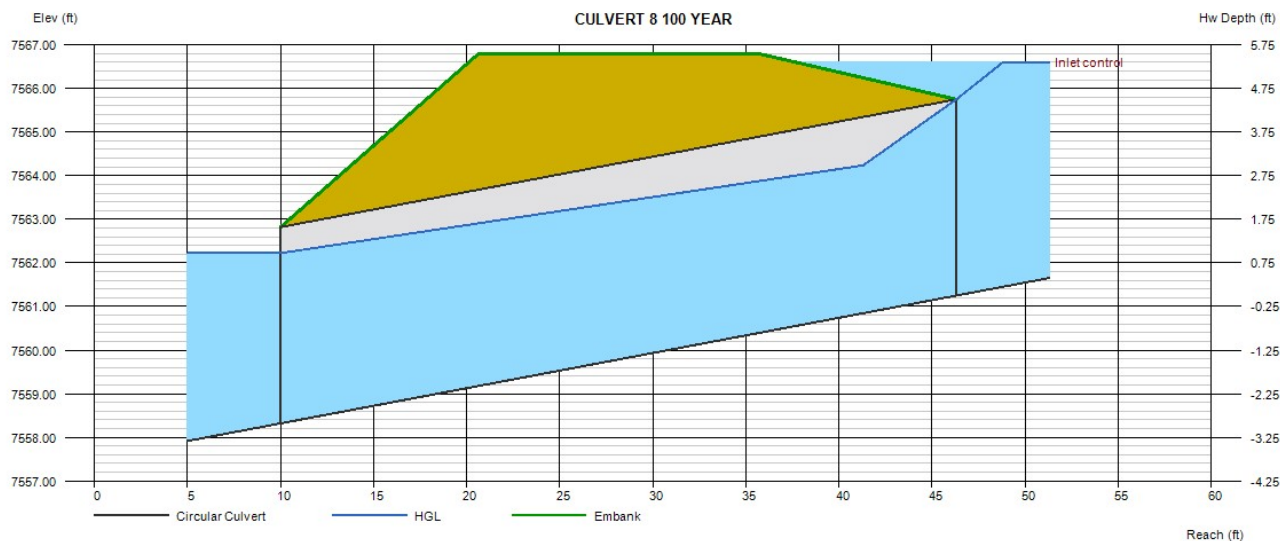
Qmin (cfs) = 28.90
Qmax (cfs) = 28.90
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 28.90
Qpipe (cfs) = 28.90
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 3.08
Veloc Up (ft/s) = 6.20
HGL Dn (ft) = 7561.12
HGL Up (ft) = 7562.84
Hw Elev (ft) = 7563.33
Hw/D (ft) = 0.52
Flow Regime = Inlet Control



Wednesday, Apr 1 2026



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Wednesday, Apr 1 2026

CULVERT 9 5 YEAR

Invert Elev Dn (ft) = 7626.39
Pipe Length (ft) = 86.32
Slope (%) = 3.16
Invert Elev Up (ft) = 7629.12
Rise (in) = 18.0
Shape = Circular
Span (in) = 18.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

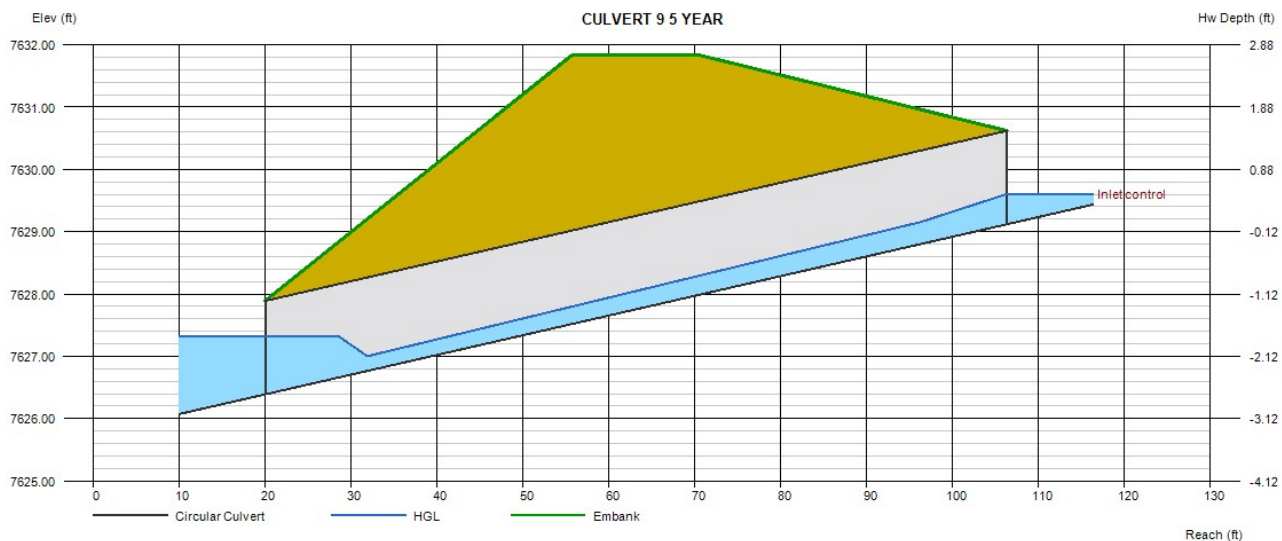
Top Elevation (ft) = 7631.83
Top Width (ft) = 15.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 1.00
Qmax (cfs) = 1.00
Tailwater Elev (ft) = $(dc+D)/2$

Highlighted

Qtotal (cfs) = 1.00
Qpipe (cfs) = 1.00
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 0.86
Veloc Up (ft/s) = 2.92
HGL Dn (ft) = 7627.33
HGL Up (ft) = 7629.49
Hw Elev (ft) = 7629.61
Hw/D (ft) = 0.32
Flow Regime = Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Wednesday, Apr 1 2026

CULVERT 9 100 YEAR

Invert Elev Dn (ft) = 7626.39
Pipe Length (ft) = 86.32
Slope (%) = 3.16
Invert Elev Up (ft) = 7629.12
Rise (in) = 18.0
Shape = Circular
Span (in) = 18.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

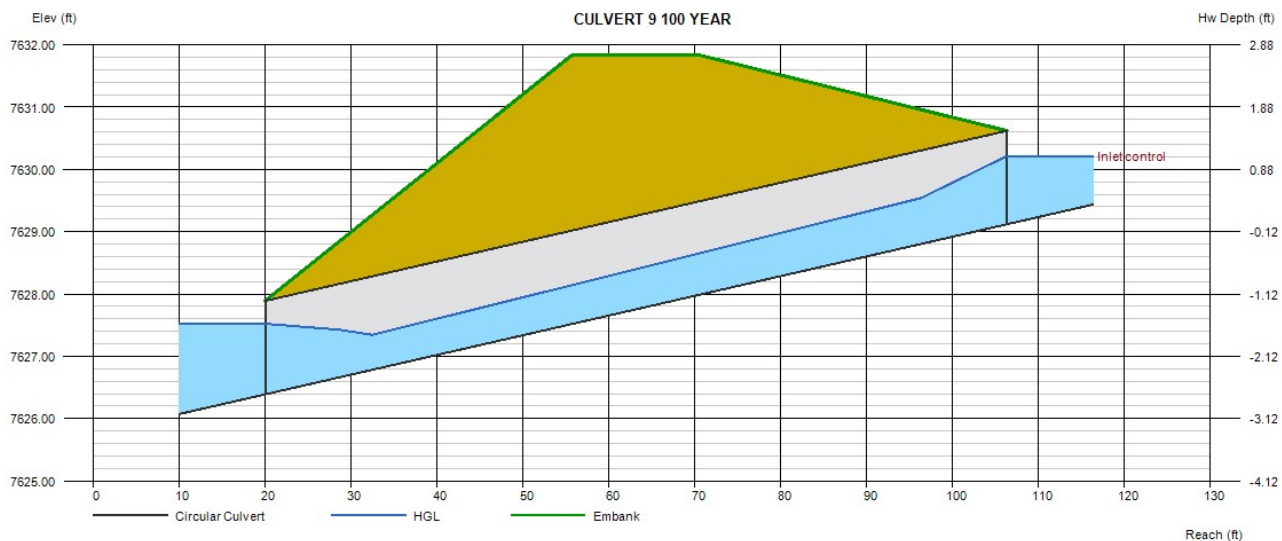
Top Elevation (ft) = 7631.83
Top Width (ft) = 15.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 4.00
Qmax (cfs) = 4.00
Tailwater Elev (ft) = $(dc+D)/2$

Highlighted

Qtotal (cfs) = 4.00
Qpipe (cfs) = 4.00
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 2.79
Veloc Up (ft/s) = 4.42
HGL Dn (ft) = 7627.52
HGL Up (ft) = 7629.89
Hw Elev (ft) = 7630.22
Hw/D (ft) = 0.73
Flow Regime = Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Monday, Oct 6 2025

CULVERT I4 5 YR

Invert Elev Dn (ft) = 7601.20
Pipe Length (ft) = 105.00
Slope (%) = 0.85
Invert Elev Up (ft) = 7602.09
Rise (in) = 18.0
Shape = Circular
Span (in) = 18.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

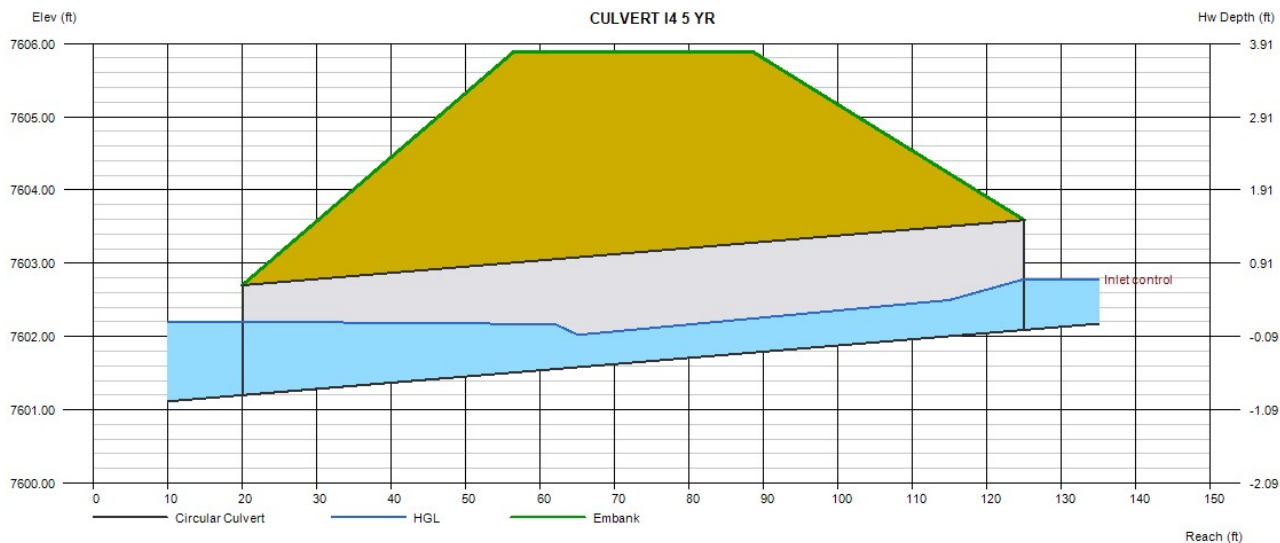
Top Elevation (ft) = 7605.89
Top Width (ft) = 32.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 1.80
Qmax (cfs) = 1.80
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 1.80
Qpipe (cfs) = 1.80
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 1.43
Veloc Up (ft/s) = 3.45
HGL Dn (ft) = 7602.20
HGL Up (ft) = 7602.59
Hw Elev (ft) = 7602.78
Hw/D (ft) = 0.46
Flow Regime = Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Monday, Oct 6 2025

CULVERT I4 100 YR

Invert Elev Dn (ft) = 7601.20
Pipe Length (ft) = 105.00
Slope (%) = 0.85
Invert Elev Up (ft) = 7602.09
Rise (in) = 18.0
Shape = Circular
Span (in) = 18.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

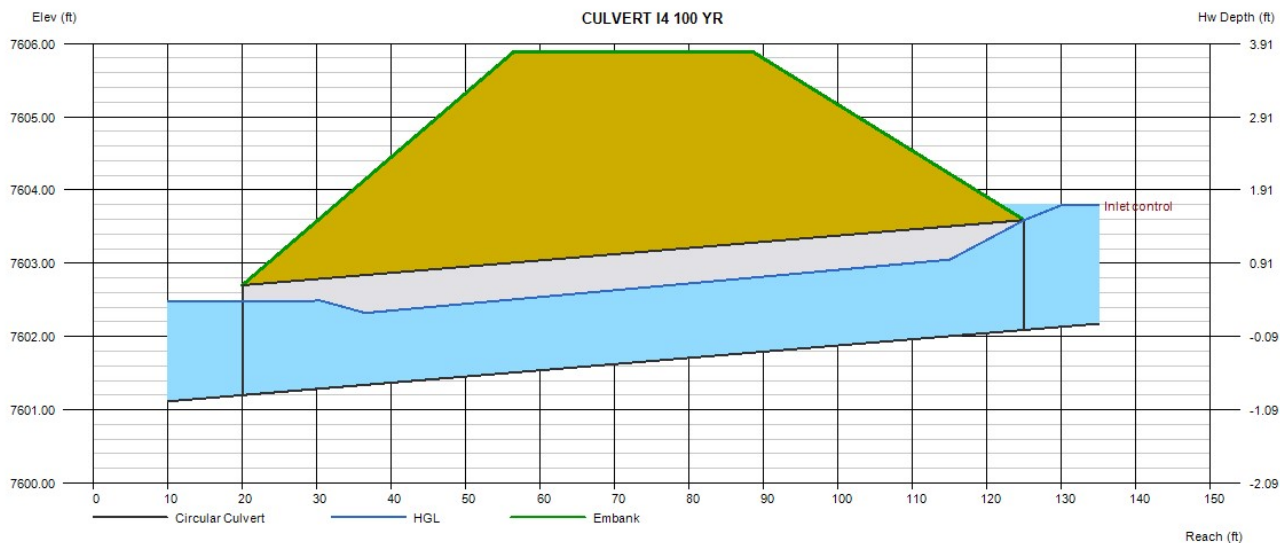
Top Elevation (ft) = 7605.89
Top Width (ft) = 32.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 7.40
Qmax (cfs) = 7.40
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 7.40
Qpipe (cfs) = 7.40
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 4.62
Veloc Up (ft/s) = 5.58
HGL Dn (ft) = 7602.48
HGL Up (ft) = 7603.14
Hw Elev (ft) = 7603.79
Hw/D (ft) = 1.14
Flow Regime = Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Monday, Nov 24 2025

CULVERT I5 5 YR

Invert Elev Dn (ft) = 7592.54
Pipe Length (ft) = 105.69
Slope (%) = 8.05
Invert Elev Up (ft) = 7601.05
Rise (in) = 42.0
Shape = Circular
Span (in) = 42.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

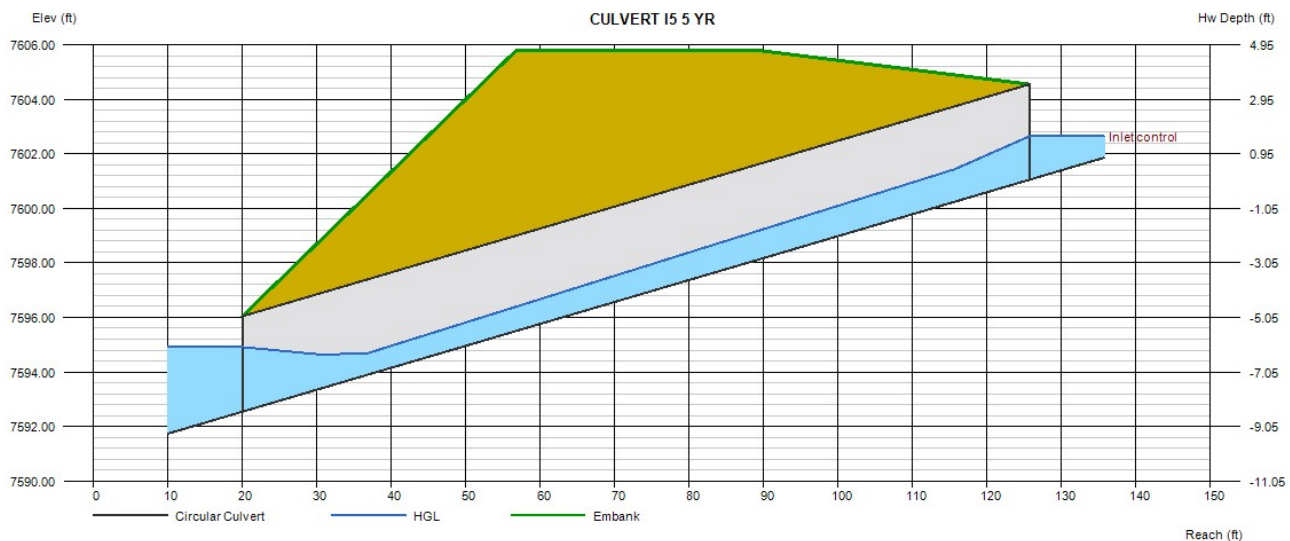
Top Elevation (ft) = 7605.81
Top Width (ft) = 32.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 16.70
Qmax (cfs) = 16.70
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 16.70
Qpipe (cfs) = 16.70
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 2.40
Veloc Up (ft/s) = 5.44
HGL Dn (ft) = 7594.91
HGL Up (ft) = 7602.30
Hw Elev (ft) = 7602.64
Hw/D (ft) = 0.46
Flow Regime = Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Monday, Mar 30 2026

CULVERT I5 100 YR

Invert Elev Dn (ft) = 7592.54
Pipe Length (ft) = 105.69
Slope (%) = 8.05
Invert Elev Up (ft) = 7601.05
Rise (in) = 42.0
Shape = Circular
Span (in) = 42.0
No. Barrels = 1
n-Value = 0.013
Culvert Type = Circular Concrete
Culvert Entrance = Square edge w/headwall (C)
Coeff. K,M,c,Y,k = 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

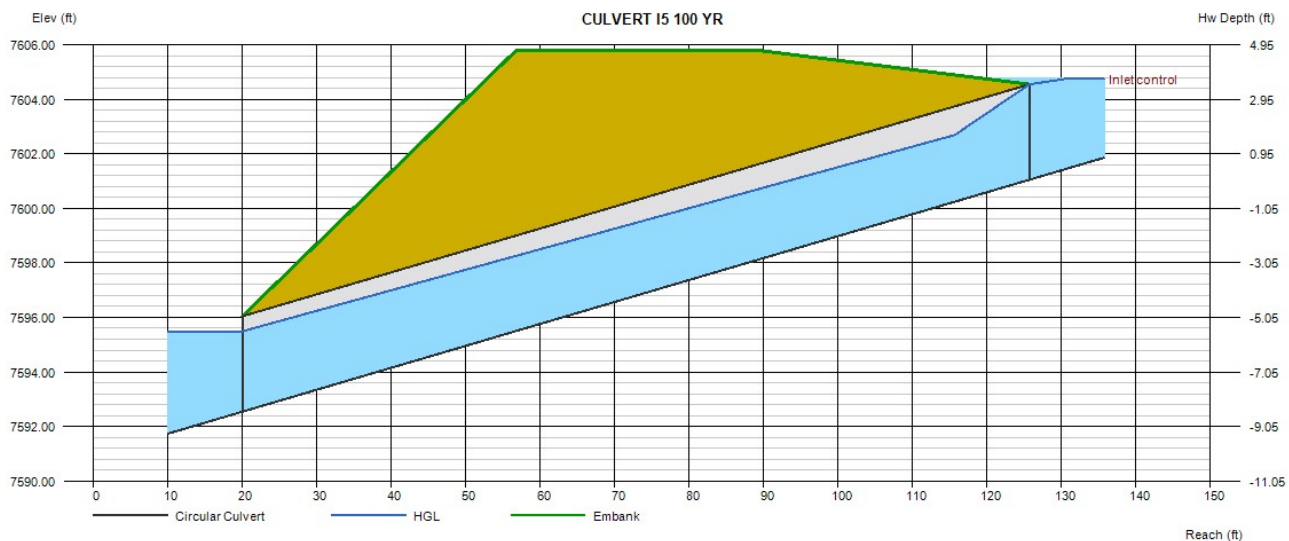
Top Elevation (ft) = 7605.81
Top Width (ft) = 32.00
Crest Width (ft) = 0.00

Calculations

Qmin (cfs) = 58.70
Qmax (cfs) = 58.70
Tailwater Elev (ft) = (dc+D)/2

Highlighted

Qtotal (cfs) = 58.70
Qpipe (cfs) = 58.70
Qovertop (cfs) = 0.00
Veloc Dn (ft/s) = 6.79
Veloc Up (ft/s) = 8.36
HGL Dn (ft) = 7595.49
HGL Up (ft) = 7603.45
Hw Elev (ft) = 7604.76
Hw/D (ft) = 1.06
Flow Regime = Inlet Control





FLYING HORSE NORTH FILING NO. 8

Calc'd by:

CMD

211030

Checked by:

RDL

CULVERT 1 RIPRAP

Date:

12/16/2025

Input Parameters	
Flow (Q)	12.1 cfs
Tailwater depth (Y _t)	0.60 ft
Conduit Diameter (D _c)	18 in
Expansion Factor (per Fig. 9-35)	3
Soil Type	Non-Cohesive Soils

Calculated Parameters	
Froude Parameter (Q/D ^{2.5})	4.39
D ₅₀ =	5.46 in
UDFCD Riprap Type =	Type VL
Design D ₅₀ =	6 in
Minimum Mantle Thickness =	12 in
Minimum Length of Apron =	7.6 ft

Calculated D₅₀ for riprap was calculated using Equation 9-16 in the USDCM Vol 2.

$$d_{50} = \frac{0.023Q}{Y_t^{1.2} D_c^{0.3}}$$

Calculated minimum length of apron was calculated using Equations 9-11 and 9-12 in the USDCM Vol. 2

$$L_p = \left(\frac{1}{2 \tan \theta} \right) \left(\frac{A_t}{Y_t} - W \right)$$

Equation 9-11

$$A_t = \frac{Q}{V}$$

Equation 9-12

Where:

L_p = length of protection (ft)

W = width of the conduit (ft, use diameter for circular conduits)

Y_t = tailwater depth (ft)

θ = the expansion angle of the culvert flow

Where:

Q = design discharge (cfs)

V = the allowable non-eroding velocity in the downstream channel (ft/sec)

A_t = required area of flow at allowable velocity (ft²)

Note:

¹ Calculations follow criteria in the USDCM Vol.2 Chapter 9

² Calculations assume a circular culvert

³ This spreadsheet assumes y_t/D_c=0.4 in cases where y_t is unknown or a hydraulic jump is suspected downstream of the outlet.

⁴ Per the USDCM Vol.2 in no case should L_p be less than 3D, nor does L_p need to be greater than 10D whenever the Froude parameter is less than 6.0. whenever the Froude parameter is greater than 6, increase the maximum L_p required by 1/4 D_c for each whole number by which the Froude parameter is greater than 6



FLYING HORSE NORTH FILING NO. 8

Calc'd by:

CMD

211030

Checked by:

RDL

CULVERT 2 RIPRAP

Date:

4/1/2026

Input Parameters	
Flow (Q)	70.2 cfs
Tailwater depth (Y _t)	1.40 ft
Conduit Diameter (D _c)	42 in
Expansion Factor (per Fig. 9-35)	4.5
Soil Type	Non-Cohesive Soils

Calculated Parameters	
Froude Parameter (Q/D ^{2.5})	3.06
D ₅₀ =	8.89 in
UDFCD Riprap Type =	Type L
Design D ₅₀ =	9 in
Minimum Mantle Thickness =	18 in
Minimum Length of Apron =	29.4 ft

Calculated D₅₀ for riprap was calculated using Equation 9-16 in the USDCM Vol 2.

$$d_{50} = \frac{0.023Q}{Y_t^{1.2} D_c^{0.3}}$$

Calculated minimum length of apron was calculated using Equations 9-11 and 9-12 in the USDCM Vol. 2

$$L_p = \left(\frac{1}{2 \tan \theta} \right) \left(\frac{A_t}{Y_t} - W \right)$$

Equation 9-11

$$A_t = \frac{Q}{V}$$

Equation 9-12

Where:

L_p = length of protection (ft)

W = width of the conduit (ft, use diameter for circular conduits)

Y_t = tailwater depth (ft)

θ = the expansion angle of the culvert flow

Where:

Q = design discharge (cfs)

V = the allowable non-eroding velocity in the downstream channel (ft/sec)

A_t = required area of flow at allowable velocity (ft²)

Note:

¹ Calculations follow criteria in the USDCM Vol.2 Chapter 9

² Calculations assume a circular culvert

³ This spreadsheet assumes y_t/D_c=0.4 in cases where y_t is unknown or a hydraulic jump is suspected downstream of the outlet.

⁴ Per the USDCM Vol.2 in no case should L_p be less than 3D, nor does L_p need to be greater than 10D whenever the Froude parameter is less than 6.0. whenever the Froude parameter is greater than 6, increase the maximum L_p required by 1/4 D_c for each whole number by which the Froude parameter is greater than 6



FLYING HORSE NORTH FILING NO. 8

Calc'd by:

CMD

211030

Checked by:

RDL

CULVERT 3 RIPRAP

Date:

12/16/2025

Input Parameters

Flow (Q)	58.4	cfs
Tailwater depth (Y _t)	1.40	ft
Conduit Diameter (D _c)	42	in
Expansion Factor (per Fig. 9-35)	5	
Soil Type	Non-Cohesive Soils	

Calculated Parameters

Froude Parameter (Q/D ^{2.5})	2.55	
D ₅₀ =	7.39	in
UDFCD Riprap Type =	Type L	
Design D ₅₀ =	9	in
Minimum Mantle Thickness =	18	in
Minimum Length of Apron =	24.2	ft

Calculated D₅₀ for riprap was calculated using Equation 9-16 in the USDCM Vol 2.

$$d_{50} = \frac{0.023Q}{Y_t^{1.2} D_c^{0.3}}$$

Calculated minimum length of apron was calculated using Equations 9-11 and 9-12 in the USDCM Vol. 2

$$L_p = \left(\frac{1}{2 \tan \theta} \right) \left(\frac{A_t}{Y_t} - W \right)$$

Equation 9-11

$$A_t = \frac{Q}{V}$$

Equation 9-12

Where:

L_p = length of protection (ft)

W = width of the conduit (ft, use diameter for circular conduits)

Y_t = tailwater depth (ft)

θ = the expansion angle of the culvert flow

Where:

Q = design discharge (cfs)

V = the allowable non-eroding velocity in the downstream channel (ft/sec)

A_t = required area of flow at allowable velocity (ft²)

Note:

¹ Calculations follow criteria in the USDCM Vol.2 Chapter 9

² Calculations assume a circular culvert

³ This spreadsheet assumes y_t/D_c=0.4 in cases where y_t is unknown or a hydraulic jump is suspected downstream of the outlet.

⁴ Per the USDCM Vol.2 in no case should L_p be less than 3D, nor does L_p need to be greater than 10D whenever the Froude parameter is less than 6.0. whenever the Froude parameter is greater than 6, increase the maximum L_p required by 1/4 D_c for each whole number by which the Froude parameter is greater than 6



FLYING HORSE NORTH FILING NO. 8

Calc'd by:

CMD

211030

Checked by:

RDL

CULVERT 4 RIPRAP

Date:

12/16/2025

Input Parameters	
Flow (Q)	61.6 cfs
Tailwater depth (Y _t)	1.20 ft
Conduit Diameter (D _c)	36 in
Expansion Factor (per Fig. 9-35)	5.75
Soil Type	Non-Cohesive Soils

Calculated Parameters	
Froude Parameter (Q/D ^{2.5})	3.50
D ₅₀ =	9.83 in
UDFCD Riprap Type =	Type M
Design D ₅₀ =	12 in
Minimum Mantle Thickness =	24 in
Minimum Length of Apron =	41.8 ft

Calculated D₅₀ for riprap was calculated using Equation 9-16 in the USDCM Vol 2.

$$d_{50} = \frac{0.023Q}{Y_t^{1.2} D_c^{0.3}}$$

Calculated minimum length of apron was calculated using Equations 9-11 and 9-12 in the USDCM Vol. 2

$$L_p = \left(\frac{1}{2 \tan \theta} \right) \left(\frac{A_t}{Y_t} - W \right)$$

Equation 9-11

$$A_t = \frac{Q}{V}$$

Equation 9-12

Where:

L_p = length of protection (ft)

W = width of the conduit (ft, use diameter for circular conduits)

Y_t = tailwater depth (ft)

θ = the expansion angle of the culvert flow

Where:

Q = design discharge (cfs)

V = the allowable non-eroding velocity in the downstream channel (ft/sec)

A_t = required area of flow at allowable velocity (ft²)

Note:

¹ Calculations follow criteria in the USDCM Vol.2 Chapter 9

² Calculations assume a circular culvert

³ This spreadsheet assumes y_t/D_c=0.4 in cases where y_t is unknown or a hydraulic jump is suspected downstream of the outlet.

⁴ Per the USDCM Vol.2 in no case should L_p be less than 3D, nor does L_p need to be greater than 10D whenever the Froude parameter is less than 6.0. whenever the Froude parameter is greater than 6, increase the maximum L_p required by 1/4 D_c for each whole number by which the Froude parameter is greater than 6



FLYING HORSE NORTH FILING NO. 8

Calc'd by:

CMD

211030

Checked by:

RDL

CULVERT 5 RIPRAP

Date:

12/16/2025

Input Parameters	
Flow (Q)	3.5 cfs
Tailwater depth (Y _t)	0.60 ft
Conduit Diameter (D _c)	18 in
Expansion Factor (per Fig. 9-35)	6.5
Soil Type	Non-Cohesive Soils

Calculated Parameters	
Froude Parameter (Q/D ^{2.5})	1.27
D ₅₀ =	1.58 in
UDFCD Riprap Type =	Type VL
Design D ₅₀ =	6 in
Minimum Mantle Thickness =	12 in
Minimum Length of Apron =	4.5 ft

Calculated D₅₀ for riprap was calculated using Equation 9-16 in the USDCM Vol 2.

$$d_{50} = \frac{0.023Q}{Y_t^{1.2} D_c^{0.3}}$$

Calculated minimum length of apron was calculated using Equations 9-11 and 9-12 in the USDCM Vol. 2

$$L_p = \left(\frac{1}{2 \tan \theta} \right) \left(\frac{A_t}{Y_t} - W \right)$$

Equation 9-11

$$A_t = \frac{Q}{V}$$

Equation 9-12

Where:

L_p = length of protection (ft)

W = width of the conduit (ft, use diameter for circular conduits)

Y_t = tailwater depth (ft)

θ = the expansion angle of the culvert flow

Where:

Q = design discharge (cfs)

V = the allowable non-eroding velocity in the downstream channel (ft/sec)

A_t = required area of flow at allowable velocity (ft²)

Note:

¹ Calculations follow criteria in the USDCM Vol.2 Chapter 9

² Calculations assume a circular culvert

³ This spreadsheet assumes y_t/D_c=0.4 in cases where y_t is unknown or a hydraulic jump is suspected downstream of the outlet.

⁴ Per the USDCM Vol.2 in no case should L_p be less than 3D, nor does L_p need to be greater than 10D whenever the Froude parameter is less than 6.0. whenever the Froude parameter is greater than 6, increase the maximum L_p required by 1/4 D_c for each whole number by which the Froude parameter is greater than 6



FLYING HORSE NORTH FILING NO. 8

Calc'd by:

CMD

211030

Checked by:

RDL

(POND J OUTLET) RIPRAP

Date:

4/1/2026

Input Parameters	
Flow (Q)	197.9 cfs
Tailwater depth (Y _t)	2.00 ft
Conduit Diameter (D _c)	60 in
Expansion Factor (per Fig. 9-35)	5.75
Soil Type	Non-Cohesive Soils

Calculated Parameters	
Froude Parameter (Q/D ^{2.5})	4.00
D ₅₀ =	14.67 in
UDFCD Riprap Type =	Type H
Design D ₅₀ =	18 in
Minimum Mantle Thickness =	36 in
Minimum Length of Apron =	85.0 ft

Calculated D₅₀ for riprap was calculated using Equation 9-16 in the USDCM Vol 2.

$$d_{50} = \frac{0.023Q}{Y_t^{1.2} D_c^{0.3}}$$

Calculated minimum length of apron was calculated using Equations 9-11 and 9-12 in the USDCM Vol. 2

$$L_p = \left(\frac{1}{2 \tan \theta} \right) \left(\frac{A_t}{Y_t} - W \right)$$

Equation 9-11

$$A_t = \frac{Q}{V}$$

Equation 9-12

Where:

L_p = length of protection (ft)

W = width of the conduit (ft, use diameter for circular conduits)

Y_t = tailwater depth (ft)

θ = the expansion angle of the culvert flow

Where:

Q = design discharge (cfs)

V = the allowable non-eroding velocity in the downstream channel (ft/sec)

A_t = required area of flow at allowable velocity (ft²)

Note:

¹ Calculations follow criteria in the USDCM Vol.2 Chapter 9

² Calculations assume a circular culvert

³ This spreadsheet assumes y_t/D_c=0.4 in cases where y_t is unknown or a hydraulic jump is suspected downstream of the outlet.

⁴ Per the USDCM Vol.2 in no case should L_p be less than 3D, nor does L_p need to be greater than 10D whenever the Froude parameter is less than 6.0. whenever the Froude parameter is greater than 6, increase the maximum L_p required by 1/4 D_c for each whole number by which the Froude parameter is greater than 6



FLYING HORSE NORTH FILING NO. 8

Calc'd by:

CMD

211030

Checked by:

RDL

POND J SPILLWAY RIPRAP

Date:

12/16/2025

Input Parameters	
Q (cfs)	197.90
Spillway Width (ft)	100.00
q (cfs/ft)	1.98
S (ft/ft)	0.33
C _f	2
η	0

Calculated Parameters	
D ₅₀ =	8.30 in
UDFCD Riprap Type =	Type L
Design D ₅₀ =	9 in
Minimum Mantle Thickness =	18 in

Calculated D₅₀ for riprap was calculated using Equation 13-9 in the CCS DCM Vol 1.

$$D_{50} = 5.23 S^{0.43} (1.35 C_f q)^{0.56}$$

Equation 13-9

Where:

- D₅₀ = median rock size (in)
- S = longitudinal slope (ft/ft)
- C_f = concentration factor (1.0 to 3.0)
- q = unit discharge (cfs/ft)

When:

η (porosity) = 0.0 (i.e., for buried soil riprap)

Note:

¹ Calculations follow criteria in the CCS DCM Vol. 1 Chapter 13

² Calculations assume buried soil riprap



FLYING HORSE NORTH FILING NO. 8

Calc'd by:

CMD

211030

Checked by:

RDL

CULVERT I4 RIPRAP

Date:

12/16/2025

Input Parameters	
Flow (Q)	7.4 cfs
Tailwater depth (Y _t)	0.60 ft
Conduit Diameter (D _c)	18 in
Expansion Factor (per Fig. 9-35)	4.9
Soil Type	Non-Cohesive Soils

Calculated Parameters	
Froude Parameter (Q/D ^{2.5})	2.69
D ₅₀ =	3.34 in
UDFCD Riprap Type =	Type VL
Design D ₅₀ =	6 in
Minimum Mantle Thickness =	12 in
Minimum Length of Apron =	4.7 ft

Calculated D₅₀ for riprap was calculated using Equation 9-16 in the USDCM Vol 2.

$$d_{50} = \frac{0.023Q}{Y_t^{1.2} D_c^{0.3}}$$

Calculated minimum length of apron was calculated using Equations 9-11 and 9-12 in the USDCM Vol. 2

$$L_p = \left(\frac{1}{2 \tan \theta} \right) \left(\frac{A_t}{Y_t} - W \right)$$

Equation 9-11

$$A_t = \frac{Q}{V}$$

Equation 9-12

Where:

L_p = length of protection (ft)

W = width of the conduit (ft, use diameter for circular conduits)

Y_t = tailwater depth (ft)

θ = the expansion angle of the culvert flow

Where:

Q = design discharge (cfs)

V = the allowable non-eroding velocity in the downstream channel (ft/sec)

A_t = required area of flow at allowable velocity (ft²)

Note:

¹ Calculations follow criteria in the USDCM Vol.2 Chapter 9

² Calculations assume a circular culvert

³ This spreadsheet assumes y_t/D_c=0.4 in cases where y_t is unknown or a hydraulic jump is suspected downstream of the outlet.

⁴ Per the USDCM Vol.2 in no case should L_p be less than 3D, nor does L_p need to be greater than 10D whenever the Froude parameter is less than 6.0. whenever the Froude parameter is greater than 6, increase the maximum L_p required by 1/4 D_c for each whole number by which the Froude parameter is greater than 6



FLYING HORSE NORTH FILING NO. 8

Calc'd by:

CMD

211030

Checked by:

RDL

CULVERT I5 RIPRAP

Date:

12/16/2025

Input Parameters	
Flow (Q)	58.7 cfs
Tailwater depth (Y _t)	1.40 ft
Conduit Diameter (D _c)	42 in
Expansion Factor (per Fig. 9-35)	5
Soil Type	Non-Cohesive Soils

Calculated Parameters	
Froude Parameter (Q/D ^{2.5})	2.56
D ₅₀ =	7.43 in
UDFCD Riprap Type =	Type L
Design D ₅₀ =	9 in
Minimum Mantle Thickness =	18 in
Minimum Length of Apron =	24.4 ft

Calculated D₅₀ for riprap was calculated using Equation 9-16 in the USDCM Vol 2.

$$d_{50} = \frac{0.023Q}{Y_t^{1.2} D_c^{0.3}}$$

Calculated minimum length of apron was calculated using Equations 9-11 and 9-12 in the USDCM Vol. 2

$$L_p = \left(\frac{1}{2 \tan \theta} \right) \left(\frac{A_t}{Y_t} - W \right)$$

Equation 9-11

$$A_t = \frac{Q}{V}$$

Equation 9-12

Where:

L_p = length of protection (ft)

W = width of the conduit (ft, use diameter for circular conduits)

Y_t = tailwater depth (ft)

θ = the expansion angle of the culvert flow

Where:

Q = design discharge (cfs)

V = the allowable non-eroding velocity in the downstream channel (ft/sec)

A_t = required area of flow at allowable velocity (ft²)

Note:

¹ Calculations follow criteria in the USDCM Vol.2 Chapter 9

² Calculations assume a circular culvert

³ This spreadsheet assumes y_t/D_c=0.4 in cases where y_t is unknown or a hydraulic jump is suspected downstream of the outlet.

⁴ Per the USDCM Vol.2 in no case should L_p be less than 3D, nor does L_p need to be greater than 10D whenever the Froude parameter is less than 6.0. whenever the Froude parameter is greater than 6, increase the maximum L_p required by 1/4 D_c for each whole number by which the Froude parameter is greater than 6

FROUDE NUMBER CALCULATIONS		CALCULATED BY:	EWB	DATE:	4/1/2026
PROJECT: 211030 FILING NO. 8		CHECKED BY:	RDL		

Froude Number Calculations: 100-YR						
Section	Velocity ft/s	Gravitational Constant ft/s ²	Hydraulic depth ft	Xsectional Area ft ²	top Width ft	Froude #
Channel Section HH	5.94	32.17	0.42	3.16	7.51	1.61
Channel Section II	5.22	32.17	0.54	13.44	25.12	1.26
Channel Section KK	6.18	32.17	1.44	32.74	22.66	0.91
Channel Section I1	4.97	32.17	0.37	11.81	31.74	1.44
Channel Section J1 (Left)	3.14	32.17	0.53	3.86	7.35	0.76
Channel Section J1 (Right)	3.14	32.17	0.53	3.86	7.35	0.76
Channel Section J5	5.39	32.17	0.52	23.46	45.28	1.32
Channel Section S1	4.77	32.17	0.26	4.27	16.45	1.65

Shear Stress Calculations: 100-YR					
Section	unit weight of water lb/ft ³	Depth of flow ft	Slope ft/ft	Shear Stress lb/ft ²	
Channel Section HH	63.43	0.55	0.049	1.71	
Channel Section II	63.43	0.68	0.027	1.16	
Channel Section KK	63.43	2.89	0.010	1.83	
Channel Section I1	63.43	0.40	0.040	1.01	
Channel Section J1 (Left)	63.43	1.05	0.010	0.67	
Channel Section J1 (Right)	63.43	1.05	0.010	0.67	
Channel Section J5	63.43	0.74	0.030	1.41	
Channel Section S1	63.43	0.30	0.080	1.52	

Channel Lining Determination					
Calculated Values			P300 Max Values		
Section	Shear Stress	Velocity	Shear Stress	Velocity	Lining Required
Channel Section HH	1.71	5.94	3	9	P300
Channel Section II	1.16	5.22	3	9	P300
Channel Section KK	1.83	6.18	3	9	P300
Channel Section I1	1.01	4.97	3	9	NONE
Channel Section J1 (Left)	0.67	3.14	3	9	NONE
Channel Section J1 (Right)	0.67	3.14	3	9	NONE
Channel Section J5	1.41	5.39	3	9	P300
Channel Section S1	1.52	4.77	3	9	NONE

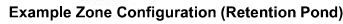
Note: Permissible Shear Stress of 1.5lb/ft² and Permissible Velocity of 5ft/s assumed for existing channels (grassland)
Note: Permissible Shear Stress of 4lb/ft² and Permissible Velocity of 7ft/s assumed for Channel Section S1 (forested area - southwest)



APPENDIX D

WATER QUALITY AND DETENTION CALCULATIONS

MHFD-Detention, Version 4.07 (June 2025)

Basin ID: Pond J

7555.00

Water Quality Capture Volume (WQCV) =	0.896	acre-feet
Excess Urban Runoff Volume (EURV) =	1.487	acre-feet
2-yr Runoff Volume (P1 = 0.92 in.) =	0.730	acre-feet
5-yr Runoff Volume (P1 = 1.19 in.) =	1.827	acre-feet
10-yr Runoff Volume (P1 = 1.44 in.) =	3.977	acre-feet
25-yr Runoff Volume (P1 = 1.82 in.) =	10.192	acre-feet
50-yr Runoff Volume (P1 = 2.14 in.) =	14.332	acre-feet
100-yr Runoff Volume (P1 = 2.48 in.) =	20.051	acre-feet
500-yr Runoff Volume (P1 = 3.37 in.) =	32.614	acre-feet
Approximate 2-yr Detention Volume =	0.730	acre-feet
Approximate 5-yr Detention Volume =	1.180	acre-feet
Approximate 10-yr Detention Volume =	2.612	acre-feet
Approximate 25-yr Detention Volume =	4.075	acre-feet
Approximate 50-yr Detention Volume =	4.447	acre-feet
Approximate 100-yr Detention Volume =	5.970	acre-feet

Zone 1 Volume (WQCV) =	0.896	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.591	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	4.483	acre-feet
Total Detention Basin Volume =	5.970	acre-feet

	acre-feet
	acre-feet
0.92	inches
1.19	inches
1.44	inches
1.82	inches
2.14	inches
2.48	inches
3.37	inches

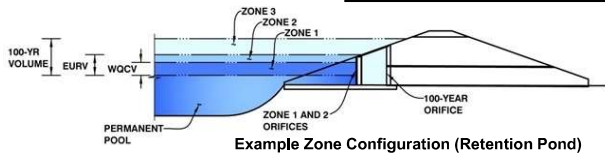
[illegible]

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.07 (June 2025)

Project: **Flying Horse North PUD Amendment**

Basin ID: **Pond J**



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.11	0.896	Orifice Plate
Zone 2 (EURV)	3.66	0.591	Circular Orifice
Zone 3 (100-year)	5.86	4.483	Weir&Pipe (Restrict)
Total (all zones)		5.970	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration SCM)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain
Underdrain Orifice Area = ft²
Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation SCM)

Centroid of Lowest Orifice = ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate = ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing = inches
Orifice Plate: Orifice Area per Row = sq. inches

Calculated Parameters for Plate
WQ Orifice Area per Row = ft²
Elliptical Half-Width = feet
Elliptical Slot Centroid = feet
Elliptical Slot Area = ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.00	2.00					
Orifice Area (sq. inches)	3.14	1.48	1.48					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

Invert of Vertical Orifice = ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice = ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter = inches

Calculated Parameters for Vertical Orifice
Zone 2 Circular ft²
Vertical Orifice Area = ft²
Vertical Orifice Centroid = feet

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

Overflow Weir Front Edge Height, Ho = ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length = feet
Overflow Weir Grate Slope = H:V
Horiz. Length of Weir Sides = feet
Overflow Grate Type =
Debris Clogging % = %

Calculated Parameters for Overflow Weir
Zone 3 Weir feet
Height of Grate Upper Edge, H_u = feet
Overflow Weir Slope Length = feet
Grate Open Area / 100-yr Orifice Area =
Overflow Grate Open Area w/o Debris = ft²
Overflow Grate Open Area w/ Debris = ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

Depth to Invert of Outlet Pipe = ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter = inches
Restrictor Plate Height Above Pipe Invert = inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate
Outlet Orifice Area = ft²
Outlet Orifice Centroid = feet
Half-Central Angle of Restrictor Plate on Pipe = radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage = ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length = feet
Spillway End Slopes = H:V
Freeboard above Max Water Surface = feet

Calculated Parameters for Spillway
Spillway Design Flow Depth = feet
Stage at Top of Freeboard = feet
Basin Area at Top of Freeboard = acres
Basin Volume at Top of Freeboard = acre-ft

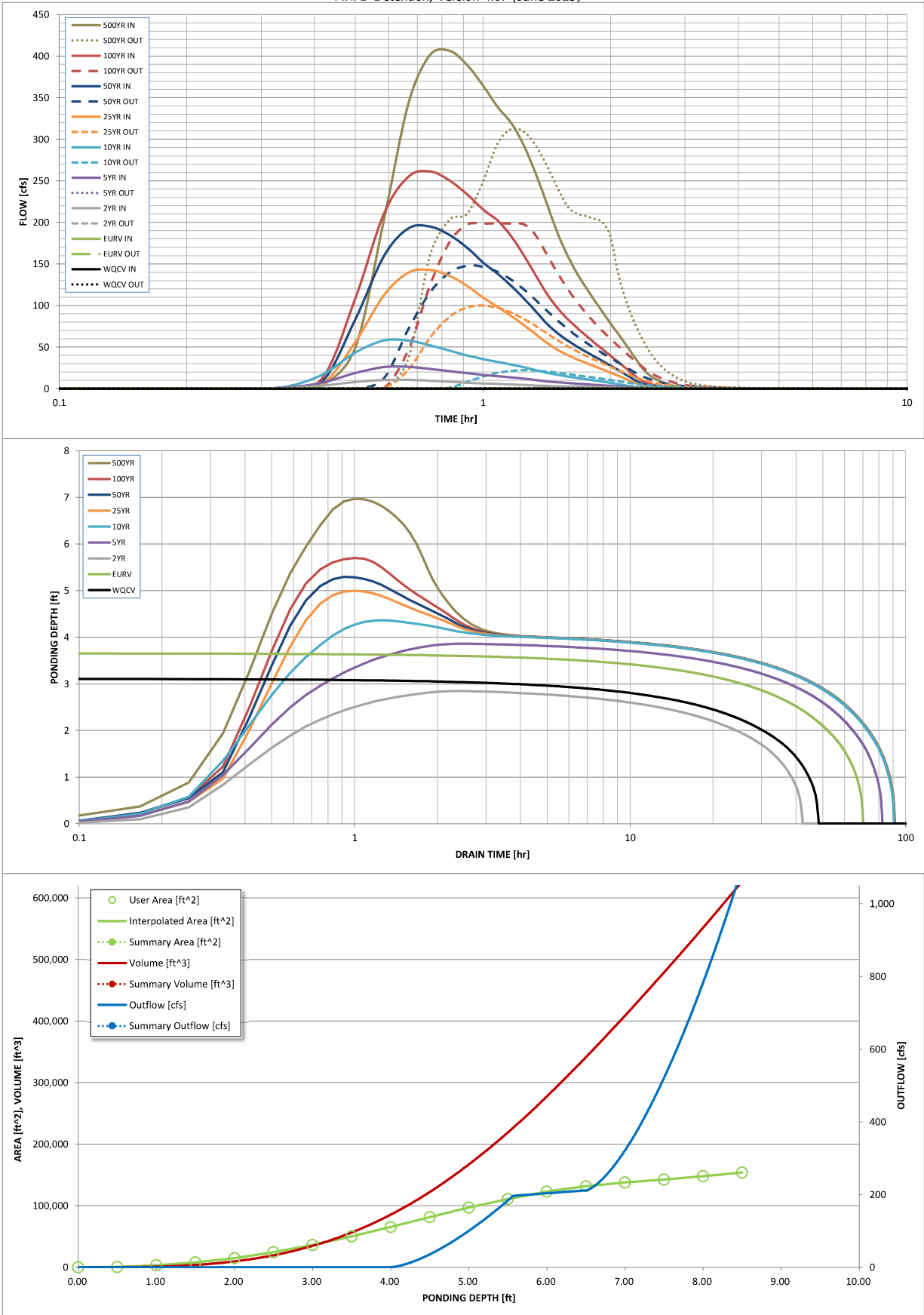
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =	N/A	N/A	0.92	1.19	1.44	1.82	2.14	2.48	3.37
One-Hour Rainfall Depth (in) =	N/A	N/A	0.92	1.19	1.44	1.82	2.14	2.48	3.37
CUHP Runoff Volume (acre-ft) =	0.896	1.487	0.730	1.827	3.977	10.192	14.332	20.051	32.614
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.730	1.827	3.977	10.192	14.332	20.051	32.614
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	5.6	19.3	50.8	136.2	188.5	254.7	399.8
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.03	0.11	0.30	0.80	1.11	1.50	2.36
Peak Inflow Q (cfs) =	N/A	N/A	10.5	26.2	57.8	142.8	194.9	261.4	406.6
Peak Outflow Q (cfs) =	0.3	0.3	0.3	0.4	22.2	100.0	147.8	198.9	312.2
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.0	0.4	0.7	0.8	0.8	0.8
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	0.2	1.0	1.4	1.9	2.1
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	45	65	39	76	80	69	63	55	40
Time to Drain 99% of Inflow Volume (hours) =	47	68	41	80	86	81	78	75	68
Maximum Ponding Depth (ft) =	3.11	3.66	2.84	3.86	4.36	5.00	5.30	5.70	6.97
Area at Maximum Ponding Depth (acres) =	0.91	1.27	0.75	1.40	1.77	2.22	2.41	2.66	3.16
Maximum Volume Stored (acre-ft) =	0.897	1.493	0.674	1.746	2.554	3.812	4.507	5.549	9.288

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.07 (June 2025)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

HR GREEN FOREBAY SIZING

PROJECT: FLYING HORSE NORTH FILING 8

DATE: 4/1/2026

DESIGNED BY: RDL

CHECKED BY: RDL

POND OR DP: POND 1 FOREBAY A

PIPE DIA. (FT) 2

INNER DIMENSIONS

LENGTH		OUTER DIMENSIONS	
L1	2 FT	2.83 FT	
L2	8.00 FT	8.83 FT	
L3	2 FT	2.83 FT	
INNER L	12.00 FT	13.66 FT	OUTER TOTAL L

WIDTH

W1	2 FT	2.83 FT	
W2	4.00 FT (2*D)	4.83 FT	
W3	2 FT	2.83 FT	
INNER W	8.00 FT	9.67 FT	OUTER TOTAL W

BAFFLE AREA (W2 x 30.83' + 4' x 0.83')
6.664 SF

TRIANGLES
RECTANGLE
BAFFLE
TOTAL SURFACE AREA
33.3 SQ FT

FOREBAY HT. 1.25 FT

FOREBAY VOLUME
42 CF
1.5 CY
0.001 AC-FT

SUFFICIENT VOLUME?	YES
--------------------	-----

Sub-Watershed Imperviousness	0.075	AC
Sub-Watershed Area	17.2	AC-FT
Sub-Watershed WQCV	0.075	AC-FT
REQ'D VOL. (1% SUB-WATERSHED WQCV)	0.001	AC-FT
(per UD-BMP calc)	33	CF
Notch Width per UD-BMP	0.9	in

*t = 5 mins

TABLE 4-12. FOREBAY SIZING CRITERIA

FOREBAY SIZING CRITERIA	WATERSHED IMPERVIOUS AREA (IA)				
	IA UP TO 2 ACRES	IA 2 UP TO 5 ACRES	IA 5 UP TO 10 ACRES	IA 10 UP TO 20 ACRES	IA GREATER THAN 20 ACRES
Forebay/Release Rate and Configuration	Concrete sediment grasses surrounding concrete pad with slotted metal edge.				
Minimum Forebay Volume ¹	Size to drain in 4 to 5 minutes using Equation 4-1				
Forebay Depth ¹ or similar design	% of WQCV				
	12 to 15 inches	15 to 18 inches	18 to 24 inches	24 to 30 inches	

¹Approximate volume and depth should consider maintenance and access needs. The values provided are approximate and provide a starting point for design.

Equation 4-1

$$w = 9.23 (A_{rg} / t) (1 / \sqrt{h_{max}})$$

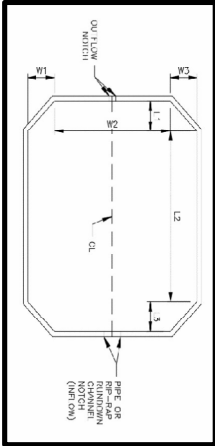
Where:

w = width of the rectangular vertical notch (inches)

A_{rg} = surface area of the forebay (square feet)

t = emptying time of the brim-full forebay (seconds)

h_{max} = maximum depth of the forebay (feet)



HR GREEN FOREBAY SIZING

PROJECT: FLYING HORSE NORTH FILING 8

DATE: 4/1/2026

DESIGNED BY: RDL

CHECKED BY: RDL

POND OR DP: POND J FOREBAY B

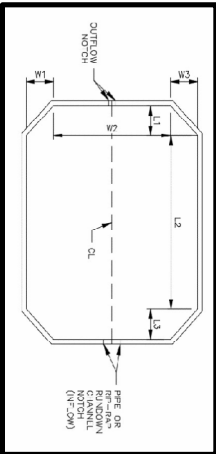
INNER DIMENSIONS		OUTER DIMENSIONS	
LENGTH			
L1	5 FT	5.83 FT	
L2	12.670 FT	13.50 FT	
L3	5 FT	5.83 FT	
INNER L	22.67 FT	24.336 FT	
OUTER TOTAL L			

WIDTH			
W1	5 FT	5.83 FT	
W2	9.50 FT (75% of L2)	10.34 FT	
W3	5 FT	5.83 FT	
INNER W	19.50 FT	21.17 FT	
OUTER TOTAL W			

BAFFLE AREA (W2 x 30.83' + 4' x 0.83')
11.24758 SF

TRIANGLES
RECTANGLE
BAFFLE
TOTAL SURFACE AREA
120.4
11.2475825
159.1 SQ FT

FOREBAY HT.
2.5 FT



FOREBAY VOLUME
398 CF
14.7 CY
0.009 AC-FT

SUFFICIENT VOLUME?	YES
--------------------	-----

Sub-Watershed Imperviousness	0.098
Sub-Watershed Area	90.14 AC
Sub-Watershed WQCV	0.495 AC-FT
REQ'D VOL (1% SUB-WATERSHED WQCV)	0.005 AC-FT
(per UD-BMP calc)	216 CF
Notch Width per UD-BMP	3.1 in

*t = 5 mins

TABLE 4-12. FOREBAY SIZING CRITERIA					
FOREBAY SIZING CRITERIA	WATERSHED IMPERVIOUS AREA (IA)				
	1A UP TO 2 ACRES	1A 2 UP TO 5 ACRES	1A 5 UP TO 10 ACRES	1A 10 UP TO 20 ACRES	1A GREATER THAN 20 ACRES
Forebay Release Rate and Configuration	Concrete sediment pad with dense grasses surrounding.				
Minimum Forebay Volume ¹	grasses surrounding, concrete pad with slotted metal edge.				
Forebay Depth ¹	or similar design				
	12 to 15 inches	15 to 18 inches	18 to 24 inches	24 to 30 inches	

¹Appropriate volume and depth should consider maintenance and access needs. The values provided are approximate and provide a starting point for design.

$$w = 9.23 (A_{fb} / t) (1 / \sqrt{h_{max}})$$

Equation 4-1

Where:

w = width of the rectangular vertical notch (inches)

A_{fb} = surface area of the forebay (square feet)

t = emptying time of the brim-full forebay (seconds)

h_{max} = maximum depth of the forebay (feet)

HR GREEN FOREBAY SIZING

PROJECT: FLYING HORSE NORTH FILING 8

DATE: 4/1/2026

DESIGNED BY: RDL

CHECKED BY: RDL

POND OR DP: POND J FOREBAY C

INNER DIMENSIONS		OUTER DIMENSIONS	
LENGTH			
L1	5 FT	5.83 FT	
L2	13,000 FT	13.83 FT	
L3	5 FT	5.83 FT	
INNER L	23.00 FT		
		OUTER TOTAL	24.666 FT

TABLE 4-12. FOREBAY SIZING CRITERIA					
FOREBAY SIZING CRITERIA	WATERSHED IMPERVIOUS AREA (IA)				
	IA UP TO 2 ACRES	IA 2 UP TO 5 ACRES	IA 5 UP TO 10 ACRES	IA 10 UP TO 20 ACRES	IA GREATER THAN 20 ACRES
Forebay Release Rate and Configuration	Concrete sediment pad with dense grasses surrounding.	Size to drain in 4 to 5 minutes using Equation 4-1			
Minimum Forebay Volume ¹	concrete pad with slotted metal edge, or similar design				
Forebay Depth ¹		12 to 15 inches	15 to 18 inches	18 to 24 inches	24 to 30 inches

¹ Appropriate volume and depth should consider maintenance and access needs. The values provided are approximate and provide a starting point for design.

$$w = 9.23 (A_{rg} / t) (1 / \sqrt{h_{max}})$$
Equation 4-1

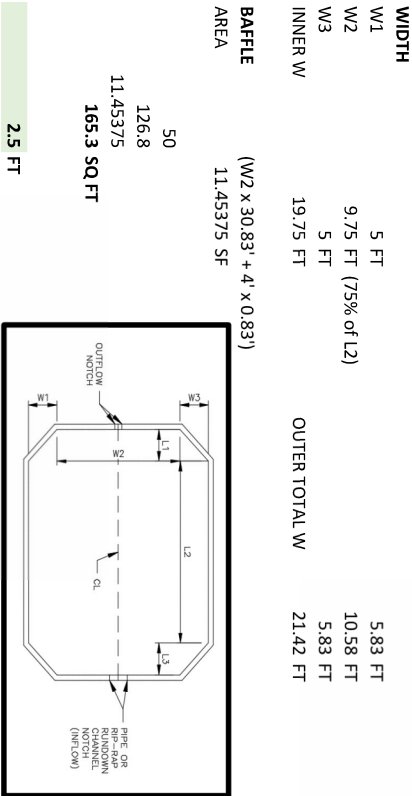
Where:

w = width of the rectangular vertical notch (inches)

A_{rg} = surface area of the forebay (square feet)

t = emptying time of the brim-full forebay (seconds)

h_{max} = maximum depth of the forebay (feet)



TRIANGLES
RECTANGLE
BAFFLE
TOTAL SURFACE AREA

50
126.8
11.45375
165.3 SQ FT

FOREBAY HT. 2.5 FT

FOREBAY VOLUME	413	CF	SUFFICIENT VOLUME?	YES
	15.3	CY		
	0.009	AC-FT		

REQ'D VOL (1% WQCV) (per UD-BMP calc)	0.009	AC-FT	CF
Notch Width per UD-BMP	3.2	in	

*t = 5 mins

how did you get this value? On the CDs, the forebay is 7.62ft tall on when end and 1.5ft tall on the other.

Based on your inputs on the other 2 forebay calc sheets above, I think you meant to input 1.5ft here.

Subsequently, this value will need to be updated on the CDs (Sht 26).



APPENDIX E

REFERENCE MATERIALS

SUMMARY RUNOFF TABLE

BASIN	AREA (ac)	% IMP	C _o	C _u (in)	C _u (in)
OS1	2.25	7.0	0.14	0.39	7.4
OS2	4.66	7.0	0.14	0.39	2.4
OS3	4.46	7.0	0.14	0.39	2.3
OS4	1.56	7.0	0.14	0.39	3.7
OS5	1.56	7.0	0.14	0.39	3.7
A1	71.49	6.9	0.14	0.39	26.5
A2	66.46	2.1	0.08	0.35	12.3
B1	15.42	1.9	0.08	0.35	3.1
B2	11.26	2.0	0.08	0.35	3.1
B3	12.36	7.0	0.14	0.39	4.8
B4	12.36	7.0	0.14	0.39	4.8
C1	1.56	2.0	0.08	0.35	1.4
C2	1.56	2.0	0.08	0.35	1.4
C3	1.56	2.0	0.08	0.35	1.4
C4	1.56	2.0	0.08	0.35	1.4
F1	37.48	5.0	0.11	0.37	12.1
F2	34.75	6.1	0.12	0.38	14.4
F3	34.75	6.1	0.12	0.38	14.4
F4	34.75	6.1	0.12	0.38	14.4
F5	34.75	6.1	0.12	0.38	14.4
F6	34.75	6.1	0.12	0.38	14.4
F7	34.75	6.1	0.12	0.38	14.4
F8	34.75	6.1	0.12	0.38	14.4
F9	34.75	6.1	0.12	0.38	14.4
T1	5.08	10.8	0.17	0.42	2.9
T2	5.08	10.8	0.17	0.42	2.9
H1	66.36	2.8	0.09	0.36	18.3
H2	66.36	2.8	0.09	0.36	18.3
H3	66.36	2.8	0.09	0.36	18.3
H4	66.36	2.8	0.09	0.36	18.3
H5	66.36	2.8	0.09	0.36	18.3
H6	66.36	2.8	0.09	0.36	18.3
H7	66.36	2.8	0.09	0.36	18.3
H8	66.36	2.8	0.09	0.36	18.3
H9	66.36	2.8	0.09	0.36	18.3
H10	66.36	2.8	0.09	0.36	18.3
H11	66.36	2.8	0.09	0.36	18.3
H12	66.36	2.8	0.09	0.36	18.3
H13	66.36	2.8	0.09	0.36	18.3
H14	66.36	2.8	0.09	0.36	18.3
H15	66.36	2.8	0.09	0.36	18.3
H16	66.36	2.8	0.09	0.36	18.3
H17	66.36	2.8	0.09	0.36	18.3
H18	66.36	2.8	0.09	0.36	18.3
H19	66.36	2.8	0.09	0.36	18.3
H20	66.36	2.8	0.09	0.36	18.3
H21	66.36	2.8	0.09	0.36	18.3
H22	66.36	2.8	0.09	0.36	18.3
H23	66.36	2.8	0.09	0.36	18.3
H24	66.36	2.8	0.09	0.36	18.3
H25	66.36	2.8	0.09	0.36	18.3
H26	66.36	2.8	0.09	0.36	18.3
H27	66.36	2.8	0.09	0.36	18.3
H28	66.36	2.8	0.09	0.36	18.3
H29	66.36	2.8	0.09	0.36	18.3
H30	66.36	2.8	0.09	0.36	18.3
H31	66.36	2.8	0.09	0.36	18.3
H32	66.36	2.8	0.09	0.36	18.3
H33	66.36	2.8	0.09	0.36	18.3
H34	66.36	2.8	0.09	0.36	18.3
H35	66.36	2.8	0.09	0.36	18.3
H36	66.36	2.8	0.09	0.36	18.3
H37	66.36	2.8	0.09	0.36	18.3
H38	66.36	2.8	0.09	0.36	18.3
H39	66.36	2.8	0.09	0.36	18.3
H40	66.36	2.8	0.09	0.36	18.3
H41	66.36	2.8	0.09	0.36	18.3
H42	66.36	2.8	0.09	0.36	18.3
H43	66.36	2.8	0.09	0.36	18.3
H44	66.36	2.8	0.09	0.36	18.3
H45	66.36	2.8	0.09	0.36	18.3
H46	66.36	2.8	0.09	0.36	18.3
H47	66.36	2.8	0.09	0.36	18.3
H48	66.36	2.8	0.09	0.36	18.3
H49	66.36	2.8	0.09	0.36	18.3
H50	66.36	2.8	0.09	0.36	18.3
H51	66.36	2.8	0.09	0.36	18.3
H52	66.36	2.8	0.09	0.36	18.3
H53	66.36	2.8	0.09	0.36	18.3
H54	66.36	2.8	0.09	0.36	18.3
H55	66.36	2.8	0.09	0.36	18.3
H56	66.36	2.8	0.09	0.36	18.3
H57	66.36	2.8	0.09	0.36	18.3
H58	66.36	2.8	0.09	0.36	18.3
H59	66.36	2.8	0.09	0.36	18.3
H60	66.36	2.8	0.09	0.36	18.3
H61	66.36	2.8	0.09	0.36	18.3
H62	66.36	2.8	0.09	0.36	18.3
H63	66.36	2.8	0.09	0.36	18.3
H64	66.36	2.8	0.09	0.36	18.3
H65	66.36	2.8	0.09	0.36	18.3
H66	66.36	2.8	0.09	0.36	18.3
H67	66.36	2.8	0.09	0.36	18.3
H68	66.36	2.8	0.09	0.36	18.3
H69	66.36	2.8	0.09	0.36	18.3
H70	66.36	2.8	0.09	0.36	18.3
H71	66.36	2.8	0.09	0.36	18.3
H72	66.36	2.8	0.09	0.36	18.3
H73	66.36	2.8	0.09	0.36	18.3
H74	66.36	2.8	0.09	0.36	18.3
H75	66.36	2.8	0.09	0.36	18.3
H76	66.36	2.8	0.09	0.36	18.3
H77	66.36	2.8	0.09	0.36	18.3
H78	66.36	2.8	0.09	0.36	18.3
H79	66.36	2.8	0.09	0.36	18.3
H80	66.36	2.8	0.09	0.36	18.3
H81	66.36	2.8	0.09	0.36	18.3
H82	66.36	2.8	0.09	0.36	18.3
H83	66.36	2.8	0.09	0.36	18.3
H84	66.36	2.8	0.09	0.36	18.3
H85	66.36	2.8	0.09	0.36	18.3
H86	66.36	2.8	0.09	0.36	18.3
H87	66.36	2.8	0.09	0.36	18.3
H88	66.36	2.8	0.09	0.36	18.3
H89	66.36	2.8	0.09	0.36	18.3
H90	66.36	2.8	0.09	0.36	18.3
H91	66.36	2.8	0.09	0.36	18.3
H92	66.36	2.8	0.09	0.36	18.3
H93	66.36	2.8	0.09	0.36	18.3
H94	66.36	2.8	0.09	0.36	18.3
H95	66.36	2.8	0.09	0.36	18.3
H96	66.36	2.8	0.09	0.36	18.3
H97	66.36	2.8	0.09	0.36	18.3
H98	66.36	2.8	0.09	0.36	18.3
H99	66.36	2.8	0.09	0.36	18.3
H100	66.36	2.8	0.09	0.36	18.3

CUMULATIVE DESIGN POINT SUMMARY TABLE

DESIGN CONTRIBUTING BASIN	Q _o (cfs)	Q _u (cfs)	Q _u (cfs)	Q _u (cfs)
OS1	0.61	1.5	7.4	2.99
OS2	0.62	2.4	11.4	4.66
OS3	0.62	3.1	17.9	7.51
OS4	0.62	3.7	21.6	9.09
OS5	0.62	4.4	26.0	11.09
A1	1.42	38.6	216.4	127.54
B1	1.82	48.1	281.1	166.36
B2	1.82	48.1	281.1	166.36
B3	1.82	48.1	281.1	166.36
B4	1.82	48.1	281.1	166.36
C1	0.22	14.7	15.9	54.63
C2	0.22	14.7	15.9	54.63
C3	0.22	14.7	15.9	54.63
C4	0.22	14.7	15.9	54.63
F1	1.15	3.9	32.9	11.09
F2	1.15	3.9	32.9	11.09
F3	1.15	3.9	32.9	11.09
F4	1.15	3.9	32.9	11.09
F5	1.15	3.9	32.9	11.09
F6	1.15	3.9	32.9	11.09
F7	1.15	3.9	32.9	11.09
F8	1.15	3.9	32.9	11.09
F9	1.15	3.9	32.9	11.09
T1	1.15	3.9	32.9	11.09
T2	1.15	3.9	32.9	11.09
H1	1.15	3.9	32.9	11.09
H2	1.15	3.9	32.9	11.09
H3	1.15	3.9	32.9	11.09
H4	1.15	3.9	32.9	11.09
H5	1.15	3.9	32.9	11.09
H6	1.15	3.9	32.9	11.09
H7	1.15	3.9	32.9	11.09
H8	1.15	3.9	32.9	11.09
H9	1.15	3.9	32.9	11.09
H10	1.15	3.9	32.9	11.09
H11	1.15	3.9	32.9	11.09
H12	1.15	3.9	32.9	11.09
H13	1.15	3.9	32.9	11.09
H14	1.15	3.9	32.9	11.09
H15	1.15	3.9	32.9	11.09
H16	1.15	3.9	32.9	11.09
H17	1.15	3.9	32.9	11.09
H18	1.15	3.9	32.9	11.09
H19	1.15	3.9	32.9	11.09
H20	1.15	3.9	32.9	11.09
H21	1.15	3.9	32.9	11.09
H22	1.15	3.9	32.9	11.09
H23	1.15	3.9	32.9	11.09
H24	1.15	3.9	32.9	11.09
H25	1.15	3.9	32.9	11.09
H26	1.15	3.9	32.9	11.09
H27	1.15	3.9	32.9	11.09
H28	1.15	3.9	32.9	11.09
H29	1.15	3.9	32.9	11.09
H30	1.15	3.9	32.9	11.09
H31	1.15	3.9	32.9	11.09
H32	1.15	3.9	32.9	11.09
H33	1.15	3.9	32.9	11.09
H34	1.15	3.9	32.9	11.09
H35	1.15	3.9	32.9	11.09
H36	1.15	3.9	32.9	11.09
H37	1.15	3.9	32.9	11.09
H38	1.15	3.9	32.9	11.09
H39	1.15	3.9	32.9	11.09
H40	1.15	3.9	32.9	11.09
H41	1.15	3.9	32.9	11.09
H42	1.15	3.9	32.9	11.09
H43	1.15	3.9	32.9	11.09
H44	1.15	3.9	32.9	11.09
H45	1.15	3.9	32.9	11.09
H46	1.15	3.9	32.9	11.09
H47	1.15	3.9	32.9	11.09
H48	1.15	3.9	32.9	11.09
H49	1.15	3.9	32.9	11.09
H50	1.15	3.9	32.9	11.09
H51	1.15	3.9	32.9	11.09
H52	1.15	3.9	32.9	11.09
H53	1.15	3.9	32.9	11.09
H54	1.15	3.9	32.9	11.09
H55	1.15	3.9	32.9	11.09
H56	1.15	3.9	32.9	11.09
H57	1.15	3.9	32.9	11.09
H58	1.15	3.9	32.9	11.09
H59	1.15	3.9	32.9	11.09
H60	1.15	3.9	32.9	11.09
H61	1.15	3.9	32.9	11.09
H62	1.15	3.9	32.9	11.09
H63	1.15	3.9	32.9	11.09
H64	1.15	3.9	32.9	11.09
H65	1.15	3.9	32.9	11.09
H66	1.15	3.9	32.9	11.09
H67	1.15	3.9	32.9	11.09
H68	1.15	3.9	32.9	11.09
H69	1.15	3.9	32.9	11.09
H70	1.15	3.9	32.9	11.09
H71	1.15	3.9	32.9	11.09
H72	1.15	3.9	32.9	11.09
H73	1.15	3.9	32.9	11.09
H74	1.15	3.9	32.9	11.09
H75	1.15	3.9	32.9	11.09
H76	1.15	3.9	32.9	11.09
H77	1.15	3.9	32.9	11.09
H78	1.15	3.9	32.9	11.09
H79	1.15	3.9	32.9	11.09
H80	1.15	3.9	32.9	11.09
H81	1.15	3.9	32.9	11.09
H82	1.15	3.9	32.9	11.09
H83	1.15	3.9	32.9	11.09
H84	1.15	3.9	32.9	11.09
H85	1.15	3.9	32.9	11.09
H86	1.15	3.9	32.9	11.09
H87	1.15	3.9	32.9	11.09
H88	1.15	3.9	32.9	11.09
H89	1.15	3.9	32.9	11.09
H90	1.15	3.9	32.9	11.09
H91	1.15	3.9	32.9	11.09
H92	1.15	3.9	32.9	11.09
H93	1.15	3.9	32.9	11.09
H94	1.15	3.9	32.9	11.09
H95	1.15	3.9	32.9	11.09
H96	1.15	3.9	32.9	11.09
H97	1.15	3.9	32.9	11.09
H98	1.15	3.9	32.9	11.09
H99	1.15	3.9	32.9	11.09
H100	1.15	3.9	32.9	11.09
J1	2.2	10.5	10.50	2.0
J2	2.2	10.5	10.50	2.0
J3	2.2	10.5	10.50	2.0
J4	2.2	10.5	10.50	2.0
J5	2.2	10.5	10.50	2.0
J6	2.2	10.5	10.50	2.0
J7	2.2	10.5	10.50	2.0
J8	2.2	10.5	10.50	2.0
J9	2.2	10.5	10.50	2.0
J10	2.2	10.5	10.50	2.0
J11	2.2	10.5	10.50	2.0
J12	2.2	10.5	10.50	2.0
J13	2.2	10.5	10.50	2.0
J14	2.2	10.5	10.50	2.0
J15	2.2	10.5	10.50	2.0
J16	2.2	10.5	10.50	2.0
J17	2.2	10.5	10.50	2.0
J18	2.2	10.5	10.50	2.0
J19	2.2	10.5	10.50	2.0
J20	2.2	10.5	10.50	2.0
J21	2.2	10.5	10.50	2.0
J22	2.2	10.5	10.50	2.0
J23	2.2	10.5	10.50	2.0
J24	2.2	10.5	10.50	2.0
J25	2.2	10.5	10.50	2.0
J26	2.2	10.5	10.50	2.0
J27	2.2	10.5	10.50	2.0
J28	2.2	10.5	10.50	2.0
J29	2.2	10.5	10.50	2.0
J30	2.2	10.5	10.50	2.0
J31	2.2	10.5	10.50	2.0
J32	2.2	10.5	10.50	2.0
J33	2.2	10.5	10.50	2.0
J34	2.2	10.5	10.50	2.0
J35	2.2	10.5	10.50	2.0
J36	2.2	10.5	10.50	2.0
J37	2.2	10.5	10.50	2.0
J38	2.2	10.5	10.50	2.0
J39	2.2	10.5	10.50	2.0
J40	2.2	10.5	10.50	2.0
J41	2.2	10.5	10.50	2.0
J42	2.2	10.5	10.50	2.0
J43	2.2	10.5	10.50	2.0
J44	2.2	10.5	10.50	2.0
J45	2.2	10.5	10.50	2.0
J46	2.2	10.5	10.50	2.0
J47	2.2	10.5	10.50	2.0
J48	2.2	10.5	10.50	2.0
J49	2.2	10.5	10.50	2.0
J50	2.2	10.5	10.50	2.0
J51	2.2	10.5	10.50	2.0
J52	2.2	10.5	10.50	2.0
J53	2.2	10.5	10.50	2.0
J54	2.2	10.5	10.50	2.0
J55	2.2	10.5	10.50	2.0
J56	2.2	10.5	10.50	2.0
J57	2.2	10.5	10.50	2.0
J58	2.2	10.5	10.50	2.0
J59	2.2	10.5	10.50	2.0
J60	2.2	10.5	10.50	2.0
J61	2.2	10.5	10.50	2.0
J62	2.2	10.5	10.50	2.0
J63	2.2	10.5	10.50	2.0
J64	2.2	10.5	10.50	2.0
J65	2.2	10.5	10.50	2.0
J66	2.2	10.5	10.50	2.0
J67	2.2	10.5	10.50	2.0
J68	2.2	10.5	10.50	2.0
J69	2.2	10.5	10.50	2.0
J70	2.2	10.5	10.50	2.0
J71	2.2	10.5	10.50	2.0
J72	2.2	10.5	10.50	2.0
J73	2.2	10.5	10.50	2.0
J74	2.2	10.5	10.50	2.0
J75	2.2	10.5	10.50	2.0
J76	2.2	10.5	10.50	2.0
J77	2.2	10.5	10.50	2.0
J78	2.2	10.5	10.50	2.0
J79	2.2	10.5	10.50	2.0
J80	2.2	10.5	10.50	2.0
J81	2.2	10.5	10.50	2.0
J82	2.2	10.5	10.50	2.0
J83	2.2	10.5	10.50	2.0
J84	2.2	10.5	10.50	2.0
J85	2.2	10.5	10.50	2.0
J86	2.2	10.5	10.50	2.0
J87	2.2	10.5	10.50	2.0
J88	2.2	10.5	10.50	2.0
J89	2.2	10.5	10.50	2.0
J90	2.2	10.5	10.50	2.0
J91	2.2	10.5	10.50	2.0
J92	2.2	10.5	10.50	2.0
J93	2.2	10.5	10.50	2.0
J94	2.2	10.5	10.50	2.0
J95	2.2	10.5	10.50	2.0
J96	2.2	10.5	10.50	2.0
J97	2.2	10.5	10.50	2.0
J98	2.2	10.5	10.50	2.0
J99	2.2	10.5	10.50	2.0
J100	2.2	10.5	10.50	2.0