FINAL DRAINAGE REPORT

GRANDVIEW RESERVE FILING NO. 1

El Paso County, Colorado

PREPARED FOR:
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9555 S. Kingston Court
Englewood, CO

PREPARED BY:
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1155 Kelly Johnson Blvd., Suite 305
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DATE:
March 15, 2024

PCD Filing No.: SF2311

ENGINEER'S STATEMENT

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Treven Edwards, PE #60124 Date
For and on behalf of Galloway & Company, Inc.

DEVELOPER'S CERTIFICATION

I, The developer, have read and will comply with all of the requirements specified in this drainage report and plan.

By: Date

Address: D.R. Horton
9555 S. Kingston Court
Englewood, CO

EL PASO COUNTY CERTIFICATION

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

Joshua Palmer, P.E. Date
County Engineer/ECM Administrator

Conditions:

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I. Purpose

The purpose of this Final Drainage Report is to identify on and offsite drainage patterns, locate and identify tributary or downstream drainage features and facilities that impact the site, and to identify which types of drainage facilities will be needed and where they will be located. This report will remain in general compliance with the approved MDDP prepared by HR Green, dated November 2020 and Preliminary Drainage Report (PDR) prepared by Galloway & Company, Inc., dated January 19, 2024.

II. General Description

The Filing No. 1 project site is a single-family residential development located in the Falcon area of El Paso County, Colorado. The Filing No. 1 project site is located in a portion of the South half of Section 21, the North half of Section 28, Township 12 South, Range 64 West of the 6th Principal Meridian, County of El Paso, State of Colorado. The subject property includes Eastonville Road to the west, which was studied separately in the "*Eastonville Road Preliminary Drainage Report*", by HR Green, September 2023, EPC # CDR2321 (**E-PDR**), and is currently in review with El Paso County. The project site is bounded by undeveloped land proposed as future development to the east, and undeveloped land within the Waterbury Development to the south. A Vicinity Map is included in **Appendix A**..

This final drainage report is the basis for the drainage facility design in conformance with the previously approved MDDP for the site prepared by HR Green, "*Grandview Reserve Master Development Drainage Plan",* HR Green, November 2020, EPC # SKP201 (**MDDP**) and the approved preliminary drainage report, "*Preliminary Drainage Report - Grandview Reserve Filing No. 1",* Galloway & Company, Inc., January 19, 2024 (**PDR**). The site consists of approximately 37.564 acres and includes 119 dwelling units.

The existing soil types within the proposed site as determined by the NRCS Web Soil Survey for El Paso County Area consist of Columbine gravelly sandy loam (hydrologic soil group A) and Stapleton sandy loam (hydrologic soil group B). See the soils map included in **Appendix A**.

III. Drainage Criteria

Hydrology calculations were performed using the City of Colorado Springs/El Paso County Drainage Criteria Manual, as revised in November 1991 and October 1994 with County adopted Chapter 6 and Section 3.2.1 of Chapter 13 of the City of Colorado Springs/El Paso County Drainage Criteria Manual as revised in May 2014.

The drainage calculations were based on the criteria manual Figure 6-5 and IDF equations to determine the intensity and are listed in Table 1 below.

Table 1 - Precipitation Data

Return Period	One Hour Depth (in).	Intensity (in/hr)
5-year	1.50	5.17
100-year	2.52	8.68

The rational method was used to calculate peak flows as the tributary areas are less than 100 acres. The rational method has been proven to be accurate for basins of this size and is based on the following formula:

Q = CIA

Where:

Q = Peak Discharge (cfs)
C = Runoff Coefficient
I = Runoff intensity (inches/hour)
A = Drainage area (acres)

The runoff coefficients are calculated based on land use, percent imperviousness, and design storm for each basin, as shown in the drainage criteria manual (Table 6-6). Composite percent impervious and C values were calculated using the residential, streets, roofs, and lawns coefficients found in Table 6-6 of the manual.

The 100-year event was used as the major storm event. The 5-year event was used as the minor event. The UD-Inlets v5.01 spreadsheet was utilized for the sizing of the proposed sump inlets.

The UD-Detention v4.04 spreadsheet was utilized for the design of the proposed on-site water quality ponds, Ponds D, E, and Eastonville Pond.

IV. Interim Drainage Conditions

HISTORIC CONDITIONS

The site is contained fully within one major drainage basin; the Gieck Ranch Drainage Basin and is tributary to Black Squirrel Creek. The site generally drains from north to south with an average slope of 2% outside of the channel. The rational method was used to analyze the individual basins within the site because their size permits it.

HISTORIC OFF-SITE FLOWS

As described in the approved "*Preliminary Drainage Report for Grandview Reserve Filing No. 1",* Galloway & Company, February 2024, EPC # *PUDSP2110* (**PDR**). There is one (1) major drainageway bordering the Grandview Reserve Filing No. 1 project site to the northeast that currently conveys existing on & off-site flows through and adjacent to the project site to the southeast; This is the Gieck Ranch Tributary #1 (Hereon referred to as Channel A), located along the northeastern boundary of the project site. Channel A drainageway generally flows to the southeast towards Highway 24, before crossing via existing drainage structures. This drainageway is analyzed in the report titled "Grandview Reserve CLOMR Report," Prepared by HR Green. This report is still in review – a discussion will be included in the report about the difference between FEMA flows and the Meridian Ranch MDDP. Subsequent Final Drainage Reports will be revised as necessary to incorporate any changes from the CLOMR report.

Existing upstream tributary analysis (the areas west of Eastonville Road) was performed as part of the **E-PDR** and includes basins EX1, EX2, EX3, EX4, EX5, EX6, and EX7. See the **E-PDR** in **Appendix F** for reference. A description of critical design points from the **E-PDR** that enter the site are summarized below.

Channel A: enters the site via an existing 18" (Public) CMP under Eastonville Rd. The flows at this point are associated with **Design Point 4** of the **E-PDR** and correlates to **Design Point G06** of "*The Sanctuary Filing 1 FDR (Meridian Ranch)",* Tech Contractors, August 2022; Per the **E-PDR**, the total upstream tributary area is 832.7 acres, and Channel A flows entering the existing pipe culvert at **Design Point 4** are: Q_5 = 22.4 cfs, Q_{100} = 491.0 cfs.

Design Point 5: off-site flows enter the site via an existing 18" (Public) CMP crossing Eastonville Rd. The off-site flows are associated with **Design Point 5** of the **E-PDR**; Per the **E-PDR**, the total upstream tributary area is 22.35 acres, and flows entering the existing pipe culvert at **Design Point 5** are: Q_5 = 7.0 cfs, Q_{100} = 43.3 cfs.

Design Point 6: off-site flows enter the site via an existing 18" (Public) CMP crossing Eastonville Rd. The off-site flows are associated with **Design Point 6** of the **E-PDR**; Per the **E-PDR**, the total upstream tributary area is 3.05 acres, and flows entering the existing pipe culvert at **Design Point 5** are: Q_5 = 1.2 cfs, Q_{100} = 6.9 cfs.

Following the preliminary drainage report (PDR), the "existing" condition for this FDR will be after the preliminary / interim overlot grading on the site has taken place.

In the interim condition, overland grading operations will have taken place within the Grandview Reserve Subdivision in preparation for the ultimate proposed condition. While this activity is taking place within the proposed subdivision, no activity is anticipated west of Eastonville Road. The proposed project site lies completely within the Gieck Ranch Drainage Basin and is also situated within two (2) of the larger identified basins (D & E) which have been broken down into four (4) smaller sub-basins. More specifically, within the interim drainage condition, the project site is located within Basins EA-1, TSB-D1, TSB-E1, & TSB-E2. Site runoff will be collected via swales and diverted to one of the three proposed temporary sediment basins. All necessary calculations can be found within the appendices of this report.

INTERIM OFF-SITE FLOWS

Existing upstream tributary analysis (the areas west of Eastonville Road) was performed as part of the **E-PDR** and was discussed earlier in the report under **Section IV – Off-Site Flows**. These design basins remain the same as the existing condition during the interim phase and discussion of them are not included in this section.

INTERIM ON-SITE FLOWS

Basin TSB-D1 (10.09 AC, Q_5 = 2.8 cfs, Q_{100} = 20.0 cfs): Located at the southwestern portion of the site, Basin TSB-D1 consists of future residential lots and future roadways. In the interim overland graded phase of development, imperviousness for this sub-basin can be described as nearly bare ground (2%). Runoff from this basin will sheet flow to the east where it is intercepted by proposed TSB-D1 at **DP 18**. From there, treated runoff from TSB-D1 will be discharged downstream directly to existing Channel A.

Basin TSB-E1 (8.21 AC, Q_5 = 2.5 cfs, Q_{100} = 18.0 cfs): Located at the southern portion of the site, Basin TSB-E1 consists of future residential lots and future roadways. In the interim overland graded phase of development, imperviousness for this sub-basin can be described as nearly bare ground (2%). Runoff from this basin will sheet flow to the east where it is intercepted by proposed TSB-E1 at **DP 19**. From there, treated runoff from TSB-E1 will be discharged downstream directly to **Basin TSB-E2**.

Basin TSB-E2 (13.57 AC, Q_5 = 4.0 cfs, Q_{100} = 28.3 cfs): Located at the southeastern portion of the site, Basin TSB-E2 consists of future residential lots and future roadways. In the interim overland graded phase of development, imperviousness for this sub-basin can be described as nearly bare ground (2%). Runoff from this basin will sheet flow to the east where it is intercepted by proposed TSB-E2 at **DP 20**. From there, treated runoff from TSB-E2 will be discharged downstream directly to existing Channel A.

Design Point 20 (Q_5 = 6.5 cfs, Q_{100} = 46.3 cfs): Located at the south portion of the site, this design point accounts for the total combined flows from **Basin TSB-E1** & **TSB-E2**. Flows from this design point are discharged directly into the existing Channel A.

Basin EA-1 (2.50 AC, Q_5 = 0.7 cfs, Q_{100} = 5.1 cfs): Located along the southeastern property line, Basin EA-1 consists primarily of un-developed disturbed area with a temporary diversion swale put in place to convey existing off-site flows from **DP 5 & 6** through the site to Channel A, as they had in the existing condition. Runoff from this basin will sheet flow into a temporary trapezoidal diversion swale (Swale OS-1) with a 4' bottom width and 3' deep. Flows will then be conveyed north and discharge directly into Channel A at **DP 21**.

Each of the temporary sediment basins (TSBs) has been sized according to the detail from City of Colorado Springs Stormwater Quality Manual, Figure SB-1 and the pond calculations in the Mile High Flood District (MHFD) spreadsheet. Riser pipes within each TSB will discharge flows downstream, following the interim grading patterns, which will adhere to historic drainage patterns and eventually enter respective drainageway (Channel A). Similarly, each TSB will have an overflow spillway which will discharge excess flows downstream in the same drainage pattern as the discharge from the riser pipes within the corresponding TSB. See **Appendix D** for calculation spreadsheets.

V. Four Step Process

The Four Step Process is used to minimize the adverse impacts of urbanization and is a vital component of developing a balanced, sustainable project. Below identifies the approach to the four-step process:

1. Employ Runoff Reduction Practices

This step uses low impact development (LID) practices to reduce runoff at the source. Generally, rather than creating point discharges that are directly connected to impervious areas runoff is routed through pervious areas to promote infiltration. The Impervious Reduction Factor (IRF) method was used and calculations can be found in **Appendix E.**

2. Stabilize Channels

This step implements stabilization to channels to accommodate developed flows while protecting infrastructure and controlling sediment loading from erosion in the drainageways. Erosion protection in the form of riprap pads at all outfall points to the channel to prevent scouring of the channel from point discharges. The existing channel analysis and design for the Main Stem Tributary #2 (MST) is to be completed by others and a report for the channel improvements will be submitted for review separately.

3. Provide Water Quality Capture Volume (WQCV)

This step utilizes formalized water quality capture volume to slow the release of runoff from the site. The EURV volume will release in 72 hours, while the WQCV will release in no less than 40 hours. On-site water quality control volume detention ponds will provide water quality treatment for all of the

developed areas, prior to the runoff being released into either of the major drainage ways. Refer to WQCV Plan in **Appendix F.**

4. Consider Need for Industrial and Commercial BMPs

As this project is all residential development and no commercial or industrial development is proposed, there will be no need for any specialized BMPs which would be associated with an industrial or commercial site.

VI. Proposed Drainage Conditions

The proposed project site lies completely within the Gieck Ranch Drainage Basin and consists of two (2) larger basins (D & E) which have been broken down into thirteen (13) smaller sub-basins. Adjacent Off-site Basins (OS) were analyzed as part of the **E-PDR**. Site runoff for Grandview Reserve Filing No. 1 will be collected via inlets & pipes and diverted to one of the two proposed full spectrum detention ponds. No offsite flows enter the Grandview Reserve Filing No. 1 project site. All necessary calculations can be found within the appendices of this report.

There are no proposed major channel improvements for Channel A associated with this project site / development. The analysis for the channel was completed by HR Green (*Grandview Reserve CLOMR Report*, HR Green; March 22, 2023 (**CLOMR**). A copy of the CLOMR Report is included in **Appendix G** for reference – the CLOMR Report is currently still in review. Final design values will be revised as necessary in subsequent Final Drainage Report submittals.

The project site will provide two (2) Full Spectrum Extended Detention Basins (EDBs). Ponds D & E will discharge treated runoff at historic rates directly into Gieck Ranch Tributary #1 (**MDDP**) / Channel A (**E-PDR**).

As has been mentioned previously, the project site is proposed to have a land use of single family residential. The project site will consist primarily of 1/8 Acre lots, with some 1/4 Acre and 1/3 Acre lots, public roadways, along with dedicated Tracts for amenity uses.

Basin D-1 (2.73 AC, Q_5 = 2.6 cfs, Q_{100} = 8.0 cfs): Located on the western portion of the project site, adjacent to Eastonville Road. This basin consists of residential lots and the west half of Kate Meadow Lane. Runoff from this basin will sheet flow to the adjacent roadways. Flows will then be routed, via curb & gutter, to a proposed (public) 5' CDOT Type 'R' sump inlet, located on the west side of Kate Meadow Lane **(DP D1)**, just north of the intersection of Kate Meadow Lane & Farm Close Court. In the major storm event, flows will overtop the roadway crown and will be split between basins D-1 and D-2. Emergency overflows will be routed downstream via proposed curb and gutter to Design Point D4 within Farm Close Court.

Basin D-2 (0.57 AC, Q_5 = 1.0 cfs, Q_{100} = 2.5 cfs): Located on the western portion of the project site, this basin consists of residential lots and the eastern half of Kate Meadow Lane. Runoff from this basin will sheet flow to the adjacent roadway. Flows will then be routed, via curb & gutter, to a proposed (public) 5' CDOT Type 'R' sump inlet, located on the east side of Kate Meadow Lane **(DP D2)**, just north of the intersection of Kate Meadow Lane & Farm Close Court. In the major storm event, flows will overtop the roadway crown and will be split between basins D-1 and D-2. Emergency overflows will be routed downstream via proposed curb and gutter to Design Point D4 within Farm Close Court.

Basin D-3 (4.33 AC, Q_5 = 6.1 cfs, Q_{100} = 16.3 cfs): Located in the west-central portion of the project site, this basin consists of residential lots and the western half of Farm Close Court. Runoff from this basin will sheet flow to the adjacent roadway. Flows will then be routed, via curb & gutter, to a proposed (public) 15' CDOT Type 'R' inlet in sump condition, located on the west side of Farm Close Court **(DP D4)**, southeast of the intersection of Kate Meadow Lane & Farm Close Court cul-de-sac. In the major storm event, flows will overtop the roadway crown and will be split between basins D-3 and D-4. Emergency overflows will overtop the crown and be routed downstream via an emergency overflow swale to the east which conveys runoff directly to Pond D.

Basin D-4 (3.65 AC, Q_5 = 4.4 cfs, Q_{100} = 11.8 cfs): Located in the north-central portion of the project site, this basin consists of residential lots and the eastern half of Farm Close Court. Runoff from this basin will sheet flow to the adjacent roadway. Flows will then be routed, via curb & gutter, to a proposed (public) 15' CDOT Type 'R' inlet in sump condition, located on the east side of Farm Close Court **(DP D6)**, just southeast of the intersection of Kate Meadow Lane & Farm Close Court cul-de-sac. In the major storm event, flows will overtop the roadway crown and will be split between basins D-3 and D-4. Emergency overflows will overtop curb & gutter and be routed downstream via a graded swale within the maintenance access path to Pond D.

Basin D-5 (1.59 AC, Q_5 = 0.7 cfs, Q_{100} = 3.0 cfs): Located along the northwest corner of the project site, adjacent to the Gieck Ranch Tributary #1 / Channel A drainageway. This basin consists partially of residential lots and the proposed (private) Full Spectrum Detention Pond D. Runoff from this basin will sheet flow directly to Pond D. Flows will then be routed to the outlet structure **(DP D7)**, via a concrete trickle channel, where it will eventually discharge, at historic rates, into the adjacent Main Stem channel.

Basin D-6 (0.92 AC, Q_5 = 0.2 cfs, Q_{100} = 1.5 cfs): Located along the northwest corner of the project site, adjacent to the Gieck Ranch Tributary #1 / Channel A drainageway. This basin consists of the undeveloped area outside and downstream of the proposed (private) Full Spectrum Detention Pond D. Runoff from this basin will sheet flow directly to the Gieck Ranch Tributary #1 / Channel A drainageway. All roof drains (for lots 18-20) within this sub-basin will be directed toward Farm Close Court, no impervious surfaces will be allowed within the rear lot setbacks and runoff reduction will be implemented within this sub-basin.

Basin E-1 (4.47 AC, Q_5 = 4.1 cfs, Q_{100} = 12.4 cfs): Located in the southwestern portion of the project site, this basin consists of residential lots, the southern half of Brixham Drive, Starcross Court, and the southern half of Kate Meadow Lane. Runoff from this basin will sheet flow to the adjacent roadways. Flows will then be routed, via curb & gutter, to a proposed (public) 15' CDOT Type 'R' at-grade inlet, located approximately 150-feet to the northeast of the intersection between Kate Meadow Lane and Starcross Court **(DP E1)**. Bypass flows are conveyed downstream via curb & gutter to **DP E4**.

Basin E-2 (1.94 AC, Q_5 = 3.3 cfs, Q_{100} = 8.4 cfs): Located on the southwestern portion of the project site, this basin consists of residential lots, a small portion of Mill Yard Circle, and the north half of Kate Meadow Lane. Runoff from this basin will sheet flow to the adjacent roadways. Flows will then be routed, via curb & gutter, to a proposed (public) 15' CDOT Type 'R' at-grade inlet, located approximately 150-feet to the northeast of the intersection between Kate Meadow Lane and Starcross Court **(DP E2)**. Bypass flows are conveyed downstream via curb & gutter to **DP E4**.

Basin E-3a (2.90 AC, Q_5 = 4.3 cfs, Q_{100} = 11.0 cfs): Located on the south-central portion of the project site, this basin consists of residential lots the western and southern half of Mill Yard Circle as well as a portion of Kate Meadow Lane. Runoff from this basin will sheet flow to the adjacent roadways. Flows will

then be routed, via curb & gutter, to a proposed (public) 15' CDOT Type 'R' at-grade inlet, located just southeast from the intersection between Kate Meadow Lane and Mill Yard Circle **(DP E4)**. In the major storm event, flows will overtop the roadway crown and will be split between basins E-3a and E-4a. Bypass flows are conveyed downstream via curb & gutter to **DP E7**.

Basin E-3b (2.12 AC, Q_5 = 3.5 cfs, Q_{100} = 8.9 cfs): Located on the southeastern portion of the project site, this basin consists of the rear portion of residential lots along Kate Meadow Lane and full residential lots and the western half of Mill Yard Circle near the cul-de-sac. Runoff from this basin will sheet flow to the adjacent roadways. Flows will then be routed, via curb & gutter, to a proposed (public) 15' CDOT Type 'R' sump inlet, located just northeast from the cul-de-sac of Mill Yard Circle **(DP E7)**. In the major storm event, flows will overtop the roadway crown and will be split between basins E-3b and E-4b. Emergency overflows will overtop the crown and be routed downstream via an emergency overflow swale to the southeast which conveys runoff directly to Pond E via a graded emergency overflow swale.

Basin E-4a (7.45 AC, Q_5 = 6.8 cfs, Q_{100} = 20.3 cfs): Located in the central portion of the project site, this basin consists of residential lots and the northern and eastern half of Mill Yard Circle. Runoff from this basin will sheet flow to the adjacent roadway. Flows will then be routed, via curb & gutter, to a proposed (public) 15' CDOT Type 'R' at-grade inlet, located just southeast from the intersection between Kate Meadow Lane and Mill Yard Circle **(DP E5)**. In the major storm event, flows will overtop the roadway crown and will be split between basins E-4a and E-3a. Bypass flows are conveyed downstream via curb & gutter to **DP E9**.

Basin E-4b (1.00 AC, Q_5 = 1.7 cfs, Q_{100} = 4.2 cfs): Located on the southeastern corner of the project site, this basin consists of residential lots and the eastern half of Mill Yard Circle near the cul-de-sac. Runoff from this basin will sheet flow to the adjacent roadway. Flows will then be routed, via curb & gutter, to a proposed (public) 15' CDOT Type 'R' sump inlet, located just southeast from the intersection between Kate Meadow Lane and Mill Yard Circle **(DP E5)**. In the major storm event, flows will overtop the roadway crown and will be split between basins E-3b and E-4b.Emergency overflows will overtop the curb and be routed downstream via an emergency overflow swale to the southeast which conveys runoff directly to Pond E via a graded emergency overflow swale.

Basin E-5 (1.43 AC, Q_5 = 0.3 cfs, Q_{100} = 1.8 cfs): Located on the southeast corner of the project site, adjacent to the Gieck Ranch Tributary #1 / Channel A drainageway. This basin consists of the proposed (private) Full Spectrum Detention Pond E. Runoff from this basin will sheet flow directly to Pond E. Flows will then be routed to the outlet structure **(DP E10)**, via a concrete trickle channel, where it will eventually discharge, at historic rates, into the adjacent Gieck Ranch Tributary #1 / Channel A drainageway.

Basin E-6 (2.40 AC, Q_5 = 0.7 cfs, Q_{100} = 4.4 cfs): Located on the southeast corner of the project site, adjacent to the Gieck Ranch Tributary #1 / Channel A drainageway. This basin consists of the undeveloped area outside and downstream of the proposed (private) Full Spectrum Detention Pond E. Runoff from this basin will sheet flow directly to the Gieck Ranch Tributary #1 / Channel A drainageway and offsite to the east.

VII. Storm Sewer System

All development is anticipated to be urban and will include storm sewer & street inlets. Storm sewers collect storm water runoff and convey the water to the water quality facilities prior to discharging. Storm sewer systems will be designed to the 100-year storm and checked with the 5-year storm. Inlets will be

placed at sump areas and intersections where street flow is larger than street capacity. UDFCD Inlet spreadsheet has been used to determine the size of all sump inlets.

There will be two (2) proposed storm systems within the project site. Each of the two storm sewer systems will discharge storm water into its correlated WQCV pond.

Each system will consist of reinforced concrete pipe (RCP), CDOT Type 'R' inlets, and storm sewer manholes.

Furthermore, there are two (2) proposed overflow swales at the low-points within Farm Close Court and Mill Yard Circle which convey flows to Ponds D and E, respectively. The swales were analyzed using the Bentley software FlowMaster to properly size channels (trapezoidal 3' W x 2.0' D for Pond D overflow swale) and (triangular 7.38' top W x 2.5' D for Pond E overflow swale), to convey the 100-year flows from the respective basins to corresponding outfall location (Pond D and Pond E), while providing 1.0-ft of freeboard. The sizing calculations can be found in **Appendix D.**

This Final drainage report includes details concerning sump and at-grade inlet locations, street capacity, storm sewer sizing, outlet protection and locations. The calculations can be found in **Appendix D**.

VIII. Proposed Water Quality Detention Ponds

Two (2) Full Spectrum Detention Ponds will be provided for the proposed site. Both of these ponds (Ponds D & E) are private and will be maintained by the DISTRICT, once established. These detention ponds are proposed to be full spectrum and will provide water quality and detention. The WQCV and EURV release will be controlled with an orifice plate. The release rates for the WQCV and EURV will be 40-hours and 72-hours, respectively. All storm event volumes up to the 100-year event will be controlled by orifice and/or restrictor plate and will be designed to release at or below the pre-development flow rate. Outlet structures, forebays, trickle channels, etc. are included with this final drainage report. The required FSD pond volumes are as described below:

Pond D: Located centrally on the site, just west of the Gieck Ranch Tributary #1 / Channel A drainageway. This pond will discharge into the Main Stem channel. The required volume WQCV and EURV are 0.227 Ac-Ft & 0.782 Ac-Ft, respectively. The provided storage for the WQCV and EURV are 0.229 Ac-Ft & 0.787 Ac-Ft, respectively. The total required detention basin volume is 1.596 Ac-Ft. The total provided detention basin storage is 1.115 Ac-Ft. In the 100-year event, Pond D releases 90% of the predeveloped peak flow (8.0 cfs).

Pond E: Located on the south side of the site, just west of Gieck Ranch Tributary #1 / Channel A drainageway. This pond will discharge into the Main Stem channel. The required volume WQCV and EURV are 0.377 Ac-Ft & 1.295 Ac-Ft, respectively. The provided storage for the WQCV and EURV are 0.377 Ac-Ft & 1.301 Ac-Ft, respectively. The total required detention basin volume is 2.639 Ac-Ft. The total provided detention basin storage is 1.824 Ac-Ft. In the 100-year event, Pond D releases 90% of the predeveloped peak flow (8.0 cfs). In the 100-year event, Pond E releases 90% of the predeveloped peak flow (14.9 cfs).

IX. Proposed Channel Improvements

According to the **MDDP**, there is one (1) major drainageway that runs immediately adjacent to the project site. The Gieck Ranch Tributary #1 / Channel A drainageway (**E-PDR**) along the northeastern boundary

of the project site conveying runoff from the northwest to the southeast. There are no proposed major channel improvements for Channel A as part of this project (to be determined with EPC # CDR-22-008; *Grandview Reserve Geick Basin Channel*). An analysis has been done for Channel A with both existing and future condition flows as described within the *Grandview Reserve CLOMR Report*, HR Green; March 22, 2023 (**CLOMR**). All HEC-RAS modelling, velocities, shear, depths, etc. are included within the CLOMR, which can be found in **Appendix E**. Both scenarios, throughout the channel fall within the channel stability criteria.

A majority of the developed runoff will be captured and conveyed to one of the corresponding water quality and detention facilities and release at or below historic levels. Ponds D and E will release directly into the Gieck Ranch Tributary #1 / Channel A drainageway. These basins are contained within the backs of lots and will provide water quality through runoff reduction; impervious areas will not be permitted in the back of these lots and roof drains are to drain to the front. Therefore, there will be no adverse impact to downstream facilities. The analysis for the drainageway (Channel A) and offsite upstream tributary capture was done by HR Green within the *Grandview Reserve CLOMR Report*, HR Green; March 22, 2023 (**CLOMR**) which will be submitted separately for review. A copy of this report is included in **Appendix E**.

X. Maintenance

After completion of construction and upon the Board of County Commissioners acceptance, it is anticipated all drainage facilities within the public Right-of-Way are to be owned and maintained by El Paso County (i.e. Eastonville Road FSD).

Both private detention ponds are to be owned and maintained by the Grandview Reserve Metropolitan District No. 2 (DISTRICT), once established, unless an agreement is reached stating otherwise. The proposed Gieck Ranch Tributary #1 / Channel A drainageway is not proposed to be disturbed. Maintenance access for all full spectrum detention facilities will be provided from public Right-of-Way.

XI. Wetlands Mitigation

There are two existing wetlands on site associated with the one (1) major channel, Gieck Ranch Tributary #1 / Channel A drainageway. The wetlands are contained within the existing channel with the wetland in Gieck Ranch Tributary #1 / Channel A drainageway being classified as jurisdictional. The wetlands USACE determination will be provided with the *Grandview Reserve CLOMR Report*, HR Green; March 22, 2023 (**CLOMR)**, which can be found in Appendix D. Wetlands maintenance will be the responsibility of the Grandview Reserve Metropolitan District No. 2 (DISTRICT).

XII. Floodplain Statement

A portion of the project sit lies with Zone A Special Flood Hazard Area as defined by the FIRM Map number 08041C0552G effective December 7, 2018. A copy of the FIRM Panel is included in **Appendix A.** FEMA-approved floodplain elevations are required to be shown on final plats.

XIII. Drainage Fees & Maintenance

Gieck Ranch Basin is not listed as part of the El Paso County drainage basin fee program. Unless otherwise instructed, no drainage fees will be assessed.

COST OPINION

Item	Quantity	Unit	Unit Cost	Cost
Storm Drain Infrastructure (Public)				
24" RCP	655	LF	$96.00	$62,880.00
30" RCP	305	LF	$120.00	$36,600.00
36" RCP	440	LF	$150.00	$66,000.00
42" RCP	165	LF	$275.00	$45,375.00
CDOT TYPE R 5' Curb Inlet	3	EA	$5,500.00	$16,500.00
CDOT TYPE R 15' Curb Inlet	7	EA	$10,000.00	$70,000.00
CDOT Storm 5' DIA Manhole	15	EA	$7,500.00	$112,500.00
CDOT Storm 6' DIA Manhole	1	EA	$10,000.00	$10,000.00
Subtotal				**$419,855.00**
Total (Public)				**$419,855.00**
Contingency			**10%**	**$41,985.50**
Grand Total (Public)				**$461,840.50**
Storm Drain Infrastructure (Private)				
Trapezoidal Channel	175	LF	$12.00	$2,100.00
Triangular Channel	150	LF	$8.00	$1,200.00
Channel RECP (North American Green)	4,538	SY	$8.00	$36,304.00
18" Flared End Section	2	EA	$2,750.00	$5,500.00
Subtotal				**$45,104.00**
Pond D Improvements (Private)				
Earthwork	7,435	CY	$20.00	$148,700.00
Forebay	1	EA	$10,000.00	$10,000.00
Hand Rail Fence (Forebays)	180	LF	$6.00	$1,080.00
Type L Rip-Rap (Emergency Spillway)	75	CY	$120.00	$9,000.00
Trickle Channel	325	LF	$15.00	$4,875.00
Outlet Structure w/ Micropool	1	EA	$15,000.00	$15,000.00
18" RCP Storm Pipe	100	LF	$80.00	$8,000.00
Gravel Maintenance Access	39	CY	$45.00	$1,755.00
Subtotal				**$198,410.00**
Pond E Improvements (Private)				
Earthwork	5,775	CY	$20.00	$115,500.00
Forebay	1	EA	$10,000.00	$10,000.00
Hand Rail Fence (Forebays)	180	LF	$6.00	$1,080.00
Type L Rip-Rap (Emergency Spillway)	75	CY	$120.00	$9,000.00
Trickle Channel	450	LF	$15.00	$6,750.00
Outlet Structure w/ Micropool	1	EA	$15,000.00	$15,000.00
18" RCP Storm Pipe	70	LF	$80.00	$5,600.00
Gravel Maintenance Access	26	CY	$45.00	$1,170.00
Subtotal				**$164,100.00**
Total (Private)				**$407,614.00**
Contingency			**10%**	**$40,761.40**
Grand Total (Private)				**$448,375.40**

XIV. Conclusion

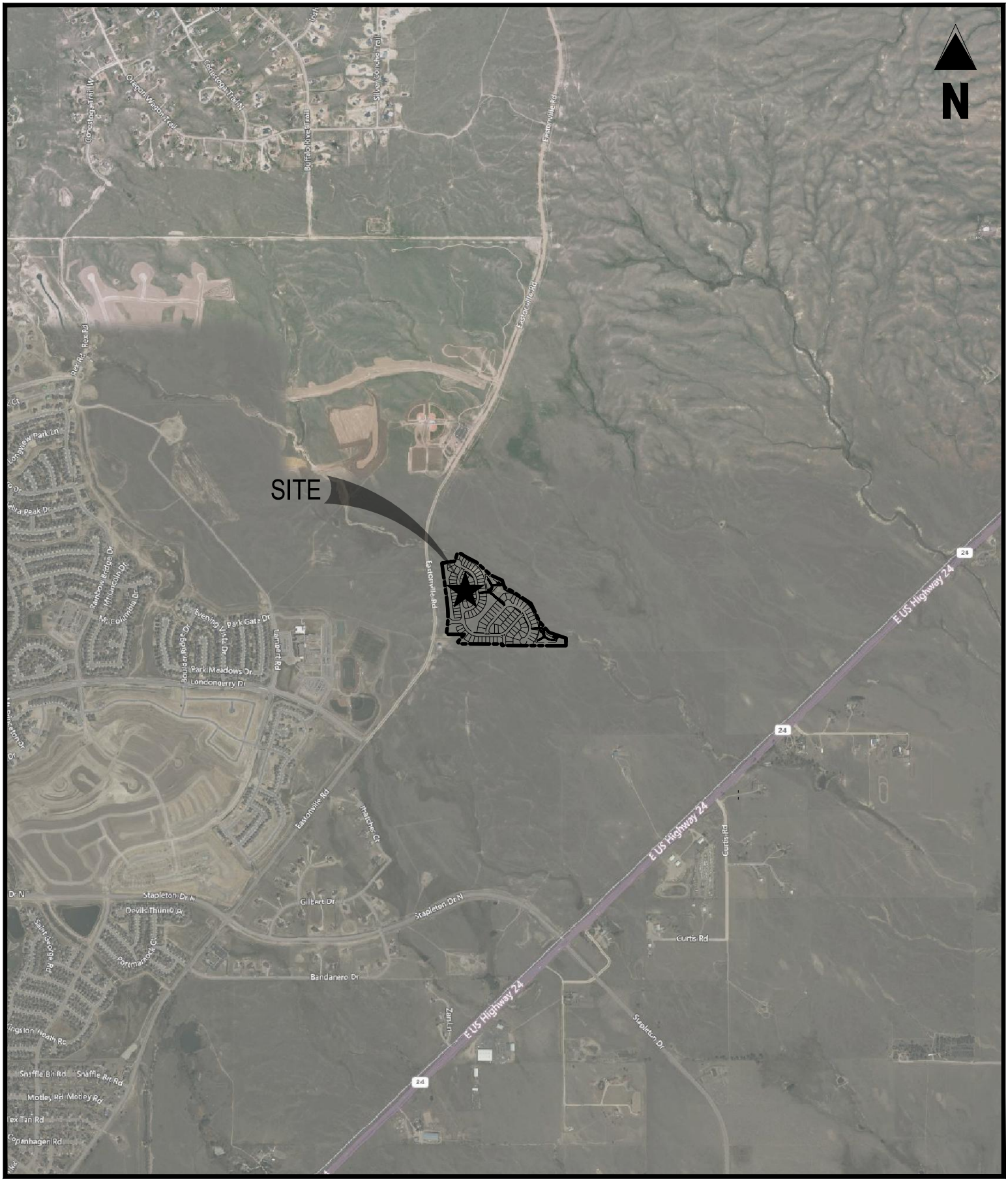
The Grandview Reserve residential subdivision lies within the Gieck Ranch Drainage Basin. Water quality for the project site is provided in two (2) on-site Full Spectrum Detention Ponds; Ponds D & E. All drainage facilities within this report were sized according to the El Paso County Drainage Criteria Manuals. The proposed facilities are adequate to protect the site from generated runoff. The site runoff will not adversely affect the downstream facilities and surrounding developments. There is one (1) major drainageway bordering the Grandview Reserve Filing No. 1 project site to the northeast, which will be addressed by the report titled “Grandview Reserve CLOMR Report,” Prepared by HR Green. The two (2) WQCV ponds will be maintained by a newly established Grandview Reserve Metropolitan District No. 2 (DISTRICT).

XV. References

1. *El Paso County Drainage Criteria Manual*, 1990.
2. *Drainage Criteria Manual, Volume 2*, City of Colorado Springs, 2002.
3. *El Paso County Drainage Criteria Manual Update*, 2015.
4. *El Paso County Engineering Criteria Manual*, 2020.
5. *Urban Storm Drainage Criteria Manual*, Urban Drainage and Flood Control District, January 2016 (with current revisions).
6. *Gieck Ranch Drainage Basin Study (DBPS),* Drexel Barrell, October 2010 (Not adopted by County).
7. *Grandview Reserve Master Development Drainage Plan (MDDP),* HR Green, November 2020.
8. *Grandview Reserve CLOMR Report*, HR Green; March 22, 2023.
9. *Meridian Ranch MDDP,* January 2018, updated 2021.
10. *Eastonville Road Preliminary Drainage Report*", HR Green, September 2023.
11. *The Sanctuary Filing 1 FDR (Meridian Ranch),* Tech Contractors, August 2022.

APPENDIX A

Exhibits and Figures



GRANDVIEW RESERVE
FILING NO. 1

EASTONVILLE RD
SCALE: 1"=2,000'

VICINITY MAP

Project No:	HRG02
Drawn By:	JDM
Checked By:	CMWJ
Date:	03/15/2024

Galloway
1155 Kelly Johnson Blvd., Suite 305
Colorado Springs, CO 80920
719.900.7220 • GallowayUS.com

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The **community map repository** should be consulted for possible updated or additional flood hazard information.

To obtain more detailed information in areas where **Base Flood Elevations** (BFEs) and/or **floodways** have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report that accompanies this FIRM. Users should be aware that BFEs shown on the FIRM represent rounded whole-foot elevations. These BFEs are intended for flood insurance rating purposes only and should not be used as the sole source of flood elevation information. Accordingly, flood elevation data presented in the FIS report should be utilized in conjunction with the FIRM for purposes of construction and/or floodplain management.

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD88). Users of this FIRM should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations table in the Flood Insurance Study report for this jurisdiction. Elevations shown in the Summary of Stillwater Elevations table should be used for construction and/or floodplain management purposes when they are higher than the elevations shown on this FIRM.

Boundaries of the **floodways** were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program. Floodway widths and other pertinent floodway data are provided in the Flood Insurance Study report for this jurisdiction.

Certain areas not in Special Flood Hazard Areas may be protected by **flood control structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The **projection** used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The **horizontal datum** was NAD83, GRS80 spheroid. Differences in datum, spheroid, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the **North American Vertical Datum of 1988 (NAVD88)**. These flood elevations must be compared to structure and ground elevations referenced to the same **vertical datum**. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services
NOAA, N/NGS12
National Geodetic Survey
SSMC-3, #9202
1315 East-West Highway
Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for **bench marks** shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

Base Map information shown on this FIRM was provided in digital format by El Paso County, Colorado Springs Utilities, City of Fountain, Bureau of Land Management, National Oceanic and Atmospheric Administration, United States Geological Survey, and Anderson Consulting Engineers, Inc. These data are current as of 2006.

This map reflects more detailed and up-to-date **stream channel configurations and floodplain delineations** than those shown on the previous FIRM for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRM may have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood Insurance Study Report (which contains authoritative hydraulic data) may reflect stream channel distances that differ from what is shown on this map. The profile baselines depicted on this map represent the hydraulic modeling baselines that match the flood profiles and Floodway Data Tables if applicable, in the FIS report. As a result, the profile baselines may deviate significantly from the new base map channel representation and may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

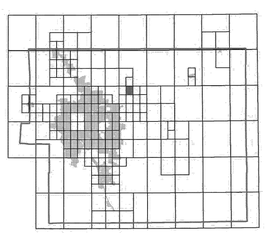
Contact **FEMA Map Service Center** (MSC) via the FEMA Map Information eXchange (FMIX) 1-877-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.fema.gov/.

If you have **questions about this map** or questions concerning the National Flood Insurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) or visit the FEMA website at http://www.fema.gov/business/nfip.

El Paso County Vertical Datum Offset Table

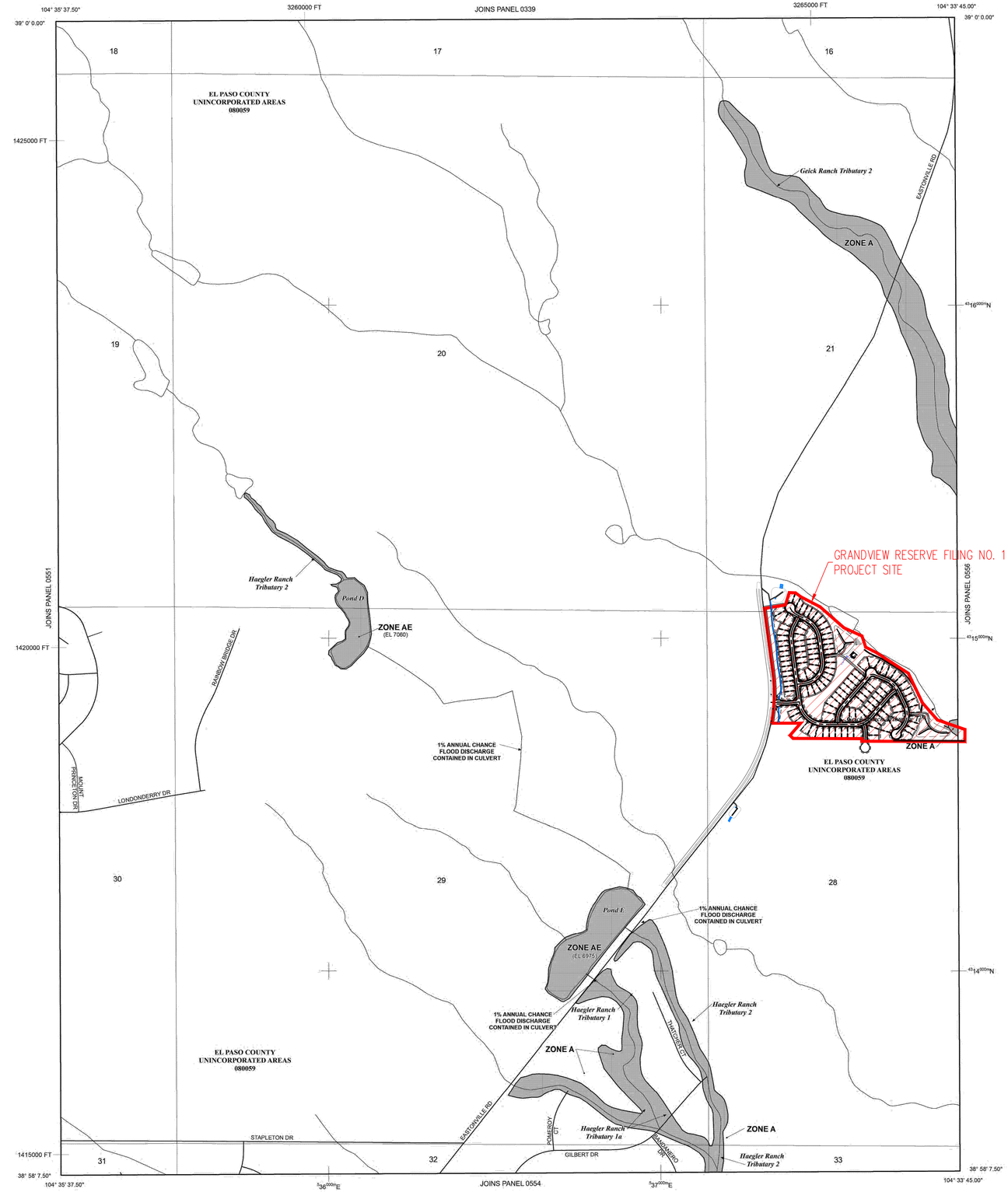
Flooding Source	Vertical Datum Offset (ft)
REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION	

Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).

Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 12 SOUTH, RANGE 64 WEST.

LEGEND

SPECIAL FLOOD HAZARD AREAS (SFHAS) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD

The 1% annual chance flood (100-year flood), also known as the base flood, is the flood that has a 1% chance of being equaled or exceeded in any given year. The Special Flood Hazard Area is the area subject to flooding by the 1% annual chance flood. Areas of Special Flood Hazard include Zones A, AE, AH, AO, AR, A99, V, and VE. The Base Flood Elevation is the water-surface elevation of the 1% annual chance flood.

ZONE A	No Base Flood Elevations determined.
ZONE AE	Base Flood Elevations determined.
ZONE AH	Flood depths of 1 to 3 feet (usually areas of ponding); Base Flood Elevations determined.
ZONE AO	Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined. For areas of alluvial fan flooding, velocities also determined.
ZONE AR	Special Flood Hazard Area Formerly protected from the 1% annual chance flood by a flood control system that was subsequently decertified. Zone AR indicates that the former flood control system is being restored to provide protection from the 1% annual chance or greater flood.
ZONE A99	Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V	Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE	Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.

FLOODWAY AREAS IN ZONE AE

The floodway is the channel of a stream plus any adjacent floodplain areas that must be kept free of encroachment so that the 1% annual chance flood can be carried without substantial increases in flood heights.

OTHER FLOOD AREAS

ZONE X	Areas of 0.2% annual chance flood; areas of 1% annual chance flood with average depths of less than 1 foot or with drainage areas less than 1 square mile; and areas protected by levees from 1% annual chance flood.

OTHER AREAS

ZONE X	Areas determined to be outside the 0.2% annual chance floodplain.
ZONE D	Areas in which flood hazards are undetermined, but possible.

COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS

OTHERWISE PROTECTED AREAS (OPAs)

CBRS areas and OPAs are normally located within or adjacent to Special Flood Hazard Areas.

	Floodplain boundary
	Floodway boundary
	Zone D Boundary
	CBRS and OPA boundary
	Boundary dividing Special Flood Hazard Areas of different Base Flood Elevations, flood depths or flood velocities.
513	Base Flood Elevation line and value; elevation in feet*
(EL 987)	Base Flood Elevation value where uniform within zone; elevation in feet*

* Referenced to the North American Vertical Datum of 1988 (NAVD 88)

A — A	Cross section line
23 — 23	Transect line
97° 07' 30.00" 32° 22' 30.00"	Geographic coordinates referenced to the North American Datum of 1983 (NAD 83)
4375000mN	1000-meter Universal Transverse Mercator grid ticks, zone 13
6000000 FT	5000-foot grid ticks: Colorado State Plane coordinate system, central zone (FIPSZONE 0502), Lambert Conformal Conic Projection
DX5510	Bench mark (see explanation in Notes to Users section of this FIRM panel)
M1.5	River Mile

MAP REPOSITORIES
Refer to Map Repositories list on Map Index

EFFECTIVE DATE OF COUNTYWIDE FLOOD INSURANCE RATE MAP
MARCH 17, 1997

EFFECTIVE DATE(S) OF REVISION(S) TO THIS PANEL
DECEMBER 7, 2018 - to update corporate limits, to change Base Flood Elevations and Special Flood Hazard Areas, to update map format, to add roads and road names, and to incorporate previously issued Letters of Map Revision.

For community map revision history prior to countywide mapping, refer to the Community Map History Table located in the Flood Insurance Study report for this jurisdiction.

To determine if flood insurance is available in this community, contact your insurance agent or call the National Flood Insurance Program at 1-800-638-6620.

MAP SCALE 1" = 500'

250 0 500 1000 FEET

150 0 150 300 METERS

NFIP

PANEL 0552G

NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RATE MAP

EL PASO COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 552 OF 1300

(SEE MAP INDEX FOR FIRM PANEL LAYOUT)

CONTAINS:

COMMUNITY	NUMBER	PANEL	SUFFIX
EL PASO COUNTY	080059	0552	G

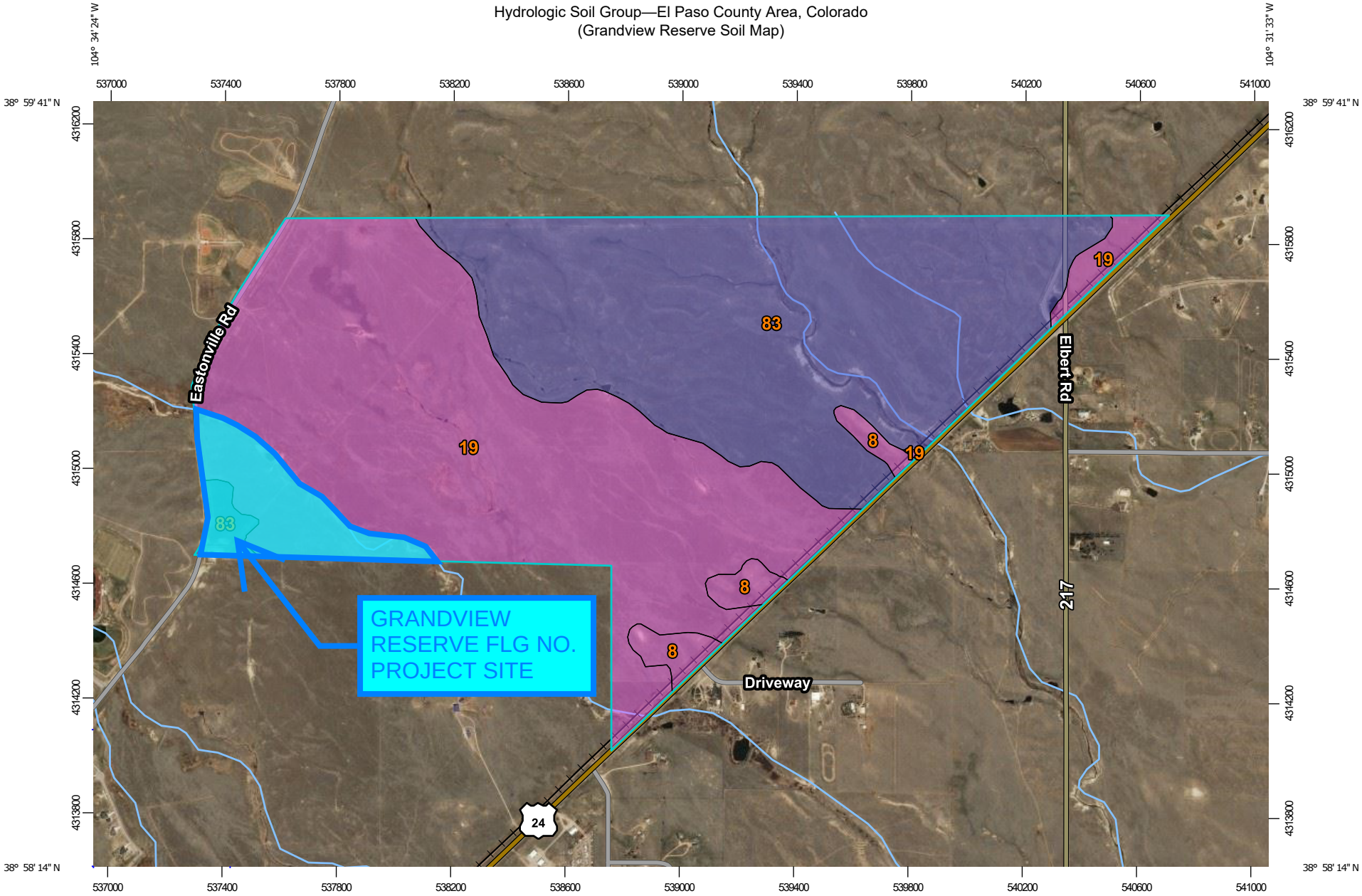
Notice to User: The **Map Number** shown below should be used when placing map orders; the **Community Number** shown above should be used on insurance applications for the subject community.

MAP NUMBER
08041C0552G

MAP REVISED
DECEMBER 7, 2018

Federal Emergency Management Agency

Hydrologic Soil Group—El Paso County Area, Colorado
(Grandview Reserve Soil Map)



Map Scale: 1:18,800 if printed on A landscape (11" x 8.5") sheet.

N

Meters: 0 250 500 1000 1500

Feet: 0 500 1000 2000 3000

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

Area of Interest (AOI)

Soils

Soil Rating Polygons

A
A/D
B
B/D
C
C/D
D
Not rated or not available

Soil Rating Lines

A
A/D
B
B/D
C
C/D
D
Not rated or not available

Soil Rating Points

A
A/D
B
B/D
C
C/D
D
Not rated or not available

Water Features

Streams and Canals

Transportation

Rails
Interstate Highways
US Routes
Major Roads
Local Roads

Background

Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 17, Sep 13, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Sep 8, 2018—May 26, 2019

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	A	22.4	2.6%
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A	450.7	52.5%
83	Stapleton sandy loam, 3 to 8 percent slopes	B	385.4	44.9%
Totals for Area of Interest			**858.5**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

NOAA Atlas 14, Volume 8, Version 2
Location name: Peyton, Colorado, USA*
Latitude: 38.985°, Longitude: -104.565°
Elevation: 6975.71 ft**
* source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Deborah Martin, Sandra Pavlovic, Ishani Roy, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Michael Yekta, Geoffery Bonnin

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	**0.239** (0.189-0.303)	**0.291** (0.231-0.370)	**0.381** (0.301-0.486)	**0.461** (0.361-0.589)	**0.576** (0.440-0.768)	**0.671** (0.499-0.904)	**0.770** (0.554-1.06)	**0.875** (0.604-1.24)	**1.02** (0.678-1.48)	**1.14** (0.733-1.67)
10-min	**0.350** (0.277-0.444)	**0.426** (0.338-0.542)	**0.558** (0.441-0.711)	**0.674** (0.529-0.863)	**0.844** (0.644-1.13)	**0.982** (0.731-1.32)	**1.13** (0.811-1.56)	**1.28** (0.884-1.81)	**1.49** (0.992-2.17)	**1.66** (1.07-2.44)
15-min	**0.426** (0.338-0.541)	**0.520** (0.412-0.660)	**0.681** (0.537-0.867)	**0.823** (0.645-1.05)	**1.03** (0.785-1.37)	**1.20** (0.891-1.62)	**1.37** (0.988-1.90)	**1.56** (1.08-2.21)	**1.82** (1.21-2.65)	**2.03** (1.31-2.98)
30-min	**0.608** (0.482-0.771)	**0.740** (0.586-0.940)	**0.968** (0.764-1.23)	**1.17** (0.916-1.49)	**1.46** (1.11-1.94)	**1.70** (1.26-2.29)	**1.94** (1.40-2.68)	**2.21** (1.52-3.12)	**2.57** (1.71-3.73)	**2.86** (1.85-4.19)
60-min	**0.775** (0.615-0.984)	**0.933** (0.739-1.19)	**1.21** (0.956-1.54)	**1.46** (1.15-1.87)	**1.84** (1.41-2.47)	**2.16** (1.61-2.92)	**2.49** (1.80-3.45)	**2.85** (1.97-4.05)	**3.37** (2.24-4.90)	**3.78** (2.44-5.55)
2-hr	**0.943** (0.754-1.19)	**1.13** (0.898-1.42)	**1.46** (1.16-1.84)	**1.76** (1.39-2.23)	**2.22** (1.72-2.97)	**2.62** (1.97-3.52)	**3.04** (2.21-4.19)	**3.50** (2.45-4.95)	**4.16** (2.80-6.03)	**4.70** (3.06-6.85)
3-hr	**1.03** (0.829-1.29)	**1.22** (0.978-1.53)	**1.57** (1.25-1.97)	**1.90** (1.51-2.40)	**2.41** (1.88-3.22)	**2.86** (2.17-3.84)	**3.35** (2.45-4.60)	**3.88** (2.73-5.48)	**4.66** (3.15-6.74)	**5.29** (3.46-7.69)
6-hr	**1.20** (0.968-1.49)	**1.40** (1.13-1.74)	**1.78** (1.44-2.22)	**2.16** (1.73-2.70)	**2.76** (2.18-3.66)	**3.28** (2.52-4.39)	**3.86** (2.86-5.29)	**4.51** (3.21-6.34)	**5.46** (3.73-7.86)	**6.24** (4.12-9.01)
12-hr	**1.38** (1.13-1.70)	**1.61** (1.31-1.98)	**2.05** (1.67-2.53)	**2.48** (2.00-3.07)	**3.15** (2.51-4.15)	**3.74** (2.89-4.96)	**4.39** (3.28-5.96)	**5.12** (3.67-7.13)	**6.17** (4.25-8.82)	**7.04** (4.69-10.1)
24-hr	**1.60** (1.31-1.95)	**1.87** (1.54-2.28)	**2.38** (1.94-2.91)	**2.85** (2.32-3.51)	**3.60** (2.88-4.67)	**4.24** (3.29-5.56)	**4.94** (3.71-6.63)	**5.71** (4.12-7.87)	**6.82** (4.73-9.66)	**7.73** (5.20-11.0)
2-day	**1.85** (1.54-2.24)	**2.18** (1.80-2.63)	**2.76** (2.28-3.35)	**3.29** (2.70-4.01)	**4.11** (3.30-5.27)	**4.80** (3.76-6.22)	**5.54** (4.19-7.36)	**6.35** (4.62-8.68)	**7.50** (5.25-10.5)	**8.44** (5.73-11.9)
3-day	**2.03** (1.69-2.44)	**2.39** (1.98-2.87)	**3.02** (2.50-3.64)	**3.60** (2.97-4.36)	**4.47** (3.60-5.69)	**5.20** (4.09-6.70)	**5.98** (4.55-7.90)	**6.83** (4.99-9.28)	**8.03** (5.65-11.2)	**9.00** (6.15-12.7)
4-day	**2.18** (1.82-2.61)	**2.56** (2.13-3.06)	**3.22** (2.68-3.87)	**3.82** (3.16-4.62)	**4.73** (3.83-6.00)	**5.49** (4.33-7.04)	**6.30** (4.81-8.30)	**7.18** (5.26-9.72)	**8.43** (5.95-11.7)	**9.43** (6.46-13.3)
7-day	**2.58** (2.17-3.07)	**2.98** (2.50-3.54)	**3.68** (3.08-4.39)	**4.32** (3.60-5.18)	**5.29** (4.31-6.65)	**6.09** (4.84-7.76)	**6.96** (5.34-9.09)	**7.89** (5.82-10.6)	**9.21** (6.55-12.8)	**10.3** (7.10-14.4)
10-day	**2.93** (2.48-3.47)	**3.37** (2.84-3.98)	**4.13** (3.47-4.90)	**4.81** (4.02-5.74)	**5.83** (4.76-7.29)	**6.68** (5.32-8.45)	**7.58** (5.85-9.86)	**8.55** (6.34-11.4)	**9.92** (7.09-13.7)	**11.0** (7.65-15.4)
20-day	**3.91** (3.33-4.58)	**4.51** (3.84-5.29)	**5.52** (4.68-6.50)	**6.39** (5.39-7.55)	**7.63** (6.25-9.37)	**8.62** (6.90-10.8)	**9.64** (7.47-12.4)	**10.7** (7.98-14.1)	**12.2** (8.74-16.6)	**13.3** (9.31-18.4)
30-day	**4.70** (4.02-5.47)	**5.44** (4.65-6.34)	**6.65** (5.66-7.78)	**7.66** (6.49-9.00)	**9.06** (7.44-11.0)	**10.1** (8.15-12.5)	**11.2** (8.74-14.3)	**12.3** (9.24-16.2)	**13.8** (9.98-18.7)	**15.0** (10.5-20.6)
45-day	**5.67** (4.88-6.57)	**6.55** (5.63-7.60)	**7.97** (6.82-9.27)	**9.12** (7.77-10.7)	**10.7** (8.79-12.9)	**11.9** (9.56-14.5)	**13.0** (10.2-16.4)	**14.2** (10.6-18.4)	**15.6** (11.3-21.0)	**16.7** (11.9-23.0)
60-day	**6.49** (5.60-7.48)	**7.46** (6.43-8.62)	**9.01** (7.74-10.4)	**10.3** (8.77-11.9)	**11.9** (9.82-14.3)	**13.1** (10.6-16.0)	**14.3** (11.2-18.0)	**15.5** (11.7-20.0)	**16.9** (12.3-22.6)	**18.0** (12.8-24.6)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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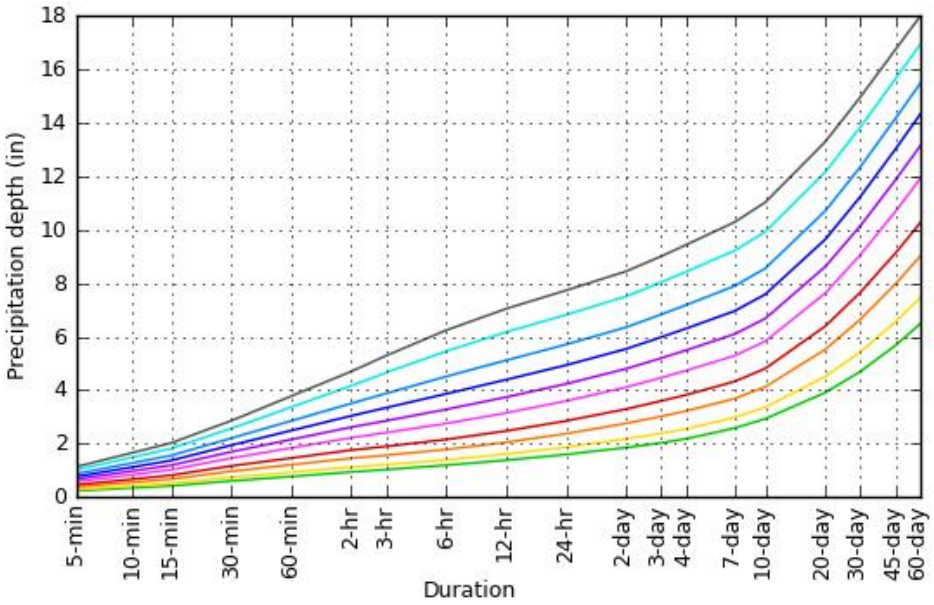
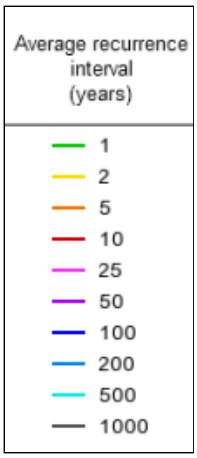
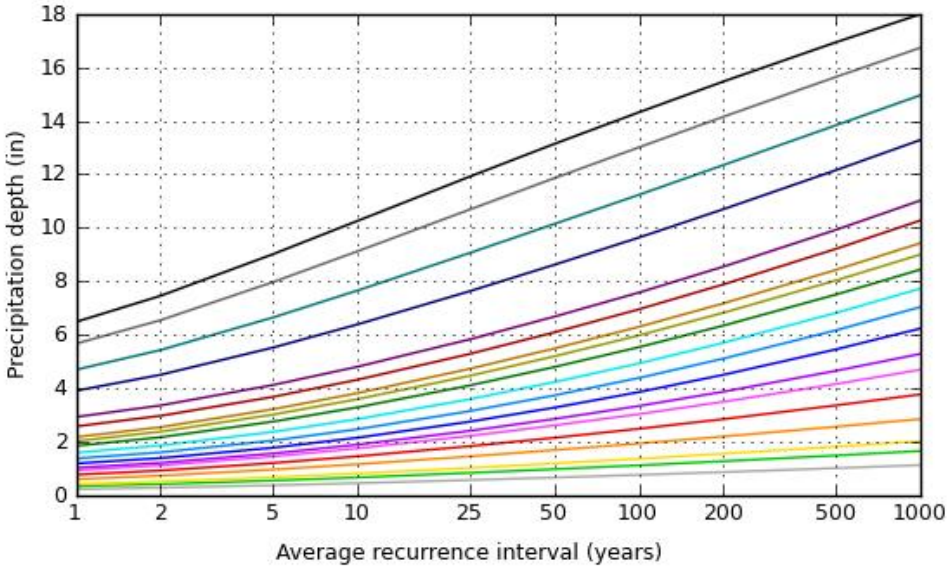
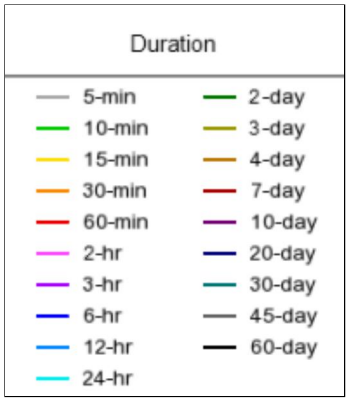
PF graphical

PDS-based depth-duration-frequency (DDF) curves
Latitude: 38.9850°, Longitude: -104.5650°









NOAA Atlas 14, Volume 8, Version 2

Created (GMT): Thu Dec 2 17:16:51 2021

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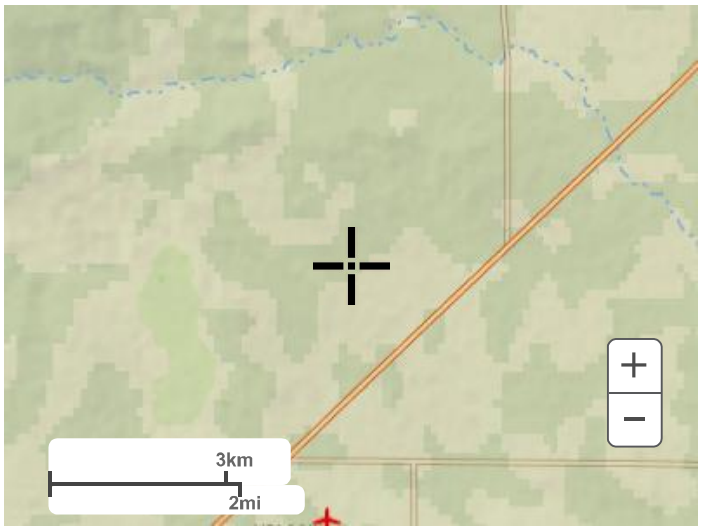
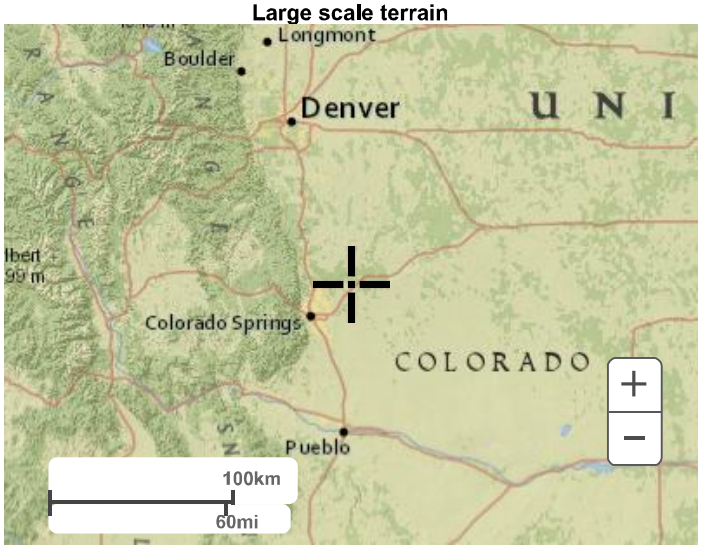
Maps & aerials

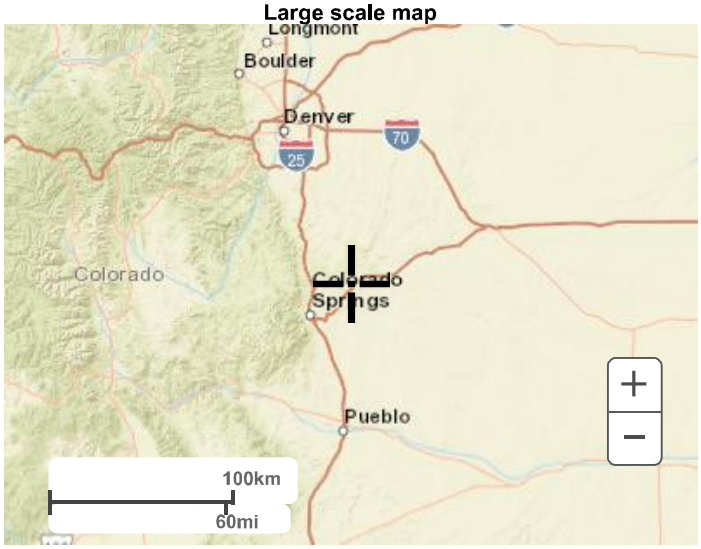
Small scale terrain



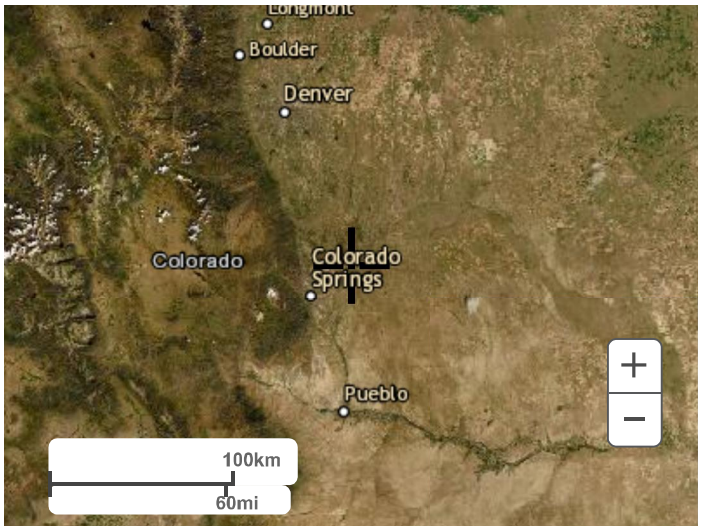
Large scale terrain



Large scale map



Large scale aerial



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US Department of Commerce
National Oceanic and Atmospheric Administration
National Weather Service
National Water Center
1325 East West Highway
Silver Spring, MD 20910
Questions?: HDSC.Questions@noaa.gov

Disclaimer

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

APPENDIX C

Hydrologic Computations

COMPOSITE % IMPERVIOUS CALCULATIONS: EXISTING & INTERIM

Subdivision: Grandview Reserve
Location: CO, El Paso County

Project Name: Grandview Subdivision PDR - Interim Conditions
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 12/21/23

1	2	3	4	5	6	7	8	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27
Basin ID	**Total Area (ac)**	**Paved/Gravel Roads**			**Lawns/Undeveloped**			**Residential - 1/8 Acre**			**Residential - 1/4 Acre**			**Residential - 1/3 Acre**			**Residential - 1/2 Acre**			**Residential - 1 Acre**			**Basins Total Weighted % Imp.**
		% Imp.	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	**% Imp.**	**Area (ac)**	**Weighted % Imp.**	
EXISTING																							
For Existing Western Offsite Sub-basin analysis, see Rational Calcs Included, from titled "Eastonville Road Preliminary Drainage Report", by HR Green, September 2023																							
ES-1	16.37	100	0	0	2	16.37	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
ES-2	46.05	100	0	0	2	46.05	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
ES-3	64.3	100	0	0	2	64.3	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
ES-4	2.68	100	0	0	2	2.68	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
ES-5	26.15	100	0	0	2	26.15	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
ES-6	31.26	100	0	0	2	31.26	2	65	0	0	40	0	0	30	0	0	25	0	0	20	0	0	2
INTERIM																							
For Existing Western Offsite Sub-basin analysis, see Rational Calcs Included, from titled "Eastonville Road Preliminary Drainage Report", by HR Green, September 2023																							
A-1	2.29	100	0.00	0.0	2	2.29	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
A-2	3.96	100	0.00	0.0	2	3.96	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
EA-1	2.50	100	0.00	0.0	2	2.50	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
TSB-A1	10.67	100	0.00	0.0	2	10.67	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
TSB-A2	4.56	100	0.00	0.0	2	4.56	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
TSB-A3	13.72	100	0.00	0.0	2	13.72	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
TSB-B1	14.03	100	0.00	0.0	2	14.03	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
TSB-B2	14.48	100	0.00	0.0	2	14.48	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
TSB-C1	11.26	100	0.00	0.0	2	11.26	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
TSB-C2	11.92	100	0.00	0.0	2	11.92	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
TSB-C3	15.29	100	0.00	0.0	2	15.29	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
TSB-D1	10.09	100	0.00	0.0	2	10.09	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
TSB-E1	8.21	100	0.00	0.0	2	8.21	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0
TSB-E2	13.57	100	0.00	0.0	2	13.57	2.0	65.0	0.00	0.0	40	0.00	0.0	30	0.00	0.0	25	0.00	0.0	20	0.00	0.0	2.0

Lot Type Identification:	
Lot Size (SF)	*Lot Size (Acre)*
0 - 8,167	1/8 Acre
8,168 - 12,704	1/4 Acre
12,705 - 18,149	1/3 Acre
18,150 - 32,670	1/2 Acre
32,671 - 43,560	1 Acre

NOTES:
% Impervious values are taken directly from Table 6-6 in the Colorado Springs DCM Vol. 1. CH. 6 (Referencing UDFCD 2001)

COMPOSITE RUNOFF COEFFICIENT CALCULATIONS: EXISTING & INTERIM

Subdivision: Grandview Reserve
Location: CO, El Paso County

Project Name: Grandview Subdivision PDR - Interim Conditions
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 12/21/23

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28
Basin ID	**Total Area (ac)**	**Paved/Gravel Roads**			**Lawns/Undeveloped**			**Roofs**			**Residential - 1/8 Acre**			**Residential - 1/4 Acre**			**Residential - 1/3 Acre**			**Residential - 1/2 Acre**			**Residential - 1 Acre**			**Composite C_5**	**Composite C_{100}**
		C_5	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**	**C_5**	**C_{100}**	**Area (ac)**		
EXISTING																											
For Existing Western Offsite Sub-basin analysis, see Rational Calcs Included, from titled "Eastonville Road Preliminary Drainage Report", by HR Green, September 2023																											
ES-1	16.37	0.90	0.96	0.00	0.09	0.36	16.37	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
ES-2	46.05	0.90	0.96	0.00	0.09	0.36	46.05	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
ES-3	64.30	0.90	0.96	0.00	0.09	0.36	64.30	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
ES-4	2.68	0.90	0.96	0.00	0.09	0.36	2.68	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
ES-5	26.15	0.90	0.96	0.00	0.09	0.36	26.15	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
ES-6	31.26	0.90	0.96	0.00	0.09	0.36	31.26	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
INTERIM																											
For Existing Western Offsite Sub-basin analysis, see Rational Calcs Included, from titled "Eastonville Road Preliminary Drainage Report", by HR Green, September 2023																											
A-1	2.29	0.90	0.96	0.00	0.09	0.36	2.29	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
A-2	3.96	0.90	0.96	0.00	0.09	0.36	3.96	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
EA-1	2.50	0.90	0.96	0.00	0.09	0.36	2.50	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
TSB-A1	10.67	0.90	0.96	0.00	0.09	0.36	10.67	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
TSB-A2	4.56	0.90	0.96	0.00	0.09	0.36	4.56	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
TSB-A3	13.72	0.90	0.96	0.00	0.09	0.36	13.72	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
TSB-B1	14.03	0.90	0.96	0.00	0.09	0.36	14.03	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
TSB-B2	14.48	0.90	0.96	0.00	0.09	0.36	14.48	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
TSB-C1	11.26	0.90	0.96	0.00	0.09	0.36	11.26	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
TSB-C2	11.92	0.90	0.96	0.00	0.09	0.36	11.92	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
TSB-C3	15.29	0.90	0.96	0.00	0.09	0.36	15.29	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
TSB-D1	10.09	0.90	0.96	0.00	0.09	0.36	10.09	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
TSB-E1	8.21	0.90	0.96	0.00	0.09	0.36	8.21	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36
TSB-E2	13.57	0.90	0.96	0.00	0.09	0.36	13.57	0.73	0.81	0.00	0.45	0.59	0.00	0.30	0.50	0.00	0.25	0.47	0.00	0.22	0.46	0.00	0.20	0.44	0.00	0.09	0.36

Lot Type Identification:	
Lot Size (SF)	*Lot Size (Acre)*
0 - 8,167	</= 1/8 Acre
8,168 - 12,704	1/4 Acre
12,705 - 18,149	1/3 Acre
18,150 - 32,670	1/2 Acre
32,671 - 43,560	1 Acre

NOTES:
C values are taken directly from Table 6-6 in the Colorado Springs DCM Vol. 1. CH. 6 (Referencing UDFCD 2001)
Coeffficients use HSG A&B soils - Refer to "Appendix A: Exhibits and Figures" for soil map

STANDARD FORM SF-2: EXISTING & INTERIM
TIME OF CONCENTRATION

Subdivision: Grandview Reserve
Location: CO, El Paso County

Project Name: Grandview Subdivision PDR - Interim Conditions
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 12/21/23

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18
SUB-BASIN DATA						INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)					Tc CHECK (T_c)			FINAL
BASIN ID	D.A. (AC)	Hydrologic Soils Group	Impervious (%)	C_5	C_{100}	L (FT)	S (%)	T_i (MIN)	L (FT)	S (%)	Cv	VEL. (FPS)	T_t (MIN)	COMP. T_c (MIN)	TOTAL LENGTH(FT)	Calculated T_c (MIN)	T_c (MIN)
EXISTING																	
For Existing Western Offsite Sub-basin analysis, see Rational Calcs Included, from titled "Eastonville Road Preliminary Drainage Report", by HR Green, September 2023																	
ES-1	16.37	A	2.0	0.09	0.36	300	3.3	21.6	1433	2.5	15	2.4	10.0	31.6	1732.7	19.6	31.6
ES-2	46.05	A	2.0	0.09	0.36	300	2.5	23.6	3127	2.0	15	2.1	24.7	48.3	3427.0	29.0	48.3
ES-3	64.30	A	2.0	0.09	0.36	300	3.2	21.7	3964	2.1	15	2.2	30.4	52.1	4263.6	33.7	52.1
ES-4	2.68	A	2.0	0.09	0.36	300	2.5	23.8	462	2.4	15	2.3	3.3	27.1	762.3	14.2	27.1
ES-5	26.15	A	2.0	0.09	0.36	300	3.1	22.1	2121	2.3	15	2.3	15.6	37.7	2420.8	23.4	37.7
ES-6	31.26	A	2.0	0.09	0.36	300	2.6	20.9	1488	2.1	15	2.2	11.4	32.3	1788.5	19.9	32.3
INTERIM																	
For Existing Western Offsite Sub-basin analysis, see Rational Calcs Included, from titled "Eastonville Road Preliminary Drainage Report", by HR Green, September 2023																	
A-1	2.29	A	2.0	0.09	0.36	40	2.0	9.3	927	2.9	10	1.7	9.1	18.4	967.0	15.4	15.4
A-2	3.96	A	2.0	0.09	0.36	56	2.0	11.0	828	2.5	10	1.6	8.7	19.7	884.0	14.9	14.9
EA-1	2.50	A	2.0	0.09	0.36	160	5.0	13.7	1254	0.5	10	0.7	29.6	43.2	1414.0	17.9	17.9
TSB-A1	10.67	A	2.0	0.09	0.36	136	2.0	17.1	865	3.0	10	1.7	8.4	25.5	1001.0	15.6	15.6
TSB-A2	4.56	A	2.0	0.09	0.36	163	2.0	18.7	749	3.8	10	1.9	6.4	25.1	912.0	15.1	15.1
TSB-A3	13.72	A	2.0	0.09	0.36	159	2.0	18.5	1220	2.3	10	1.5	13.4	31.9	1379.0	17.7	17.7
TSB-B1	14.03	A	2.0	0.09	0.36	212	2.0	21.4	1035	3.2	10	1.8	9.6	31.0	1247.0	16.9	16.9
TSB-B2	14.48	A	2.0	0.09	0.36	60	2.0	11.4	1245	2.8	10	1.7	12.4	23.7	1305.0	17.3	17.3
TSB-C1	11.26	A	2.0	0.09	0.36	300	2.0	25.4	1105	2.0	10	1.4	12.9	38.3	1405.0	17.8	17.8
TSB-C2	11.92	A	2.0	0.09	0.36	50	2.0	10.4	1151	3.1	10	1.8	10.9	21.3	1201.0	16.7	16.7
TSB-C3	15.29	A	2.0	0.09	0.36	181	2.0	19.7	1745	2.6	10	1.6	18.2	38.0	1926.0	20.7	20.7
TSB-D1	10.09	A	2.0	0.09	0.36	155	2.0	18.3	1450	2.0	10	1.4	17.1	35.4	1605.0	18.9	18.9
TSB-E1	8.21	A	2.0	0.09	0.36	150	2.0	18.0	842	4.1	10	2.0	6.9	24.9	992.0	15.5	15.5
TSB-E2	13.57	A	2.0	0.09	0.36	300	2.0	25.4	989	2.0	10	1.4	11.7	37.1	1289.0	17.2	17.2

NOTES:

$T_i = (0.395*(1.1 - C_5)*(L)^{0.5})/((S)^{0.33})$, S in ft/ft

T_t=L/60V (Velocity From Fig. 501)

Velocity V=Cv*S^0.5, S in ft/ft

T_c Check = 10+L/180

For Urbanized basins a minimum T_c of 5.0 minutes is required.

For non-urbanized basins a minimum T_c of 10.0 minutes is required

STANDARD FORM SF-3: EXISTING & INTERIM
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Grandview Reserve
Location: CO, El Paso County
Design Storm: 5-Year

Project Name: Grandview Subdivision PDR - Interim Conditions
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 12/21/23

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	
EXISTING																					
	1	**EX1**	**321.53**					**28.3**				**28.3**									****SEE NOTE**
	2	**EX2**	**18.88**					**1.7**				**1.7**									****SEE NOTE**
	3	**EX3**	**131.26**					**6.1**				**6.1**									****SEE NOTE**
	4	**EX4**	**832.70**					**22.4**				**22.4**									****SEE NOTE**
	5	**EX5**	**22.35**					**7.0**				**7.0**									****SEE NOTE**
	6	**EX6**	**3.05**					**1.2**				**1.2**									****SEE NOTE**
	7	**EX7**	**1.47**					**0.9**				**0.9**									****SEE NOTE**
	X1	ES-1	16.37	0.09	31.6	1.47	2.35	3.5				4.7									Sheet flow to Channel A Total Flow from DP 6 & Basin ES-1
	X2	ES-2	46.05	0.09	48.3	4.14	1.82	7.5				36.9									Sheet flow to Channel A Total Flow from DP 4, DP 5 & Basin ES-2
	X3	ES-3	64.30	0.09	52.1	5.79	1.73	10.0				10.0									Sheet flow offiste - outfalls to Channel B
	X4	ES-4	2.68	0.09	27.1	0.24	2.57	0.6				0.6									Sheet flow offiste - outfalls to Channel B
	X5	ES-5	26.15	0.09	37.7	2.35	2.12	5.0				5.0									Sheet flow offiste - outfalls to Channel B
	X6	ES-6	31.26	0.09	32.3	2.81	2.32	6.5				40.9									Sheet flow offiste - outfalls to Channel B Total Flow from DP 1, DP 3 & ES-6
	X7											56.5									Total Existing Flow offsite - outfalls to Channel B
****For Existing Western Offsite Sub-basin analysis, see Rational Calcs Included, from titled "Eastonville Road Preliminary Drainage Report", by HR Green, September 2023**																					
INTERIM																					
	8	A-1	2.29	0.09	15.4	0.21	3.42	0.7				0.7									Flows offsite through Pr. Swale A-1
	9	TSB-A1	10.67	0.09	15.6	0.96	3.40	3.3				3.3									Residential Undeveloped-Overland Graded
	10	A-2	3.96	0.09	14.9	0.36	3.47	1.2				10.6									Flows offsite through Pr. Swale A-2 Combined flow of Basin A-2, DP 3 & DP 9
	11	TSB-A2	4.56	0.09	15.1	0.41	3.46	1.4				1.4									Residential Undeveloped-Overland Graded
	12	TSB-A3	13.72	0.09	17.7	1.23	3.21	3.9				5.4									Residential Undeveloped-Overland Graded Combined flow of Basin TSB-A3 & DP 11
	13	TSB-B1	14.03	0.09	16.9	1.26	3.27	4.1				4.1									Residential Undeveloped-Overland Graded
	14	TSB-B2	14.48	0.09	17.3	1.30	3.24	4.2				8.3									Residential Undeveloped-Overland Graded Combined flow of Basin TSB-B2 & DP13
	15	TSB-C1	11.26	0.09	17.8	1.01	3.19	3.2				3.2									Residential Undeveloped-Overland Graded
	16	TSB-C2	11.92	0.09	16.7	1.07	3.30	3.5				3.5									Residential Undeveloped-Overland Graded
	17	TSB-C3	15.29	0.09	20.7	1.38	2.96	4.1				7.3									Residential Undeveloped-Overland Graded Combined flow of Basin TSB-C3 & DP 15
	18	TSB-D1	10.09	0.09	18.9	0.91	3.10	2.8				2.8									Residential Undeveloped-Overland Graded
	19	TSB-E1	8.21	0.09	15.5	0.74	3.41	2.5				2.5									Residential Undeveloped-Overland Graded
	20	TSB-E2	13.57	0.09	17.2	1.22	3.25	4.0				6.5									Residential Undeveloped-Overland Graded Combined flow of Basin TSB-E2 & DP 19
	21	EA-1	2.50	0.09	17.9	0.23	3.19	0.7				8.9									Existing Eastonville Road Combined flow of Basin EA-1, DP 5 & DP 6

STANDARD FORM SF-3: EXISTING & INTERIM
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Grandview Reserve
Location: CO, El Paso County
Design Storm: 100-Year

Project Name: Grandview Subdivision PDR - Interim Conditions
Project No.: HRG01
Calculated By: TJE
Checked By: BAS
Date: 12/21/23

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	
EXISTING																					
	1	**EX1**	**321.53**					**365.2**				**365.2**									****SEE NOTE**
	2	**EX2**	**18.88**					**18.8**				**18.8**									****SEE NOTE**
	3	**EX3**	**131.26**					**112.1**				**112.1**									****SEE NOTE**
	4	**EX4**	**832.70**					**491.0**				**491.0**									****SEE NOTE**
	5	**EX5**	**22.35**					**43.3**				**43.3**									****SEE NOTE**
	6	**EX6**	**3.05**					**6.9**				**6.9**									****SEE NOTE**
	7	**EX7**	**1.47**					**4.2**				**4.2**									****SEE NOTE**
	X1	ES-1	16.37	0.36	31.6	5.89	4.19	24.7				31.6									Sheet flow to Channel A Total Flow from DP 6 & Basin ES-1
	X2	ES-2	46.05	0.36	48.3	16.58	3.24	53.7				588.0									Sheet flow to Channel A Total Flow from DP 4, DP 5 & Basin ES-2
	X3	ES-3	64.30	0.36	52.1	23.15	3.09	71.5				71.5									Sheet flow offiste - outfalls to Channel B
	X4	ES-4	2.68	0.36	27.1	0.96	4.57	4.4				4.4									Sheet flow offiste - outfalls to Channel B
	X5	ES-5	26.15	0.36	37.7	9.41	3.77	35.5				35.5									Sheet flow offiste - outfalls to Channel B
	X6	ES-6	31.26	0.36	32.3	11.25	4.13	46.5				523.8									Sheet flow offiste - outfalls to Channel B Total Flow from DP 1, DP 3 & ES-6
	X7											635.2									Total Existing Flow offsite - outfalls to Channel B
****For Existing Western Offsite Sub-basin analysis, see Rational Calcs Included, from titled "Eastonville Road Preliminary Drainage Report", by HR Green, September 2023**																					
INTERIM																					
	8	A-1	2.29	0.36	15.4	0.82	6.09	5.0				5.0									Flows offsite through Pr. Swale A-1
	9	TSB-A1	10.67	0.36	15.6	3.84	6.06	23.3				23.3									Residential Undeveloped-Overland Graded
	10	A-2	3.96	0.36	14.9	1.43	6.18	8.8				144.2									Flows offsite through Pr. Swale A-2 Combined flow of Basin A-2, DP 3 & DP 9
	11	TSB-A2	4.56	0.36	15.1	1.64	6.15	10.1				10.1									Residential Undeveloped-Overland Graded
	12	TSB-A3	13.72	0.36	17.7	4.94	5.71	28.2				38.3									Residential Undeveloped-Overland Graded Combined flow of Basin TSB-A3 & DP 11
	13	TSB-B1	14.03	0.36	16.9	5.05	5.82	29.4				29.4									Residential Undeveloped-Overland Graded
	14	TSB-B2	14.48	0.36	17.3	5.21	5.77	30.1				59.5									Residential Undeveloped-Overland Graded Combined flow of Basin TSB-B2 & DP13
	15	TSB-C1	11.26	0.36	17.8	4.05	5.68	23.0				23.0									Residential Undeveloped-Overland Graded
	16	TSB-C2	11.92	0.36	16.7	4.29	5.87	25.2				25.2									Residential Undeveloped-Overland Graded
	17	TSB-C3	15.29	0.36	20.7	5.50	5.27	29.0				52.0									Residential Undeveloped-Overland Graded Combined flow of Basin TSB-C3 & DP 15
	18	TSB-D1	10.09	0.36	18.9	3.63	5.52	20.0				20.0									Residential Undeveloped-Overland Graded
	19	TSB-E1	8.21	0.36	15.5	2.96	6.07	18.0				18.0									Residential Undeveloped-Overland Graded
	20	TSB-E2	13.57	0.36	17.2	4.89	5.79	28.3				46.3									Residential Undeveloped-Overland Graded Combined flow of Basin TSB-E2 & DP 19
	21	EA-1	2.50	0.36	17.9	0.90	5.68	5.1				55.3									Existing Eastonville Road Combined flow of Basin EA-1, DP 5 & DP 6

COMPOSITE % IMPERVIOUS CALCULATIONS

Subdivision: Grandview Reserve Filing No. 1
Location: CO, Falcon (El Paso County)

Project Name: Grandview Reserve Filing No. 1
Project No.: HRG02.20
Calculated By: TJE
Checked By: BAS
Date: 3/14/24

Basin ID	Total Area (ac)	Paved Roads			Lawns / Undeveloped			Residential - 1/8 Acre			Basins Total Weighted % Imp.
		% Imp.	Area (ac)	Weighted % Imp.	% Imp.	Area (ac)	Weighted % Imp.	% Imp.	Area (ac)	Weighted % Imp.	
D-1	2.73	100		0.0	2	0.80	0.6	65	1.93	46.0	46.6
D-2	0.57	100		0.0	2		0.0	65	0.57	65.0	65.0
D-3	4.33	100		0.0	2	0.36	0.2	65	3.97	59.6	59.8
D-4	3.65	100	0.11	3.0	2	0.48	0.3	65	3.06	54.5	57.8
D-5	1.59	100		0.0	2	1.07	1.3	65	0.52	21.3	22.6
D-6	0.92	100		0.0	2	0.75	1.6	65	0.17	12.0	13.6
E-1	4.47	100		0.0	2	1.26	0.6	65	3.21	46.7	47.3
E-2	1.94	100		0.0	2		0.0	65	1.94	65.0	65.0
E-3a	2.90	100		0.0	2		0.0	65	2.90	65.0	65.0
E-3b	2.12	100		0.0	2		0.0	65	2.12	65.0	65.0
E-4a	7.45	100		0.0	2	1.92	0.5	65	5.53	48.2	48.7
E-4b	1.00	100		0.0	2		0.0	65	1.00	65.0	65.0
E-5	1.43	100		0.0	2	1.18	1.7	65	0.25	11.4	13.1
E-6	2.40	100	0.25	10.4	2	2.00	1.7	65	0.15	4.1	16.2

Galloway

STANDARD FORM SF-2
TIME OF CONCENTRATION

Subdivision: Grandview Reserve Filing No. 1
Location: CO, Falcon (El Paso County)

Project Name: Grandview Reserve Filing No. 1
Project No.: HRG02.20
Calculated By: TJE
Checked By: BAS
Date: 3/14/24

SUB-BASIN DATA						INITIAL/OVERLAND (T_i)			TRAVEL TIME (T_t)					Tc CHECK (URBANIZED BASINS)			FINAL
BASIN ID	D.A. (AC)	Hydrologic Soils Group	Impervious (%)	C_{100}	C_5	L (FT)	S (%)	T_i (MIN)	L (FT)	S (%)	Cv	VEL. (FPS)	T_t (MIN)	COMP. T_c (MIN)	TOTAL LENGTH (FT)	Urbanized T_c (MIN)	T_c (MIN)
D-1	2.73	A	46.6	0.47	0.32	64	4.8	6.8	425	2.2	20.0	3.0	2.4	9.2	489.0	12.7	9.2
D-2	0.57	A	65.0	0.62	0.50	18	2.0	3.7	313	1.0	20.0	2.0	2.6	6.3	331.0	11.8	6.3
D-3	4.33	A	59.8	0.58	0.45	25	2.0	4.7	522	1.5	20.0	2.4	3.6	8.3	547.0	13.0	8.3
D-4	3.65	A	57.8	0.56	0.43	70	4.0	6.5	679	1.5	20.0	2.4	4.6	11.1	749.0	14.2	11.1
D-5	1.59	A	22.6	0.29	0.13	72	25.0	5.2	238	0.5	20.0	1.4	2.8	8.0	310.0	11.7	8.0
D-6	0.92	A	13.6	0.22	0.07	40	33.3	3.7				0.0	0.0	3.7	40.0	10.2	5.0
E-1	4.47	A	47.3	0.48	0.33	55	3.0	7.3	804	3.0	20.0	3.5	3.9	11.1	859.0	14.8	11.1
E-2	1.94	A	65.0	0.62	0.50	31	2.0	4.9	346	3.0	20.0	3.5	1.7	6.5	377.0	12.1	6.5
E-3a	2.90	A	65.0	0.62	0.50	55	4.0	5.1	644	1.5	20.0	2.4	4.4	9.5	699.0	13.9	9.5
E-3b	2.12	A	65.0	0.62	0.50	55	4.0	5.1	248	1.0	20.0	2.0	2.1	7.2	303.0	11.7	7.2
E-4a	7.45	A	48.7	0.49	0.34	55	4.0	6.5	813	1.5	20.0	2.4	5.5	12.0	868.0	14.8	12.0
E-4b	1.00	A	65.0	0.62	0.50	55	4.0	5.1	248	1.0	20.0	2.0	2.1	7.2	303.0	11.7	7.2
E-5	1.43	A	13.1	0.21	0.06	75	15.0	6.7	318	0.5	20.0	1.4	3.7	10.4	393.0	12.2	10.4
E-6	2.40	A	16.2	0.24	0.08	50	33.3	4.1				0.0	0.0	4.1	50.0	10.3	5.0

NOTES:

$T_i = (0.395*(1.1 - C_5)*(L)^{0.5})/((S)^{0.33})$, S in ft/ft

T_t=L/60V (Velocity From Fig. 501)

Velocity V=Cv*S^0.5, S in ft/ft

Tc Check = 10+L/180

For Urbanized basins a minimum T_c of 5.0 minutes is required.

For non-urbanized basins a minimum T_c of 10.0 minutes is required

Galloway

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Grandview Reserve Filing No. 1
Location: CO, Falcon (El Paso County)
Design Storm: 5-Year

Project Name: Grandview Reserve Filing No. 1
Project No.: HRG02.20
Calculated By: TJE
Checked By: BAS
Date: 3/14/24

		DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
	D1	D-1	2.73	0.32	9.2	0.87	3.03	2.6							2.6						CDOT TYPE 'R' INLET (SUMP)
	D2	D-2	0.57	0.50	6.3	0.29	3.46	1.0							1.0						CDOT TYPE 'R' INLET (SUMP)
	D3														3.6						DP D1 + D2
	D4	D-3	4.33	0.45	8.3	1.95	3.15	6.1							6.1						CDOT TYPE 'R' INLET (SUMP)
	D5														9.8						DP D3 + D4
	D6	D-4	3.65	0.43	11.1	1.57	2.80	4.4							14.2						CDOT TYPE 'R' INLET (SUMP) -> BASIN D-4 + DP D5
	D7	D-5	1.59	0.13	8.0	0.21	3.19	0.7							14.8						TOTAL FLOW ENTERING POND D
	D8														0.2						DISCHARGE FROM POND D (MHFD - DETENTION)
		D-6	0.92	0.07	5.0	0.06	3.70	0.2													FLOWS OFF SITE TO CHANNEL B
	E1	E-1	4.47	0.33	11.1	1.48	2.79	4.1					3.0	0.0	4.1						CDOT TYPE 'R' INLET (AT-GRADE) Qcap=4.1 cfs, Qco=0 cfs to DP E4
	E2	E-2	1.94	0.50	6.5	0.97	3.42	3.3					3.0	0.0	3.3						CDOT TYPE 'R' INLET (AT-GRADE) Qcap=3.3 cfs, Qco=0 cfs to DP E4
	E3														7.4						DP E1 + E2
	E4	E-3a	2.9	0.50	9.5	1.45	2.98	4.3					1.5	0.0	4.3						CDOT TYPE 'R' INLET (AT-GRADE) Qcap=4.3 cfs, Qco=0 cfs to DP E7
	E5	E-4a	7.45	0.34	12.0	2.53	2.70	6.8					1.5	0.2	6.6						CDOT TYPE 'R' INLET (AT-GRADE) Qcap=6.6 cfs, Qco=0.2 cfs to DP E9
	E6														18.3						DP E3 + E4 + E5
	E7	E-3b	2.12	0.50	7.2	1.06	3.31	3.5							3.5						CDOT TYPE 'R' INLET (SUMP)
	E8														21.8						DP E6 + E7
	E9	E-4b	1.00	0.50	7.2	0.50	3.31	1.7	12.0	0.59	2.70	1.7			1.7						CDOT TYPE 'R' INLET (SUMP) -> BASIN E-4b + DP E8
	E10	E-5	1.43	0.06	10.4	0.09	2.87	0.3							2.0						TOTAL FLOW ENTERING POND E
	E11														0.4						DISCHARGE FROM POND E (MHFD - DETENTION)
		E-6	2.40	0.08	5.0	0.19	3.70	0.7													FLOWS OFF SITE TO CHANNEL B

Galloway

STANDARD FORM SF-3
STORM DRAINAGE SYSTEM DESIGN
(RATIONAL METHOD PROCEDURE)

Subdivision: Grandview Reserve Filing No. 1
Location: CO, Falcon (El Paso County)
Design Storm: 100-Year

Project Name: Grandview Reserve Filing No. 1
Project No.: HRG02.20
Calculated By: TJE
Checked By: BAS
Date: 3/14/24

STREET	Design Point	DIRECT RUNOFF							TOTAL RUNOFF				STREET		PIPE			TRAVEL TIME			REMARKS
		Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	I (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	
	D1	D-1	2.73	0.47	9.2	1.28	6.23	8.0							5.3						FLOWS OVERTOP CROWN -> Q=(8.0+2.5)/2=5.3 CFS CDOT TYPE 'R' INLET (SUMP)
	D2	D-2	0.57	0.62	6.3	0.35	7.11	2.5							5.3						FLOWS OVERTOP CROWN -> Q=(8.0+2.5)/2=5.3 CFS CDOT TYPE 'R' INLET (SUMP)
	D3														10.5						DP D1 + D2
	D4	D-3	4.33	0.58	8.3	2.51	6.48	16.3							14.1						FLOWS OVERTOP CROWN -> Q=(16.3+11.8)/2=14.1 CFS CDOT TYPE 'R' INLET (SUMP)
	D5														24.6						DP D3 + D4
	D6	D-4	3.65	0.56	11.1	2.04	5.76	11.8							25.8						FLOWS OVERTOP CROWN -> Q=(16.3+11.8)/2=14.1 CFS CDOT TYPE 'R' INLET (SUMP) -> BASIN D-4 + DP D5
	D7	D-5	1.59	0.29	8.0	0.46	6.57	3.0							28.8						TOTAL FLOW ENTERING POND D
	D8														8.0						DISCHARGE FROM POND D (MHFD - DETENTION)
		D-6	0.92	0.22	5.0	0.20	7.62	1.5													FLOWS OFF SITE TO CHANNEL B
	E1	E-1	4.47	0.48	11.1	2.15	5.75	12.4					3.0	2.5	9.9						CDOT TYPE 'R' INLET (AT-GRADE) Qcap=9.9 cfs, Qco=2.5 cfs to DP E4
	E2	E-2	1.94	0.62	6.5	1.20	7.04	8.4					3.0	0.6	7.8						CDOT TYPE 'R' INLET (AT-GRADE) Qcap=7.8 cfs, Qco=0.6 cfs to DP E4
	E3														17.7						DP E1 + E2
	E4	E-3a	2.9	0.62	9.5	1.80	6.13	11.0	11.1	2.32	5.75	13.3	1.5	5.1	11.7						CDOT TYPE 'R' INLET (AT-GRADE)-> Q=(13.3+20.3)/2=16.8 CFS Qcap=11.7 cfs, Qco=5.1 cfs to DP E7
	E5	E-4a	7.45	0.49	12.0	3.65	5.55	20.3				16.8	1.5	5.1	11.7						CDOT TYPE 'R' INLET (AT-GRADE)-> Q=(13.3+20.3)/2=16.8 CFS Qcap=11.7 cfs, Qco=5.1 cfs to DP E9
	E6														41.1						DP E3 + E4 + E5
	E7	E-3b	2.12	0.62	7.2	1.31	6.81	8.9	9.5	2.20	6.13	13.5			11.0						FLOWS OVERTOP CROWN -> Q=(13.5+8.5)/2=11.0 CFS CDOT TYPE 'R' INLET (SUMP)
	E8														52.1						DP E6 + E7
	E9	E-4b	1	0.62	7.2	0.62	6.81	4.2	12.0	1.54	5.55	8.5			63.1						FLOWS OVERTOP CROWN -> Q=(13.5+8.5)/2=11.0 CFS CDOT TYPE 'R' INLET (SUMP) -> BASIN E-4b + DP E8
	E10	E-5	1.43	0.21	10.4	0.30	5.91	1.8							64.9						TOTAL FLOW ENTERING POND E
	E11														14.9						DISCHARGE FROM POND E (MHFD - DETENTION)
		E-6	2.40	0.24	5.0	0.58	7.62	4.4													FLOWS OFF SITE TO CHANNEL B

Galloway

APPENDIX D

Hydraulic Computations

MHFD-Inlet, Version 5.03 (August 2023)

INLET MANAGEMENT

Worksheet Protected

INLET NAME	Inlet D1 (Basin D-1)	Inlet D2 (Basin D-2)	Inlet D4 (Basin D-3)	Inlet D6 (Basin D-4)
Site Type (Urban or Rural)	URBAN	URBAN	URBAN	URBAN
Inlet Application (Street or Area)	STREET	STREET	STREET	STREET
Hydraulic Condition	In Sump	In Sump	In Sump	In Sump
Inlet Type	CDOT Type R Curb Opening	CDOT Type R Curb Opening	CDOT Type R Curb Opening	CDOT Type R Curb Opening

USER-DEFINED INPUT

User-Defined Design Flows

Minor Q_{known} (cfs)	2.6	1.0	6.1	4.4
Major Q_{known} (cfs)	5.3	5.3	14.1	14.1

Bypass (Carry-Over) Flow from Upstream Inlets must be organized from upstream (left) to downstream (right) in order for bypass flows to be linked.

Receive Bypass Flow from:	No Bypass Flow Received	No Bypass Flow Received	No Bypass Flow Received	No Bypass Flow Received
Minor Bypass Flow Received, Q_b (cfs)	0.0	0.0	0.0	0.0
Major Bypass Flow Received, Q_b (cfs)	0.0	0.0	0.0	0.0

Watershed Characteristics

Subcatchment Area (acres)				
Percent Impervious				
NRCS Soil Type				

Watershed Profile

Overland Slope (ft/ft)				
Overland Length (ft)				
Channel Slope (ft/ft)				
Channel Length (ft)				

Minor Storm Rainfall Input

Design Storm Return Period, T_r (years)				
One-Hour Precipitation, P_1 (inches)				

Major Storm Rainfall Input

Design Storm Return Period, T_r (years)				
One-Hour Precipitation, P_1 (inches)				

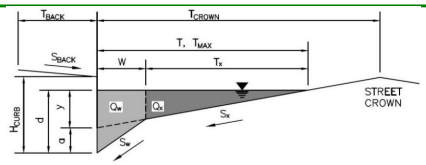
CALCULATED OUTPUT

Minor Total Design Peak Flow, Q (cfs)	**2.6**	**1.0**	**6.1**	**4.4**
Major Total Design Peak Flow, Q (cfs)	**5.3**	**5.3**	**14.1**	**14.1**
Minor Flow Bypassed Downstream, Q_b (cfs)	N/A	N/A	N/A	N/A
Major Flow Bypassed Downstream, Q_b (cfs)	N/A	N/A	N/A	N/A

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Grandview Reserve Filing No. 1**
Inlet ID: **Inlet D1 (Basin D-1)**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	16.0	ft
Gutter Width	W =	0.83	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	16.0	16.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.4	7.7	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

Maximum Capacity for 1/2 Street based On Allowable Spread

		Minor Storm	Major Storm	
Water Depth without Gutter Depression (T * S_x * 12)	y =	3.84	3.84	inches
Vertical Depth between Gutter Lip and Gutter Flowline (W * S_w * 12)	d_C =	0.8	0.8	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	0.63	0.63	inches
Water Depth at Gutter Flowline (y + a)	d =	4.47	4.47	inches
Allowable Spread for Discharge outside the Gutter Section (T - W)	T_X =	15.2	15.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.149	0.149	
Discharge outside the Gutter Section, carried in Section T_X	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_T - Q_X - Q_{BACK})	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**SUMP**	**SUMP**	**cfs**
Flow Velocity within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	0.0	0.0	

Maximum Capacity for 1/2 Street based on Allowable Depth

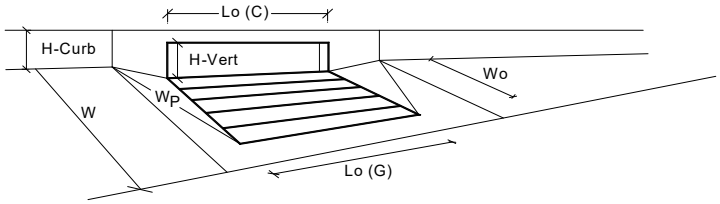
		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	15.7	29.5	ft
Theoretical Spread for Discharge outside the Gutter Section (T - W)	$T_{X\,TH}$ =	14.9	28.6	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.152	0.079	
Theoretical Discharge outside the Gutter Section, carried in Section $T_{X\,TH}$	$Q_{X\,TH}$ =	0.0	0.0	cfs
Actual Discharge outside the Gutter Section, (limited by distance T_{CROWN})	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_d - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	SUMP	SUMP	cfs
Average Flow Velocity Within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	0.0	0.0	
Slope-Based Safety Factor for Minor/Major Storm depth reduction, d ≥ 6"	R =	SUMP	SUMP	
Max Flow based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**SUMP**	**SUMP**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =			inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =			inches

MINOR STORM Allowable Capacity is not applicable to Sump Condition
MAJOR STORM Allowable Capacity is not applicable to Sump Condition

	Minor Storm	Major Storm	
Q_{allow} =	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

MHFD-Inlet, Version 5.03 (August 2023)

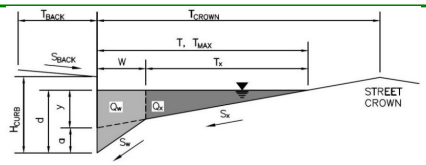


Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	4.4	7.7	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Open Area Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	0.83	0.83	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as an Orifice (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	**Q_{Grate} =**	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.00	1.00	
Clogging Factor for Multiple Units	Clog =	0.10	0.10	
Curb Capacity as a Weir (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	3.8	10.1	cfs
Interception with Clogging	Q_{wa} =	3.4	9.1	cfs
Curb Capacity as an Orifice (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	8.4	11.0	cfs
Interception with Clogging	Q_{oa} =	7.6	9.9	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	5.3	9.8	cfs
Interception with Clogging	Q_{ma} =	4.7	8.8	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	**Q_{Curb} =**	**3.4**	**8.8**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	5.00	5.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	15.7	29.5	**ft. >T-Crown**
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.0	3.2	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.30	0.57	ft
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	1.00	1.00	
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**3.4**	**8.8**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q Peak)	$Q_{PEAK\ REQUIRED}$ =	2.6	5.3	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Grandview Reserve Filing No. 1**
Inlet ID: **Inlet D2 (Basin D-2)**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	16.0	ft
Gutter Width	W =	0.83	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	16.0	16.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.4	7.7	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

Maximum Capacity for 1/2 Street based On Allowable Spread

		Minor Storm	Major Storm	
Water Depth without Gutter Depression (T * S_x * 12)	y =	3.84	3.84	inches
Vertical Depth between Gutter Lip and Gutter Flowline (W * S_w * 12)	d_C =	0.8	0.8	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	0.63	0.63	inches
Water Depth at Gutter Flowline (y + a)	d =	4.47	4.47	inches
Allowable Spread for Discharge outside the Gutter Section (T - W)	T_X =	15.2	15.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.149	0.149	
Discharge outside the Gutter Section, carried in Section T_X	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_T - Q_X - Q_{BACK})	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**SUMP**	**SUMP**	**cfs**
Flow Velocity within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	0.0	0.0	

Maximum Capacity for 1/2 Street based on Allowable Depth

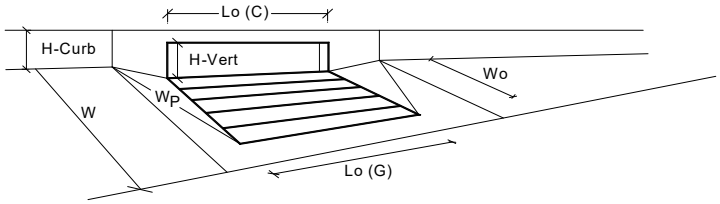
		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	15.7	29.5	ft
Theoretical Spread for Discharge outside the Gutter Section (T - W)	$T_{X\,TH}$ =	14.9	28.6	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.152	0.079	
Theoretical Discharge outside the Gutter Section, carried in Section $T_{X\,TH}$	$Q_{X\,TH}$ =	0.0	0.0	cfs
Actual Discharge outside the Gutter Section, (limited by distance T_{CROWN})	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_d - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	SUMP	SUMP	cfs
Average Flow Velocity Within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	0.0	0.0	
Slope-Based Safety Factor for Minor/Major Storm depth reduction, d ≥ 6"	R =	SUMP	SUMP	
Max Flow based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**SUMP**	**SUMP**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =			inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =			inches

MINOR STORM Allowable Capacity is not applicable to Sump Condition
MAJOR STORM Allowable Capacity is not applicable to Sump Condition

	Minor Storm	Major Storm	
Q_{allow} =	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

MHFD-Inlet, Version 5.03 (August 2023)

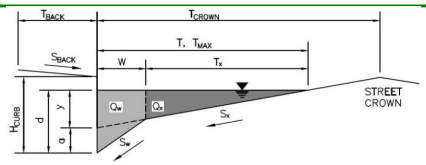


Design Information (Input)	CDOT Type R Curb Opening			
		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	4.4	7.7	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Open Area Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	5.00	5.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	0.83	0.83	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as an Orifice (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	**Q_{Grate} =**	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.00	1.00	
Clogging Factor for Multiple Units	Clog =	0.10	0.10	
Curb Capacity as a Weir (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	3.8	10.1	cfs
Interception with Clogging	Q_{wa} =	3.4	9.1	cfs
Curb Capacity as an Orifice (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	8.4	11.0	cfs
Interception with Clogging	Q_{oa} =	7.6	9.9	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	5.3	9.8	cfs
Interception with Clogging	Q_{ma} =	4.7	8.8	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	**Q_{Curb} =**	**3.4**	**8.8**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	5.00	5.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	15.7	29.5	**ft. >T-Crown**
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.0	3.2	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.30	0.57	ft
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	1.00	1.00	
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**3.4**	**8.8**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q Peak)	$Q_{PEAK\ REQUIRED}$ =	1.0	5.3	cfs

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Grandview Reserve Filing No. 1**
Inlet ID: **Inlet D4 (Basin D-3)**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	16.0	ft
Gutter Width	W =	0.83	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	16.0	16.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.4	7.7	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

Maximum Capacity for 1/2 Street based On Allowable Spread

		Minor Storm	Major Storm	
Water Depth without Gutter Depression (T * S_x * 12)	y =	3.84	3.84	inches
Vertical Depth between Gutter Lip and Gutter Flowline (W * S_w * 12)	d_C =	0.8	0.8	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	0.63	0.63	inches
Water Depth at Gutter Flowline (y + a)	d =	4.47	4.47	inches
Allowable Spread for Discharge outside the Gutter Section (T - W)	T_X =	15.2	15.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.149	0.149	
Discharge outside the Gutter Section, carried in Section T_X	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_T - Q_X - Q_{BACK})	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**SUMP**	**SUMP**	**cfs**
Flow Velocity within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	0.0	0.0	

Maximum Capacity for 1/2 Street based on Allowable Depth

		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	15.7	29.5	ft
Theoretical Spread for Discharge outside the Gutter Section (T - W)	$T_{X\,TH}$ =	14.9	28.6	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.152	0.079	
Theoretical Discharge outside the Gutter Section, carried in Section $T_{X\,TH}$	$Q_{X\,TH}$ =	0.0	0.0	cfs
Actual Discharge outside the Gutter Section, (limited by distance T_{CROWN})	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_d - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	SUMP	SUMP	cfs
Average Flow Velocity Within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	0.0	0.0	
Slope-Based Safety Factor for Minor/Major Storm depth reduction, d ≥ 6"	R =	SUMP	SUMP	
Max Flow based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**SUMP**	**SUMP**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =			inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =			inches

MINOR STORM Allowable Capacity is not applicable to Sump Condition
MAJOR STORM Allowable Capacity is not applicable to Sump Condition

	Minor Storm	Major Storm	
Q_{allow} =	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

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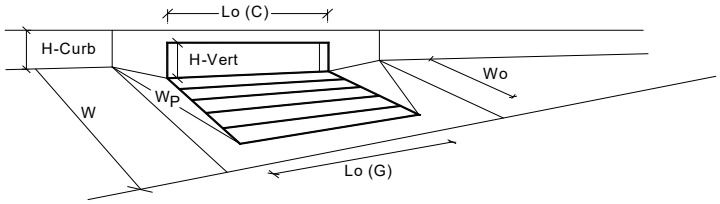

Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	4.4	7.7	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Open Area Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	0.83	0.83	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as an Orifice (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	**Q_{Grate} =**	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.31	1.31	
Clogging Factor for Multiple Units	Clog =	0.04	0.04	
Curb Capacity as a Weir (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	6.5	22.6	cfs
Interception with Clogging	Q_{wa} =	6.2	21.6	cfs
Curb Capacity as an Orifice (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	25.3	33.0	cfs
Interception with Clogging	Q_{oa} =	24.2	31.5	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	11.9	25.4	cfs
Interception with Clogging	Q_{ma} =	11.4	24.3	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	**Q_{Curb} =**	**6.2**	**21.6**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	15.00	15.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	15.7	29.5	**ft. >T-Crown**
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.0	3.2	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.30	0.57	ft
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.67	0.88	
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**6.2**	**21.6**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q Peak)	$Q_{PEAK\ REQUIRED}$ =	6.1	14.1	cfs

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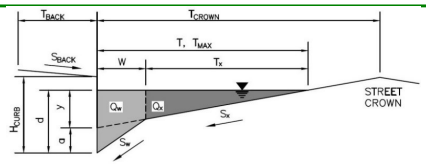
ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project: **Grandview Reserve Filing No. 1**
Inlet ID: **Inlet D6 (Basin D-4)**



Gutter Geometry:				
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5		ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020		ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020		
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00		inches
Distance from Curb Face to Street Crown	T_{CROWN} =	16.0		ft
Gutter Width	W =	0.83		ft
Street Transverse Slope	S_X =	0.020		ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083		ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000		ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016		

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	16.0	16.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.4	7.7	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

Maximum Capacity for 1/2 Street based On Allowable Spread		Minor Storm	Major Storm	
Water Depth without Gutter Depression (T * S_x * 12)	y =	3.84	3.84	inches
Vertical Depth between Gutter Lip and Gutter Flowline (W * S_w * 12)	d_C =	0.8	0.8	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	0.63	0.63	inches
Water Depth at Gutter Flowline (y + a)	d =	4.47	4.47	inches
Allowable Spread for Discharge outside the Gutter Section (T - W)	T_X =	15.2	15.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.149	0.149	
Discharge outside the Gutter Section, carried in Section T_X	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_T - Q_X - Q_{BACK})	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**SUMP**	**SUMP**	**cfs**
Flow Velocity within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	0.0	0.0	

Maximum Capacity for 1/2 Street based on Allowable Depth		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	15.7	29.5	ft
Theoretical Spread for Discharge outside the Gutter Section (T - W)	$T_{X\,TH}$ =	14.9	28.6	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.152	0.079	
Theoretical Discharge outside the Gutter Section, carried in Section $T_{X\,TH}$	$Q_{X\,TH}$ =	0.0	0.0	cfs
Actual Discharge outside the Gutter Section, (limited by distance T_{CROWN})	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_d - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	SUMP	SUMP	cfs
Average Flow Velocity Within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	0.0	0.0	
Slope-Based Safety Factor for Minor/Major Storm depth reduction, d ≥ 6"	R =	SUMP	SUMP	
Max Flow based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**SUMP**	**SUMP**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =			inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =			inches

		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is not applicable to Sump Condition				
MAJOR STORM Allowable Capacity is not applicable to Sump Condition	**Q_{allow} =**	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

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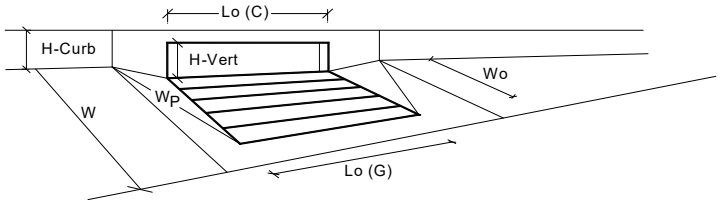

Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	4.4	7.7	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Open Area Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	0.83	0.83	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.30	0.57	ft
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.67	0.88	
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**6.2**	**21.6**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q Peak)	$Q_{PEAK\ REQUIRED}$ =	4.4	14.1	cfs

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INLET MANAGEMENT

Worksheet Protected

INLET NAME	Inlet E1 (Basin E-1)	Inlet E2 (Basin E-2)	Inlet E4 (Basin E-3a)	Inlet E5 (Basin E-4a)	Inlet E7 (Basin E-3b)	Inlet E9 (Basin E-4b)
Site Type (Urban or Rural)	URBAN	URBAN	URBAN	URBAN	URBAN	URBAN
Inlet Application (Street or Area)	STREET	STREET	STREET	STREET	STREET	STREET
Hydraulic Condition	On Grade	On Grade	On Grade	On Grade	In Sump	In Sump
Inlet Type	CDOT Type R Curb Opening	CDOT Type R Curb Opening	CDOT Type R Curb Opening	CDOT Type R Curb Opening	CDOT Type R Curb Opening	CDOT Type R Curb Opening

USER-DEFINED INPUT

User-Defined Design Flows

Minor Q_{Known} (cfs)	4.1	3.3	4.3	6.8	3.5	1.7
Major Q_{Known} (cfs)	12.4	8.4	14.5	16.8	8.9	4.2

Bypass (Carry-Over) Flow from Upstream Inlets must be organized from upstream (left) to downstream (right) in order for bypass flows to be linked.

Receive Bypass Flow from:	No Bypass Flow Received	No Bypass Flow Received	User-Defined	No Bypass Flow Received	User-Defined	User-Defined
Minor Bypass Flow Received, Q_b (cfs)	0.0	0.0	0.0	0.0	0.0	0.0
Major Bypass Flow Received, Q_b (cfs)	0.0	0.0	2.3	0.0	2.1	6.8

Watershed Characteristics

Subcatchment Area (acres)						
Percent Impervious						
NRCS Soil Type						

Watershed Profile

Overland Slope (ft/ft)						
Overland Length (ft)						
Channel Slope (ft/ft)						
Channel Length (ft)						

Minor Storm Rainfall Input

Design Storm Return Period, T_r (years)						
One-Hour Precipitation, P_1 (inches)						

Major Storm Rainfall Input

Design Storm Return Period, T_r (years)						
One-Hour Precipitation, P_1 (inches)						

CALCULATED OUTPUT

Minor Total Design Peak Flow, Q (cfs)	**4.1**	**3.3**	**4.3**	**6.8**	**3.5**	**1.7**
Major Total Design Peak Flow, Q (cfs)	**12.4**	**8.4**	**16.8**	**16.8**	**11.0**	**11.0**
Minor Flow Bypassed Downstream, Q_b (cfs)	0.0	0.0	0.0	0.2	N/A	N/A
Major Flow Bypassed Downstream, Q_b (cfs)	2.5	0.6	5.1	5.1	N/A	N/A

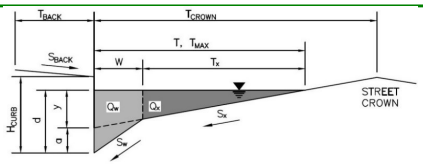
MHFD-Inlet, Version 5.03 (August 2023)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Inlet ID: **Inlet E1 (Basin E-1)**



Gutter Geometry:			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	16.0	ft
Gutter Width	W =	0.83	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.030	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	16.0	16.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.4	7.7	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

Maximum Capacity for 1/2 Street based On Allowable Spread		Minor Storm	Major Storm	
Water Depth without Gutter Depression (T * S_x * 12)	y =	3.84	3.84	inches
Vertical Depth between Gutter Lip and Gutter Flowline (W * S_w * 12)	d_C =	0.8	0.8	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	0.63	0.63	inches
Water Depth at Gutter Flowline (y + a)	d =	4.47	4.47	inches
Allowable Spread for Discharge outside the Gutter Section (T - W)	T_X =	15.2	15.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.149	0.149	
Discharge outside the Gutter Section, carried in Section T_X	Q_X =	12.6	12.6	cfs
Discharge within the Gutter Section (Q_T - Q_X - Q_{BACK})	Q_W =	2.2	2.2	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**14.8**	**14.8**	**cfs**
Flow Velocity within the Gutter Section	V =	7.9	7.9	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	2.9	2.9	

Maximum Capacity for 1/2 Street based on Allowable Depth		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	15.7	29.5	ft
Theoretical Spread for Discharge outside the Gutter Section (T - W)	$T_{X\,TH}$ =	14.9	28.6	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.152	0.079	
Theoretical Discharge outside the Gutter Section, carried in Section $T_{X\,TH}$	$Q_{X\,TH}$ =	12.0	68.6	cfs
Actual Discharge outside the Gutter Section, (limited by distance T_{CROWN})	Q_X =	12.0	59.4	cfs
Discharge within the Gutter Section (Q_d - Q_X)	Q_W =	2.1	5.8	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	1.3	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	14.1	66.6	cfs
Average Flow Velocity Within the Gutter Section	V =	7.8	11.6	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	2.8	7.4	
Slope-Based Safety Factor for Minor/Major Storm depth reduction, d ≥ 6"	R =	1.00	0.60	
Max Flow based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**14.1**	**40.2**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =	4.40	6.34	inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =	0.00	1.88	inches

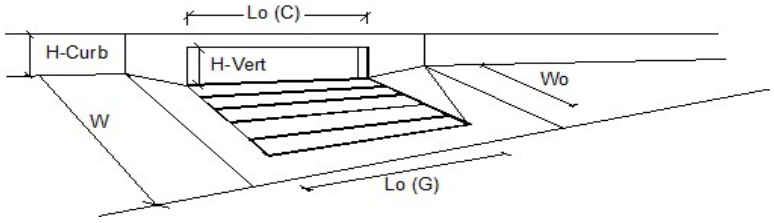
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Spread Criterion	**Q_{allow} =**	**14.1**	**14.8**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design peak flow of 4.10 cfs on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design peak flow of 12.40 cfs on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.03 (August 2023)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f (G) =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f (C) =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Design Discharge for Half of Street (from *Inlet Management*)	Q_o =	4.1	12.4	cfs
Water Spread Width	T =	9.8	15.0	ft
Water Depth at Flowline (outside of local depression)	d =	3.0	4.2	inches
Water Depth at Street Crown (or at T_{MAX})	d_{CROWN} =	0.0	0.0	inches
Ratio of Gutter Flow to Design Flow	E_o =	0.250	0.159	
Discharge outside the Gutter Section W, carried in Section T_x	Q_x =	3.1	10.4	cfs
Discharge within the Gutter Section W	Q_w =	1.0	2.0	cfs
Discharge Behind the Curb Face	Q_{BACK} =	0.0	0.0	cfs
Flow Area within the Gutter Section W	A_W =	0.18	0.26	sq ft
Velocity within the Gutter Section W	V_W =	5.8	7.5	fps
Water Depth for Design Condition	d_{LOCAL} =	6.0	7.2	inches
Grate Analysis (Calculated)		MINOR	MAJOR	
Total Length of Inlet Grate Opening	L =	N/A	N/A	ft
Ratio of Grate Flow to Design Flow	$E_{o\text{-}GRATE}$ =	N/A	N/A	
Under No-Clogging Condition		MINOR	MAJOR	
Minimum Velocity Where Grate Splash-Over Begins	V_o =	N/A	N/A	fps
Interception Rate of Frontal Flow	R_f =	N/A	N/A	
Interception Rate of Side Flow	R_x =	N/A	N/A	
Interception Capacity	Q_i =	N/A	N/A	cfs
Under Clogging Condition		MINOR	MAJOR	
Clogging Coefficient for Multiple-unit Grate Inlet	GrateCoeff =	N/A	N/A	
Clogging Factor for Multiple-unit Grate Inlet	GrateClog =	N/A	N/A	
Effective (unclogged) Length of Multiple-unit Grate Inlet	L_e =	N/A	N/A	ft
Minimum Velocity Where Grate Splash-Over Begins	V_o =	N/A	N/A	fps
Interception Rate of Frontal Flow	R_f =	N/A	N/A	
Interception Rate of Side Flow	R_x =	N/A	N/A	
Actual Interception Capacity	**Q_a =**	**N/A**	**N/A**	**cfs**
Carry-Over Flow = Q_o-Q_a (to be applied to curb opening or next d/s inlet)	**Q_b =**	**N/A**	**N/A**	**cfs**
Curb Opening or Slotted Inlet Analysis (Calculated)		MINOR	MAJOR	
Equivalent Slope S_e	S_e =	0.111	0.078	ft/ft
Required Length L_T to Have 100% Interception	L_T =	11.73	24.25	ft
Under No-Clogging Condition		MINOR	MAJOR	
Effective Length of Curb Opening or Slotted Inlet (minimum of L, L_T)	L =	11.73	15.00	ft
Interception Capacity	Q_i =	4.1	10.2	cfs
Under Clogging Condition		MINOR	MAJOR	
Clogging Coefficient	CurbCoeff =	1.31	1.31	
Clogging Factor for Multiple-unit Curb Opening or Slotted Inlet	CurbClog =	0.04	0.04	
Effective (Unclogged) Length	L_e =	11.73	14.35	ft
Actual Interception Capacity	**Q_a =**	**4.1**	**9.9**	**cfs**
Carry-Over Flow = $Q_{b(GRATE)}$-Q_a	**Q_b =**	**0.0**	**2.5**	**cfs**
Summary		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**4.1**	**9.9**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**2.5**	**cfs**
Capture Percentage = Q_a/Q_o	**C% =**	**100**	**80**	**%**

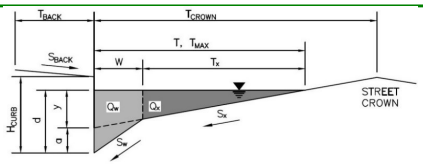
MHFD-Inlet, Version 5.03 (August 2023)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Inlet ID: **Inlet E2 (Basin E-2)**



Gutter Geometry:			
Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	16.0	ft
Gutter Width	W =	0.83	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.030	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	16.0	16.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.4	7.7	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☐	

Maximum Capacity for 1/2 Street based On Allowable Spread		Minor Storm	Major Storm	
Water Depth without Gutter Depression (T * S_x * 12)	y =	3.84	3.84	inches
Vertical Depth between Gutter Lip and Gutter Flowline (W * S_w * 12)	d_C =	0.8	0.8	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	0.63	0.63	inches
Water Depth at Gutter Flowline (y + a)	d =	4.47	4.47	inches
Allowable Spread for Discharge outside the Gutter Section (T - W)	T_X =	15.2	15.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.149	0.149	
Discharge outside the Gutter Section, carried in Section T_X	Q_X =	12.6	12.6	cfs
Discharge within the Gutter Section (Q_T - Q_X - Q_{BACK})	Q_W =	2.2	2.2	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**14.8**	**14.8**	**cfs**
Flow Velocity within the Gutter Section	V =	7.9	7.9	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	2.9	2.9	

Maximum Capacity for 1/2 Street based on Allowable Depth		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	15.7	29.5	ft
Theoretical Spread for Discharge outside the Gutter Section (T - W)	$T_{X\,TH}$ =	14.9	28.6	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.152	0.079	
Theoretical Discharge outside the Gutter Section, carried in Section $T_{X\,TH}$	$Q_{X\,TH}$ =	12.0	68.6	cfs
Actual Discharge outside the Gutter Section, (limited by distance T_{CROWN})	Q_X =	12.0	59.4	cfs
Discharge within the Gutter Section (Q_d - Q_X)	Q_W =	2.1	5.8	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	1.3	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	14.1	66.6	cfs
Average Flow Velocity Within the Gutter Section	V =	7.8	11.6	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	2.8	7.4	
Slope-Based Safety Factor for Minor/Major Storm depth reduction, d ≥ 6"	R =	1.00	0.60	
Max Flow based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**14.1**	**40.2**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =	4.40	6.34	inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =	0.00	1.88	inches

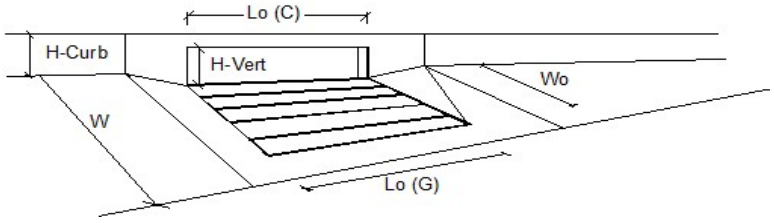
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Spread Criterion	**Q_{allow} =**	**14.1**	**14.8**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design peak flow of 3.30 cfs on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design peak flow of 8.40 cfs on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.03 (August 2023)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f (G) =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f (C) =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Design Discharge for Half of Street (from *Inlet Management*)	Q_o =	3.3	8.4	cfs
Water Spread Width	T =	9.0	12.9	ft
Water Depth at Flowline (outside of local depression)	d =	2.8	3.7	inches
Water Depth at Street Crown (or at T_{MAX})	d_{CROWN} =	0.0	0.0	inches
Ratio of Gutter Flow to Design Flow	E_o =	0.272	0.187	
Discharge outside the Gutter Section W, carried in Section T_x	Q_x =	2.4	6.8	cfs
Discharge within the Gutter Section W	Q_w =	0.9	1.6	cfs
Discharge Behind the Curb Face	Q_{BACK} =	0.0	0.0	cfs
Flow Area within the Gutter Section W	A_W =	0.16	0.23	sq ft
Velocity within the Gutter Section W	V_W =	5.5	6.8	fps
Water Depth for Design Condition	d_{LOCAL} =	5.8	6.7	inches
Grate Analysis (Calculated)		MINOR	MAJOR	
Total Length of Inlet Grate Opening	L =	N/A	N/A	ft
Ratio of Grate Flow to Design Flow	$E_{o\text{-}GRATE}$ =	N/A	N/A	
Under No-Clogging Condition		MINOR	MAJOR	
Minimum Velocity Where Grate Splash-Over Begins	V_o =	N/A	N/A	fps
Interception Rate of Frontal Flow	R_f =	N/A	N/A	
Interception Rate of Side Flow	R_x =	N/A	N/A	
Interception Capacity	Q_i =	N/A	N/A	cfs
Under Clogging Condition		MINOR	MAJOR	
Clogging Coefficient for Multiple-unit Grate Inlet	GrateCoeff =	N/A	N/A	
Clogging Factor for Multiple-unit Grate Inlet	GrateClog =	N/A	N/A	
Effective (unclogged) Length of Multiple-unit Grate Inlet	L_e =	N/A	N/A	ft
Minimum Velocity Where Grate Splash-Over Begins	V_o =	N/A	N/A	fps
Interception Rate of Frontal Flow	R_f =	N/A	N/A	
Interception Rate of Side Flow	R_x =	N/A	N/A	
Actual Interception Capacity	**Q_a =**	**N/A**	**N/A**	**cfs**
Carry-Over Flow = Q_o-Q_a (to be applied to curb opening or next d/s inlet)	**Q_b =**	**N/A**	**N/A**	**cfs**
Curb Opening or Slotted Inlet Analysis (Calculated)		MINOR	MAJOR	
Equivalent Slope S_e	S_e =	0.119	0.088	ft/ft
Required Length L_T to Have 100% Interception	L_T =	10.17	18.82	ft
Under No-Clogging Condition		MINOR	MAJOR	
Effective Length of Curb Opening or Slotted Inlet (minimum of L, L_T)	L =	10.17	15.00	ft
Interception Capacity	Q_i =	3.3	7.9	cfs
Under Clogging Condition		MINOR	MAJOR	
Clogging Coefficient	CurbCoeff =	1.31	1.31	
Clogging Factor for Multiple-unit Curb Opening or Slotted Inlet	CurbClog =	0.04	0.04	
Effective (Unclogged) Length	L_e =	10.17	14.35	ft
Actual Interception Capacity	**Q_a =**	**3.3**	**7.8**	**cfs**
Carry-Over Flow = $Q_{b(GRATE)}$-Q_a	**Q_b =**	**0.0**	**0.6**	**cfs**
Summary		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**3.3**	**7.8**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**0.6**	**cfs**
Capture Percentage = Q_a/Q_o	**C% =**	**100**	**92**	**%**

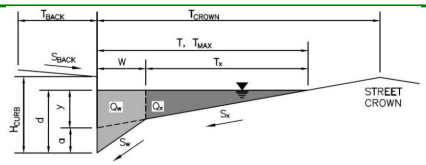
MHFD-Inlet, Version 5.03 (August 2023)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Inlet ID: **Inlet E4 (Basin E-3a)**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	16.0	ft
Gutter Width	W =	0.83	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	16.0	16.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.4	7.7	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☑	

Maximum Capacity for 1/2 Street based On Allowable Spread

		Minor Storm	Major Storm	
Water Depth without Gutter Depression (T * S_x * 12)	y =	3.84	3.84	inches
Vertical Depth between Gutter Lip and Gutter Flowline (W * S_w * 12)	d_C =	0.8	0.8	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	0.63	0.63	inches
Water Depth at Gutter Flowline (y + a)	d =	4.47	4.47	inches
Allowable Spread for Discharge outside the Gutter Section (T - W)	T_X =	15.2	15.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.149	0.149	
Discharge outside the Gutter Section, carried in Section T_X	Q_X =	8.9	8.9	cfs
Discharge within the Gutter Section (Q_T - Q_X - Q_{BACK})	Q_W =	1.6	1.6	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**10.5**	**10.5**	**cfs**
Flow Velocity within the Gutter Section	V =	5.6	5.6	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	2.1	2.1	

Maximum Capacity for 1/2 Street based on Allowable Depth

		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	15.7	29.5	ft
Theoretical Spread for Discharge outside the Gutter Section (T - W)	$T_{X\,TH}$ =	14.9	28.6	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.152	0.079	
Theoretical Discharge outside the Gutter Section, carried in Section $T_{X\,TH}$	$Q_{X\,TH}$ =	8.5	48.5	cfs
Actual Discharge outside the Gutter Section, (limited by distance T_{CROWN})	Q_X =	8.5	42.0	cfs
Discharge within the Gutter Section (Q_d - Q_X)	Q_W =	1.5	4.1	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.9	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	10.0	47.1	cfs
Average Flow Velocity Within the Gutter Section	V =	5.5	8.2	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	2.0	5.3	
Slope-Based Safety Factor for Minor/Major Storm depth reduction, d ≥ 6"	R =	1.00	1.00	
Max Flow based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**10.0**	**47.1**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =	4.40	7.70	inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =	0.00	3.23	inches

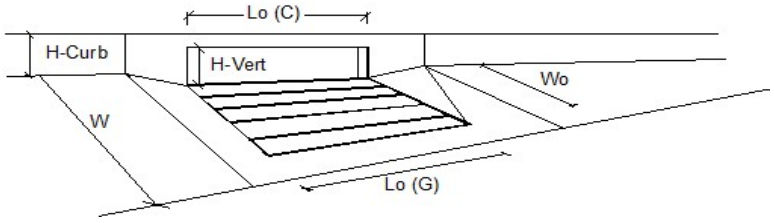
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**10.0**	**47.1**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design peak flow of 4.30 cfs on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design peak flow of 16.80 cfs on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.03 (August 2023)



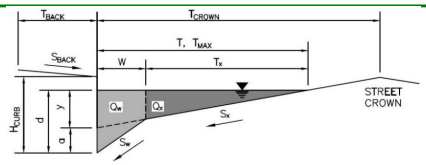
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f (G) =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f (C) =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Design Discharge for Half of Street (from *Inlet Management*)	Q_o =	4.3	16.8	cfs
Water Spread Width	T =	11.4	16.0	ft
Water Depth at Flowline (outside of local depression)	d =	3.4	5.2	inches
Water Depth at Street Crown (or at T_{MAX})	d_{CROWN} =	0.0	0.8	inches
Ratio of Gutter Flow to Design Flow	E_o =	0.212	0.124	
Discharge outside the Gutter Section W, carried in Section T_x	Q_x =	3.4	14.7	cfs
Discharge within the Gutter Section W	Q_w =	0.9	2.1	cfs
Discharge Behind the Curb Face	Q_{BACK} =	0.0	0.0	cfs
Flow Area within the Gutter Section W	A_W =	0.20	0.33	sq ft
Velocity within the Gutter Section W	V_W =	4.5	6.2	fps
Water Depth for Design Condition	d_{LOCAL} =	6.4	8.2	inches
Grate Analysis (Calculated)		MINOR	MAJOR	
Total Length of Inlet Grate Opening	L =	N/A	N/A	ft
Ratio of Grate Flow to Design Flow	$E_{o\text{-}GRATE}$ =	N/A	N/A	
Under No-Clogging Condition		MINOR	MAJOR	
Minimum Velocity Where Grate Splash-Over Begins	V_o =	N/A	N/A	fps
Interception Rate of Frontal Flow	R_f =	N/A	N/A	
Interception Rate of Side Flow	R_x =	N/A	N/A	
Interception Capacity	Q_i =	N/A	N/A	cfs
Under Clogging Condition		MINOR	MAJOR	
Clogging Coefficient for Multiple-unit Grate Inlet	GrateCoeff =	N/A	N/A	
Clogging Factor for Multiple-unit Grate Inlet	GrateClog =	N/A	N/A	
Effective (unclogged) Length of Multiple-unit Grate Inlet	L_e =	N/A	N/A	ft
Minimum Velocity Where Grate Splash-Over Begins	V_o =	N/A	N/A	fps
Interception Rate of Frontal Flow	R_f =	N/A	N/A	
Interception Rate of Side Flow	R_x =	N/A	N/A	
Actual Interception Capacity	**Q_a =**	**N/A**	**N/A**	**cfs**
Carry-Over Flow = Q_o-Q_a (to be applied to curb opening or next d/s inlet)	**Q_b =**	**N/A**	**N/A**	**cfs**
Curb Opening or Slotted Inlet Analysis (Calculated)		MINOR	MAJOR	
Equivalent Slope S_e	S_e =	0.097	0.065	ft/ft
Required Length L_T to Have 100% Interception	L_T =	12.28	29.57	ft
Under No-Clogging Condition		MINOR	MAJOR	
Effective Length of Curb Opening or Slotted Inlet (minimum of L, L_T)	L =	12.28	15.00	ft
Interception Capacity	Q_i =	4.3	12.1	cfs
Under Clogging Condition		MINOR	MAJOR	
Clogging Coefficient	CurbCoeff =	1.31	1.31	
Clogging Factor for Multiple-unit Curb Opening or Slotted Inlet	CurbClog =	0.04	0.04	
Effective (Unclogged) Length	L_e =	12.28	14.35	ft
Actual Interception Capacity	**Q_a =**	**4.3**	**11.7**	**cfs**
Carry-Over Flow = $Q_{b(GRATE)}$-Q_a	**Q_b =**	**0.0**	**5.1**	**cfs**
Summary		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**4.3**	**11.7**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.0**	**5.1**	**cfs**
Capture Percentage = Q_a/Q_o	**C% =**	**100**	**70**	**%**

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Inlet ID: **Inlet E5 (Basin E-4a)**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	16.0	ft
Gutter Width	W =	0.83	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.015	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	16.0	16.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.4	7.7	inches
Allow Flow Depth at Street Crown (check box for yes, leave blank for no)		☐	☑	

Maximum Capacity for 1/2 Street based On Allowable Spread

		Minor Storm	Major Storm	
Water Depth without Gutter Depression (T * S_x * 12)	y =	3.84	3.84	inches
Vertical Depth between Gutter Lip and Gutter Flowline (W * S_w * 12)	d_C =	0.8	0.8	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	0.63	0.63	inches
Water Depth at Gutter Flowline (y + a)	d =	4.47	4.47	inches
Allowable Spread for Discharge outside the Gutter Section (T - W)	T_X =	15.2	15.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.149	0.149	
Discharge outside the Gutter Section, carried in Section T_X	Q_X =	8.9	8.9	cfs
Discharge within the Gutter Section (Q_T - Q_X - Q_{BACK})	Q_W =	1.6	1.6	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**10.5**	**10.5**	**cfs**
Flow Velocity within the Gutter Section	V =	5.6	5.6	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	2.1	2.1	

Maximum Capacity for 1/2 Street based on Allowable Depth

		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	15.7	29.5	ft
Theoretical Spread for Discharge outside the Gutter Section (T - W)	$T_{X\,TH}$ =	14.9	28.6	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.152	0.079	
Theoretical Discharge outside the Gutter Section, carried in Section $T_{X\,TH}$	$Q_{X\,TH}$ =	8.5	48.5	cfs
Actual Discharge outside the Gutter Section, (limited by distance T_{CROWN})	Q_X =	8.5	42.0	cfs
Discharge within the Gutter Section (Q_d - Q_X)	Q_W =	1.5	4.1	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.9	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	10.0	47.1	cfs
Average Flow Velocity Within the Gutter Section	V =	5.5	8.2	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	2.0	5.3	
Slope-Based Safety Factor for Minor/Major Storm depth reduction, d ≥ 6"	R =	1.00	1.00	
Max Flow based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**10.0**	**47.1**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =	4.40	7.70	inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =	0.00	3.23	inches

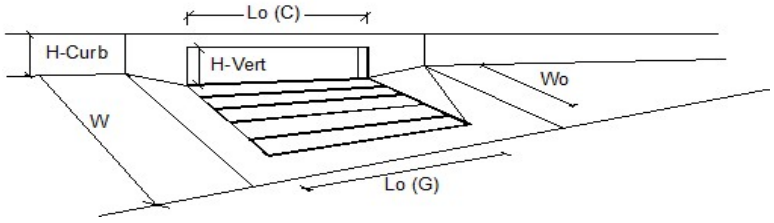
		Minor Storm	Major Storm	
MINOR STORM Allowable Capacity is based on Depth Criterion				
MAJOR STORM Allowable Capacity is based on Depth Criterion	**Q_{allow} =**	**10.0**	**47.1**	**cfs**

Minor storm max. allowable capacity GOOD - greater than the design peak flow of 6.80 cfs on sheet 'Inlet Management'

Major storm max. allowable capacity GOOD - greater than the design peak flow of 16.80 cfs on sheet 'Inlet Management'

INLET ON A CONTINUOUS GRADE

MHFD-Inlet, Version 5.03 (August 2023)



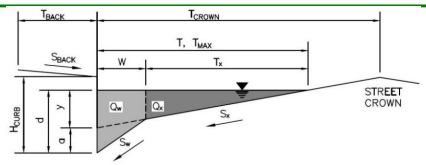
Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a')	a_{LOCAL} =	3.0	3.0	inches
Total Number of Units in the Inlet (Grate or Curb Opening)	No =	1	1	
Length of a Single Unit Inlet (Grate or Curb Opening)	L_o =	15.00	15.00	ft
Width of a Unit Grate (cannot be greater than W, Gutter Width)	W_o =	N/A	N/A	ft
Clogging Factor for a Single Unit Grate (typical min. value = 0.5)	C_f (G) =	N/A	N/A	
Clogging Factor for a Single Unit Curb Opening (typical min. value = 0.1)	C_f (C) =	0.10	0.10	
Street Hydraulics: OK - Q < Allowable Street Capacity'		MINOR	MAJOR	
Design Discharge for Half of Street (from *Inlet Management*)	Q_o =	6.8	16.8	cfs
Water Spread Width	T =	13.6	16.0	ft
Water Depth at Flowline (outside of local depression)	d =	3.9	5.2	inches
Water Depth at Street Crown (or at T_{MAX})	d_{CROWN} =	0.0	0.8	inches
Ratio of Gutter Flow to Design Flow	E_o =	0.176	0.124	
Discharge outside the Gutter Section W, carried in Section T_x	Q_x =	5.6	14.7	cfs
Discharge within the Gutter Section W	Q_w =	1.2	2.1	cfs
Discharge Behind the Curb Face	Q_{BACK} =	0.0	0.0	cfs
Flow Area within the Gutter Section W	A_W =	0.24	0.33	sq ft
Velocity within the Gutter Section W	V_W =	5.0	6.2	fps
Water Depth for Design Condition	d_{LOCAL} =	6.9	8.2	inches
Grate Analysis (Calculated)		MINOR	MAJOR	
Total Length of Inlet Grate Opening	L =	N/A	N/A	ft
Ratio of Grate Flow to Design Flow	$E_{o\text{-}GRATE}$ =	N/A	N/A	
Under No-Clogging Condition		MINOR	MAJOR	
Minimum Velocity Where Grate Splash-Over Begins	V_o =	N/A	N/A	fps
Interception Rate of Frontal Flow	R_f =	N/A	N/A	
Interception Rate of Side Flow	R_x =	N/A	N/A	
Interception Capacity	Q_i =	N/A	N/A	cfs
Under Clogging Condition		MINOR	MAJOR	
Clogging Coefficient for Multiple-unit Grate Inlet	GrateCoeff =	N/A	N/A	
Clogging Factor for Multiple-unit Grate Inlet	GrateClog =	N/A	N/A	
Effective (unclogged) Length of Multiple-unit Grate Inlet	L_e =	N/A	N/A	ft
Minimum Velocity Where Grate Splash-Over Begins	V_o =	N/A	N/A	fps
Interception Rate of Frontal Flow	R_f =	N/A	N/A	
Interception Rate of Side Flow	R_x =	N/A	N/A	
Actual Interception Capacity	**Q_a =**	**N/A**	**N/A**	**cfs**
Carry-Over Flow = Q_o-Q_a (to be applied to curb opening or next d/s inlet)	**Q_b =**	**N/A**	**N/A**	**cfs**
Curb Opening or Slotted Inlet Analysis (Calculated)		MINOR	MAJOR	
Equivalent Slope S_e	S_e =	0.084	0.065	ft/ft
Required Length L_T to Have 100% Interception	L_T =	16.57	29.57	ft
Under No-Clogging Condition		MINOR	MAJOR	
Effective Length of Curb Opening or Slotted Inlet (minimum of L, L_T)	L =	15.00	15.00	ft
Interception Capacity	Q_i =	6.7	12.1	cfs
Under Clogging Condition		MINOR	MAJOR	
Clogging Coefficient	CurbCoeff =	1.31	1.31	
Clogging Factor for Multiple-unit Curb Opening or Slotted Inlet	CurbClog =	0.04	0.04	
Effective (Unclogged) Length	L_e =	14.35	14.35	ft
Actual Interception Capacity	**Q_a =**	**6.6**	**11.7**	**cfs**
Carry-Over Flow = $Q_{b(GRATE)}$-Q_a	**Q_b =**	**0.2**	**5.1**	**cfs**
Summary		MINOR	MAJOR	
Total Inlet Interception Capacity	**Q =**	**6.6**	**11.7**	**cfs**
Total Inlet Carry-Over Flow (flow bypassing inlet)	**Q_b =**	**0.2**	**5.1**	**cfs**
Capture Percentage = Q_a/Q_o	**C% =**	**97**	**70**	**%**

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Inlet ID: **Inlet E7 (Basin E-3b)**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	16.0	ft
Gutter Width	W =	0.83	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	16.0	16.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.4	7.7	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

Maximum Capacity for 1/2 Street based On Allowable Spread

		Minor Storm	Major Storm	
Water Depth without Gutter Depression (T * S_x * 12)	y =	3.84	3.84	inches
Vertical Depth between Gutter Lip and Gutter Flowline (W * S_w * 12)	d_C =	0.8	0.8	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	0.63	0.63	inches
Water Depth at Gutter Flowline (y + a)	d =	4.47	4.47	inches
Allowable Spread for Discharge outside the Gutter Section (T - W)	T_X =	15.2	15.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.149	0.149	
Discharge outside the Gutter Section, carried in Section T_X	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_T - Q_X - Q_{BACK})	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**SUMP**	**SUMP**	**cfs**
Flow Velocity within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	0.0	0.0	

Maximum Capacity for 1/2 Street based on Allowable Depth

		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	15.7	29.5	ft
Theoretical Spread for Discharge outside the Gutter Section (T - W)	$T_{X\,TH}$ =	14.9	28.6	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.152	0.079	
Theoretical Discharge outside the Gutter Section, carried in Section $T_{X\,TH}$	$Q_{X\,TH}$ =	0.0	0.0	cfs
Actual Discharge outside the Gutter Section, (limited by distance T_{CROWN})	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_d - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	SUMP	SUMP	cfs
Average Flow Velocity Within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	0.0	0.0	
Slope-Based Safety Factor for Minor/Major Storm depth reduction, d ≥ 6"	R =	SUMP	SUMP	
Max Flow based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**SUMP**	**SUMP**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =			inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =			inches

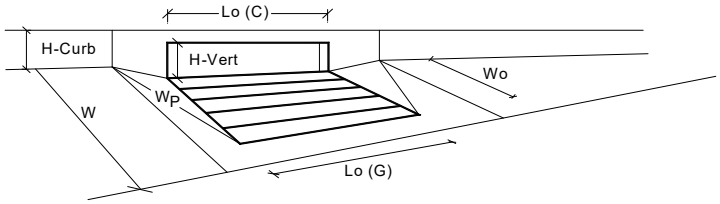
MINOR STORM Allowable Capacity is not applicable to Sump Condition

MAJOR STORM Allowable Capacity is not applicable to Sump Condition

	Minor Storm	Major Storm	
Q_{allow} =	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

MHFD-Inlet, Version 5.03 (August 2023)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	4.4	7.7	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Open Area Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	10.00	10.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	0.83	0.83	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as an Orifice (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	**Q_{Grate} =**	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.25	1.25	
Clogging Factor for Multiple Units	Clog =	0.06	0.06	
Curb Capacity as a Weir (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	5.5	17.9	cfs
Interception with Clogging	Q_{wa} =	5.2	16.8	cfs
Curb Capacity as an Orifice (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	16.9	22.0	cfs
Interception with Clogging	Q_{oa} =	15.8	20.6	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	9.0	18.5	cfs
Interception with Clogging	Q_{ma} =	8.4	17.3	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	**Q_{Curb} =**	**5.2**	**16.8**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	10.00	10.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	15.7	29.5	**ft. >T-Crown**
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.0	3.2	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.30	0.57	ft
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.82	1.00	
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**5.2**	**16.8**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q Peak)	$Q_{PEAK\ REQUIRED}$ =	3.5	11.0	cfs

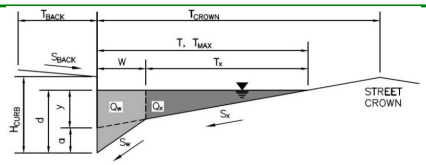
MHFD-Inlet, Version 5.03 (August 2023)

ALLOWABLE CAPACITY FOR ONE-HALF OF STREET (Minor & Major Storm)

(Based on Regulated Criteria for Maximum Allowable Flow Depth and Spread)

Project:

Inlet ID: **Inlet E9 (Basin E-4b)**



Gutter Geometry:

Maximum Allowable Width for Spread Behind Curb	T_{BACK} =	7.5	ft
Side Slope Behind Curb (leave blank for no conveyance credit behind curb)	S_{BACK} =	0.020	ft/ft
Manning's Roughness Behind Curb (typically between 0.012 and 0.020)	n_{BACK} =	0.020	
Height of Curb at Gutter Flow Line	H_{CURB} =	6.00	inches
Distance from Curb Face to Street Crown	T_{CROWN} =	16.0	ft
Gutter Width	W =	0.83	ft
Street Transverse Slope	S_X =	0.020	ft/ft
Gutter Cross Slope (typically 2 inches over 24 inches or 0.083 ft/ft)	S_W =	0.083	ft/ft
Street Longitudinal Slope - Enter 0 for sump condition	S_O =	0.000	ft/ft
Manning's Roughness for Street Section (typically between 0.012 and 0.020)	n_{STREET} =	0.016	

		Minor Storm	Major Storm	
Max. Allowable Spread for Minor & Major Storm	T_{MAX} =	16.0	16.0	ft
Max. Allowable Depth at Gutter Flowline for Minor & Major Storm	d_{MAX} =	4.4	7.7	inches
Check boxes are not applicable in SUMP conditions		☐	☐	

Maximum Capacity for 1/2 Street based On Allowable Spread

		Minor Storm	Major Storm	
Water Depth without Gutter Depression (T * S_x * 12)	y =	3.84	3.84	inches
Vertical Depth between Gutter Lip and Gutter Flowline (W * S_w * 12)	d_C =	0.8	0.8	inches
Gutter Depression (d_C - (W * S_x * 12))	a =	0.63	0.63	inches
Water Depth at Gutter Flowline (y + a)	d =	4.47	4.47	inches
Allowable Spread for Discharge outside the Gutter Section (T - W)	T_X =	15.2	15.2	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.149	0.149	
Discharge outside the Gutter Section, carried in Section T_X	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_T - Q_X - Q_{BACK})	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Maximum Flow Based On Allowable Spread	**Q_T =**	**SUMP**	**SUMP**	**cfs**
Flow Velocity within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity times Gutter Flowline Depth	V*d =	0.0	0.0	

Maximum Capacity for 1/2 Street based on Allowable Depth

		Minor Storm	Major Storm	
Theoretical Water Spread	T_{TH} =	15.7	29.5	ft
Theoretical Spread for Discharge outside the Gutter Section (T - W)	$T_{X\,TH}$ =	14.9	28.6	ft
Gutter Flow to Design Flow Ratio by FHWA HEC-22 method (Eq. 7-7)	E_O =	0.152	0.079	
Theoretical Discharge outside the Gutter Section, carried in Section $T_{X\,TH}$	$Q_{X\,TH}$ =	0.0	0.0	cfs
Actual Discharge outside the Gutter Section, (limited by distance T_{CROWN})	Q_X =	0.0	0.0	cfs
Discharge within the Gutter Section (Q_d - Q_X)	Q_W =	0.0	0.0	cfs
Discharge Behind the Curb (e.g., sidewalk, driveways, & lawns)	Q_{BACK} =	0.0	0.0	cfs
Total Discharge for Major & Minor Storm (Pre-Safety Factor)	Q =	SUMP	SUMP	cfs
Average Flow Velocity Within the Gutter Section	V =	0.0	0.0	fps
V*d Product: Flow Velocity Times Gutter Flowline Depth	V*d =	0.0	0.0	
Slope-Based Safety Factor for Minor/Major Storm depth reduction, d ≥ 6"	R =	SUMP	SUMP	
Max Flow based on Allowable Depth (Safety Factor Applied)	**Q_d =**	**SUMP**	**SUMP**	**cfs**
Resultant Flow Depth at Gutter Flowline (Safety Factor Applied)	d =			inches
Resultant Flow Depth at Street Crown (Safety Factor Applied)	d_{CROWN} =			inches

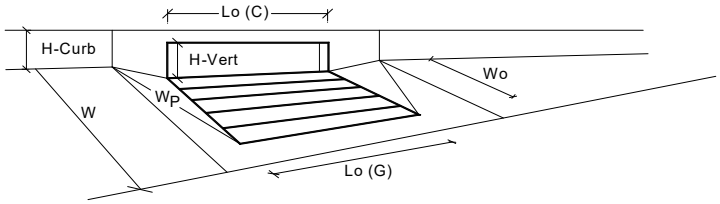
MINOR STORM Allowable Capacity is not applicable to Sump Condition

MAJOR STORM Allowable Capacity is not applicable to Sump Condition

	Minor Storm	Major Storm	
Q_{allow} =	**SUMP**	**SUMP**	**cfs**

INLET IN A SUMP OR SAG LOCATION

MHFD-Inlet, Version 5.03 (August 2023)



Design Information (Input)		MINOR	MAJOR	
Type of Inlet	Type =	CDOT Type R Curb Opening		
Local Depression (additional to continuous gutter depression 'a' from above)	a_{local} =	3.00	3.00	inches
Number of Unit Inlets (Grate or Curb Opening)	No =	1	1	
Water Depth at Flowline (outside of local depression)	Ponding Depth =	4.4	7.7	inches
Grate Information		MINOR	MAJOR	☑ Override Depths
Length of a Unit Grate	L_o (G) =	N/A	N/A	feet
Width of a Unit Grate	W_o =	N/A	N/A	feet
Open Area Ratio for a Grate (typical values 0.15-0.90)	A_{ratio} =	N/A	N/A	
Clogging Factor for a Single Grate (typical value 0.50 - 0.70)	C_f (G) =	N/A	N/A	
Grate Weir Coefficient (typical value 2.15 - 3.60)	C_w (G) =	N/A	N/A	
Grate Orifice Coefficient (typical value 0.60 - 0.80)	C_o (G) =	N/A	N/A	
Curb Opening Information		MINOR	MAJOR	
Length of a Unit Curb Opening	L_o (C) =	15.00	15.00	feet
Height of Vertical Curb Opening in Inches	H_{vert} =	6.00	6.00	inches
Height of Curb Orifice Throat in Inches	H_{throat} =	6.00	6.00	inches
Angle of Throat	Theta =	63.40	63.40	degrees
Side Width for Depression Pan (typically the gutter width of 2 feet)	W_p =	0.83	0.83	feet
Clogging Factor for a Single Curb Opening (typical value 0.10)	C_f (C) =	0.10	0.10	
Curb Opening Weir Coefficient (typical value 2.3-3.7)	C_w (C) =	3.60	3.60	
Curb Opening Orifice Coefficient (typical value 0.60 - 0.70)	C_o (C) =	0.67	0.67	
Grate Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	N/A	N/A	
Clogging Factor for Multiple Units	Clog =	N/A	N/A	
Grate Capacity as a Weir (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	N/A	N/A	cfs
Interception with Clogging	Q_{wa} =	N/A	N/A	cfs
Grate Capacity as an Orifice (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	N/A	N/A	cfs
Interception with Clogging	Q_{oa} =	N/A	N/A	cfs
Grate Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	N/A	N/A	cfs
Interception with Clogging	Q_{ma} =	N/A	N/A	cfs
Resulting Grate Capacity (assumes clogged condition)	**Q_{Grate} =**	**N/A**	**N/A**	**cfs**
Curb Opening Flow Analysis (Calculated)		MINOR	MAJOR	
Clogging Coefficient for Multiple Units	Coef =	1.31	1.31	
Clogging Factor for Multiple Units	Clog =	0.04	0.04	
Curb Capacity as a Weir (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{wi} =	6.5	22.6	cfs
Interception with Clogging	Q_{wa} =	6.2	21.6	cfs
Curb Capacity as an Orifice (based on MHFD - CSU 2010 Study)		MINOR	MAJOR	
Interception without Clogging	Q_{oi} =	25.3	33.0	cfs
Interception with Clogging	Q_{oa} =	24.2	31.5	cfs
Curb Opening Capacity as Mixed Flow		MINOR	MAJOR	
Interception without Clogging	Q_{mi} =	11.9	25.4	cfs
Interception with Clogging	Q_{ma} =	11.4	24.3	cfs
Resulting Curb Opening Capacity (assumes clogged condition)	**Q_{Curb} =**	**6.2**	**21.6**	**cfs**
Resultant Street Conditions		MINOR	MAJOR	
Total Inlet Length	L =	15.00	15.00	feet
Resultant Street Flow Spread (based on street geometry from above)	T =	15.7	29.5	**ft. >T-Crown**
Resultant Flow Depth at Street Crown	d_{CROWN} =	0.0	3.2	inches
Low Head Performance Reduction (Calculated)		MINOR	MAJOR	
Depth for Grate Midwidth	d_{Grate} =	N/A	N/A	ft
Depth for Curb Opening Weir Equation	d_{Curb} =	0.30	0.57	ft
Grated Inlet Performance Reduction Factor for Long Inlets	RF_{Grate} =	N/A	N/A	
Curb Opening Performance Reduction Factor for Long Inlets	RF_{Curb} =	0.67	0.88	
Combination Inlet Performance Reduction Factor for Long Inlets	$RF_{Combination}$ =	N/A	N/A	
		MINOR	MAJOR	
Total Inlet Interception Capacity (assumes clogged condition)	**Q_a =**	**6.2**	**21.6**	**cfs**
Inlet Capacity IS GOOD for Minor and Major Storms (>Q Peak)	$Q_{PEAK\ REQUIRED}$ =	1.7	11.0	cfs

Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Friday, Mar 15 2024

Pond D Emergency Overflow Swale

Trapezoidal

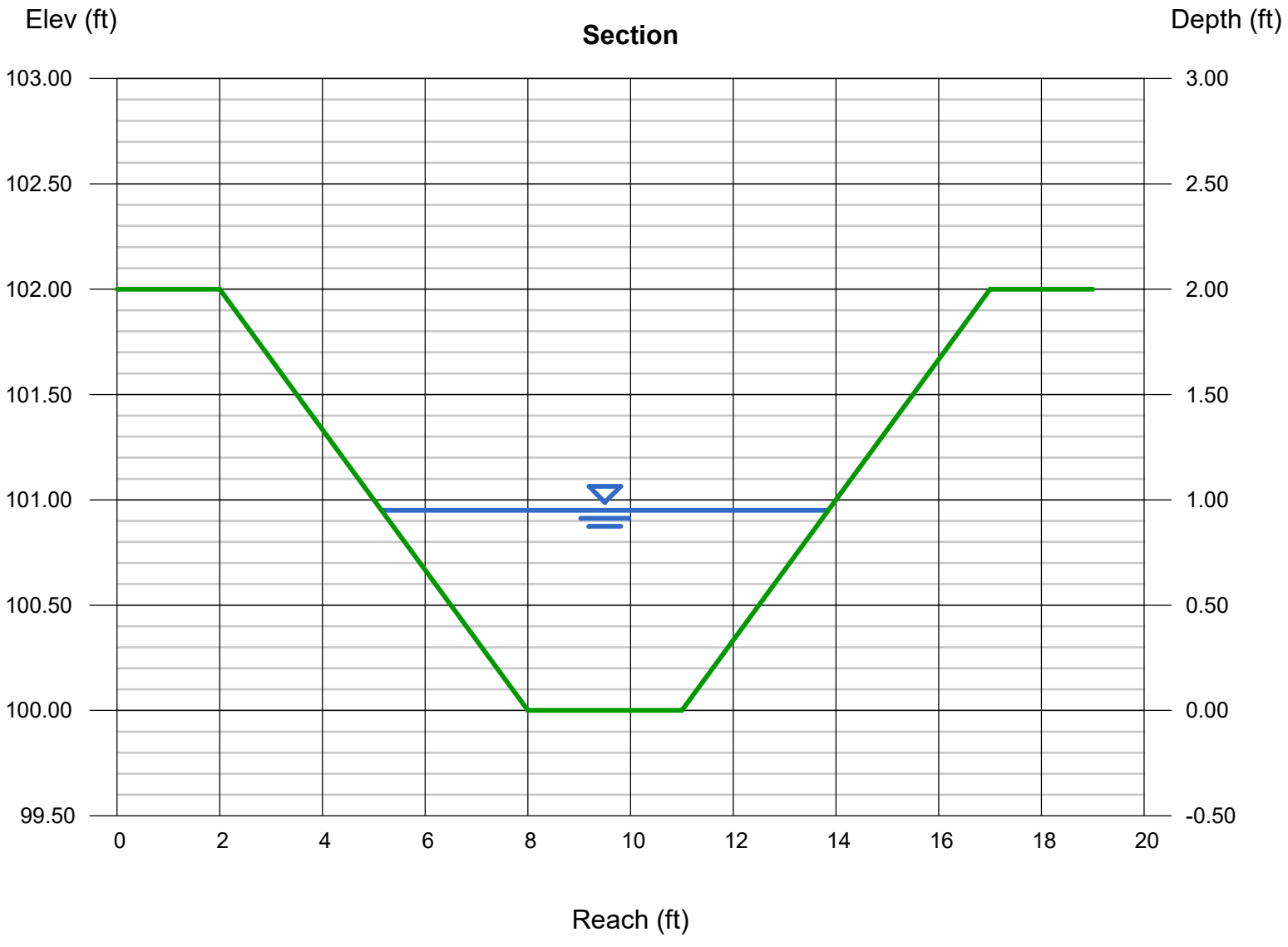
Bottom Width (ft)	= 3.00
Side Slopes (z:1)	= 3.00, 3.00
Total Depth (ft)	= 2.00
Invert Elev (ft)	= 100.00
Slope (%)	= 2.00
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 28.10

Highlighted

Depth (ft)	= 0.95
Q (cfs)	= 28.10
Area (sqft)	= 5.56
Velocity (ft/s)	= 5.06
Wetted Perim (ft)	= 9.01
Crit Depth, Yc (ft)	= 1.01
Top Width (ft)	= 8.70
EGL (ft)	= 1.35



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Friday, Mar 15 2024

Pond E Emergency Overflow Swale

Triangular

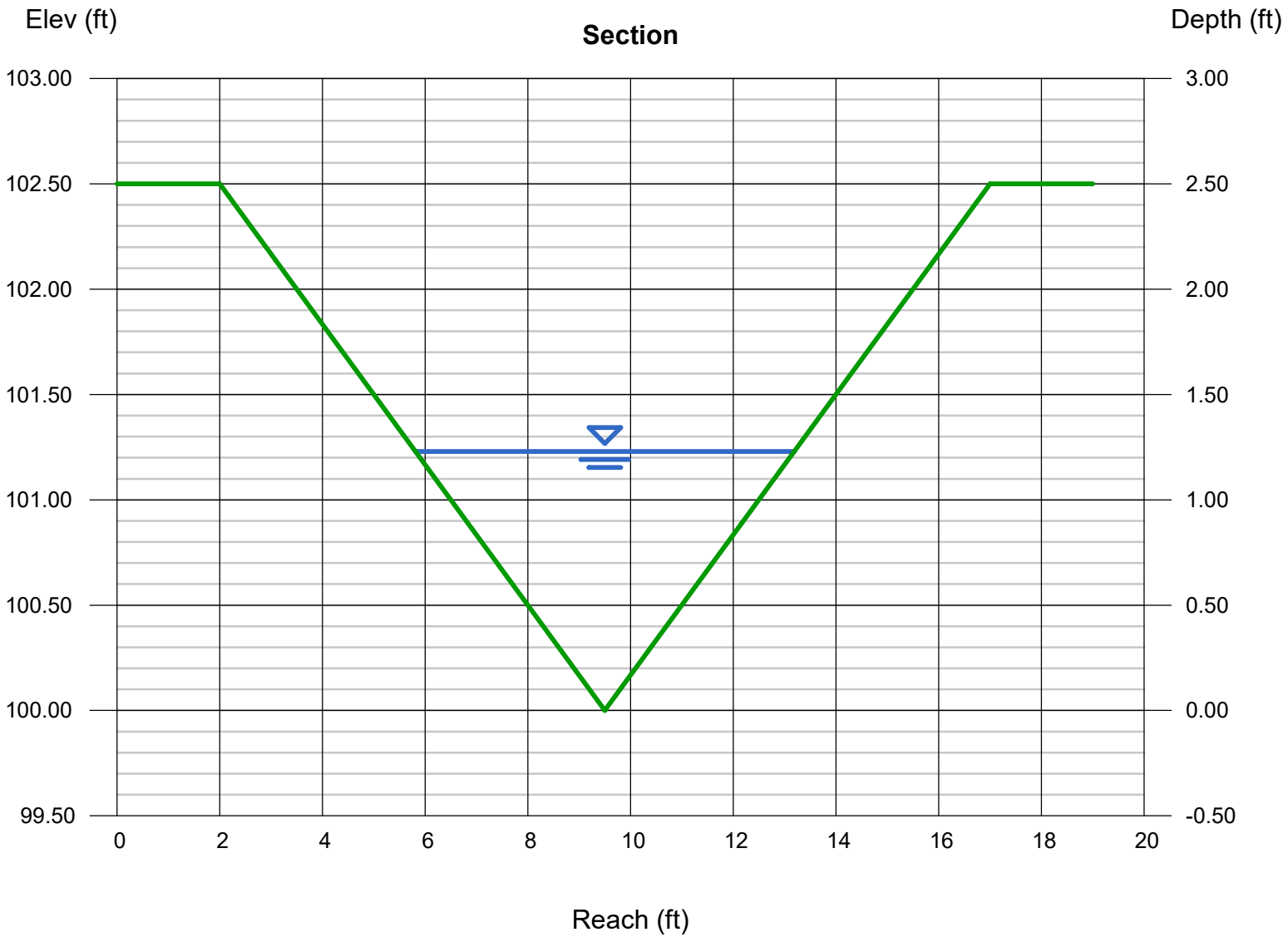
Side Slopes (z:1)	= 3.00, 3.00
Total Depth (ft)	= 2.50
Invert Elev (ft)	= 100.00
Slope (%)	= 2.00
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 22.00

Highlighted

Depth (ft)	= 1.23
Q (cfs)	= 22.00
Area (sqft)	= 4.54
Velocity (ft/s)	= 4.85
Wetted Perim (ft)	= 7.78
Crit Depth, Yc (ft)	= 1.28
Top Width (ft)	= 7.38
EGL (ft)	= 1.60



OVERFLOW SWALE RIPRAP SIZING CALCULATIONS

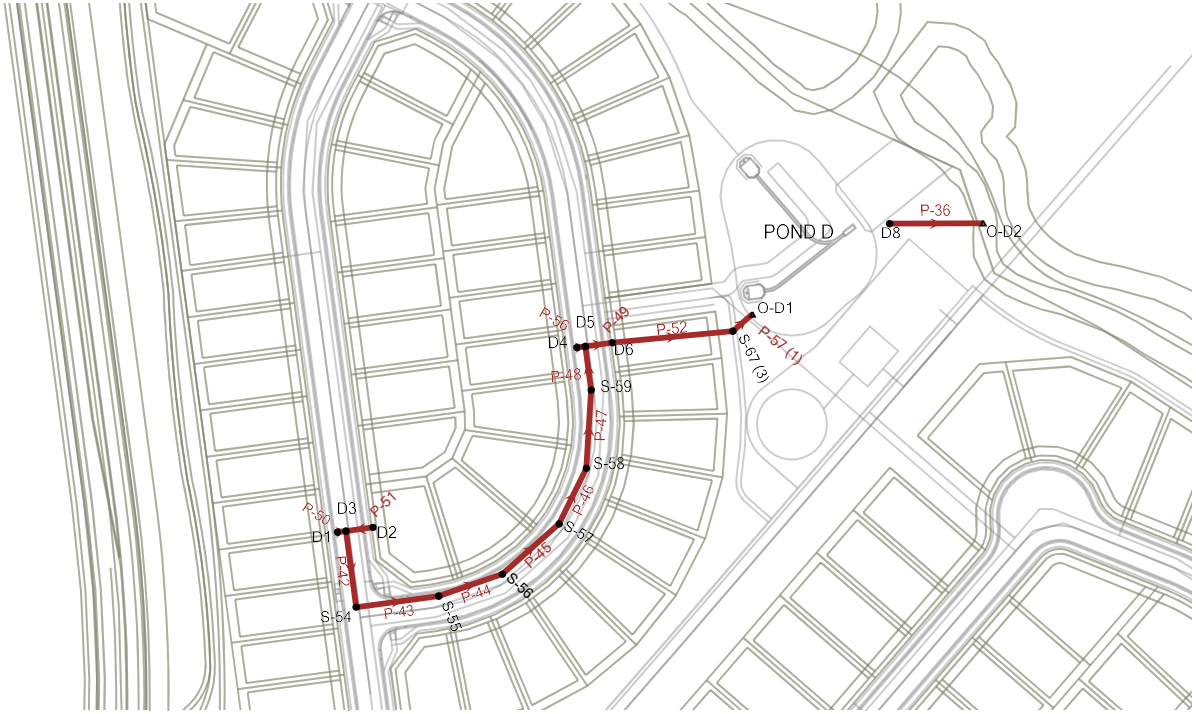
Subdivision: Grandview Reserve Filing No. 1
Location: CO, Falcon (El Paso County)

Project Name: Grandview Reserve Filing No. 1
Project No.: HRG02.20
Calculated By: TJE
Checked By: BAS
Date: 3/20/24

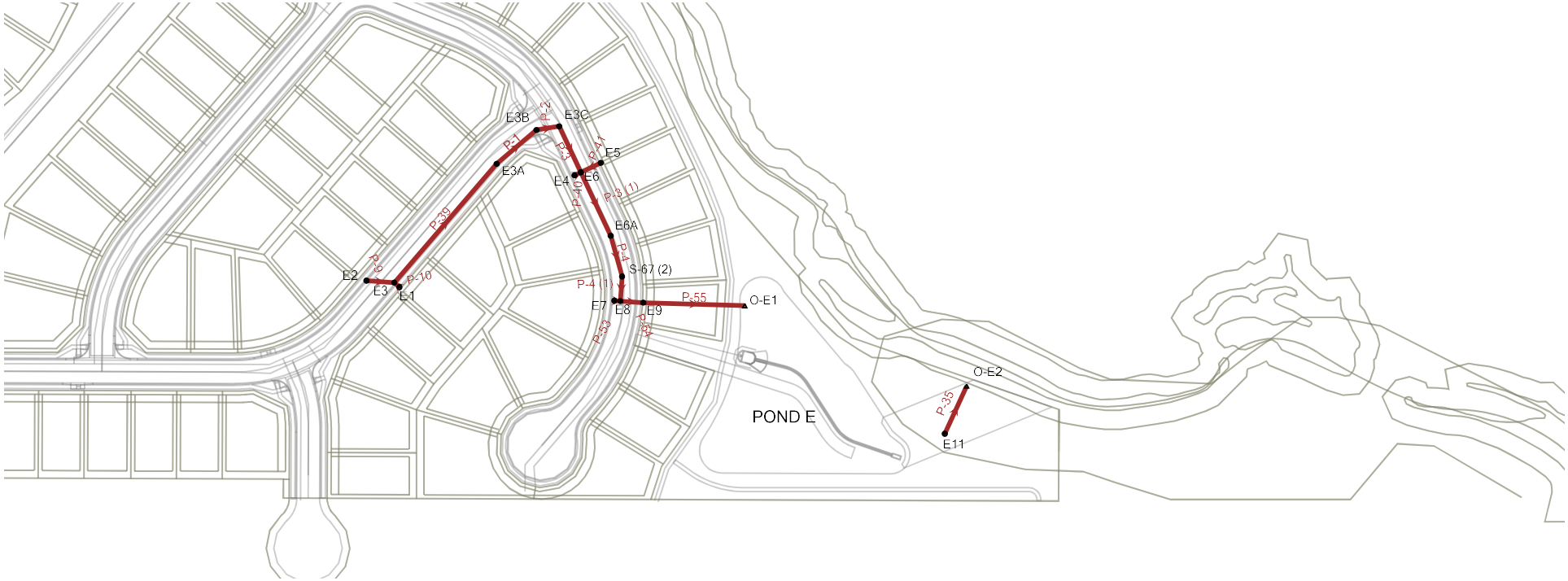
	STORM DRAIN SYSTEM				
	Pond D Overflow Swale	Pond E Overflow Swale	Pond D FES	Pond E FES	
Q100 (cfs)	28.1	22.0	8.0	14.9	Flows are the greater of proposed vs. future
D or H (in)	24	30	18	18	
W (ft)	3	0			
Slope (%)	2.00	2	1	1	
Yn (in)	11.40	14.76	18.00	18.00	
Yt (ft)	unknown	unknown	unknown	unknown	If "unknown" Yt/D=0.4
Yt/D, Yt/H	0.40	0.40	0.40	0.40	
Supercritical	Yes	Yes	Yes	Yes	
Q/D^2.5, Q/WH^1.5	3.31	2.23	2.90	5.41	
Q/D^1.5, Q/WH^0.5					
Da, Ha (in) *	17.70	22.38	18.00	18.00	Da=0.5(D+Yn), Ha=0.5(H+Yn)
Q/Da^1.5, Q/WHa^0.5 *	7.71	8.64	4.35	8.11	
d50 (in), Required	3.24	7.47	3.77	7.01	
d50 (in)	9	9	9	9	
RipRap Size	**Type L**	**Type L**	**Type L**	**Type L**	
1/(2 tan q)	1.00	1.00	4.50	2.25	Fig. 9-35 OR Fig 9-36
Erosive Soils	Yes	Yes	Yes	Yes	
At	5.11	4.00	1.45	2.71	At=Q/5.5
L	3.4	1.5	4.2	6.8	L=(1/(2 tan q))(At/Yt - D)
Min L	6.0	7.5	4.5	4.5	Min L=3D or 3H
Max L	20.0	25.0	15.0	15.0	Max L=10D or 10H
Length (ft)	6.0	7.5	4.5	7.0	
Bottom Width (ft)	6.0	7.5	4.5	4.5	Width=3D (Minimum)
Riprap Depth (in)	18	18	18	18	Depth=2(d50)
Type II Base Depth (in)	6	6	6	6	
Cutoff Wall	No	No	Yes	Yes	
Cutoff Wall Depth (ft)			3.0	3.0	Depth of Riprap and Base
Cutoff Wall Width (ft)			6.2	6.2	

Galloway

Grandview Reserve Filing No. 1
D Basin Schematic



Grandview Reserve Filing No. 1
E Basin Schematic



Grandview Reserve Filing No. 1

FlexTable: Conduit Table

Active Scenario: 5-YR Event

Label	Start Node	Stop Node	Diameter (in)	Material	Manning's n	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Invert (Start) (ft)	Invert (Stop) (ft)	Flow (cfs)	Velocity (ft/s)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)
P-1	E3A	E3B	30.0	Concrete	0.012	68.6	0.007	6,955.27	6,954.75	7.40	6.05	6,956.17	6,955.50
P-2	E3B	E3C	30.0	Concrete	0.012	30.2	0.007	6,954.55	6,954.32	7.40	6.05	6,955.45	6,955.48
P-3	E3C	E6	36.0	Concrete	0.012	66.7	0.007	6,953.82	6,953.32	7.40	5.94	6,955.41	6,955.42
P-3 (1)	E6	E6A	36.0	Concrete	0.012	92.0	0.007	6,953.22	6,952.53	18.30	7.68	6,954.59	6,953.66
P-4	E6A	S-67 (2)	36.0	Concrete	0.012	55.4	0.007	6,952.43	6,952.01	18.30	7.68	6,953.80	6,953.16
P-4 (1)	S-67 (2)	E8	36.0	Concrete	0.012	32.3	0.007	6,951.92	6,951.67	18.30	7.68	6,953.29	6,953.43
P-9	E2	E3	24.0	Concrete	0.012	35.4	0.027	6,958.38	6,957.40	3.30	7.78	6,959.01	6,958.32
P-10	E1	E3	24.0	Concrete	0.012	9.0	0.027	6,957.65	6,957.40	4.10	8.29	6,958.36	6,958.32
P-35	E11	O-E2	18.0	Concrete	0.012	66.2	0.010	6,945.69	6,945.01	0.40	3.05	6,945.92	6,945.21
P-36	D8	O-D2	18.0	Concrete	0.012	98.7	0.010	6,964.99	6,964.01	0.20	2.45	6,965.16	6,964.15
P-39	E3	E3A	30.0	Concrete	0.012	205.8	0.007	6,956.91	6,955.36	7.40	6.05	6,957.81	6,956.11
P-40	E4	E6	24.0	Concrete	0.012	9.2	0.020	6,954.51	6,954.32	4.30	7.51	6,955.33	6,955.42
P-41	E5	E6	24.0	Concrete	0.012	29.1	0.010	6,954.62	6,954.32	6.60	6.62	6,955.53	6,955.42
P-42	D3	S-54	24.0	Concrete	0.012	82.0	0.046	6,980.35	6,976.60	3.60	9.55	6,981.02	6,976.96
P-43	S-54	S-55	24.0	Concrete	0.012	89.0	0.010	6,976.31	6,975.42	1.32	4.16	6,976.70	6,975.73
P-44	S-55	S-56	24.0	Concrete	0.012	72.0	0.009	6,975.31	6,974.70	1.32	3.93	6,975.71	6,975.03
P-45	S-56	S-57	24.0	Concrete	0.012	81.0	0.009	6,974.60	6,973.91	1.32	3.93	6,974.99	6,974.23
P-46	S-57	S-58	24.0	Concrete	0.012	66.0	0.009	6,973.81	6,973.24	1.32	3.93	6,974.20	6,973.57
P-47	S-58	S-59	24.0	Concrete	0.012	83.5	0.009	6,973.14	6,972.43	1.32	3.93	6,973.54	6,972.75
P-48	S-59	D5	24.0	Concrete	0.012	47.0	0.009	6,972.33	6,971.92	1.32	3.93	6,972.72	6,972.47
P-49	D5	D6	36.0	Concrete	0.012	27.7	0.015	6,970.93	6,970.51	9.80	8.25	6,971.92	6,971.63
P-50	D3	D1	24.0	Concrete	0.012	9.2	-0.010	6,980.66	6,980.75	2.60	5.08	6,981.31	6,981.39
P-51	D3	D2	24.0	Concrete	0.012	29.2	-0.005	6,980.66	6,980.80	1.00	3.00	6,981.38	6,981.39
P-52	D6	S-67 (3)	36.0	Concrete	0.012	127.8	0.015	6,970.41	6,968.49	14.20	9.18	6,971.61	6,969.31
P-53	E7	E8	36.0	Concrete	0.012	8.2	0.005	6,951.72	6,951.67	3.50	4.14	6,953.42	6,953.43
P-54	E8	E9	42.0	Concrete	0.012	30.2	0.005	6,951.17	6,951.02	21.80	6.89	6,952.61	6,952.33
P-55	E9	O-E1	42.0	Concrete	0.012	131.8	0.005	6,950.92	6,950.26	1.70	3.27	6,951.31	6,951.30
P-56	D4	D5	24.0	Concrete	0.012	9.2	0.015	6,972.06	6,971.92	6.10	7.49	6,972.94	6,972.64
P-57 (1)	S-67 (3)	O-D1	36.0	Concrete	0.012	27.8	0.010	6,968.29	6,968.01	14.20	7.94	6,969.49	6,968.99

Grandview Reserve Filing No. 1

FlexTable: Manhole Table

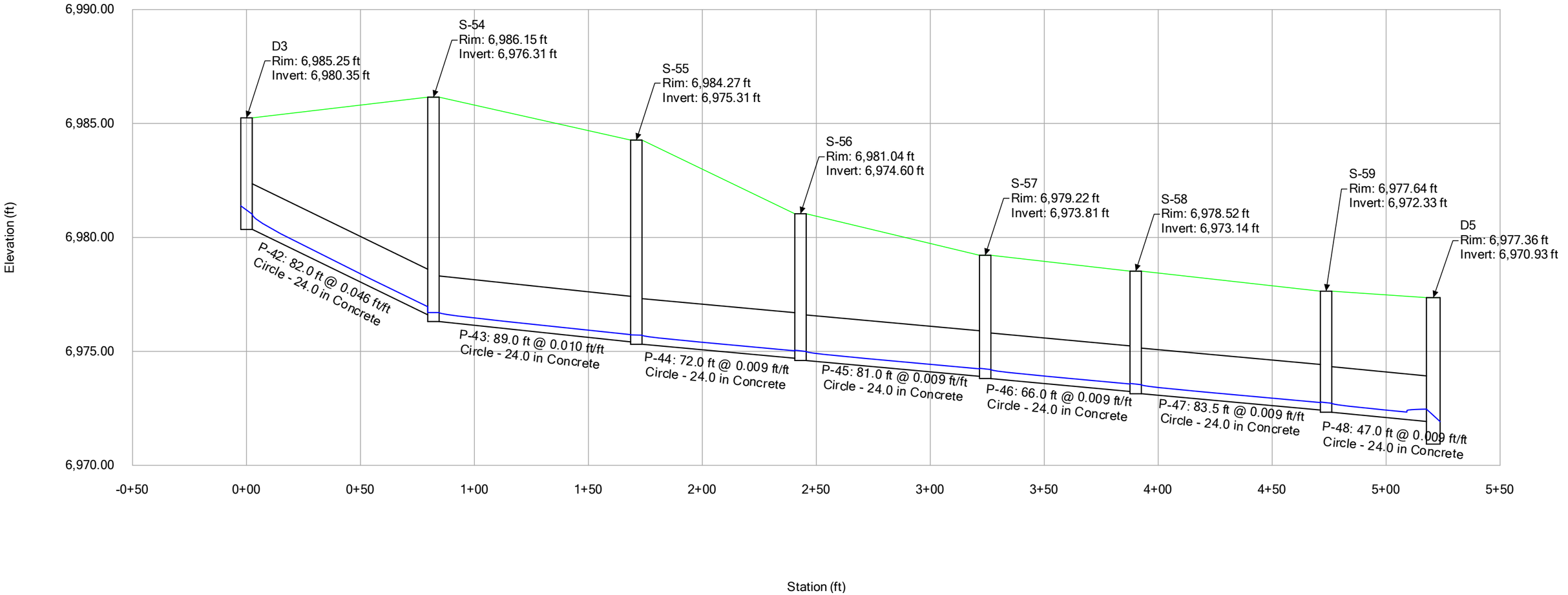
Active Scenario: 5-YR Event

Label	Notes	Elevation (Ground) (ft)	Elevation (Invert in 1) (ft)	Elevation (Invert Out) (ft)	Flow (Total Out) (cfs)	Headloss Method	Headloss Coefficient (Standard)	Hydraulic Grade Line (Out) (ft)	Hydraulic Grade Line (In) (ft)
D1	CDOT-TYPE R INLET (5')	6,985.58	(N/A)	6,980.75	2.60	Standard	0.000	6,981.31	6,981.31
D2	CDOT-TYPE R INLET (5')	6,985.58	(N/A)	6,980.80	1.00	Standard	0.000	6,981.38	6,981.38
D3	MH-ECCENTRIC (5' %%C)	6,985.25	6,980.66	6,980.35	3.60	Standard	1.520	6,981.02	6,981.39
D4	CDOT-TYPE R INLET (15') - (PUB)	6,977.69	(N/A)	6,972.06	6.10	Standard	0.000	6,972.94	6,972.94
D5	MH-ECCENTRIC (6' %%C)	6,977.36	6,971.92	6,970.93	9.80	Standard	1.520	6,971.92	6,972.47
D6	CDOT-TYPE R INLET (15') - (PUB)	6,977.69	6,970.51	6,970.41	14.20	Standard	0.050	6,971.61	6,971.63
D8	MODIFIED CDOT TYPE DPOND D OUTLET STRUCTURE(SEE GEC PLAN)	6,969.69	(N/A)	6,964.99	0.20	Standard	0.000	6,965.16	6,965.16
E1	CDOT-TYPE R INLET (15') - (PUB)	6,962.81	(N/A)	6,957.65	4.10	Standard	0.000	6,958.36	6,958.36
E2	CDOT-TYPE R INLET (15') - (PUB)	6,963.03	(N/A)	6,958.38	3.30	Standard	0.000	6,959.01	6,959.01
E3	5' %%C SDMH - (PUB)	6,962.47	6,957.40	6,956.91	7.40	Standard	1.520	6,957.81	6,958.32
E3A	5' %%C SDMH - (PUB)	6,960.42	6,955.36	6,955.27	7.40	Standard	0.100	6,956.17	6,956.21
E3B	5' %%C SDMH - (PUB)	6,959.71	6,954.75	6,954.55	7.40	Standard	0.100	6,955.45	6,955.49
E3C	5' %%C SDMH - (PUB)	6,959.59	6,954.32	6,953.82	7.40	Standard	1.320	6,955.41	6,955.48
E4	CDOT-TYPE R INLET (15') - (PUB)	6,959.26	(N/A)	6,954.51	4.30	Standard	0.000	6,955.33	6,955.33
E5	CDOT-TYPE R INLET (15') - (PUB)	6,959.26	(N/A)	6,954.62	6.60	Standard	0.000	6,955.53	6,955.53
E6	6' %%C SDMH - (PUB)	6,958.92	6,953.32	6,953.22	18.30	Standard	1.570	6,954.59	6,955.42
E6A	5' %%C SDMH - (PUB)	6,958.00	6,952.53	6,952.43	18.30	Standard	0.100	6,953.80	6,953.85
E7	CDOT-TYPE R INLET (10')	6,957.60	(N/A)	6,951.72	3.50	Standard	0.000	6,953.42	6,953.42
E8	MH-ECCENTRIC (6' %%C)	6,957.24	6,951.67	6,951.17	21.80	Standard	1.520	6,952.61	6,953.43
E9	CDOT-TYPE R INLET (15')	6,957.59	6,951.02	6,950.92	1.70	Standard	0.050	6,951.31	6,951.32
E11	MODIFIED CDOT TYPE DPOND E OUTLET STRUCTURE(SEE GEC PLAN)	6,951.93	(N/A)	6,945.69	0.40	Standard	0.050	6,945.92	6,945.93
S-54	MH-ECCENTRIC (5' %%C)	6,986.15	6,976.60	6,976.31	1.32	Standard	0.000	6,976.70	6,976.70
S-55	MH-ECCENTRIC (5' %%C)	6,984.27	6,975.42	6,975.31	1.32	Standard	0.100	6,975.71	6,975.72
S-56	MH-ECCENTRIC (5' %%C)	6,981.04	6,974.70	6,974.60	1.32	Standard	0.400	6,974.99	6,975.05
S-57	MH-ECCENTRIC (5' %%C)	6,979.22	6,973.91	6,973.81	1.32	Standard	0.400	6,974.20	6,974.26
S-58	MH-ECCENTRIC (5' %%C)	6,978.52	6,973.24	6,973.14	1.32	Standard	0.400	6,973.54	6,973.59
S-59	MH-ECCENTRIC (5' %%C)	6,977.64	6,972.43	6,972.33	1.32	Standard	0.400	6,972.72	6,972.78
S-67 (2)	MH-ECCENTRIC (5' %%C)	6,957.45	6,952.01	6,951.92	18.30	Standard	0.100	6,953.29	6,953.34
S-67 (3)	MH-ECCENTRIC (5' %%C)	6,973.84	6,968.49	6,968.29	14.20	Standard	0.400	6,969.49	6,969.67

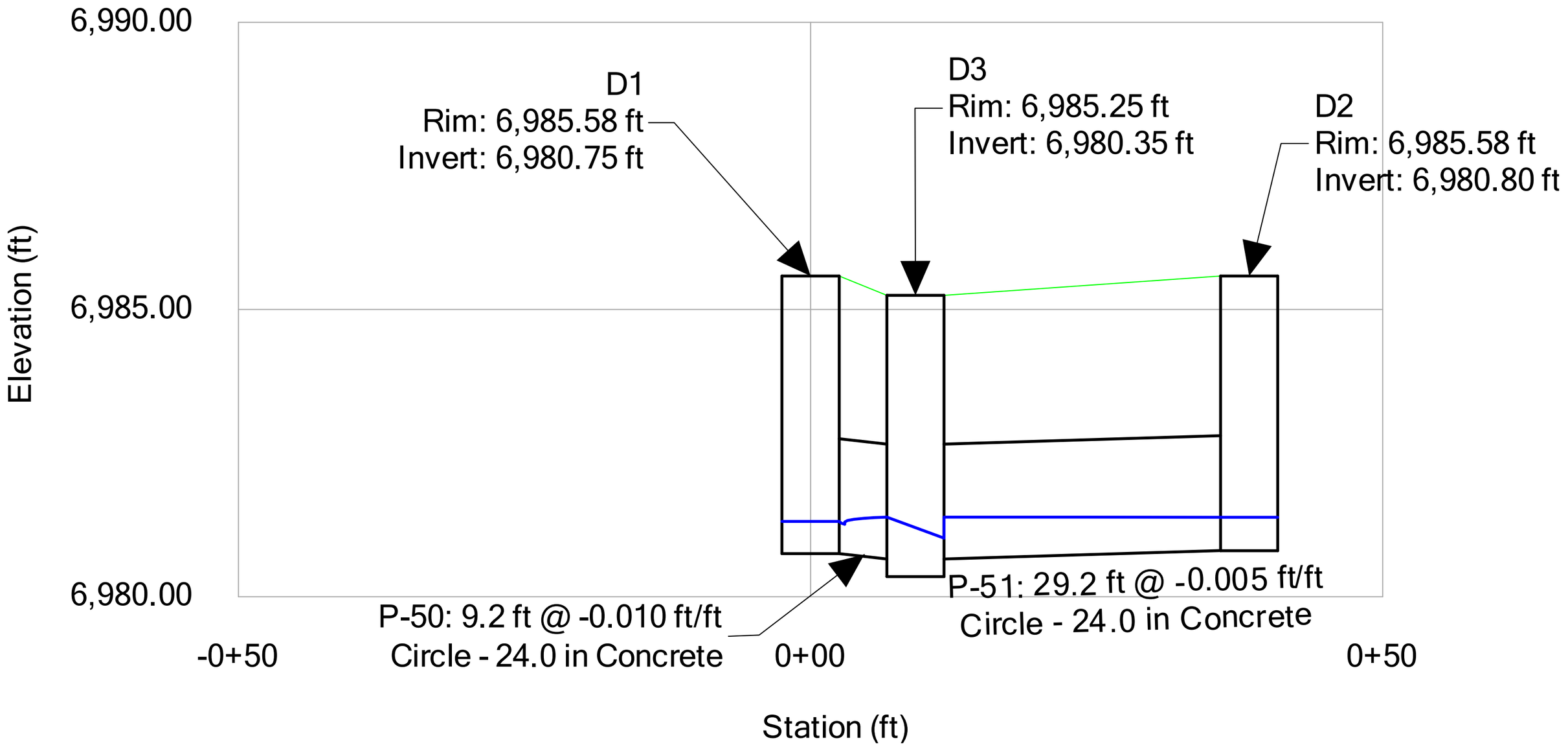
Grandview Reserve Filing No. 1
FlexTable: Outfall Table
Active Scenario: 5-YR Event

Label	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Boundary Condition Type	Elevation (User Defined Tailwater) (ft)	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
O-D2	6,966.22	6,964.01	Free Outfall		6,964.15	0.20
O-E2	6,947.22	6,945.01	Free Outfall		6,945.21	0.40
O-D1	6,973.84	6,968.01	User Defined Tailwater	6,969.15	6,968.99	14.20
O-E1	6,957.59	6,950.26	User Defined Tailwater	6,951.30	6,951.30	1.70

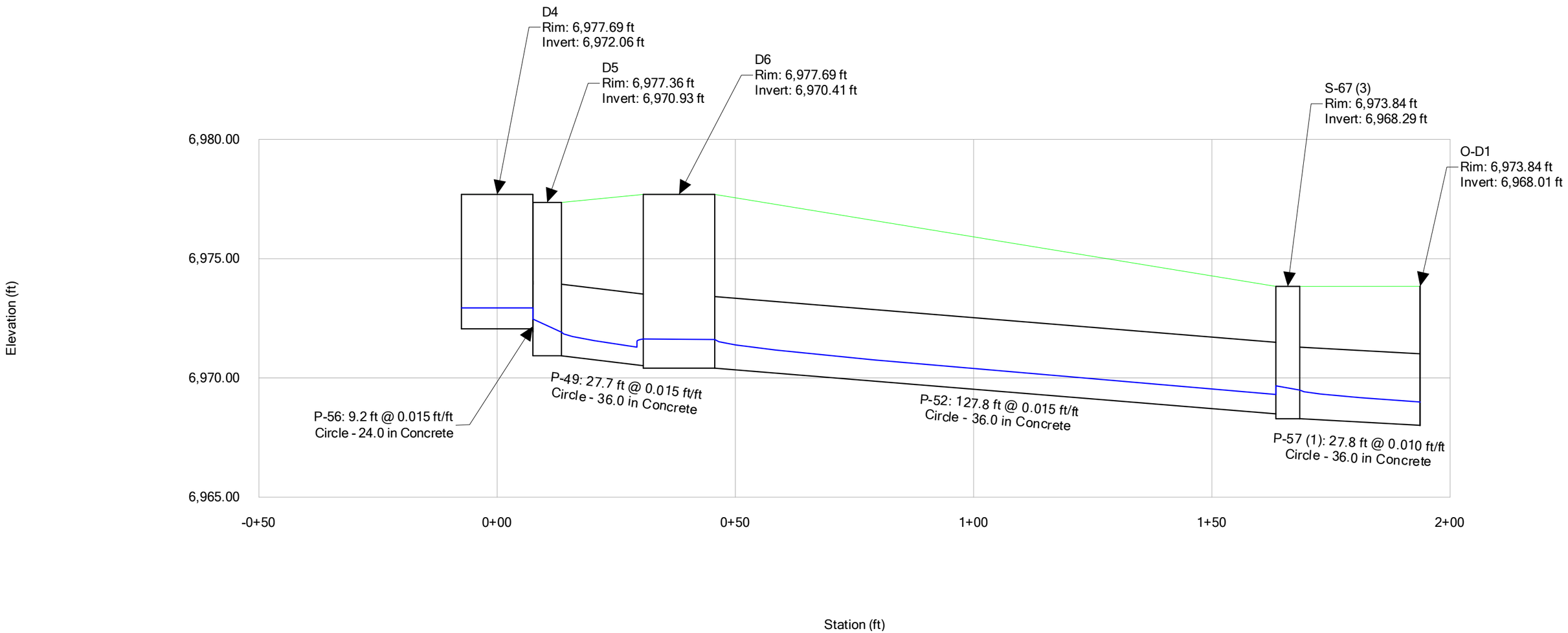
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - D3 to D5 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 5-YR Event



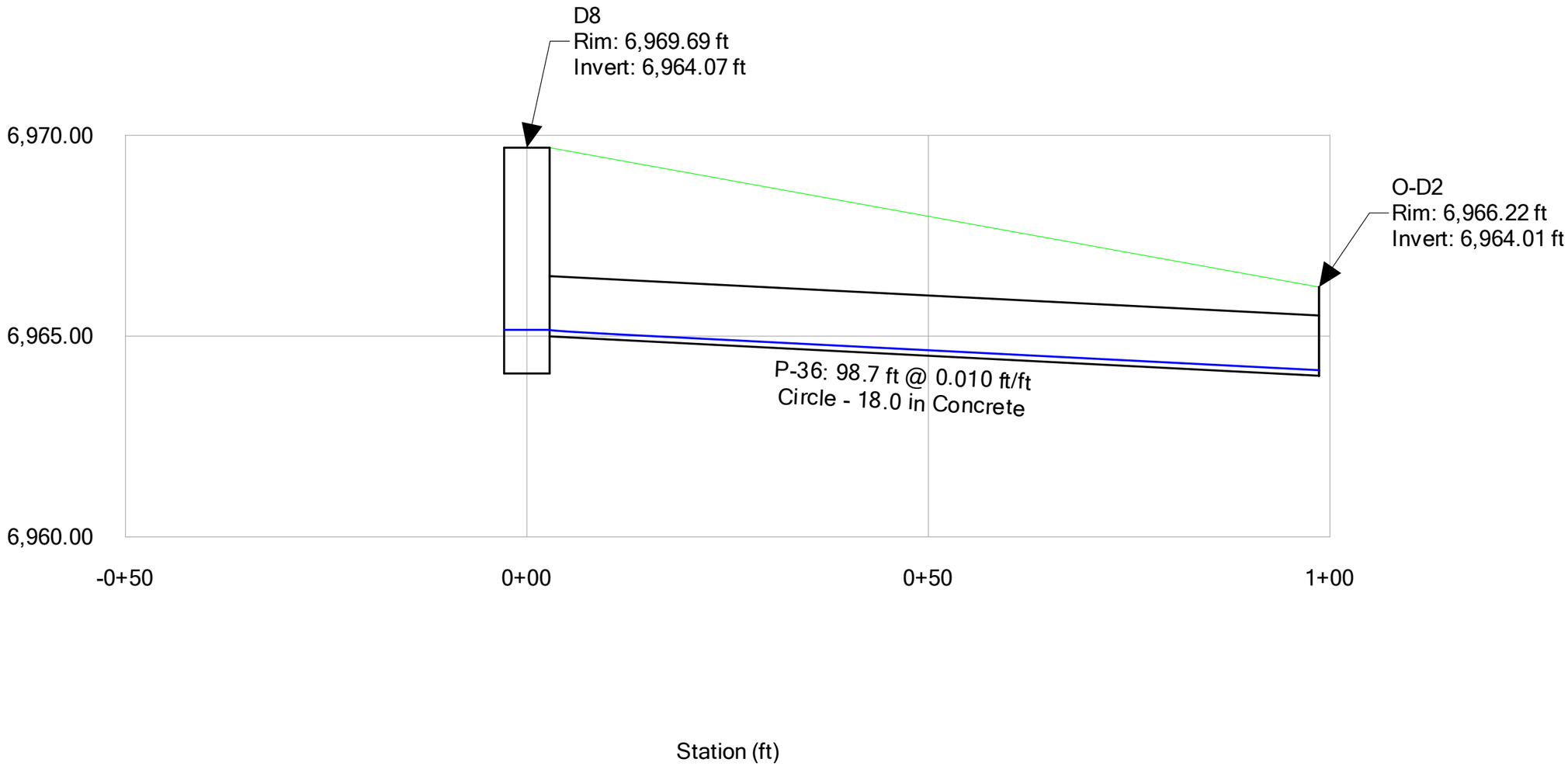
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - D1 to D2 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 5-YR Event



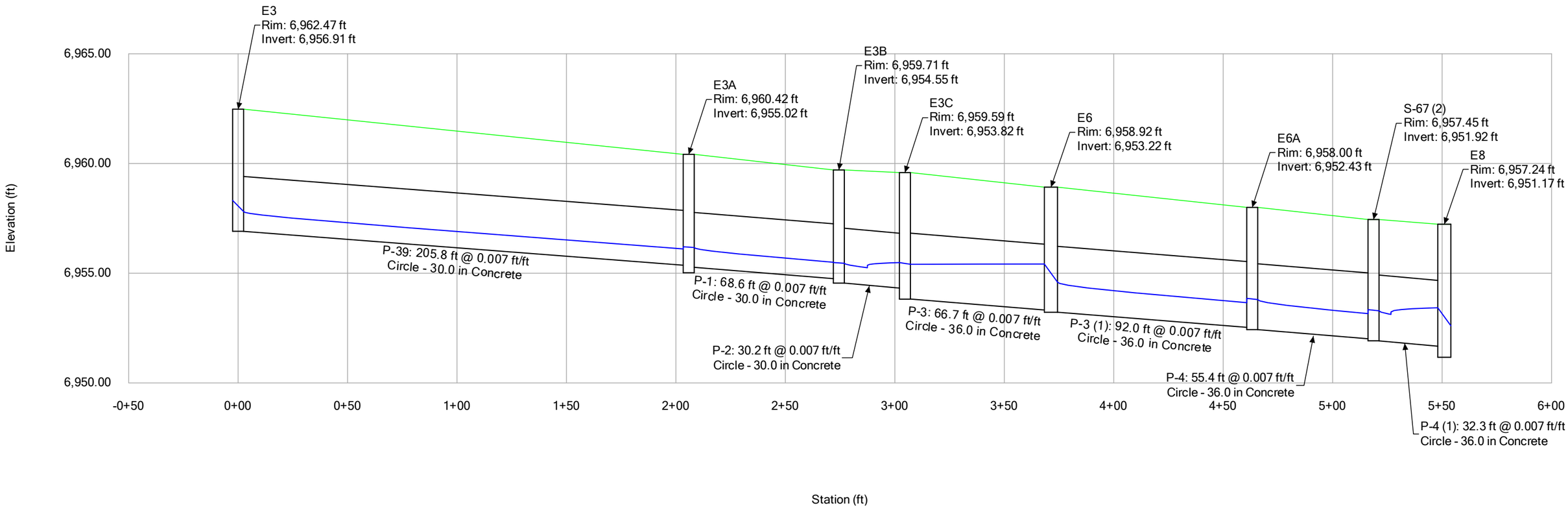
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - D4 to O-D1 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 5-YR Event



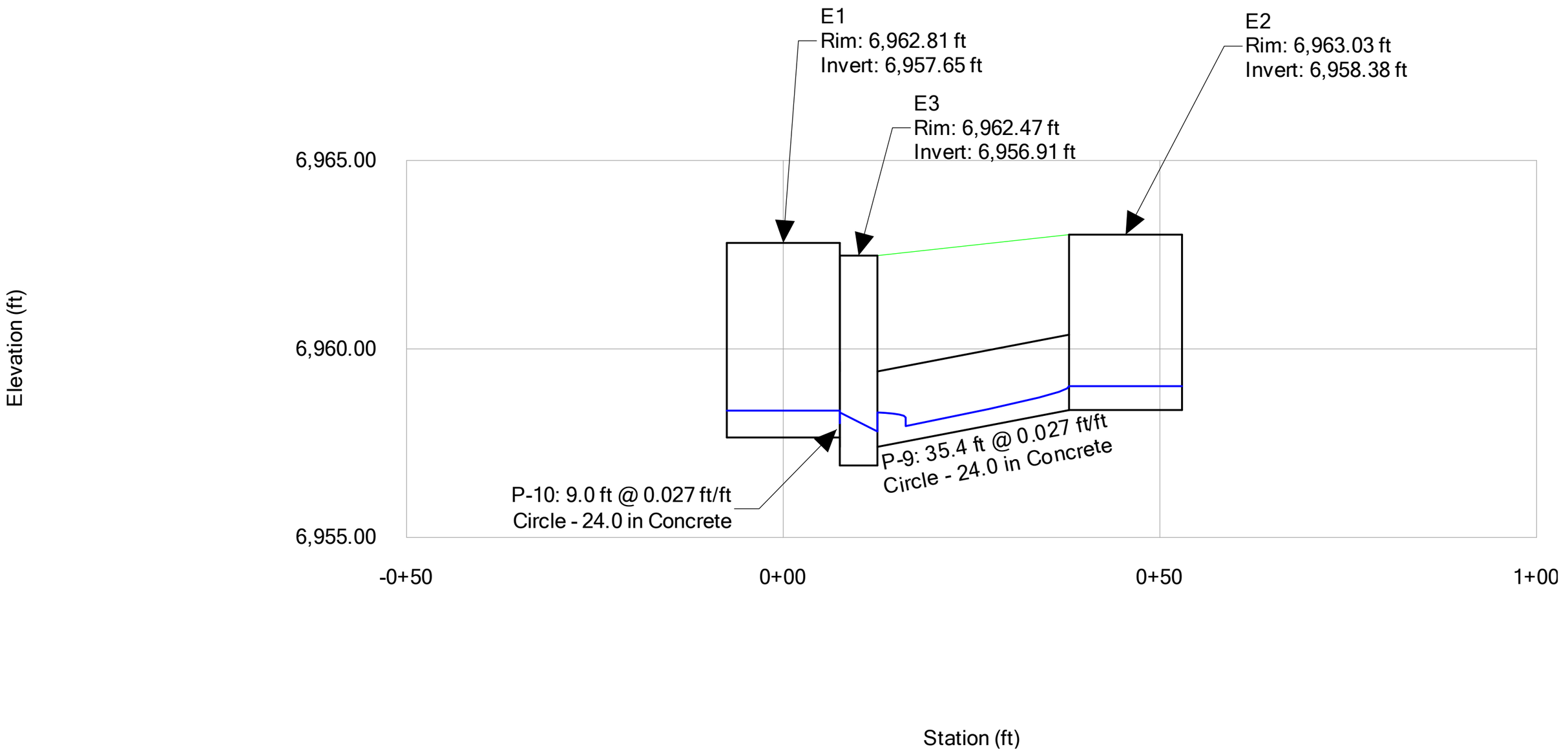
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - D7 to O-D2 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 5-YR Event



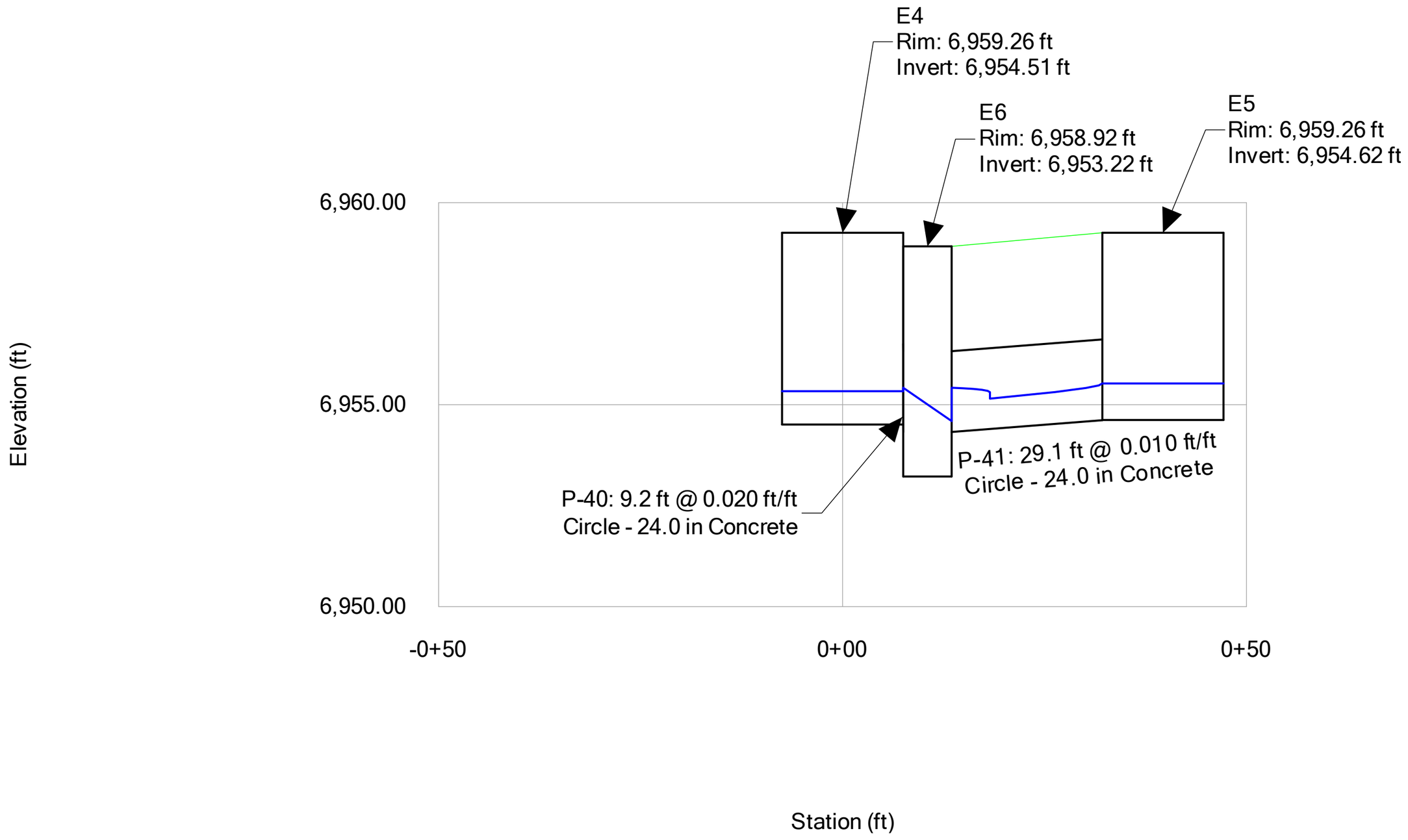
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - E3 to E8 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 5-YR Event



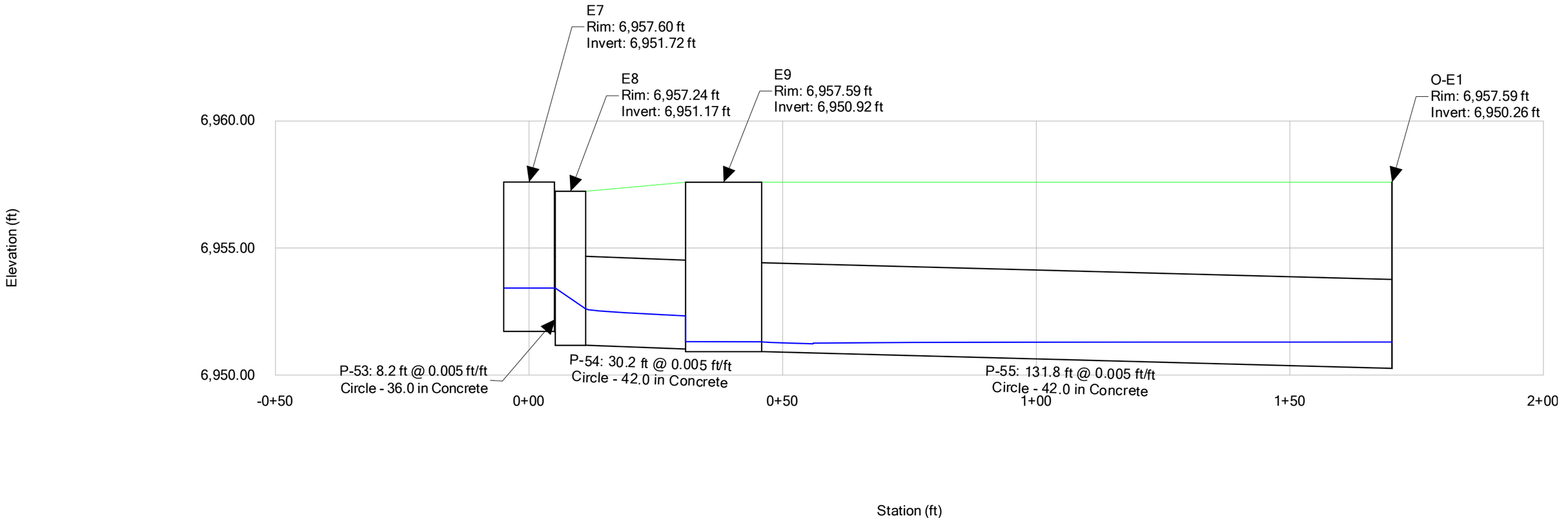
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - E1 to E2 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 5-YR Event



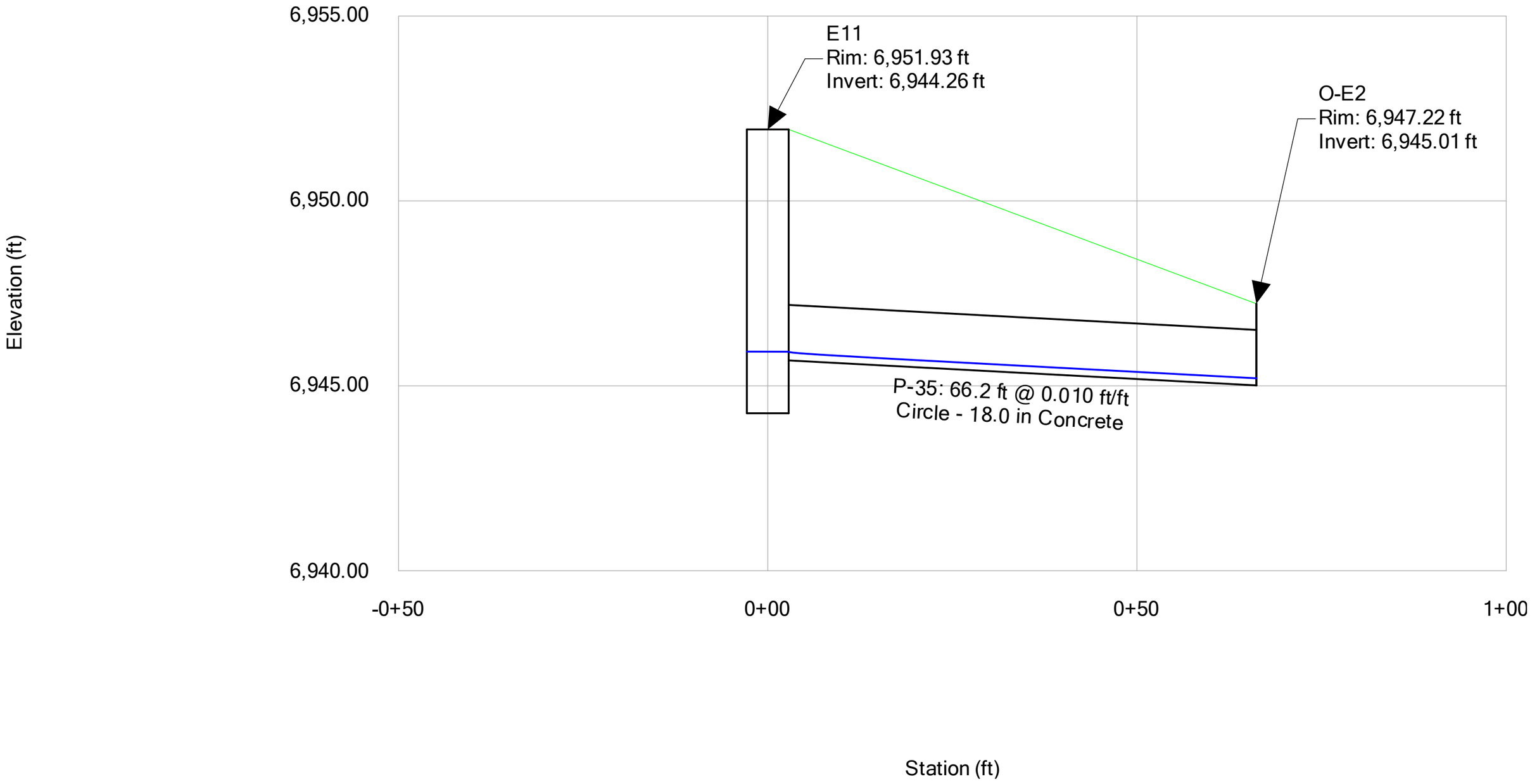
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - E4 to E5 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 5-YR Event



Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - E7 to O-E1 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 5-YR Event



Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - E10 to O-E2 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 5-YR Event



Grandview Reserve Filing No. 1

FlexTable: Conduit Table

Active Scenario: 100-YR Event

Label	Start Node	Stop Node	Diameter (in)	Material	Manning's n	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Invert (Start) (ft)	Invert (Stop) (ft)	Flow (cfs)	Velocity (ft/s)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)
P-1	E3A	E3B	30.0	Concrete	0.012	68.6	0.007	6,955.27	6,954.75	17.70	7.68	6,957.01	6,957.04
P-2	E3B	E3C	30.0	Concrete	0.012	30.2	0.007	6,954.55	6,954.32	17.70	7.68	6,957.02	6,956.97
P-3	E3C	E6	36.0	Concrete	0.012	66.7	0.007	6,953.82	6,953.32	17.70	2.50	6,956.85	6,956.81
P-3 (1)	E6	E6A	36.0	Concrete	0.012	92.0	0.007	6,953.22	6,952.53	41.10	9.44	6,955.31	6,955.27
P-4	E6A	S-67 (2)	36.0	Concrete	0.012	55.4	0.007	6,952.43	6,952.01	41.10	9.44	6,955.21	6,955.08
P-4 (1)	S-67 (2)	E8	36.0	Concrete	0.012	32.3	0.007	6,951.92	6,951.67	41.10	5.81	6,955.03	6,954.92
P-9	E2	E3	24.0	Concrete	0.012	35.4	0.027	6,958.38	6,957.40	7.80	9.98	6,959.37	6,959.22
P-10	E1	E3	24.0	Concrete	0.012	9.0	0.027	6,957.65	6,957.40	9.90	10.67	6,959.17	6,959.22
P-35	E11	O-E2	18.0	Concrete	0.012	66.2	0.010	6,945.69	6,945.01	14.90	8.43	6,947.60	6,946.42
P-36	D8	O-D2	18.0	Concrete	0.012	98.7	0.010	6,964.99	6,964.01	8.00	6.95	6,966.09	6,964.94
P-39	E3	E3A	30.0	Concrete	0.012	205.8	0.007	6,956.91	6,955.36	17.70	7.68	6,958.33	6,957.05
P-40	E4	E6	24.0	Concrete	0.012	9.2	0.020	6,954.51	6,954.32	11.70	3.72	6,956.83	6,956.81
P-41	E5	E6	24.0	Concrete	0.012	29.1	0.010	6,954.62	6,954.32	11.70	3.72	6,956.87	6,956.81
P-42	D3	S-54	24.0	Concrete	0.012	82.0	0.046	6,980.35	6,976.60	10.50	13.03	6,981.51	6,977.22
P-43	S-54	S-55	24.0	Concrete	0.012	89.0	0.010	6,976.31	6,975.42	10.50	7.50	6,977.47	6,976.34
P-44	S-55	S-56	24.0	Concrete	0.012	72.0	0.009	6,975.31	6,974.70	10.50	7.08	6,976.48	6,975.95
P-45	S-56	S-57	24.0	Concrete	0.012	81.0	0.009	6,974.60	6,973.91	10.50	7.08	6,975.76	6,975.16
P-46	S-57	S-58	24.0	Concrete	0.012	66.0	0.009	6,973.81	6,973.24	10.50	7.08	6,974.97	6,974.49
P-47	S-58	S-59	24.0	Concrete	0.012	83.5	0.009	6,973.14	6,972.43	10.50	7.08	6,974.30	6,973.68
P-48	S-59	D5	24.0	Concrete	0.012	47.0	0.009	6,972.33	6,971.92	10.50	7.08	6,973.49	6,973.50
P-49	D5	D6	36.0	Concrete	0.012	27.7	0.015	6,970.93	6,970.51	24.60	10.72	6,972.53	6,971.76
P-50	D3	D1	24.0	Concrete	0.012	9.2	-0.010	6,980.66	6,980.75	5.30	6.23	6,982.24	6,982.24
P-51	D3	D2	24.0	Concrete	0.012	29.2	-0.005	6,980.66	6,980.80	5.30	4.85	6,982.25	6,982.24
P-52	D6	S-67 (3)	36.0	Concrete	0.012	127.8	0.015	6,970.41	6,968.49	25.80	10.86	6,972.05	6,970.20
P-53	E7	E8	36.0	Concrete	0.012	8.2	0.005	6,951.72	6,951.67	11.00	1.56	6,954.92	6,954.92
P-54	E8	E9	42.0	Concrete	0.012	30.2	0.005	6,951.17	6,951.02	52.10	8.60	6,953.43	6,953.47
P-55	E9	O-E1	42.0	Concrete	0.012	131.8	0.005	6,950.92	6,950.26	63.10	8.94	6,953.41	6,952.67
P-56	D4	D5	24.0	Concrete	0.012	9.2	0.015	6,972.06	6,971.92	14.10	9.40	6,973.41	6,973.50
P-57 (1)	S-67 (3)	O-D1	36.0	Concrete	0.012	27.8	0.010	6,968.29	6,968.01	25.80	9.37	6,969.93	6,970.18

Grandview Reserve Filing No. 1

FlexTable: Manhole Table

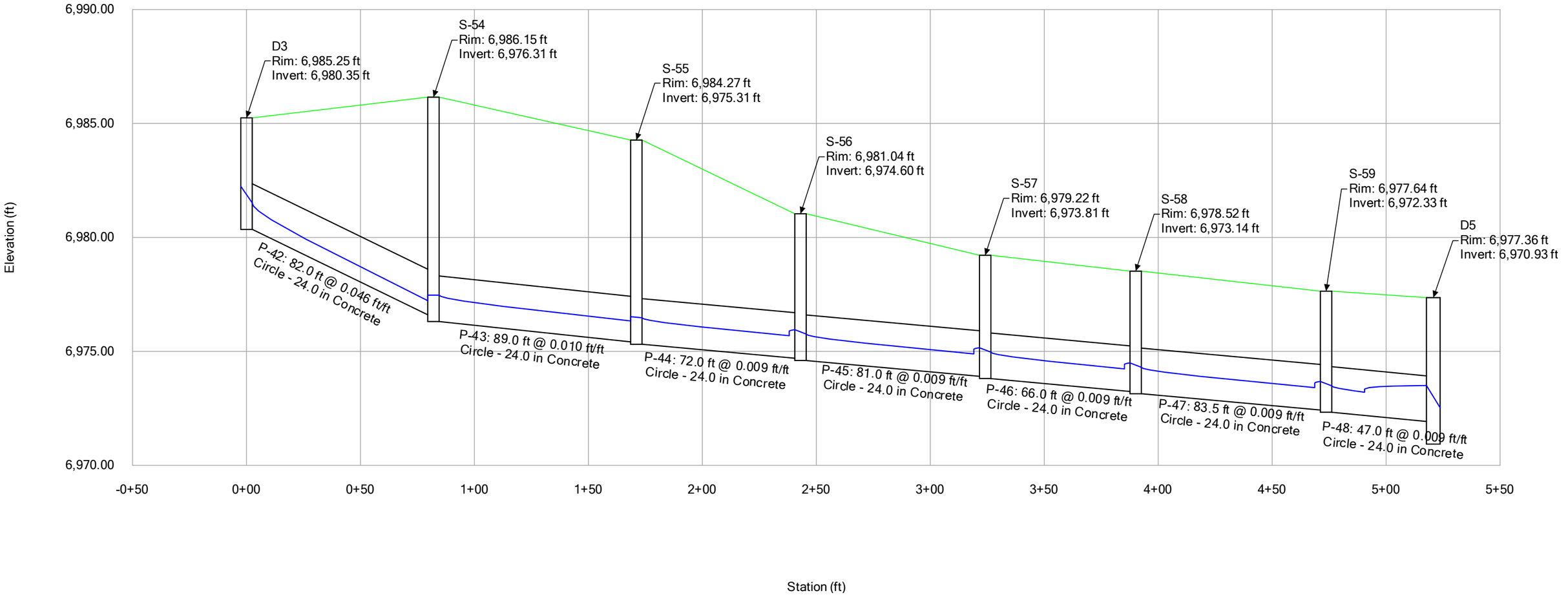
Active Scenario: 100-YR Event

Label	Notes	Elevation (Ground) (ft)	Elevation (Invert in 1) (ft)	Elevation (Invert Out) (ft)	Flow (Total Out) (cfs)	Headloss Method	Headloss Coefficient (Standard)	Hydraulic Grade Line (Out) (ft)	Hydraulic Grade Line (In) (ft)
D1	CDOT-TYPE R INLET (5')	6,985.58	(N/A)	6,980.75	5.30	Standard	0.000	6,982.24	6,982.24
D2	CDOT-TYPE R INLET (5')	6,985.58	(N/A)	6,980.80	5.30	Standard	0.000	6,982.25	6,982.25
D3	MH-ECCENTRIC (5' %%C)	6,985.25	6,980.66	6,980.35	10.50	Standard	1.520	6,981.51	6,982.24
D4	CDOT-TYPE R INLET (15') - (PUB)	6,977.69	(N/A)	6,972.06	14.10	Standard	0.000	6,973.41	6,973.41
D5	MH-ECCENTRIC (6' %%C)	6,977.36	6,971.92	6,970.93	24.60	Standard	1.520	6,972.53	6,973.50
D6	CDOT-TYPE R INLET (15') - (PUB)	6,977.69	6,970.51	6,970.41	25.80	Standard	0.050	6,972.05	6,972.08
D8	MODIFIED CDOT TYPE DPOND D OUTLET STRUCTURE(SEE GEC PLAN)	6,969.69	(N/A)	6,964.99	8.00	Standard	0.000	6,966.09	6,966.09
E1	CDOT-TYPE R INLET (15') - (PUB)	6,962.81	(N/A)	6,957.65	9.90	Standard	0.000	6,959.17	6,959.17
E2	CDOT-TYPE R INLET (15') - (PUB)	6,963.03	(N/A)	6,958.38	7.80	Standard	0.000	6,959.37	6,959.37
E3	5' %%C SDMH - (PUB)	6,962.47	6,957.40	6,956.91	17.70	Standard	1.520	6,958.33	6,959.22
E3A	5' %%C SDMH - (PUB)	6,960.42	6,955.36	6,955.27	17.70	Standard	0.100	6,957.01	6,957.05
E3B	5' %%C SDMH - (PUB)	6,959.71	6,954.75	6,954.55	17.70	Standard	0.100	6,957.02	6,957.04
E3C	5' %%C SDMH - (PUB)	6,959.59	6,954.32	6,953.82	17.70	Standard	1.320	6,956.85	6,956.97
E4	CDOT-TYPE R INLET (15') - (PUB)	6,959.26	(N/A)	6,954.51	11.70	Standard	0.000	6,956.83	6,956.83
E5	CDOT-TYPE R INLET (15') - (PUB)	6,959.26	(N/A)	6,954.62	11.70	Standard	0.000	6,956.87	6,956.87
E6	6' %%C SDMH - (PUB)	6,958.92	6,953.32	6,953.22	41.10	Standard	1.570	6,955.31	6,956.81
E6A	5' %%C SDMH - (PUB)	6,958.00	6,952.53	6,952.43	41.10	Standard	0.100	6,955.21	6,955.27
E7	CDOT-TYPE R INLET (10')	6,957.60	(N/A)	6,951.72	11.00	Standard	0.000	6,954.92	6,954.92
E8	MH-ECCENTRIC (6' %%C)	6,957.24	6,951.67	6,951.17	52.10	Standard	1.520	6,953.43	6,954.92
E9	CDOT-TYPE R INLET (15')	6,957.59	6,951.02	6,950.92	63.10	Standard	0.050	6,953.41	6,953.47
E11	MODIFIED CDOT TYPE DPOND E OUTLET STRUCTURE(SEE GEC PLAN)	6,951.93	(N/A)	6,945.69	14.90	Standard	0.050	6,947.60	6,947.66
S-54	MH-ECCENTRIC (5' %%C)	6,986.15	6,976.60	6,976.31	10.50	Standard	0.000	6,977.47	6,977.47
S-55	MH-ECCENTRIC (5' %%C)	6,984.27	6,975.42	6,975.31	10.50	Standard	0.100	6,976.48	6,976.52
S-56	MH-ECCENTRIC (5' %%C)	6,981.04	6,974.70	6,974.60	10.50	Standard	0.400	6,975.76	6,975.95
S-57	MH-ECCENTRIC (5' %%C)	6,979.22	6,973.91	6,973.81	10.50	Standard	0.400	6,974.97	6,975.16
S-58	MH-ECCENTRIC (5' %%C)	6,978.52	6,973.24	6,973.14	10.50	Standard	0.400	6,974.30	6,974.49
S-59	MH-ECCENTRIC (5' %%C)	6,977.64	6,972.43	6,972.33	10.50	Standard	0.400	6,973.49	6,973.68
S-67 (2)	MH-ECCENTRIC (5' %%C)	6,957.45	6,952.01	6,951.92	41.10	Standard	0.100	6,955.03	6,955.08
S-67 (3)	MH-ECCENTRIC (5' %%C)	6,973.84	6,968.49	6,968.29	25.80	Standard	0.400	6,969.93	6,970.20

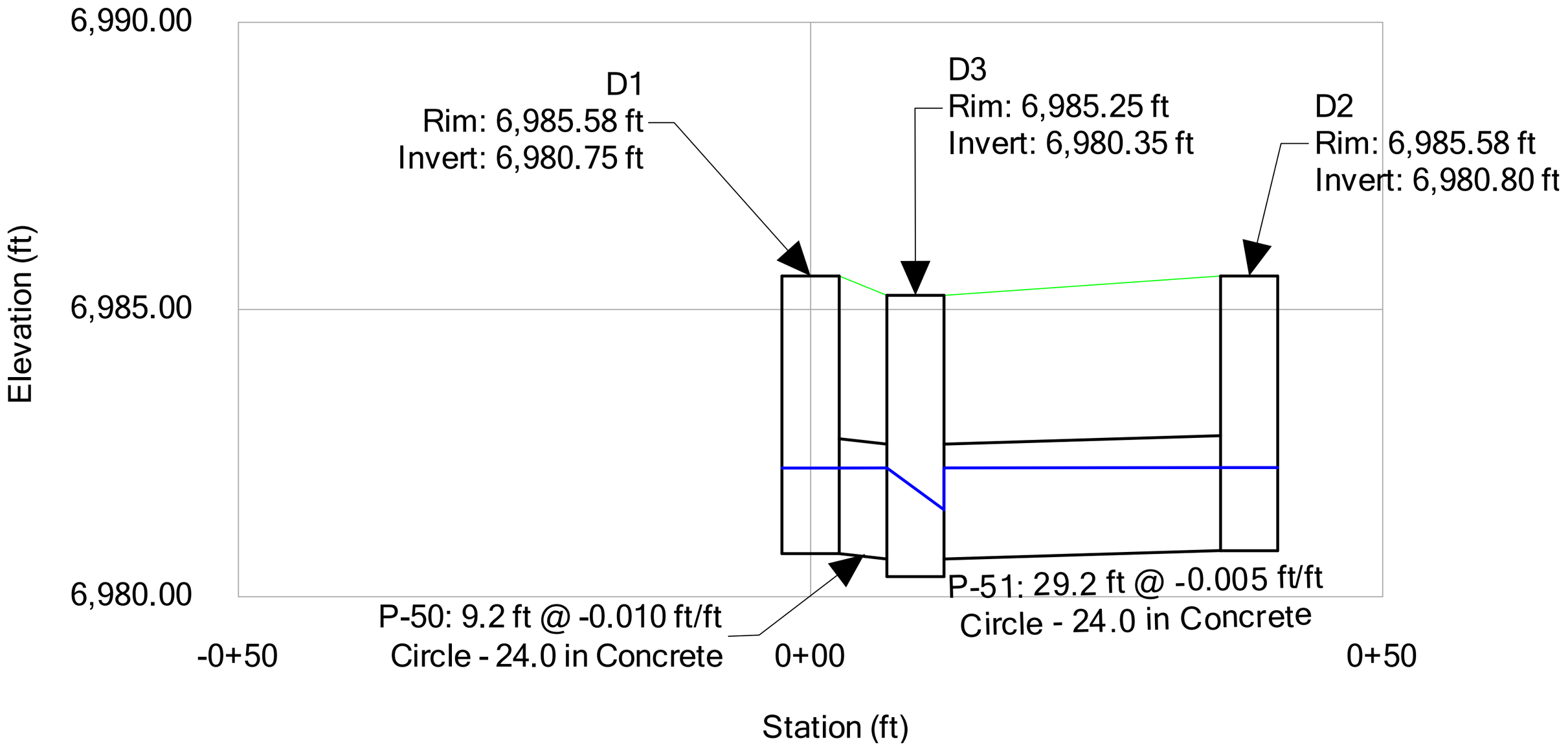
Grandview Reserve Filing No. 1
FlexTable: Outfall Table
Active Scenario: 100-YR Event

Label	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Boundary Condition Type	Elevation (User Defined Tailwater) (ft)	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
O-D2	6,966.22	6,964.01	Free Outfall		6,964.94	8.00
O-E2	6,947.22	6,945.01	Free Outfall		6,946.42	14.90
O-D1	6,973.84	6,968.01	User Defined Tailwater	6,970.18	6,970.18	25.80
O-E1	6,957.59	6,950.26	User Defined Tailwater	6,952.60	6,952.67	63.10

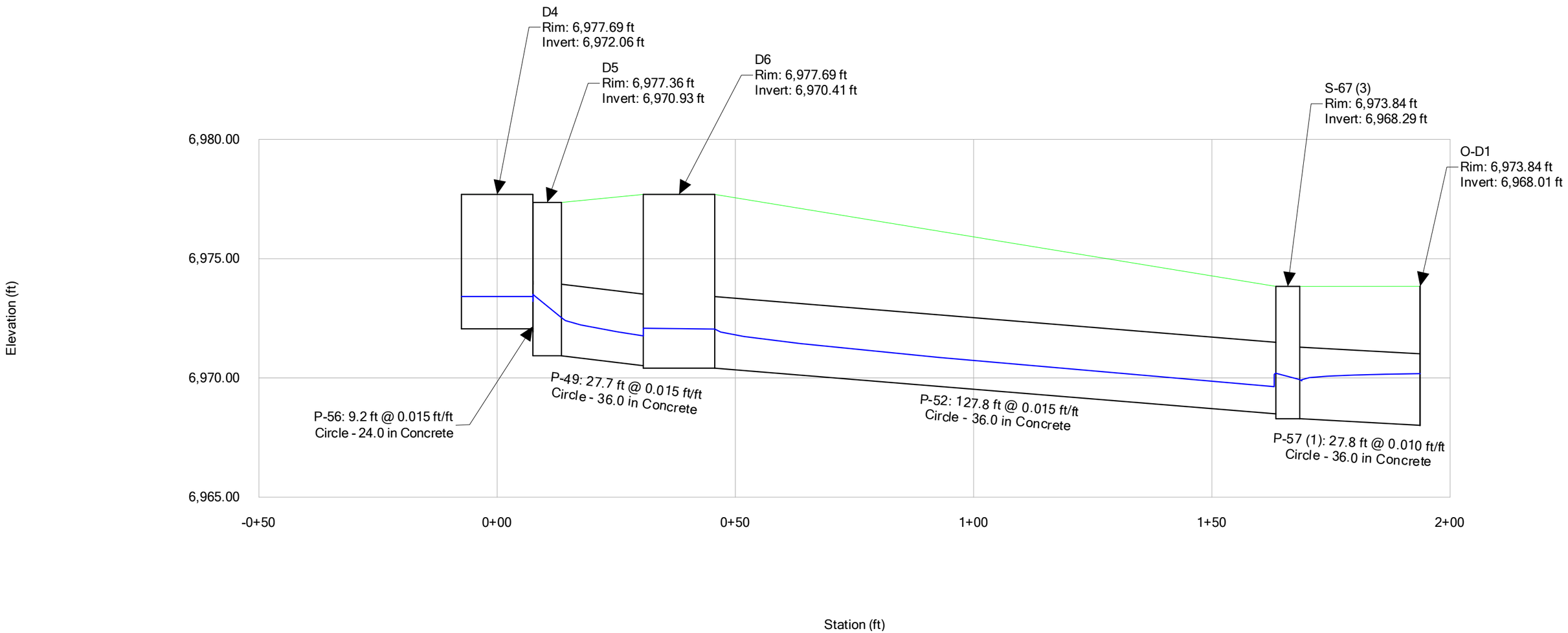
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - D3 to D5 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 100-YR Event



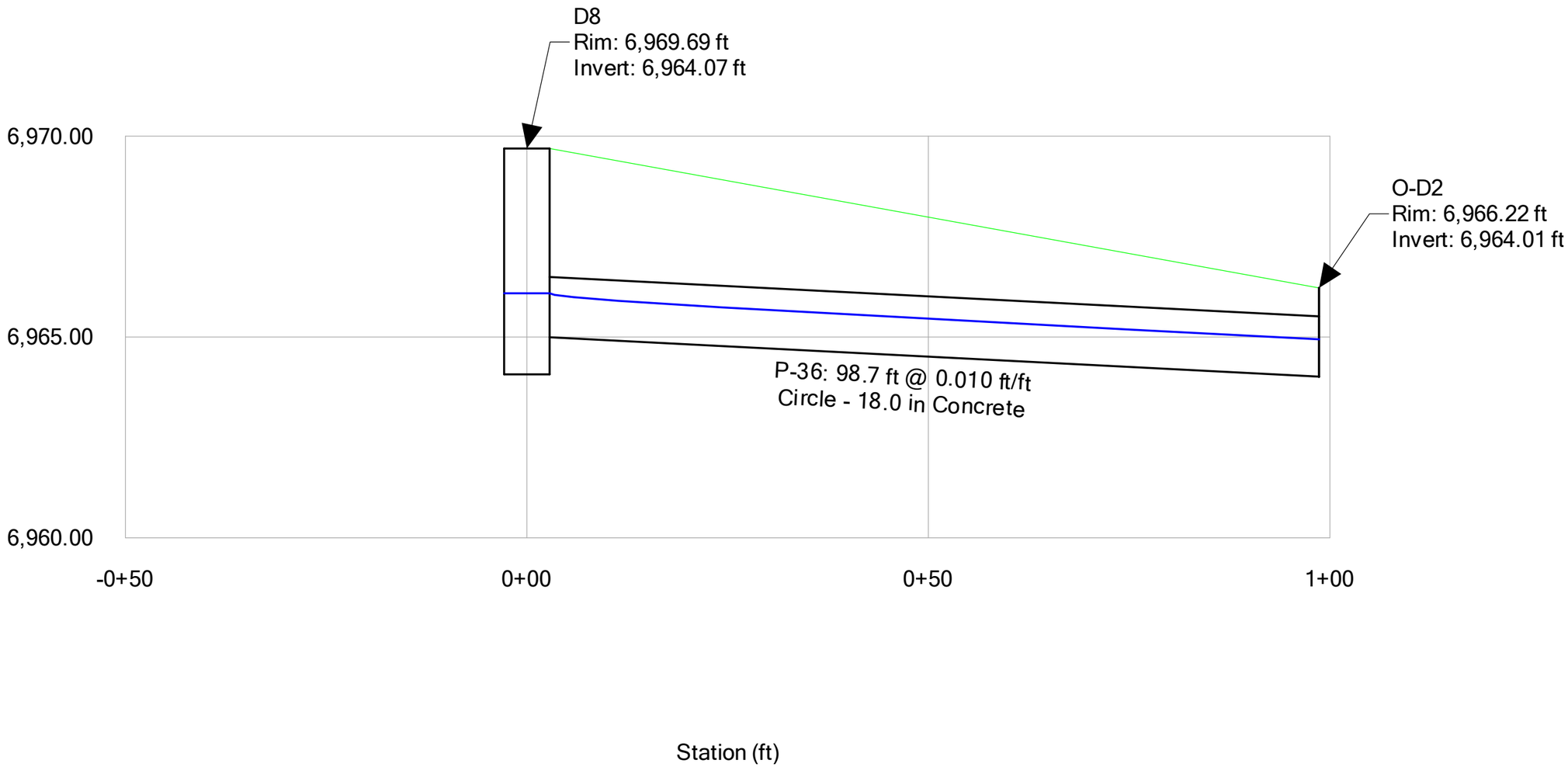
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - D1 to D2 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 100-YR Event



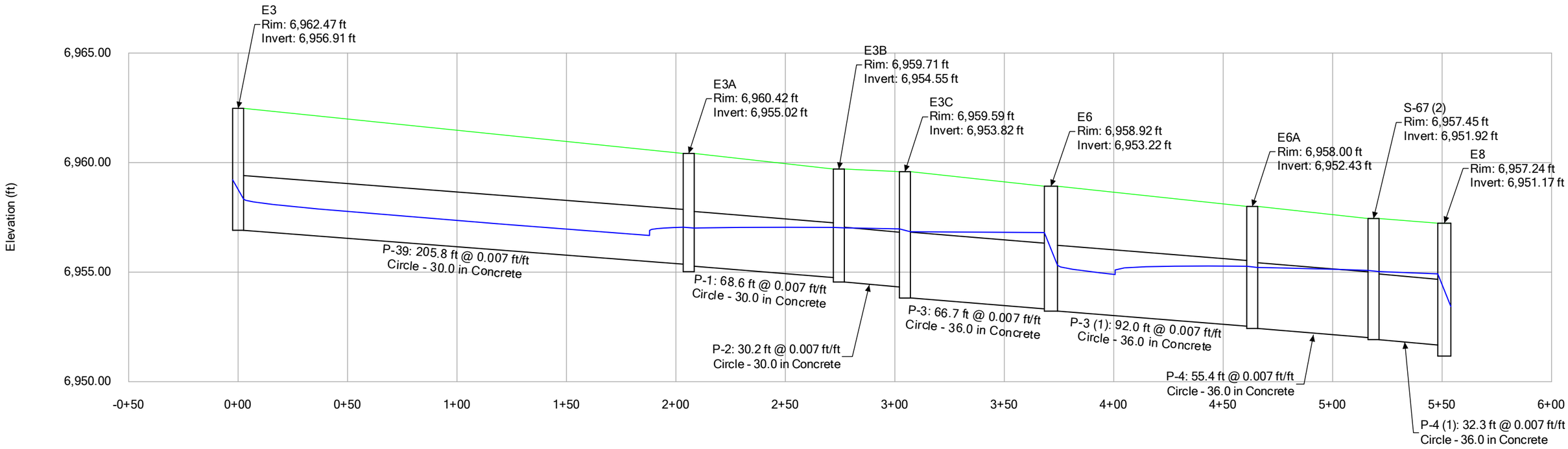
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - D4 to O-D1 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 100-YR Event



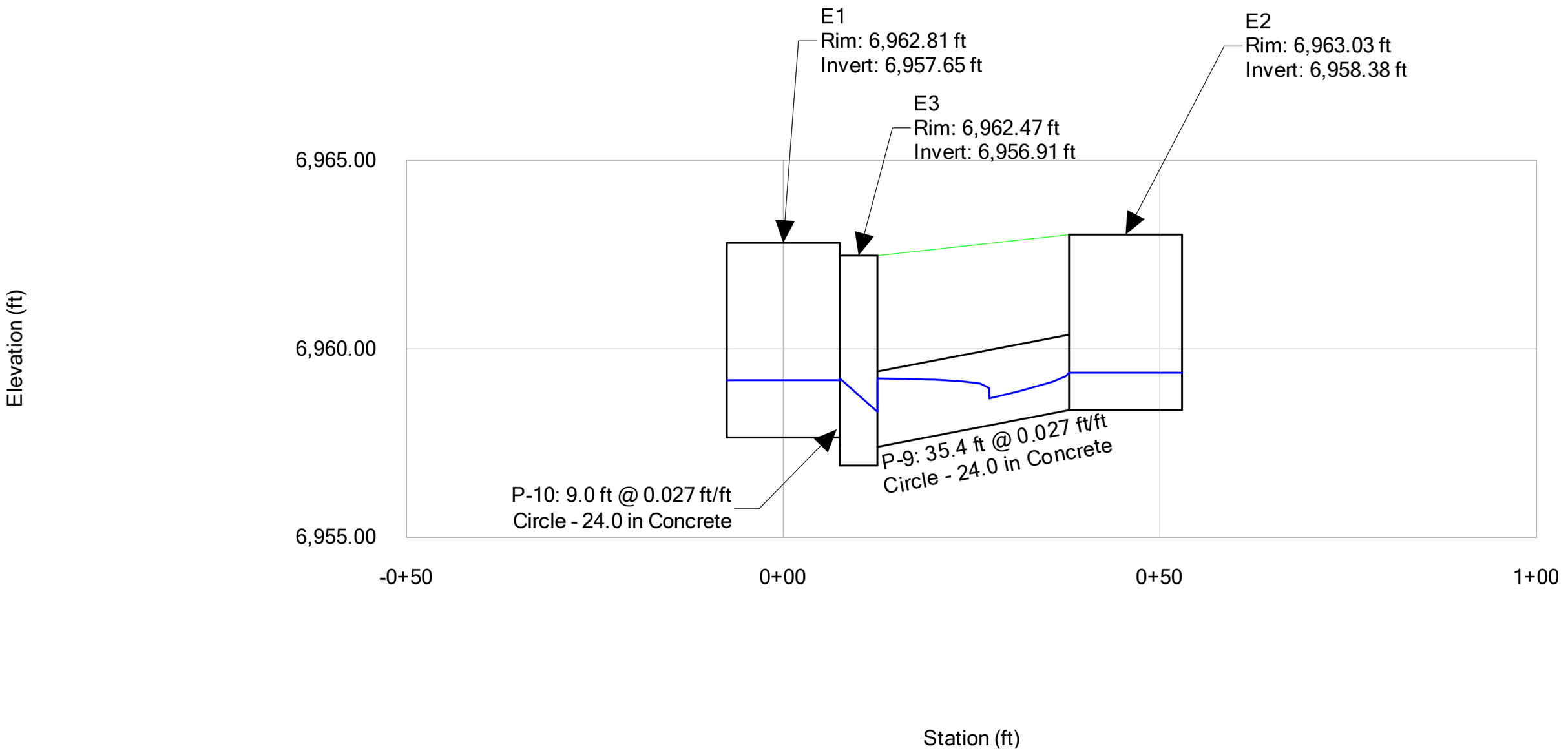
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - D7 to O-D2 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 100-YR Event



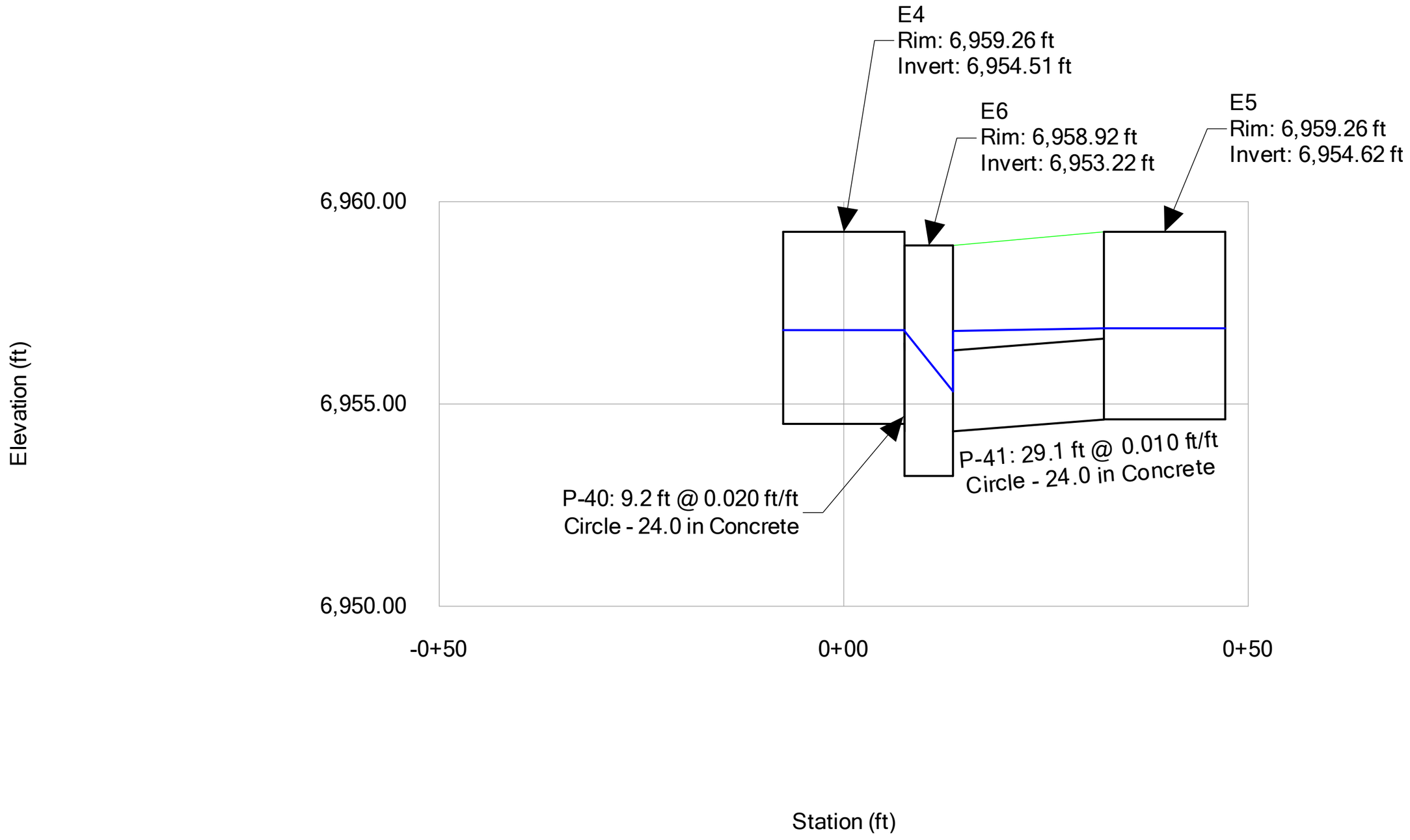
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - E3 to E8 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 100-YR Event



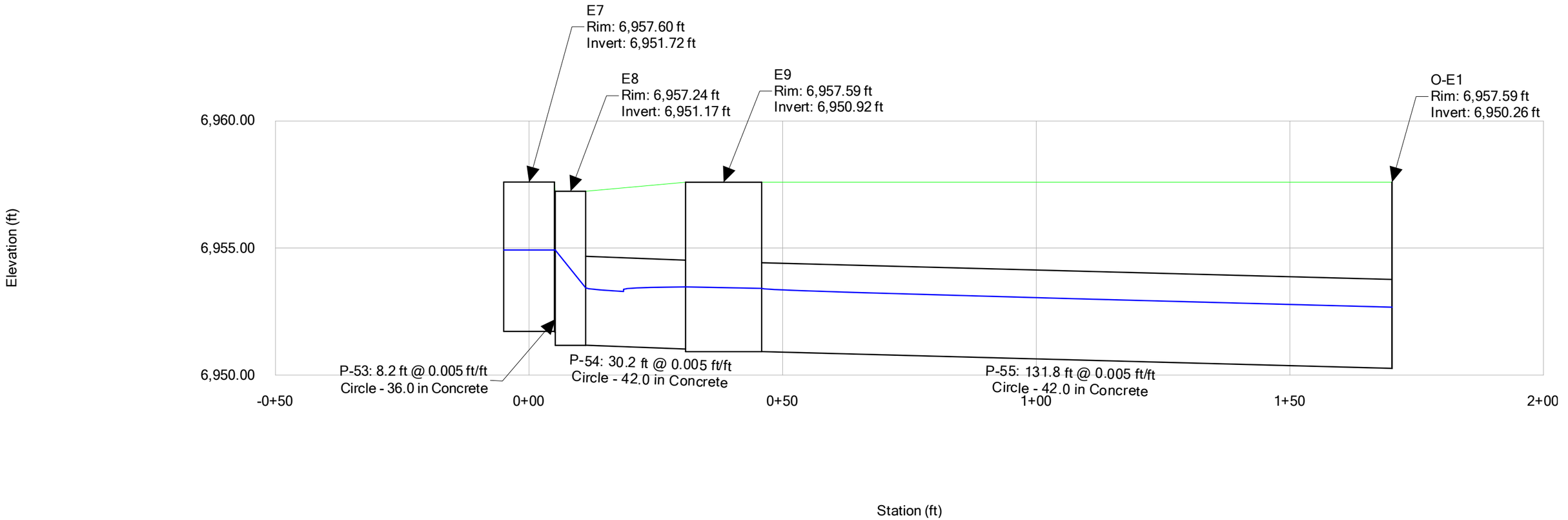
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - E1 to E2 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 100-YR Event



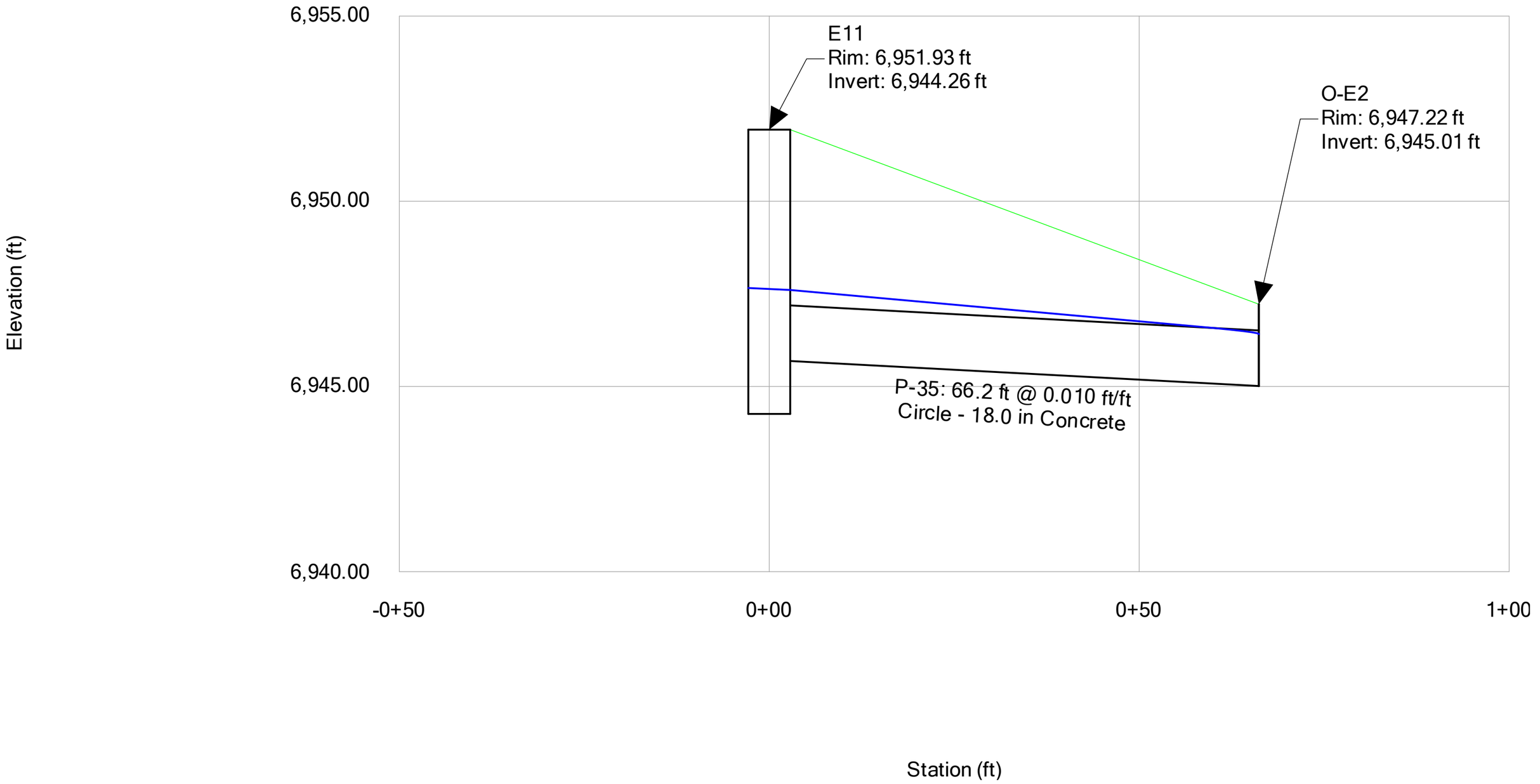
Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - E4 to E5 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 100-YR Event



Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - E7 to O-E1 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 100-YR Event



Grandview Reserve Filing No. 1
Profile Report
Engineering Profile - E10 to O-E2 (HRG02_FDR Storm Analysis.stsw)
Active Scenario: 100-YR Event



APPENDIX E

Water Quality Computations

DETENTION POND TRIBUTARY AREAS

Subdivision: Grandview Reserve Filing No. 1
Location: CO, Falcon (El Paso County)

Project Name: Grandview Reserve Filing No. 1
Project No.: HRG02.20
Calculated By: TJE
Checked By: BAS
Date: 3/14/24

Pond 'D'

Basin	Area	% Imp
D-1	2.73	46.6
D-2	0.57	65.0
D-3	4.33	59.8
D-4	3.65	57.8
D-5	1.59	22.6
Total	**12.87**	**52.1**

Pond 'E'

Basin	Area	% Imp
E-1	4.47	47.3
E-2	1.94	65.0
E-3a	2.90	65.0
E-3b	2.12	65.0
E-4a	7.45	48.7
E-4b	1.00	65.0
E-5	1.43	13.1
Total	**21.31**	**52.1**

Galloway

Design Procedure Form: Runoff Reduction

UD-BMP (Version 3.07, March 2018)

Designer: TJE
Company: Galloway
Date: March 15, 2024
Project: Grandview Reserve Filing No. 1
Location: Falcon, CO -> D Basins

SITE INFORMATION (User Input in Blue Cells)

WQCV Rainfall Depth: 0.60 inches
Depth of Average Runoff Producing Storm, d_6 = 0.43 inches (for Watersheds Outside of the Denver Region, Figure 3-1 in USDCM Vol. 3)

Area Type	DCIA	SPA		UIA:RPA	SPA							
Area ID				Basin D-6	Basin D 6							
Downstream Design Point ID	Pond D	Pond D		Offsite	Offsite							
Downstream BMP Type	EDB	EDB		None	None							
DCIA (ft^2)	292,082	--		--	--							
UIA (ft^2)	--	--		5,450	--							
RPA (ft^2)	--	--		2,592	--							
SPA (ft^2)	--	268,535		--	32,033							
HSG A (%)	--	100%		100%	100%							
HSG B (%)	--	0%		0%	0%							
HSG C/D (%)	--	0%		0%	0%							
Average Slope of RPA (ft/ft)	--	--		0.200	--							
UIA:RPA Interface Width (ft)	--	--		180.00	--							

CALCULATED RUNOFF RESULTS

Area ID				Basin D-6	Basin D 6							
UIA:RPA Area (ft^2)	--	--		8,042	--							
L / W Ratio	--	--		0.25	--							
UIA / Area	--	--		0.6777	--							
Runoff (in)	0.50	0.00		0.00	0.00							
Runoff (ft^3)	12170	0		0	0							
Runoff Reduction (ft^3)	0	13427		227	1602							

CALCULATED WQCV RESULTS

Area ID				Basin D-6	Basin D 6							
WQCV (ft^3)	12170	0		227	0							
WQCV Reduction (ft^3)	0	0		227	0							
WQCV Reduction (%)	0%	0%		100%	0%							
Untreated WQCV (ft^3)	12170	0		0	0							

CALCULATED DESIGN POINT RESULTS (sums results from all columns with the same Downstream Design Point ID)

Downstream Design Point ID	Pond D	Offsite										
DCIA (ft^2)	292,082	0										
UIA (ft^2)	0	5,450										
RPA (ft^2)	0	2,592										
SPA (ft^2)	268,535	32,033										
Total Area (ft^2)	560,617	40,075										
Total Impervious Area (ft^2)	292,082	5,450										
WQCV (ft^3)	12,170	227										
WQCV Reduction (ft^3)	0	227										
WQCV Reduction (%)	0%	100%										
Untreated WQCV (ft^3)	12,170	0										

CALCULATED SITE RESULTS (sums results from all columns in worksheet)

Total Area (ft^2)	600,692
Total Impervious Area (ft^2)	297,532
WQCV (ft^3)	12,397
WQCV Reduction (ft^3)	227
WQCV Reduction (%)	2%
Untreated WQCV (ft^3)	12,170

Design Procedure Form: Runoff Reduction

UD-BMP (Version 3.07, March 2018)

Designer:
Company:
Date: March 15, 2024
Project:
Location:

SITE INFORMATION (User Input in Blue Cells)

WQCV Rainfall Depth: 0.60 inches
Depth of Average Runoff Producing Storm, d_6 = 0.43 inches (for Watersheds Outside of the Denver Region, Figure 3-1 in USDCM Vol. 3)

Area Type	DCIA	SPA		UIA:RPA	SPA							
Area ID				Basin E-6	Basin E 6							
Downstream Design Point ID	Pond E	Pond E		Offsite	Offsite							
Downstream BMP Type	EDB	EDB		None	None							
DCIA (ft^2)	483,625	--		--	--							
UIA (ft^2)	--	--		16,936	--							
RPA (ft^2)	--	--		2,823	--							
SPA (ft^2)	--	444,639		--	84,785							
HSG A (%)	--	100%		100%	100%							
HSG B (%)	--	0%		0%	0%							
HSG C/D (%)	--	0%		0%	0%							
Average Slope of RPA (ft/ft)	--	--		0.200	--							
UIA:RPA Interface Width (ft)	--	--		70.00	--							

CALCULATED RUNOFF RESULTS

Area ID				Basin E-6	Basin E 6							
UIA:RPA Area (ft^2)	--	--		19,759	--							
L / W Ratio	--	--		4.03	--							
UIA / Area	--	--		0.8571	--							
Runoff (in)	0.50	0.00		0.11	0.00							
Runoff (ft^3)	20151	0		179	0							
Runoff Reduction (ft^3)	0	22232		527	4239							

CALCULATED WQCV RESULTS

Area ID				Basin E-6	Basin E 6							
WQCV (ft^3)	20151	0		706	0							
WQCV Reduction (ft^3)	0	0		527	0							
WQCV Reduction (%)	0%	0%		75%	0%							
Untreated WQCV (ft^3)	20151	0		179	0							

CALCULATED DESIGN POINT RESULTS (sums results from all columns with the same Downstream Design Point ID)

Downstream Design Point ID	Pond E	Offsite										
DCIA (ft^2)	483,625	0										
UIA (ft^2)	0	16,936										
RPA (ft^2)	0	2,823										
SPA (ft^2)	444,639	84,785										
Total Area (ft^2)	928,264	104,544										
Total Impervious Area (ft^2)	483,625	16,936										
WQCV (ft^3)	20,151	706										
WQCV Reduction (ft^3)	0	527										
WQCV Reduction (%)	0%	75%										
Untreated WQCV (ft^3)	20,151	179										

CALCULATED SITE RESULTS (sums results from all columns in worksheet)

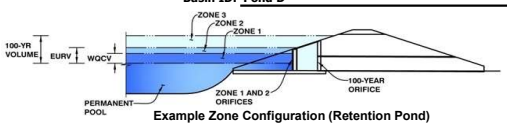
Total Area (ft^2)	1,032,808
Total Impervious Area (ft^2)	500,561
WQCV (ft^3)	20,857
WQCV Reduction (ft^3)	527
WQCV Reduction (%)	3%
Untreated WQCV (ft^3)	20,330

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: **Grandview Reserve Filing No. 1 - Final Drainage Report**

Basin ID: **Pond D**



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	**EDB**	
Watershed Area =	12.87	acres
Watershed Length =	900	ft
Watershed Length to Centroid =	450	ft
Watershed Slope =	0.020	ft/ft
Watershed Imperviousness =	52.10%	percent
Percentage Hydrologic Soil Group A =	100.0%	percent
Percentage Hydrologic Soil Group B =	0.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

		Optional User Overrides	
Water Quality Capture Volume (WQCV) =	0.227 acre-feet		acre-feet
Excess Urban Runoff Volume (EURV) =	0.782 acre-feet		acre-feet
2-yr Runoff Volume (P1 = 0.93 in.) =	0.430 acre-feet	0.93	inches
5-yr Runoff Volume (P1 = 1.21 in.) =	0.575 acre-feet	1.21	inches
10-yr Runoff Volume (P1 = 1.46 in.) =	0.724 acre-feet	1.46	inches
25-yr Runoff Volume (P1 = 1.84 in.) =	0.987 acre-feet	1.84	inches
50-yr Runoff Volume (P1 = 2.16 in.) =	1.266 acre-feet	2.16	inches
100-yr Runoff Volume (P1 = 2.49 in.) =	1.596 acre-feet	2.49	inches
500-yr Runoff Volume (P1 = 3.37 in.) =	2.464 acre-feet	3.37	inches
Approximate 2-yr Detention Volume =	0.395 acre-feet		
Approximate 5-yr Detention Volume =	0.534 acre-feet		
Approximate 10-yr Detention Volume =	0.673 acre-feet		
Approximate 25-yr Detention Volume =	0.906 acre-feet		
Approximate 50-yr Detention Volume =	1.053 acre-feet		
Approximate 100-yr Detention Volume =	1.215 acre-feet		

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.227	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.555	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	0.433	acre-feet
Total Detention Basin Volume =	1.215	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	

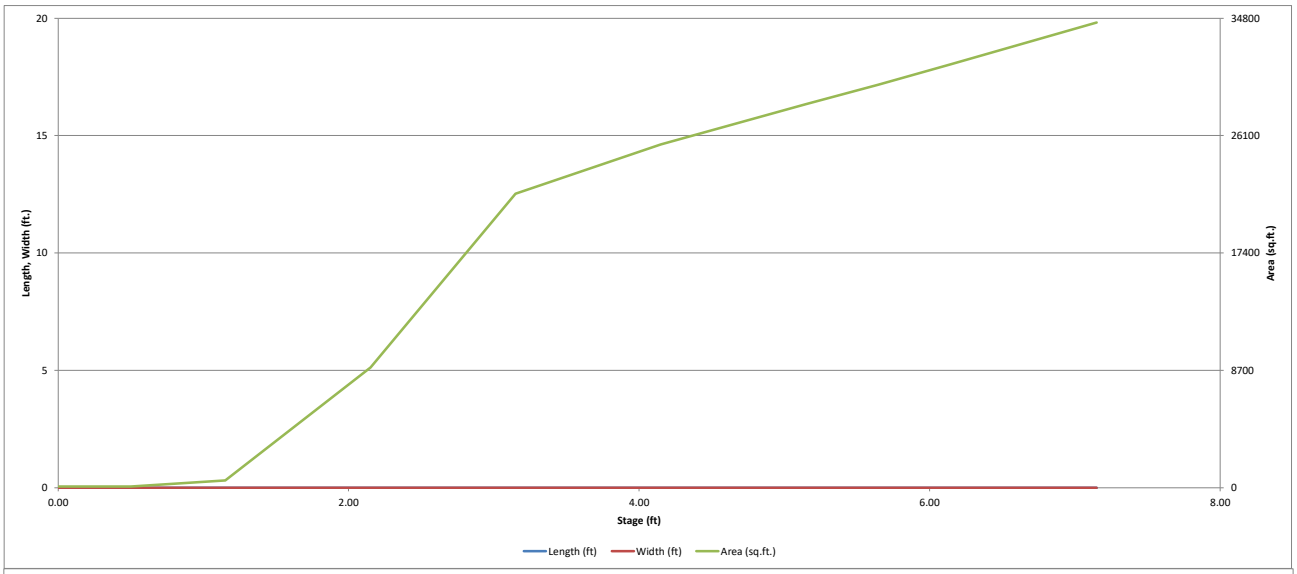
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

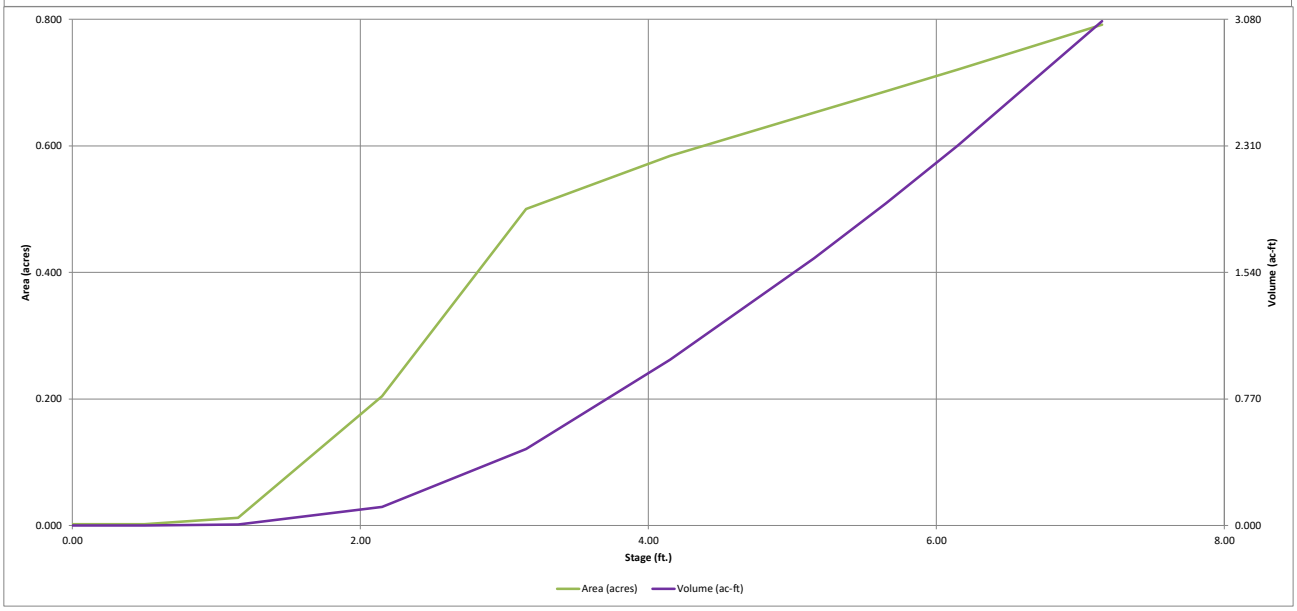
Depth Increment = ft

	Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
6965.85	**Top of Micropool**	--	0.00	--	--	--	90	0.002		
6966.35	**Trickel Channel Inv**	--	0.50	--	--	--	90	0.002	45	0.001
	6967	--	1.15	--	--	--	532	0.012	247	0.006
	6968	--	2.15	--	--	--	8,902	0.204	4,964	0.114
	6969	--	3.15	--	--	--	21,787	0.500	20,308	0.466
	6970	--	4.15	--	--	--	25,444	0.584	43,924	1.008
	6971	--	5.15	--	--	--	28,426	0.653	70,859	1.627
6971.50	**Spillway Invert**	--	5.65	--	--	--	29,894	0.686	85,439	1.961
	6972	--	6.15	--	--	--	31,395	0.721	100,761	2.313
6973.00	**Top of Pond**	--	7.15	--	--	--	34,492	0.792	133,705	3.069

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)



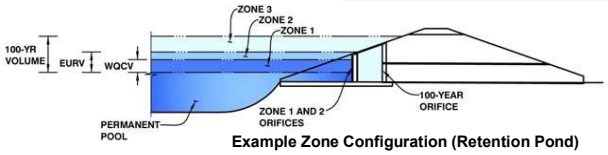


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: **Grandview Reserve Filing No. 1 - Final Drainage Report**

Basin ID: **Pond D**



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.58	0.227	Orifice Plate
Zone 2 (EURV)	3.76	0.555	Rectangular Orifice
Zone 3 (100-year)	4.50	0.433	Weir&Pipe (Restrict)
Total (all zones)		1.215	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

Calculated Parameters for Underdrain

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	5.347E-03	ft^2
Depth at top of Zone using Orifice Plate =	2.58	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	N/A	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	0.77	sq. inches (diameter = 1 inch)	Elliptical Slot Area =	N/A	ft^2

Calculated Parameters for Plate

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.00	2.00					
Orifice Area (sq. inches)	0.77	0.77	0.77					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Rectangular	Not Selected			Zone 2 Rectangular	Not Selected	
Invert of Vertical Orifice =	2.58	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	0.03	N/A	ft^2
Depth at top of Zone using Vertical Orifice =	3.76	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	0.08	N/A	feet
Vertical Orifice Height =	2.00	N/A	inches				
Vertical Orifice Width =	2.25		inches				

Calculated Parameters for Vertical Orifice

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected			Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	3.83	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	3.83	N/A	feet
Overflow Weir Front Edge Length =	6.00	N/A	feet	Overflow Weir Slope Length =	3.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V	Grate Open Area / 100-yr Orifice Area =	16.51	N/A	
Horiz. Length of Weir Sides =	3.00	N/A	feet	Overflow Grate Open Area w/o Debris =	12.53	N/A	ft^2
Overflow Grate Type =	Type C Grate	N/A		Overflow Grate Open Area w/ Debris =	6.26	N/A	ft^2
Debris Clogging % =	50%	N/A	%				

Calculated Parameters for Overflow Weir

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected			Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.87	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	0.76	N/A	ft^2
Outlet Pipe Diameter =	18.00	N/A	inches	Outlet Orifice Centroid =	0.39	N/A	feet
Restrictor Plate Height Above Pipe Invert =	8.00		inches	Half-Central Angle of Restrictor Plate on Pipe =	1.46	N/A	radians

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	5.65	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.33	feet
Spillway Crest Length =	50.00	feet	Stage at Top of Freeboard =	6.98	feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	0.78	acres
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =	2.94	acre-ft

Calculated Parameters for Spillway

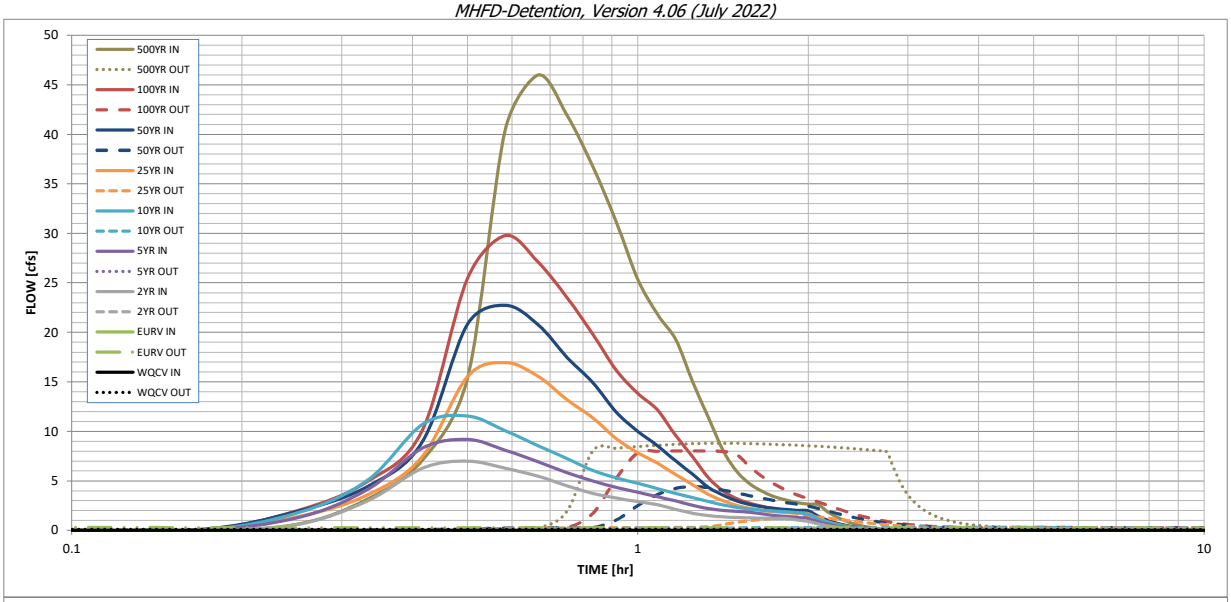
Routed Hydrograph Results

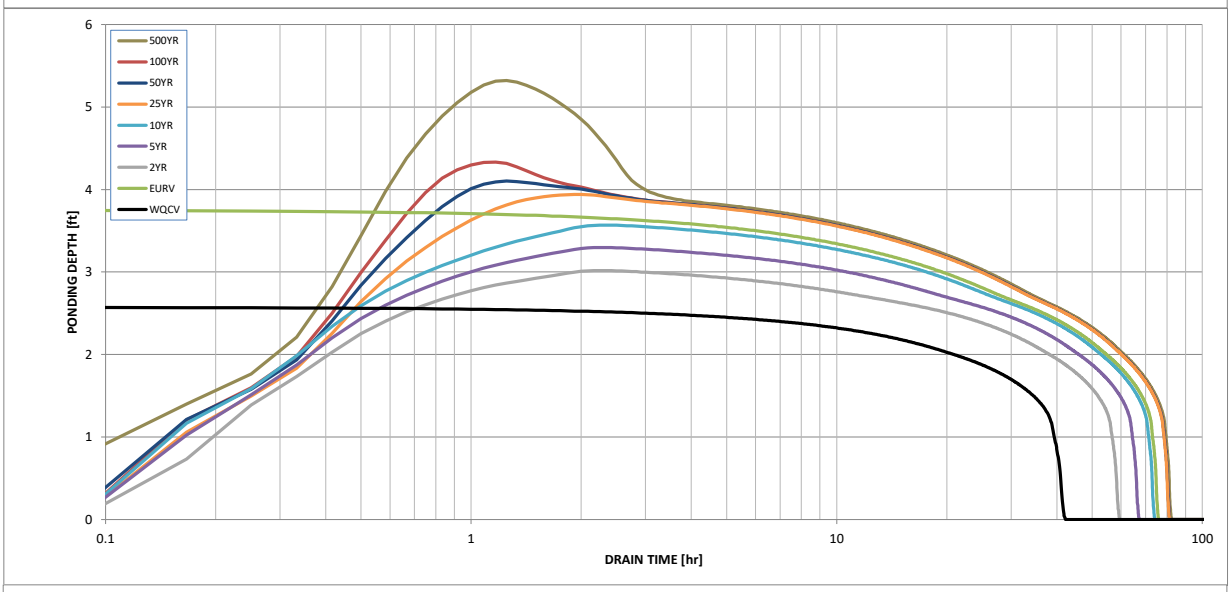
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

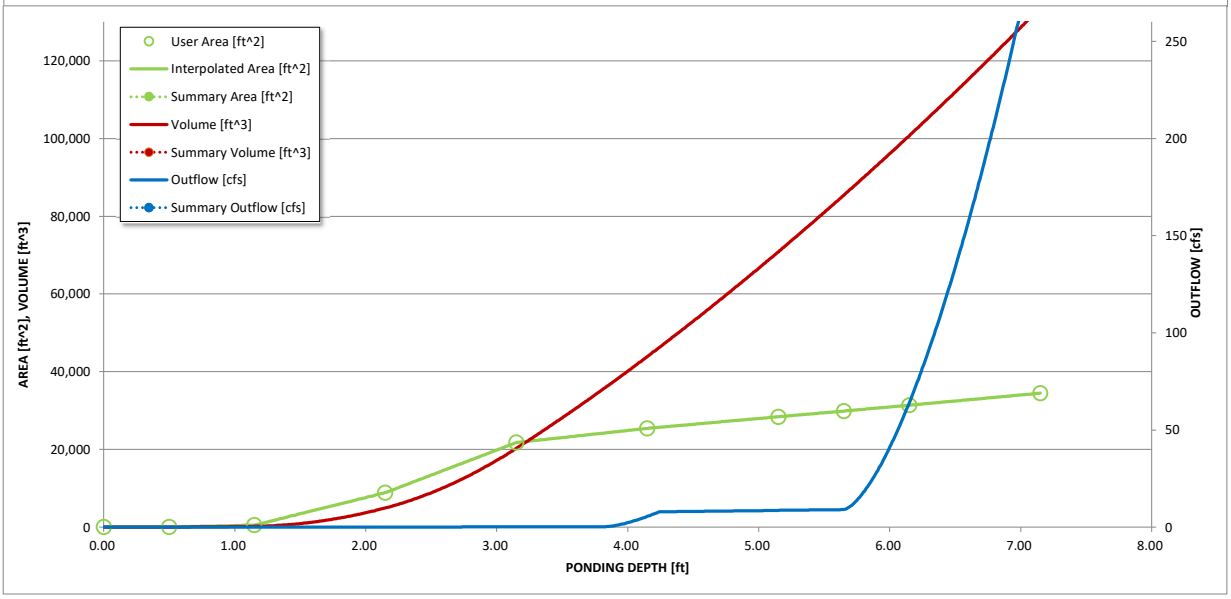
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	0.93	1.21	1.46	1.84	2.16	2.49	3.37
CUHP Runoff Volume (acre-ft) =	0.227	0.782	0.430	0.575	0.724	0.987	1.266	1.596	2.464
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.430	0.575	0.724	0.987	1.266	1.596	2.464
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.0	0.1	0.2	1.1	4.5	8.6	18.7
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.00	0.01	0.01	0.08	0.35	0.67	1.46
Peak Inflow Q (cfs) =	N/A	N/A	7.0	9.2	11.6	17.0	22.7	29.8	46.0
Peak Outflow Q (cfs) =	0.1	0.3	0.2	0.2	0.3	1.4	4.4	8.0	8.8
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	2.3	1.4	1.3	1.0	0.9	0.5
Structure Controlling Flow =	Plate	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Outlet Plate 1
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	0.1	0.3	0.6	0.7
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	39	68	54	61	67	72	70	68	63
Time to Drain 99% of Inflow Volume (hours) =	**40**	72	57	64	70	77	76	75	74
Maximum Ponding Depth (ft) =	2.58	3.76	3.02	3.30	3.57	3.94	4.10	4.33	5.32
Area at Maximum Ponding Depth (acres) =	0.33	0.55	0.46	0.51	0.53	0.57	0.58	0.60	0.66
Maximum Volume Stored (acre-ft) =	0.229	0.787	0.399	0.537	0.678	0.888	0.979	1.115	1.739

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.06	0.00	0.61
	0:15:00	0.00	0.00	0.58	1.26	1.75	1.38	1.91	1.98	3.17
	0:20:00	0.00	0.00	2.99	4.13	5.09	3.59	4.45	4.97	7.10
	0:25:00	0.00	0.00	6.28	8.31	10.70	7.46	8.96	10.12	15.36
	0:30:00	0.00	0.00	7.00	9.19	11.56	15.54	20.83	25.44	40.48
	0:35:00	0.00	0.00	6.26	8.10	10.07	16.96	22.74	29.77	45.99
	0:40:00	0.00	0.00	5.47	6.93	8.55	15.55	20.77	27.08	41.91
	0:45:00	0.00	0.00	4.49	5.83	7.24	13.19	17.47	23.51	36.70
	0:50:00	0.00	0.00	3.74	4.98	6.04	11.37	14.91	19.80	31.12
	0:55:00	0.00	0.00	3.26	4.33	5.30	9.24	11.93	16.15	25.36
	1:00:00	0.00	0.00	2.91	3.85	4.75	7.85	10.00	13.83	21.81
	1:05:00	0.00	0.00	2.58	3.40	4.22	6.78	8.55	12.15	19.27
	1:10:00	0.00	0.00	2.12	2.98	3.73	5.62	7.01	9.58	14.97
	1:15:00	0.00	0.00	1.74	2.53	3.32	4.61	5.65	7.41	11.38
	1:20:00	0.00	0.00	1.50	2.20	2.92	3.63	4.35	5.30	7.98
	1:25:00	0.00	0.00	1.37	2.01	2.56	2.99	3.54	3.95	5.87
	1:30:00	0.00	0.00	1.30	1.91	2.32	2.51	2.96	3.17	4.63
	1:35:00	0.00	0.00	1.26	1.83	2.15	2.21	2.60	2.71	3.88
	1:40:00	0.00	0.00	1.24	1.64	2.03	2.01	2.36	2.40	3.37
	1:45:00	0.00	0.00	1.22	1.50	1.94	1.88	2.20	2.20	3.04
	1:50:00	0.00	0.00	1.20	1.39	1.89	1.79	2.09	2.05	2.79
	1:55:00	0.00	0.00	1.03	1.31	1.79	1.73	2.02	1.96	2.65
	2:00:00	0.00	0.00	0.91	1.22	1.62	1.69	1.98	1.92	2.59
	2:05:00	0.00	0.00	0.66	0.88	1.17	1.22	1.43	1.39	1.87
	2:10:00	0.00	0.00	0.47	0.63	0.83	0.87	1.01	0.99	1.33
	2:15:00	0.00	0.00	0.33	0.44	0.58	0.61	0.71	0.70	0.94
	2:20:00	0.00	0.00	0.23	0.30	0.40	0.42	0.49	0.48	0.64
	2:25:00	0.00	0.00	0.15	0.20	0.27	0.28	0.33	0.32	0.43
	2:30:00	0.00	0.00	0.10	0.13	0.18	0.19	0.22	0.22	0.29
	2:35:00	0.00	0.00	0.06	0.08	0.11	0.12	0.14	0.13	0.18
	2:40:00	0.00	0.00	0.03	0.05	0.06	0.07	0.07	0.07	0.09
	2:45:00	0.00	0.00	0.01	0.02	0.02	0.03	0.03	0.03	0.03
	2:50:00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.00	0.00
	2:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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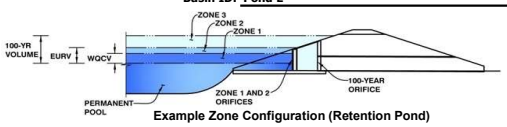
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: **Grandview Reserve Filing No. 1 - Final Drainage Report**

Basin ID: **Pond E**



Example Zone Configuration (Retention Pond)

Watershed Information

Selected BMP Type =	**EDB**	
Watershed Area =	21.31	acres
Watershed Length =	1,165	ft
Watershed Length to Centroid =	500	ft
Watershed Slope =	0.030	ft/ft
Watershed Imperviousness =	52.10%	percent
Percentage Hydrologic Soil Group A =	100.0%	percent
Percentage Hydrologic Soil Group B =	0.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

			Optional User Overrides
Water Quality Capture Volume (WQCV) =	0.377	acre-feet	acre-feet
Excess Urban Runoff Volume (EURV) =	1.295	acre-feet	acre-feet
2-yr Runoff Volume (P1 = 0.93 in.) =	0.710	acre-feet	0.93 inches
5-yr Runoff Volume (P1 = 1.21 in.) =	0.949	acre-feet	1.21 inches
10-yr Runoff Volume (P1 = 1.46 in.) =	1.195	acre-feet	1.46 inches
25-yr Runoff Volume (P1 = 1.84 in.) =	1.631	acre-feet	1.84 inches
50-yr Runoff Volume (P1 = 2.16 in.) =	2.092	acre-feet	2.16 inches
100-yr Runoff Volume (P1 = 2.49 in.) =	2.639	acre-feet	2.49 inches
500-yr Runoff Volume (P1 = 3.37 in.) =	4.078	acre-feet	3.37 inches
Approximate 2-yr Detention Volume =	0.654	acre-feet	
Approximate 5-yr Detention Volume =	0.885	acre-feet	
Approximate 10-yr Detention Volume =	1.115	acre-feet	
Approximate 25-yr Detention Volume =	1.500	acre-feet	
Approximate 50-yr Detention Volume =	1.743	acre-feet	
Approximate 100-yr Detention Volume =	2.012	acre-feet	

Define Zones and Basin Geometry

Zone 1 Volume (WQCV) =	0.377	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.918	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	0.717	acre-feet
Total Detention Basin Volume =	2.012	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	

Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

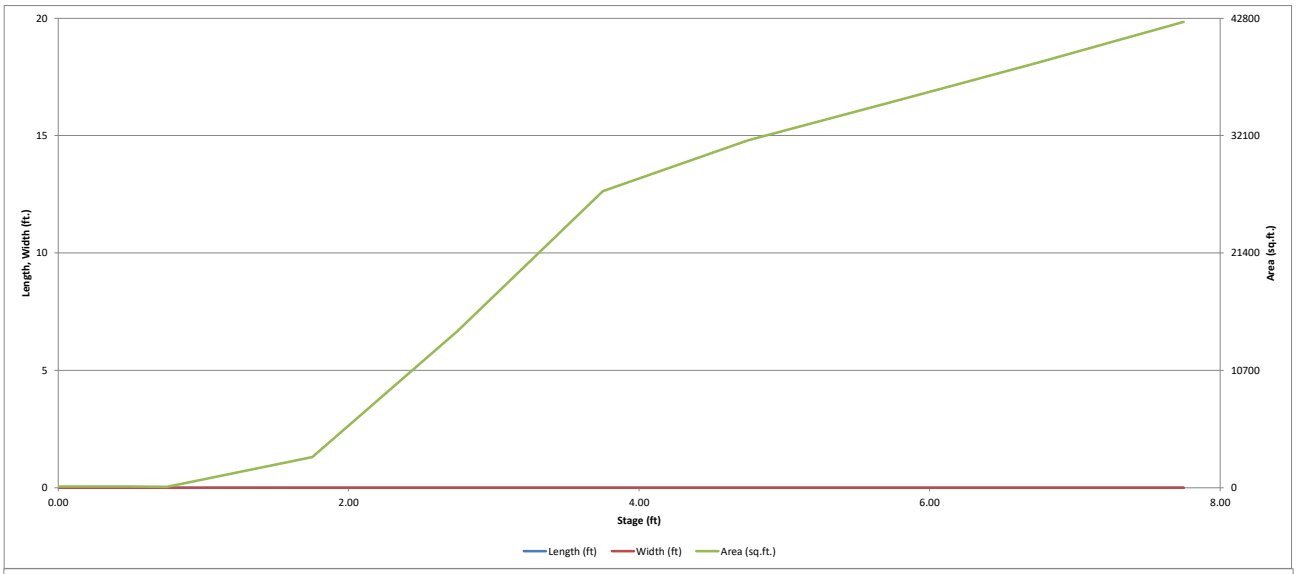
Depth Increment = ft

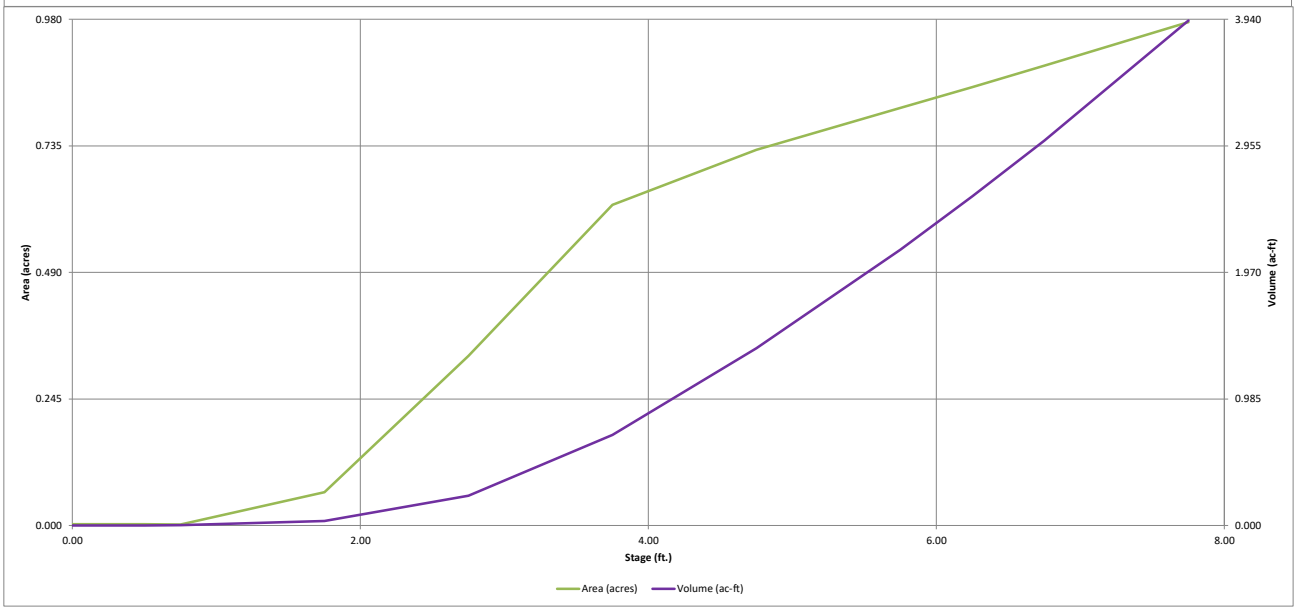
Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	102	0.002		
Trickle Channel Inv	--	0.50	--	--	--	102	0.002	51	0.001
6948	--	0.75	--	--	--	87	0.002	75	0.002
6949	--	1.75	--	--	--	2,819	0.065	1,528	0.035
6950	--	2.75	--	--	--	14,292	0.328	10,083	0.231
6951	--	3.75	--	--	--	27,045	0.621	30,751	0.706
6952	--	4.75	--	--	--	31,693	0.728	60,120	1.380
6953	--	5.75	--	--	--	35,219	0.809	93,576	2.148
Spillway Invert	--	6.25	--	--	--	36,987	0.849	111,628	2.563
6954	--	6.75	--	--	--	38,781	0.890	130,570	2.997
Top of Pond	--	7.75	--	--	--	42,466	0.975	171,193	3.930

6947.25
6947.75
6953.5
6955

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)



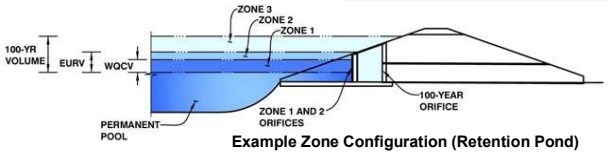


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: **Grandview Reserve Filing No. 1 - Final Drainage Report**

Basin ID: **Pond E**



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.13	0.377	Orifice Plate
Zone 2 (EURV)	4.64	0.918	Rectangular Orifice
Zone 3 (100-year)	5.58	0.717	Weir&Pipe (Restrict)
Total (all zones)		2.012	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

Calculated Parameters for Underdrain

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	7.431E-03	ft^2
Depth at top of Zone using Orifice Plate =	3.13	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	12.50	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	1.07	sq. inches (diameter = 1-1/8 inches)	Elliptical Slot Area =	N/A	ft^2

Calculated Parameters for Plate

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.04	2.09					
Orifice Area (sq. inches)	1.07	1.07	1.07					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Rectangular	Not Selected			Zone 2 Rectangular	Not Selected	
Invert of Vertical Orifice =	3.13	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	0.05	N/A	ft^2
Depth at top of Zone using Vertical Orifice =	4.64	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	0.08	N/A	feet
Vertical Orifice Height =	2.00	N/A	inches				
Vertical Orifice Width =	3.25		inches				

Calculated Parameters for Vertical Orifice

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected			Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	4.67	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	4.67	N/A	feet
Overflow Weir Front Edge Length =	6.00	N/A	feet	Overflow Weir Slope Length =	3.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V	Grate Open Area / 100-yr Orifice Area =	10.25	N/A	
Horiz. Length of Weir Sides =	3.00	N/A	feet	Overflow Grate Open Area w/o Debris =	12.53	N/A	ft^2
Overflow Grate Type =	Type C Grate	N/A		Overflow Grate Open Area w/ Debris =	6.26	N/A	ft^2
Debris Clogging % =	50%	N/A	%				

Calculated Parameters for Overflow Weir

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected			Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	1.58	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	1.22	N/A	ft^2
Outlet Pipe Diameter =	18.00	N/A	inches	Outlet Orifice Centroid =	0.55	N/A	feet
Restrictor Plate Height Above Pipe Invert =	11.75		inches	Half-Central Angle of Restrictor Plate on Pipe =	1.88	N/A	radians

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	6.25	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.39 feet
Spillway Crest Length =	70.00	feet	Stage at Top of Freeboard =	7.64 feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	0.97 acres
Freeboard above Max Water Surface =	1.00	feet	Basin Volume at Top of Freeboard =	3.82 acre-ft

Calculated Parameters for Spillway

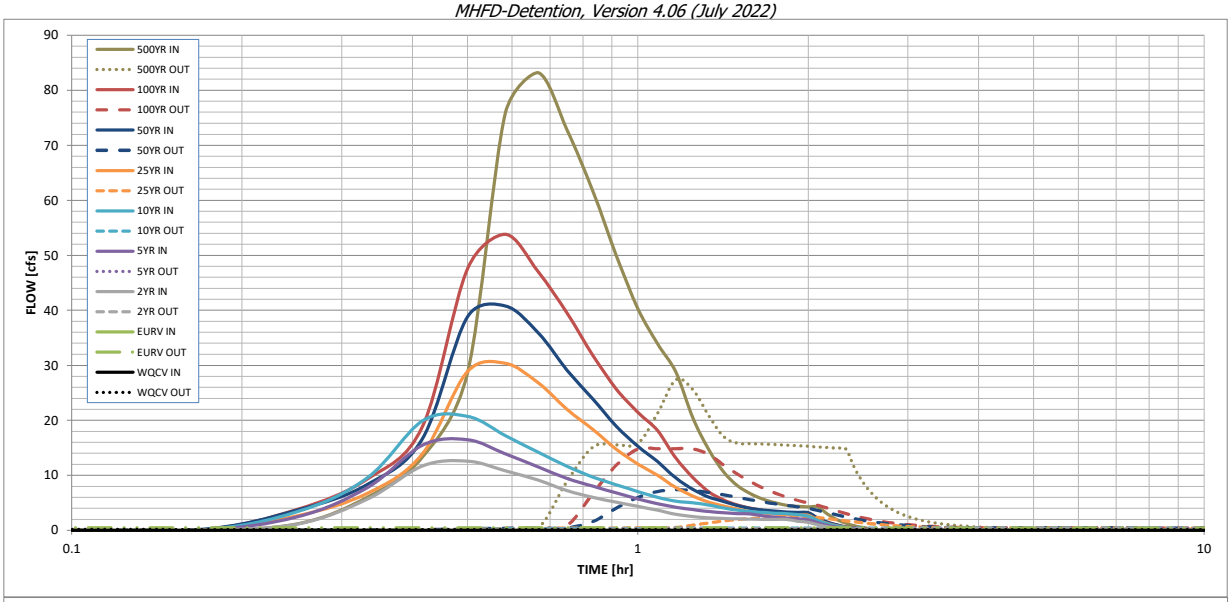
Routed Hydrograph Results

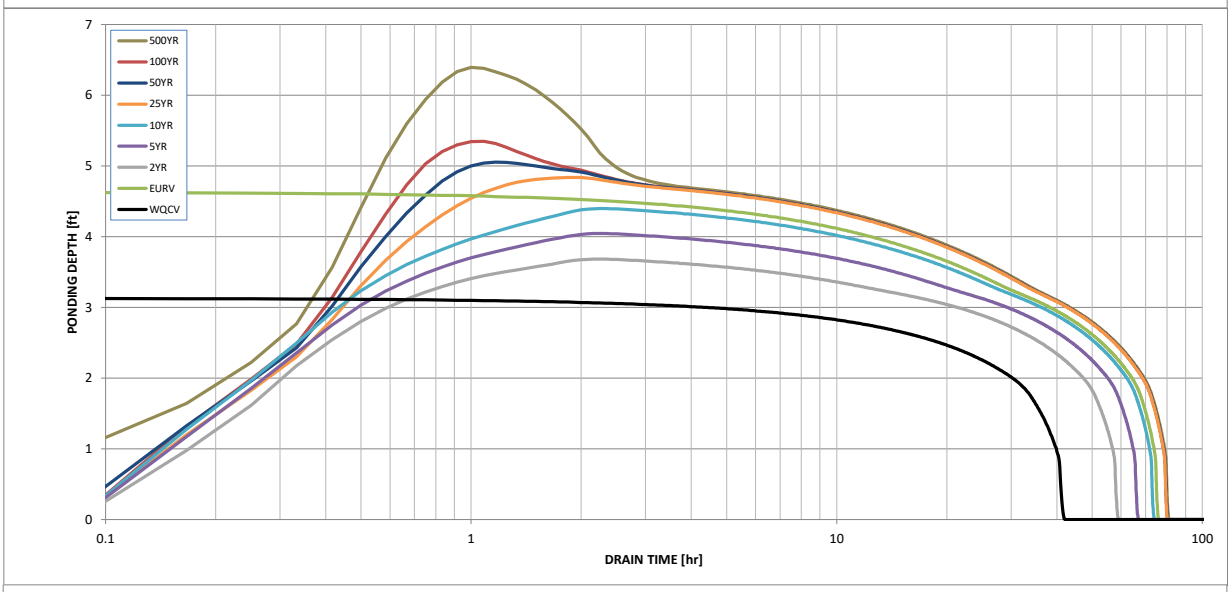
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

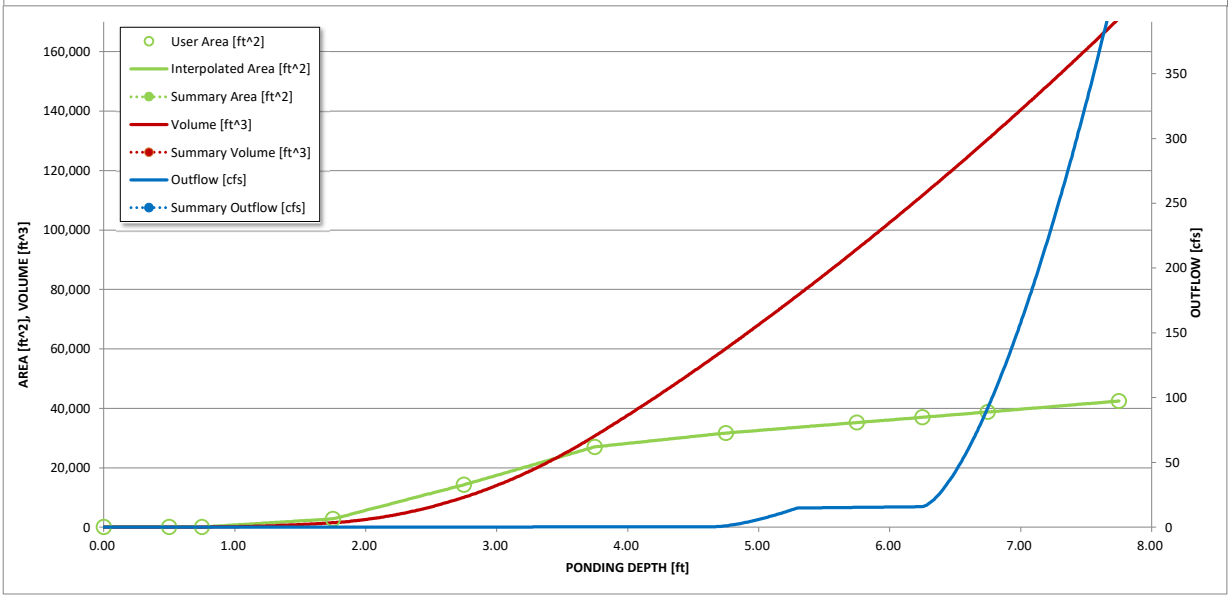
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	N/A	N/A	0.93	1.21	1.46	1.84	2.16	2.49	3.37
CUHP Runoff Volume (acre-ft) =	0.377	1.295	0.710	0.949	1.195	1.631	2.092	2.639	4.078
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.710	0.949	1.195	1.631	2.092	2.639	4.078
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.0	0.2	0.4	2.0	8.3	16.0	34.8
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.00	0.01	0.02	0.09	0.39	0.75	1.63
Peak Inflow Q (cfs) =	N/A	N/A	12.6	16.5	20.7	30.3	40.8	53.8	83.2
Peak Outflow Q (cfs) =	0.2	0.5	0.3	0.4	0.4	2.5	7.4	14.9	27.4
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	1.9	1.2	1.3	0.9	0.9	0.8
Structure Controlling Flow =	Plate	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	0.2	0.5	1.1	1.2
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	67	53	60	66	70	68	66	62
Time to Drain 99% of Inflow Volume (hours) =	**40**	72	56	63	70	76	75	74	72
Maximum Ponding Depth (ft) =	3.13	4.64	3.68	4.05	4.40	4.84	5.05	5.35	6.39
Area at Maximum Ponding Depth (acres) =	0.44	0.72	0.60	0.65	0.69	0.73	0.75	0.78	0.86
Maximum Volume Stored (acre-ft) =	0.377	1.301	0.663	0.890	1.125	1.439	1.602	1.824	2.682

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
Time Interval	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.12	0.00	1.17
	0:15:00	0.00	0.00	1.12	2.43	3.37	2.65	3.67	3.80	6.03
	0:20:00	0.00	0.00	5.70	7.81	9.59	6.76	8.36	9.35	13.31
	0:25:00	0.00	0.00	11.71	15.47	19.91	13.90	16.65	18.85	28.64
	0:30:00	0.00	0.00	12.60	16.49	20.73	28.85	38.83	47.56	75.79
	0:35:00	0.00	0.00	10.81	13.90	17.24	30.35	40.81	53.82	83.19
	0:40:00	0.00	0.00	9.12	11.51	14.19	26.85	35.91	47.01	72.80
	0:45:00	0.00	0.00	7.24	9.41	11.64	21.99	29.12	39.49	61.63
	0:50:00	0.00	0.00	5.99	8.01	9.68	18.33	23.87	31.79	49.93
	0:55:00	0.00	0.00	5.11	6.79	8.29	14.71	18.89	25.66	40.35
	1:00:00	0.00	0.00	4.34	5.73	7.07	12.07	15.29	21.48	33.90
	1:05:00	0.00	0.00	3.68	4.84	6.02	10.01	12.50	18.13	28.81
	1:10:00	0.00	0.00	2.91	4.22	5.30	7.79	9.53	13.15	20.46
	1:15:00	0.00	0.00	2.50	3.77	5.03	6.25	7.48	9.63	14.76
	1:20:00	0.00	0.00	2.29	3.43	4.62	5.12	6.06	7.05	10.65
	1:25:00	0.00	0.00	2.16	3.20	4.06	4.46	5.25	5.51	8.11
	1:30:00	0.00	0.00	2.09	3.05	3.68	3.83	4.50	4.60	6.62
	1:35:00	0.00	0.00	2.04	2.96	3.42	3.41	3.99	4.00	5.63
	1:40:00	0.00	0.00	2.00	2.62	3.24	3.14	3.69	3.61	4.99
	1:45:00	0.00	0.00	1.97	2.37	3.11	2.96	3.47	3.35	4.56
	1:50:00	0.00	0.00	1.96	2.21	3.03	2.85	3.34	3.21	4.35
	1:55:00	0.00	0.00	1.66	2.09	2.87	2.78	3.26	3.16	4.28
	2:00:00	0.00	0.00	1.44	1.94	2.57	2.74	3.21	3.14	4.25
	2:05:00	0.00	0.00	0.98	1.32	1.76	1.87	2.19	2.15	2.90
	2:10:00	0.00	0.00	0.65	0.88	1.18	1.25	1.46	1.44	1.93
	2:15:00	0.00	0.00	0.43	0.58	0.78	0.83	0.97	0.95	1.27
	2:20:00	0.00	0.00	0.27	0.36	0.50	0.53	0.61	0.60	0.80
	2:25:00	0.00	0.00	0.16	0.23	0.31	0.34	0.39	0.38	0.50
	2:30:00	0.00	0.00	0.08	0.13	0.17	0.20	0.22	0.22	0.29
	2:35:00	0.00	0.00	0.04	0.06	0.08	0.09	0.10	0.10	0.13
	2:40:00	0.00	0.00	0.01	0.02	0.02	0.03	0.03	0.03	0.03
	2:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

FOREBAY SIZING CALCULATIONS

Subdivision: Grandview Reserve Filing No. 1
Location: CO, Falcon (El Paso County)

Project Name: Grandview Reserve Filing No. 1
Project No.: HRG02.20
Calculated By: TJE
Checked By: BAS
Date: 3/14/24

	Forebay D-1	Forebay E-1		
Impervious % (I)	56.2%	54.90%		Total impervious area of contributing upstream basins
WQCV Drain Time Coeff (a)	1	1		a = 1 for 40 Hr WQCV Drain Time
Tributary Area (Ac)	11.28	19.88		
Forebay Depth (Ft)	1.50	1.50		(see Table EDB-4 of the USDCM Volume 3 for depth requirement)
% of WQCV for Forebay Volume	3.0%	3.0%		(see Table EDB-4 of the USDCM Volume 3 for requirement)
100-year Discharge (Q)	25.80	63.10		100-Year Flow entering Forebay (un-detained)
WQCV Depth (in)	0.22	0.22		WQCV Depth = $a(0.91*I^3 - 1.19*I^2 + 0.78*I)$
WQCV Volume (Ac-Ft)	0.21	0.36		
Forebay Volume (Cu. Ft.)	275	477		
Forebay Discharge (Q)	0.52	1.26		(Release 2% of 100-year discharge via notch or berm/pipe configuration)
Forebay Notch Height (in)	15.00	15.00		(3" depression @ top of forebay assumed per COS DCM Volume 1, 13-30)
Forebay Deisgn Results				
Minimum Forebay Area (Sq. Ft.)	**183**	**318**		
Forebay Notch width (in)	**3**	**3**		From $Q=C_w*W*H^{1.5}$ assuming C_w=3.33 for sharp-crested weir - **If notch width <3", use 3" minimum.**

Galloway

Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Friday, Mar 15 2024

Pond D Trickle Channel

Rectangular

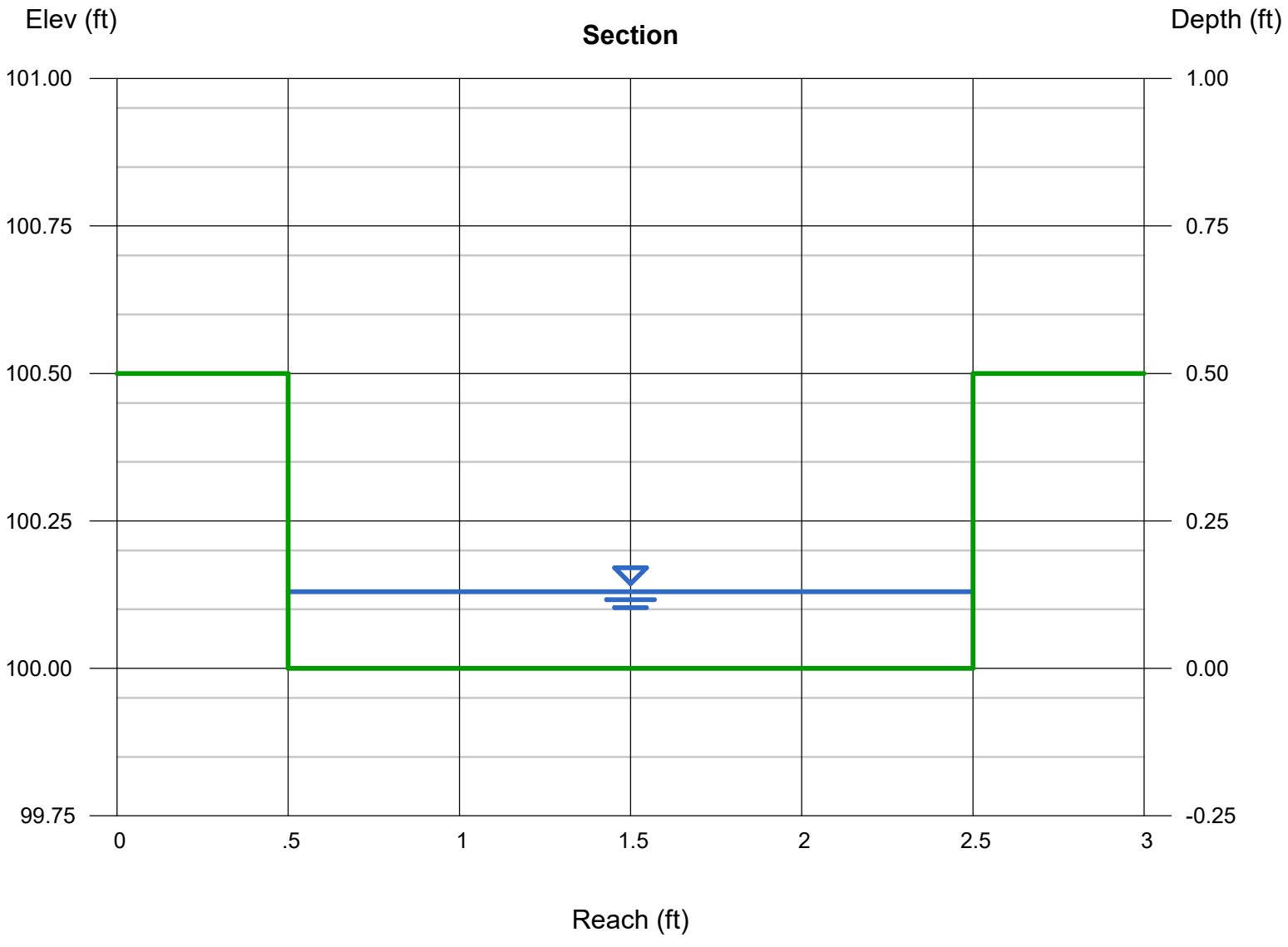
Bottom Width (ft)	= 2.00
Total Depth (ft)	= 0.50
Invert Elev (ft)	= 100.00
Slope (%)	= 0.50
N-Value	= 0.012

Calculations

Compute by:	Known Q
Known Q (cfs)	= 0.52

Highlighted

Depth (ft)	= 0.13
Q (cfs)	= 0.520
Area (sqft)	= 0.26
Velocity (ft/s)	= 2.00
Wetted Perim (ft)	= 2.26
Crit Depth, Yc (ft)	= 0.13
Top Width (ft)	= 2.00
EGL (ft)	= 0.19



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc. Friday, Mar 15 2024

Pond E Trickle Channel

Rectangular

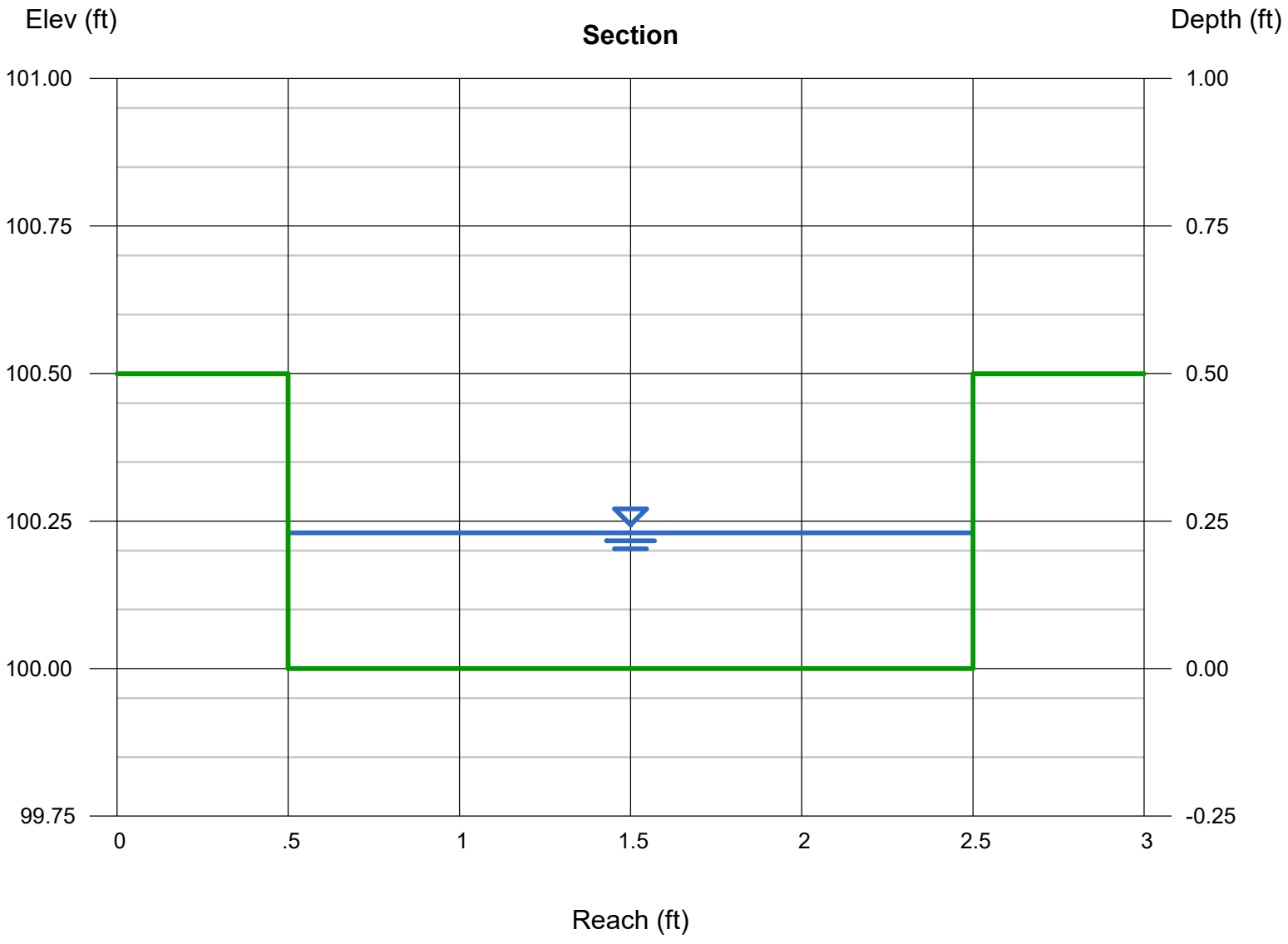
Bottom Width (ft)	= 2.00
Total Depth (ft)	= 0.50
Invert Elev (ft)	= 100.00
Slope (%)	= 0.50
N-Value	= 0.012

Calculations

Compute by:	Known Q
Known Q (cfs)	= 1.26

Highlighted

Depth (ft)	= 0.23
Q (cfs)	= 1.260
Area (sqft)	= 0.46
Velocity (ft/s)	= 2.74
Wetted Perim (ft)	= 2.46
Crit Depth, Yc (ft)	= 0.24
Top Width (ft)	= 2.00
EGL (ft)	= 0.35



Micropool/ISV SIZING CALCULATIONS

Project Name: Grandview Reserve Filing No. 1
Project No.: HRG02.20
Calculated By: TJE
Checked By: BAS
Date: 3/14/24

	Pond D	Pond E	
WQCV Volume (Ac-Ft)	0.227	0.377	From MHFD-Detention Spreadsheet
Provided ISV Depth (in)	6.00	6.00	4" Min. per USDCM, Volume 3
Provided Micropool/ISV Area (Sq. Ft.)	90.00	102.00	
Provided ISV Volume (Cu. Ft.)	45.00	51.00	
Micropool/ISV Deisgn Results			
Minimum Micropool Area (Sq. Ft.)	**59**	**99**	Assuming ISV above - Min. 10 ft^2 per USDCM, Volume 3
Required ISV Volume (Cu. Ft.)	**30**	**49**	0.3% of WQCV, per USDCM, Volume 3
Is Required Micropool Area Met?	**YES**	**YES**	
Is Required ISV Volume Met?	**YES**	**YES**	

Galloway

Figure 13-12c. Emergency Spillway Protection

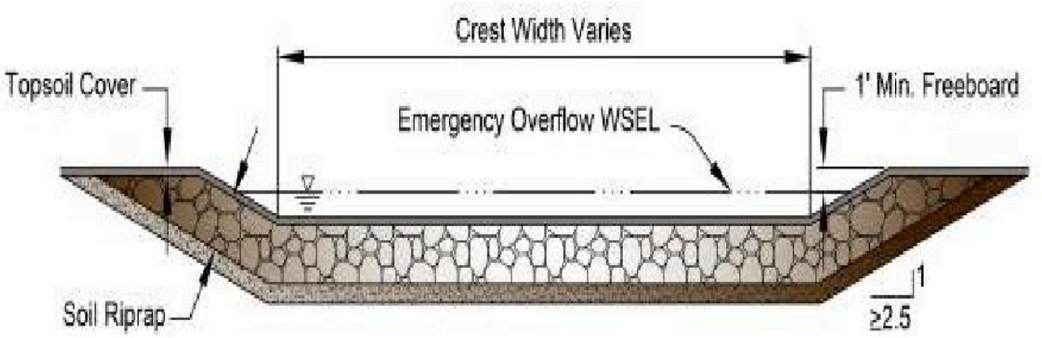
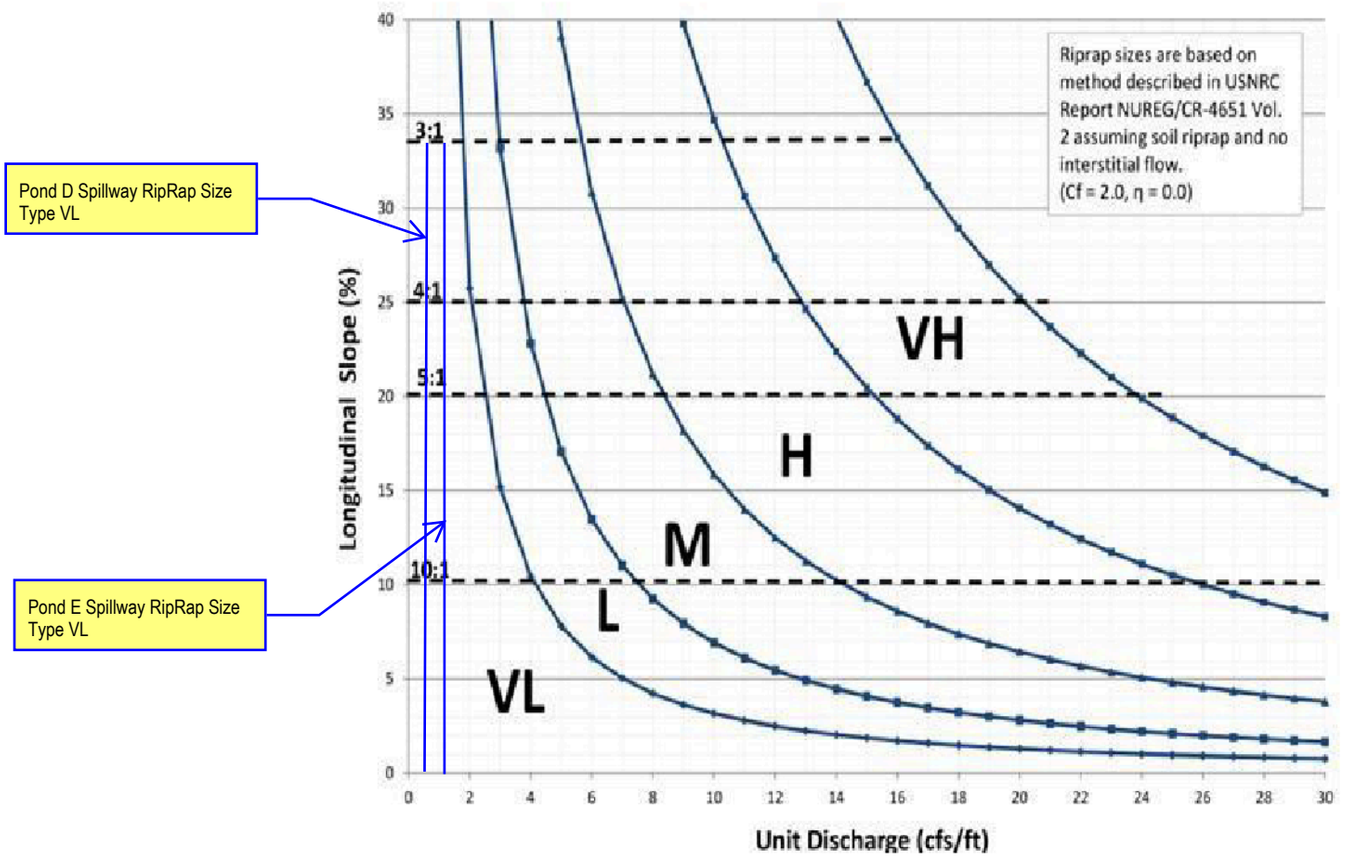


Figure 13-12d. Riprap Types for Emergency Spillway Protection



Pond D Unit Discharge = 28.8 cfs / 50.0 ft = 0.576 cfs/ft

Pond E Unit Discharge = 64.9 cfs / 70.0 ft = 0.927 cfs/ft

APPENDIX F

Drainage Maps

RUNOFF SUMMARY TABLE			
Basin	Area	Q_5	Q_{100}
ID	(acres)	(cfs)	(cfs)
*EX1	321.53	28.3	365.2
*EX2	18.88	1.7	18.8
*EX3	131.26	6.1	112.1
*EX4	832.70	22.4	491.0
*EX5	22.35	7.0	43.3
*EX6	3.05	1.2	6.9
*EX7	1.47	0.9	4.2
ES-1	16.37	3.5	24.7
ES-2	46.05	7.5	53.7
ES-3	64.30	10.0	71.5
ES-4	2.68	0.6	4.4
ES-5	26.15	5.0	35.5
ES-6	31.26	6.5	46.5

*Values taken from Eastonville Road PDR prepared by HR Green, Dated September 2023

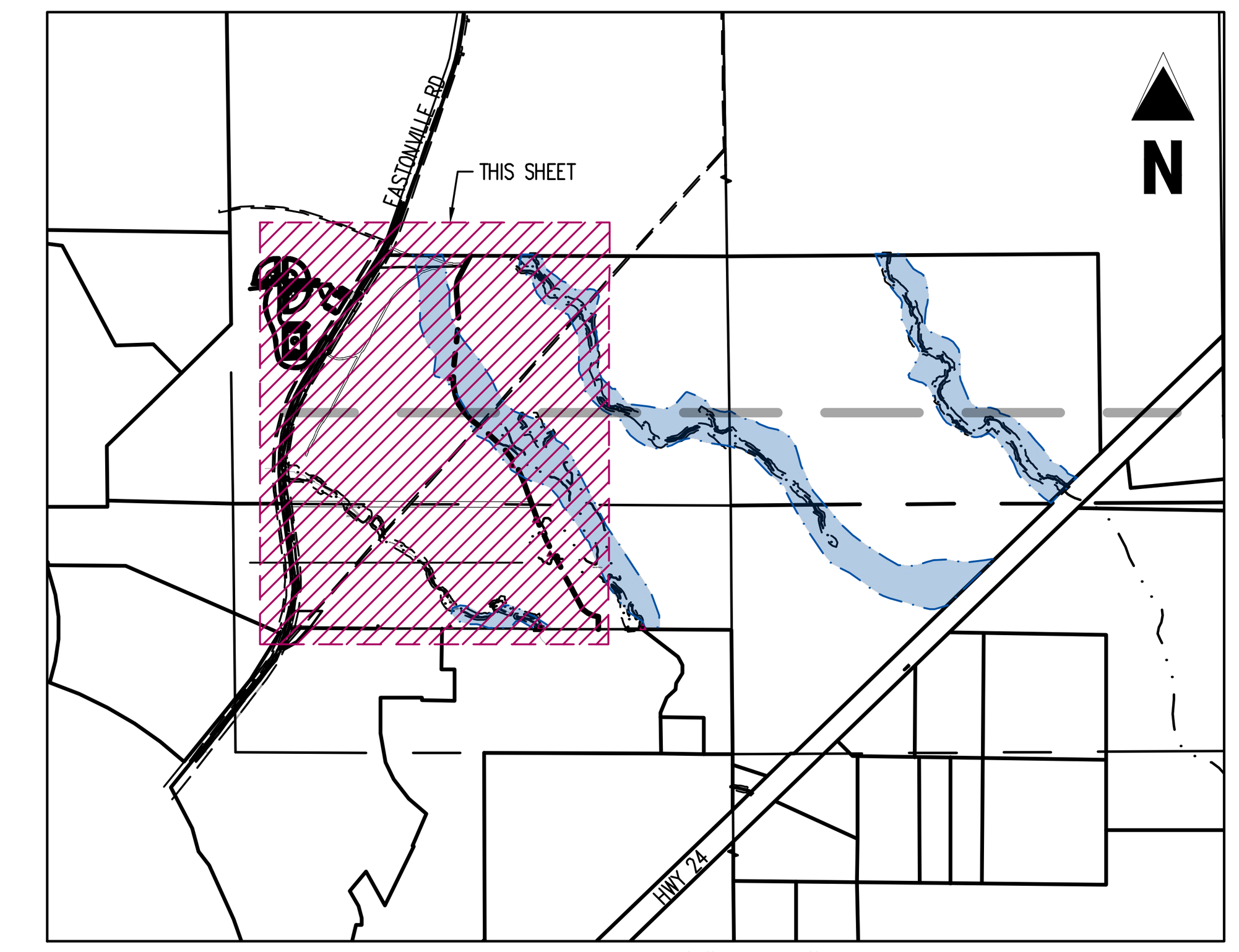
DESIGN POINT SUMMARY TABLE			
	Design	Q_5	Q_{100}
	Point	(cfs)	(cfs)
	X1	4.7	31.6
	X2	36.9	588.0
	X3	10.0	71.5
	X4	0.6	4.4
	X5	5.0	35.5
	X6	40.9	523.8
	X7	56.5	635.2
*	1	28.3	365.2
*	2	1.7	18.8
*	3	6.1	112.1
*	4	22.4	491.0
*	5	7.0	43.3
*	6	1.2	6.9
*	7	0.9	4.2

*Values taken from Eastonville Road PDR prepared by HR Green, Dated September 2023

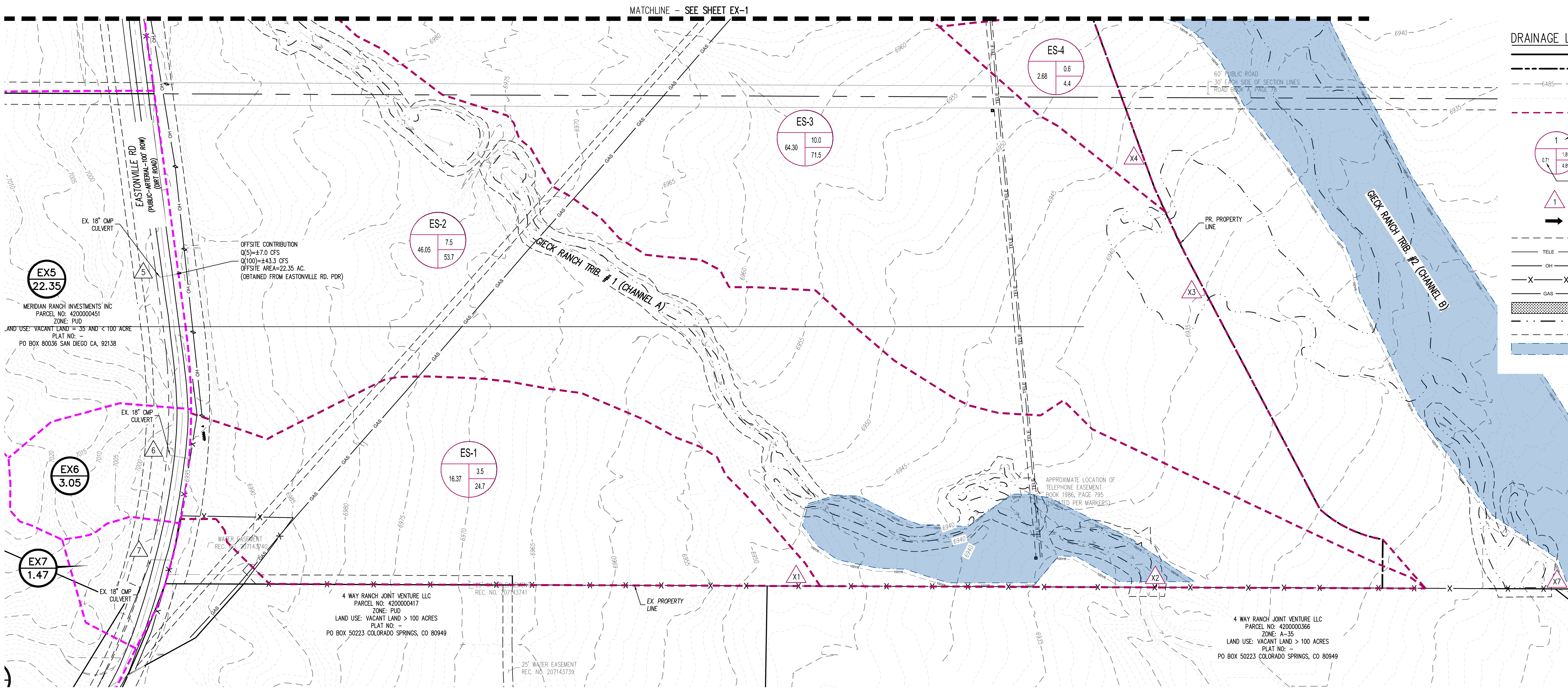
NOTE:

1. FOR EXISTING WESTERN OFFSITE SUB-BASIN ANALYSIS AS WELL AS PROPOSED EASTONVILLE ROAD SUB-BASIN ANALYSIS, SEE "EASTONVILLE ROAD FINAL DRAINAGE REPORT", BY HR GREEN, SEPTEMBER 2022.

GRANDVIEW RESERVE HISTORIC DRAINAGE MAP



KEY MAP SCALE: (1"=2,000')



DRAINAGE LEGEND

- EXISTING PROPERTY LINE
- PROPOSED PROPERTY LINE
- 6485 EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- BASIN BOUNDARY LINE
- BASIN DESIGNATION
- 5-YEAR RUNOFF IN CUBIC FEET PER SECOND
- 100-YEAR RUNOFF IN CUBIC FEET PER SECOND
- BASIN AREA IN ACRES
- DESIGN POINT
- DIRECTION OF RUNOFF
- EXISTING BOUNDARY EASEMENT
- TELE EXISTING TELEPHONE LINE
- OH EXISTING POWER LINE
- EXISTING FENCE
- GAS EXISTING GAS LINE
- EXISTING WETLANDS
- EXISTING LIMITS OF WETLAND
- EXISTING WETLAND SETBACK
- EXISTING FEMA FLOOD PLAIN, ZONE A

Galloway
6162 S. Willow Drive, Suite 320
Greenwood Village, CO 80111
303.770.8884
GallowayUS.com

PRELIMINARY
NOT FOR BIDDING
NOT FOR CONSTRUCTION

PRELIMINARY DRAINAGE PLAN
GRANDVIEW RESERVE
FOR
HR GREEN, INC

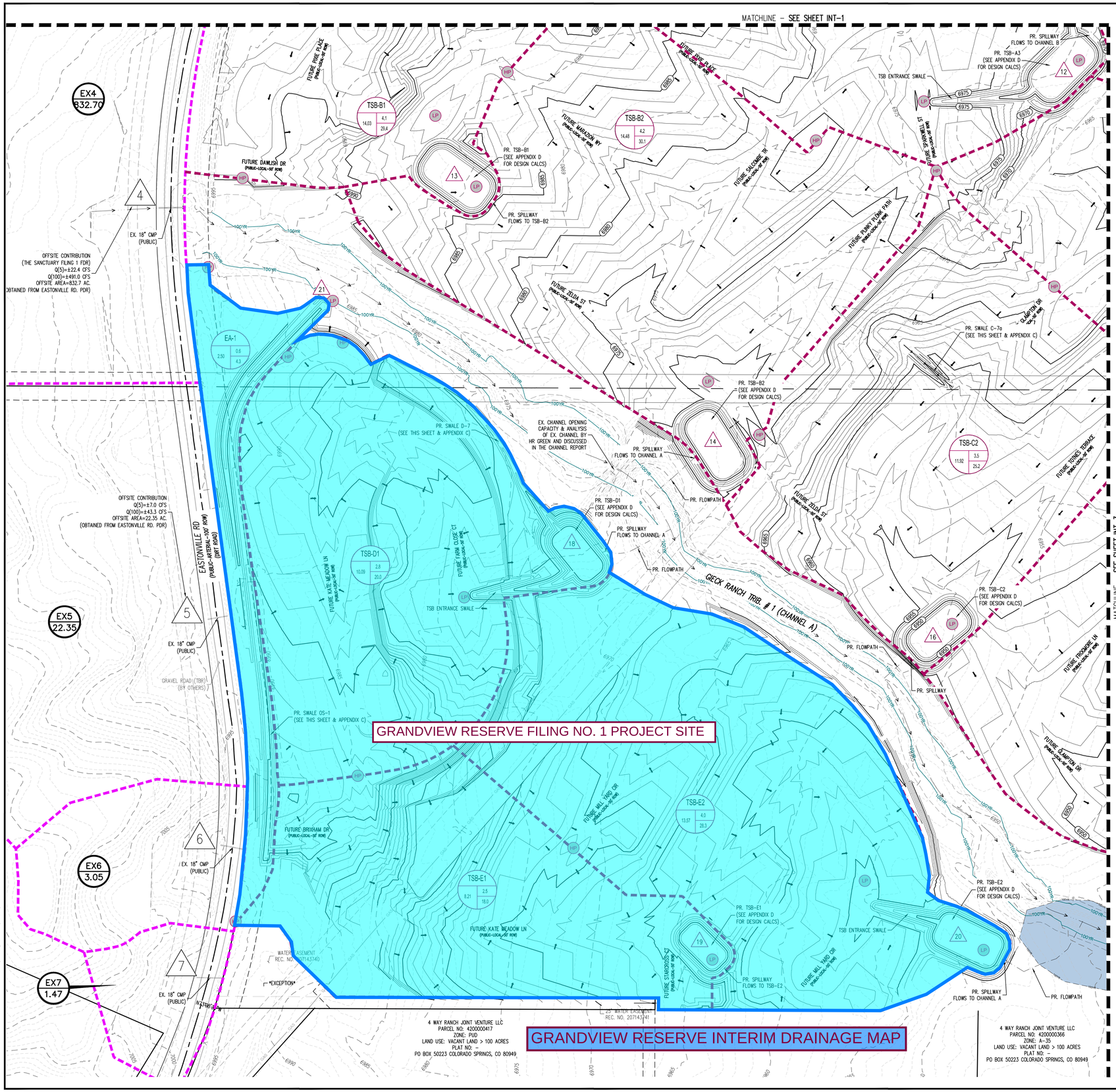
EASTONVILLE RD
EL PASO COUNTY, PEYTON, CO 80831

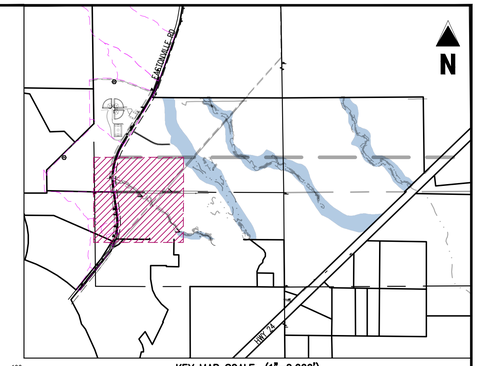
#	Date	Issue / Description	Init.

Project No:	HRG 1.20
Drawn By:	TJE
Checked By:	GRD
Date:	1/23/2024

EXISTING DRAINAGE MAP

EX-2





KEY MAP SCALE: (1"=2,000')

DRAINAGE LEGEND

- EXISTING PROPERTY LINE
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- TSB BASIN BOUNDARY LINE
- EXISTING WETLANDS
- EXISTING LIMITS OF WETLAND
- EXISTING WETLAND SETBACK
- EXISTING FEMA FLOOD PLAIN, ZONE A
- 100-YEAR WATER SURFACE ELEVATION AND FLOODPLAIN
- EX3 / 131.26 — OFF-SITE BASIN DESIGNATION / OFF-SITE BASIN TRIBUTARY AREA
- BASIN DESIGNATION
- 5-YEAR RUNOFF IN CUBIC FEET PER SECOND
- 100-YEAR RUNOFF IN CUBIC FEET PER SECOND
- BASIN AREA IN ACRES
- DESIGN POINT
- DIRECTION OF RUNOFF

DESIGN POINT SUMMARY TABLE

Design Point	Q_5 (cfs)	Q_{100} (cfs)
* 2	1.7	18.8
* 3	6.1	112.1
* 4	22.4	491.0
* 5	7.0	43.3
* 6	1.2	6.9
* 7	0.9	4.2
8	0.7	5.0
9	3.3	23.3
10	10.6	144.2
11	1.4	10.1
12	5.4	38.3
13	4.1	29.4
14	8.3	59.5
15	3.2	23.0
16	3.5	25.2
17	7.3	52.0
18	2.8	20.0
19	2.5	18.0
20	6.5	46.3
21	8.9	55.3

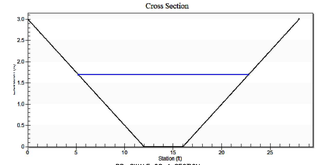
*Values taken from Eastonville Road PDR prepared by HR Green, Dated September 2023

RUNOFF SUMMARY TABLE

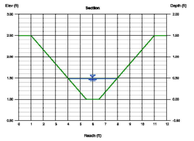
Basin ID	Area (acres)	Q_5 (cfs)	Q_{100} (cfs)
*EX2	18.88	1.7	18.8
*EX3	131.26	6.1	112.1
*EX4	632.70	22.4	491.0
*EX5	22.35	7.0	43.3
*EX6	3.05	1.2	6.9
*EX7	1.47	0.9	4.2
A-1	2.29	0.7	5.0
A-2	3.96	1.2	8.8
EA-1	2.50	0.7	5.1
TSB-A1	10.67	3.3	23.3
TSB-A2	4.56	1.4	10.1
TSB-A3	13.72	3.9	28.2
TSB-B1	14.03	4.1	29.4
TSB-B2	14.48	4.2	30.1
TSB-C1	11.26	3.2	23.0
TSB-C2	11.92	3.5	25.2
TSB-C3	15.29	4.1	29.0
TSB-D1	10.09	2.8	20.0
TSB-E1	8.21	2.5	18.0
TSB-E2	13.57	4.0	28.3

*Values taken from Eastonville Road PDR prepared by HR Green, Dated September 2023

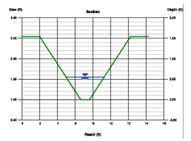
NOTE:
1. FOR EXISTING WESTERN OFFSITE SUB-BASIN ANALYSIS AS WELL AS PROPOSED EASTONVILLE ROAD SUB-BASIN ANALYSIS, SEE "EASTONVILLE ROAD– LONDONDERRY DR. TO REX RD. PRELIMINARY DRAINAGE REPORT", BY HR GREEN, DATED SEPTEMBER 2023.
2. ALL OFF SITE FLOWS NORTHWEST OF REX ROAD NOTED IN THIS REPORT WILL VARY FROM THOSE IN THE CLOMR REPORT PREPARED BY HR GREEN. THIS IS DUE TO SLIGHT DIFFERENCES IN HYDROLOGIC MODELING. THE FLOW AT THE INLET OF THE PROPOSED REX ROAD CULVERT IS 262 CFS IN THE SUBMITTED CLOMR MODEL, AS OPPOSED TO 280 CFS FROM THE ACCEPTED MERIDIAN RANCH DEVELOPMENT DRAINAGE PLAN MODEL OUTPUT. THE FLOW AT THE OUTLET OF THE PROPOSED REX ROAD CULVERT IS 536 CFS. THIS VALUE COMES FROM THE NEXT DOWNSTREAM DESIGN POINT AT THE INLET OF THE PROPOSED DAWLISH ROAD AND IS CARRIED UPSTREAM TO THE PREVIOUS DESIGN POINT.
3. PROPOSED CHANNEL DESIGN AND ANALYSIS FOR BOTH ON-SITE TRIBUTARIES (MAIN STEM AND MAIN STEM TRIBUTARY NUMBER 2) IS PROVIDED IN A SEPARATE REPORT "GRANDVIEW RESERVE CLOMR REPORT", BY HR GREEN, DATED MARCH 22, 2023. ALL CHANNEL CULVERTS ARE SHOWN FOR REFERENCE ONLY AND ACTUAL SIZING AND ANALYSIS IS PROVIDED WITHIN THE CLOMR REPORT.



PR. SWALE OS–1 SECTION
BTM WIDTH: 4.0'; DEPTH: 2.03'; SIDE SLOPES: 4:1; SLOPE: 0.50%; Q: 55.3 cfs NATURALLY VEGETATED (SEE APPENDIX C FOR DESIGN CALCS)



PR. SWALE C–7a SECTION
(BTM WIDTH: 1.0'; DEPTH: 0.49'; SIDE SLOPES: 3:1; SLOPE: 2.0%; Q: 3.2 cfs)



PR. SWALE D–7 SECTION
(BTM WIDTH: 1.0'; DEPTH: 0.55'; SIDE SLOPES: 3:1; SLOPE: 2.0%; Q: 4.0 cfs)

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PRELIMINARY
NOT FOR BIDDING
NOT FOR CONSTRUCTION

PRELIMINARY DRAINAGE PLAN
GRANDVIEW RESERVE FILING NO. 1
FOR
HR GREEN, INC

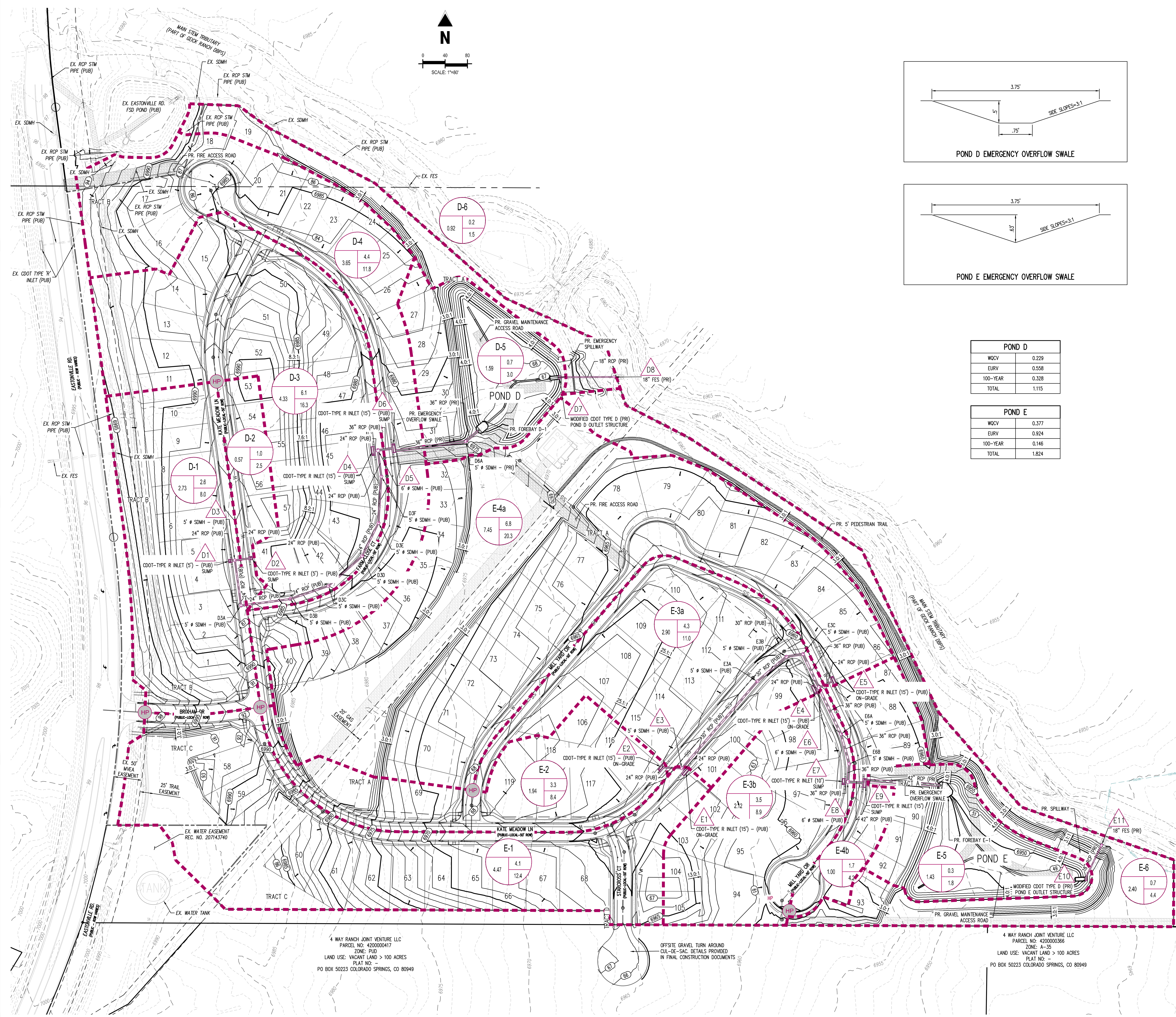
EASTONVILLE RD
EL PASO COUNTY, PEYTON, CO 80831

#	Date	Issue / Description	Init.

Project No:	HRG 1.20
Drawn By:	TJE
Checked By:	GRD
Date:	1/23/2024

INTERIM DRAINAGE MAP

INT-2
Sheet 2 of 3



POND D EMERGENCY OVERFLOW SWALE

POND E EMERGENCY OVERFLOW SWALE

POND D	
WQCV	0.229
EURV	0.558
100-YEAR	0.328
TOTAL	1.115

POND E	
WQCV	0.377
EURV	0.924
100-YEAR	0.146
TOTAL	1.824

DRAINAGE LEGEND

- EXISTING PROPERTY LINE
- PROPOSED PROPERTY LINE / ROW
- LOT BOUNDARY LINE
- EXISTING MAJOR CONTOUR
- EXISTING MINOR CONTOUR
- PROPOSED MAJOR CONTOUR
- PROPOSED MINOR CONTOUR
- PROPOSED ROAD CENTERLINE
- BASIN BOUNDARY LINE
- PROPOSED STORM SEWER
- PROPOSED STORM STRUCTURES
- EXISTING LIMITS OF WETLAND
- EXISTING WETLAND SETBACK
- CENTERLINE OF STREAM
- 100-YEAR WATER SURFACE ELEVATION AND FLOODPLAIN
- PROPOSED RIPRAP
- PROPOSED GRAVEL ACCESS
- BASIN DESIGNATION
- 5-YEAR RUNOFF IN CUBIC FEET PER SECOND
- 100-YEAR RUNOFF IN CUBIC FEET PER SECOND
- BASIN AREA IN ACRES
- DESIGN POINT
- DIRECTION OF RUNOFF

BASIN SUMMARY TABLE			
Tributary Sub-basin	Area (acres)	Q_5 (cfs)	Q_{100} (cfs)
D-1	2.73	2.6	8.0
D-2	0.57	1.0	2.5
D-3	4.33	6.1	16.3
D-4	3.65	4.4	11.8
D-5	1.59	0.7	3.0
D-6	0.92	0.2	1.5
E-1	4.47	4.1	12.4
E-2	1.94	3.3	8.4
E-3a	2.90	4.3	11.0
E-3b	2.12	3.5	8.9
E-4a	7.45	6.8	20.3
E-4b	1.00	1.7	4.2
E-5	1.43	0.3	1.8
E-6	2.40	0.7	4.4

DESIGN POINT SUMMARY TABLE		
Design Point	Q_5 (cfs)	Q_{100} (cfs)
D1	2.6	5.3
D2	1.0	5.3
D3	3.6	10.5
D4	6.1	14.1
D5	9.8	24.6
D6	14.2	25.8
D7	14.8	28.8
D8	0.2	8.0
E1	4.1	9.9
E2	3.3	7.8
E3	7.4	17.7
E4	4.3	11.7
E5	6.6	11.7
E6	18.3	41.1
E7	3.5	11.0
E8	21.8	52.1
E9	1.7	63.1
E10	2.0	64.9
E11	0.4	14.9

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DRAINAGE MAPS
GRANDVIEW RESERVE FILING NO. 1
4 SITE INVESTMENTS, LLC (D.R. HORTON)

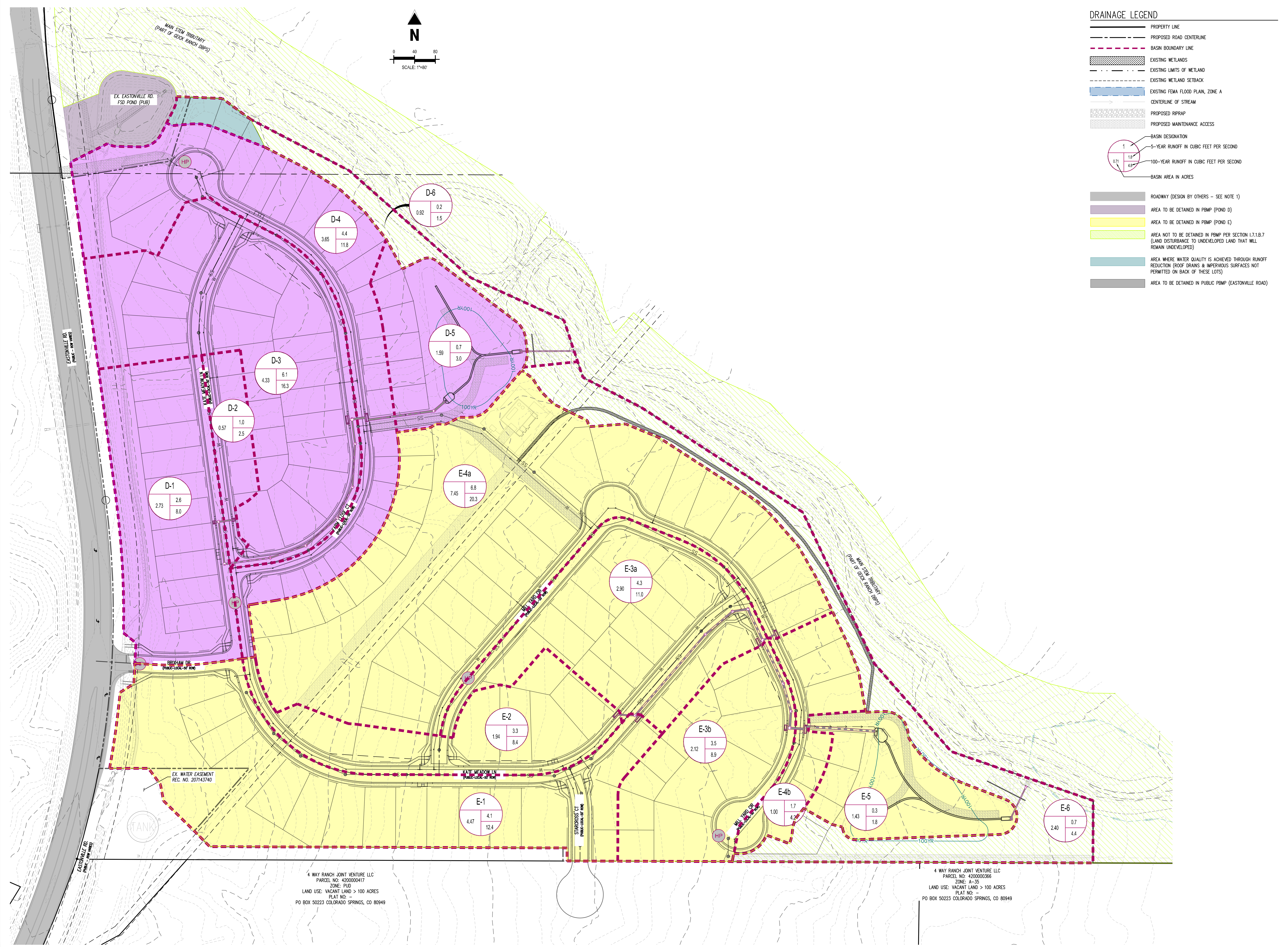
EASTONVILLE RD & REX RD
EL PASO COUNTY, FALCON, CO 80831

#	Date	Issue / Description	Init.

Project No:	HRG02
Drawn By:	TJE
Checked By:	BAS
Date:	03/21/2024

PROPOSED DRAINAGE MAP

DR-2


DRAINAGE LEGEND
PROPERTY LINE
PROPOSED ROAD CENTERLINE
BASIN BOUNDARY LINE
EXISTING WETLANDS
EXISTING LIMITS OF WETLAND
EXISTING WETLAND SETBACK
EXISTING FEMA FLOOD PLAIN, ZONE A
CENTERLINE OF STREAM
PROPOSED RIPRAP
PROPOSED MAINTENANCE ACCESS
BASIN DESIGNATION
5-YEAR RUNOFF IN CUBIC FEET PER SECOND
100-YEAR RUNOFF IN CUBIC FEET PER SECOND
BASIN AREA IN ACRES
1
1.8
4.8
0.71
ROADWAY (DESIGN BY OTHERS - SEE NOTE 1)
AREA TO BE DETAINED IN PBMP (POND D)
AREA TO BE DETAINED IN PBMP (POND E)
AREA NOT TO BE DETAINED IN PBMP PER SECTION I.7.1.B.7 (LAND DISTURBANCE TO UNDEVELOPED LAND THAT WILL REMAIN UNDEVELOPED)
AREA WHERE WATER QUALITY IS ACHIEVED THROUGH RUNOFF REDUCTION (ROOF DRAINS & IMPERVIOUS SURFACES NOT PERMITTED ON BACK OF THESE LOTS)
AREA TO BE DETAINED IN PUBLIC PBMP (EASTONVILLE ROAD)
N
0 40 80
SCALE: 1"=80'
MAIN STEM TRIBUTARY (PART OF GIECK RANCH DBPS)
EX. EASTONVILLE RD. FSD POND (PUB)
HP
D-1 2.73 2.6 8.0
D-2 0.57 1.0 2.5
D-3 4.33 6.1 16.3
D-4 3.65 4.4 11.8
D-5 1.59 0.7 3.0
D-6 0.92 0.2 1.5
E-1 4.47 4.1 12.4
E-2 1.94 3.3 8.4
E-3a 2.90 4.3 11.0
E-3b 2.12 3.5 8.9
E-4a 7.45 6.8 20.3
E-4b 1.00 1.7 4.2
E-5 1.43 0.3 1.8
E-6 2.40 0.7 4.4
100YR
EASTONVILLE RD.
BRIXHAM DR.
KATE MEADOW LN
MILL YARD DR
STARCROSS CT
EX. WATER EASEMENT REC. NO. 207143740
TANK
4 WAY RANCH JOINT VENTURE LLC
PARCEL NO: 4200000417
ZONE: PUD
LAND USE: VACANT LAND > 100 ACRES
PLAT NO: -
PO BOX 50223 COLORADO SPRINGS, CO 80949
4 WAY RANCH JOINT VENTURE LLC
PARCEL NO: 4200000366
ZONE: A-35
LAND USE: VACANT LAND > 100 ACRES
PLAT NO: -
PO BOX 50223 COLORADO SPRINGS, CO 80949

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DRAINAGE MAPS
GRANDVIEW RESERVE FILING NO. 1
4 SITE INVESTMENTS, LLC (D.R. HORTON)

EASTONVILLE RD & REX RD
EL PASO COUNTY, FALCON, CO 80831

#	Date	Issue / Description	Init.
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-			

Project No:	HRG02
Drawn By:	TJE
Checked By:	BAS
Date:	03/15/2024

PROPOSED WATER QUALITY MAP

DR-3