

**FINAL DRAINAGE REPORT**

**FOR**

**WIDEFIELD WATER & SANITATION  
DISTRICT**

**BOOSTER 2 PUMP STATION REPLACEMENT**

**EL PASO COUNTY, COLORADO**

**100% Submittal**

February 2022

Prepared for:

**JDS Hydro Consultants, Inc.  
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Colorado Springs, CO 80919**

Prepared by:



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Project #70-077  
CID No. 2019.815.2027  
PCD Project # PPR-21-069

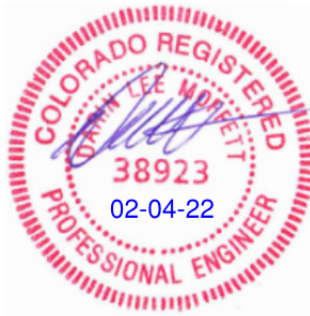
**FINAL DRAINAGE REPORT  
FOR  
WIDEFIELD WATER & SANITATION DISTRICT  
BOOSTER 2 PUMP STATION REPLACEMENT**

**DRAINAGE PLAN STATEMENTS**

ENGINEERS STATEMENT

The attached drainage plan and report was prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

\_\_\_\_\_  
Darin L. Moffett, P.E. #38923  
For and on Behalf of M&S Civil Consultants, Inc



DEVELOPER'S STATEMENT

I, the developer have read and will comply with all the requirements specified in this drainage report and plan.

BY: *Robert D. Bangler*

TITLE: *District Engineer*  
DATE: *2/1/2022*

ADDRESS: Widefield Water and Sanitation District  
8495 Fontaine Blvd.  
Colorado Springs, CO80925

EL PASO COUNTY'S STATEMENT

Filed in accordance with the requirements of El Paso County Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the El Paso County Engineering Criteria Manual, as amended.

APPROVED  
Engineering Department

BY: \_\_\_\_\_ DATE: \_\_\_\_\_  
05/31/2022 3:21:28 PM  
dsdnijkamp  
EPC Planning & Community  
Development Department

County Engineer/ECM Administrator

CONDITIONS

**FINAL DRAINAGE REPORT  
FOR  
WIDEFIELD WATER & SANITATION DISTRICT  
BOOSTER 2 PUMP STATION REPLACEMENT**

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**FINAL DRAINAGE REPORT  
FOR  
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BOOSTER 2 PUMP STATION REPLACEMENT**

**PURPOSE**

This document is intended to serve as the Final Drainage report for the Widefield Water & Sanitation District Booster 2 Pump Station. The purpose of this document is to identify and analyze the on and offsite drainage patterns and to ensure that post development runoff is routed through the sitesafely.

**GENERAL LOCATION AND DESCRIPTION**

The Widefield Water & Sanitation District (WWSD) Booster 2 Pump Station and associated infrastructure and site improvements are to be constructed within both an un-platted parcel and Tract D of Widefield County Club Heights East both of which contiguous and under the ownership of the WWSD.

The parcels of land are located within a portion of the northwest quarter of the southeast quarter and within a portion of the southwest corner of the northeast quarter of Section 19, Township 15 South, Range 65 West of the 6th P.M. in El Paso County, Colorado. The site to the north and south by developed residential lots (Lot 7 & Lot 8 of Block 1) within Widefield Country Club Heights East and to east by existing Metropolitan Street Rights-of-Way and to the west by a portion of the Fountain Mutual Irrigation Company Channel (FMIC). Flows from this site are tributary to the East Big Johnson drainage basin and are ultimately tributary to Fountain Creek.

With the exception of the existing tank, pump house, fences and underground utilities the two parcels remain largely undeveloped. Vegetation is sparse, consisting of native grasses. The site has likely experienced overlot grading activities associated within the construction of the existing facilities and adjacent subdivision which occurred in the latter part of the 1970's. The existing site terrain generally slopes from east to west at grade rates that vary between 1% and 10%. The side slopes of the adjacent FMIC channel, directly to the west and southwest of the subject possess steep vegetative slopes that typically range between 1:1-2:1 (H:V).

As previously discussed the proposed site improvement will include a new pump station building and gravel access/parking lot. A small asphalt or concrete access drive with cantilever gate and additional security fencing will added along with below ground utility extensions necessary to connect the pump house to the existing tank site and subsequent waterlines. A small storm water collection system and landscaping will also be provided.

**SOILS**

Soils for this project are delineated on the Soils map in the appendix as have been determined to be Truckton Sandy Loam (97) and Wiley Silt Loam which have been characterized as Hydrologic Soil Types "A". Soils in the study area are shown as mapped by S.C.S. in the "Soils Survey of El Paso County Area". As previously discussed, vegetation on the site is sparse, consisting of native grasses and weeds.

## **HYDROLOGIC CALCULATIONS**

Hydrologic calculations were performed using the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual and where applicable the Urban Storm Drainage Criteria Manual. The Rational Method was used to estimate stormwater runoff anticipated from design storms with 5-year and 100-year recurrence intervals.

## **HYDRAULIC CALCULATIONS**

Hydraulic calculations were estimated using the Manning's Formula and the methods described in the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual. The relevant data sheets are included in the appendix of this report.

## **FLOODPLAIN STATEMENT**

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Panel No. 08041C0952 G, effective date December 07, 2018. An annotated FIRM Panel (Floodplain Map) is included in the Appendix to illustrate that NO portion of the site lies within the regulatory 100-year floodplain.

## **DRAINAGE CRITERIA**

This drainage analysis has been prepared in accordance with the current City of Colorado Springs/El Paso County Drainage Criteria Manuals. Calculations were performed to determine runoff quantities for the 5-year and 100-year frequency storms for developed conditions using the Rational Method as required for basins having areas less than 100 acres.

## **FOUR STEP PROCESS/WATER QUALITY**

The proposed project is considered to be development with less than 1 acre of disturbance and is not part of a large common plan of development or sale, as a result no water quality improvements are required per the El Paso County Municipal Separate Storm Sewer System (MS4) Permit.

The proposed and existing conditions have been developed for information purposes and to ensure runoff is safely routed and erosion control is properly implemented. As the proposed earthwork moving activities are anticipated to exceed 500 cubic yards a Stormwater manage Program Plan (SWMP) and Erosion and Stormwater Quality Control Permit (ESQCP) will be provided. A copy of the Proposed Grading and Erosion Control Plans are provided within the appendix of this report.

## **EXISTING DRAINAGE CONDITIONS**

The two parcels that make up the existing site consist of approximately 1.69 acres. With the exception of the existing tank, pump house, fences and underground utilities the parcels remain largely undeveloped. The existing site terrain generally slopes from east to west at grade rates that vary between 1% and 10%. A small ridgeline which runs east to west generally divides the subject site into a north and south basin. With the exception of a small portion of the adjacent residential lot located to the south of the site and a very small portion of the adjacent Rights-of-Way (0.01 ac), the site is not burdened by offsite runoff, having nearly all of the site drainage directed to the adjacent Fountain Mutual Irrigation Company Channel as sheet flow. Onsite inspection shows limited reeling along the channel slopes from subject site

runoff. A small onsite area located at the southeast corner of the site (0.03 acres) discharges to the adjacent Metropolitan street curb and gutter where it is direct to a low point and sump inlet located north of the subject site. Flows intercepted by the inlet appear to be directed to the FMIC via an existing storm sewer (located to the north of lot 7).

### **Detailed Existing Drainage Discussion**

**Basin OS1**, 0.01 acres, ( $Q_5 < 0.1$  cfs,  $Q_{100} < 0.1$  cfs), consists of a small area of existing vegetated rights of way located to the west of the subject site. Runoff produced within the Basin OS1 sheet flows to into Basin A.

**Basin A**, 0.83 acres, ( $Q_5 = 0.3$  cfs,  $Q_{100} = 1.9$  cfs), consists of a portion of the undeveloped Tract D. Runoff produced within the Basin A combines with flows from Basin OS1 and travels as both sheet and concentrated runoff to the northwest and to the existing FMIC channel. The combined runoff from the 0.84 acres totals 0.3 cfs and 2.0 cfs in the 5 and 100 year storm events respectively at east channel bank (Design Point 1).

**Basin OS2**, 0.11 acres, ( $Q_5 = 0.1$  cfs,  $Q_{100} = 0.4$  cfs), consists of a portion of residential Lot 3, Blk 1 of Widefield County Club Heights East. Runoff produced within the Basin OS2 travel west as sheet flow and enters Basin B along the southern property boundary.

**Basin B**, 0.83 acres, ( $Q_5 = 0.4$  cfs,  $Q_{100} = 2.3$  cfs), consists of the majority of the south half of the site and includes the existing pump house and water tank. Runoff from Basin B combines with flows from offsite basin OS2 and travels as sheet and concentrated flow to the south west property line and the existing FMIC channel at Design Point 2 ( $Q_5 = 0.3$  cfs,  $Q_{100} = 2.7$  cfs).

**Basin C**, 0.03 acres, ( $Q_5 < 0.1$  cfs,  $Q_{100} = 0.1$  cfs), consists of a segment of existing vegetated roadway rights of way. Runoff produced within the Basin C sheet flows to the west curb and gutter of existing Metropolitan Street. Flows collected with the west curb and gutter section is conveyed north to an existing offsite sump inlet.

## **PROPOSED DRAINAGE CHARACTERISTICS**

### **General Concept Drainage Discussion**

In the proposed condition the site improvements will include installation of a new pump station building and gravel access/parking lot. A small asphalt or concrete access drive with cantilever gate and additional security fencing will added along with the various below ground utility extensions necessary to connect the pump house to the existing tank site and subsequent waterlines. Runoff within the south half of the two parcels generally not be disturbed and will continue to drain as in the existing condition. While drainage from the north half of the site, where improvements are proposed, will now be collected and conveyed by a pair of earthen swales which will direct runoff to a proposed storm drain located at the far west end of the site. A small offsite easement will be required to construct and maintain the proposed storm sewer pipe and outfall. A riprap pad is proposed at the terminus of the pipe to protect the existing channel from any erosion that otherwise might be caused from the existing concentrated. In addition to the riprap armoring around the proposed flared end section, seeded permanent erosion control blanket is proposed to aid in protecting and restoring vegetation to the steep slope of the FMIC channel, where the channel bank will need to be removed and replaced in order to install the pipe. A small amount of riprap is also planned at the entrance of the storm sewer to stabilize the steep slope at the proposed FES. A detailed drainage discussion for each proposed conditions sub-basin is described

below. A worksheet detailing the calculated coefficients for each sub-basin and developed times of concentration can be found in the appendix under Hydrologic Calculations.

### Detailed Drainage Discussion

**Basin A1**, 0.07 acres, ( $Q_5 < 0.1$  cfs,  $Q_{100} = 0.2$  cfs), consists of a segment of slope grading located along the north and northwest sides of the development. Flows produced within the Basin A1 sheet flow to the existing FMIC channel as in the existing condition. Flows reaching the northwest boundary of the site at Design Point 1 are calculated at less than 0.1 cfs the 5 year event and 0.2 cfs in the 100 year storm event.

**Basin A2**, 0.50 acres, ( $Q_5 = 1.4$  cfs,  $Q_{100} = 2.5$  cfs), consists of a portion of the proposed booster pump house, proposed gravel parking area as well as re-vegetated portions of the existing lot. Runoff from Basin A1 is routed as surface runoff via a min. 1' deep earthen swale to DP1A.

**Basin OS1**, 0.01 acres, ( $Q_5 < 0.1$  cfs,  $Q_{100} < 0.1$  cfs), consists of a small area of existing vegetated rights of way located to the west of the subject site. Runoff produced within the Basin OS1 sheet flows to into Basin A3.

**Basin A3**, 0.35 acres, ( $Q_5 = 0.1$  cfs,  $Q_{100} = 0.4$  cfs), consists of a portion of the proposed booster pump house, proposed gravel parking area as well as re-vegetated portions of the existing lot. Runoff from Basin A3 is routed as surface runoff via a min. 1' deep earthen swale to DP1A, where it ultimately combines with flows from Basins OS1 and A2 at a proposed 18" private reinforced concrete culvert totaling  $Q_5 = 0.8$  cfs,  $Q_{100} = 2.6$  cfs. The installation of a culvert to convey runoff from the site directly to the FMIC, will significantly reduce any potential erosion caused by the uncontrolled release of developed runoff across the steep channel embankment slopes.

JDS Hydro has determined in correspondence with the FMIC that approximately 50 cfs is conveyed by the irrigation channel. Given an calculated slope of 0.05%, a bottom width of approximately 9' and 1 to 1 (H:V) side slopes and a n value of 0.03, the calculated normal depth of the channel is approximately 30" at a velocity of 1.7' feet per second. This low tailwater condition will not adversely affect the proposed hydraulics of the proposed storm culvert given the pipe grade and entrance elevation and limited runoff there by allowing for free discharge. Given the design slope of 13.5% the proposed 18" culvert will convey the 2.6 cfs at a normal depth of 3.2" and at a velocity of 12.4 feet per second in the 100 year event. Refer to the Hydraulic Calculations portion of the appendix for additional information.

**Basin OS2**, 0.11 acres, ( $Q_5 = 0.1$  cfs,  $Q_{100} = 0.4$  cfs), consists of a portion of residential Lot 3, Blk 1 of Widefield County Club Heights East. Runoff produced within the Basin OS2 travel west as sheet flow and enters Basin B along the southern property boundary.

**Basin B**, 0.74 acres, ( $Q_5 = 0.4$  cfs,  $Q_{100} = 2.1$  cfs), consists of the majority of the south half of the site and includes the existing pump house and water tank. Runoff from Basin B combines with flows from offsite basin OS2 and travels as sheet and concentrated flow to the south west property line and the existing FMIC channel at Design Point 2 ( $Q_5 = 0.6$  cfs,  $Q_{100} = 2.9$  cfs).

**Basin C**, 0.03 acres, ( $Q_5 < 0.1$  cfs,  $Q_{100} = 0.1$  cfs), consists of a segment of existing vegetated roadway rights of way. Runoff produced within the Basin C sheet flows to the west curb and gutter of existing Metropolitan Street. Flows collected with the west curb and gutter section is conveyed north to an existing offsite sump inlet.

It should be noted that the estimated combined runoff discharged from the site in the existing condition totals ~ 4.7 cfs (direct addition of DP1, DP1A & DP2) while the total discharge from the proposed

condition is only slightly higher at 5.7 cfs (direct addition of DP1&DP2). These minor increases in flows of approximately 1 cfs are considered negligible. With the installation of the recommended infrastructure, and the presence of an adequate outfall structure, no negative effects are anticipated to water quality or downstream facilities. Furthermore thru the implementation of outfall protection , slope stabilization as well as site stabilization (reseeding) the potential of erosion from construction and function should be mitigated.

**EROSION CONTROL**

It is the policy of the El Paso County that we submit a grading and erosion control plan with the drainage report. Proposed erosion control blanket, silt fence, vehicle traffic control, concrete washout area are proposed as erosion control measures. Recommendations for Stabilized staging area and stockpile management are also included.

**CONSTRUCTION COST OPINION**

(Private) Drainage Facilities NON-Reimbursable:

<b>Item</b>	<b>Description</b>	<b>Quantity</b>	<b>Unit Cost</b>	<b>Cost</b>
1.	18" Storm Sewer	50 LF	\$60 /LF	\$3,000.00
2.	18" Flared End Sections	2 EA	\$900 /EA	\$1,800.00
3.	D50=9" Riprap	5 CY	\$50 /CYF	\$250.00
4.	Erosion Control Blanket	26 SY	\$9 /SY	\$234.00
				\$5,284.00
<b>Total \$</b>				<b>\$5,284.00</b>

**DRAINAGE & BRIDGE FEES**

The site is currently platted as a Tract. No re-plat is being submitted and the structure is being constructed on the Tract via administrative relief (ADM-19-026) in which it was determined that the proposed project will not require a "1041" permit. Thus no drainage and bridge fees are required for the development of the site.

M &S Civil Consultants, Inc. (M&S) cannot and does not guarantee the construction cost will not vary from these opinions of probable costs. These opinions represent our best judgment as design professionals familiar with the construction industry and this development in particular. The above is only an estimate of the facility cost and drainage basin fee amounts in 2017.

**SUMMARY**

The installation of the proposed facilities associated the construction of Widefield Water and Sanitation District Booster 2 Pump Station site shall not adversely affect adjacent or downstream properties per this final drainage report. The proposed drainage facilities will adequately convey, detain and route runoff from tributary onsite and existing offsite flows to the Sand Creek Drainage channel. Maintenance of the proposed private facilities shall be the responsibility of the WWSD.

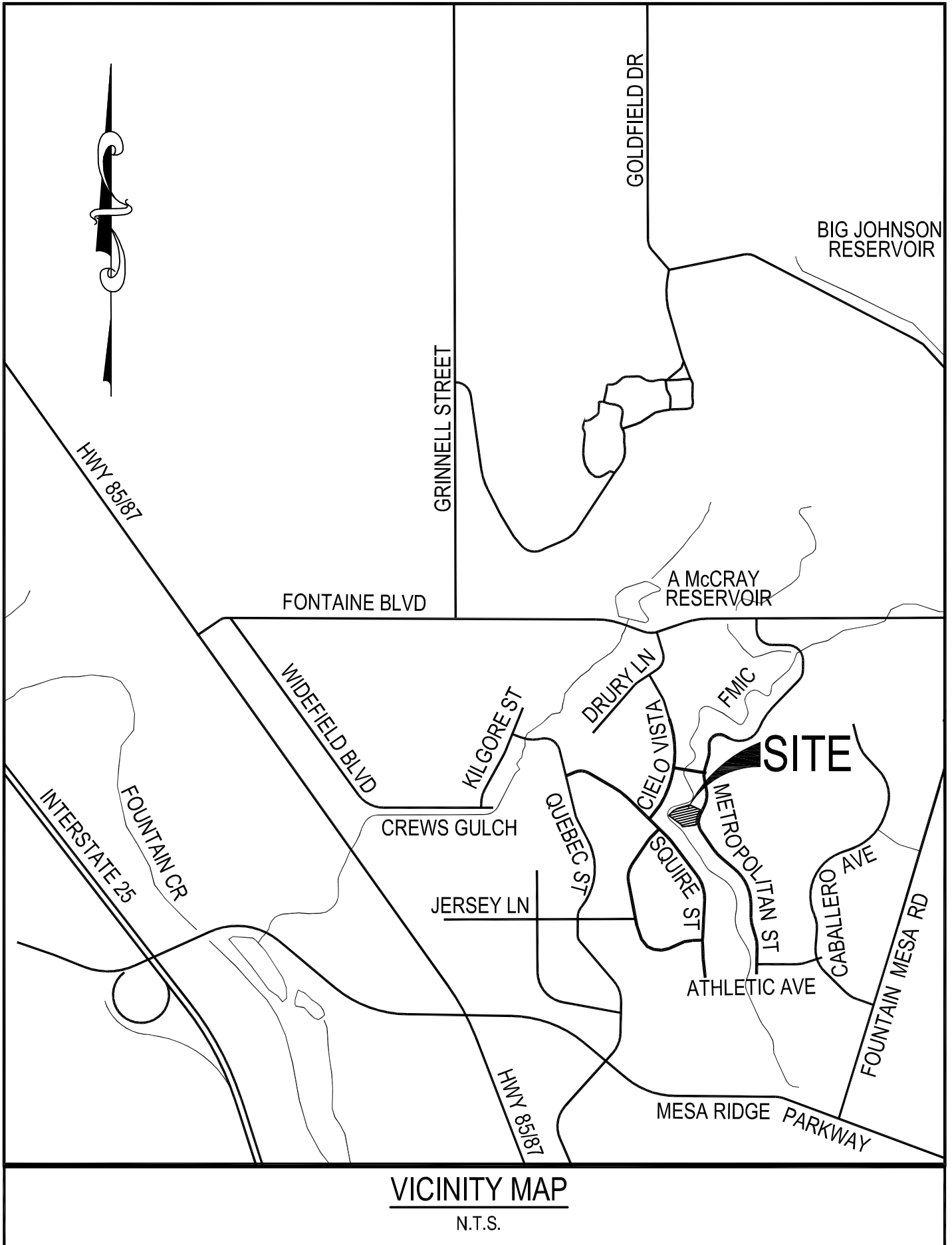


## REFERENCES

- 1.) "El Paso County and City of Colorado Springs Drainage Criteria Manual".
- 2.) "Urban Storm Drainage Criteria Manual"
- 3.) SCS Soils Map for El Paso County.
- 4.) Flood Insurance Rate Map (FIRM), Federal Emergency Management Agency, Effective date December 7, 2018.

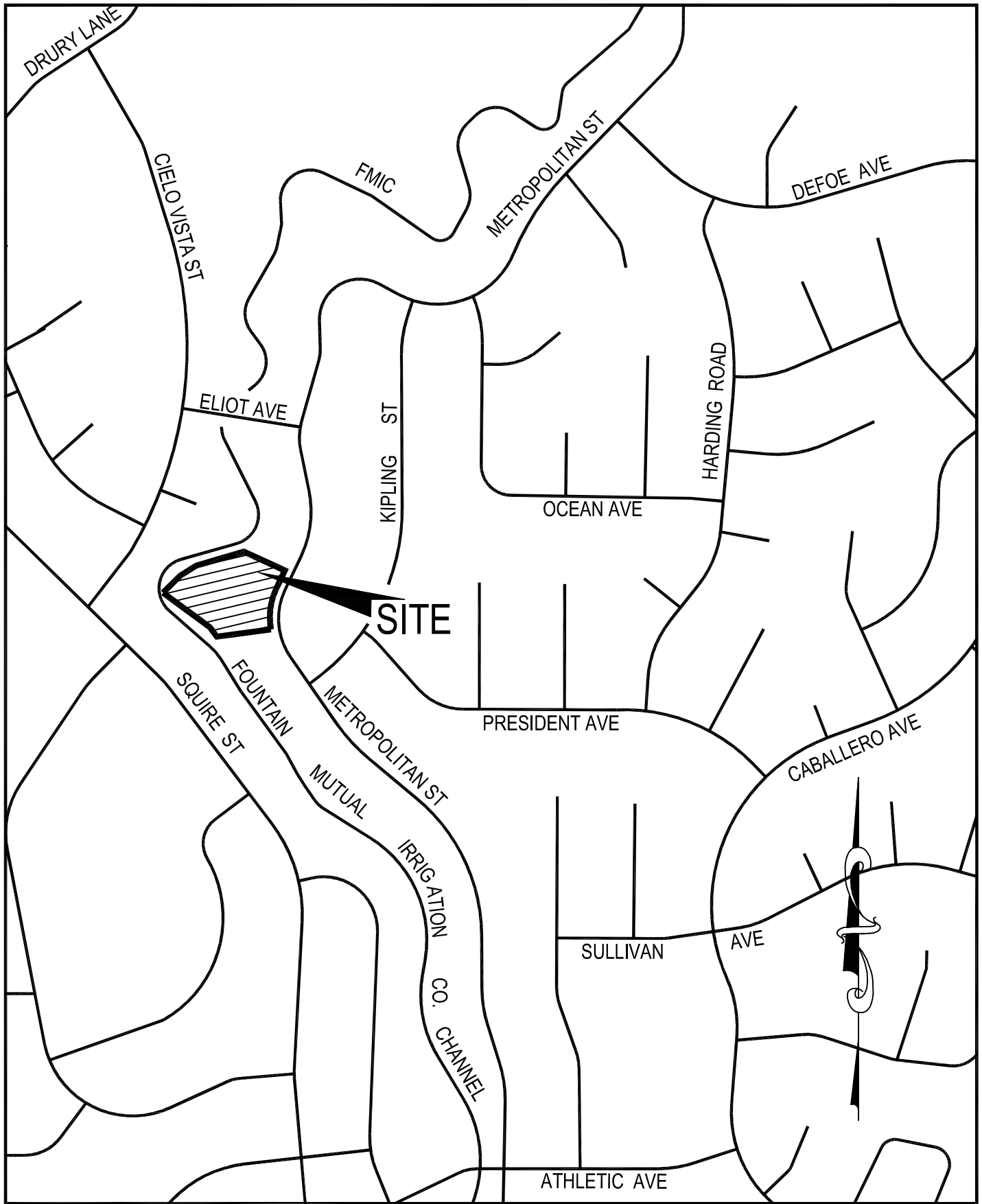
## **APPENDIX**

**VICINITY MAP**



**VICINITY MAP**

N.T.S.



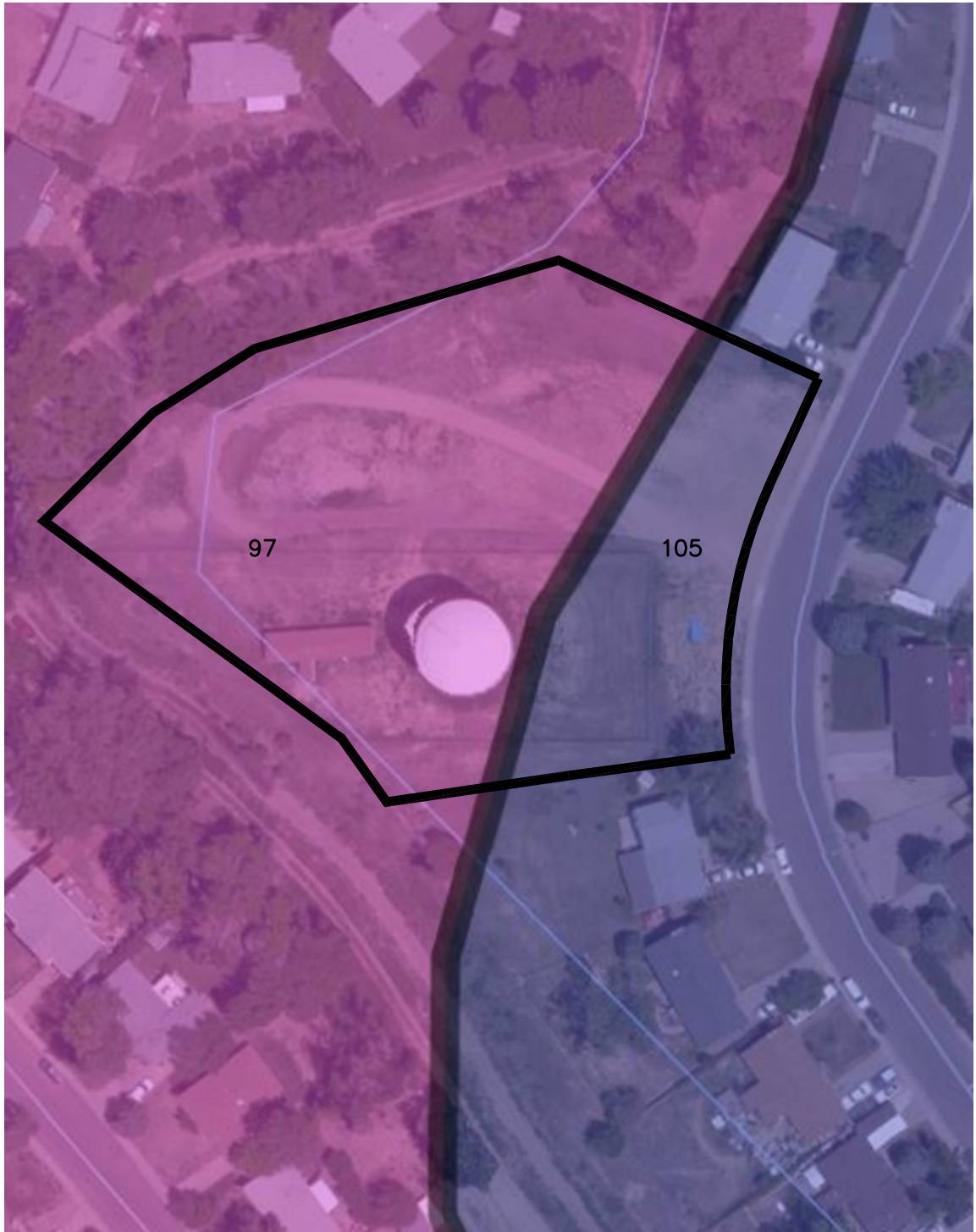
**SITE MAP**

N.T.S.

**SOILS MAP**



NOT TO SCALE



MAP UNIT SYMBOL	MAP UNIT NAME	RATING
97	Truckton sandy loam, 3 to 9 percent slopes	A
108	Wiley silt loam, 3 to 9 percent slopes	B

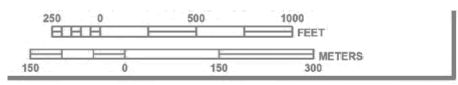
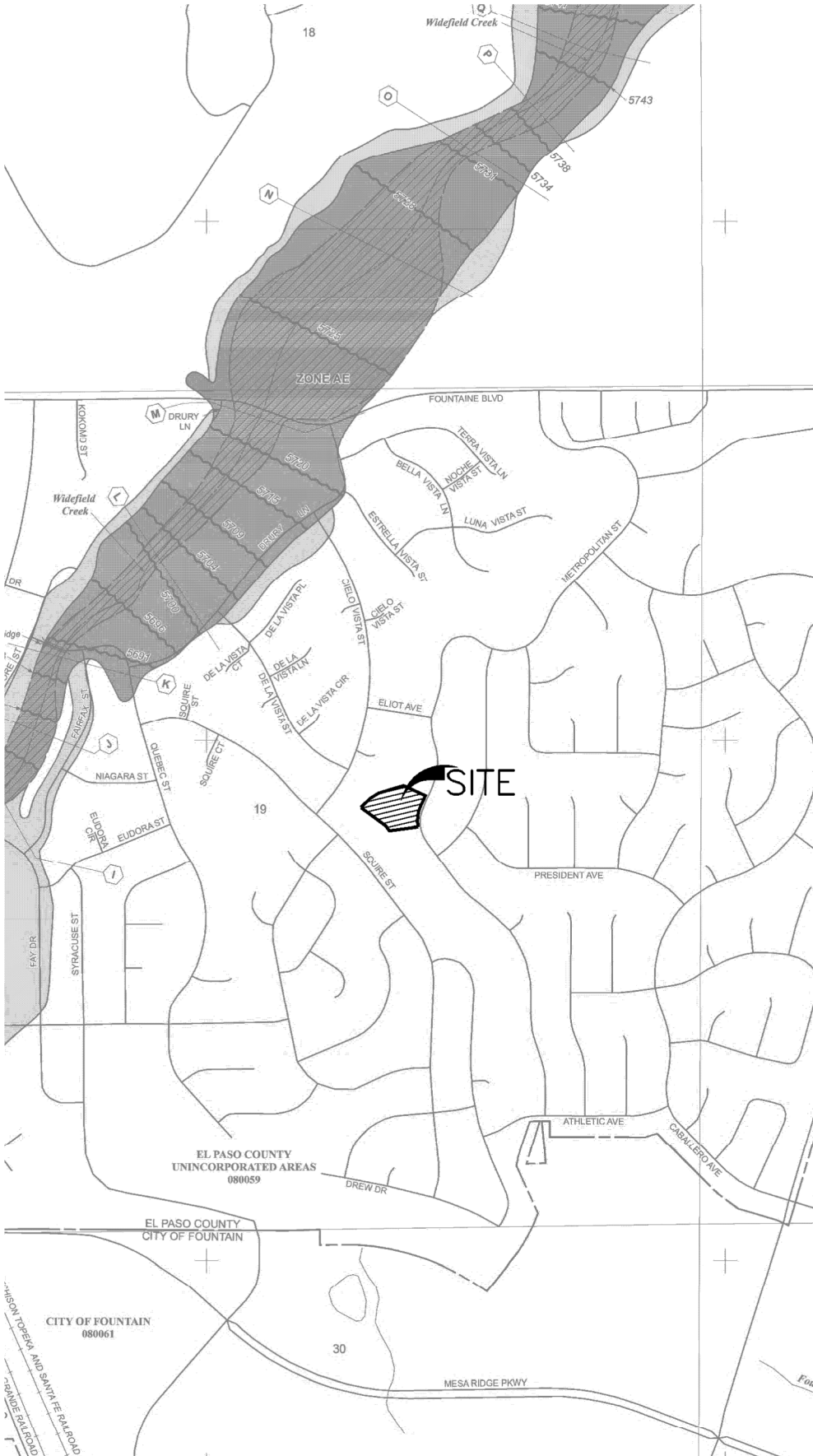
WIDEFIELD  
WATER AND  
SANITATION  
DISTRICT  
BOOSTER 2  
PUMP STATION

### SOILS MAP



**ANNOTATED FIRM PANEL**





**NFIP**

**PANEL 0952G**

**FIRM**  
**FLOOD INSURANCE RATE MAP**  
**EL PASO COUNTY, COLORADO**  
**AND INCORPORATED AREAS**

**PANEL 952 OF 1300**  
 (SEE MAP INDEX FOR FIRM PANEL LAYOUT)

**CONTAINS:**

COMMUNITY	NUMBER	PANEL	SUFFIX
EL PASO COUNTY	08059	0952	G
FOUNTAIN, CITY OF	08061	0552	G

Notice to User: The Map Number shown below should be used when placing map orders; the Community Number shown above should be used on insurance applications for the subject community.

**MAP NUMBER**  
**08041C0952G**

**MAP REVISED**  
**DECEMBER 7, 2018**

Federal Emergency Management Agency

**NATIONAL FLOOD INSURANCE PROGRAM**

WIDEFIELD WATER AND  
 SANITATION DISTRICT  
 BOOSTER 2 PUMP STATION

FLOODPLAIN MAP



## **HYDROLOGIC CALCULATIONS**

**WIDEFIELD WATER AND WASTEWATER DISTRICT - BOOSTER 2 PUMP STATION**  
**EXISTING CONDITIONS DRAINAGE CALCULATIONS**  
*(Area Runoff Coefficient Summary)*

BASIN	TOTAL AREA (SF)	TOTAL AREA (Acres)										RUNOFF COEFFICIENT	
			AREA (Acres)	C <sub>5</sub>	C <sub>100</sub>	AREA (Acres)	C <sub>5</sub>	C <sub>100</sub>	AREA (Acres)	C <sub>5</sub>	C <sub>100</sub>	C <sub>5</sub>	C <sub>100</sub>
<i>A</i>	<i>36008.1896</i>	<i>0.83</i>	<i>0.00</i>	<i>0.90</i>	<i>0.96</i>	<i>0.00</i>	<i>0.59</i>	<i>0.70</i>	<i>0.83</i>	<i>0.09</i>	<i>0.36</i>	<i>0.09</i>	<i>0.36</i>
<i>B</i>	<i>36198.4781</i>	<i>0.83</i>	<i>0.03</i>	<i>0.90</i>	<i>0.96</i>	<i>0.00</i>	<i>0.59</i>	<i>0.70</i>	<i>0.80</i>	<i>0.09</i>	<i>0.36</i>	<i>0.12</i>	<i>0.38</i>
<i>C</i>	<i>1281.6639</i>	<i>0.03</i>	<i>0.00</i>	<i>0.90</i>	<i>0.96</i>	<i>0.00</i>	<i>0.59</i>	<i>0.70</i>	<i>0.03</i>	<i>0.09</i>	<i>0.36</i>	<i>0.09</i>	<i>0.36</i>
<i>OS1</i>	<i>520.2481</i>	<i>0.01</i>	<i>0.00</i>	<i>0.90</i>	<i>0.96</i>	<i>0.00</i>	<i>0.30</i>	<i>0.50</i>	<i>0.01</i>	<i>0.09</i>	<i>0.36</i>	<i>0.09</i>	<i>0.36</i>
<i>OS2</i>	<i>4830.2806</i>	<i>0.11</i>	<i>0.02</i>	<i>0.90</i>	<i>0.96</i>	<i>0.00</i>	<i>0.30</i>	<i>0.50</i>	<i>0.10</i>	<i>0.09</i>	<i>0.36</i>	<i>0.20</i>	<i>0.44</i>

**WIDFIELD WATER AND WASTEWATER DISTRICT - BOOSTER 2 PUMP STATION**  
**EXISTING CONDITIONS DRAINAGE CALCULATIONS**  
**(Area Drainage Summary)**

<i>From Area Runoff Coefficient Summary</i>				<b>OVERLAND</b>				<b>STREET / CHANNEL FLOW</b>				<b>Time of Travel (T<sub>t</sub>)</b>		<b>INTENSITY *</b>		<b>TOTAL FLOWS</b>	
<b>BASIN</b>	<b>AREA TOTAL (Acres)</b>	<b>C<sub>5</sub></b>	<b>C<sub>100</sub></b>	<b>C<sub>5</sub></b>	<b>Length (ft)</b>	<b>Height (ft)</b>	<b>T<sub>c</sub> (min)</b>	<b>Length (ft)</b>	<b>Slope (%)</b>	<b>Velocity (fps)</b>	<b>T<sub>t</sub> (min)</b>	<b>TOTAL (min)</b>	<b>CHECK (min)</b>	<b>I<sub>5</sub> (in/hr)</b>	<b>I<sub>100</sub> (in/hr)</b>	<b>Q<sub>5</sub> (c.f.s.)</b>	<b>Q<sub>100</sub> (c.f.s.)</b>
		<i>From DCM Table 5-1</i>															
<b>A</b>	0.83	0.09	0.36	0.09	100	5.5	10.4	90	2.9%	1.2	1.3	11.7	11.1	3.9	6.5	0.3	1.9
<b>B</b>	0.83	0.12	0.38	0.12	60	4	7.3	150	5.3%	1.6	1.5	8.9	11.2	4.3	7.2	0.4	2.3
<b>C</b>	0.03	0.09	0.36	0.09	15	0.3	5.6	0	0.0%	0.0	0.0	5.6	10.1	5.0	8.4	0.0	0.1
<b>OS1</b>	0.01	0.09	0.36	0.09	10	0.2	4.6	0	0.0%	0.0	0.0	4.6	10.1	5.3	8.9	0.0	0.0
<b>OS2</b>	0.11	0.20	0.44	0.20	50	2	7.3	0	0.0%	0.0	0.0	7.3	10.3	4.6	7.7	0.1	0.4

\* Intensity equations assume a minimum travel time of 5 minutes.

Calculated by: DLM  
Date: 8/26/2019  
Checked by: VAS

**WIDEFIELD WATER AND WASTEWATER DISTRICT - BOOSTER 2 PUMP STATION**  
**EXISTING CONDITIONS DRAINAGE CALCULATIONS**  
**(Basin Routing Summary)**

<i>From Area Runoff Coefficient Summary</i>				<b>OVERLAND</b>				<b>PIPE / CHANNEL FLOW</b>				<b>Time of Travel (T<sub>t</sub>)</b>	<b>INTENSITY *</b>		<b>TOTAL FLOWS</b>		COMMENTS	
DESIGN POINT	CONTRIBUTING BASINS	CA <sub>5</sub>	CA <sub>100</sub>	C <sub>s</sub>	Length (ft)	Height (ft)	T <sub>c</sub> (min)	Length (ft)	Slope (%)	Velocity (fps)	T <sub>t</sub> (min)	TOTAL (min)	I <sub>5</sub> (in/hr)	I <sub>100</sub> (in/hr)	Q <sub>5</sub> (c.f.s.)	Q <sub>100</sub> (c.f.s.)		
1	OS1, A	0.08	0.30									11.7	3.9	6.5	0.3	2.0		
				Basin B Tc was used														
2	OS2, B, C	0.13	0.38									8.9	4.3	7.2	0.6	2.7		
				Basin A Tc was used														

**WIDEFIELD WATER AND WASTEWATER DISTRICT - BOOSTER 2 PUMP STATION**  
**PROPOSED CONDITIONS DRAINAGE CALCULATIONS**  
*(Area Runoff Coefficient Summary)*

BASIN	TOTAL AREA (SF)	TOTAL AREA (Acres)										RUNOFF COEFFICIENT	
			AREA (Acres)	C <sub>5</sub>	C <sub>100</sub>	AREA (Acres)	C <sub>5</sub>	C <sub>100</sub>	AREA (Acres)	C <sub>5</sub>	C <sub>100</sub>	C <sub>5</sub>	C <sub>100</sub>
<i>A1</i>	<i>2864.3099</i>	<i>0.07</i>	<i>0.00</i>	<i>0.90</i>	<i>0.96</i>	<i>0.00</i>	<i>0.59</i>	<i>0.70</i>	<i>0.07</i>	<i>0.09</i>	<i>0.36</i>	<i>0.09</i>	<i>0.36</i>
<i>A2</i>	<i>21650.5603</i>	<i>0.50</i>	<i>0.06</i>	<i>0.90</i>	<i>0.96</i>	<i>0.09</i>	<i>0.59</i>	<i>0.70</i>	<i>0.35</i>	<i>0.09</i>	<i>0.36</i>	<i>0.27</i>	<i>0.49</i>
<i>A3</i>	<i>15244.5339</i>	<i>0.35</i>	<i>0.02</i>	<i>0.90</i>	<i>0.96</i>	<i>0.07</i>	<i>0.59</i>	<i>0.70</i>	<i>0.27</i>	<i>0.09</i>	<i>0.36</i>	<i>0.22</i>	<i>0.45</i>
<i>B</i>	<i>32440.8858</i>	<i>0.74</i>	<i>0.03</i>	<i>0.90</i>	<i>0.96</i>	<i>0.00</i>	<i>0.59</i>	<i>0.70</i>	<i>0.71</i>	<i>0.09</i>	<i>0.36</i>	<i>0.13</i>	<i>0.39</i>
<i>C</i>	<i>1281.6639</i>	<i>0.03</i>	<i>0.00</i>	<i>0.90</i>	<i>0.96</i>	<i>0.00</i>	<i>0.59</i>	<i>0.70</i>	<i>0.03</i>	<i>0.09</i>	<i>0.36</i>	<i>0.09</i>	<i>0.36</i>
<i>OS1</i>	<i>207.67</i>	<i>0.00</i>	<i>0.00</i>	<i>0.90</i>	<i>0.96</i>	<i>0.00</i>	<i>0.30</i>	<i>0.50</i>	<i>0.00</i>	<i>0.09</i>	<i>0.36</i>	<i>0.09</i>	<i>0.36</i>
<i>OS2</i>	<i>4830.2806</i>	<i>0.11</i>	<i>0.02</i>	<i>0.90</i>	<i>0.96</i>	<i>0.00</i>	<i>0.30</i>	<i>0.50</i>	<i>0.10</i>	<i>0.09</i>	<i>0.36</i>	<i>0.20</i>	<i>0.44</i>

**WIDFIELD WATER AND WASTEWATER DISTRICT - BOOSTER 2 PUMP STATION  
PROPOSED CONDITIONS DRAINAGE CALCULATIONS  
(Area Drainage Summary)**

From Area Runoff Coefficient Summary				OVERLAND				STREET / CHANNEL FLOW				Time of Travel ( $T_t$ )		INTENSITY *		TOTAL FLOWS	
BASIN	AREA TOTAL (Acres)	C <sub>5</sub>	C <sub>100</sub>	C <sub>5</sub>	Length (ft)	Height (ft)	T <sub>c</sub> (min)	Length (ft)	Slope (%)	Velocity (fps)	T <sub>i</sub> (min)	TOTAL (min)	CHECK (min)	I <sub>5</sub> (in/hr)	I <sub>100</sub> (in/hr)	Q <sub>5</sub> (c.f.s.)	Q <sub>100</sub> (c.f.s.)
		From DCM Table 5-1															
<i>A1</i>	0.07	0.09	0.36	0.09	10	0.5	3.4	0	0.0%	0.0	0.0	5.0	10.1	5.2	8.7	0.0	0.2
<i>A2</i>	0.50	0.27	0.49	0.27	90	5.5	7.8	285	1.9%	1.4	3.4	11.2	12.1	3.8	6.5	0.5	1.6
<i>A3</i>	0.35	0.22	0.45	0.22	90	5.5	8.3	250	2.2%	1.5	2.8	11.1	11.9	3.9	6.5	0.3	1.0
<i>B</i>	0.74	0.13	0.39	0.13	60	4	7.3	150	5.3%	1.6	1.5	8.8	11.2	4.3	7.2	0.4	2.1
<i>C</i>	0.03	0.09	0.36	0.09	15	0.3	5.6	0	0.0%	0.0	0.0	5.6	10.1	5.0	8.4	0.0	0.1
<i>OS1</i>	0.00	0.09	0.36	0.09	10	0.2	4.6	0	0.0%	0.0	0.0	5.0	10.1	5.2	8.7	0.0	0.0
<i>OS2</i>	0.11	0.20	0.44	0.20	50	2	7.3	0	0.0%	0.0	0.0	7.3	10.3	4.6	7.7	0.1	0.4

\* Intensity equations assume a minimum travel time of 5 minutes.

Calculated by: DLM  
Date: 8/26/2019  
Checked by: VAS

**WIDEFIELD WATER AND WASTEWATER DISTRICT - BOOSTER 2 PUMP STATION**  
**PROPOSED CONDITIONS DRAINAGE CALCULATIONS**  
**(Basin Routing Summary)**

<i>From Area Runoff Coefficient Summary</i>				<b>OVERLAND</b>				<b>PIPE / CHANNEL FLOW</b>				<b>Time of Travel (T<sub>t</sub>)</b>	<b>INTENSITY *</b>		<b>TOTAL FLOWS</b>		COMMENTS
DESIGN POINT	CONTRIBUTING BASINS	CA <sub>5</sub>	CA <sub>100</sub>	C <sub>s</sub>	Length (ft)	Height (ft)	T <sub>c</sub> (min)	Length (ft)	Slope (%)	Velocity (fps)	T <sub>t</sub> (min)	TOTAL (min)	I <sub>5</sub> (in/hr)	I <sub>100</sub> (in/hr)	Q <sub>5</sub> (c.f.s.)	Q <sub>100</sub> (c.f.s.)	
1	A1	0.01	0.02									5.0	5.2	8.7	<b>0.0</b>	<b>0.2</b>	Sheet flows to FMIR
				Basin A1 Tc was used													
1A	OS1, A2, A3	0.21	0.40									11.9	3.9	6.5	<b>0.8</b>	<b>2.6</b>	Prop 18" RCP (private)
				Basin A3 Tc was used													
2	OS2, B, C	0.12	0.35									5.6	5.0	8.4	<b>0.6</b>	<b>2.9</b>	Sheet flows to FMIR
				Basin C Tc was used													



## **HYDRAULIC CALCULATIONS**

## Worksheet for 18" Pipe @ 13.5% - 2.6 cfs

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth
Input Data	
Roughness Coefficient	0.013
Channel Slope	0.135 ft/ft
Diameter	18.0 in
Discharge	2.60 cfs
Results	
Normal Depth	3.2 in
Flow Area	0.2 ft <sup>2</sup>
Wetted Perimeter	1.3 ft
Hydraulic Radius	1.9 in
Top Width	1.14 ft
Critical Depth	7.3 in
Percent Full	17.6 %
Critical Slope	0.005 ft/ft
Velocity	12.44 ft/s
Velocity Head	2.40 ft
Specific Energy	2.67 ft
Froude Number	5.125
Maximum Discharge	41.52 cfs
Discharge Full	38.59 cfs
Slope Full	0.001 ft/ft
Flow Type	Supercritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Average End Depth Over Rise	0.0 %
Normal Depth Over Rise	17.6 %
Downstream Velocity	Infinity ft/s
Upstream Velocity	Infinity ft/s
Normal Depth	3.2 in
Critical Depth	7.3 in
Channel Slope	0.135 ft/ft
Critical Slope	0.005 ft/ft

## Worksheet for FMIC Irrigation Channel - 50 cfs

Project Description	
Friction Method	Manning Formula
Solve For	Normal Depth
Input Data	
Roughness Coefficient	0.030
Channel Slope	0.001 ft/ft
Left Side Slope	1.000 H:V
Right Side Slope	1.000 H:V
Bottom Width	9.00 ft
Discharge	50.00 cfs
Results	
Normal Depth	30.3 in
Flow Area	29.1 ft <sup>2</sup>
Wetted Perimeter	16.1 ft
Hydraulic Radius	21.6 in
Top Width	14.05 ft
Critical Depth	11.4 in
Critical Slope	0.015 ft/ft
Velocity	1.72 ft/s
Velocity Head	0.05 ft
Specific Energy	2.57 ft
Froude Number	0.211
Flow Type	Subcritical
GVF Input Data	
Downstream Depth	0.0 in
Length	0.0 ft
Number Of Steps	0
GVF Output Data	
Upstream Depth	0.0 in
Profile Description	N/A
Profile Headloss	0.00 ft
Downstream Velocity	0.00 ft/s
Upstream Velocity	0.00 ft/s
Normal Depth	30.3 in
Critical Depth	11.4 in
Channel Slope	0.001 ft/ft
Critical Slope	0.015 ft/ft

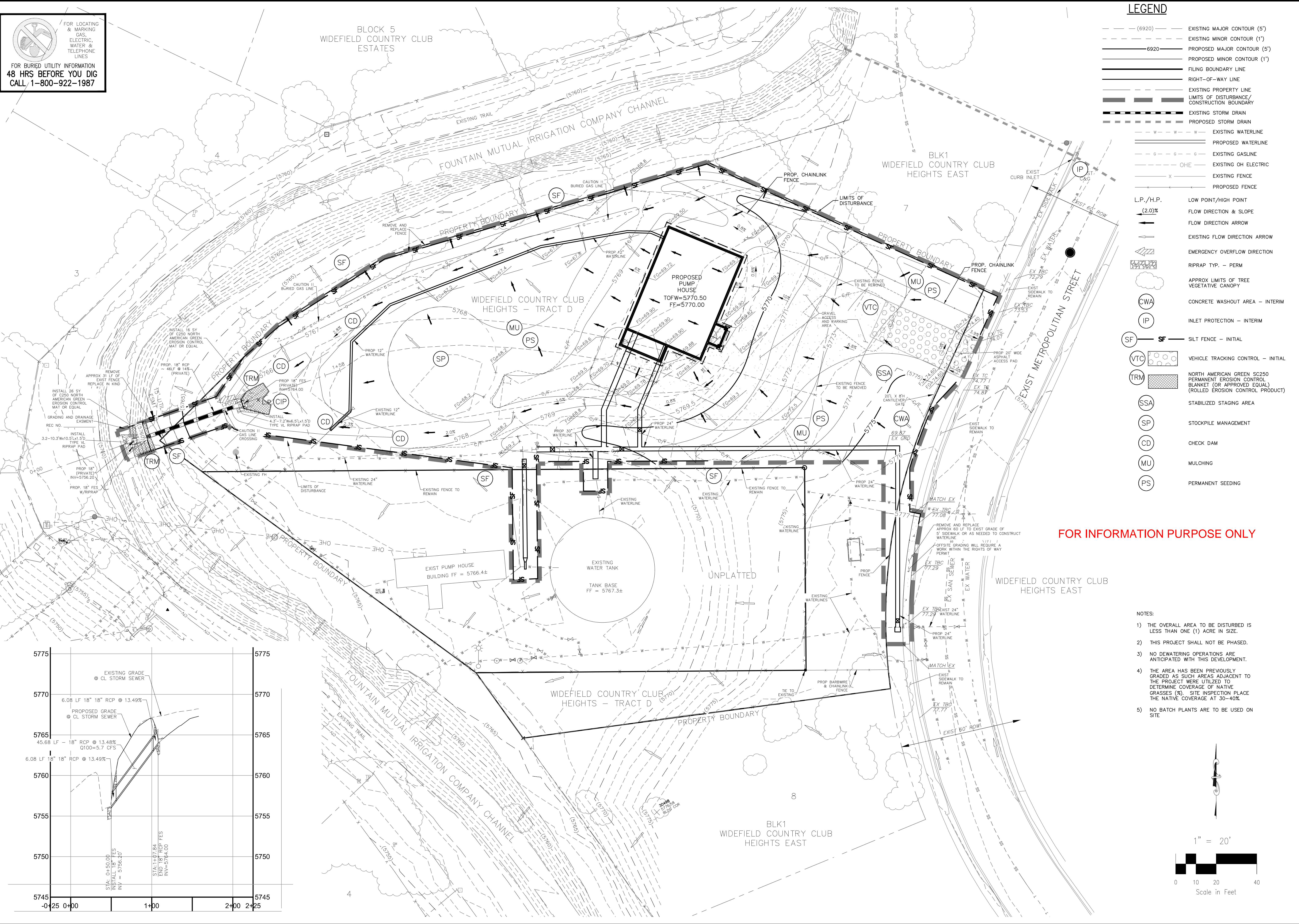
**GRADING AND EROSION CONTROL PLAN**



FOR LOCATING & MARKING GAS, ELECTRIC, WATER & TELEPHONE LINES

FOR BURIED UTILITY INFORMATION  
48 HRS BEFORE YOU DIG  
CALL 1-800-922-1987

BLOCK 5  
WIDEFIELD COUNTRY CLUB  
ESTATES

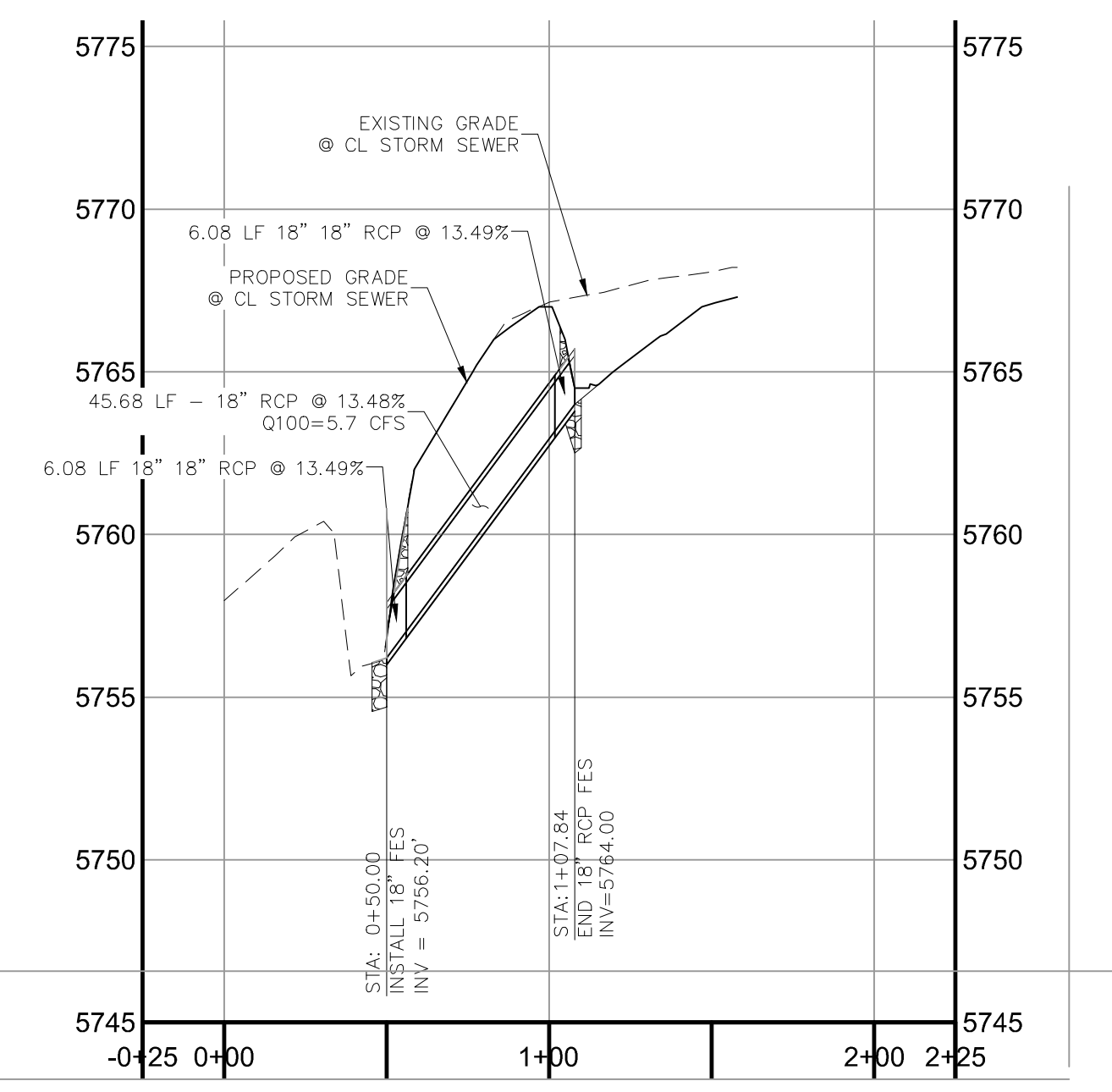
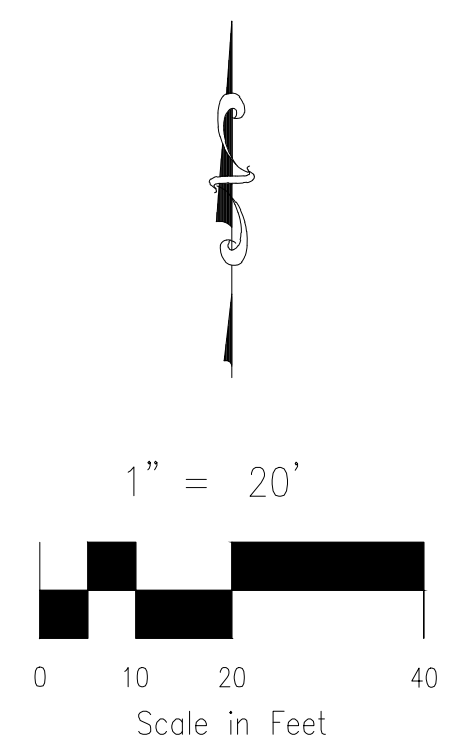


**LEGEND**

	EXISTING MAJOR CONTOUR (5')
	EXISTING MINOR CONTOUR (1')
	PROPOSED MAJOR CONTOUR (5')
	PROPOSED MINOR CONTOUR (1')
	FILING BOUNDARY LINE
	RIGHT-OF-WAY LINE
	EXISTING PROPERTY LINE
	LIMITS OF DISTURBANCE/CONSTRUCTION BOUNDARY
	EXISTING STORM DRAIN
	PROPOSED STORM DRAIN
	EXISTING WATERLINE
	PROPOSED WATERLINE
	EXISTING GASLINE
	EXISTING OH ELECTRIC
	EXISTING FENCE
	PROPOSED FENCE
	L.P./H.P. (2.0%)
	FLOW DIRECTION & SLOPE
	FLOW DIRECTION ARROW
	EXISTING FLOW DIRECTION ARROW
	EMERGENCY OVERFLOW DIRECTION
	RIPRAP TYP. - PERM
	APPROX LIMITS OF TREE VEGETATIVE CANOPY
	CONCRETE WASHOUT AREA - INTERIM
	INLET PROTECTION - INTERIM
	SILT FENCE - INITIAL
	VEHICLE TRACKING CONTROL - INITIAL
	NORTH AMERICAN GREEN SC250 PERMANENT EROSION CONTROL BLANKET (OR APPROVED EQUAL) (ROLLED EROSION CONTROL PRODUCT)
	STABILIZED STAGING AREA
	STOCKPILE MANAGEMENT
	CHECK DAM
	MULCHING
	PERMANENT SEEDING

FOR INFORMATION PURPOSE ONLY

- NOTES:
- 1) THE OVERALL AREA TO BE DISTURBED IS LESS THAN ONE (1) ACRE IN SIZE.
  - 2) THIS PROJECT SHALL NOT BE PHASED.
  - 3) NO DEWATERING OPERATIONS ARE ANTICIPATED WITH THIS DEVELOPMENT.
  - 4) THE AREA HAS BEEN PREVIOUSLY GRADED AS SUCH AREAS ADJACENT TO THE PROJECT WERE UTILIZED TO DETERMINE COVERAGE OF NATIVE GRASSES (%). SITE INSPECTION PLACE THE NATIVE COVERAGE AT 30-40%.
  - 5) NO BATCH PLANTS ARE TO BE USED ON SITE



**WWS BOOSTER 2 PUMP STATION**

**FINAL GRADING / EROSION CONTROL PLAN**

DATE: 02-04-2022

PROJECT NO. 70-077

SCALE: HORIZONTAL: 1"=20' VERTICAL: N/A

DESIGNED BY: DLM

DRAWN BY: DLM

CHECKED BY: VAS

SHEET 2 OF 8

FCR02

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102 E. PINE PEAK AVE. 5TH FLOOR  
COLORADO SPRINGS, CO 80903  
PHONE: 719.555.4485

CIVIL CONSULTANTS, INC.

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FOR AND ON BEHALF OF  
M&S CIVIL CONSULTANTS, INC.

---

100% DESIGN DRAWINGS

REVISIONS:

NO.	DATE	BY	DESCRIPTION	APPROVED BY	DATE

THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE OR LIABLE FOR UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.

CAUTION



**SC-6 Inlet Protection (IP)**

- IP-3. Rock Sock Inlet Protection for Sump/Area Inlet
- IP-4. Silt Fence Inlet Protection for Sump/Area Inlet
- IP-5. Over-excavation Inlet Protection
- IP-6. Straw Bale Inlet Protection for Sump/Area Inlet
- CIP-1. Culvert Inlet Protection

Proprietary inlet protection devices should be installed in accordance with manufacturer specifications. More information is provided below on selecting inlet protection for sump and on-grade locations.

**Inlets Located in a Sump**

When applying inlet protection in sump conditions, it is important that the inlet continue to function during larger runoff events. For curb inlets, the maximum height of the protective barrier should be below the top of the curb opening to allow overflow into the inlet during larger storms without excessive localized flooding. If the inlet protection height is greater than the curb elevation, particularly if the filter becomes clogged with sediment, runoff will not enter the inlet and may bypass it, possibly causing localized flooding, public safety issues, and downstream erosion and damage from bypassed flows.

Area inlets located in a sump setting can be protected through the use of silt fence, concrete block and rock socks (on paved surfaces), sediment control logs/straw wattles embedded in the adjacent soil and stacked around the area inlet (on pervious surfaces), over-excavation around the inlet, and proprietary products providing equivalent functions.

**Inlets Located on a Slope**

For curb and gutter inlets on paved sloping streets, block and rock sock inlet protection is recommended in conjunction with curb socks in the gutter leading to the inlet. For inlets located along unpaved roads, also see the Check Dam Fact Sheet.

**Maintenance and Removal**

Inspect inlet protection frequently. Inspection and maintenance guidance includes:

- Inspect for tears that can result in sediment directly entering the inlet, as well as result in the contents of the BMP (e.g., gravel) washing into the inlet.
- Check for improper installation resulting in untreated flows bypassing the BMP and directly entering the inlet or bypassing to an unprotected downstream inlet. For example, silt fence that has not been properly trenched around the inlet can result in flows under the silt fence and directly into the inlet.
- Look for displaced BMPs that are no longer protecting the inlet. Displacement may occur following larger storm events that wash away or reposition the inlet protection. Traffic or equipment may also crush or displace the BMP.
- Monitor sediment accumulation upgradient of the inlet protection.

IP-2 Urban Drainage and Flood Control District August 2013  
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**SC-6 Inlet Protection (IP)**

**GENERAL INLET PROTECTION INSTALLATION NOTES**

- SEE PLAN VIEW FOR:
  - LOCATION OF INLET PROTECTION.
  - TYPE OF INLET PROTECTION (IP-1, IP-2, IP-3, IP-4, IP-5, IP-6)
- INLET PROTECTION SHALL BE INSTALLED PROMPTLY AFTER INLET CONSTRUCTION OR PAVING IS COMPLETE (TYPICALLY WITHIN 48 HOURS). IF A RAINFALL/RUNOFF EVENT IS FORECAST, INSTALL INLET PROTECTION PRIOR TO ONSET OF EVENT.
- MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

**INLET PROTECTION MAINTENANCE NOTES**

- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
- FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
- WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
- SEDIMENT ACCUMULATED UPSTREAM OF INLET PROTECTION SHALL BE REMOVED AS NECESSARY TO MAINTAIN BMP EFFECTIVENESS, TYPICALLY WHEN STORAGE VOLUME REACHES SOLE OF CAPACITY, A DEPTH OF 6" WHEN SILT FENCE IS USED, OR 1/4 OF THE HEIGHT FOR STRAW BALES.
- INLET PROTECTION IS TO REMAIN IN PLACE UNTIL THE UPSTREAM DISTURBED AREA IS PERMANENTLY STABILIZED, UNLESS THE LOCAL JURISDICTION APPROVES EARLIER REMOVAL OF INLET PROTECTION IN STREETS.
- WHEN INLET PROTECTION AT AREA INLETS IS REMOVED, THE DISTURBED AREA SHALL BE COVERED WITH TOP SOIL, SEEDING AND MULCHED, OR OTHERWISE STABILIZED IN A MANNER APPROVED BY THE LOCAL JURISDICTION.

(DETAIL ADAPTED FROM TOWN OF PARKER, COLORADO AND CITY OF AURORA, COLORADO, NOT AVAILABLE IN AUTOCAD)

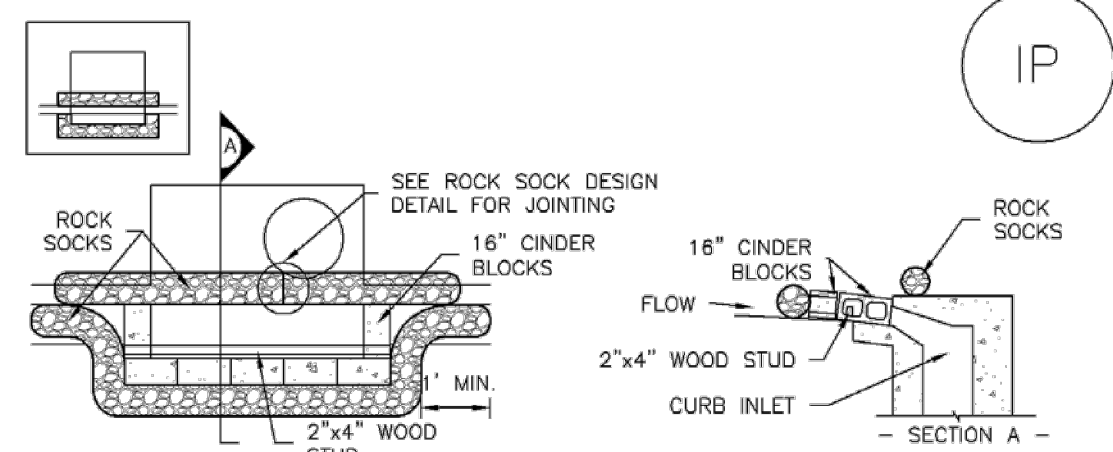
NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

NOTE: THE DETAILS INCLUDED WITH THIS FACT SHEET SHOW COMMONLY USED, CONVENTIONAL METHODS OF INLET PROTECTION IN THE DENVER METROPOLITAN AREA. THERE ARE MANY PROPRIETARY INLET PROTECTION METHODS ON THE MARKET. UDFCD NEITHER ENDORSES NOR DISCOURAGES USE OF PROPRIETARY INLET PROTECTION; HOWEVER, IN THE EVENT PROPRIETARY METHODS ARE USED, THE APPROPRIATE DETAIL FROM THE MANUFACTURER MUST BE INCLUDED IN THE SWMP AND THE BMP MUST BE INSTALLED AND MAINTAINED AS SHOWN IN THE MANUFACTURER'S DETAILS.

NOTE: SOME MUNICIPALITIES DISCOURAGE OR PROHIBIT THE USE OF STRAW BALES FOR INLET PROTECTION. CHECK WITH LOCAL JURISDICTION TO DETERMINE IF STRAW BALE INLET PROTECTION IS ACCEPTABLE.

IP-8 Urban Drainage and Flood Control District August 2013  
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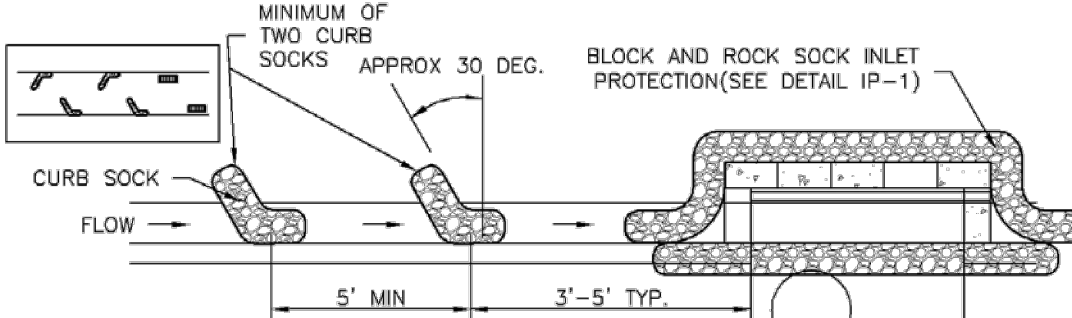
**SC-6 Inlet Protection (IP)**



**IP-1. BLOCK AND ROCK SOCK SUMP OR ON GRADE INLET PROTECTION**

**BLOCK AND CURB SOCK INLET PROTECTION INSTALLATION NOTES**

- SEE ROCK SOCK DESIGN DETAIL FOR INSTALLATION REQUIREMENTS.
- CONCRETE "CINDER" BLOCKS SHALL BE LAID ON THEIR SIDES AROUND THE INLET IN A SINGLE ROW, ABUTTING ONE ANOTHER WITH THE OPEN END FACING AWAY FROM THE CURB.
- GRAVEL BAGS SHALL BE PLACED AROUND CONCRETE BLOCKS, CLOSELY ABUTTING ONE ANOTHER AND JOINED TOGETHER IN ACCORDANCE WITH ROCK SOCK DESIGN DETAIL.



**IP-2. CURB ROCK SOCKS UPSTREAM OF INLET PROTECTION**

**CURB ROCK SOCK INLET PROTECTION INSTALLATION NOTES**

- SEE ROCK SOCK DESIGN DETAIL INSTALLATION REQUIREMENTS.
- PLACEMENT OF THE SOCK SHALL BE APPROXIMATELY 30 DEGREES FROM PERPENDICULAR IN THE OPPOSITE DIRECTION OF FLOW.
- SOCKS ARE TO BE FLUSH WITH THE CURB AND SPACED A MINIMUM OF 5 FEET APART.
- AT LEAST TWO CURB SOCKS IN SERIES ARE REQUIRED UPSTREAM OF ON-GRADE INLETS.

IP-4 Urban Drainage and Flood Control District August 2013  
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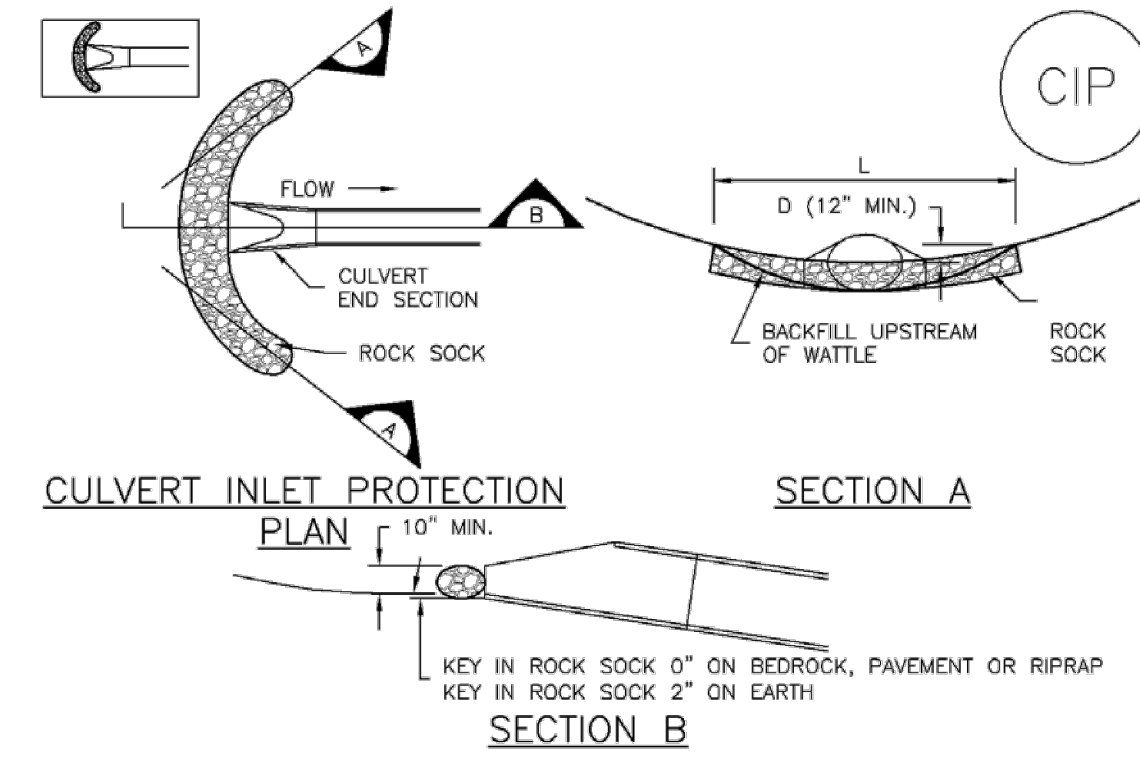
**Inlet Protection (IP) SC-6**

- Remove sediment accumulation from the area upstream of the inlet protection, as needed to maintain BMP effectiveness, typically when it reaches no more than half the storage capacity of the inlet protection. For silt fence, remove sediment when it accumulates to a depth of no more than 6 inches. Remove sediment accumulation from the area upstream of the inlet protection as needed to maintain the functionality of the BMP.
- Proprietary inlet protection devices should be inspected and maintained in accordance with manufacturer specifications. If proprietary inlet insert devices are used, sediment should be removed in a timely manner to prevent devices from breaking and spilling sediment into the storm drain.

Inlet protection must be removed and properly disposed of when the drainage area for the inlet has reached final stabilization.

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**Inlet Protection (IP) SC-6**



**CIP-1. CULVERT INLET PROTECTION**

**CULVERT INLET PROTECTION INSTALLATION NOTES**

- SEE PLAN VIEW FOR:
  - LOCATION OF CULVERT INLET PROTECTION.
- SEE ROCK SOCK DESIGN DETAIL FOR ROCK GRADATION REQUIREMENTS AND JOINING DETAIL.

**CULVERT INLET PROTECTION MAINTENANCE NOTES**

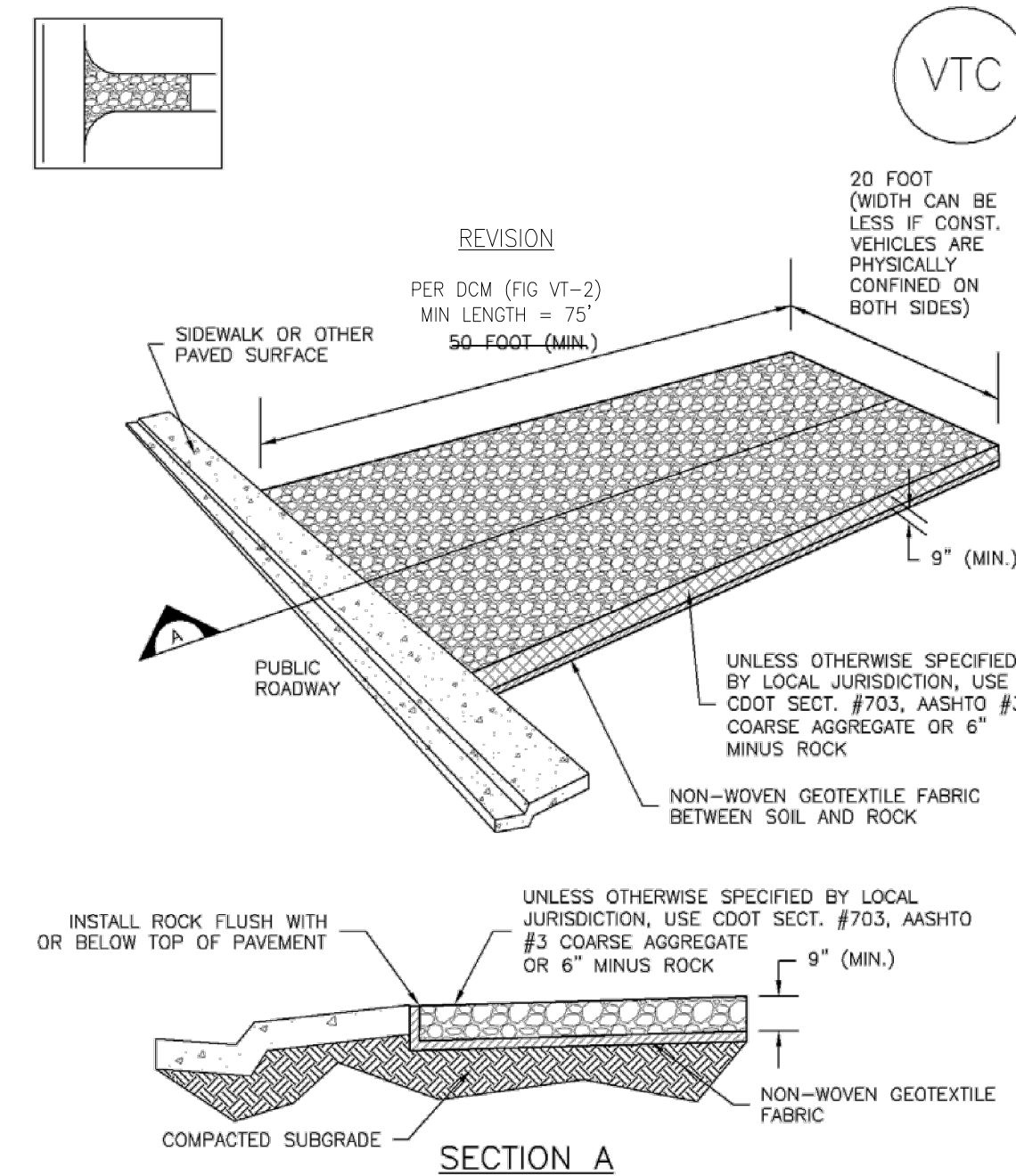
- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
- FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
- WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
- SEDIMENT ACCUMULATED UPSTREAM OF THE CULVERT SHALL BE REMOVED WHEN THE SEDIMENT DEPTH IS 1/2 THE HEIGHT OF THE ROCK SOCK.
- CULVERT INLET PROTECTION SHALL REMAIN IN PLACE UNTIL THE UPSTREAM DISTURBED AREA IS PERMANENTLY STABILIZED AND APPROVED BY THE LOCAL JURISDICTION.

(DETAILS ADAPTED FROM AURORA, COLORADO, NOT AVAILABLE IN AUTOCAD)

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

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**Vehicle Tracking Control (VTC) SM-4**



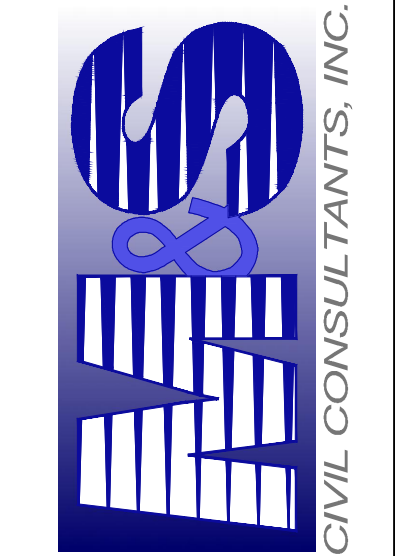
**VTC-1. AGGREGATE VEHICLE TRACKING CONTROL**

November 2010 Urban Drainage and Flood Control District VTC-3  
Urban Storm Drainage Criteria Manual Volume 3

FOR INFORMATION PURPOSE ONLY



102 E. Pikes Peak Ave., 5th Floor  
Colorado Springs, CO 80903  
PHONE: 719.555.5485



FOR AND ON BEHALF OF  
M&S CIVIL CONSULTANTS, INC.

DARIN L. MOFFETT, COLORADO P.E. NO. 38923

100% DESIGN DRAWINGS

NO.	DATE	BY	DESCRIPTION

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CAUTION



**SM-4 Vehicle Tracking Control (VTC)**

**STABILIZED CONSTRUCTION ENTRANCE/EXIT INSTALLATION NOTES**

- SEE PLAN VIEW FOR:
  - LOCATION OF CONSTRUCTION ENTRANCE(S)/EXIT(S).
  - TYPE OF CONSTRUCTION ENTRANCE(S)/EXIT(S) (WITH/WITHOUT WHEEL WASH, CONSTRUCTION MAT OR TRM).
- CONSTRUCTION MAT OR TRM STABILIZED CONSTRUCTION ENTRANCES ARE ONLY TO BE USED ON SHORT DURATION PROJECTS (TYPICALLY RANGING FROM A WEEK TO A MONTH) WHERE THERE WILL BE LIMITED VEHICULAR ACCESS.
- A STABILIZED CONSTRUCTION ENTRANCE/EXIT SHALL BE LOCATED AT ALL ACCESS POINTS WHERE VEHICLES ACCESS THE CONSTRUCTION SITE FROM PAVED RIGHT-OF-WAYS.
- STABILIZED CONSTRUCTION ENTRANCE/EXIT SHALL BE INSTALLED PRIOR TO ANY LAND DISTURBING ACTIVITIES.
- A NON-WOVEN GEOTEXTILE FABRIC SHALL BE PLACED UNDER THE STABILIZED CONSTRUCTION ENTRANCE/EXIT PRIOR TO THE PLACEMENT OF ROCK.
- UNLESS OTHERWISE SPECIFIED BY LOCAL JURISDICTION, ROCK SHALL CONSIST OF DOT SECT. #703, AASHTO #3 COARSE AGGREGATE OR 6" (MINUS) ROCK.

**STABILIZED CONSTRUCTION ENTRANCE/EXIT MAINTENANCE NOTES**

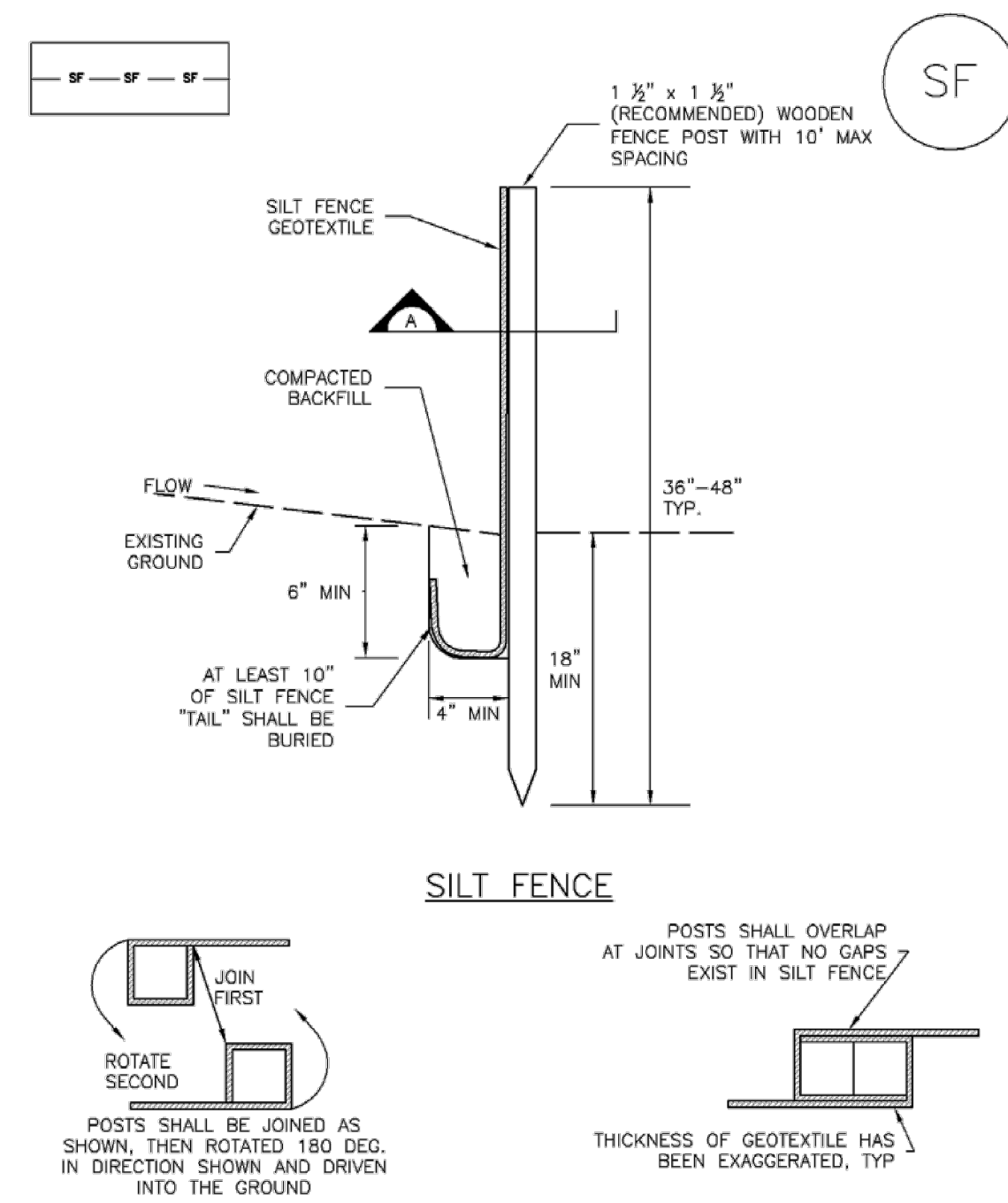
- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
- FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
- WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
- ROCK SHALL BE REAPPLIED OR REGRADED AS NECESSARY TO THE STABILIZED ENTRANCE/EXIT TO MAINTAIN A CONSISTENT DEPTH.
- SEDIMENT TRACKED ONTO PAVED ROADS IS TO BE REMOVED THROUGHOUT THE DAY AND AT THE END OF THE DAY BY SHOVELING OR SWEEPING. SEDIMENT MAY NOT BE WASHED DOWN STORM SEWER GRANS.

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

(DETAILS ADAPTED FROM CITY OF BRIDGFIELD, COLORADO, NOT AVAILABLE IN AUTOCAD)

VTC-6 Urban Drainage and Flood Control District November 2010  
Urban Storm Drainage Criteria Manual Volume 3

**Silt Fence (SF) SC-1**



SECTION A

SF-1. SILT FENCE

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**SC-1 Silt Fence (SF)**

**SILT FENCE INSTALLATION NOTES**

- SILT FENCE MUST BE PLACED AWAY FROM THE TOE OF THE SLOPE TO ALLOW FOR WATER PONDING. SILT FENCE AT THE TOE OF A SLOPE SHOULD BE INSTALLED IN A FLAT LOCATION AT LEAST SEVERAL FEET (2-5 FT) FROM THE TOE OF THE SLOPE TO ALLOW ROOM FOR PONDING AND DEPOSITION.
- A UNIFORM 6" X 4" ANCHOR TRENCH SHALL BE EXCAVATED USING TRENCHER OR SILT FENCE INSTALLATION DEVICE. NO ROAD GRADERS, BACKHOES, OR SIMILAR EQUIPMENT SHALL BE USED.
- COMPACT ANCHOR TRENCH BY HAND WITH A "JUMPING JACK" OR BY WHEEL ROLLING. COMPACTION SHALL BE SUCH THAT SILT FENCE RESISTS BEING PULLED OUT OF ANCHOR TRENCH BY HAND.
- SILT FENCE SHALL BE PULLED TIGHT AS IT IS ANCHORED TO THE STAKES. THERE SHOULD BE NO NOTICEABLE SAG BETWEEN STAKES AFTER IT HAS BEEN ANCHORED TO THE STAKES.
- SILT FENCE FABRIC SHALL BE ANCHORED TO THE STAKES USING 1" HEAVY DUTY STAPLES OR NAILS WITH 1" HEADS. STAPLES AND NAILS SHOULD BE PLACED 3" ALONG THE FABRIC DOWN THE STAKE.
- AT THE END OF A RUN OF SILT FENCE ALONG A CONTOUR, THE SILT FENCE SHOULD BE TURNED PERPENDICULAR TO THE CONTOUR TO CREATE A "J-HOOK." THE "J-HOOK" EXTENDING PERPENDICULAR TO THE CONTOUR SHOULD BE OF SUFFICIENT LENGTH TO KEEP RUNOFF FROM FLOWING AROUND THE END OF THE SILT FENCE (TYPICALLY 10' - 20').
- SILT FENCE SHALL BE INSTALLED PRIOR TO ANY LAND DISTURBING ACTIVITIES.

**SILT FENCE MAINTENANCE NOTES**

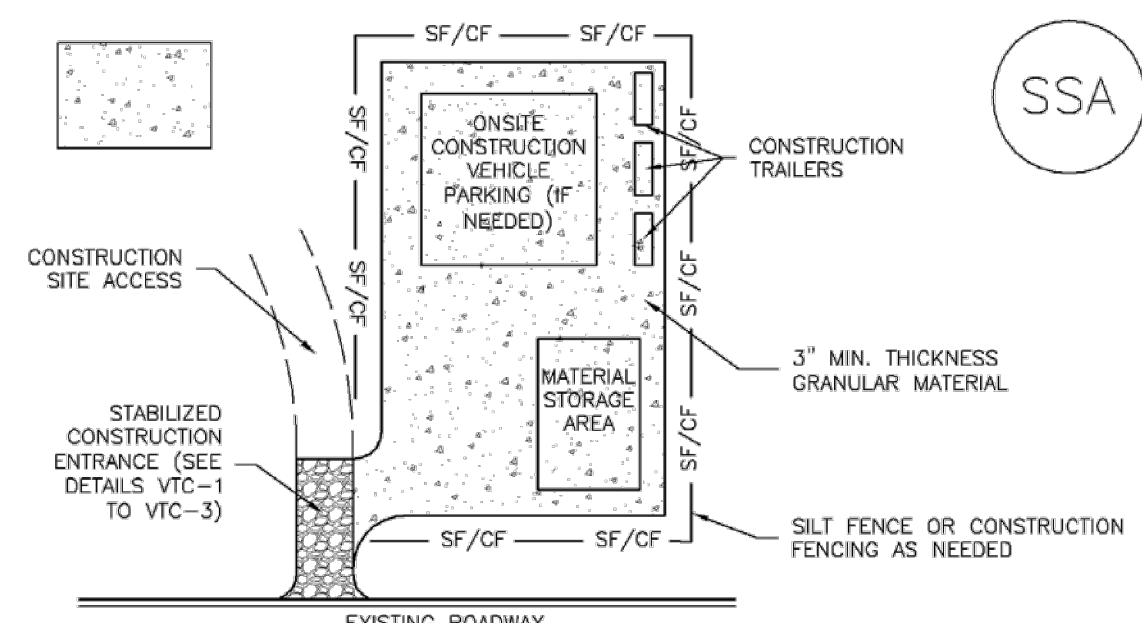
- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
- FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
- WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
- SEDIMENT ACCUMULATED UPSTREAM OF THE SILT FENCE SHALL BE REMOVED AS NEEDED TO MAINTAIN THE FUNCTIONALITY OF THE BMP, TYPICALLY WHEN DEPTH OF ACCUMULATED SEDIMENTS IS APPROXIMATELY 6".
- REPAIR OR REPLACE SILT FENCE WHEN THERE ARE SIGNS OF WEAR, SUCH AS SAGGING, TEARING, OR COLLAPSE.
- SILT FENCE IS TO REMAIN IN PLACE UNTIL THE UPSTREAM DISTURBED AREA IS STABILIZED AND APPROVED BY THE LOCAL JURISDICTION, OR IS REPLACED BY AN EQUIVALENT PERIMETER SEDIMENT CONTROL BMP.
- WHEN SILT FENCE IS REMOVED, ALL DISTURBED AREAS SHALL BE COVERED WITH TOPSOIL, SEEDED AND MULCHED OR OTHERWISE STABILIZED AS APPROVED BY LOCAL JURISDICTION.

(DETAIL ADAPTED FROM TOWN OF PARKER, COLORADO AND CITY OF AURORA, NOT AVAILABLE IN AUTOCAD)

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

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**Stabilized Staging Area (SSA) SM-6**



SSA-1. STABILIZED STAGING AREA

**STABILIZED STAGING AREA INSTALLATION NOTES**

- SEE PLAN VIEW FOR:
  - LOCATION OF STAGING AREA(S).
  - CONTRACTOR MAY ADJUST LOCATION AND SIZE OF STAGING AREA WITH APPROVAL FROM THE LOCAL JURISDICTION.
- STABILIZED STAGING AREA SHOULD BE APPROPRIATE FOR THE NEEDS OF THE SITE. OVERSIZING RESULTS IN A LARGER AREA TO STABILIZE FOLLOWING CONSTRUCTION.
- STAGING AREA SHALL BE STABILIZED PRIOR TO OTHER OPERATIONS ON THE SITE.
- THE STABILIZED STAGING AREA SHALL CONSIST OF A MINIMUM 3" THICK GRANULAR MATERIAL.
- UNLESS OTHERWISE SPECIFIED BY LOCAL JURISDICTION, ROCK SHALL CONSIST OF DOT SECT. #703, AASHTO #3 COARSE AGGREGATE OR 6" (MINUS) ROCK.
- ADDITIONAL PERIMETER BMPs MAY BE REQUIRED INCLUDING BUT NOT LIMITED TO SILT FENCE AND CONSTRUCTION FENCING.

**STABILIZED STAGING AREA MAINTENANCE NOTES**

- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
- FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
- WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
- ROCK SHALL BE REAPPLIED OR REGRADED AS NECESSARY IF RUTTING OCCURS OR UNDERLYING SUBGRADE BECOMES EXPOSED.

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**SM-6 Stabilized Staging Area (SSA)**

**Minimizing Long-Term Stabilization Requirements**

- Utilize off-site parking and restrict vehicle access to the site.
- Use construction mats in lieu of rock when staging is provided in an area that will not be disturbed otherwise.
- Consider use of a bermed contained area for materials and equipment that do not require a stabilized surface.
- Consider phasing of staging areas to avoid disturbance in an area that will not be otherwise disturbed.

See Detail SSA-1 for a typical stabilized staging area and SSA-2 for a stabilized staging area when materials staging in roadways is required.

**Maintenance and Removal**

Maintenance of stabilized staging areas includes maintaining a stable surface cover of gravel, repairing perimeter controls, and following good housekeeping practices.

When construction is complete, debris, unused stockpiles and materials should be recycled or properly disposed. In some cases, this will require disposal of contaminated soil from equipment leaks in an appropriate landfill. Staging areas should then be permanently stabilized with vegetation or other surface cover planned for the development.

SSA-2 Urban Drainage and Flood Control District November 2010  
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**SM-6 Stabilized Staging Area (SSA)**

**STABILIZED STAGING AREA MAINTENANCE NOTES**

- STABILIZED STAGING AREA SHALL BE ENLARGED IF NECESSARY TO CONTAIN PARKING, STORAGE, AND UNLOADING/LOADING OPERATIONS.
- THE STABILIZED STAGING AREA SHALL BE REMOVED AT THE END OF CONSTRUCTION. THE GRANULAR MATERIAL SHALL BE REMOVED OR, IF APPROVED BY THE LOCAL JURISDICTION, USED ON SITE, AND THE AREA COVERED WITH TOPSOIL, SEEDED AND MULCHED OR OTHERWISE STABILIZED IN A MANNER APPROVED BY LOCAL JURISDICTION.

NOTE: MANY MUNICIPALITIES PROHIBIT THE USE OF RECYCLED CONCRETE AS GRANULAR MATERIAL FOR STABILIZED STAGING AREAS DUE TO DIFFICULTIES WITH RE-ESTABLISHMENT OF VEGETATION IN AREAS WHERE RECYCLED CONCRETE WAS PLACED.

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

(DETAILS ADAPTED FROM DOUGLAS COUNTY, COLORADO, NOT AVAILABLE IN AUTOCAD)

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Urban Storm Drainage Criteria Manual Volume 3

FOR INFORMATION PURPOSE ONLY

**WMSD BOOSTER 2 PUMP STATION**

**GRADING & EROSION CONTROL DETAILS**

PROJECT NO. 70-077 DATE: 02-04-2022

SCALE: HORIZONTAL: N/A VERTICAL: N/A

DESIGNED BY: DLM DRAWN BY: DLM CHECKED BY: VAS

SHEET 5 OF 8 FGR05

102 E. Pikes Peak Ave., 5th Floor  
Colorado Springs, CO 80903  
PHONE: 719.555.5485

**CIVIL CONSULTANTS, INC.**

FOR AND ON BEHALF OF M&S CIVIL CONSULTANTS, INC.

DAREN L. MOFFETT, COLORADO P.E. NO. 38923

100% DESIGN DRAWINGS

NO.	DATE	BY	DESCRIPTION	APPROVED BY	DATE

THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE OR LIABLE FOR UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.

CAUTION

**EC-6 Rolled Erosion Control Products (RECP)**

**Turf Reinforcement Mat (TRM):** A rolled erosion control product composed of non-degradable synthetic fibers, filaments, nets, wire mesh, and/or other elements, processed into a permanent, three-dimensional matrix of sufficient thickness. TRMs, which may be supplemented with degradable components, are designed to impart immediate erosion protection, enhance vegetation establishment and provide long-term functionality by permanently reinforcing vegetation during and after maturation. Note: TRMs are typically used in hydraulic applications, such as high flow ditches and channels, steep slopes, stream banks, and shorelines, where erosive forces may exceed the limits of natural, unreinforced vegetation or in areas where limited vegetation establishment is anticipated.

Tables RECP-1 and RECP-2 provide guidelines for selecting rolled erosion control products appropriate to site conditions and desired longevity. Table RECP-1 is for conditions where natural vegetation alone will provide permanent erosion control, whereas Table RECP-2 is for conditions where vegetation alone will not be adequately stable to provide long-term erosion protection due to flow or other conditions.

RECP-2 Urban Drainage and Flood Control District  
Urban Storm Drainage Criteria Manual Volume 3 November 2010

**Roller Erosion Control Products (RECP) EC-6**

**Table RECP-1. ECTC Standard Specification for Temporary Rolled Erosion Control Products**  
(Adapted from Erosion Control Technology Council 2005)

Product Description	Slope Applications*		Channel Applications*	Minimum Tensile Strength <sup>1,4,6</sup>	Expected Longevity
	Maximum Gradient	C Factor <sup>2,5</sup>			
Mulch Control Nets	5:1 (H:V)	≤0.10 @ 5:1	Max. Shear Stress <sup>3,4,6</sup> 0.25 lbs/ft <sup>2</sup> (12 Pa)	5 lbs/ft (0.073 kN/m)	Up to 12 months
Netless Rolled Erosion Control Blankets	4:1 (H:V)	≤0.10 @ 4:1	0.5 lbs/ft <sup>2</sup> (24 Pa)	5 lbs/ft (0.073 kN/m)	
Single-net Erosion Control Blankets & Open Weave Textiles	3:1 (H:V)	≤0.15 @ 3:1	1.5 lbs/ft <sup>2</sup> (72 Pa)	50 lbs/ft (0.73 kN/m)	
Double-net Erosion Control Blankets	2:1 (H:V)	≤0.20 @ 2:1	1.75 lbs/ft <sup>2</sup> (84 Pa)	75 lbs/ft (1.09 kN/m)	
Mulch Control Nets	5:1 (H:V)	≤0.10 @ 5:1	0.25 lbs/ft <sup>2</sup> (12 Pa)	25 lbs/ft (0.36 kN/m)	24 months
Erosion Control Blankets & Open Weave Textiles (slowly degrading)	1.5:1 (H:V)	≤0.25 @ 1.5:1	2.00 lbs/ft <sup>2</sup> (96 Pa)	100 lbs/ft (1.45 kN/m)	24 months
Erosion Control Blankets & Open Weave Textiles	1:1 (H:V)	≤0.25 @ 1:1	2.25 lbs/ft <sup>2</sup> (108 Pa)	125 lbs/ft (1.82 kN/m)	36 months

\* C Factor and shear stress for mulch control nettings must be obtained with netting used in conjunction with pre-applied mulch material. (See Section 5.3 of Chapter 7 Construction BMPs for more information on the C Factor.)  
<sup>1</sup> Minimum Average Roll Values, Machine direction using ECTC Mod. ASTM D 5035.  
<sup>2</sup> C Factor calculated as ratio of soil loss from RECP protected slope (tested at specified or greater gradient, H:V) to ratio of soil loss from unprotected (control) plot in large-scale testing.  
<sup>3</sup> Required minimum shear stress RECP (unvegetated) can sustain without physical damage or excess erosion (> 12.7 mm (0.5 in) soil loss) during a 30-minute flow event in large-scale testing.  
<sup>4</sup> The permissible shear stress levels established for each performance category are based on historical experience with products characterized by Manning's roughness coefficients in the range of 0.01 - 0.05.  
<sup>5</sup> Acceptable large-scale test methods may include ASTM D 6459, or other independent testing deemed acceptable by the engineer.  
<sup>6</sup> Per the engineer's discretion. Recommended acceptable large-scale testing protocol may include ASTM D 6460, or other independent testing deemed acceptable by the engineer.

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**EC-6 Rolled Erosion Control Products (RECP)**

**Table RECP-2. ECTC Standard Specification for Permanent<sup>1</sup> Rolled Erosion Control Products**  
(Adapted from: Erosion Control Technology Council 2005)

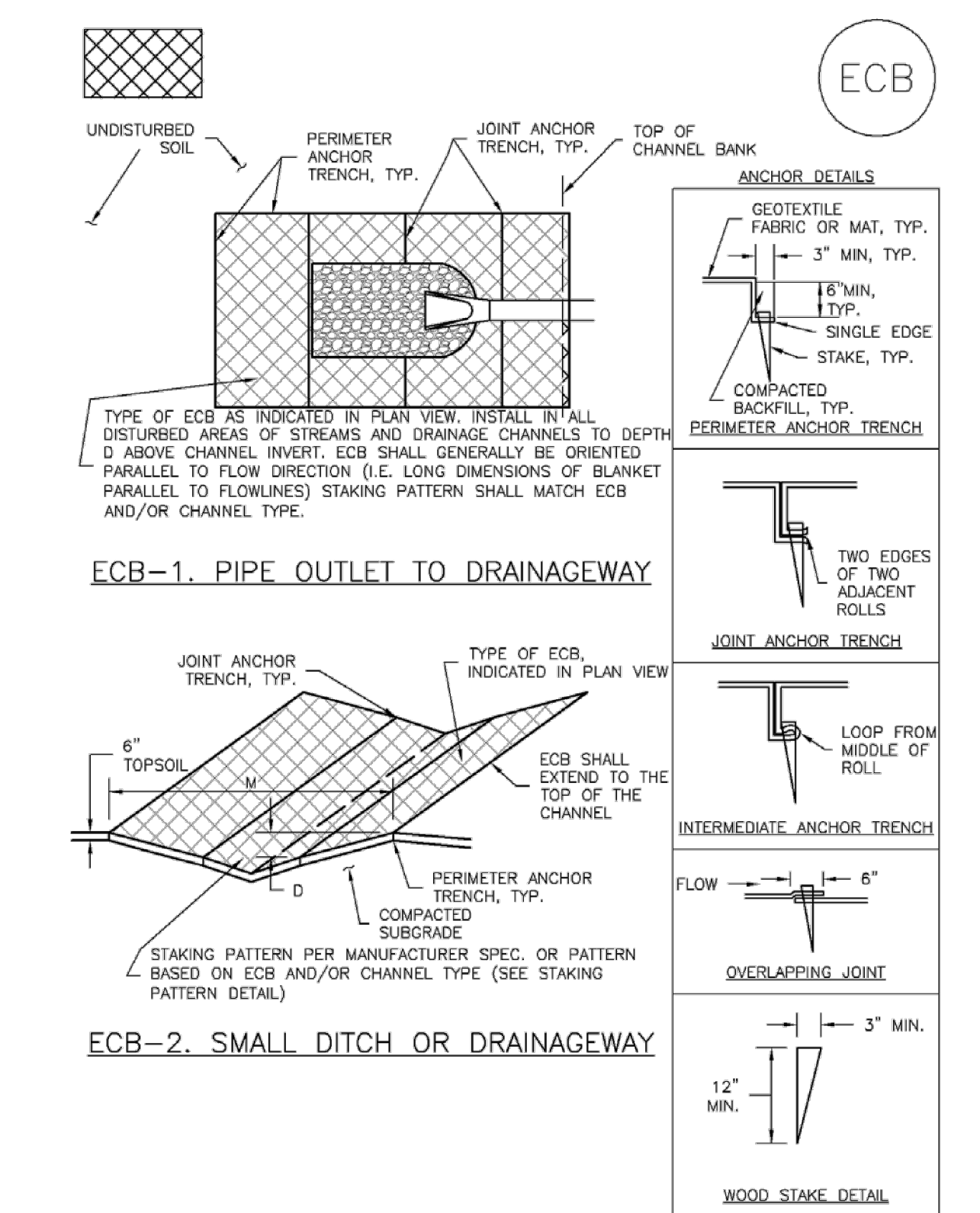
Product Type	Slope Applications	Channel Applications	
		Maximum Shear Stress <sup>1,5</sup>	Minimum Tensile Strength <sup>2,3</sup>
TRMs with a minimum thickness of 0.25 inches (6.35 mm) per ASTM D 6525 and UV stability of 80% per ASTM D 4355 (500 hours exposure).	0.5:1 (H:V)	6.0 lbs/ft <sup>2</sup> (288 Pa)	125 lbs/ft (1.82 kN/m)
	0.5:1 (H:V)	8.0 lbs/ft <sup>2</sup> (384 Pa)	150 lbs/ft (2.19 kN/m)
	0.5:1 (H:V)	10.0 lbs/ft <sup>2</sup> (480 Pa)	175 lbs/ft (2.55 kN/m)

<sup>1</sup> For TRMs containing degradable components, all property values must be obtained on the non-degradable portion of the matting alone.  
<sup>2</sup> Minimum Average Roll Values, machine direction only for tensile strength determination using ASTM D 6818 (Supersedes Mod. ASTM D 5035 for RECPs)  
<sup>3</sup> Field conditions with high loading and/or high survivability requirements may warrant the use of a TRM with a tensile strength of 44 kN/m (3,000 lb/ft) or greater.  
<sup>4</sup> Required minimum shear stress TRM (fully vegetated) can sustain without physical damage or excess erosion (> 12.7 mm (0.5 in.) soil loss) during a 30-minute flow event in large scale testing.  
<sup>5</sup> Acceptable large-scale testing protocols may include ASTM D 6460, or other independent testing deemed acceptable by the engineer.

**Design and Installation**  
 RECPs should be installed according to manufacturer's specifications and guidelines. Regardless of the type of product used, it is important to ensure no gaps or voids exist under the material and that all corners of the material are secured using stakes and trenching. Continuous contact between the product and the soil is necessary to avoid failure. Never use metal stakes to secure temporary erosion control products. Often wooden stakes are used to anchor RECPs; however, wood stakes may present installation and maintenance challenges and generally take a long time to biodegrade. Some local jurisdictions have had favorable experiences using biodegradable stakes.  
 This BMP Fact Sheet provides design details for several commonly used ECB applications, including:  
 ECB-1 Pipe Outlet to Drainageway  
 ECB-2 Small Ditch or Drainageway  
 ECB-3 Outside of Drainageway

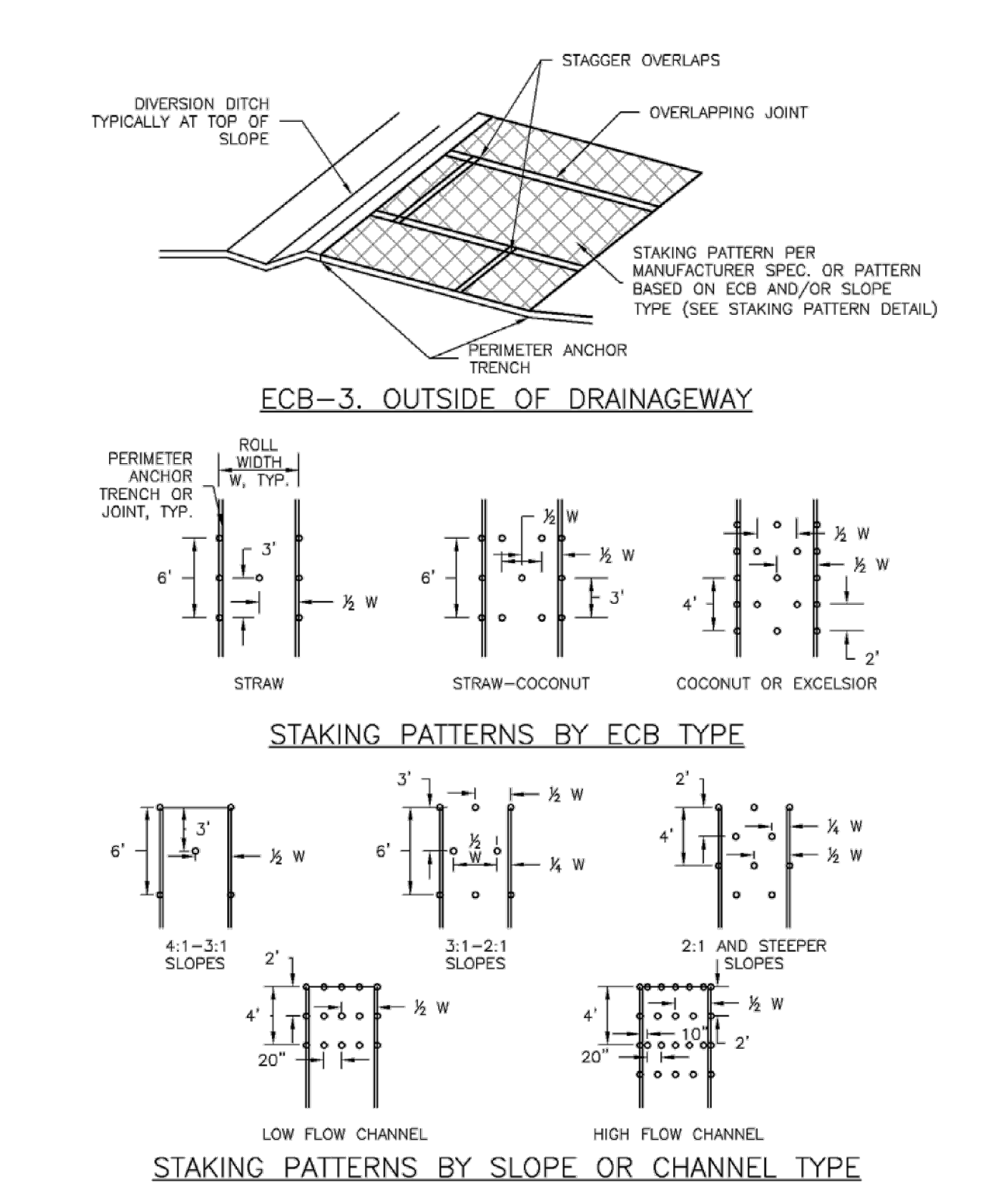
RECP-4 Urban Drainage and Flood Control District  
Urban Storm Drainage Criteria Manual Volume 3 November 2010

**EC-6 Rolled Erosion Control Products (RECP)**



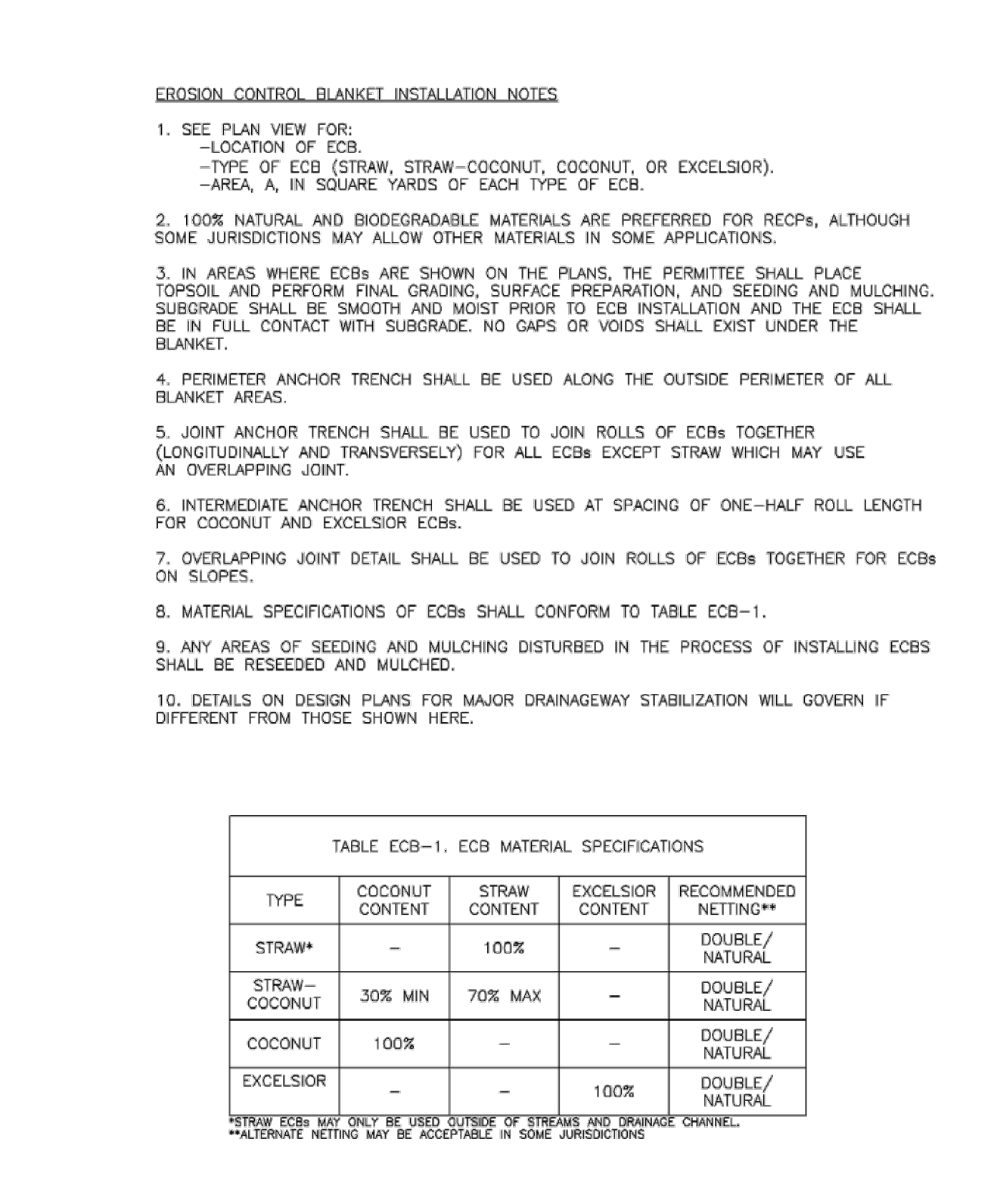
RECP-6 Urban Drainage and Flood Control District  
Urban Storm Drainage Criteria Manual Volume 3 November 2010

**Roller Erosion Control Products (RECP) EC-6**



RECP-7 Urban Drainage and Flood Control District  
Urban Storm Drainage Criteria Manual Volume 3 November 2010

**EC-6 Rolled Erosion Control Products (RECP)**



RECP-8 Urban Drainage and Flood Control District  
Urban Storm Drainage Criteria Manual Volume 3 November 2010

FOR INFORMATION PURPOSE ONLY



WWSO BOOSTER 2 PUMP STATION  
 GRADING & EROSION CONTROL DETAILS  
 PROJECT NO. 70-077  
 SCALE: HORIZONTAL: N/A VERTICAL: N/A  
 DATE: 02-04-2022  
 SHEET 6 OF 8  
 FGR06

20 BOULDER CRESCENT, SUITE 110  
 COLORADO SPRINGS, CO 80903  
 PHONE: 719.555.5485

CIVIL CONSULTANTS, INC.

FOR AND ON BEHALF OF M&S CIVIL CONSULTANTS, INC.

DAREN L. MOFFETT, COLORADO P.E. NO. 38923

100% DESIGN DRAWINGS

REVISIONS: NO. DATE: BY: DESCRIPTION: APPROVED BY: DATE:

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CAUTION

**Rolled Erosion Control Products (RECP) EC-6**

**EROSION CONTROL BLANKET MAINTENANCE NOTES**

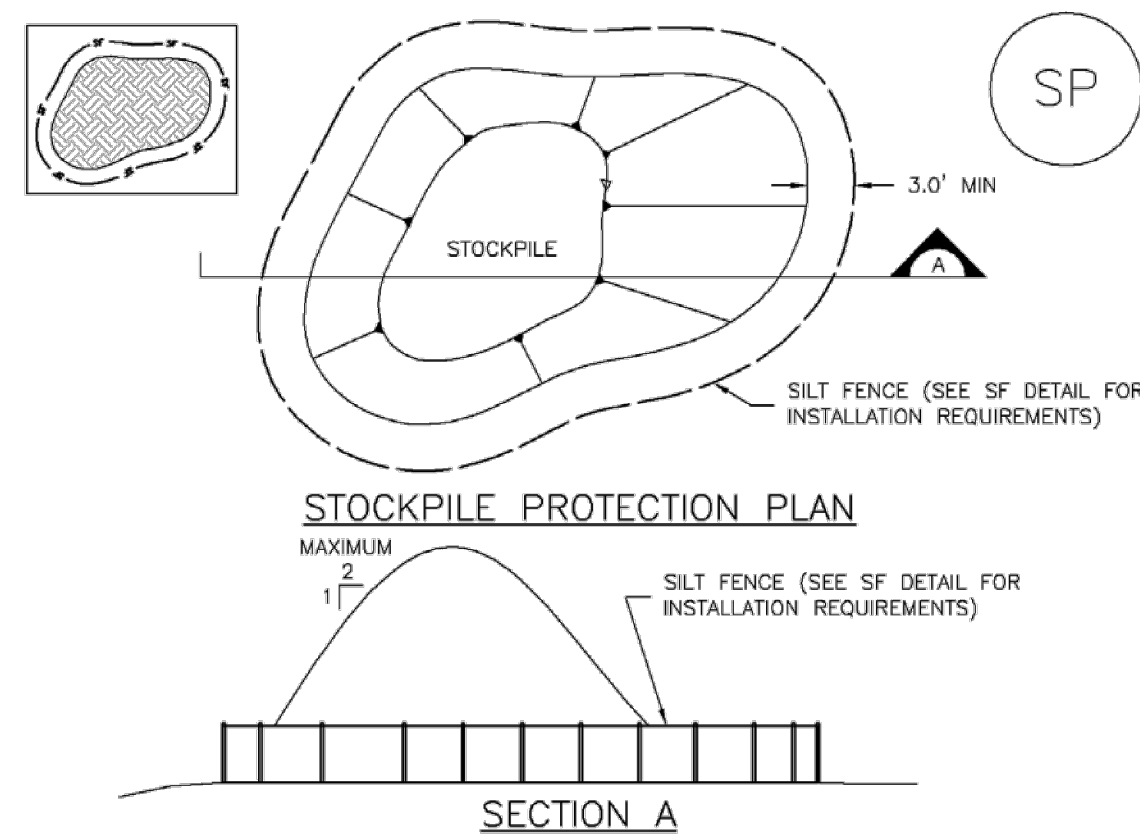
1. INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
2. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
3. WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
4. ECBs SHALL BE LEFT IN PLACE TO EVENTUALLY BIODEGRADE, UNLESS REQUESTED TO BE REMOVED BY THE LOCAL JURISDICTION.
5. ANY ECB PULLED OUT, TORN, OR OTHERWISE DAMAGED SHALL BE REPAIRED OR REINSTALLED. ANY SUBGRADE AREAS BELOW THE GEOTEXTILE THAT HAVE ERODED TO CREATE A VOID UNDER THE BLANKET, OR THAT REMAIN DEVOID OF GRASS SHALL BE REPAIRED, RESEEDED AND MULCHED AND THE ECB REINSTALLED.

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

(DETAILS ADAPTED FROM DOUGLAS COUNTY, COLORADO AND TOWN OF PARKER COLORADO, NOT AVAILABLE IN AUTOCAD)

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**Stockpile Management (SP) MM-2**



**SP-1. STOCKPILE PROTECTION**

**STOCKPILE PROTECTION INSTALLATION NOTES**

1. SEE PLAN VIEW FOR:
  - LOCATION OF STOCKPILES.
  - TYPE OF STOCKPILE PROTECTION.
2. INSTALL PERIMETER CONTROLS IN ACCORDANCE WITH THEIR RESPECTIVE DESIGN DETAILS. SILT FENCE IS SHOWN IN THE STOCKPILE PROTECTION DETAILS; HOWEVER, OTHER TYPES OF PERIMETER CONTROLS INCLUDING SEDIMENT CONTROL LOGS OR ROCK SOCKS MAY BE SUITABLE IN SOME CIRCUMSTANCES. CONSIDERATIONS FOR DETERMINING THE APPROPRIATE TYPE OF PERIMETER CONTROL FOR A STOCKPILE INCLUDE WHETHER THE STOCKPILE IS LOCATED ON A PERVIOUS OR IMPERVIOUS SURFACE, THE RELATIVE HEIGHTS OF THE PERIMETER CONTROL AND STOCKPILE, THE ABILITY OF THE PERIMETER CONTROL TO CONTAIN THE STOCKPILE WITHOUT FAILING IN THE EVENT THAT MATERIAL FROM THE STOCKPILE SLIPS OR SLUMPS AGAINST THE PERIMETER, AND OTHER FACTORS.
3. STABILIZE THE STOCKPILE SURFACE WITH SURFACE ROUGHENING, TEMPORARY SEEDING AND MULCHING, EROSION CONTROL BLANKETS, OR SOIL BINDERS. SOILS STOCKPILED FOR AN EXTENDED PERIOD (TYPICALLY FOR MORE THAN 60 DAYS) SHOULD BE SEEDING AND MULCHED WITH A TEMPORARY GRASS COVER ONCE THE STOCKPILE IS PLACED (TYPICALLY WITHIN 14 DAYS). USE OF MULCH ONLY OR A SOIL BINDER IS ACCEPTABLE IF THE STOCKPILE WILL BE IN PLACE FOR A MORE LIMITED TIME PERIOD (TYPICALLY 30-60 DAYS).
4. FOR TEMPORARY STOCKPILES ON THE INTERIOR PORTION OF A CONSTRUCTION SITE, WHERE OTHER DOWNGRADIENT CONTROLS, INCLUDING PERIMETER CONTROL, ARE IN PLACE, STOCKPILE PERIMETER CONTROLS MAY NOT BE REQUIRED.

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**MM-2 Stockpile Management (SM)**

**STOCKPILE PROTECTION MAINTENANCE NOTES**

1. INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
2. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
3. WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
4. IF PERIMETER PROTECTION MUST BE MOVED TO ACCESS SOIL STOCKPILE, REPLACE PERIMETER CONTROLS BY THE END OF THE WORKDAY.
5. STOCKPILE PERIMETER CONTROLS CAN BE REMOVED ONCE ALL THE MATERIAL FROM THE STOCKPILE HAS BEEN USED.

**STOCKPILE PROTECTION MAINTENANCE NOTES**

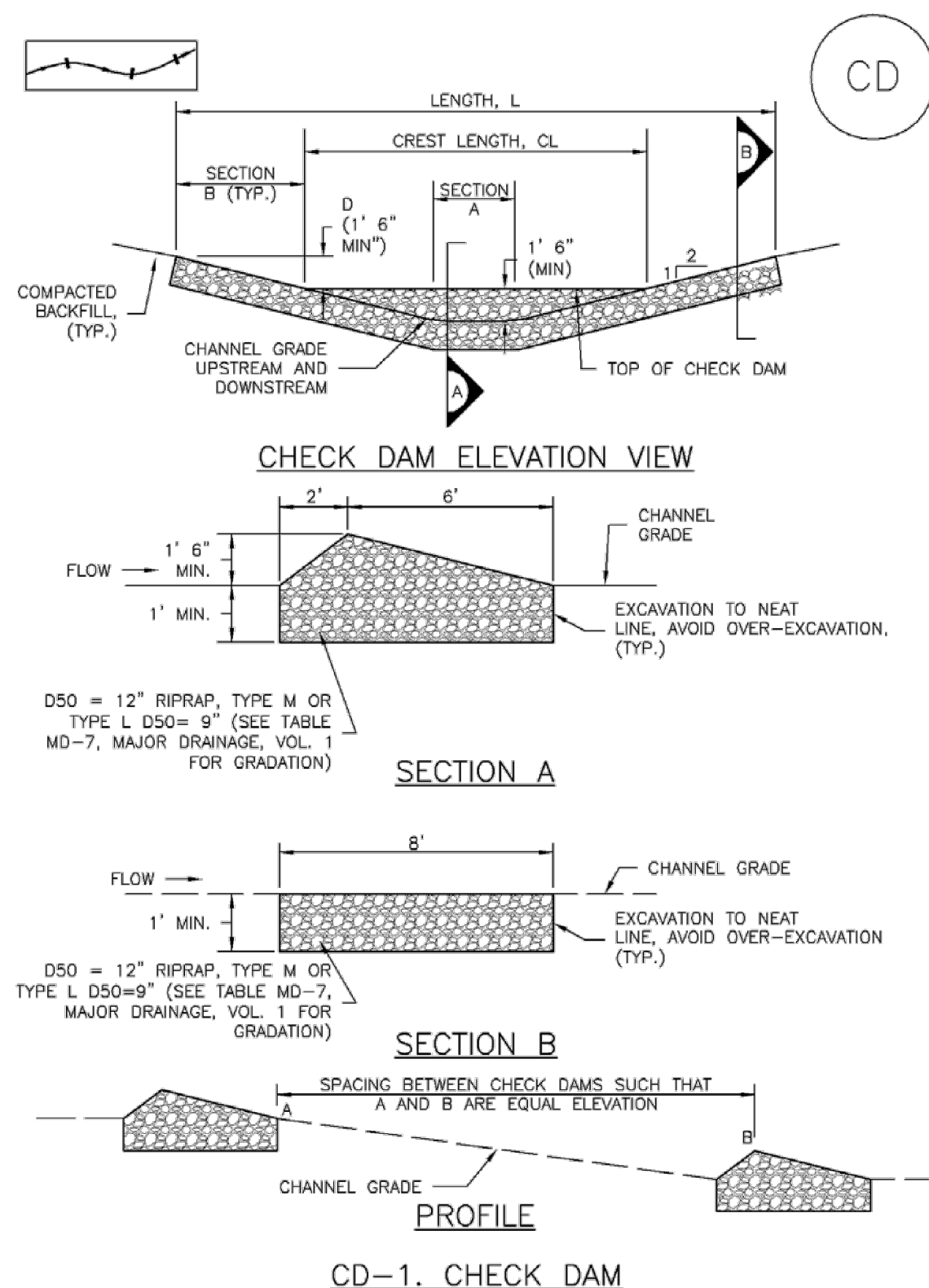
4. IF PERIMETER PROTECTION MUST BE MOVED TO ACCESS SOIL STOCKPILE, REPLACE PERIMETER CONTROLS BY THE END OF THE WORKDAY.
5. STOCKPILE PERIMETER CONTROLS CAN BE REMOVED ONCE ALL THE MATERIAL FROM THE STOCKPILE HAS BEEN USED.

(DETAILS ADAPTED FROM PARKER, COLORADO, NOT AVAILABLE IN AUTOCAD)

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

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**Check Dams (CD) EC-12**



**CD-1. CHECK DAM**

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**EC-12 Check Dams (CD)**

**CHECK DAM INSTALLATION NOTES**

1. SEE PLAN VIEW FOR:
  - LOCATION OF CHECK DAMS.
  - CHECK DAM TYPE (CHECK DAM OR REINFORCED CHECK DAM).
  - LENGTH (L), CREST LENGTH (CL), AND DEPTH (D).
2. CHECK DAMS INDICATED ON INITIAL SWMP SHALL BE INSTALLED AFTER CONSTRUCTION FENCE, BUT PRIOR TO ANY UPSTREAM LAND DISTURBING ACTIVITIES.
3. RIPRAP UTILIZED FOR CHECK DAMS SHOULD BE OF APPROPRIATE SIZE FOR THE APPLICATION. TYPICAL TYPES OF RIPRAP USED FOR CHECK DAMS ARE TYPE M (D50 12") OR TYPE L (D50 9").
4. RIPRAP PAD SHALL BE TRENCHED INTO THE GROUND A MINIMUM OF 1".
5. THE ENDS OF THE CHECK DAM SHALL BE A MINIMUM OF 1' 6" HIGHER THAN THE CENTER OF THE CHECK DAM.

**CHECK DAM MAINTENANCE NOTES**

1. INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
2. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
3. WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
4. SEDIMENT ACCUMULATED UPSTREAM OF THE CHECK DAMS SHALL BE REMOVED WHEN THE SEDIMENT DEPTH IS WITHIN 1/2 OF THE HEIGHT OF THE CREST.
5. CHECK DAMS ARE TO REMAIN IN PLACE UNTIL THE UPSTREAM DISTURBED AREA IS STABILIZED AND APPROVED BY THE LOCAL JURISDICTION.
6. WHEN CHECK DAMS ARE REMOVED, EXCAVATIONS SHALL BE FILLED WITH SUITABLE COMPACTED BACKFILL. DISTURBED AREA SHALL BE SEEDING AND MULCHED AND COVERED WITH GEOTEXTILE OR OTHERWISE STABILIZED IN A MANNER APPROVED BY THE LOCAL JURISDICTION.

(DETAILS ADAPTED FROM DOUGLAS COUNTY, COLORADO, NOT AVAILABLE IN AUTOCAD)

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

CD-4 Urban Drainage and Flood Control District Urban Storm Drainage Criteria Manual Volume 3 November 2010

**EC-12 Check Dams (CD)**

**REINFORCED CHECK DAM MAINTENANCE NOTES**

1. INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
2. FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
3. WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
4. SEDIMENT ACCUMULATED UPSTREAM OF REINFORCED CHECK DAMS SHALL BE REMOVED AS NEEDED TO MAINTAIN THE EFFECTIVENESS OF BMP, TYPICALLY WHEN THE UPSTREAM SEDIMENT DEPTH IS WITHIN 1/2 THE HEIGHT OF THE CREST.
5. REPAIR OR REPLACE REINFORCED CHECK DAMS WHEN THERE ARE SIGNS OF DAMAGE SUCH AS HOLES IN THE GABION OR UNDERCUTTING.
6. REINFORCED CHECK DAMS ARE TO REMAIN IN PLACE UNTIL THE UPSTREAM DISTURBED AREA IS STABILIZED AND APPROVED BY THE LOCAL JURISDICTION.
7. WHEN REINFORCED CHECK DAMS ARE REMOVED, ALL DISTURBED AREAS SHALL BE COVERED WITH TOPSOIL, SEEDING AND MULCHED, AND COVERED WITH A GEOTEXTILE BLANKET, OR OTHERWISE STABILIZED AS APPROVED BY LOCAL JURISDICTION.

(DETAIL ADAPTED FROM DOUGLAS COUNTY, COLORADO AND CITY OF AURORA, COLORADO, NOT AVAILABLE IN AUTOCAD)

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CD-6 Urban Drainage and Flood Control District Urban Storm Drainage Criteria Manual Volume 3 November 2010

FOR INFORMATION PURPOSE ONLY



**WWSO BOOSTER 2 PUMP STATION**

**GRADING & EROSION CONTROL DETAILS**

PROJECT NO. 70-077 DATE: 02-04-2022

SCALE: HORIZONTAL: N/A VERTICAL: N/A

DESIGNED BY: DLM DRAWN BY: JWP CHECKED BY: WAS

SHEET 7 OF 8 FGR07

102 E. Pikes Peak Ave., 5th Floor  
 Colorado Springs, CO 80903  
 PHONE: 719.555.5485

**M&S CIVIL CONSULTANTS, INC.**

FOR AND ON BEHALF OF M&S CIVIL CONSULTANTS, INC.

DAREN L. MOFFETT, COLORADO P.E. NO. 38923

100% DESIGN DRAWINGS

NO.	DATE	BY	DESCRIPTION	APPROVED BY

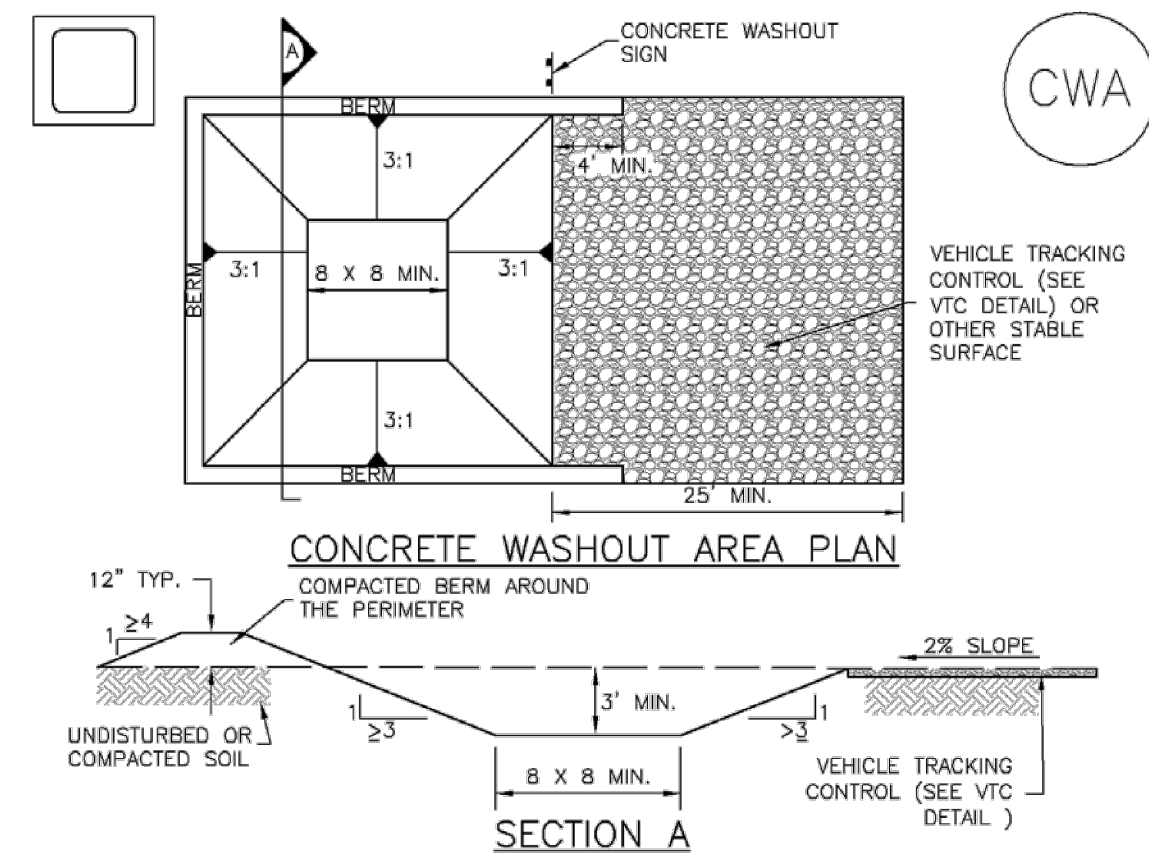
THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE OR LIABLE FOR UNAUTHORIZED CHANGES TO OR USE OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.

CAUTION

File: 0:\70077A\_Webfield\_M&S\_Hydr\7010 metropolitan street\wg\Const\_Dwg\FGR07.dwg PlotStamp: 2/4/2022 12:35 PM

**Concrete Washout Area (CWA)**

**MM-1**



**CWA-1. CONCRETE WASHOUT AREA**

**CWA INSTALLATION NOTES**

- SEE PLAN VIEW FOR:  
 --CWA INSTALLATION LOCATION.
- DO NOT LOCATE AN UNLINED CWA WITHIN 400' OF ANY NATURAL DRAINAGE PATHWAY OR WATERBODY. DO NOT LOCATE WITHIN 1,000' OF ANY WELLS OR DRINKING WATER SOURCES. IF SITE CONSTRAINTS MAKE THIS INFEASIBLE, OR IF HIGHLY PERMEABLE SOILS EXIST ON SITE, THE CWA MUST BE INSTALLED WITH AN IMPERMEABLE LINER (15 MIL MIN. THICKNESS) OR SURFACE STORAGE ALTERNATIVES USING PREFABRICATED CONCRETE WASHOUT DEVICES OR A LINED ABOVE GROUND STORAGE ARE SHOULD BE USED.
- THE CWA SHALL BE INSTALLED PRIOR TO CONCRETE PLACEMENT ON SITE.
- CWA SHALL INCLUDE A FLAT SUBSURFACE PIT THAT IS AT LEAST 8' BY 8' SLOPES LEADING OUT OF THE SUBSURFACE PIT SHALL BE 3:1 OR FLATTER. THE PIT SHALL BE AT LEAST 3' DEEP.
- BERM SURROUNDING SIDES AND BACK OF THE CWA SHALL HAVE MINIMUM HEIGHT OF 1'.
- VEHICLE TRACKING PAD SHALL BE SLOPED 2% TOWARDS THE CWA.
- SIGNS SHALL BE PLACED AT THE CONSTRUCTION ENTRANCE, AT THE CWA, AND ELSEWHERE AS NECESSARY TO CLEARLY INDICATE THE LOCATION OF THE CWA TO OPERATORS OF CONCRETE TRUCKS AND PUMP RIGS.
- USE EXCAVATED MATERIAL FOR PERIMETER BERM CONSTRUCTION.

November 2010 Urban Drainage and Flood Control District Urban Storm Drainage Criteria Manual Volume 3 CWA-3

**MM-1**

**Concrete Washout Area (CWA)**

**CWA MAINTENANCE NOTES**

- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
  - FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
  - WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
  - THE CWA SHALL BE REPAIRED, CLEANED, OR ENLARGED AS NECESSARY TO MAINTAIN CAPACITY FOR CONCRETE WASTE. CONCRETE MATERIALS, ACCUMULATED IN PIT, SHALL BE REMOVED ONCE THE MATERIALS HAVE REACHED A DEPTH OF 2'.
  - CONCRETE WASHOUT WATER, WASTED PIECES OF CONCRETE AND ALL OTHER DEBRIS IN THE SUBSURFACE PIT SHALL BE TRANSPORTED FROM THE JOB SITE IN A WATER-TIGHT CONTAINER AND DISPOSED OF PROPERLY.
  - THE CWA SHALL REMAIN IN PLACE UNTIL ALL CONCRETE FOR THE PROJECT IS PLACED.
  - WHEN THE CWA IS REMOVED, COVER THE DISTURBED AREA WITH TOP SOIL, SEED AND MULCH OR OTHERWISE STABILIZED IN A MANNER APPROVED BY THE LOCAL JURISDICTION.
- (DETAIL ADAPTED FROM DOUGLAS COUNTY, COLORADO AND THE CITY OF PARKER, COLORADO, NOT AVAILABLE IN AUTOCAD).  
 NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

CWA-4 Urban Drainage and Flood Control District Urban Storm Drainage Criteria Manual Volume 3 November 2010

**WUSD BOOSTER 2 PUMP STATION**  
**GRADING & EROSION CONTROL DETAILS**  
 PROJECT NO. 70-077 SCALE: HORIZONTAL: N/A VERTICAL: N/A  
 DATE: 02-04-2022  
 SHEET 8 OF 8  
**FCR08**

102 E. Pikes Peak Ave., 5th Floor  
 COLORADO SPRINGS, CO 80903  
 PHONE: 719.555.4485  
  
**CIVIL CONSULTANTS, INC.**

DARIN L. MOFFETT, COLORADO P.E. NO. 38923  
 FOR AND ON BEHALF OF M&S CIVIL CONSULTANTS, INC.

100% DESIGN DRAWINGS

NO.	DATE	BY	DESCRIPTION	APPROVED BY	DATE

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**CAUTION**

**FOR INFORMATION PURPOSE ONLY**

FOR LOCATING & MARKING GAS, ELECTRIC, WATER & TELEPHONE LINES  
**FOR BURIED UTILITY INFORMATION CALL 1-800-922-1987**

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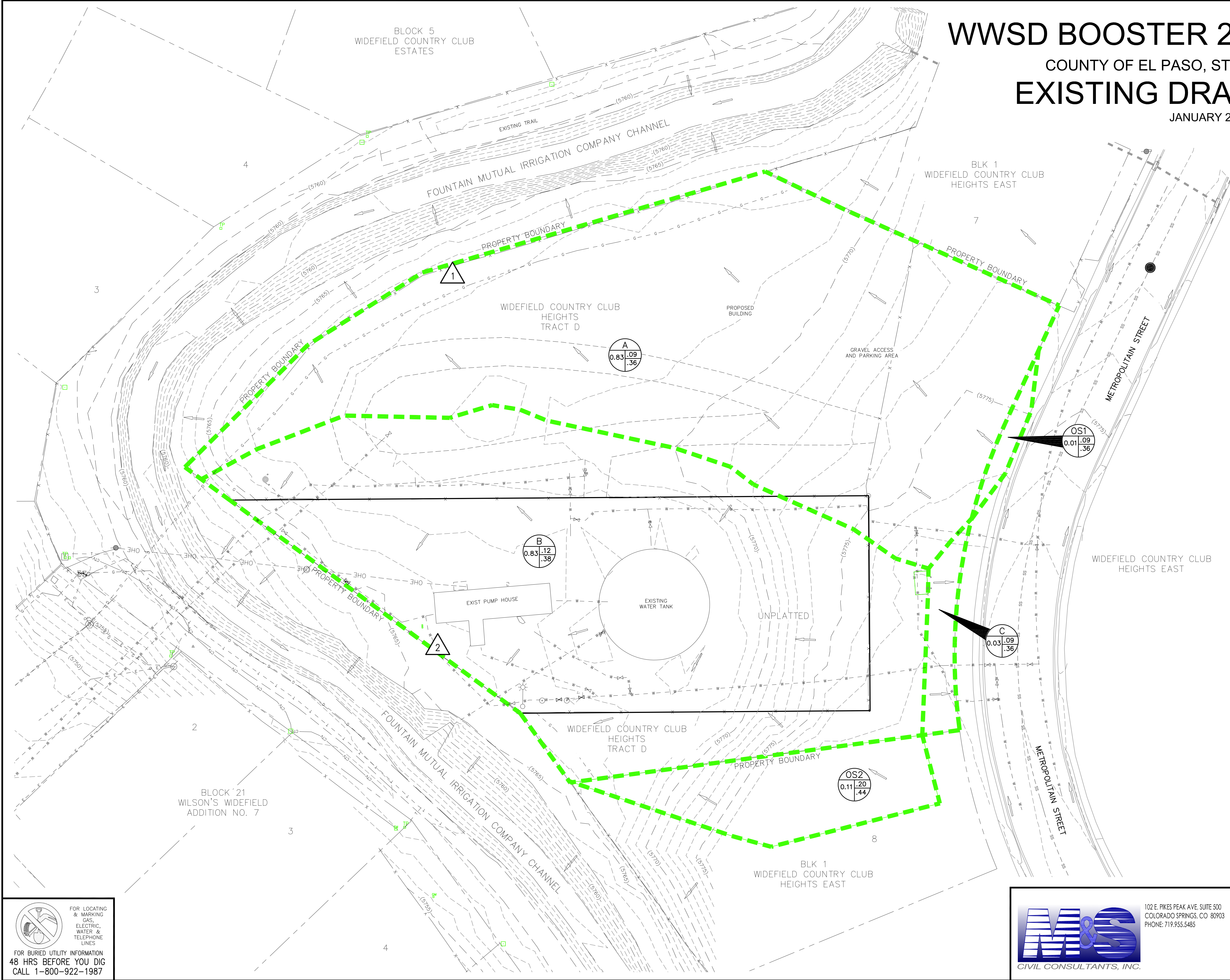
**PROPOSED AND EXISTING DRAINAGE MAP**

# WWSD BOOSTER 2 PUMP STATION

## COUNTY OF EL PASO, STATE OF COLORADO

### EXISTING DRAINAGE MAP

JANUARY 2020



- LEGEND**
- BASIN DESIGNATION
  - ACRES
  - SURFACE DESIGN POINT (DP)
  - PIPE RUN REFERENCE LABEL
  - BASIN BOUNDARY
  - EXISTING INDEX CONTOUR (5')
  - EXISTING NOMINAL CONTOUR (1')
  - WWSD SITE BOUNDARY
  - EXISTING FLOW DIRECTION ARROW
  - HIGH POINT
  - LOW POINT
  - PROPOSED STORM SEWER PIPE
  - FLARED END SECTION
  - EXISTING FLOW DIRECTION ARROW
  - PROPOSED FLOW DIRECTION

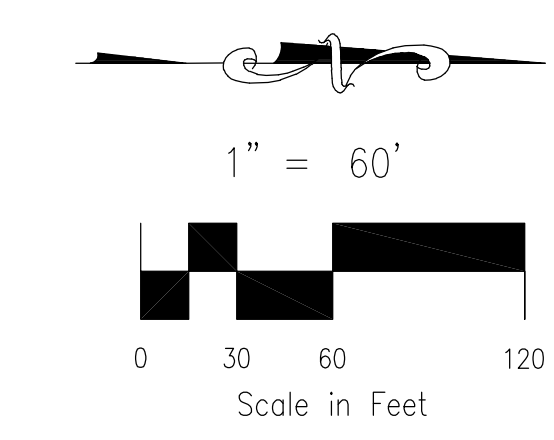
**BASIN SUMMARY**

BASIN	AREA (ACRES)	Q <sub>5</sub>	Q <sub>100</sub>
A	0.83	0.3	1.9
B	0.83	0.4	2.3
C	0.03	0.0*	0.1
OS1	0.01	0.0*	0.0*
OS2	0.11	0.1	0.4

\* RUNOFF FROM BASIN IS LESS THAN 0.1 CFS

**DESIGN POINT SUMMARY**

DESIGN POINT	Q <sub>5</sub>	Q <sub>100</sub>	BASIN & DES. PTS
1	0.3	2.0	OS1, OS2, A
2	0.6	2.7	A1, OS3



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FOR BURIED UTILITY INFORMATION 48 HRS BEFORE YOU DIG CALL 1-800-922-1987

102 E. PIKES PEAK AVE. SUITE 500  
COLORADO SPRINGS, CO 80903  
PHONE: 719.955.5485

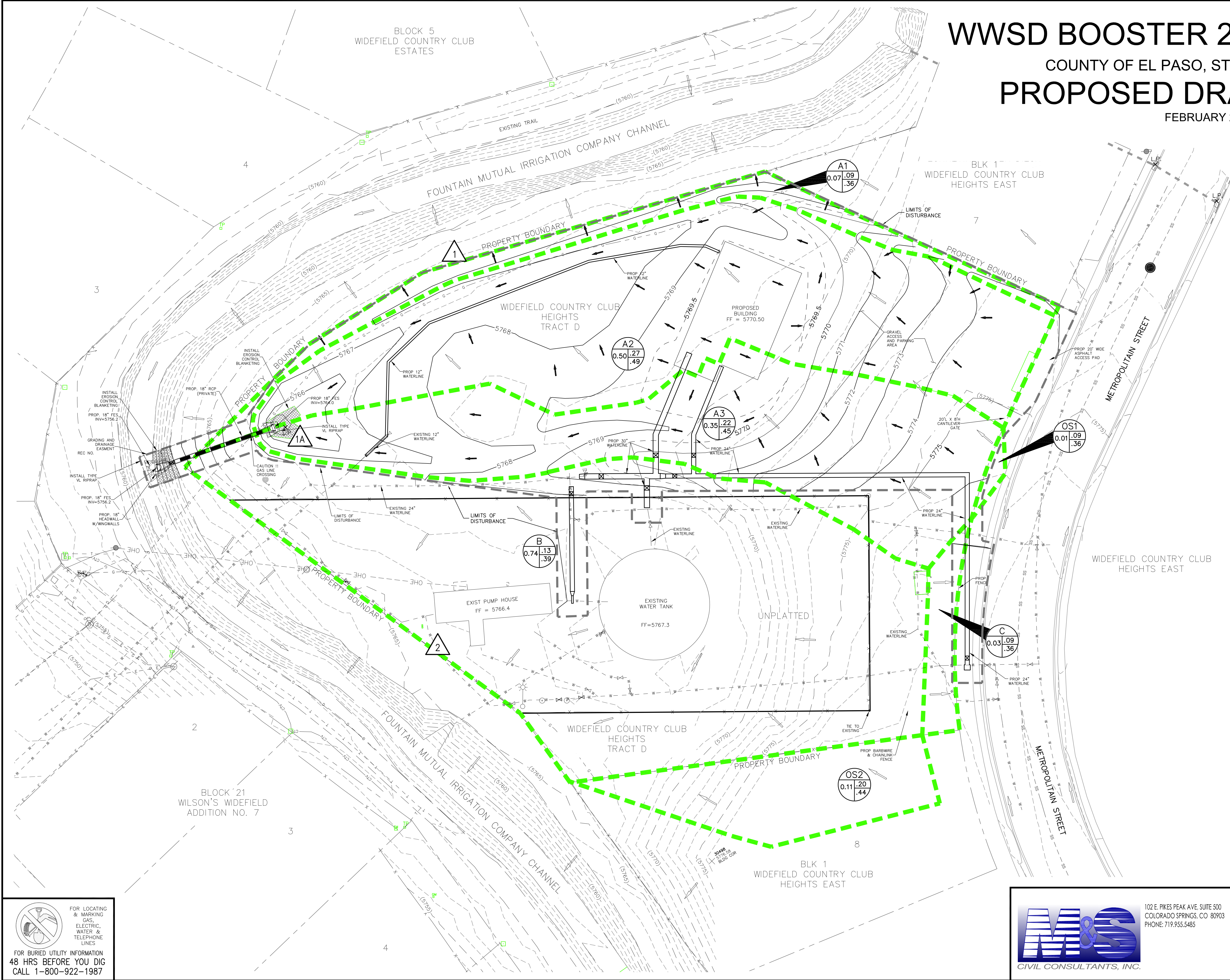
WWSD BOOSTER 2 PUMP STATION			
EXISTING DRAINAGE MAP			
PROJECT NO. 70-077	SCALE:	DATE: 8/21/2019	
DESIGNED BY: DLM	HORIZONTAL: 1"=20'	SHEET 1 OF 1	EDM
DRAWN BY: DLM	VERTICAL: N/A		
CHECKED BY: VAS			

# WWSD BOOSTER 2 PUMP STATION

## COUNTY OF EL PASO, STATE OF COLORADO

### PROPOSED DRAINAGE MAP

FEBRUARY 2020



#### LEGEND

- BASIN DESIGNATION
- ACRES
- SURFACE DESIGN POINT (DP)
- PIPE RUN REFERENCE LABEL
- BASIN BOUNDARY
- EXISTING INDEX CONTOUR (5')
- EXISTING NOMINAL CONTOUR (1')
- WWSd SITE BOUNDARY
- EXISTING FLOW DIRECTION ARROW
- HIGH POINT
- LOW POINT
- PROPOSED STORM SEWER PIPE
- FLARED END SECTION
- EXISTING FLOW DIRECTION ARROW
- PROPOSED FLOW DIRECTION

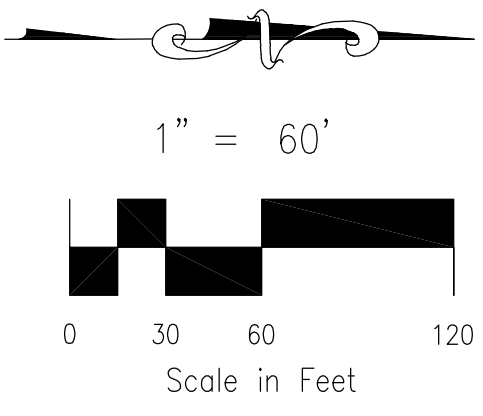
#### BASIN SUMMARY

BASIN	AREA (ACRES)	Q <sub>5</sub>	Q <sub>100</sub>
A1	0.07	0.0*	0.2
A2	0.50	0.5	1.6
A3	0.35	0.3	1.0
B	0.74	0.4	2.1
C	0.03	0.0*	0.1
OS1	<0.01	0.0*	0.0*
OS2	0.11	0.1	0.4

\* RUNOFF FROM BASIN IS LESS THAN 0.1 CFS

#### DESIGN POINT SUMMARY

DESIGN POINT	Q <sub>5</sub>	Q <sub>100</sub>	BASIN & DES. PTS
1	0.0	0.2	A1
1A	0.8	2.6	OS1, A2, A3
2	0.6	2.9	OS2, B, C



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COLORADO SPRINGS, CO 80903  
PHONE: 719.955.5485

WWSD BOOSTER 2 PUMP STATION  
PROPOSED DRAINAGE MAP

PROJECT NO. 70-077	SCALE:	DATE: 2/4/2022
DESIGNED BY: DLM	HORIZONTAL: 1"=20'	SHEET 1 OF 1
DRAWN BY: DLM	VERTICAL: N/A	
CHECKED BY: VAS		PDM