

FINAL DRAINAGE REPORT
FOR
WIDEFIELD WATER & SANITATION
DISTRICT
BOOSTER 2 PUMP STATION REPLACEMENT
EL PASO COUNTY, COLORADO

100% Submittal

January 2020

Prepared for:

JDS Hydro Consultants, Inc.
5455 Tech Center Drive, Suite 100
Colorado Springs, CO 80919

Prepared by:



20 Boulder Crescent, Suite 110
Colorado Springs, CO 80903
(719) 955-5485

Project #70-077
CID No. 2019.815.2027
PCD Project # PPR-19-XXX

PPR2169

project number added

**FINAL DRAINAGE REPORT
FOR
WIDEFIELD WATER & SANITATION DISTRICT
BOOSTER 2 PUMP STATION REPLACEMENT**

DRAINAGE PLAN STATEMENTS

ENGINEERS STATEMENT

The attached drainage plan and report was prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Darin L. Moffett, P.E. #38923
For and on Behalf of M&S Civil Consultants, Inc



DEVELOPER'S STATEMENT

I, the developer have read and will comply with all the requirements specified in this drainage report and plan.

BY: _____

TITLE: _____

DATE: _____

ADDRESS: Widefield Water and Sanitation District
8495 Fontaine Blvd.
Colorado Springs, CO80925

Please sign.

signature added

EL PASO COUNTY'S STATEMENT

Filed in accordance with the requirements of El Paso County Land Development Code, Drainage Criteria Manual Volumes 1 and 2, and the El Paso County Engineering Criteria Manual, as amended.

BY: _____

DATE: _____

Jennifer Irvine, P.E.
County Engineer/ECM Administrator

CONDITIONS

**FINAL DRAINAGE REPORT
FOR
WIDEFIELD WATER & SANITATION DISTRICT
BOOSTER 2 PUMP STATION REPLACEMENT**

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Vicinity Map
Soils Map
Annotated FIRM Panel
Hydrologic Calculations
Hydraulic Calculations
Grading Erosion Control Plan
Proposed and Existing Drainage Maps

**FINAL DRAINAGE REPORT
FOR
WIDEFIELD WATER & SANITATION DISTRICT
BOOSTER 2 PUMP STATION REPLACEMENT**

PURPOSE

This document is intended to serve as the Final Drainage report for the Widefield Water & Sanitation District Booster 2 Pump Station. The purpose of this document is to identify and analyze the on and offsite drainage patterns and to ensure that post development runoff is routed through the site safely.

GENERAL LOCATION AND DESCRIPTION

The Widefield Water & Sanitation District (WWSD) Booster 2 Pump Station and associated infrastructure and site improvements are to be constructed within both an un-platted parcel and Tract D of Widefield County Club Heights East both of which contiguous and under the ownership of the WWSD.

The parcels of land are located within a portion of the northwest quarter of the southeast quarter and within a portion of the southwest corner of the northeast quarter of Section 19, Township 15 South, Range 65 West of the 6th P.M. in El Paso County, Colorado. The site to the north and south by developed residential lots (Lot 7 & Lot 8 of Block 1) within Widefield Country Club Heights East and to east by existing Metropolitan Street Rights-of-Way and to the west by a portion of the Fountain Mutual Irrigation Company Channel (FMIC). Flows from this site are tributary to the East Big Johnson drainage basin and are ultimately tributary to Fountain Creek.

With the exception of the existing tank, pump house, fences and underground utilities the two parcels remain largely undeveloped. Vegetation is sparse, consisting of native grasses. The site has likely experienced overlot grading activities associated within the construction of the existing facilities and adjacent subdivision which occurred in the latter part of the 1970's. The existing site terrain generally slopes from east to west at grade rates that vary between 1% and 10%. The side slopes of the adjacent FMIC channel, directly to the west and southwest of the subject possess steep vegetative slopes that typically range between 1:1-2:1 (H:V).

As previously discussed the proposed site improvement will include a new pump station building and gravel access/parking lot. A small asphalt or concrete access drive with cantilever gate and additional security fencing will added along with below ground utility extensions necessary to connect the pump house to the existing tank site and subsequent waterlines. A small storm water collection system and landscaping will also be provided.

SOILS

Soils for this project are delineated on the Soils map in the appendix as have been determined to be Truckton Sandy Loam (97) and Wiley Silt Loam which have been characterized as Hydrologic Soil Types "A". Soils in the study area are shown as mapped by S.C.S. in the "Soils Survey of El Paso County Area". As previously discussed, vegetation on the site is sparse, consisting of native grasses and weeds.

HYDROLOGIC CALCULATIONS

Hydrologic calculations were performed using the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual and where applicable the Urban Storm Drainage Criteria Manual. The Rational Method was used to estimate stormwater runoff anticipated from design storms with 5-year and 100-year recurrence intervals.

HYDRAULIC CALCULATIONS

Hydraulic calculations were estimated using the Manning's Formula and the methods described in the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual. The relevant data sheets are included in the appendix of this report.

FLOODPLAIN STATEMENT

number and map in appendix updated

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) Panel No. 08041C0756 E, effective date December 07, 2018. An annotated FIRM Panel (Floodplain Map) is included in the Appendix to illustrate that NO portion of the site lies within the regulatory 100-year floodplain.

Per FEMA website panel number is 08041C0952G. Please revise.

DRAINAGE CRITERIA

This drainage analysis has been prepared in accordance with the current City of Colorado Springs/El Paso County Drainage Criteria Manuals. Calculations were performed to determine runoff quantities for the 5-year and 100-year frequency storms for developed conditions using the Rational Method as required for basins having areas less than 100 acres.

Provide sub-headings and text for each of the 4 steps per ECM Appendix I.7.2 - BMP Selection.

FOUR STEP PROCESS/WATER QUALITY

The proposed project is considered to be development with less than 1 acre of disturbance and is not part of a large common plan of development or sale, as a result no water quality improvements are required per the El Paso County Municipal Separate Storm Sewer System (MS4) Permit.

four step process added

The proposed and existing conditions have been developed for information purposes and to ensure runoff is safely routed and erosion control is properly implemented. As the proposed earthwork moving activities are anticipated to exceed 500 cubic yards a Stormwater manage Program Plan (SWMP) and Erosion and Stormwater Quality Control Permit (ESQCP) will be provided. A copy of the Proposed Grading and Erosion Control Plans are provided within the appendix of this report.

EXISTING DRAINAGE CONDITIONS

The two parcels that make up the existing site consist of approximately 1.69 acres. With the exception of the existing tank, pump house, fences and underground utilities the parcels remain largely undeveloped. The existing site terrain generally slopes from east to west at grade rates that vary between 1% and 10%. A small ridgeline which runs east to west generally divides the subject site into a north and south basin. With the exception of a small portion of the adjacent residential lot located to the south of the site and a very small portion of the adjacent Rights-of-Way (0.01 ac), the site is not burdened by offsite runoff, having nearly all of the site drainage directed to the adjacent Fountain Mutual Irrigation Company Channel as sheet flow. Onsite inspection shows limited reeling along the channel slopes from subject site

runoff. A small onsite area located at the southeast corner of the site (0.03 acres) discharges to the adjacent Metropolitan street curb and gutter where it is direct to a low point and sump inlet located north of the subject site. Flows intercepted by the inlet appear to be directed to the FMIC via an existing storm sewer (located to the north of lot 7).

Detailed Existing Drainage Discussion

Basin OS1, 0.01 acres, ($Q_5 < 0.1$ cfs, $Q_{100} < 0.1$ cfs), consists of a small area of existing vegetated rights of way located to the west of the subject site. Runoff produced within the Basin OS1 sheet flows to into Basin A.

Basin A, 0.83 acres, ($Q_5 = 0.3$ cfs, $Q_{100} = 1.9$ cfs), consists of a portion of the undeveloped Tract D. Runoff produced within the Basin A combines with flows from Basin OS1 and travels as both sheet and concentrated runoff to the northwest and to the existing FMIC channel. The combined runoff from the 0.84 acres totals 0.3 cfs and 2.0 cfs in the 5 and 100 year storm events respectively at east channel bank (Design Point 1).

Basin OS2, 0.11 acres, ($Q_5 = 0.1$ cfs, $Q_{100} = 0.4$ cfs), consists of a portion of residential Lot 3, Blk 1 of Widefield County Club Heights East. Runoff produced within the Basin OS2 travel west as sheet flow and enters Basin B along the southern property boundary.

Basin B, 0.83 acres, ($Q_5 = 0.4$ cfs, $Q_{100} = 2.3$ cfs), consists of the majority of the south half of the site and includes the existing pump house and water tank. Runoff from Basin B combines with flows from offsite basin OS2 and travels as sheet and concentrated flow to the south west property line and the existing FMIC channel at Design Point 2 ($Q_5 = 0.3$ cfs, $Q_{100} = 2.7$ cfs).

Basin C, 0.03 acres, ($Q_5 < 0.1$ cfs, $Q_{100} = 0.1$ cfs), consists of a segment of existing vegetated roadway rights of way. Runoff produced within the Basin C sheet flows to the west curb and gutter of existing Metropolitan Street. Flows collected with the west curb and gutter section is conveyed north to an existing offsite sump inlet.

PROPOSED DRAINAGE CHARACTERISTICS

General Concept Drainage Discussion

In the proposed condition the site improvements will include installation of a new pump station building and gravel access/parking lot. A small asphalt or concrete access drive with cantilever gate and additional security fencing will added along with the various below ground utility extensions necessary to connect the pump house to the existing tank site and subsequent waterlines. Runoff within the south half of the two parcels generally not be disturbed and will continue to drain as in the existing condition. While drainage from the north half of the site, where improvements are proposed, will now be collected and conveyed by a pair of earthen swales which will direct runoff to a proposed storm drain located at the far west end of the site. A small offsite easement will be required to construct and maintain the proposed storm sewer pipe and outfall. A riprap pad is proposed at the terminus of the pipe to protect the existing channel from any erosion that otherwise might be caused from the existing concentrated. In addition to the riprap armoring around the proposed flared end section, seeded permanent erosion control blanket is proposed to aid in protecting and restoring vegetation to the steep slope of the FMIC channel, where the channel bank will need to be removed and replaced in order to install the pipe. A small amount of riprap is also planned at the entrance of the storm sewer to stabilize the steep slope at the proposed FES. A detailed drainage discussion for each proposed conditions sub-basin is described

below. A worksheet detailing the calculated coefficients for each sub-basin and developed times of concentration can be found in the appendix under Hydrologic Calculations.

Detailed Drainage Discussion

Basin A1, 0.07 acres, ($Q_5 < 0.1$ cfs, $Q_{100} = 0.2$ cfs), consists of a segment of slope grading located along the north and northwest sides of the development. Flows produced within the Basin A1 sheet flow to the existing FMIC channel as in the existing condition. Flows reaching the northwest boundary of the site at Design Point 1 are calculated at less than 0.1 cfs the 5 year event and 0.2 cfs in the 100 year storm event.

Basin A2, 0.50 acres, ($Q_5 = 1.4$ cfs, $Q_{100} = 2.5$ cfs), consists of a portion of the proposed booster pump house, proposed gravel parking area as well as re-vegetated portions of the existing lot. Runoff from Basin A1 is routed as surface runoff via a min. 1' deep earthen swale to DP1A.

Basin OS1, 0.01 acres, ($Q_5 < 0.1$ cfs, $Q_{100} < 0.1$ cfs), consists of a small area of existing vegetated rights of way located to the west of the subject site. Runoff produced within the Basin OS1 sheet flows to into Basin A3.

0.35 according to map

Basin A3, 0.10 acres, ($Q_5 = 0.1$ cfs, $Q_{100} = 0.4$ cfs), consists of a portion of the proposed booster pump house, proposed gravel parking area as well as re-vegetated portions of the existing lot. Runoff from Basin A3 is routed as surface runoff via a min. 1' deep earthen swale to DP1A, where it ultimately combines with flows from Basins OS1 and A2 at a proposed 18" private reinforced concrete culvert totaling $Q_5 = 0.8$ cfs, $Q_{100} = 2.6$ cfs. The installation of a culvert to convey runoff from the site directly to the FMIC, will significantly reduce any potential erosion caused by the uncontrolled release of developed runoff across the steep channel embankment slopes.

JDS Hydro has determined in correspondence with the FMIC that approximately 50 cfs is conveyed by the irrigation channel. Given an calculated slope of 0.05%, a bottom width of approximately 9' and 1 to 1 (H:V) side slopes and a n value of 0.03, the calculated normal depth of the channel is approximately 30" at a velocity of 1.7' feet per second. This low tailwater condition will not adversely affect the proposed hydraulics of the proposed storm culvert given the pipe grade and entrance elevation and limited runoff there by allowing for free discharge. Given the design slope of 13.5% the proposed 18" culvert will convey the 2.6 cfs at a normal depth of 3.2" and at a velocity of 12.4 feet per second in the 100 year event. Refer to the Hydraulic Calculations portion of the appendix for additional information.

Basin OS2, 0.11 acres, ($Q_5 = 0.1$ cfs, $Q_{100} = 0.4$ cfs), consists of a portion of residential Lot 3, Blk 1 of Widefield County Club Heights East. Runoff produced within the Basin OS2 travel west as sheet flow and enters Basin B along the southern property boundary.

0.74 according to map

Basin B, 0.83 acres, ($Q_5 = 0.4$ cfs, $Q_{100} = 2.1$ cfs), consists of the majority of the south half of the site and includes the existing pump house and water tank. Runoff from Basin B combines with flows from offsite basin OS2 and travels as sheet and concentrated flow to the south west property line and the existing FMIC channel at Design Point 2 ($Q_5 = 0.6$ cfs, $Q_{100} = 2.9$ cfs).

Basin C, 0.03 acres, ($Q_5 < 0.1$ cfs, $Q_{100} = 0.1$ cfs), consists of a segment of existing vegetated roadway rights of way. Runoff produced within the Basin C sheet flows to the west curb and gutter of existing Metropolitan Street. Flows collected with the west curb and gutter section is conveyed north to an existing offsite sump inlet.

It should be noted that the estimated combined runoff discharged from the site in the existing condition totals ~ 4.7 cfs (direct addition of DP1, DP1A & DP2) while the total discharge from the proposed

information added per email exchange and Lupe's input

condition is only slightly higher at 5.7 cfs (direct addition of DP1&DP2). Installation of proposed infrastructure should mitigate any additional runoff produced and reduce the potential from erosion.

EROSION CONTROL

Please provide a discussion about detention for this development. If detention has been provided with the subdivision development please discuss.

It is the policy of the El Paso County that we submit a grading and erosion control plan with the drainage report. Proposed erosion control blanket, silt fence, vehicle traffic control, concrete washout area are proposed as erosion control measures. Recommendations for Stabilized staging area and stockpile management are also included.

CONSTRUCTION COST OPINION

Private Drainage Facilities NON-Reimbursable:

Item	Description	Quantity	Unit Cost	Cost
1.	18" Storm Sewer	50 LF	\$60 /LF	\$3,000.00
2.	18" Flared End Sections	2 EA	\$900 /EA	\$1,800.00
3.	D50=9" Riprap	5 CY	\$50 /CYF	\$250.00
7.	Erosion Control Blanket	26 SY	\$9 /SY	\$234.00

Please revise to say administrative relief was approved for site and reference the file number. Technically site is a tract, which would have to be replatted to build on.

DRAINAGE & BRIDGE FEES

The site is currently platted. No re-plat is being submitted. Thus no drainage and bridge fees are required for the development of the site.

information added regarding approval

M &S Civil Consultants, Inc. (M&S) cannot and does not guarantee the construction cost will not vary from these opinions of probable costs. These opinions represent our best judgment as design professionals familiar with the construction industry and this development in particular. The above is only an estimate of the facility cost and drainage basin fee amounts in 2017.

SUMMARY

The installation of the proposed facilities associated the construction of Widefield Water and Sanitation District Booster 2 Pump Station site shall not adversely affect adjacent or downstream properties per this final drainage report. The proposed drainage facilities will adequately convey, detain and route runoff from tributary onsite and existing offsite flows to the Sand Creek Drainage channel. Maintenance of the proposed private facilities shall be the responsibility of the WWSD.

updated/completed references

Please complete references.

REFERENCES

- 1.) "El Paso County and City of Colorado Springs Drainage Criteria Manual".
- 2.) "Urban Storm Drainage Criteria Manual"
- 3.) SCS Soils Map for El Paso County.
- 4.) Flood Insurance Rate Map (FIRM), Federal Emergency Management Agency, Effective date December 7, 2018.

DISCUSSED VIA
EMAIL NEGLIGIBLE

For Basin A, discuss increased flows from existing in report text above.

Increases from the historical flowrates is allowable per DCMv1, Chap 2.5.3 if you describe in greater detail how the flow increase will be accommodated downstream (ie: is there a suitable outfall for the increased flows?). Show some calcs and/or provide reference to the downstream facilities in a DBPS or MDDP.

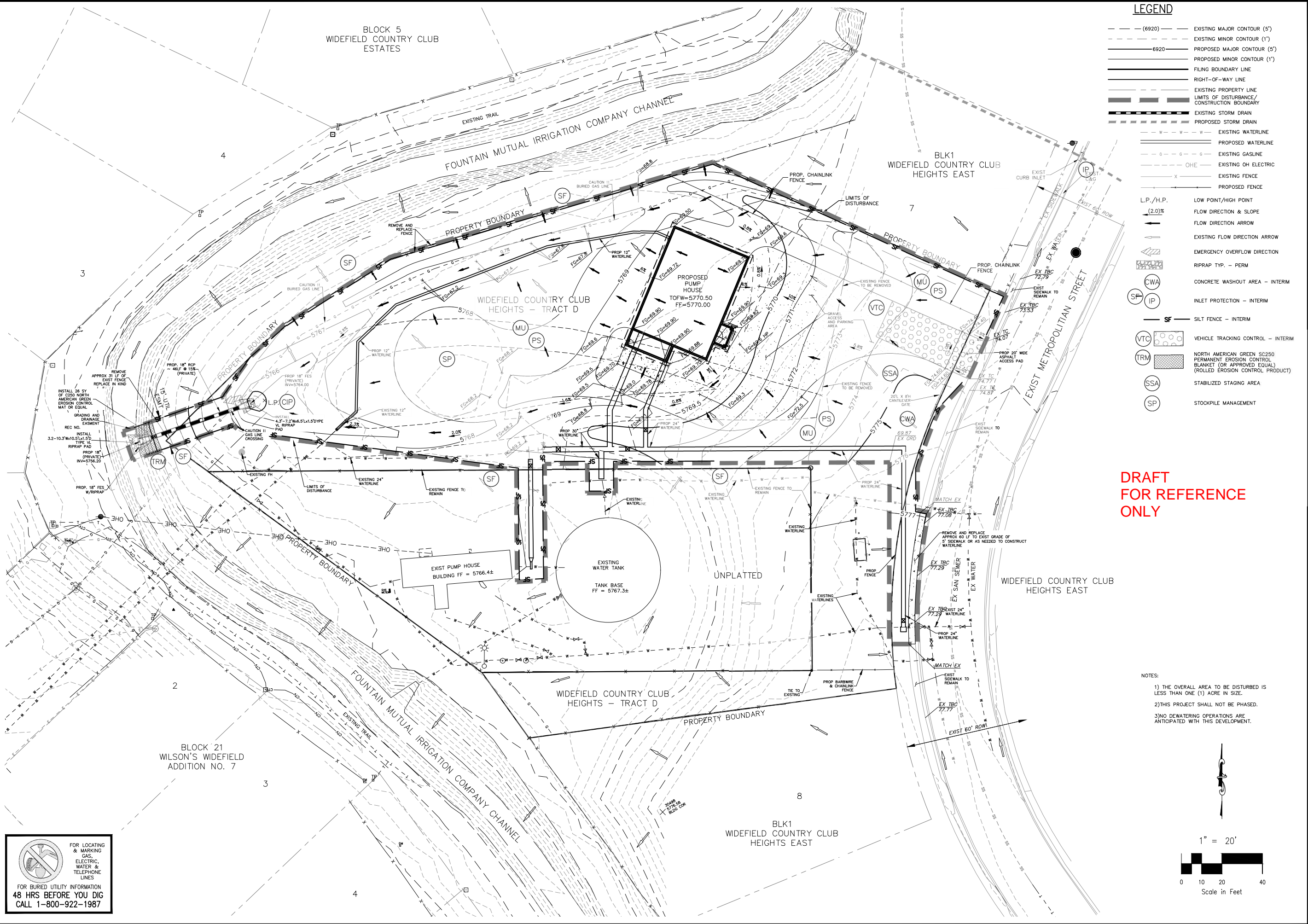
WIDEFIELD WATER AND WASTEWATER DISTRICT - BOOSTER 2 PUMP STATION PROPOSED CONDITIONS DRAINAGE CALCULATIONS (Area Drainage Summary)

From Area Runoff Coefficient Summary				OVERLAND				STREET / CHANNEL FLOW				Time of Travel (T_t)		INTENSITY *		TOTAL FLOWS	
BASIN	AREA TOTAL (Acres)	C ₅	C ₁₀₀	C ₅	Length (ft)	Height (ft)	T _c (min)	Length (ft)	Slope (%)	Velocity (fps)	T _i (min)	TOTAL (min)	CHECK (min)	I ₅ (in/hr)	I ₁₀₀ (in/hr)	Q ₅	Q ₁₀₀
		From DCM Table 5-1														(c.f.s.)	(c.f.s.)
A1	0.07	0.09	0.36	0.09	10	0.5	3.4	0	0.0%	0.0	0.0	5.0	10.1	5.2	8.7	0.0	0.2
A2	0.50	0.27	0.49	0.27	90	5.5	7.8	285	1.9%	1.4	3.4	11.2	12.1	3.8	6.5	0.5	1.6
A3	0.35	0.22	0.45	0.22	90	5.5	8.3	250	2.2%	1.5	2.8	11.1	11.9	3.9	6.5	0.3	1.0
B	0.74	0.13	0.39	0.13	60	4	7.3	150	5.3%	1.6	1.5	8.8	11.2	4.3	7.2	0.4	2.1
C	0.03	0.09	0.36	0.09	15	0.3	5.6	0	0.0%	0.0	0.0	5.6	10.1	5.0	8.4	0.0	0.1
OS1	0.00	0.09	0.36	0.09	10	0.2	4.6	0	0.0%	0.0	0.0	5.0	10.1	5.2	8.7	0.0	0.0
OS2	0.11	0.20	0.44	0.20	50	2	7.3	0	0.0%	0.0	0.0	7.3	10.3	4.6	7.7	0.1	0.4

* Intensity equations assume a minimum travel time of 5 minutes.

Calculated by: DLM
Date: 8/26/2019
Checked by: VAS

File: G:\00077A_Widefield_W&S_BDS_Hydra\0100 metropolian street\dwg\C10.dwg Plotstamp: 8/29/2019 12:13 PM

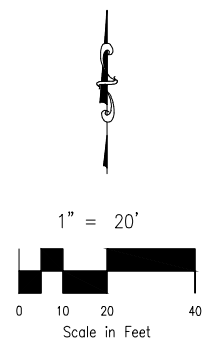


LEGEND

- (6920) --- EXISTING MAJOR CONTOUR (5')
- EXISTING MINOR CONTOUR (1')
- 6920 --- PROPOSED MAJOR CONTOUR (5')
- PROPOSED MINOR CONTOUR (1')
- FILING BOUNDARY LINE
- RIGHT-OF-WAY LINE
- EXISTING PROPERTY LINE
- LIMITS OF DISTURBANCE/ CONSTRUCTION BOUNDARY
- EXISTING STORM DRAIN
- PROPOSED STORM DRAIN
- EXISTING WATERLINE
- PROPOSED WATERLINE
- EXISTING GASLINE
- OHE --- EXISTING OH ELECTRIC
- EXISTING FENCE
- PROPOSED FENCE
- L.P./H.P. --- LOW POINT/HIGH POINT
- (2.0)% --- FLOW DIRECTION & SLOPE
- FLOW DIRECTION ARROW
- EXISTING FLOW DIRECTION ARROW
- EMERGENCY OVERFLOW DIRECTION
- RIPRAP TYP. - PERM
- (CWA) --- CONCRETE WASHOUT AREA - INTERIM
- (SF) (IP) --- INLET PROTECTION - INTERIM
- (SF) --- SILT FENCE - INTERIM
- (VTC) --- VEHICLE TRACKING CONTROL - INTERIM
- (TRM) --- NORTH AMERICAN GREEN SC250 PERMANENT EROSION CONTROL BLANKET (OR APPROVED EQUAL) (ROLLED EROSION CONTROL PRODUCT)
- (SSA) --- STABILIZED STAGING AREA
- (SP) --- STOCKPILE MANAGEMENT

DRAFT FOR REFERENCE ONLY

- NOTES:
- 1) THE OVERALL AREA TO BE DISTURBED IS LESS THAN ONE (1) ACRE IN SIZE.
 - 2) THIS PROJECT SHALL NOT BE PHASED.
 - 3) NO DEWATERING OPERATIONS ARE ANTICIPATED WITH THIS DEVELOPMENT.



FOR LOCATING & MARKING GAS, ELECTRIC, WATER & TELEPHONE LINES

FOR BURIED UTILITY INFORMATION 48 HRS BEFORE YOU DIG CALL 1-800-922-1987

WWSO BOOSTER 2 PUMP STATION

FINAL GRADING / EROSION CONTROL PLAN

PROJECT NO. 70-077 DATE: 08-28-19

SCALE: HORIZONTAL: 1"=20' VERTICAL: N/A

DESIGNED BY: DLM DRAWN BY: DLM CHECKED BY: VAS

20 BOULDER CRESCENT SUITE 110 COLORADO SPRINGS, CO 80903 PHONE: 719.555.5465

W&S CIVIL CONSULTANTS, INC.

DARIN L. MOFFETT, COLORADO P.E. NO. 35923

FOR AND ON BEHALF OF W&S CIVIL CONSULTANTS, INC.

60% DESIGN DRAWINGS

NO.	DATE	DESCRIPTION	APPROVED BY

THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE FOR UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.

CAUTION

GRADING AND EROSION CONTROL NOTES:

- 1. STORMWATER DISCHARGES FROM CONSTRUCTION SITES SHALL NOT CAUSE OR THREATEN TO CAUSE POLLUTION, CONTAMINATION, OR DEGRADATION OF STATE WATERS...
2. NOTWITHSTANDING ANYTHING DEPICTED IN THESE PLANS IN WORDS OR GRAPHIC REPRESENTATION...
3. A SEPARATE STORMWATER MANAGEMENT PLAN (SWMP) FOR THIS PROJECT SHALL BE COMPLETED AND AN EROSION AND STORMWATER QUALITY CONTROL PERMIT (ESQCP) ISSUED PRIOR TO COMMENCING CONSTRUCTION...
4. ONCE THE ESQCP IS APPROVED AND A "NOTICE TO PROCEED" HAS BEEN ISSUED, THE CONTRACTOR MAY INSTALL THE INITIAL STAGE EROSION AND SEDIMENT CONTROL MEASURES AS INDICATED ON THE ECM...
5. CONTROL MEASURES MUST BE INSTALLED PRIOR TO COMMENCEMENT OF ACTIVITIES THAT MAY CONTRIBUTE POLLUTANTS TO STORMWATER...
6. ALL TEMPORARY SEDIMENT AND EROSION CONTROL MEASURES SHALL BE MAINTAINED AND REMAIN IN EFFECTIVE OPERATING CONDITION UNTIL PERMANENT SOIL EROSION CONTROL MEASURES ARE IMPLEMENTED AND FINAL STABILIZATION IS ESTABLISHED...
7. TEMPORARY STABILIZATION SHALL BE IMPLEMENTED ON DISTURBED AREAS AND STOCKPILES WHERE GROUND DISTURBING CONSTRUCTION ACTIVITY HAS PERMANENTLY CEASED...
8. FINAL STABILIZATION MUST BE IMPLEMENTED AT ALL APPLICABLE CONSTRUCTION SITES...
9. ALL PERMANENT STORMWATER MANAGEMENT FACILITIES SHALL BE INSTALLED AS DEFINED IN THE APPROVED PLANS...
10. ANY EARTH DISTURBANCE SHALL BE CONDUCTED IN SUCH A MANNER SO AS TO EFFECTIVELY REDUCE ACCELERATED SOIL EROSION AND RESULTING SEDIMENTATION...
11. COMPACTION OF SOIL MUST BE PREVENTED IN AREAS DESIGNATED FOR INFILTRATION CONTROL MEASURES...
12. ANY TEMPORARY OR PERMANENT FACILITY DESIGNED AND CONSTRUCTED FOR THE CONVEYANCE OF STORMWATER AROUND, THROUGH, OR FROM THE EARTH DISTURBANCE AREA SHALL BE A STABILIZED CONVEYANCE DESIGNED TO MINIMIZE EROSION AND THE DISCHARGE OF SEDIMENT OFF SITE...
13. CONCRETE WASH WATER SHALL BE CONTAINED AND DISPOSED OF IN ACCORDANCE WITH THE SWMP...
14. DEWATERING OPERATIONS: UNCONTAMINATED GROUND WATER MAY BE DISCHARGED ON SITE, BUT MAY NOT LEAVE THE SITE IN THE FORM OF SURFACE RUNOFF...
15. EROSION CONTROL BLANKETING IS TO BE USED ON SLOPES STEEPER THAN 3:1...
16. BUILDING, CONSTRUCTION, EXCAVATION, OR OTHER WASTE MATERIALS SHALL NOT BE TEMPORARILY PLACED OR STORED IN THE STREET, ALLEY, OR OTHER PUBLIC WAY...
17. VEHICLE TRACKING OF SOILS AND CONSTRUCTION DEBRIS OFF-SITE SHALL BE MINIMIZED...
18. CONTRACTOR SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL WASTES FROM THE CONSTRUCTION SITE FOR DISPOSAL IN ACCORDANCE WITH LOCAL AND STATE REGULATORY REQUIREMENTS...
19. THE OWNER, SITE DEVELOPER, CONTRACTOR, AND/OR THEIR AUTHORIZED AGENTS SHALL BE RESPONSIBLE FOR THE REMOVAL OF ALL CONSTRUCTION DEBRIS, DIRT, TRASH, ROCK, SEDIMENT, AND ACCUMULATE IN THE STORM SEWER OR OTHER DRAINAGE CONVEYANCE SYSTEM AND STORMWATER APPURTENANCES AS A RESULT OF SITE DEVELOPMENT...
20. THE QUANTITY OF MATERIALS STORED ON THE PROJECT SITE SHALL BE LIMITED, AS MUCH AS PRACTICAL, TO THAT QUANTITY REQUIRED TO PERFORM THE WORK IN AN ORDERLY SEQUENCE...
21. NO CHEMICALS ARE TO BE USED BY THE CONTRACTOR, WHICH HAVE THE POTENTIAL TO BE RELEASED IN STORMWATER UNLESS PERMISSION FOR THE USE OF A SPECIFIC CHEMICAL IS GRANTED IN WRITING BY THE ECA ADMINISTRATOR...
22. BULK STORAGE OF PETROLEUM PRODUCTS OR OTHER LIQUID CHEMICALS IN EXCESS OF 55 GALLONS SHALL HAVE ADEQUATE SECONDARY CONTAINMENT PROTECTION TO CONTAIN ALL SPILLS AND PREVENT ANY SPILLED MATERIAL FROM ENTERING STATE WATERS...
23. NO PERSON SHALL CAUSE THE IMPEDIMENT OF STORMWATER FLOW IN THE FLOW LINE OF THE CURB AND GUTTER OR IN THE DITCH FLOW LINE...
24. INDIVIDUALS SHALL COMPLY WITH THE COLORADO WATER QUALITY CONTROL ACT (TITLE 25, ARTICLE 8, CRS), AND THE CLEAN WATER ACT (33 USC 1344), IN ADDITION TO THE REQUIREMENTS IN THIS VOLUME I AND THE ECM APPENDIX 1...
25. ALL CONSTRUCTION TRAFFIC MUST ENTER/EXIT THE SITE AT APPROVED CONSTRUCTION ACCESS POINTS...
26. PRIOR TO ACTUAL CONSTRUCTION THE PERMITEE SHALL VERIFY THE LOCATION OF EXISTING UTILITIES...
27. A WATER SOURCE SHALL BE AVAILABLE ON SITE DURING EARTHWORK OPERATIONS AND UTILIZED AS REQUIRED TO MINIMIZE DUST FROM EARTHWORK EQUIPMENT AND WIND...
28. THE SOILS REPORT FOR THIS SITE HAS BEEN PREPARED BY xxxxxxxxxxxxxxx, INC., ENTITLED GEOLOGIC HAZARD / LAND STUDY AND PRELIMINARY SUBSURFACE SOIL INVESTIGATION STERLING RANCH, DATED OCTOBER 31, 2006, AND SHALL BE CONSIDERED A PART OF THESE PLANS...
29. AT LEAST TEN DAYS PRIOR TO THE ANTICIPATED START OF CONSTRUCTION, FOR PROJECTS THAT WILL DISTURB 1 ACRE OR MORE, THE OWNER OR OPERATOR OF CONSTRUCTION ACTIVITY SHALL SUBMIT A PERMIT APPLICATION FOR STORMWATER DISCHARGE TO THE COLORADO DEPARTMENT OF PUBLIC HEALTH AND ENVIRONMENT, WATER QUALITY DIVISION...

EC-2 Temporary and Permanent Seeding (TS/PS)

soil amendments and rototill them into the soil to a depth of 6 inches or more.

Topsoil should be salvaged during grading operations for use and spread on areas to be revegetated later. Topsoil should be viewed as an important resource to be utilized for vegetation establishment...

Where topsoil is not available, subsoils should be amended to provide an appropriate plant-growth medium. Organic matter, such as well digested compost, can be added to improve soil characteristics conducive to plant growth.

If the disturbed ground surface is compacted, rip or rototill the surface prior to placing topsoil. If adding compost to the existing soil surface, rototilling is necessary.

Prior to seeding, the soil surface should be rough and the seedbed should be firm, but neither too loose nor compacted. The upper layer of soil should be in a condition suitable for seeding at the proper depth and conducive to plant growth.

Seed Mix for Temporary Vegetation

To provide temporary vegetative cover on disturbed areas which will not be paved, built upon, or fully landscaped or worked for an extended period (typically 30 days or more), plant an annual grass appropriate for the time of planting and mulch the planted areas.

Seed Mix for Permanent Revegetation

To provide vegetative cover on disturbed areas that have reached final grade, a perennial grass mix should be established. Permanent seeding should be performed promptly (typically within 14 days) after reaching final grade.

If desired for wildlife habitat or landscape diversity, shrubs such as rubber rabbitbrush (Chrysothamnus nauseosus), fourwing saltbush (Atriplex canescens) and skunkbrush sumac (Rhus trilobata) could be added to the upland seedmixes at 0.25, 0.5 and 1 pound PLS/acre, respectively.

Table with 4 columns: TS/PS-2, Urban Drainage and Flood Control District, June 2012, Urban Storm Drainage Criteria Manual Volume 3

Temporary and Permanent Seeding (TS/PS) EC-2

Table TS/PS-2. Minimum Drill Seeding Rates for Perennial Grasses (cont.)

Table with 6 columns: Common Name, Botanical Name, Growth Season, Growth Form, Seeds/Pound, Pounds of PLS/acre. Lists various grass species and their seeding rates.

All of the above seeding mixes and rates are based on drill seeding followed by crimped straw mulch. These rates should be doubled if seed is broadcast and should be increased by 50 percent if the seeding is done using a Brillion Drill or S applied through hydraulic seeding.

Table with 4 columns: June 2012, Urban Drainage and Flood Control District, TS/PS-5, Urban Storm Drainage Criteria Manual Volume 3

Temporary and Permanent Seeding (TS/PS) EC-2

Seeding dates for the highest success probability of perennial species along the Front Range are generally in the spring from April through early May and in the fall after the first of September until the ground freezes.

Table TS/PS-1. Minimum Drill Seeding Rates for Various Temporary Annual Grasses

Table with 4 columns: Species (Common name), Growth Season, Pounds of Pure Live Seed (PLS)/acre, Planting Depth (Inches). Lists annual grass species and their seeding rates.

Successful seeding of annual grass resulting in adequate plant growth will usually produce enough dead-plant residue to provide protection from wind and water erosion for an additional year.

Hydraulic seeding may be substituted for drilling only where slopes are steeper than 3:1 or where access limitations exist. When hydraulic seeding is used, hydraulic mulching should be applied as a separate operation.

See Table TS/PS-3 for seeding dates. Irrigation, if consistently applied, may extend the use of cool season species during the summer months.

Seeding rates should be doubled if seed is broadcast, or increased by 50 percent if done using a Brillion Drill or by hydraulic seeding.

Table with 4 columns: June 2012, Urban Drainage and Flood Control District, TS/PS-6, Urban Storm Drainage Criteria Manual Volume 3

Temporary and Permanent Seeding (TS/PS) EC-2

Table TS/PS-3. Seeding Dates for Annual and Perennial Grasses

Table with 5 columns: Seeding Dates, Annual Grasses (Warm, Cool), Perennial Grasses (Warm, Cool). Shows seeding windows for various grass types.

Mulch

Cover seeded areas with mulch or an appropriate rolled erosion control product to promote establishment of vegetation. Anchor mulch by crimping, netting or use of a non-toxic tackifier.

Maintenance and Removal

Monitor and observe seeded areas to identify areas of poor growth or areas that fail to germinate. Reseed and mulch these areas, as needed.

An area that has been permanently seeded should have a good stand of vegetation within one growing season if irrigated and within three growing seasons without irrigation in Colorado.

Seeded areas may require irrigation, particularly during extended dry periods. Targeted weed control may also be necessary.

Protect seeded areas from construction equipment and vehicle access.

Table with 4 columns: TS/PS-6, Urban Drainage and Flood Control District, June 2012, Urban Storm Drainage Criteria Manual Volume 3

DRAFT FOR REFERENCE ONLY

EC-2 Temporary and Permanent Seeding (TS/PS)

Table TS/PS-2. Minimum Drill Seeding Rates for Perennial Grasses

Table with 6 columns: Common Name, Botanical Name, Growth Season, Growth Form, Seeds/Pound, Pounds of PLS/acre. Lists various grass species and their seeding rates.

Table with 4 columns: TS/PS-4, Urban Drainage and Flood Control District, June 2012, Urban Storm Drainage Criteria Manual Volume 3

EC-4 Mulching (MU)

Clean, weed-free and seed-free cereal grain straw should be applied evenly at a rate of 2 tons per acre and must be tacked or fastened by a method suitable for the condition of the site.

Grass hay may be used in place of straw; however, because hay is comprised of the entire plant including seed, mulching with hay may seed the site with non-native grass species which might in turn out-compete the native seed.

On small areas sheltered from the wind and heavy runoff, spraying a tackifier on the mulch is satisfactory for holding it in place.

Hydraulic mulching consists of wood cellulose fibers mixed with water and a tackifying agent and should be applied at a rate of no less than 1,500 pounds per acre (1,425 lbs of fibers mixed with at least 75 lbs of tackifier) with a hydraulic mulcher.

Erosion control mats, blankets, or nets are recommended to help stabilize steep slopes (generally 3:1 and steeper) and waterways. Depending on the product, these may be used alone or in conjunction with grass or straw mulch.

Some tackifiers or binders may be used to anchor mulch. Check with the local jurisdiction for allowed tackifiers. Manufacturer's recommendations should be followed at all times.

Rock can also be used as mulch. It provides protection of exposed soils to wind and water erosion and allows infiltration of precipitation.

Maintenance and Removal

After mulching, the bare ground surface should not be more than 10 percent exposed. Reapply mulch, as needed, to cover bare areas.

Table with 4 columns: MU-2, Urban Drainage and Flood Control District, June 2012, Urban Storm Drainage Criteria Manual Volume 3

Project details and company information: WWSO BOOSTER 2 PUMP STATION, GRADING & EROSION CONTROL DETAILS, CIVIL CONSULTANTS, INC.

Professional seal and contact information: DARRIN L. MOFFETT, COLORADO P.E. NO. 38923, FOR AND ON BEHALF OF M&S CIVIL CONSULTANTS, INC.

Revisions table and disclaimer: REVISIONS NO., DATE, DESCRIPTION, APPROVED BY, DATE. THE OWNER, PREPARER, THESE PLANS WILL NOT BE RESPONSIBLE FOR UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS...

SC-6 Inlet Protection (IP)

- IP-3. Rock Sock Inlet Protection for Sump/Area Inlet
- IP-4. Silt Fence Inlet Protection for Sump/Area Inlet
- IP-5. Over-excavation Inlet Protection
- IP-6. Straw Bale Inlet Protection for Sump/Area Inlet
- CIP-1. Culvert Inlet Protection

Proprietary inlet protection devices should be installed in accordance with manufacturer specifications. More information is provided below on selecting inlet protection for sump and on-grade locations.

Inlets Located in a Sump

When applying inlet protection in sump conditions, it is important that the inlet continue to function during larger runoff events. For curb inlets, the maximum height of the protective barrier should be lower than the top of the curb opening to allow overflow into the inlet during larger storms without excessive localized flooding. If the inlet protection height is greater than the curb elevation, particularly if the filter becomes clogged with sediment, runoff will not enter the inlet and may bypass it, possibly causing localized flooding, public safety issues, and downstream erosion and damage from bypassed flows.

Area inlets located in a sump setting can be protected through the use of silt fence, concrete block and rock socks (on paved surfaces), sediment control logs/straw wattles embedded in the adjacent soil and stacked around the area inlet (on pervious surfaces), over-excavation around the inlet, and proprietary products providing equivalent functions.

Inlets Located on a Slope

For curb and gutter inlets on paved sloping streets, block and rock sock inlet protection is recommended in conjunction with curb socks in the gutter leading to the inlet. For inlets located along unpaved roads, also see the Check Dam Fact Sheet.

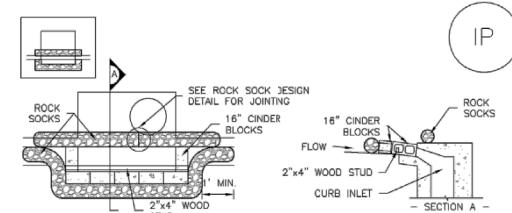
Maintenance and Removal

Inspect inlet protection frequently. Inspection and maintenance guidance includes:

- Inspect for tears that can result in sediment directly entering the inlet, as well as result in the contents of the BMP (e.g., gravel) washing into the inlet.
- Check for improper installation resulting in untreated flows bypassing the BMP and directly entering the inlet or bypassing to an unprotected downstream inlet. For example, silt fence that has not been properly trenched around the inlet can result in flows under the silt fence and directly into the inlet.
- Look for displaced BMPs that are no longer protecting the inlet. Displacement may occur following larger storm events that wash away or reposition the inlet protection. Traffic or equipment may also crush or displace the BMP.
- Monitor sediment accumulation upgradient of the inlet protection.

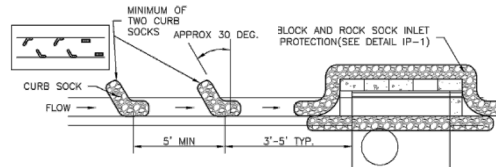
IP-2 Urban Drainage and Flood Control District August 2013
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SC-6 Inlet Protection (IP)



IP-1. BLOCK AND ROCK SOCK SUMP OR ON GRADE INLET PROTECTION

- BLOCK AND ROCK SOCK INLET PROTECTION INSTALLATION NOTES**
- SEE ROCK SOCK DESIGN DETAIL FOR INSTALLATION REQUIREMENTS.
 - CONCRETE "CINDER" BLOCKS SHALL BE LAID ON THEIR SIDES AROUND THE INLET IN A SINGLE ROW, ABUTTING ONE ANOTHER WITH THE OPEN END FACING AWAY FROM THE CURB.
 - GRAVEL BAGS SHALL BE PLACED AROUND CONCRETE BLOCKS, CLOSELY ABUTTING ONE ANOTHER AND JOINTED TOGETHER IN ACCORDANCE WITH ROCK SOCK DESIGN DETAIL.

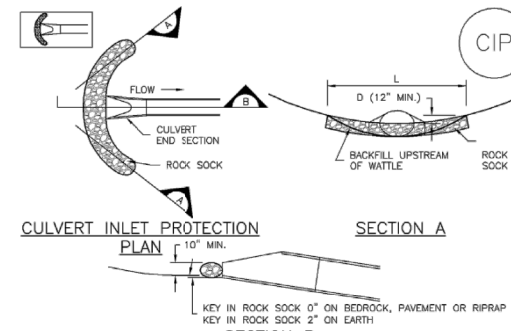


IP-2. CURB ROCK SOCKS UPSTREAM OF INLET PROTECTION

- CURB ROCK SOCK INLET PROTECTION INSTALLATION NOTES**
- SEE ROCK SOCK DESIGN DETAIL INSTALLATION REQUIREMENTS.
 - PLACEMENT OF THE SOCK SHALL BE APPROXIMATELY 30 DEGREES FROM PERPENDICULAR IN THE OPPOSITE DIRECTION OF FLOW.
 - SOCKS ARE TO BE FLUSH WITH THE CURB AND SPACED A MINIMUM OF 5 FEET APART.
 - AT LEAST TWO CURB SOCKS IN SERIES ARE REQUIRED UPSTREAM OF ON-GRADE INLETS.

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Inlet Protection (IP) SC-6



CIP-1. CULVERT INLET PROTECTION

- CULVERT INLET PROTECTION INSTALLATION NOTES**
- SEE PLAN VIEW FOR -LOCATION OF CULVERT INLET PROTECTION.
 - SEE ROCK SOCK DESIGN DETAIL FOR ROCK GRADATION REQUIREMENTS AND JOINTING DETAIL.
- CULVERT INLET PROTECTION MAINTENANCE NOTES**
- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
 - FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
 - WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
 - SEDIMENT ACCUMULATED UPSTREAM OF THE CULVERT SHALL BE REMOVED WHEN THE SEDIMENT DEPTH IS 1/2 THE HEIGHT OF THE ROCK SOCK.
 - CULVERT INLET PROTECTION SHALL REMAIN IN PLACE UNTIL THE UPSTREAM DISTURBED AREA IS PERMANENTLY STABILIZED AND APPROVED BY THE LOCAL JURISDICTION.
- (DETAILS ADAPTED FROM AURORA, COLORADO, NOT AVAILABLE IN AUTOCAD)
NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

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SC-6 Inlet Protection (IP)

GENERAL INLET PROTECTION INSTALLATION NOTES

- SEE PLAN VIEW FOR:
 - LOCATION OF INLET PROTECTION.
 - TYPE OF INLET PROTECTION (IP-1, IP-2, IP-3, IP-4, IP-5, IP-6)
- INLET PROTECTION SHALL BE INSTALLED PROMPTLY AFTER INLET CONSTRUCTION OR PAVING IS COMPLETE (TYPICALLY WITHIN 48 HOURS), IF A RAINFALL/RUNOFF EVENT IS FORECAST, INSTALL INLET PROTECTION PRIOR TO ONSET OF EVENT.
- MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

INLET PROTECTION MAINTENANCE NOTES

- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
- FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
- WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
- SEDIMENT ACCUMULATED UPSTREAM OF INLET PROTECTION SHALL BE REMOVED AS NECESSARY TO MAINTAIN BMP EFFECTIVENESS, TYPICALLY WHEN STORAGE VOLUME REACHES 50% OF CAPACITY, A DEPTH OF 6" WHEN SILT FENCE IS USED, OR 1/4 OF THE HEIGHT FOR STRAW BALES.
- INLET PROTECTION IS TO REMAIN IN PLACE UNTIL THE UPSTREAM DISTURBED AREA IS PERMANENTLY STABILIZED, UNLESS THE LOCAL JURISDICTION APPROVES EARLIER REMOVAL OF INLET PROTECTION IN STREETS.
- WHEN INLET PROTECTION AT AREA INLETS IS REMOVED, THE DISTURBED AREA SHALL BE COVERED WITH TOP SOIL, SEEDED AND MULCHED, OR OTHERWISE STABILIZED IN A MANNER APPROVED BY THE LOCAL JURISDICTION.

(DETAIL ADAPTED FROM TOWN OF PARKER, COLORADO AND CITY OF AURORA, COLORADO, NOT AVAILABLE IN AUTOCAD)

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDFCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

NOTE: THE DETAILS INCLUDED WITH THIS FACT SHEET SHOW COMMONLY USED, CONVENTIONAL METHODS OF INLET PROTECTION IN THE DENVER METROPOLITAN AREA. THERE ARE MANY PROPRIETARY METHODS ON THE MARKET. UDFCD NEITHER ENDORSES NOR DISCOURAGES USE OF PROPRIETARY INLET PROTECTION; HOWEVER, IN THE EVENT PROPRIETARY METHODS ARE USED, THE APPROPRIATE DETAIL FROM THE MANUFACTURER MUST BE INCLUDED IN THE SWMP AND THE BMP MUST BE INSTALLED AND MAINTAINED AS SHOWN IN THE MANUFACTURER'S DETAILS.

NOTE: SOME MUNICIPALITIES DISCOURAGE OR PROHIBIT THE USE OF STRAW BALES FOR INLET PROTECTION. CHECK WITH LOCAL JURISDICTION TO DETERMINE IF STRAW BALE INLET PROTECTION IS ACCEPTABLE.

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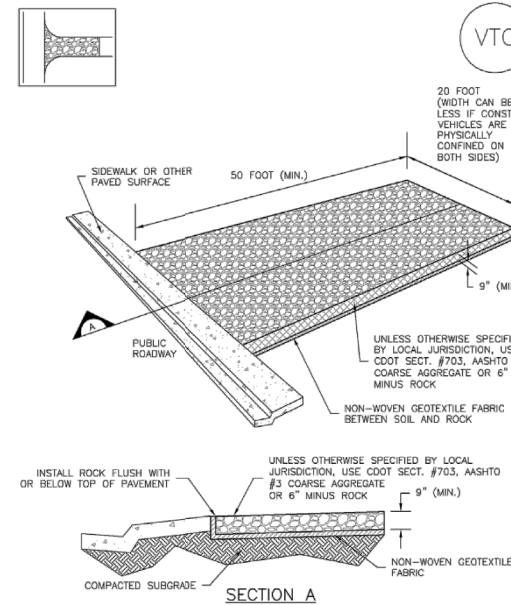
Inlet Protection (IP) SC-6

- Remove sediment accumulation from the area upstream of the inlet protection, as needed to maintain BMP effectiveness, typically when it reaches no more than half the storage capacity of the inlet protection. For silt fence, remove sediment when it accumulates to a depth of no more than 6 inches. Remove sediment accumulation from the area upstream of the inlet protection as needed to maintain the functionality of the BMP.
- Proprietary inlet protection devices should be inspected and maintained in accordance with manufacturer specifications. If proprietary inlet insert devices are used, sediment should be removed in a timely manner to prevent devices from breaking and spilling sediment into the storm drain.

Inlet protection must be removed and properly disposed of when the drainage area for the inlet has reached final stabilization.

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Vehicle Tracking Control (VTC) SM-4



VTC-1. AGGREGATE VEHICLE TRACKING CONTROL

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Urban Storm Drainage Criteria Manual Volume 3 VTC-3

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FOR LOCATING & MARKING GAS, ELECTRIC, WATER & TELEPHONE LINES
FOR BURIED UTILITY INFORMATION
CALL 1-800-922-1987

WWSO BOOSTER 2 PUMP STATION GRADING & EROSION CONTROL PLAN

PROJECT NO. 70-077 SCALE: HORIZONTAL: N/A VERTICAL: N/A
DATE: 08-28-19 SHEET X OF X C12

DESIGNED BY: DLW
DRAWN BY: DLW
CHECKED BY: VAS

20 BOULDER CRESCENT, SUITE 110
COLORADO SPRINGS, CO 80903
PHONE: 719.555.5465

M&S CIVIL CONSULTANTS, INC.

DARIN L. MOFFETT, COLORADO P.E. NO. 38923

FOR AND ON BEHALF OF M&S CIVIL CONSULTANTS, INC.

60% DESIGN DRAWINGS

NO.	DATE	DESCRIPTION	BY	DATE

THE OWNER PREPARED THESE PLANS AND WILL BE RESPONSIBLE FOR UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.

CAUTION

SM-4 Vehicle Tracking Control (VTC)

STABILIZED CONSTRUCTION ENTRANCE/EXIT INSTALLATION NOTES

- SEE PLAN VIEW FOR
 - LOCATION OF CONSTRUCTION ENTRANCE(S)/EXIT(S).
 - TYPE OF CONSTRUCTION ENTRANCE(S)/EXIT(S) (WITH/WITHOUT WHEEL WASH, CONSTRUCTION MAT OR TRM).
- CONSTRUCTION MAT OR TRM STABILIZED CONSTRUCTION ENTRANCES ARE ONLY TO BE USED ON SHORT DURATION PROJECTS (TYPICALLY RANGING FROM A WEEK TO A MONTH) WHERE THERE WILL BE LIMITED VEHICULAR ACCESS.
- A STABILIZED CONSTRUCTION ENTRANCE/EXIT SHALL BE LOCATED AT ALL ACCESS POINTS WHERE VEHICLES ACCESS THE CONSTRUCTION SITE FROM PAVED RIGHT-OF-WAYS.
- STABILIZED CONSTRUCTION ENTRANCE/EXIT SHALL BE INSTALLED PRIOR TO ANY LAND DISTURBING ACTIVITIES.
- A NON-WOVEN GEOTEXTILE FABRIC SHALL BE PLACED UNDER THE STABILIZED CONSTRUCTION ENTRANCE/EXIT PRIOR TO THE PLACEMENT OF ROCK.
- UNLESS OTHERWISE SPECIFIED BY LOCAL JURISDICTION, ROCK SHALL CONSIST OF DOT SECT. #703, AASHTO #3 COARSE AGGREGATE OR 6" (MINUS) ROCK.

STABILIZED CONSTRUCTION ENTRANCE/EXIT MAINTENANCE NOTES

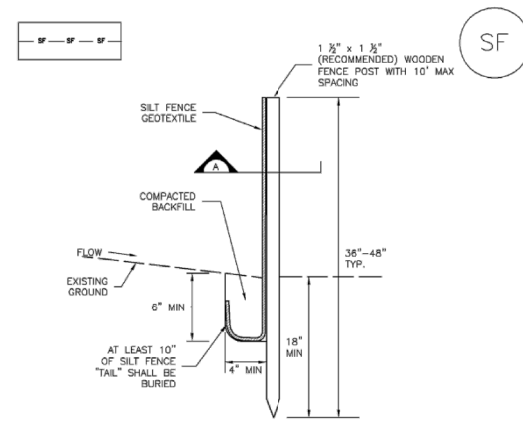
- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
- FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
- WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
- ROCK SHALL BE REAPPLIED OR REGRADED AS NECESSARY TO THE STABILIZED CONSTRUCTION ENTRANCE/EXIT TO MAINTAIN A CONSISTENT DEPTH.
- SEDIMENT TRACKED ONTO PAVED ROADS IS TO BE REMOVED THROUGHOUT THE DAY AND AT THE END OF THE DAY BY SHOVELING OR SWEEPING. SEDIMENT MAY NOT BE WASHED DOWN STORM SEWER DRAINS.

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDPCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

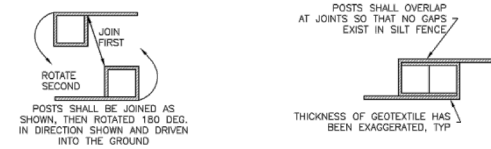
(DETAILS ADAPTED FROM CITY OF BROOMFIELD, COLORADO, NOT AVAILABLE IN AUTOCAD)

VTC-6 Urban Drainage and Flood Control District
Urban Storm Drainage Criteria Manual Volume 3 November 2010

Silt Fence (SF) SC-1



SILT FENCE



SECTION A

SF-1. SILT FENCE

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SC-1 Silt Fence (SF)

SILT FENCE INSTALLATION NOTES

- SILT FENCE MUST BE PLACED AWAY FROM THE TOE OF THE SLOPE TO ALLOW FOR WATER PONDING. SILT FENCE AT THE TOE OF A SLOPE SHOULD BE INSTALLED IN A FLAT LOCATION AT LEAST SEVERAL FEET (2-5 FT) FROM THE TOE OF THE SLOPE TO ALLOW ROOM FOR PONDING AND DEPOSITION.
- A UNIFORM 6" X 4" ANCHOR TRENCH SHALL BE EXCAVATED USING TRENCHER OR SILT FENCE INSTALLATION DEVICE. NO ROAD GRADERS, BACKHOES, OR SIMILAR EQUIPMENT SHALL BE USED.
- COMPACT ANCHOR TRENCH BY HAND WITH A "JUMPING JACK" OR BY WHEEL ROLLING. COMPACTION SHALL BE SUCH THAT SILT FENCE RESISTS BEING PULLED OUT OF ANCHOR TRENCH BY HAND.
- SILT FENCE SHALL BE PULLED TIGHT AS IT IS ANCHORED TO THE STAKES. THERE SHOULD BE NO NOTICEABLE SAG BETWEEN STAKES AFTER IT HAS BEEN ANCHORED TO THE STAKES.
- SILT FENCE FABRIC SHALL BE ANCHORED TO THE STAKES USING 1" HEAVY DUTY STAPLES OR NAILS WITH 1" HEADS. STAPLES AND NAILS SHOULD BE PLACED 3" ALONG THE FABRIC DOWN THE STAKE.
- AT THE END OF A RUN OF SILT FENCE ALONG A CONTOUR, THE SILT FENCE SHOULD BE TURNED PERPENDICULAR TO THE CONTOUR TO CREATE A "J-HOOK." THE "J-HOOK" EXTENDING PERPENDICULAR TO THE CONTOUR SHOULD BE OF SUFFICIENT LENGTH TO KEEP RUNOFF FROM FLOWING AROUND THE END OF THE SILT FENCE (TYPICALLY 10' - 20').
- SILT FENCE SHALL BE INSTALLED PRIOR TO ANY LAND DISTURBING ACTIVITIES.

SILT FENCE MAINTENANCE NOTES

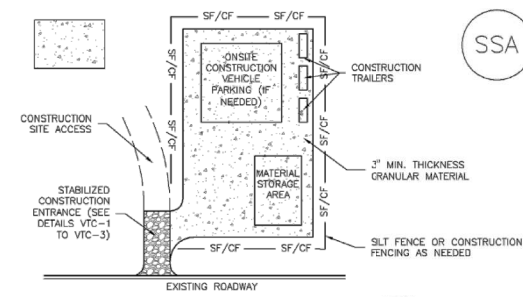
- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
- FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
- WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
- SEDIMENT ACCUMULATED UPSTREAM OF THE SILT FENCE SHALL BE REMOVED AS NEEDED TO MAINTAIN THE FUNCTIONALITY OF THE BMP, TYPICALLY WHEN DEPTH OF ACCUMULATED SEDIMENTS IS APPROXIMATELY 6".
- REPAIR OR REPLACE SILT FENCE WHEN THERE ARE SIGNS OF WEAR, SUCH AS SAGGING, TEARING, OR COLLAPSE.
- SILT FENCE IS TO REMAIN IN PLACE UNTIL THE UPSTREAM DISTURBED AREA IS STABILIZED AND APPROVED BY THE LOCAL JURISDICTION, OR IS REPLACED BY AN EQUIVALENT PERIMETER SEDIMENT CONTROL BMP.
- WHEN SILT FENCE IS REMOVED, ALL DISTURBED AREAS SHALL BE COVERED WITH TOPSOIL, SEEDED AND MULCHED OR OTHERWISE STABILIZED AS APPROVED BY LOCAL JURISDICTION.

(DETAIL ADAPTED FROM TOWN OF PARKER, COLORADO AND CITY OF AURORA, NOT AVAILABLE IN AUTOCAD)

NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDPCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

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Stabilized Staging Area (SSA) SM-6



SSA-1. STABILIZED STAGING AREA

STABILIZED STAGING AREA INSTALLATION NOTES

- SEE PLAN VIEW FOR
 - LOCATION OF STAGING AREA(S).
 - CONTRACTOR MAY ADJUST LOCATION AND SIZE OF STAGING AREA WITH APPROVAL FROM THE LOCAL JURISDICTION.
- STABILIZED STAGING AREA SHOULD BE APPROPRIATE FOR THE NEEDS OF THE SITE. OVERSIZING RESULTS IN A LARGER AREA TO STABILIZE FOLLOWING CONSTRUCTION.
- STAGING AREA SHALL BE STABILIZED PRIOR TO OTHER OPERATIONS ON THE SITE.
- THE STABILIZED STAGING AREA SHALL CONSIST OF A MINIMUM 3" THICK GRANULAR MATERIAL.
- UNLESS OTHERWISE SPECIFIED BY LOCAL JURISDICTION, ROCK SHALL CONSIST OF DOT SECT. #703, AASHTO #3 COARSE AGGREGATE OR 6" (MINUS) ROCK.
- ADDITIONAL PERIMETER BMPs MAY BE REQUIRED INCLUDING BUT NOT LIMITED TO SILT FENCE AND CONSTRUCTION FENCING.

STABILIZED STAGING AREA MAINTENANCE NOTES

- INSPECT BMPs EACH WORKDAY, AND MAINTAIN THEM IN EFFECTIVE OPERATING CONDITION. MAINTENANCE OF BMPs SHOULD BE PROACTIVE, NOT REACTIVE. INSPECT BMPs AS SOON AS POSSIBLE (AND ALWAYS WITHIN 24 HOURS) FOLLOWING A STORM THAT CAUSES SURFACE EROSION, AND PERFORM NECESSARY MAINTENANCE.
- FREQUENT OBSERVATIONS AND MAINTENANCE ARE NECESSARY TO MAINTAIN BMPs IN EFFECTIVE OPERATING CONDITION. INSPECTIONS AND CORRECTIVE MEASURES SHOULD BE DOCUMENTED THOROUGHLY.
- WHERE BMPs HAVE FAILED, REPAIR OR REPLACEMENT SHOULD BE INITIATED UPON DISCOVERY OF THE FAILURE.
- ROCK SHALL BE REAPPLIED OR REGRADED AS NECESSARY IF RUTTING OCCURS OR UNDERLYING SUBGRADE BECOMES EXPOSED.

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Urban Storm Drainage Criteria Manual Volume 3 SSA-3

SM-6 Stabilized Staging Area (SSA)

- Minimizing Long-Term Stabilization Requirements**
- Utilize off-site parking and restrict vehicle access to the site.
 - Use construction mats in lieu of rock when staging is provided in an area that will not be disturbed otherwise.
 - Consider use of a bermed contained area for materials and equipment that do not require a stabilized surface.
 - Consider phasing of staging areas to avoid disturbance in an area that will not be otherwise disturbed.

See Detail SSA-1 for a typical stabilized staging area and SSA-2 for a stabilized staging area when materials staging in roadways is required.

Maintenance and Removal

Maintenance of stabilized staging areas includes maintaining a stable surface cover of gravel, repairing perimeter controls, and following good housekeeping practices.

When construction is complete, debris, unused stockpiles and materials should be recycled or properly disposed. In some cases, this will require disposal of contaminated soil from equipment leaks in an appropriate landfill. Staging areas should then be permanently stabilized with vegetation or other surface cover planned for the development.

SSA-2 Urban Drainage and Flood Control District
Urban Storm Drainage Criteria Manual Volume 3 November 2010

SM-6 Stabilized Staging Area (SSA)

STABILIZED STAGING AREA MAINTENANCE NOTES

- STABILIZED STAGING AREA SHALL BE ENLARGED IF NECESSARY TO CONTAIN PARKING, STORAGE, AND UNLOADING/LOADING OPERATIONS.
 - THE STABILIZED STAGING AREA SHALL BE REMOVED AT THE END OF CONSTRUCTION. THE GRANULAR MATERIAL SHALL BE REMOVED OR, IF APPROVED BY THE LOCAL JURISDICTION, USED ON SITE, AND THE AREA COVERED WITH TOPSOIL, SEEDED AND MULCHED OR OTHERWISE STABILIZED IN A MANNER APPROVED BY LOCAL JURISDICTION.
- NOTE: MANY MUNICIPALITIES PROHIBIT THE USE OF RECYCLED CONCRETE AS GRANULAR MATERIAL FOR STABILIZED STAGING AREAS DUE TO DIFFICULTIES WITH RE-ESTABLISHMENT OF VEGETATION IN AREAS WHERE RECYCLED CONCRETE WAS PLACED.
- NOTE: MANY JURISDICTIONS HAVE BMP DETAILS THAT VARY FROM UDPCD STANDARD DETAILS. CONSULT WITH LOCAL JURISDICTIONS AS TO WHICH DETAIL SHOULD BE USED WHEN DIFFERENCES ARE NOTED.

(DETAILS ADAPTED FROM DOUGLAS COUNTY, COLORADO, NOT AVAILABLE IN AUTOCAD)

SSA-4 Urban Drainage and Flood Control District
Urban Storm Drainage Criteria Manual Volume 3 November 2010

WWSO BOOSTER 2 PUMP STATION

GRADING & EROSION CONTROL DETAILS

PROJECT NO. 70-077 DATE: 08-28-19

SCALE: HORIZONTAL: N/A VERTICAL: N/A

DESIGNED BY: DLM DRAWN BY: DLM CHECKED BY: VAS

SHEET X OF X **C13**

20 BOULDER CRESCENT, SUITE 110
COLORADO SPRINGS, CO 80903
PHONE: 719.555.5465

S&S CIVIL CONSULTANTS, INC.

DARIN L. MOFFETT, COLORADO P.E. NO. 38923

FOR AND ON BEHALF OF S&S CIVIL CONSULTANTS, INC.

60% DESIGN DRAWINGS

REVISIONS:	NO.	DATE:	DESCRIPTION:	APPROV. BY:	DATE:

THE ENGINEER PREPARING THESE PLANS WILL NOT BE RESPONSIBLE OR LIABLE FOR UNAUTHORIZED CHANGES TO OR USES OF THESE PLANS. ALL CHANGES TO THE PLANS MUST BE IN WRITING AND MUST BE APPROVED BY THE PREPARER OF THESE PLANS.

CAUTION

FOR LOCATING & MARKING GAS, ELECTRIC, WATER & TELEPHONE LINES

FOR BURIED UTILITY INFORMATION
48 HRS BEFORE YOU DIG
CALL 1-800-922-1987

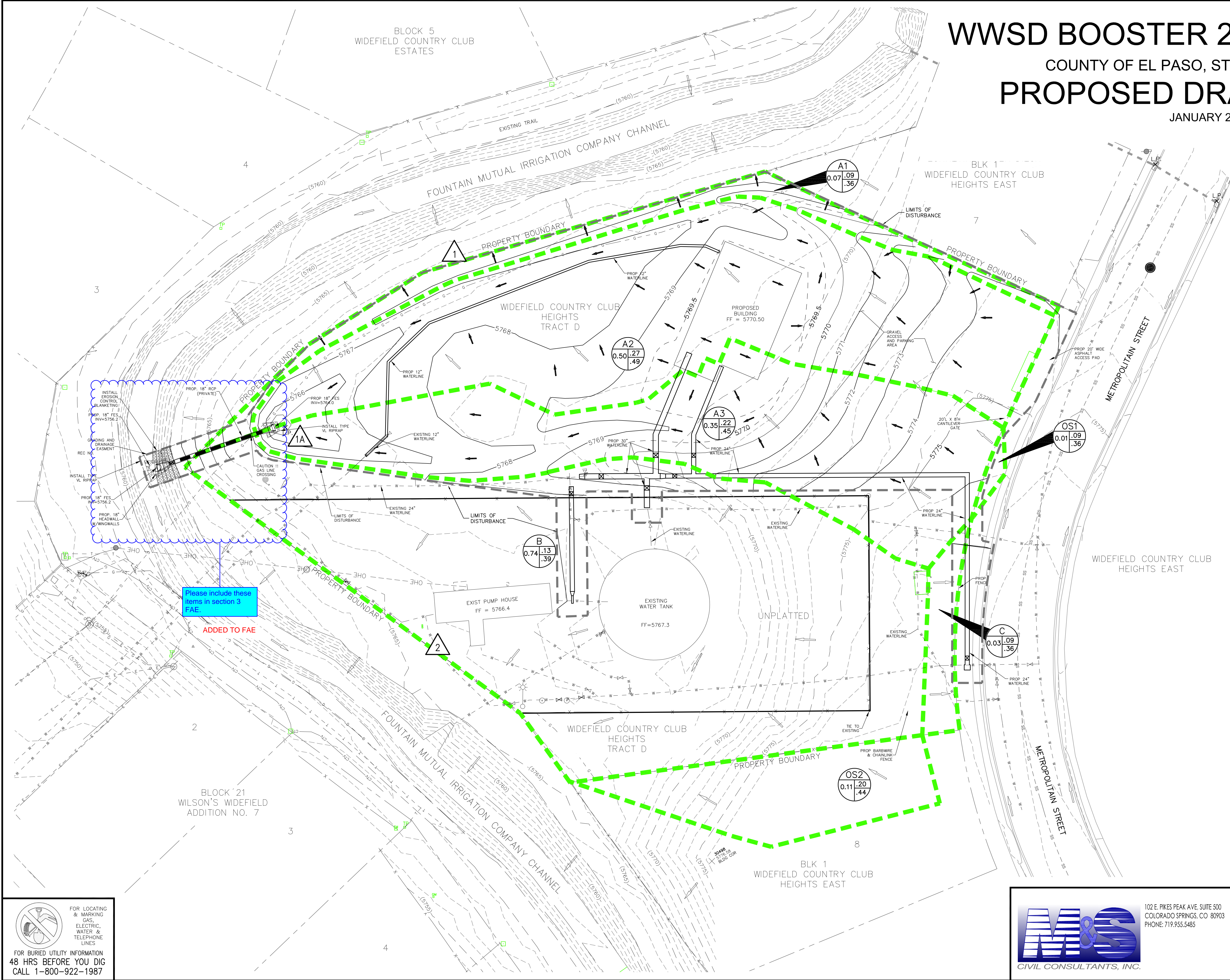
DRAFT FOR REFERENCE ONLY

WWSD BOOSTER 2 PUMP STATION

COUNTY OF EL PASO, STATE OF COLORADO

PROPOSED DRAINAGE MAP

JANUARY 2020



- LEGEND**
- BASIN DESIGNATION
 - SURFACE DESIGN POINT (DP)
 - PIPE RUN REFERENCE LABEL
 - BASIN BOUNDARY
 - EXISTING INDEX CONTOUR (5')
 - EXISTING NOMINAL CONTOUR (1')
 - WWSD SITE BOUNDARY
 - EXISTING FLOW DIRECTION ARROW
 - HIGH POINT / LOW POINT
 - PROPOSED STORM SEWER PIPE
 - FLARED END SECTION
 - EXISTING FLOW DIRECTION ARROW
 - PROPOSED FLOW DIRECTION

BASIN SUMMARY

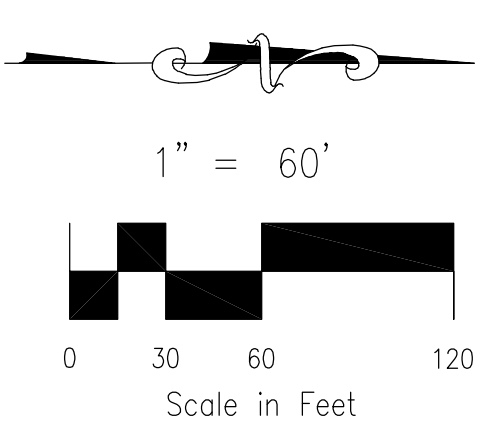
BASIN	AREA (ACRES)	Q ₅	Q ₁₀₀
A1	0.07	0.0*	0.2
A2	0.50	0.5	1.6
A3	0.35	0.3	1.0
B	0.74	0.4	2.1
C	0.03	0.0*	0.1*
OS1	<0.01	0.0*	0.0*
OS2	0.11	0.1	0.4

* RUNOFF FROM BASIN IS LESS THAN 0.1 CFS

DESIGN POINT SUMMARY

DESIGN POINT	Q ₅	Q ₁₀₀	BASIN & DES. PTS
1	0.0	0.2	A1
1A	0.8	2.6	OS1, A2, A3
2	0.6	2.9	OS2, B, C

Please include these items in section 3 FAE.
ADDED TO FAE



File: 0:\70077A_Widefield_WAS_05_Hydro\7010 metropolitan sheet\Map\Eng_Cambis\70077-PDM.dwg Plotstamp: 1/10/2020 1:59 PM

FOR LOCATING & MARKING GAS, ELECTRIC, WATER & TELEPHONE LINES
FOR BURIED UTILITY INFORMATION 48 HRS BEFORE YOU DIG CALL 1-800-922-1987

102 E. PIKES PEAK AVE. SUITE 500
COLORADO SPRINGS, CO 80903
PHONE: 719.955.5485

WWSD BOOSTER 2 PUMP STATION
PROPOSED DRAINAGE MAP

PROJECT NO. 70-077	SCALE:	DATE: 11/21/2019
DESIGNED BY: DLM	HORIZONTAL: 1"=20'	SHEET 1 OF 1
DRAWN BY: DLM	VERTICAL: N/A	
CHECKED BY: VAS		PDM