

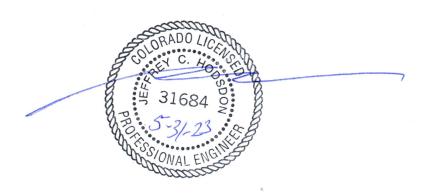
LSC TRANSPORTATION CONSULTANTS, INC. 2504 East Pikes Peak Avenue, Suite 304 Colorado Springs, CO 80909 (719) 633-2868 FAX (719) 633-5430

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Jubilee GroupRezone Traffic Impact Study EPC PCD File No. CS23I (LSC #S224600) May 26, 2023

<u>Traffic Engineer's Statement</u>

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.



Developer's Statement

I, the Developer, have read and will comply with all commitments made on my behalf within this report.

Steve H. Kana 5/24/2023

Date

Jubilee Group Rezone Traffic Impact Study

Prepared for:

Steve Kang 520 Edison St Brush CO, 80723-2011

MAY 26, 2023

LSC Transportation Consultants
Prepared by: Jeffrey C. Hodsdon, P.E.

LSC #S224600



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Figure 1 - Figure 7

Traffic Counts

Synchro LOS Reports

MTCP Maps



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May 26, 2023

Steve Kang 520 Edison St Brush CO, 80723-2011

> RE: 11401 E Highway 24 Property Rezone Traffic Impact Study El Paso County, CO EPC PCD File No. CS231

LSC # S224600

Dear Mr. Kang,

LSC Transportation Consultants, Inc. has prepared this traffic impact for the proposed rezone of the property located southeast of the intersection of US Hwy 24/Falcon Highway in El Paso County, Colorado. The property address is 11401 E HIGHWAY 24 and the El Paso County parcel number is 5313001013. The planned land use for the 14.35-acre site, once rezoned, is mini storage. One access point to Falcon Highway is envisioned for the property. The applicant is not seeking access to US Highway 24.

This report has been prepared for submittal to El Paso County.

REPORT CONTENTS

The preparation of this report included the following:

- Inventory of existing adjacent and nearby area road system. This included surface conditions, functional classifications, roadway widths, lane configurations, traffic control, posted speed limits, pavement markings, intersection and access spacing, roadway and intersection alignments, auxiliary left- and right-turn lanes, intersection sight distances, etc.;
- Morning and late-afternoon peak-hour turning-movement traffic counts at the "study-area" intersection of US Hwy 24/Falcon Highway;
- Review of previously-completed traffic studies in the vicinity of this site, the US 24 Planning & Environmental Linkages Study (PEL), and the US Highway 24 Access Control Plan for information and findings relative to this development. Other recent studies completed in the area and any applicable data/transferrable information/analysis etc. from previous LSC studies adjacent to the site were also utilized;
- Evaluation of intersection/access sight distance at the likely location of the site access -to Falcon Highway, based on current criteria in the County's Engineering Criteria Manual (ECM);

- Estimates of average weekday and peak-hour trip generation for the anticipated land use of the property, pending rezone approval;
- Estimation of directional distribution of site-generated vehicle trips on the area road system, at the study-area intersections, and at the proposed site-access point;
- Projections of site-generated turning-movement traffic volumes at the following study-area intersections:
 - US Hwy 24/Falcon Highway
 - Falcon Highway/proposed site access
- Estimates of short- and long-term background traffic volumes at the study-area intersections and access points;
- Total traffic (site traffic plus background traffic) projections at the study-area intersections for the short and long term;
- Level of service (LOS) analysis at the study-area intersections;
- Evaluation of existing, short-term, and long-term projected intersection volumes to determine the potential need for any new auxiliary right-/left-turn lanes on Falcon Highway at the proposed site access, based on the criteria in the County's Engineering Criteria Manual;
- Notice of required participation in the El Paso County Road Impact Fee Program;
- Other recommended improvements/modifications to study-area roads/intersections; and
- Summary of compiled data, analysis, findings, and recommendations.

LAND USE AND ACCESS

Proposed Land Use

Figure 1 shows the site location of the proposed rezone. The property is located on the southeast corner of the US Highway 24/Falcon Highway in El Paso County, Colorado. The 14.35-acre site is identified as El Paso County parcel ID 5313001013. The intended use for the property, once rezoned, is a mini warehouse development with 500 storage units anticipated for the initial phase and an additional 500 units in a future phase (1,000 total storage units at buildout). Details including building square footage and configuration, parking, internal circulation, etc. will follow later at the site development plan stage. A parcel detail is shown in Figure 2.

Proposed Site Access

One access point is needed for the property, The most likely location for the access would be in the northeast corner of the property. The exact access location can be determined at the Site Development Plan stage, but for this rezone report, the estimated location would be 404 feet east of the intersection of US Hwy 24/Falcon Highway (centerline spacing). This access point would be stop-sign controlled on the northbound approach and has been analyzed as a full-movement intersection with Falcon Highway.

ROAD AND TRAFFIC CONDITIONS

Figure 1 shows the roads adjacent to and in the vicinity of the site. Adjacent roads serving the site are identified below followed by a brief description of each:

US Highway (US) 24 extends east/west across Colorado connecting the Buena Vista, Colorado Springs, and Limon areas. US Hwy 24 is planned to be widened to four lanes through the Falcon area and is classified as an Expressway by the Colorado Department of Transportation (CDOT) and the 2016 *El Paso County Major Transportation Corridors Plan (MTCP)*. Adjacent to the site, US Hwy 24 has a posted speed limit of 55 miles per hour (mph). Auxiliary northbound-right and southbound-left turn lanes exist on US Hwy 24 approaching Falcon Highway.

Falcon Highway May 2023 Note: The segment of Falcon Highway adjacent to this site is now owned and maintained by the City of Colorado Springs. The City has jurisdictional control with respect to access and development-related right-of-way and infrastructure requirements. Falcon Highway extends east from US Highway 24 to Ellicott Highway. The City **PlanCOS** Major Thoroughfare Plan shows the classification of Falcon Highway as Principal Arterial. The 2040 El Paso County *MTCP* classification of Falcon Highway is a two-lane, Rural, Minor Arterial. Table 4 of the *MTCP* identifies existing Falcon Highway as a two-lane, rural, paved, "unimproved County Road." Adjacent to the site, the posted speed limit is 45 mph. Currently, the T-intersection of US Hwy 24/Falcon Highway is signalized with auxiliary turn lanes on US Highway 24.

Existing Traffic Volumes

Vehicular-turning-movement counts were conducted at the intersection of US Hwy 24/Falcon Highway. Figure 2 shows these turning-movement volumes, as well as the average weekday traffic volumes (estimated based on factored peak-hour count data) on the adjacent roadways. Raw count data is attached.

- US Hwy 24/Falcon Highway
 - Tuesday, November 17, 2022 from 6:30 8:30 a.m.
 - Tuesday, November 17, 2022 from 4:00 6:00 p.m.

SIGHT DISTANCE

The proposed access point must meet (and maintain) City of Colorado Springs Traffic Criteria Manual standards for sight distance.

Intersection Sight Distance ("Speed" Line of Sight)

With a 45-mph speed limit on Falcon Highway east and west of the site, the minimum sight distance for both approaches at the proposed site-access location is 500 feet (per Table 1 in Section 4.4 of the City *Traffic Criteria Manual*.

Sight distance to the east at the proposed site-access location exceed the required 500-foot requirement. Looking west from the proposed site-access location, sight distance is unobstructed to the signalized US Hwy 24/Falcon Highway intersection. This distance is about 350 feet. However, all vehicles arriving from the west turn onto Falcon Highway from US Highway 24 and are traveling at a turning speed, which is lower than 45 mph. Assuming vehicles turning onto Falcon Highway at about 20 to 25 mph, the requirement for sight distance for these speeds is 115 to 280 feet. The available 350 feet exceeds this range of values.

TRIP GENERATION

Estimates of the existing and projected vehicle trips to be generated by the site have been made using nationally-published average trip-generation rates for land use code "151 – Mini-Warehouse" in *Trip Generation*, 11th Edition, 2021 by the Institute of Transportation Engineers (ITE). The average weekday and weekday morning and afternoon peak-hour trip-generation estimates have been calculated by multiplying the ITE trip-generation rates for land use 151 times the number of storage units (expressed in terms of hundreds of storage units).

Table 1 (attached) presents the estimated site trip generation. The table includes estimates for Phase 1 and buildout of the project.

Based on the trip generation estimate for the proposed rezone, the site at buildout is projected to generate about 90 vehicle trips on the average weekday. During the weekday morning peak hour, approximately 3 vehicles would enter and 3 vehicles would exit the site. Approximately 4 entering vehicles and 4 exiting vehicles are projected for the weekday afternoon peak hour.

TRIP DISTRIBUTION AND ASSIGNMENT

Trip Directional Distribution

Estimating the directional distribution of site-generated vehicle trips to the study-area roads and intersections is a necessary component in determining the site's traffic impacts. Figure 3 shows the percentages of the site-generated vehicle trips projected to be oriented to and from the site's major approaches. Estimates have been based on the following factors: the proposed land use, the area road system serving the site, the traffic-count data at the intersection of US Hwy 24/Falcon Highway, previously-conducted traffic studies in the area, and the site's geographic location relative to the Falcon area, the greater City of Colorado Springs metro area, El Paso County, and the Pikes Peak region.

Site-Generated Traffic

Short Term

Figure 4 shows the projected short-term site-generated traffic volumes for the weekday morning and evening peak hours. Site-generated traffic volumes at the study-area intersections have been calculated by applying the directional-distribution percentages estimated by LSC (from Figure 3) to the Phase 1/Short Term trip-generation estimates (from Table 1).

Existing-Plus-Site-Generated Traffic Volumes

Figure 5 shows the sum of existing traffic volumes (from Figure 3) and Phase 1 site-generated peak-hour traffic volumes (shown in Figure 4). These volumes represent the projected short-term total traffic.

Estimated Future 2043 Background Traffic Volumes

Figure 6 shows the projected 20-year background traffic volumes for the year 2043. Estimated 2043 background through traffic volumes on US Highway 24 and Falcon Highway account for projected background growth of undeveloped parcels nearby and align with long-term traffic projections from previous LSC traffic studies in the vicinity of the site. Projected 20-year background traffic volumes do **not** include projected traffic to be generated by the proposed rezone.

Future 2043 Total Traffic Volumes

Figure 7 shows the projected 2043 total traffic volumes, which are the sum of 2043 background traffic volumes (from Figure 6) plus the buildout site-generated traffic volumes (from Figure 4).

LEVEL OF SERVICE ANALYSIS

The following intersections have been analyzed to determine the projected intersection levels of service for short- and long-term traffic scenarios for the morning and evening peak-hour time periods:

- US Highway 24/Falcon Highway
- Falcon Highway/proposed site access

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection and is indicated on a scale from "A" to "F." LOS A is indicative of little congestion or delay. LOS F indicates a high level of congestion or delay. Table 2 shows the level of service delay ranges for signalized and unsignalized intersections.

	Signalized Intersections	Unsignalized Intersections
Level of Service	Average Control Delay (Seconds per Vehicle)	Average Control Delay (Seconds per Vehicle) ⁽¹⁾
Level of Service	(Seconds per Venicie)	(Seconds per venicie)
Α	10.0 sec or less	10.0 sec or less
В	10.1-20.0 sec	10.1-15.0 sec
С	20.1-35.0 sec	15.1-25.0 sec
D	35.1-55.0 sec	25.1-35.0 sec
E	55.1-80.0 sec	35.1-50.0 sec
F	80.1 sec or more	50.1 sec or more

⁽¹⁾ For unsignalized intersections, if V/C ratio is greater than 1.0 the level of service is LOS F, regardless of the projected average control delay per vehicle.

Detailed Synchro reports are attached. A summary of LOS during the weekday morning and evening peak hours for the following intersections is shown in the following figures:

- Figure 2: Existing Traffic, Lane Geometry, Traffic Control, and LOS
- Figure 5: Short-Term Total Traffic, Lane Geometry, Traffic Control, and LOS
- Figure 6: 2042 Background Traffic, Lane Geometry, Traffic Control, and LOS
- Figure 7: 2042 Background + Site Traffic, Lane Geometry, Traffic Control, and LOS

US Hwy 24/Falcon Highway

Short Term

The westbound approach at the intersection of US Hwy 24/Falcon Highway currently operates at LOS E during the morning peak hour and is projected to remain at LOS E during the short-term morning peak hour with the addition of site-generated traffic. All other turning movements currently operate at and are projected to remain at LOS D or better during both short-term peak hours, with or without the addition of site-generated traffic.

Long Term

Assuming US Hwy 24 would have six lanes adjacent to Falcon Highway, the westbound approach at the intersection of US Hwy 24/Falcon Highway is projected to operate at LOS F or worse during both long-term peak hours, with or without the addition of site-generated traffic. This level of service is based on the existing westbound approach laneage. Overall, the signalized intersection of US Hwy 24/Falcon Highway is projected to operate at LOS D during both long-term peak hours, with or without the addition of site-generated traffic.

Falcon Highway/Proposed Site Access

All individual turning movements at the proposed site-access intersection with Falcon Highway are projected to operate at LOS C or better during all short-term and long-term scenarios following the addition of site-generated traffic.

AUXILIARY TURN-LANE NEEDS ANALYSIS

The City *Traffic Criteria Manual* contains turning-volume thresholds which require auxiliary left- or right-turn lanes by roadway classifications.

Falcon Highway – Principal Arterial

Falcon Highway/Proposed Site Access

Left-Turn Deceleration Lanes

Left-turn deceleration auxiliary turn lanes are required for a Principal Arterial access with a projected peak-hour left-ingress turning volume of 10 vph or greater. The westbound-left turn volume is **not** projected to exceed this 10-vph threshold during either peak hour following the completion of the proposed development. As such, no modifications would be required to the existing westbound approach on Falcon Highway approaching the proposed site access.

Right-Turn Deceleration Lanes

Right-turn deceleration auxiliary turn lanes are required for a Principal Arterial access with a projected peak-hour right-ingress turning volume of 25 vph or greater. The eastbound-right turn volume is **not** projected to exceed this 25-vph threshold during either peak hour following the completion of the proposed development. As such, no modifications would be required to the existing eastbound approach on Falcon Highway approaching the proposed site access.

Right-Turn Acceleration Lanes

Per the *Traffic Criteria Manual*, a right-turn acceleration lane is generally not required on Principal Arterial roadways. As such, an eastbound-right-turn acceleration lane would **not** be required at the proposed site access on Falcon Highway.

MAJOR TRANSPORTATION CORRIDORS PLAN (MTCP)

Roadway Classifications

The following study-area roadway improvements are shown on Map 13 and Table 5 of the El Paso County 2016 *MTCP*:

- Falcon Highway 2-Lane Minor Arterial (Rural). the Corridor Preservation Plan shows Falcon Highway as a four-lane Minor Arterial.
- The City *PlanCOS* Classification Map shows Falcon highway as a Principal Arterial.
- US Highway 24 6-Lane Principal Arterial (Rural)

Reimbursable Improvements

The following roadway improvement projects have been identified as being needed by the year 2040 per Map 13 and Table 4 of El Paso County's 2016 MTCP:

- U5 Falcon Highway from US Hwy 24 to 1 mile east of Curtis Road (\$16,509,000)
- Existing conditions 2-lane Rural Unimproved County Road
- Future conditions 2-lane Rural Minor Arterial
- SH 5 US Highway 24 (Garrett Road to Woodmen Road) 6-Lane Principal Arterial (Rural).
 Note: Road segment improvements for this section of US Highway 24 are unlikely to be required by this development, so reimbursement would not be applicable. Although not called for in this study, generally intersection improvements (including turn-lane improvements and/or signalization at US Highway 24/side street arterial intersections may be reimbursable.

See the attached *MTCP* maps for reference.

COUNTY ROAD IMPROVEMENT FEE PROGRAM

Transportation Impact Fees

This project will be required to participate in the El Paso County Road Improvement Fee Program per BOCC Resolution 19-471. The option for participation will be identified at the site development plan stage.

MULTI-MODAL TRANSPORTATION AND TDM OPPORTUNITIES

No multi-modal improvement projects have been identified as being needed by the year 2040, per Map 15 and Table 5 of El Paso County's 2016 MTCP. With respect to sidewalks, bike lanes, and/or transit stop(s) adjacent to the site on Falcon Highway, the City comments for this application indicate that "all public improvements to be built to full City standards [...] if Falcon Highway is City of Colorado Springs right-of-way" [County staff comments indicate that this is the case]. The standard City cross section for a Type I Principal Arterial includes a multi-use paved shoulder (which will accommodate

Page 9

bicycles), curb and gutter, and detached sidewalk. This project would likely only be required to construct public improvements along the property frontage. The specific city requirements for public improvements can be addressed at the site development plan stage.

There is a Park-N-Ride facility located nearby at the southeast corner of New Meridian Road and Swingline Road.

CDOT REQUIREMENTS

The site-generated traffic would not increase traffic on the east leg of US Hwy 24/Falcon Highway by more than 20 percent, and it is unlikely that CDOT would require highway improvements of this development. Therefore, an access permit is not likely to be required by CDOT. CDOT may require or request additional right-of-way for US Highway 24 along this property's frontage. We anticipate that CDOT will specify improvements or other requirements (if any) as part of a referral review letter. Any CDOT requirements can be addressed at the site development plan stage and/or as part of the Access Permit process (if CDOT determines that an access permit would be required). The current application is only for rezoning.

CITY REQUIREMENTS

City comments for this application indicate that "all public improvements to be built to full City standards [...] if Falcon Highway is City of Colorado Springs right-of-way" [County staff comments indicate that this is the case]. The standard City cross section for a Type I Principal Arterial includes four through lanes, a center median for left turns, multi-use paved outside shoulders (which will accommodate bicycles), curb and gutter, and detached sidewalks on each side. This project would likely only be required to construct public improvements along the property frontage. The specific city requirements for public improvements can be addressed at the site development plan stage.

RIGHT-OF-WAY REQUIREMENTS

City of Colorado Springs

This rezone site is adjacent to Falcon Highway, which is now a City street. The City classifies Falcon Highway as a Principal Arterial. Based on Section 16 of the Traffic Criteria Manual, and assuming the adjacent roadway to be a "Type 1 (4-lane) Principal Arterial," the corridor right-of-way width required is 107 feet.

CDOT

This rezone site is adjacent to US Highway 24, which is a CDOT facility. We anticipate that CDOT will specify their requested/required right-of-way needs along US Highway 24 as part of a referral review letter. The right-of-way needs of CDOT can be addressed at the site development plan stage. The current application is only for rezoning.

DEVIATIONS

No transportation-related deviations to *ECM* design criteria are requested.

SUMMARY OF FINDINGS

- The proposed development is projected to generate about 180 vehicle trips on the average weekday at buildout.
- During the weekday morning peak hour, 6 vehicles would enter the site while 6 vehicles would exit the site.
- During the weekday evening peak hour, 8 vehicles would enter the site while 8 vehicles would exit the site.
- Please refer to the "Level of Service" section above for detailed LOS analysis results.
- Based on projected turning movement volumes and City criteria, no auxiliary turn-lane improvements would be required at the proposed site access on Falcon Highway. Please refer to the "Auxiliary Turn-Lane Analysis" section more details.
- Additional details regarding exact access location, access design details, Roadway Improvement Fee Program option, and City/CDOT requirements can be addressed in detail at the site development plan stage.

Please contact me if you have any questions regarding this report.

Respectfully submitted,

LSC TRANSPORTATION CONSULTANTS, INC.

By: Jeffrey C. Hodsdon, P.E. Principal

JCH/JAB:jas

Enclosures: Table 1

> Figure 1 - Figure 7 **Traffic Counts** Synchro LOS Reports

MTCP Maps

Recent Area TIS Reports

Table 1



Table 1: Trip Generation Estimate

	ITC				Trip Ge	neration	Rates ²		[Orivewa	y Trips G	enerated	i
	ITE	Value	Units ¹	Averag		M.		M.	Averag	Α	.M.	P.	м.
Code	Description			е	In	Out	In	Out	е	In	Out	In	Out
Phase	1												
151	Mini-Warehouse	5.000	SU (100s)	17.96	0.62	0.59	0.84	0.84	90	3	3	4	4
Phase	2												
151	Mini-Warehouse	5.000	SU (100s)	17.96	0.62	0.59	0.84	0.84	90	3	3	4	4
	Site Buildout								180	6	6	8	8
¹ SU (10	00s) = 100 storage units												
² Sourc	e: Trip Generation, 11th	Edition (20.	21) by the Ir	nstitute of	Transpo	ortation E	ngineers	(ITE)			May	25, 2023	1

Figures 1-7



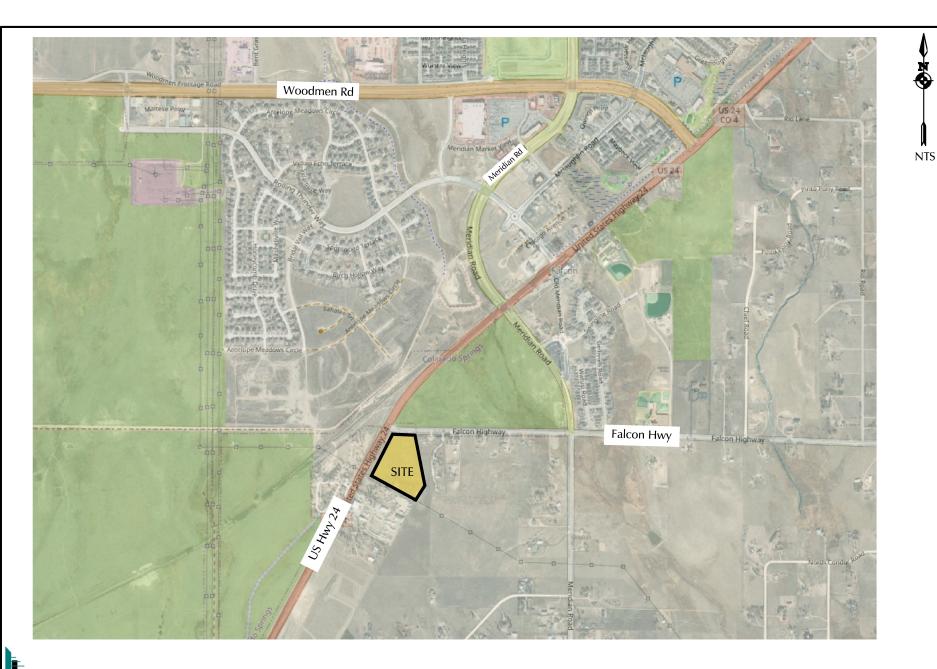






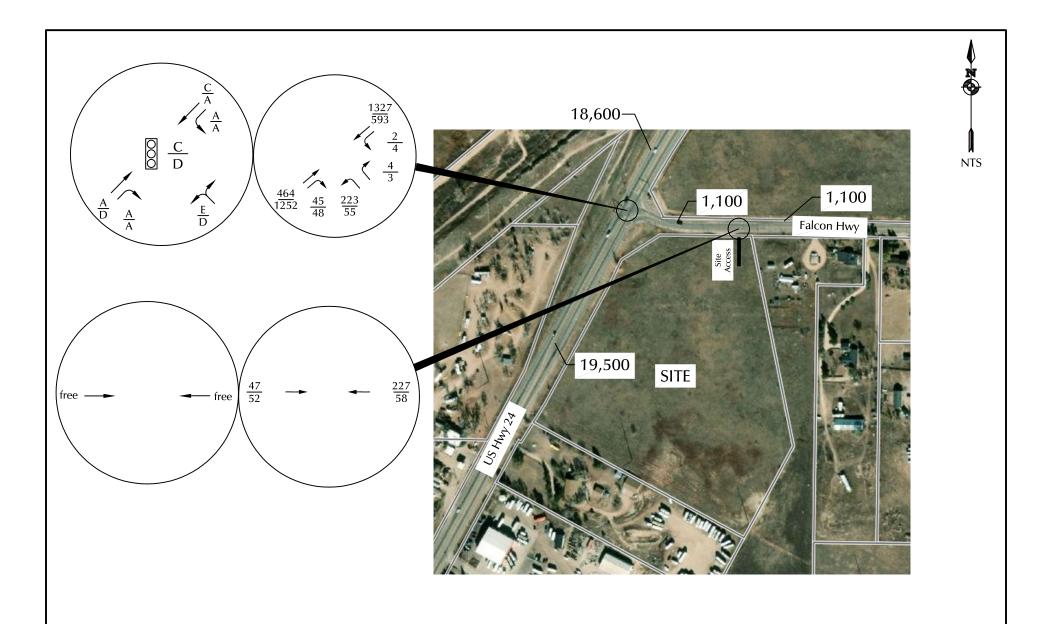


Figure 2

Site Rezone Information and Access Location

11401 E Highway 24 Property Rezone (LSC # S224600)









AM Individual Movement Peak-Hour Level-of-Service
PM Individual Movement Peak-Hour Level-of-Service

\frac{XX}{XX} = \frac{AM Weekday Peak-Hour Traffic (Veh/Hour)}{PM Weekday Peak-Hour Traffic (Veh/Hour)}

X,XXX = Average Daily Traffic (Vehicles/Day) CDOT 2022



Stop Sign

Counts by LSC (11/2022)

Existing Traffic, Lane Geometry, Traffic Control, and LOS

11401 E Highway 24 Property Rezone (LSC # S224600)





Figure 4

LEGEND:

Estimated directional distribution of site-generated trips (% of entering or exiting traffic)



Percent AM Peak-Hour Distribution
Percent PM Peak-Hour Distribution

Estimated Directional Distribution

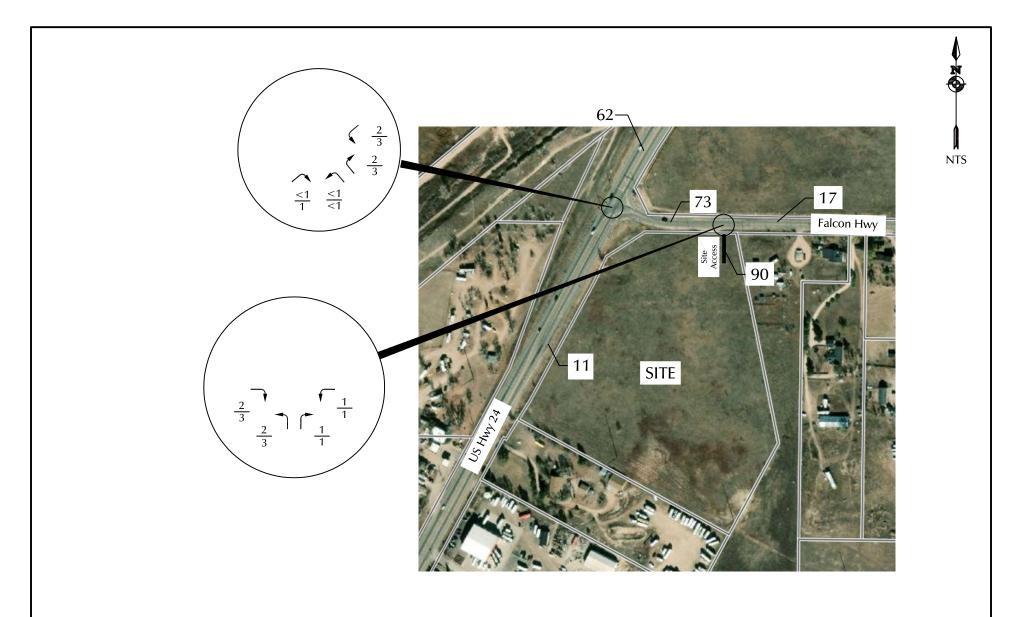


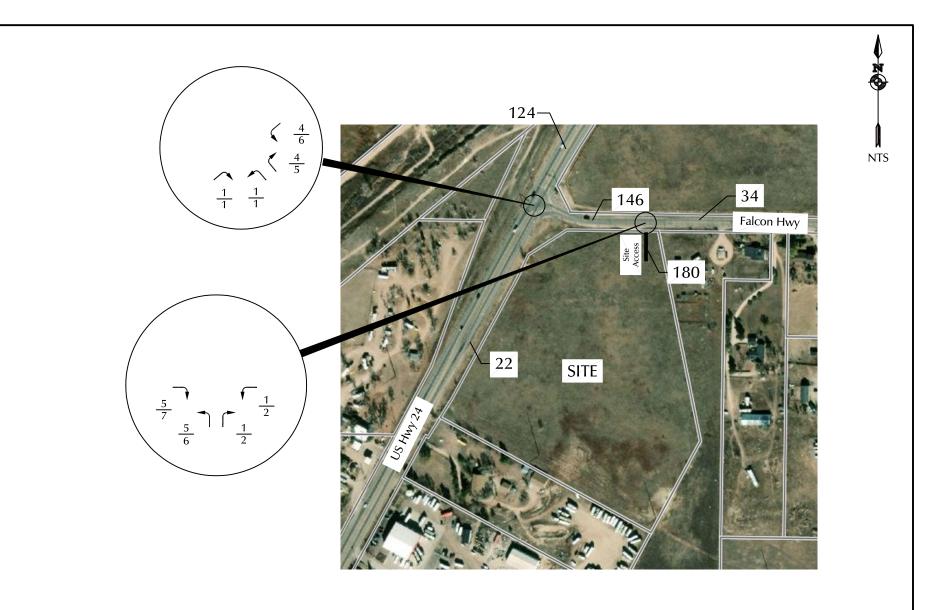
Figure 5a

Short-Term (Phase 1) Site-Generated Traffic

11401 E Highway 24 Property Rezone (LSC # S224600)



 $\frac{XX}{XX} = \frac{AM \text{ Weekday Peak-Hour Traffic (Veh/Hour)}}{PM \text{ Weekday Peak-Hour Traffic (Veh/Hour)}}$ X,XXX = Average Daily Traffic (Vehicles/Day)



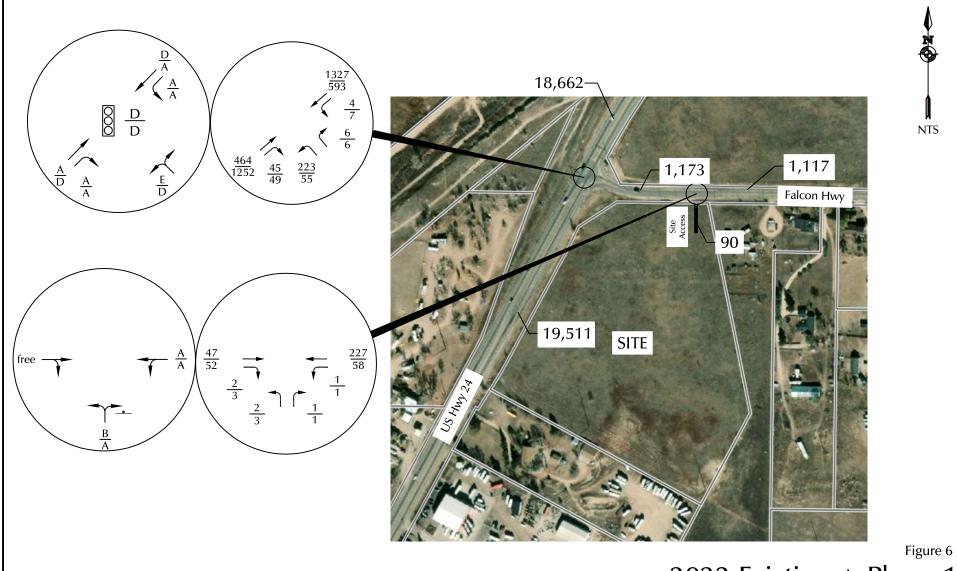


Phases 1&2/Buildout Site-Generated Traffic

11401 E Highway 24 Property Rezone (LSC # S224600)



 $\frac{XX}{XX} = \frac{AM \text{ Weekday Peak-Hour Traffic (Veh/Hour)}}{PM \text{ Weekday Peak-Hour Traffic (Veh/Hour)}}$ X,XXX = Average Daily Traffic (Vehicles/Day)



2022 Existing + Phase 1 Site-Generated Traffic, Lane Geometry, Traffic Control, and

TRANSPORTATION CONSULTANTS INC.

 $\frac{X}{X} = \frac{AM \text{ Individual Movement Peak-Hour Level-of-Service}}{PM \text{ Individual Movement Peak-Hour Level-of-Service}}$ $XX = \frac{AM \text{ Weekday Peak-Hour Traffic (Veh/Hour)}}{AM \text{ Weekday Peak-Hour Traffic (Veh/Hour)}}$

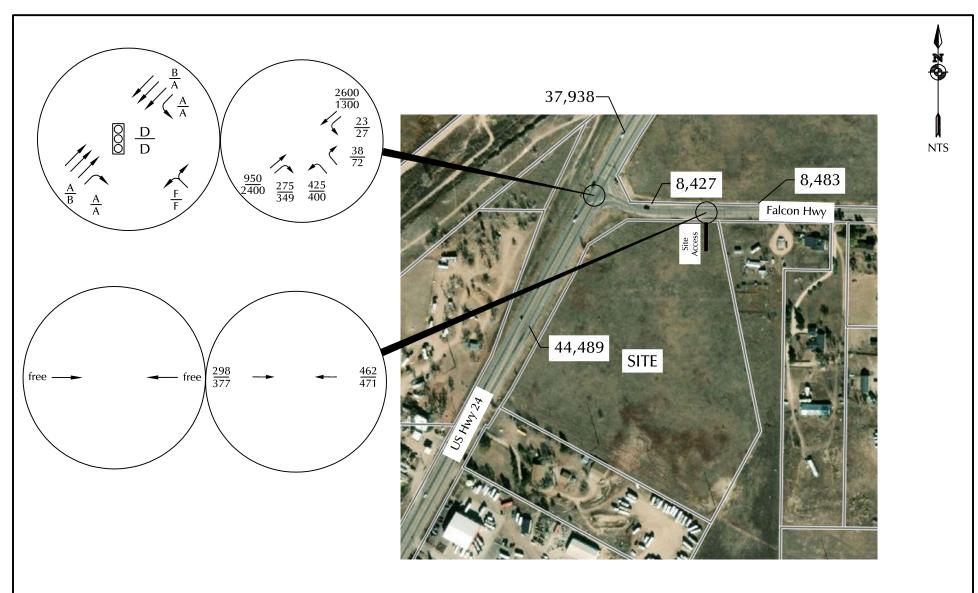
PM Weekday Peak-Hour Traffic (Veh/Hour)

X,XXX = Average Daily Traffic (Vehicles/Day)



Stop Sign

11401 E Highway 24 Property Rezone (LSC # S224600)





2042 Background Traffic, Lane Geometry, Traffic Control, and LOS

11401 E Highway 24 Property Rezone (LSC # S224600)

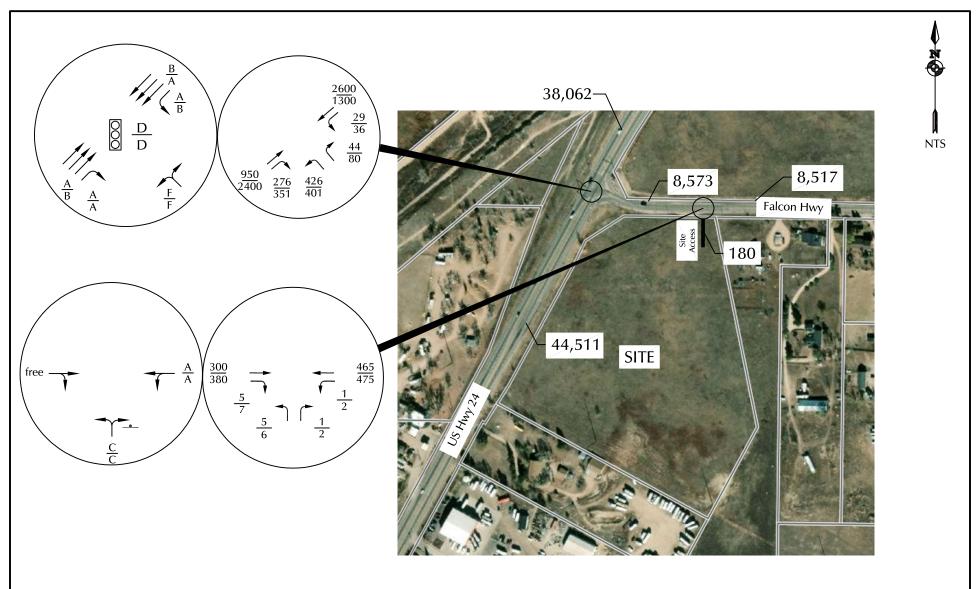
 $\frac{X}{X} = \frac{AM \text{ Individual Movement Peak-Hour Level-of-Service}}{PM \text{ Individual Movement Peak-Hour Level-of-Service}}$

 $\frac{XX}{XX}$ = $\frac{AM \text{ Weekday Peak-Hour Traffic (Veh/Hour)}}{PM \text{ Weekday Peak-Hour Traffic (Veh/Hour)}}$

X,XXX = Average Daily Traffic (Vehicles/Day)







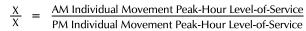
= Traffic Signal

= Stop Sign

Figure 8

2042 Total Traffic, Lane Geometry, Traffic Control, and LOS

11401 E Highway 24 Property Rezone (LSC # S224600)



 $\frac{XX}{XX}$ = $\frac{AM \text{ Weekday Peak-Hour Traffic (Veh/Hour)}}{PM \text{ Weekday Peak-Hour Traffic (Veh/Hour)}}$

X,XXX = Average Daily Traffic (Vehicles/Day)



Traffic Counts



719-633-2868

File Name: Hwy 24 - Falcon Hwy AM

Site Code: S224600 Start Date : 11/17/2022

Page No : 1

Groups Printed- Unshifted

06:30 0 123 0 0 123 0 0 10 0 10 1 52 0 0 53 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0											Printe	d- Uns										1
Start Time Right Thru Left Peds App. Total Right Thru Left Peds Right Thru Left Peds Right Thru Left Peds Right Thru				Hwy 2	24			Fa	Icon I	Hwy				Hwy 2	24			Fa	lcon l	Hwy		ĺ
06:30 0 123 0 0 123 0 0 10 0 10 1 52 0 0 53 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			So	uthbo	und			W	estbo	und			No	rthbo	und			Ea	stbo	und		
06:35 0 123 0 0 123 0 0 5 0 5 2 42 0 0 44 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
06:40 0 118 0 0 118 0 0 9 9 0 9 3 54 0 0 57 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	06:30	0	123	0	0	123	0	0	10	0	10	1	52	0	0	53	0	0	0	0	0	186
06:45 0 119 0 0 119 0 0 7 0 7 3 33 0 0 36 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	06:35	0	123	0	0	123	0	0	5	0	5	2	42	0	0	44	0	0	0	0	0	172
06:50 0 119 0 0 119 0 0 17 0 17 3 39 0 0 42 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	06:40	0	118	0	0	118	0	0	9	0	9	3	54	0	0	57	0	0	0	0	0	184
06:55 0 111 0 0 111 0 0 16 0 16 1 28 0 0 29 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	06:45	0	119	0	0	119	0	0	7	0	7	3	33	0	0	36	0	0	0	0	0	162
Total 0 713 0 0 64 0 64 13 248 0 0 261 0 0 0 0 0 1 07:00 0 106 0 0 18 0 18 0 28 0	06:50	o	119	0	0	119	0	0	17	0	17	3	39	0	0	42	0	0	0	0	0	178
07:00 0 106 0 0 106 0 0 18 0 28 0 28 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	06:55	0	111	0	0	111	0	0	16	0	16	1	28	0	0	29	0	0	0	0	0	156
07:05 0 103 0 1 04 0 0 25 0 25 1 37 0 0 38 0 0 0 0 0 0 0 07:10 0 113 0 0 113 1 0 16 0 17 2 29 0 0 31 0 0 0 0 0 0 0 0 0 0 0 07:15 0 113 0 0 113 0 0 113 0 0 27 0 27 2 47 0 0 49 0 0 0 0 0 0 0 0 0 0 0 07:20 0 129 0 0 129 0 0 17 0 17 6 39 0 0 45 0 0 0 0 0 0 0 0 0 0 0 07:25 0 109 0 0 109 0 0 109 0 0 149 0 0 17 0 17 6 29 0 0 35 0 0 0 0 0 0 0 0 0 0 0 07:30 0 109 0 0 109 0 0 17 0 17 6 29 0 0 33 0 33 2 40 0 0 42 0 0 0 0 0 0 0 0 0 0 0 07:40 0 117 0 0 117 2 0 12 0 14 5 46 0 0 51 0 0 0 0 0 0 0 0 0 0 0 0 07:45 0 113 0 0 113 0 0 114 0 0 114 0 0 11 0 11	Total	0	713	0	0	713	0	0	64	0	64	13	248	0	0	261	0	0	0	0	0	1038
07:05 0 103 0 1 1 104 0 0 25 0 25 1 37 0 0 38 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																						
07:10 0 113 0 0 113 1 0 16 0 17 2 29 0 0 31 0 0 0 0 0 0 0 07:15 0 113 0 0 113 0 0 27 27 2 47 0 0 49 0 0 0 0 0 0 07:20 0 129 0 0 129 0 0 17 0 17 6 39 0 0 45 0 0 0 0 0 0 0 07:25 0 109 0 0 109 0 0 24 24 4 31 0 0 35 0 0 0 0 0 0 07:30 0 109 0 0 109 0 0 17 0 17 6 29 0 0 35 0 0 0 0 0 0 07:35 0 107 0 0 107 0 0 33 0 33 2 40 0 42 0 0 0 0 0 07:40 0 117 0 0 117 2 0 12 14 5 46 0 0 51 0 0 0 0 0 07:45 0 113 0 0 114 0 0 16 16 5 32 0 0 37 0 0 0 0 0 07:50 0 144 0 0 114 0 0 11 0 11 0 0 0 0 0 0 0 0	07:00	0	106	0	0	106	0	0	18	0	18	0	28	0	0	28	0	0	0	0	0	152
07:15 0 113 0 0 113 0 0 27 0 27 2 47 0 0 49 0 0 0 0 0 0 0	07:05	0	103	0	1	104	0	0	25	0	25	1	37	0	0	38	0	0	0	0	0	167
07:20 0 129 0 0 129 0 0 129 0 0 17 0 17 6 39 0 0 45 0 0 0 0 0 0 0	07:10	0	113	0	0	113	1	0	16	0	17	2	29	0	0	31	0	0	0	0	0	161
07:25 0 109 0 0 109 0 0 24 0 24 4 31 0 0 35 0 0 0 0 0 0 0	07:15	0	113	0	0	113	0	0	27	0	27	2	47	0	0	49	0	0	0	0	0	189
07:30 0 109 0 0 109 0 0 17 0 17 6 29 0 0 35 0 0 0 0 0 0 0	07:20	0	129	0	0	129	0	0	17	0	17	6	39	0	0	45	0	0	0	0	0	191
07:35 0 107 0 0 107 0 0 33 0 33 2 40 0 0 42 0 0 0 0 0 0 0 <td>07:25</td> <td>0</td> <td>109</td> <td>0</td> <td>0</td> <td>109</td> <td>0</td> <td>0</td> <td>24</td> <td>0</td> <td>24</td> <td>4</td> <td>31</td> <td>0</td> <td>0</td> <td>35</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>0</td> <td>168</td>	07:25	0	109	0	0	109	0	0	24	0	24	4	31	0	0	35	0	0	0	0	0	168
07:40 0 117 0 0 117 2 0 12 0 14 5 46 0 0 51 0 0 0 0 0 0 07:45 0 113 0 0 113 0 0 16 0 16 5 32 0 0 37 0 0 0 0 0 0 07:50 0 114 0 0 114 0 0 11 0 11 7 56 0 0 63 0 0 0 0 0 07:55 0 94 2 0 96 1 0 7 0 8 5 50 0 0 55 0 0 0 0 0				0	0	109	0	0		0				0	0		0	0	0	0	0	161
07:45 0 113 0 0 113 0 0 16 0 16 5 32 0 0 37 0 0 0 0 0 0 0 0 0 07:50 0 114 0 0 114 0 0 11 0 11 7 56 0 0 63 0 0 0 0 0 0 0 0 0 07:55 0 94 2 0 96 1 0 7 0 8 5 50 0 0 55 0 0 0 0 0 0 0 0	07:35	0	107	0	0	107	0	0	33	0	33	2	40	0	0	42	0	0	0	0	0	182
07:50 0 114 0 0 114 0 0 11 0 11 7 56 0 0 63 0 0 0 0 0 0 07:55 0 94 2 0 96 1 0 7 0 8 5 50 0 0 55 0 0 0 0 0 0				0	0		2	0		0	14	_	_	0	0	51	0	0	0	0	0	182
<u>07:55</u> 0 94 2 0 96 1 0 7 0 8 5 50 0 0 55 0 0 0 0 0	07:45	0	113	0	0	113	0	0	16	0	16	5	32	0	0	37	0	0	0	0	0	166
				-	0		0	0		0				0	0		0	-	0	-	0	188
							1															159
Total 0 1327 2 1 1330 4 0 223 0 227 45 464 0 0 509 0 0 0 0 0 2	Total	0	1327	2	1	1330	4	0	223	0	227	45	464	0	0	509	0	0	0	0	0	2066
																						ı
			_	_	-	_	l	-		-	_		_		-					-	-	131
		1 -		0	-		l	-	4		5	i			-		-			-	-	120
		1 -		1	-		_			-		i .	-		-		-	_		-	-	131
			-	0	-		_	_		-							-				-	144
		1		1	-		_	-		-	-	1		_	-	-	-	-	_	_	-	110
		0		-	0		_	-	-	_	-	_		-	-		_	_	-	_	-	113
		"		-	1	2509	_	-			326		-	-	-	1018	_	-	-	-	0	3853
Apprch % 0 99.8 0.2 0 1.8 0 98.2 0 7.6 92.4 0 0 0 0 0				-	-			-		-		_										
Total % 0 65 0.1 0 65.1 0.2 0 8.3 0 8.5 2 24.4 0 0 26.4 0 0 0 0 0	Total %	0	65	0.1	0	65.1	0.2	0	8.3	0	8.5	2	24.4	0	0	26.4	0	0	0	0	0	i

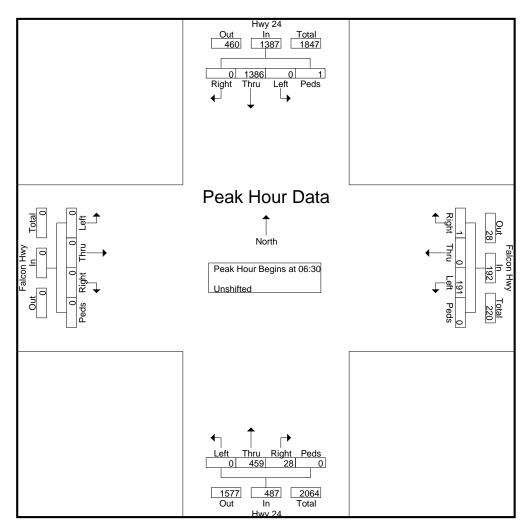
719-633-2868

File Name: Hwy 24 - Falcon Hwy AM

Site Code: S224600 Start Date : 11/17/2022

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			Hwy 2	24			Fa	lcon l	lwy				Hwy 2	24			Fa	lcon I	lwy		
		So	uthbo	und			W	estbo	und			No	rthbo	und			Ea	stbo	ınd		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour A							of 1														
Peak Hour f	or Ent	ire Inte	ersecti	ion Be	gins at	06:30															
06:30	0	123	0	0	123	0	0	10	0	10	1	52	0	0	53	0	0	0	0	0	186
06:35	0	123	0	0	123	0	0	5	0	5	2	42	0	0	44	0	0	0	0	0	172
06:40	0	118	0	0	118	0	0	9	0	9	3	54	0	0	57	0	0	0	0	0	184
06:45	0	119	0	0	119	0	0	7	0	7	3	33	0	0	36	0	0	0	0	0	162
06:50	0	119	0	0	119	0	0	17	0	17	3	39	0	0	42	0	0	0	0	0	178
06:55	0	111	0	0	111	0	0	16	0	16	1	28	0	0	29	0	0	0	0	0	156
07:00	0	106	0	0	106	0	0	18	0	18	0	28	0	0	28	0	0	0	0	0	152
07:05	0	103	0	1	104	0	0	25	0	25	1	37	0	0	38	0	0	0	0	0	167
07:10	0	113	0	0	113	1	0	16	0	17	2	29	0	0	31	0	0	0	0	0	161
07:15	0	113	0	0	113	0	0	27	0	27	2	47	0	0	49	0	0	0	0	0	189
07:20	0	129	0	0	129	0	0	17	0	17	6	39	0	0	45	0	0	0	0	0	191
07:25	0	109	0	0	109	0	0	24	0	24	4	31	0	0	35	0	0	0	0	0	168
Total Volume	0	1386	0	1	1387	1	0	191	0	192	28	459	0	0	487	0	0	0	0	0	2066
% App. Total	0	99.9	0	0.1		0.5	0	99.5	0		5.7	94.3	0	0		0	0	0	0		
PHF	.000	.895	.000	.083	.896	.083	.000	.590	.000	.593	.389	.708	.000	.000	.712	.000	.000	.000	.000	.000	.901



719-633-2868

File Name: Hwy 24 - Falcon Hwy AM

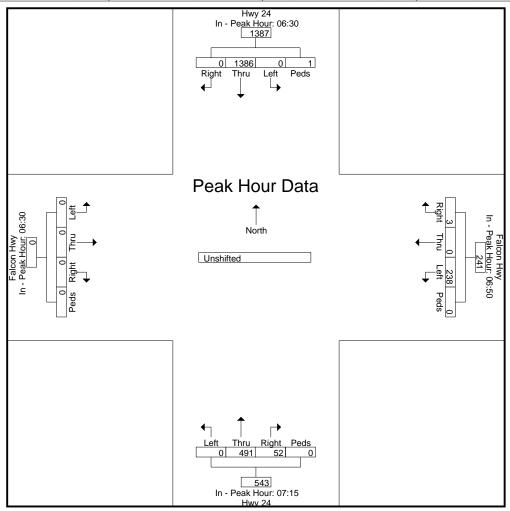
Site Code: S224600 Start Date : 11/17/2022

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			Hwy 2 uthbo					Icon I estbo	,				Hwy 2 rthbo					Icon I	•		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total

Peak Hour Analysis From 06:30 to 08:25 - Peak 1 of 1

Peak Hour f	or Eac	ch App	<u>roach</u>	Begir	s at:															
	06:30					06:50					07:15					06:30				
+0 mins.	0	123	0	0	123	0	0	17	0	17	2	47	0	0	49	0	0	0	0	0
+5 mins.	0	123	0	0	123	0	0	16	0	16	6	39	0	0	45	0	0	0	0	0
+10 mins.	0	118	0	0	118	0	0	18	0	18	4	31	0	0	35	0	0	0	0	0
+15 mins.	0	119	0	0	119	0	0	25	0	25	6	29	0	0	35	0	0	0	0	0
+20 mins.	0	119	0	0	119	1	0	16	0	17	2	40	0	0	42	0	0	0	0	0
+25 mins.	0	111	0	0	111	0	0	27	0	27	5	46	0	0	51	0	0	0	0	0
+30 mins.	0	106	0	0	106	0	0	17	0	17	5	32	0	0	37	0	0	0	0	0
+35 mins.	0	103	0	1	104	0	0	24	0	24	7	56	0	0	63	0	0	0	0	0
+40 mins.	0	113	0	0	113	0	0	17	0	17	5	50	0	0	55	0	0	0	0	0
+45 mins.	0	113	0	0	113	0	0	33	0	33	4	40	0	0	44	0	0	0	0	0
+50 mins.	0	129	0	0	129	2	0	12	0	14	5	44	0	0	49	0	0	0	0	0
+55 mins.	0	109	0	0	109	0	0	16	0	16	1	37	0	0	38	0	0	0	0	0
Total Volume	0	1386	0	1	1387	3	0	238	0	241	52	491	0	0	543	0	0	0	0	0
% App. Total	0	99.9	0	0.1		1.2	0	98.8	0		9.6	90.4	0	0		0	0	0	0	
PHF	.000	.895	.000	.083	.896	.125	.000	.601	.000	.609	.619	.731	.000	.000	.718	.000	.000	.000	.000	.000



719-633-2868

File Name: Hwy 24 - Falcon Hwy PM

Site Code: S224600 Start Date : 11/15/2022

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Groupe Printod- Unshifted

										Printe	d- Un	shifte	d								
		I	Hwy 2	24			Fa	Icon I	Hwy				Hwy 2	24							
		So	uthbo	und			W	estbo	und			No	rthbo	und			Ea	astbo	und		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
16:00	0	41	1	0	42	1	0	4	0	5	7	109	0	0	116	0	0	0	0	0	163
16:05	0	48	0	0	48	0	0	10	0	10	2	89	0	0	91	0	0	0	0	0	149
16:10	0	47	2	0	49	0	0	7	0	7	5	94	0	0	99	0	0	0	0	0	155
16:15	0	68	0	0	68	0	0	5	0	5	4	101	0	0	105	0	0	0	0	0	178
16:20	0	48	0	0	48	1	0	3	0	4	8	111	0	0	119	0	0	0	0	0	171
16:25	0	70	0	0	70	2	0	4	0	6	6	103	0	0	109	0	0	0	0	0	185
16:30	0	48	0	0	48	1	0	6	0	7	7	96	0	0	103	0	0	0	0	0	158
16:35	0	62	0	0	62	0	0	2	0	2	4	114	0	0	118	0	0	0	0	0	182
16:40	0	38	1	0	39	0	0	5	0	5	2	92	0	0	94	0	0	0	0	0	138
16:45	0	53	0	0	53	1	0	2	0	3	2	113	0	0	115	0	0	0	0	0	171
16:50	0	63	1	0	64	0	0	4	0	4	3	100	0	0	103	0	0	0	0	0	171
16:55	0	43	0	0	43	0	0	5	0	5	4	109	0	0	113	0	0	0	0	0	161
Total	0	629	5	0	634	6	0	57	0	63	54	1231	0	0	1285	0	0	0	0	0	1982
17:00	0	52	0	0	52	0	0	5	0	5	4	111	0	0	115	0	0	0	0	0	172
17:05	0	45	1	0	46	0	0	5	0	5	4	105	0	0	109	0	0	0	0	0	160
17:10	0	39	0	0	39	0	0	5	0	5	8	105	0	0	113	0	0	0	0	0	157
17:15	0	40	0	0	40	0	0	6	0	6	3	103	0	0	106	0	0	0	0	0	152
17:20	0	57	0	0	57	1	0	5	0	6	3	103	0	0	106	0	0	0	0	0	169
17:25	0	53	1	0	54	0	0	5	0	5	4	101	0	0	105	0	0	0	0	0	164
17:30	0	47	0	0	47	1	0	3	0	4	6	104	0	0	110	0	0	0	0	0	161
17:35	0	48	0	0	48	0	0	3	0	3	8	110	0	0	118	0	0	0	0	0	169
17:40	0	38	0	0	38	0	0	2	0	2	8	88	0	0	96	0	0	0	0	0	136
17:45	0	37	0	0	37	0	0	2	0	2	3	86	0	0	89	0	0	0	0	0	128
17:50	0	39	0	0	39	0	0	5	0	5	4	78	0	0	82	0	0	0	0	0	126
17:55	0	35	0	0_	35	0	0	3	0_	3	2	63	0	0	65	0	0	0	0_	0	103
Total	0	530	2	0	532	2	0	49	0	51	57	1157	0	0	1214	0	0	0	0	0	1797
			_				_													_	
Grand Total	0	1159	7	0	1166	8	0	106	0	114	111	2388	0	0	2499	0	0	0	0	0	3779
Apprch %	0	99.4	0.6	0		7	0	93	0		4.4	95.6	0	0		0	0	0	0		
Total %	0	30.7	0.2	0	30.9	0.2	0	2.8	0	3	2.9	63.2	0	0	66.1	0	0	0	0	0	

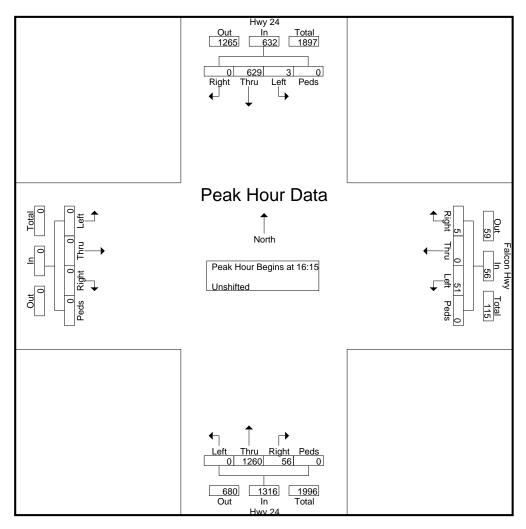
719-633-2868

File Name: Hwy 24 - Falcon Hwy PM

Site Code: S224600 Start Date : 11/15/2022

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			Hwy 2	24			Fa	lcon l	lwy				Hwy 2	24							
		So	uthbo	und			W	estbo	und			No	rthbo	und			Ea	stbo	und		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour A	Analys	is Fro	m 16:0	00 to 1	7:55 - F	Peak 1	of 1														
Peak Hour f	or Ent	ire Inte	ersecti	ion Be	gins at	16:15															
16:15	0	68	0	0	68	0	0	5	0	5	4	101	0	0	105	0	0	0	0	0	178
16:20	0	48	0	0	48	1	0	3	0	4	8	111	0	0	119	0	0	0	0	0	171
16:25	0	70	0	0	70	2	0	4	0	6	6	103	0	0	109	0	0	0	0	0	185
16:30	0	48	0	0	48	1	0	6	0	7	7	96	0	0	103	0	0	0	0	0	158
16:35	0	62	0	0	62	0	0	2	0	2	4	114	0	0	118	0	0	0	0	0	182
16:40	0	38	1	0	39	0	0	5	0	5	2	92	0	0	94	0	0	0	0	0	138
16:45	0	53	0	0	53	1	0	2	0	3	2	113	0	0	115	0	0	0	0	0	171
16:50	0	63	1	0	64	0	0	4	0	4	3	100	0	0	103	0	0	0	0	0	171
16:55	0	43	0	0	43	0	0	5	0	5	4	109	0	0	113	0	0	0	0	0	161
17:00	0	52	0	0	52	0	0	5	0	5	4	111	0	0	115	0	0	0	0	0	172
17:05	0	45	1	0	46	0	0	5	0	5	4	105	0	0	109	0	0	0	0	0	160
17:10	0	39	0	0	39	0	0	5	0	5	8	105	0	0	113	0	0	0	0	0	157
Total Volume	0	629	3	0	632	5	0	51	0	56	56	1260	0	0	1316	0	0	0	0	0	2004
% App. Total	0	99.5	0.5	0		8.9	0	91.1	0		4.3	95.7	0	0		0	0	0	0		
PHF	.000	.749	.250	.000	.752	.208	.000	.708	.000	.667	.583	.921	.000	.000	.922	.000	.000	.000	.000	.000	.903



719-633-2868

File Name: Hwy 24 - Falcon Hwy PM

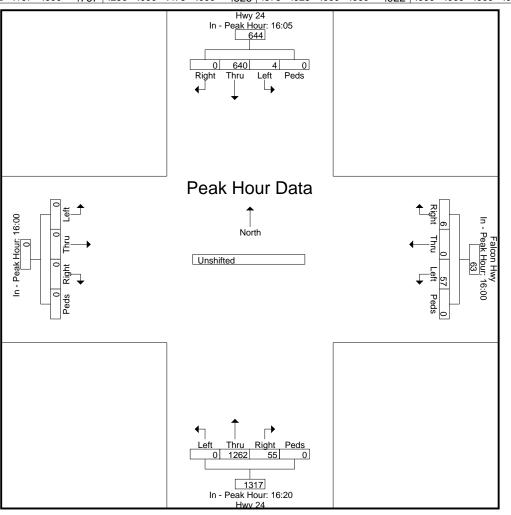
Site Code: S224600 Start Date : 11/15/2022

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		H	lwy 2	24				lcon l	•				Hwy 2	24							
		Sou	ıthbo	und			W	estbo	und			No	rthbo	und			Ea	astbo	und		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total

Peak Hour Analysis From 16:00 to 17:55 - Peak 1 of 1

Peak Hour for Each Approach Begins at:																				
	16:05	;				16:00					16:20					16:00				
+0 mins.	0	48	0	0	48	1	0	4	0	5	8	111	0	0	119	0	0	0	0	0
+5 mins.	0	47	2	0	49	0	0	10	0	10	6	103	0	0	109	0	0	0	0	0
+10 mins.	0	68	0	0	68	0	0	7	0	7	7	96	0	0	103	0	0	0	0	0
+15 mins.	0	48	0	0	48	0	0	5	0	5	4	114	0	0	118	0	0	0	0	0
+20 mins.	0	70	0	0	70	1	0	3	0	4	2	92	0	0	94	0	0	0	0	0
+25 mins.	0	48	0	0	48	2	0	4	0	6	2	113	0	0	115	0	0	0	0	0
+30 mins.	0	62	0	0	62	1	0	6	0	7	3	100	0	0	103	0	0	0	0	0
+35 mins.	0	38	1	0	39	0	0	2	0	2	4	109	0	0	113	0	0	0	0	0
+40 mins.	0	53	0	0	53	0	0	5	0	5	4	111	0	0	115	0	0	0	0	0
+45 mins.	0	63	1	0	64	1	0	2	0	3	4	105	0	0	109	0	0	0	0	0
+50 mins.	0	43	0	0	43	0	0	4	0	4	8	105	0	0	113	0	0	0	0	0
+55 mins.	0	52	0	0	52	0	0	5	0	5	3	103	0	0	106	0	0	0	0	0
Total Volume	0	640	4	0	644	6	0	57	0	63	55	1262	0	0	1317	0	0	0	0	0
% App. Total	0	99.4	0.6	0		9.5	0	90.5	0		4.2	95.8	0	0		0	0	0	0	
PHF	.000	.762	.167	.000	.767	.250	.000	.475	.000	.525	.573	.923	.000	.000	.922	.000	.000	.000	.000	.000



Levels of Service



	•	•	†	~	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		↑	7	*	↑
Traffic Volume (vph)	224	5	464	47	3	1237
Future Volume (vph)	224	5	464	47	3	1237
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	1000	490	775	1000
Storage Lanes	1	0		1	1	
		U		ı		
Taper Length (ft)	25	4.00	4.00	4.00	90	4.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.997			0.850	0.050	
Flt Protected	0.953		1===		0.950	16.5
Satd. Flow (prot)	1770	0	1759	1495	1736	1827
FIt Permitted	0.953				0.410	
Satd. Flow (perm)	1770	0	1759	1495	749	1827
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	1			51		
Link Speed (mph)	45		65			65
Link Distance (ft)	414		1195			991
Travel Time (s)	6.3		12.5			10.4
Peak Hour Factor	0.87	0.87	0.92	0.92	0.95	0.95
Heavy Vehicles (%)	2%	2%	8%	8%	4%	4%
. ,						
Adj. Flow (vph)	257	6	504	51	3	1302
Shared Lane Traffic (%)	000		E0.4	5.1		4000
Lane Group Flow (vph)	263	0	504	51	3	1302
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9	1.00	9	15	1.00
Number of Detectors	1	J	2	1	1	2
	Left			•	Left	Thru
Detector Template			Thru	Right		
Leading Detector (ft)	20		100	20	20	100
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
Detector 1 Size(ft)	20		6	20	20	6
Detector 1 Type	Cl+Ex		CI+Ex	Cl+Ex	Cl+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)	0.0		94	- 0.0	0.0	94
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
			CITEX			CITEX
Detector 2 Channel			0.0			0.0
Detector 2 Extend (s)	D (0.0			0.0
Turn Type	Prot		NA	Perm	pm+pt	NA
Protected Phases	6		8		7	4

Existing AM Lanes, Volumes, Timings

Synchro 11 Report JAB

	•	•	1	1	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Permitted Phases				8	4	
Detector Phase	6		8	8	7	4
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	12.0		41.0	41.0	9.5	41.0
Total Split (s)	30.0		95.0	95.0	15.0	110.0
Total Split (%)	21.4%		67.9%	67.9%	10.7%	78.6%
Maximum Green (s)	26.0		89.0	89.0	10.5	104.0
Yellow Time (s)	3.0		4.0	4.0	3.5	4.0
All-Red Time (s)	1.0		2.0	2.0	1.0	2.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0		6.0	6.0	4.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Max		C-Max	C-Max	None	C-Max
Act Effct Green (s)	26.0		102.0	102.0	105.5	104.0
Actuated g/C Ratio	0.19		0.73	0.73	0.75	0.74
v/c Ratio	0.80		0.39	0.05	0.00	0.96
Control Delay	72.9		8.8	1.9	4.3	33.6
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	72.9		8.8	1.9	4.3	33.6
LOS	Е		Α	Α	Α	С
Approach Delay	72.9		8.1			33.5
Approach LOS	Е		Α			С
Queue Length 50th (ft)	231		147	0	1	944
Queue Length 95th (ft)	#342		269	14	3	#1444
Internal Link Dist (ft)	334		1115			911
Turn Bay Length (ft)				490	775	
Base Capacity (vph)	329		1280	1102	638	1357
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.80		0.39	0.05	0.00	0.96

Intersection Summary

Area Type: Other

Cycle Length: 140
Actuated Cycle Length: 140

Offset: 45 (32%), Referenced to phase 4:SBTL and 8:NBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.96 Intersection Signal Delay: 31.8 Intersection Capacity Utilization 86.2%

Intersection LOS: C
ICU Level of Service E

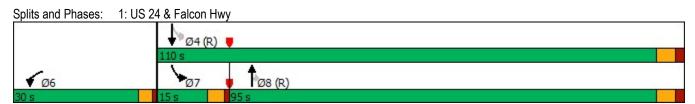
Analysis Period (min) 15

95th percentile volume exceeds capacity, queue may be longer.

Queue shown is maximum after two cycles.

Existing AM
Lanes, Volumes, Timings

Synchro 11 Report
JAB



Lane Group Lane Configurations Traffic Volume (vph) Future Volume (vph) Ideal Flow (vphpl) Storage Length (ft) Storage Lanes Taper Length (ft) Lane Util. Factor Frt Flt Protected Satd. Flow (prot) Flt Permitted Satd. Flow (perm) Right Turn on Red	WBL 55 55 1900 0 1 25 1.00 0.992 0.955 1765 0.955 1765	3 3 1900 0 0 1.00	NBT 1252 1252 1900 1.00	NBR 48 48 1900 490 1 1.00 0.850	SBL 4 4 1900 775 1 90 1.00	\$BT 593 593 1900
Lane Configurations Traffic Volume (vph) Future Volume (vph) Ideal Flow (vphpl) Storage Length (ft) Storage Lanes Taper Length (ft) Lane Util. Factor Frt Flt Protected Satd. Flow (prot) Flt Permitted Satd. Flow (perm)	55 55 1900 0 1 25 1.00 0.992 0.955 1765 0.955	3 3 1900 0 0 1.00	1252 1252 1900	48 48 1900 490 1	4 4 1900 775 1 90	593 593 1900
Traffic Volume (vph) Future Volume (vph) Ideal Flow (vphpl) Storage Length (ft) Storage Lanes Taper Length (ft) Lane Util. Factor Frt Fit Protected Satd. Flow (prot) Fit Permitted Satd. Flow (perm)	55 55 1900 0 1 25 1.00 0.992 0.955 1765 0.955 1765	3 1900 0 0 1.00	1252 1252 1900	48 48 1900 490 1	4 4 1900 775 1 90	593 593 1900
Future Volume (vph) Ideal Flow (vphpl) Storage Length (ft) Storage Lanes Taper Length (ft) Lane Util. Factor Frt Flt Protected Satd. Flow (prot) Flt Permitted Satd. Flow (perm)	55 1900 0 1 25 1.00 0.992 0.955 1765 0.955 1765	3 1900 0 0 1.00	1252 1900 1.00	48 1900 490 1	4 1900 775 1 90	593 1900
Ideal Flow (vphpl) Storage Length (ft) Storage Lanes Taper Length (ft) Lane Util. Factor Frt Fit Protected Satd. Flow (prot) Fit Permitted Satd. Flow (perm)	1900 0 1 25 1.00 0.992 0.955 1765 0.955 1765	1900 0 0 1.00	1.00	1900 490 1	1900 775 1 90	1900
Storage Length (ft) Storage Lanes Taper Length (ft) Lane Util. Factor Frt Fit Protected Satd. Flow (prot) Fit Permitted Satd. Flow (perm)	0 1 25 1.00 0.992 0.955 1765 0.955 1765	0 0 1.00	1.00	490 1 1.00	775 1 90	
Storage Lanes Taper Length (ft) Lane Util. Factor Frt Flt Protected Satd. Flow (prot) Flt Permitted Satd. Flow (perm)	1 25 1.00 0.992 0.955 1765 0.955 1765	0 1.00 0		1.00	1 90	1.00
Taper Length (ft) Lane Util. Factor Frt Flt Protected Satd. Flow (prot) Flt Permitted Satd. Flow (perm)	25 1.00 0.992 0.955 1765 0.955 1765	0		1.00	90	1.00
Lane Util. Factor Frt Flt Protected Satd. Flow (prot) Flt Permitted Satd. Flow (perm)	1.00 0.992 0.955 1765 0.955 1765	0				1.00
Frt Flt Protected Satd. Flow (prot) Flt Permitted Satd. Flow (perm)	0.992 0.955 1765 0.955 1765	0			1.00	
Fit Protected Satd. Flow (prot) Fit Permitted Satd. Flow (perm)	0.955 1765 0.955 1765	0	1759	0.850		1.00
Satd. Flow (prot) Flt Permitted Satd. Flow (perm)	1765 0.955 1765	0	1759			
Flt Permitted Satd. Flow (perm)	0.955 1765 2	0	1759		0.950	
Satd. Flow (perm)	1765 2			1495	1736	1827
	2				0.039	
	2		1759	1495	71	1827
. again control of the		Yes		Yes		
Satd. Flow (RTOR)				51		
Link Speed (mph)	45		65	J .		65
Link Distance (ft)	414		1195			991
Travel Time (s)	6.3		12.5			10.4
Peak Hour Factor	0.83	0.00	0.95	0.05	0.93	0.93
		0.83		0.95		
Heavy Vehicles (%)	2%	2%	8%	8%	4%	4%
Adj. Flow (vph)	66	4	1318	51	4	638
Shared Lane Traffic (%)						
Lane Group Flow (vph)	70	0	1318	51	4	638
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane	10		10			10
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	9	1.00	1.00
	15	9	0			0
Number of Detectors	•		2	1	1	2
Detector Template	Left		Thru	Right	Left	Thru
Leading Detector (ft)	20		100	20	20	100
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
Detector 1 Size(ft)	20		6	20	20	6
Detector 1 Type	Cl+Ex		CI+Ex	CI+Ex	CI+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)	0.0		94	0.0	0.0	94
Detector 2 Size(ft)			6			6
Detector 2 Type			CI+Ex			CI+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Turn Type	Prot		NA	Perm	pm+pt	NA
Protected Phases	6		8		7	4

2022 Existing PM Lanes, Volumes, Timings

	1		T		-	+	
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT	
Permitted Phases				8	4		
Detector Phase	6		8	8	7	4	
Switch Phase							
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0	
Minimum Split (s)	12.0		41.0	41.0	9.5	41.0	
Total Split (s)	30.0		100.0	100.0	10.0	110.0	
Total Split (%)	21.4%		71.4%	71.4%	7.1%	78.6%	
Maximum Green (s)	26.0		94.0	94.0	5.5	104.0	
Yellow Time (s)	3.0		4.0	4.0	3.5	4.0	
All-Red Time (s)	1.0		2.0	2.0	1.0	2.0	
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0	
Total Lost Time (s)	4.0		6.0	6.0	4.5	6.0	
Lead/Lag			Lag	Lag	Lead		
Lead-Lag Optimize?			Yes	Yes	Yes		
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0	
Recall Mode	Max		C-Max	C-Max	None	C-Max	
Act Effct Green (s)	26.0		102.0	102.0	105.5	104.0	
Actuated g/C Ratio	0.19		0.73	0.73	0.75	0.74	
v/c Ratio	0.21		1.03	0.05	0.03	0.47	
Control Delay	48.9		52.8	1.8	4.8	8.5	
Queue Delay	0.0		0.0	0.0	0.0	0.0	
Total Delay	48.9		52.8	1.8	4.8	8.5	
LOS	D		D	А	Α	Α	
Approach Delay	48.9		50.9			8.4	
Approach LOS	D		D			Α	
Queue Length 50th (ft)	53		~1137	0	1	205	
Queue Length 95th (ft)	91		#1641	14	4	274	
Internal Link Dist (ft)	334		1115		•	911	
Turn Bay Length (ft)				490	775		
Base Capacity (vph)	329		1281	1102	118	1357	
Starvation Cap Reductn	0		0	0	0	0	
Spillback Cap Reductn	0		0	0	0	0	
Storage Cap Reductn	0		0	0	0	0	
Reduced v/c Ratio	0.21		1.03	0.05	0.03	0.47	
	V. <u>~</u> .			2.00	5.00	J	

Area Type: Other

Cycle Length: 140
Actuated Cycle Length: 140

Offset: 45 (32%), Referenced to phase 4:SBTL and 8:NBT, Start of Green

Natural Cycle: 110

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.03
Intersection Signal Delay: 37.7
Intersection Capacity Utilization 78.4%
ICU Level of Service D

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

2022 Existing PM
Lanes, Volumes, Timings
Synchro 11 Report
JAB

	•	•	†	-	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		†	7	*	†
Traffic Volume (vph)	223	6	464	45	4	1327
Future Volume (vph)	223	6	464	45	4	1327
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
,			1300	490	775	1300
Storage Length (ft)	0	0				
Storage Lanes	1	0		1	1	
Taper Length (ft)	25	4.60	4.00		90	4.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.996			0.850		
Flt Protected	0.954				0.950	
Satd. Flow (prot)	1770	0	1759	1495	1736	1827
Flt Permitted	0.954				0.410	
Satd. Flow (perm)	1770	0	1759	1495	749	1827
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	1			49		
Link Speed (mph)	45		65			65
Link Distance (ft)	414		1195			991
Travel Time (s)	6.3		12.5			10.4
Peak Hour Factor	0.87	0.07	0.92	0.00	0.95	0.95
		0.87		0.92		
Heavy Vehicles (%)	2%	2%	8%	8%	4%	4%
Adj. Flow (vph)	256	7	504	49	4	1397
Shared Lane Traffic (%)						
Lane Group Flow (vph)	263	0	504	49	4	1397
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	9	1.00	1.00
Number of Detectors	13	3	2	1	1	2
	•			•	· ·	
Detector Template	Left		Thru	Right	Left	Thru
Leading Detector (ft)	20		100	20	20	100
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
Detector 1 Size(ft)	20		6	20	20	6
Detector 1 Type	CI+Ex		CI+Ex	CI+Ex	Cl+Ex	CI+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)	0.0		94	0.0	0.0	94
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
			OI+EX			OI+EX
Detector 2 Channel			0.0			0.0
Detector 2 Extend (s)			0.0			0.0
Turn Type	Prot		NA	Perm	pm+pt	NA
Protected Phases	6		8		7	4

	1		T		-	+
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Permitted Phases				8	4	
Detector Phase	6		8	8	7	4
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	12.0		41.0	41.0	9.5	41.0
Total Split (s)	30.0		95.0	95.0	15.0	110.0
Total Split (%)	21.4%		67.9%	67.9%	10.7%	78.6%
Maximum Green (s)	26.0		89.0	89.0	10.5	104.0
Yellow Time (s)	3.0		4.0	4.0	3.5	4.0
All-Red Time (s)	1.0		2.0	2.0	1.0	2.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0		6.0	6.0	4.5	6.0
Lead/Lag			Lag	Lag	Lead	3.0
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Max		C-Max	C-Max	None	C-Max
Act Effct Green (s)	26.0		101.9	101.9	105.5	104.0
Actuated g/C Ratio	0.19		0.73	0.73	0.75	0.74
v/c Ratio	0.13		0.73	0.73	0.73	1.03
Control Delay	72.9		8.8	1.9	4.2	51.2
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	72.9		8.8	1.9	4.2	51.2
LOS	12.5 E		Α.	Α	Α.Δ	D D
Approach Delay	72.9		8.2	^		51.1
Approach LOS	12.9 E		0.2 A			51.1 D
Queue Length 50th (ft)	231		147	0	1	~1358
• ,			270	14		*1624
Queue Length 95th (ft)	#342			14	4	
Internal Link Dist (ft)	334		1115	400	775	911
Turn Bay Length (ft)	000		4000	490	775	4057
Base Capacity (vph)	329		1280	1102	638	1357
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.80		0.39	0.04	0.01	1.03

Area Type: Other

Cycle Length: 140
Actuated Cycle Length: 140

Offset: 45 (32%), Referenced to phase 4:SBTL and 8:NBT, Start of Green

Natural Cycle: 90

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.03

Intersection Signal Delay: 43.0 Intersection LOS: D
Intersection Capacity Utilization 90.9% ICU Level of Service E

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

Splits and Phases: 1: US 24 & Falcon Hwy

Ø4 (R)

110 s

Ø5 (R)

Intersection						
Int Delay, s/veh	0.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	¥	
Traffic Vol, veh/h	47	2	1	227	2	1
Future Vol, veh/h	47	2	1	227	2	1
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-		_	None
Storage Length	-	-	-	-	0	-
Veh in Median Storag	e,# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	_
Peak Hour Factor	78	78	87	87	78	78
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	60	3	1	261	3	1
WWW.CT IOW	00		•	201	J	•
		_		_		
Major/Minor	Major1		Major2		Minor1	
Conflicting Flow All	0	0	63	0	325	62
Stage 1	-	-	-	-	62	-
Stage 2	-	-	-	-	263	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-	3.518	3.318
Pot Cap-1 Maneuver	-	-	1540	-	669	1003
Stage 1	-	-	-	-	961	-
Stage 2	-	-	-	-	781	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	_	-	1540	-	668	1003
Mov Cap-2 Maneuver		-	_	-	668	-
Stage 1	_	_	_	_	961	_
Stage 2	_	_	_	_	780	_
olago 2					100	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		9.8	
HCM LOS					Α	
Minor Lane/Major Mvr	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	1	752	-		1540	-
HCM Lane V/C Ratio		0.005	-		0.001	<u>-</u>
HCM Control Delay (s	1	9.8	_	_	7.3	0
HCM Lane LOS	7	9.0 A	_	_	7.5 A	A
HCM 95th %tile Q(vel	n)	0	_	_	0	-
	'/	U			U	

	•	•	†	~	-	ļ
Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		†	7	7	↑
Traffic Volume (vph)	55	6	1252	49	7	593
Future Volume (vph)	55	6	1252	49	7	593
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	1300	490	775	1300
Storage Lanes	1	0		1	1	
		U		l I		
Taper Length (ft)	25	1.00	1.00	1.00	90	1.00
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00
Frt	0.987			0.850		
FIt Protected	0.957				0.950	
Satd. Flow (prot)	1759	0	1759	1495	1736	1827
FIt Permitted	0.957				0.039	
Satd. Flow (perm)	1759	0	1759	1495	71	1827
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	3			52		
Link Speed (mph)	45		65			65
Link Distance (ft)	414		1195			991
Travel Time (s)	6.3		12.5			10.4
Peak Hour Factor	0.83	0.83	0.95	0.95	0.93	0.93
Heavy Vehicles (%)	2%	2%	8%	8%	4%	4%
Adj. Flow (vph)	66	7	1318	52	8	638
Shared Lane Traffic (%)	00	1	1010	JZ	U	000
Lane Group Flow (vph)	73	0	1318	52	8	638
,						
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15	9		9	15	
Number of Detectors	1		2	1	1	2
Detector Template	Left		Thru	Right	Left	Thru
Leading Detector (ft)	20		100	20	20	100
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
	20		6	20	20	6
Detector 1 Size(ft)						
Detector 1 Type	CI+Ex		CI+Ex	CI+Ex	CI+Ex	Cl+Ex
Detector 1 Channel	^ ^		0.0	0.0		2.2
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)			94			94
Detector 2 Size(ft)			6			6
Detector 2 Type			CI+Ex			CI+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Turn Type	Prot		NA	Perm	pm+pt	NA
Protected Phases	6		8	. 0.111	7	4
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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Permitted Phases				8	4	
Detector Phase	6		8	8	7	4
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	12.0		41.0	41.0	9.5	41.0
Total Split (s)	30.0		100.0	100.0	10.0	110.0
Total Split (%)	21.4%		71.4%	71.4%	7.1%	78.6%
Maximum Green (s)	26.0		94.0	94.0	5.5	104.0
Yellow Time (s)	3.0		4.0	4.0	3.5	4.0
All-Red Time (s)	1.0		2.0	2.0	1.0	2.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0		6.0	6.0	4.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Max		C-Max	C-Max	None	C-Max
Act Effct Green (s)	26.0		102.0	102.0	105.5	104.0
Actuated g/C Ratio	0.19		0.73	0.73	0.75	0.74
v/c Ratio	0.22		1.03	0.05	0.07	0.47
Control Delay	48.5		52.8	1.8	5.4	8.5
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	48.5		52.8	1.8	5.4	8.5
LOS	D		D	Α	Α	Α
Approach Delay	48.5		50.8			8.4
Approach LOS	D		D			Α
Queue Length 50th (ft)	55		~1137	0	2	205
Queue Length 95th (ft)	94		#1641	14	6	274
Internal Link Dist (ft)	334		1115			911
Turn Bay Length (ft)	001		1110	490	775	U
Base Capacity (vph)	329		1281	1102	118	1357
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	0.22		1.03	0.05	0.07	0.47

Area Type: Other

Cycle Length: 140
Actuated Cycle Length: 140

Offset: 45 (32%), Referenced to phase 4:SBTL and 8:NBT, Start of Green

Natural Cycle: 110

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.03 Intersection Signal Delay: 37.6 Intersection Capacity Utilization 78.4%

Intersection LOS: D
ICU Level of Service D

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

95th percentile volume exceeds capacity, queue may be longer.

Splits and Phases: 1: US 24 & Falcon Hwy



Intersection						
Int Delay, s/veh	0.4					
	EBT	EBR	///DI	WDT	NBL	NBR
		EDK	WBL	WBT		INDK
Lane Configurations	1	2	1	4	Y	1
Traffic Vol, veh/h	52 52	3	1	58	3	1
Future Vol, veh/h		3	1	58	3	1
Conflicting Peds, #/hr	0	0	0	0	0	0
3	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #		-	-	0	0	-
Grade, %	0	-	-	0	0	- 70
Peak Hour Factor	83	83	83	83	78	78
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	63	4	1	70	4	1
Major/Minor Ma	ajor1	N	Major2		Minor1	
Conflicting Flow All	0	0	67	0	137	65
Stage 1	-	-	-	-	65	_
Stage 2	-	-	-	_	72	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	_	5.42	-
Critical Hdwy Stg 2	-	_	_	_	5.42	_
Follow-up Hdwy	_	_	2.218	_		3.318
Pot Cap-1 Maneuver	-	_	1535	_	856	999
Stage 1	_	_	-	_	958	-
Stage 2	-	-	-	-	951	_
Platoon blocked, %	_	_		_	001	
Mov Cap-1 Maneuver	_	_	1535	_	855	999
Mov Cap-2 Maneuver	_	_	-	_	855	-
Stage 1	_	_	_	_	958	_
Stage 2			_	_	950	_
Stage 2	_		-	-	900	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.1		9.1	
HCM LOS					Α	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	<u> </u>	887	-		1535	-
HCM Lane V/C Ratio		0.006	_		0.001	-
HCM Control Delay (s)		9.1	_	_	7.3	0
HCM Lane LOS		9.1 A	-	<u> </u>	7.3 A	A
HCM 95th %tile Q(veh)		0	-	-	0	- -
HOW SOUT /OUIE Q(VEII)		U	_		U	

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		^ ^	7	*	^
Traffic Volume (vph)	425	40	950	275	25	2600
Future Volume (vph)	425	40	950	275	25	2600
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	1300	490	775	1300
	1			490	113	
Storage Lanes		0				
Taper Length (ft)	25	4.00	0.04	4.00	90	0.04
Lane Util. Factor	1.00	1.00	0.91	1.00	1.00	0.91
Frt	0.989			0.850		
FIt Protected	0.956				0.950	
Satd. Flow (prot)	1761	0	4803	1495	1736	4988
FIt Permitted	0.956				0.247	
Satd. Flow (perm)	1761	0	4803	1495	451	4988
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	3			289		
Link Speed (mph)	45		65	_50		65
Link Distance (ft)	414		1195			991
Travel Time (s)	6.3		12.5			10.4
Peak Hour Factor	0.92	0.02	0.95	0.05	0.95	0.95
		0.92		0.95		
Heavy Vehicles (%)	2%	2%	8%	8%	4%	4%
Adj. Flow (vph)	462	43	1000	289	26	2737
Shared Lane Traffic (%)						
Lane Group Flow (vph)	505	0	1000	289	26	2737
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	9	1.00	1.00
Number of Detectors	10	3	2	1	10	2
	•				· ·	
Detector Template	Left		Thru	Right	Left	Thru
Leading Detector (ft)	20		100	20	20	100
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
Detector 1 Size(ft)	20		6	20	20	6
Detector 1 Type	Cl+Ex		CI+Ex	CI+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)	0.0		94	0.0	0.0	94
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			CI+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Turn Type	Prot		NA	Perm	pm+pt	NA
Protected Phases	6		8		7	4

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Permitted Phases				8	4	
Detector Phase	6		8	8	7	4
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	12.0		41.0	41.0	9.5	41.0
Total Split (s)	30.0		95.0	95.0	15.0	110.0
Total Split (%)	21.4%		67.9%	67.9%	10.7%	78.6%
Maximum Green (s)	26.0		89.0	89.0	10.5	104.0
Yellow Time (s)	3.0		4.0	4.0	3.5	4.0
All-Red Time (s)	1.0		2.0	2.0	1.0	2.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0		6.0	6.0	4.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Max		C-Max	C-Max	None	C-Max
Act Effct Green (s)	26.0		97.5	97.5	105.5	104.0
Actuated g/C Ratio	0.19		0.70	0.70	0.75	0.74
v/c Ratio	1.53		0.30	0.26	0.07	0.74
Control Delay	293.1		8.9	1.5	4.7	11.8
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	293.1		8.9	1.5	4.7	11.8
LOS	F		Α	A	Α	В
Approach Delay	293.1		7.2			11.7
Approach LOS	F		Α			В
Queue Length 50th (ft)	~645		130	0	5	463
Queue Length 95th (ft)	#872		156	30	13	510
Internal Link Dist (ft)	334		1115			911
Turn Bay Length (ft)				490	775	
Base Capacity (vph)	329		3344	1128	436	3705
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	1.53		0.30	0.26	0.06	0.74
					2.00	

Area Type: Other

Cycle Length: 140
Actuated Cycle Length: 140

Offset: 45 (32%), Referenced to phase 4:SBTL and 8:NBT, Start of Green

Natural Cycle: 75

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.53 Intersection Signal Delay: 41.6

Intersection LOS: D
ICU Level of Service E

Intersection Capacity Utilization 84.5%

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.
 Queue shown is maximum after two cycles.

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		^	7	*	^
Traffic Volume (vph)	400	75	2400	350	30	1300
Future Volume (vph)	400	75	2400	350	30	1300
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
,			1900	490		1900
Storage Length (ft)	0	0			775	
Storage Lanes	1	0		1	1	
Taper Length (ft)	25	4.00	0.04	4.00	90	0.04
Lane Util. Factor	1.00	1.00	0.91	1.00	1.00	0.91
Frt	0.979			0.850		
Flt Protected	0.960				0.950	
Satd. Flow (prot)	1751	0	4803	1495	1736	4988
FIt Permitted	0.960				0.040	
Satd. Flow (perm)	1751	0	4803	1495	73	4988
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	6			368		
Link Speed (mph)	45		65	330		65
Link Distance (ft)	414		1195			991
()	6.3		12.5			10.4
Travel Time (s)		0.00		0.05	0.05	
Peak Hour Factor	0.92	0.92	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	2%	8%	8%	4%	4%
Adj. Flow (vph)	435	82	2526	368	32	1368
Shared Lane Traffic (%)						
Lane Group Flow (vph)	517	0	2526	368	32	1368
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane	10					10
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	9	1.00	1.00
Number of Detectors	15	9	0	1		0
	•		2		1	2
Detector Template	Left		Thru	Right	Left	Thru
Leading Detector (ft)	20		100	20	20	100
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
Detector 1 Size(ft)	20		6	20	20	6
Detector 1 Type	Cl+Ex		Cl+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)	0.0		94	0.0	0.0	94
						6
Detector 2 Size(ft)			6			
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Turn Type	Prot		NA	Perm	pm+pt	NA
Protected Phases	6		8		7	4

Lane Group WBL WBR NBT NBR SBL SBT		•	•	†	1	-	ļ
Detector Phase 6	Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Switch Phase Minimum Initial (s) 5.0 5.0 5.0 5.0 5.0 Minimum Split (s) 12.0 41.0 41.0 9.5 41.0 Total Split (s) 30.0 100.0 100.0 10.0 110.0 Total Split (%) 21.4% 71.4% 71.4% 7.1% 78.6% Maximum Green (s) 26.0 94.0 94.0 5.5 104.0 Yellow Time (s) 3.0 4.0 4.0 3.5 4.0 All-Red Time (s) 1.0 2.0 2.0 1.0 2.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 4.0 6.0 6.0 4.5 6.0 Lead/Lag Lag Lag Lag Lead Lead-Lag Optimize? Yes Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None <td>Permitted Phases</td> <td></td> <td></td> <td></td> <td>8</td> <td>4</td> <td></td>	Permitted Phases				8	4	
Minimum Initial (s) 5.0 5.0 5.0 5.0 Minimum Split (s) 12.0 41.0 41.0 9.5 41.0 Total Split (%) 30.0 100.0 100.0 10.0 110.0 Total Split (%) 21.4% 71.4% 71.4% 7.1% 78.6% Maximum Green (s) 26.0 94.0 94.0 5.5 104.0 Yellow Time (s) 3.0 4.0 4.0 3.5 4.0 All-Red Time (s) 1.0 2.0 2.0 1.0 2.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 4.0 6.0 6.0 4.5 6.0 Lead/Lag Lag Lag Lead Lead Lead-Lag Optimize? Yes Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max	Detector Phase	6		8	8	7	4
Minimum Split (s) 12.0 41.0 41.0 9.5 41.0 Total Split (s) 30.0 100.0 100.0 10.0 110.0 Total Split (%) 21.4% 71.4% 71.4% 71.4% 72.4% 73.2% 73.0% 73.0 73.0 73.0 73.0 73.0 73.0 73.0 73.0 73.0 73.0	Switch Phase						
Total Split (s) 30.0 100.0 100.0 10.0 110.0 Total Split (%) 21.4% 71.4% 71.4% 71.4% 7.1% 78.6% Maximum Green (s) 26.0 94.0 94.0 5.5 104.0 Yellow Time (s) 3.0 4.0 4.0 3.5 4.0 All-Red Time (s) 1.0 2.0 2.0 1.0 2.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 4.0 6.0 6.0 4.5 6.0 Lead/Lag Lag Lag Lead Lead-Lag Optimize? Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Act Effct Green (s) 26.0 98.0 98.0 10.5 104.0 Act Effct Green (s) 26.0 98.0 98.0 10.5 10.0	Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Total Split (%) 21.4% 71.4% 71.4% 7.1% 78.6% Maximum Green (s) 26.0 94.0 94.0 5.5 104.0 Yellow Time (s) 3.0 4.0 4.0 3.5 4.0 All-Red Time (s) 1.0 2.0 2.0 1.0 2.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 4.0 6.0 6.0 4.5 6.0 Lead/Lag Lag Lag Lead Lead Lead-Lag Optimize? Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0	Minimum Split (s)	12.0		41.0	41.0	9.5	41.0
Total Split (%) 21.4% 71.4% 71.4% 7.1% 78.6% Maximum Green (s) 26.0 94.0 94.0 5.5 104.0 Yellow Time (s) 3.0 4.0 4.0 3.5 4.0 All-Red Time (s) 1.0 2.0 2.0 1.0 2.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 4.0 6.0 6.0 4.5 6.0 Lead/Lag Lag Lag Lead Lead Lead-Lag Optimize? Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Act Effct Green (s) 26.0 98.0 98.0 10.5 104.0 Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0	Total Split (s)	30.0		100.0	100.0	10.0	110.0
Maximum Green (s) 26.0 94.0 94.0 5.5 104.0 Yellow Time (s) 3.0 4.0 4.0 3.5 4.0 All-Red Time (s) 1.0 2.0 2.0 1.0 2.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 4.0 6.0 6.0 4.5 6.0 Lead/Lag Lag Lag Lead Lead Lead-Lag Optimize? Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Recall Mode Max C-Max C-Max None C-Max Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 <t< td=""><td></td><td>21.4%</td><td></td><td>71.4%</td><td>71.4%</td><td></td><td>78.6%</td></t<>		21.4%		71.4%	71.4%		78.6%
Yellow Time (s) 3.0 4.0 4.0 3.5 4.0 All-Red Time (s) 1.0 2.0 2.0 1.0 2.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 4.0 6.0 6.0 4.5 6.0 Lead/Lag Lag Lag Lead Lead Lead-Lag Optimize? Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Act Effet Green (s) 26.0 98.0 98.0 105.5 104.0 <td< td=""><td> ,</td><td></td><td></td><td>94.0</td><td>94.0</td><td>5.5</td><td>104.0</td></td<>	,			94.0	94.0	5.5	104.0
All-Red Time (s) 1.0 2.0 2.0 1.0 2.0 Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 4.0 6.0 6.0 4.5 6.0 Lead/Lag Lag Lag Lead Lead Lead-Lag Optimize? Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Actuated g/C Ratio 0.19 0.70 0.70 0.75 0.74 v/c Ratio 1.57 0.75 0.32 0.27 0.37 Control Delay 305.9 15.8 1.5 9.7 6.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS	. ,	3.0		4.0	4.0	3.5	4.0
Lost Time Adjust (s) 0.0 0.0 0.0 0.0 0.0 Total Lost Time (s) 4.0 6.0 6.0 4.5 6.0 Lead/Lag Lag Lag Lead Lead-Lag Optimize? Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Actuated g/C Ratio 0.19 0.70 0.70 0.75 0.74 v/c Ratio 1.57 0.75 0.32 0.27 0.37 Control Delay 305.9 15.8 1.5 9.7 6.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A A Approach LOS F				2.0		1.0	
Total Lost Time (s) 4.0 6.0 6.0 4.5 6.0 Lead/Lag Lag Lag Lead Lead-Lag Optimize? Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Actuated g/C Ratio 0.19 0.70 0.70 0.75 0.74 v/c Ratio 1.57 0.75 0.32 0.27 0.37 Control Delay 305.9 15.8 1.5 9.7 6.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A Approach Delay 305.9 14.0 6.8 Approach LOS F B A Queue Length 95th		0.0		0.0	0.0	0.0	0.0
Lead/Lag Lag Lag Lead Lead-Lag Optimize? Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Actuated g/C Ratio 0.19 0.70 0.70 0.75 0.74 v/c Ratio 1.57 0.75 0.32 0.27 0.37 Control Delay 305.9 15.8 1.5 9.7 6.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A A Approach Delay 305.9 14.0 6.8 A Approach LOS F B A A Queue Length 50th (ft) ~665 542 0 <t< td=""><td></td><td></td><td></td><td></td><td></td><td>4.5</td><td></td></t<>						4.5	
Lead-Lag Optimize? Yes Yes Yes Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Actuated g/C Ratio 0.19 0.70 0.70 0.75 0.74 v/c Ratio 1.57 0.75 0.32 0.27 0.37 Control Delay 305.9 15.8 1.5 9.7 6.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A A Approach Delay 305.9 14.0 6.8 A Approach LOS F B A A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 <td>. ,</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	. ,						
Vehicle Extension (s) 3.0 3.0 3.0 3.0 3.0 Recall Mode Max C-Max C-Max None C-Max Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Actuated g/C Ratio 0.19 0.70 0.70 0.75 0.74 v/c Ratio 1.57 0.75 0.32 0.27 0.37 Control Delay 305.9 15.8 1.5 9.7 6.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A A Approach Delay 305.9 14.0 6.8 A Approach LOS F B A A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft)<	· ·			-	-	Yes	
Recall Mode Max C-Max C-Max None C-Max Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Actuated g/C Ratio 0.19 0.70 0.70 0.75 0.74 v/c Ratio 1.57 0.75 0.32 0.27 0.37 Control Delay 305.9 15.8 1.5 9.7 6.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A A Approach Delay 305.9 14.0 6.8 A Approach LOS F B A A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 Turn Bay Length (ft) 490 775<		3.0					3.0
Act Effct Green (s) 26.0 98.0 98.0 105.5 104.0 Actuated g/C Ratio 0.19 0.70 0.70 0.75 0.74 v/c Ratio 1.57 0.75 0.32 0.27 0.37 Control Delay 305.9 15.8 1.5 9.7 6.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A A Approach Delay 305.9 14.0 6.8 A Approach LOS F B A A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 Turn Bay Length (ft) 490 775 Base Capacity (vph) 330 3361 1156 120 3							
Actuated g/C Ratio 0.19 0.70 0.70 0.75 0.74 v/c Ratio 1.57 0.75 0.32 0.27 0.37 Control Delay 305.9 15.8 1.5 9.7 6.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A A Approach Delay 305.9 14.0 6.8 A Approach LOS F B A A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 Turn Bay Length (ft) 490 775 Base Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0	Act Effct Green (s)					105.5	104.0
v/c Ratio 1.57 0.75 0.32 0.27 0.37 Control Delay 305.9 15.8 1.5 9.7 6.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A A Approach Delay 305.9 14.0 6.8 A Approach LOS F B A A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 911 Turn Bay Length (ft) 490 775 8ase Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0 </td <td>` ,</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	` ,						
Control Delay 305.9 15.8 1.5 9.7 6.7 Queue Delay 0.0 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A A Approach Delay 305.9 14.0 6.8 Approach LOS F B A A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 Turn Bay Length (ft) 490 775 Base Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0							
Queue Delay 0.0 0.0 0.0 0.0 0.0 Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A A Approach Delay 305.9 14.0 6.8 Approach LOS F B A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 Turn Bay Length (ft) 490 775 Base Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0							
Total Delay 305.9 15.8 1.5 9.7 6.7 LOS F B A A A Approach Delay 305.9 14.0 6.8 Approach LOS F B A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 Turn Bay Length (ft) 490 775 Base Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0	,					0.0	0.0
LOS F B A A A Approach Delay 305.9 14.0 6.8 Approach LOS F B A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 Turn Bay Length (ft) 490 775 Base Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0	•						
Approach Delay 305.9 14.0 6.8 Approach LOS F B A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 Turn Bay Length (ft) 490 775 Base Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0							
Approach LOS F B A Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 Turn Bay Length (ft) 490 775 Base Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0							
Queue Length 50th (ft) ~665 542 0 6 144 Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 Turn Bay Length (ft) 490 775 Base Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0							
Queue Length 95th (ft) #894 602 32 15 165 Internal Link Dist (ft) 334 1115 911 Turn Bay Length (ft) 490 775 Base Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0		· · · · · · · · · · · · · · · · · · ·			0	6	
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Turn Bay Length (ft) 490 775 Base Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0					02	10	
Base Capacity (vph) 330 3361 1156 120 3705 Starvation Cap Reductn 0 0 0 0 0 Spillback Cap Reductn 0 0 0 0 0	. ,				490	775	U 11
Starvation Cap Reductn 0 0 0 0 Spillback Cap Reductn 0 0 0 0	, ,	330		3361			3705
Spillback Cap Reductn 0 0 0 0							
<u> </u>							
ctorage cap reduction	· · · · · · · · · · · · · · · · · · ·						
Reduced v/c Ratio 1.57 0.75 0.32 0.27 0.37	· ·						

Intersection Summary

Area Type: Other

Cycle Length: 140 Actuated Cycle Length: 140

Offset: 45 (32%), Referenced to phase 4:SBTL and 8:NBT, Start of Green

Natural Cycle: 100

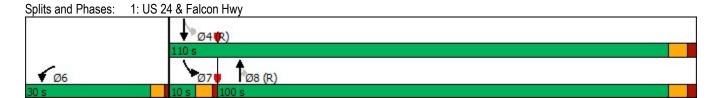
Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.57 Intersection Signal Delay: 43.2 Intersection LOS: D Intersection Capacity Utilization 81.4% ICU Level of Service D

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite.

Queue shown is maximum after two cycles.



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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		^ ^	7	*	^ ^
Traffic Volume (vph)	426	44	950	276	29	2600
Future Volume (vph)	426	44	950	276	29	2600
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	1300	490	775	1300
	1			490	113	
Storage Lanes		0				
Taper Length (ft)	25	4.00	0.04	4.00	90	0.04
Lane Util. Factor	1.00	1.00	0.91	1.00	1.00	0.91
Frt	0.987			0.850		
FIt Protected	0.957				0.950	
Satd. Flow (prot)	1759	0	4803	1495	1736	4988
FIt Permitted	0.957				0.248	
Satd. Flow (perm)	1759	0	4803	1495	453	4988
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	3			291		
Link Speed (mph)	45		65			65
Link Distance (ft)	414		1195			991
Travel Time (s)	6.3		12.5			10.4
Peak Hour Factor	0.92	0.92	0.95	0.95	0.95	0.95
Heavy Vehicles (%)	2%	2%	8%	8%	4%	4%
Adj. Flow (vph)	463	48	1000	291	31	2737
Shared Lane Traffic (%)						
Lane Group Flow (vph)	511	0	1000	291	31	2737
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	9	1.00	1.00
Number of Detectors	10	3	2	1	10	2
					· ·	
Detector Template	Left		Thru	Right	Left	Thru
Leading Detector (ft)	20		100	20	20	100
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
Detector 1 Size(ft)	20		6	20	20	6
Detector 1 Type	Cl+Ex		CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)	0.0		94	0.0	0.0	94
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
Detector 2 Channel						
Detector 2 Extend (s)			0.0			0.0
Turn Type	Prot		NA	Perm	pm+pt	NA
Protected Phases	6		8		7	4

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Permitted Phases				8	4	
Detector Phase	6		8	8	7	4
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	12.0		41.0	41.0	9.5	41.0
Total Split (s)	30.0		100.0	100.0	10.0	110.0
Total Split (%)	21.4%		71.4%	71.4%	7.1%	78.6%
Maximum Green (s)	26.0		94.0	94.0	5.5	104.0
Yellow Time (s)	3.0		4.0	4.0	3.5	4.0
All-Red Time (s)	1.0		2.0	2.0	1.0	2.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0		6.0	6.0	4.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Max		C-Max	C-Max	None	C-Max
Act Effct Green (s)	26.0		98.0	98.0	105.5	104.0
Actuated g/C Ratio	0.19		0.70	0.70	0.75	0.74
v/c Ratio	1.55		0.30	0.26	0.08	0.74
Control Delay	300.6		8.6	1.4	4.8	11.8
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	300.6		8.6	1.4	4.8	11.8
LOS	F		A	Α	A	В
Approach Delay	300.6		7.0			11.7
Approach LOS	F		A			В
Queue Length 50th (ft)	~657		128	0	6	463
Queue Length 95th (ft)	#885		152	29	14	510
Internal Link Dist (ft)	334		1115			911
Turn Bay Length (ft)	001			490	775	9.1
Base Capacity (vph)	329		3361	1133	391	3705
Starvation Cap Reductn	0		0	0	0	0/00
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	1.55		0.30	0.26	0.08	0.74
reduced v/o reduc	1.00		0.00	0.20	0.00	0.14

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Intersection Summary

Other Area Type:

Cycle Length: 140 Actuated Cycle Length: 140

Offset: 45 (32%), Referenced to phase 4:SBTL and 8:NBT, Start of Green

Natural Cycle: 80

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.55 Intersection Signal Delay: 42.7 Intersection Capacity Utilization 84.8%

Intersection LOS: D ICU Level of Service E

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.

Intersection						
Int Delay, s/veh	0.1					
		EDD	WDL	WDT	NDI	NDD
	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	7	-		4	Y	
Traffic Vol, veh/h	300	5	1	465	5	1
Future Vol, veh/h	300	5	1	465	5	1
Conflicting Peds, #/hr	0	_ 0	_ 0	_ 0	0	0
0	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	# 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	78	78
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	326	5	1	505	6	1
Major/Minor Ma	ajor1	N	Major2		Minor1	
						200
Conflicting Flow All	0	0	331	0	836	329
Stage 1	-	-	-	-	329	-
Stage 2	-	-	-	-	507	-
Critical Hdwy	-	-	4.12	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	-	-	2.218	-		
Pot Cap-1 Maneuver	-	-	1228	-	337	712
Stage 1	-	-	-	-	729	-
Stage 2	-	-	-	-	605	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1228	-	337	712
Mov Cap-2 Maneuver	-	-	-	-	337	-
Stage 1	-	-	-	-	729	-
Stage 2	-	-	-	-	604	-
Annroach	EB		WB		NB	
Approach						
HCM Control Delay, s	0		0		15	
HCM LOS					С	
Minor Lane/Major Mvmt	١	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		369	_		1228	_
HCM Lane V/C Ratio		0.021	_		0.001	_
HCM Control Delay (s)		15	_	_	7.9	0
HCM Lane LOS		C	_	_	Α.5	A
HCM 95th %tile Q(veh)		0.1	_	_	0	-
HOW JOHN JOHN Q(VOII)		0.1			U	

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	W		^	7	*	^ ^
Traffic Volume (vph)	401	80	2400	351	36	1300
Future Volume (vph)	401	80	2400	351	36	1300
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0	0	1300	490	775	1300
Storage Lanes	1	0		1	1	
Taper Length (ft)	25	4.00	0.04	4.00	90	0.04
Lane Util. Factor	1.00	1.00	0.91	1.00	1.00	0.91
Frt	0.978			0.850		
FIt Protected	0.960				0.950	
Satd. Flow (prot)	1749	0	4803	1495	1736	4988
Flt Permitted	0.960				0.040	
Satd. Flow (perm)	1749	0	4803	1495	73	4988
Right Turn on Red		Yes		Yes		
Satd. Flow (RTOR)	6			369		
Link Speed (mph)	45		65			65
Link Distance (ft)	414		1195			991
Travel Time (s)	6.3		12.5			10.4
Peak Hour Factor	0.92	0.02	0.95	0.05	0.95	0.95
		0.92		0.95		
Heavy Vehicles (%)	2%	2%	8%	8%	4%	4%
Adj. Flow (vph)	436	87	2526	369	38	1368
Shared Lane Traffic (%)	=					
Lane Group Flow (vph)	523	0	2526	369	38	1368
Enter Blocked Intersection	No	No	No	No	No	No
Lane Alignment	Left	Right	Left	Right	Left	Left
Median Width(ft)	12		12			12
Link Offset(ft)	0		0			0
Crosswalk Width(ft)	16		16			16
Two way Left Turn Lane						
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	1.00	9	1.00	9	15	1.00
Number of Detectors	1	9	2	1	1	2
	Left				Left	Thru
Detector Template	Leπ 20		Thru 100	Right 20	20	100
Leading Detector (ft)				v		
Trailing Detector (ft)	0		0	0	0	0
Detector 1 Position(ft)	0		0	0	0	0
Detector 1 Size(ft)	20		6	20	20	6
Detector 1 Type	CI+Ex		CI+Ex	Cl+Ex	Cl+Ex	Cl+Ex
Detector 1 Channel						
Detector 1 Extend (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Queue (s)	0.0		0.0	0.0	0.0	0.0
Detector 1 Delay (s)	0.0		0.0	0.0	0.0	0.0
Detector 2 Position(ft)			94			94
Detector 2 Size(ft)			6			6
Detector 2 Type			Cl+Ex			Cl+Ex
			CITEX			OI+EX
Detector 2 Channel			0.0			0.0
Detector 2 Extend (s)	F (0.0			0.0
Turn Type	Prot		NA	Perm	pm+pt	NA
Protected Phases	6		8		7	4

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Lane Group	WBL	WBR	NBT	NBR	SBL	SBT
Permitted Phases				8	4	
Detector Phase	6		8	8	7	4
Switch Phase						
Minimum Initial (s)	5.0		5.0	5.0	5.0	5.0
Minimum Split (s)	12.0		41.0	41.0	9.5	41.0
Total Split (s)	30.0		100.0	100.0	10.0	110.0
Total Split (%)	21.4%		71.4%	71.4%	7.1%	78.6%
Maximum Green (s)	26.0		94.0	94.0	5.5	104.0
Yellow Time (s)	3.0		4.0	4.0	3.5	4.0
All-Red Time (s)	1.0		2.0	2.0	1.0	2.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0	0.0
Total Lost Time (s)	4.0		6.0	6.0	4.5	6.0
Lead/Lag			Lag	Lag	Lead	
Lead-Lag Optimize?			Yes	Yes	Yes	
Vehicle Extension (s)	3.0		3.0	3.0	3.0	3.0
Recall Mode	Max		C-Max	C-Max	None	C-Max
Act Effct Green (s)	26.0		96.0	96.0	105.5	104.0
Actuated g/C Ratio	0.19		0.69	0.69	0.75	0.74
v/c Ratio	1.59		0.77	0.32	0.32	0.37
Control Delay	315.5		17.1	1.5	12.8	6.7
Queue Delay	0.0		0.0	0.0	0.0	0.0
Total Delay	315.5		17.1	1.5	12.8	6.7
LOS	F		В	Α	В	Α
Approach Delay	315.5		15.1			6.9
Approach LOS	F		В			Α
Queue Length 50th (ft)	~677		542	0	8	144
Queue Length 95th (ft)	#907		602	32	22	165
Internal Link Dist (ft)	334		1115			911
Turn Bay Length (ft)				490	775	
Base Capacity (vph)	329		3293	1141	120	3705
Starvation Cap Reductn	0		0	0	0	0
Spillback Cap Reductn	0		0	0	0	0
Storage Cap Reductn	0		0	0	0	0
Reduced v/c Ratio	1.59		0.77	0.32	0.32	0.37

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Intersection Summary

Other Area Type:

Cycle Length: 140 Actuated Cycle Length: 140

Offset: 45 (32%), Referenced to phase 4:SBTL and 8:NBT, Start of Green

Natural Cycle: 100

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.59 Intersection Signal Delay: 45.3 Intersection Capacity Utilization 81.8%

Intersection LOS: D ICU Level of Service D

Analysis Period (min) 15

Volume exceeds capacity, queue is theoretically infinite. Queue shown is maximum after two cycles.

Splits and Phases: 1: US 24 & Falcon Hwy

Ø4 (R)

110 s

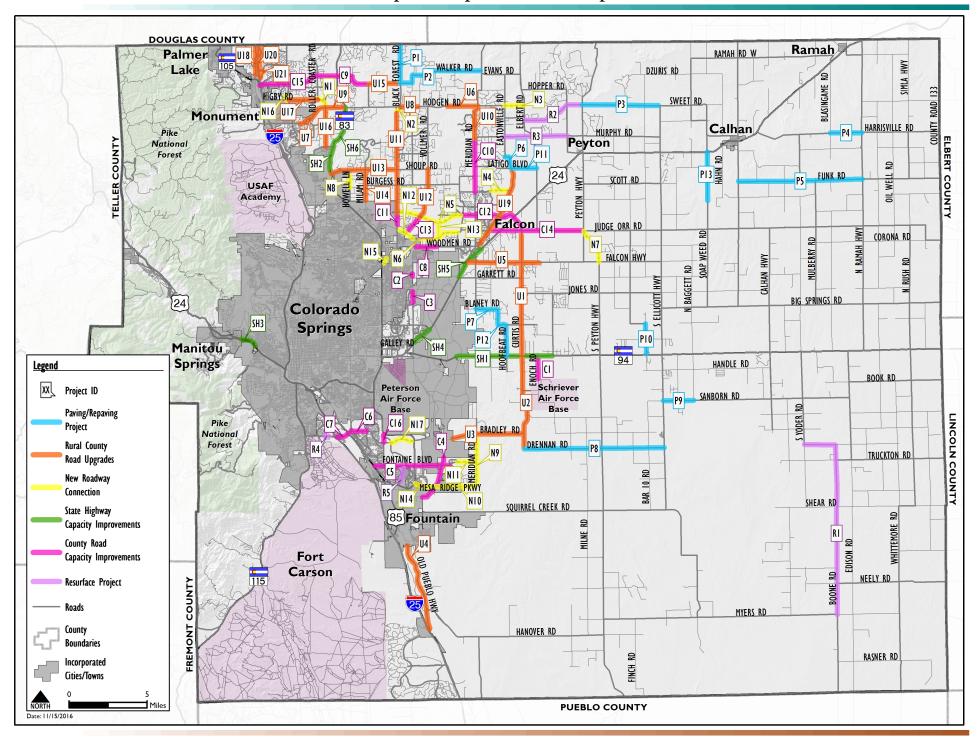
Ø6 (R)

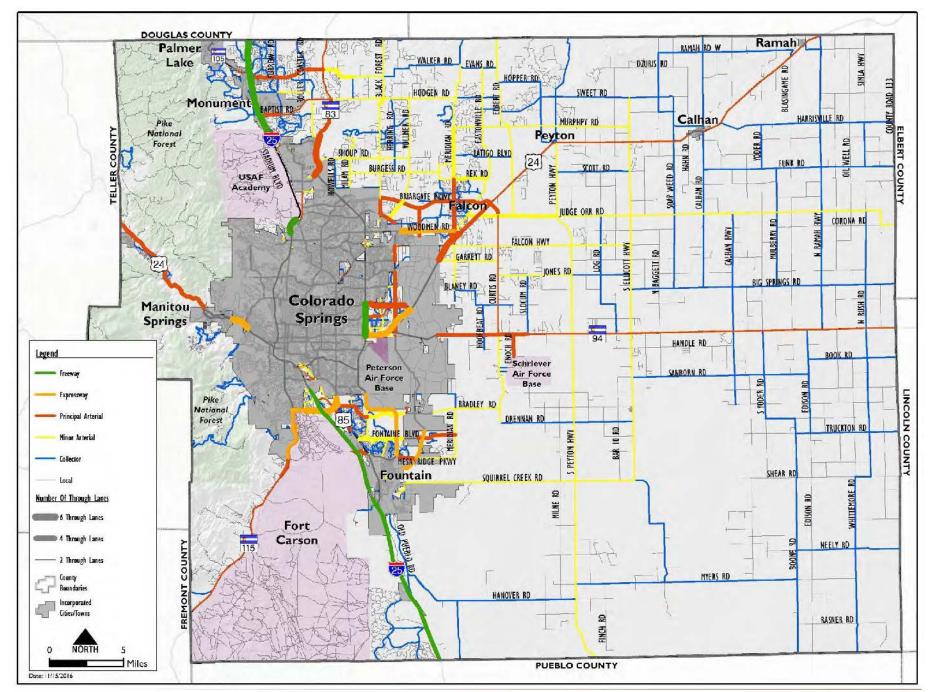
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					0
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					None
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					-
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-	-	-	-		-
-	-	-	-	597	-
-	-		-		
-	-	1138	-		636
-	-	-	-	293	-
-	-	-	-	665	-
-	-	-	-	596	-
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MTCP Maps



Map 13: Improvements Map

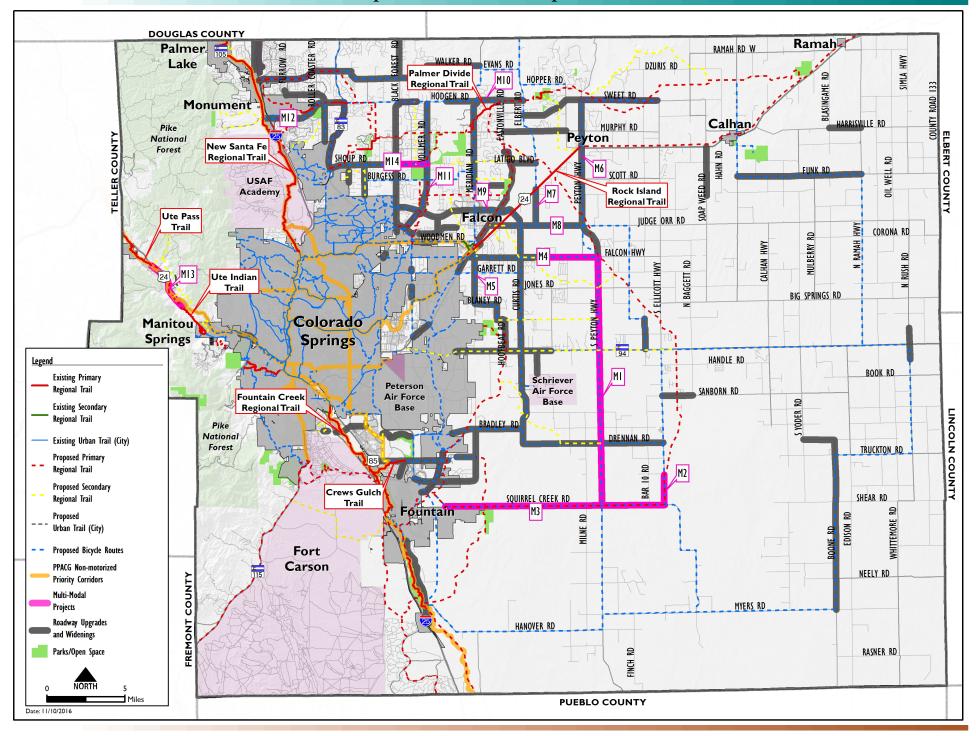




Map 14: 2040 Roadway Plan (Classification and Lanes)



Map 15: Multimodal Improvements



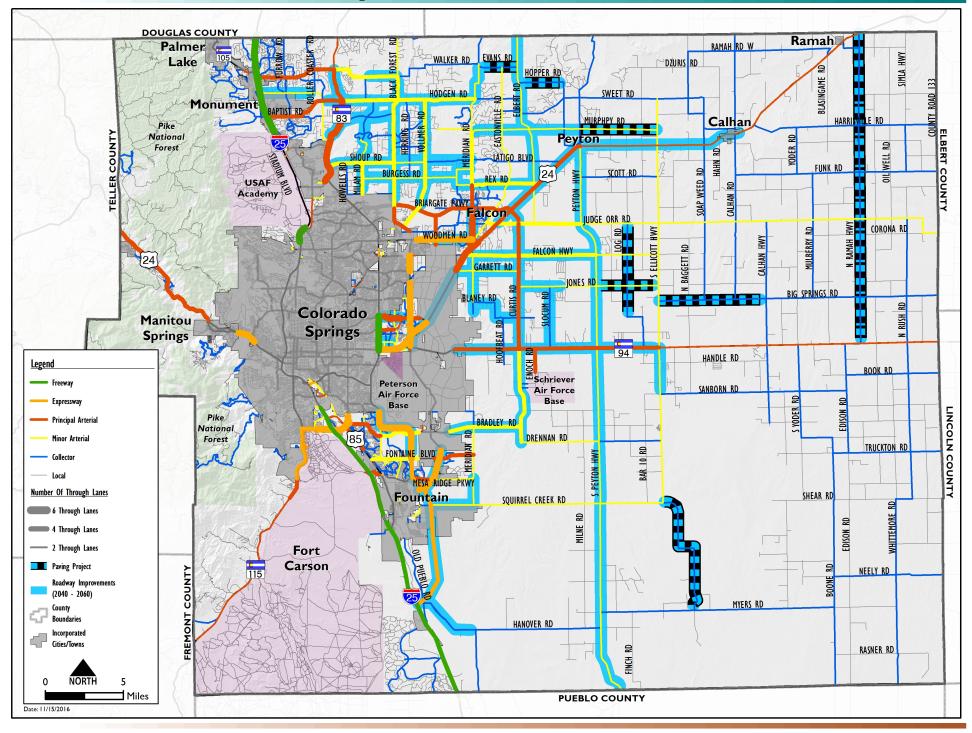


Table 4: 2040 Roadway Improvement Projects

Project		Segr	PPRTA	Urban	Exis	ting Conditions	Future Conditions								
ID	Road Segment	Beginning	End	Project	vs. Rural	Lanes	Functional Class	Lanes	Functional Class	Total Cost					
	County Road Upgrades														
U1	Curtis Rd	Judge Orr Rd.	SH 94		Rural	2	Unimproved County Road	2	Principal Arterial	\$35,549,000					
U2	Curtis Rd	SH 94	Drennan Rd		Rural	2	Unimproved County Road	2	Minor Arterial	\$23,379,000					
U3	Bradley Rd	COS City Limit	Curtis Rd		Rural	2	Unimproved County Road	2	Minor Arterial	\$24,252,000					
U4	Old Pueblo Rd	Fountain City Limits	I-25	В	Rural	2	Unimproved County Road	2	Collector	\$16,722,000					
U5	Falcon Hwy	US 24	1 mi east of Curtis Rd		Rural	2	Unimproved County Road	2	Minor Arterial	\$16,509,000					
U6	Hodgen Rd	Goshawk Rd	Meridian Rd.	В	Rural	2	Unimproved County Road	2	Minor Arterial	\$7,698,000					
U7	Baptist Rd	Desiree Dr	Roller Coaster Rd		Rural	2	Unimproved County Road	2	Collector	\$5,286,000					
U8	Hodgen Rd	Black Forest Rd	Bar X Rd	В	Rural	2	Unimproved County Road	2	Minor Arterial	\$5,053,000					
U9	Hodgen Rd	Roller Coaster Rd	SH 83		Rural	2	Unimproved County Road	2	Minor Arterial	\$3,518,000					
U10	Meridian Rd	Hodgen Rd	Murphy Rd	В	Rural	2	Unimproved County Road	2	Minor Arterial	\$7,763,000					
U11	Black Forest Rd	Hodgen Rd	Stapleton Dr	В	Rural	2	Unimproved County Road	2	Minor Arterial	\$22,714,000					
U12	Vollmer Rd	Stapleton Dr	Shoup Rd	В	Rural	2	Unimproved County Road	2	Minor Arterial	\$11,691,000					

Recent Area TIS Reports



Appendix: Recent Traffic Reports Utilized in the Preparation of this Report



LSC TRANSPORTATION CONSULTANTS, INC.
2504 East Pikes Peak Avenue, Suite 304
Colorado Springs, CO 8099
(719) 633-2688
FAX (719) 633-5430
E-mail: hc@loctrant.com
Websike: http://www.loctrans.com

D-49 Transportation Center Traffic Impact Study (LSC #S214340) PCD File No. U-221 PPR2236 December 1, 2022 (Rev. 4/20/2023)



LSC TRANSPORTATION CONSULTANTS, INC.
2504 East Pikes Peak Avenue, Suite 304
Colorado Springs, CO 80909
(719) E33-2868
FAX (719) E33-5430
E-mail: ballictum.com
Website: http://www.lsctrans.com

Meadowlake Industrial Park Master Traffic Impact Study PCD File No. CS221, I221, I222 (LSC #5214950) July 29, 2022