PRELIMINARY DRAINAGE REPORT
FLYING HORSE NORTH PRELIMINARY PLAN AND
FINAL DRAINAGE REPORT FOR FLYING HORSE NORTH FILING NO. 1

CCES COMMENT RESPONSES

TABLE OF CONTENTS:

Unresolved
05/30/2018

Will provide with final subm.

1. Provide road side ditch analysis with recommendations for ditch lining. Given the steep slopes on sections of the proposed road network, additional erosion protection.

2. Provide riprap sizing calculations for the proposed outlet protections.

APPENDICES

Include an existing drainage map and provide the narrative for the sub-basin

Resolved
05/30/2018

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Design Point 1 (Q_2 = 2 cfs Q_5 = 3 cfs, Q_{100} = 11 cfs) represents the existing off-site and on-site developed flows from Basins OS-1A and BS-2B. The combined flow from these basins travel to a low point just east of Stagecoach Road where a proposed 24" RCP culvert will be installed to convey these flows under the road. (See Appendix for culvert design)

Design Point 2 (Q_2 = 3 cfs Q_5 = 9 cfs, Q_{100} = 35 cfs) represents flows from DP 1 and Basin BS-4. These combined flows are collected at a low point where a proposed 30" RCP culvert will be installed to replace the temporary sediment basin installed with early grading. (See Appendix for culvert design)

The total developed flows entering **Detention Facility 1** including Basin BS-1A equals **(Q_2 = 3 cfs Q_5 = 9 cfs, Q_{100} = 38 cfs)**. These combined flows will travel in the natural drainage corridor across lot 1 and enter the detention facility. The following describes the design of this facility (See Appendix for UD Detention pond design sheets):

Resolved
05/30/2018

6.6 per UD Detention Worksheet

Detention Pond 1 (Full Spectrum – see multiple storm relea

0.43 Ac.-ft. EURV required

0.50 Ac.-ft. EURV design with 3:1 max. slopes

0.99 Ac.-ft. 100-yr. Storage

Total In-flow:	**Q_2 = 3 cfs,**	**Q_5 = 9 cfs,**	**Q_{100} = 38 cfs**
Pond Design Release:	**Q_2 = 0.1 cfs,**	**Q_5 = 0.2 cfs,**	**Q_{100} = 25 cfs**
Pre-development Release:	**Q_2 = 0.2 cfs,**	**Q_5 = 0.4 cfs,**	**Q_{100} = 23 cfs**

(Ownership and maintenance by the Flying Horse North HOA)

Design Point 4 (Q_2 = 3 cfs Q_5 = 4 cfs, Q_{100} = 8 cfs) represents existing and developed flows from Basin BS-2 (north side of Stagecoach Rd.) These flows will travel in a side road ditch towards Hwy. 83. A temporary sediment basin will be installed during construction of this portion of the roadway. This development will be required to provide improvements to this intersection and Hwy. 83 per the site traffic study. Upon review/approval from CDOT, these improvements will be constructed along with final design of drainage at this intersection which will include the relocation of the dual 18" ERCP culverts.

BS-2B

Resolved
05/30/2018

BS-2 updated on map

Identify the trigger for removal of the TSB.

Resolved
05/30/2018

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Design Point 5 (Q_2 = 2 cfs Q_5 = 4 cfs, Q_{100} = 13 cfs) represents existing and developed flows from Basins OS-1B and BS-2A (south side of Stagecoach Rd.) These flows will travel in a side road ditch towards Hwy. 83. A temporary sediment basin will be installed during construction of this portion of the roadway. Upon review/approval from CDOT, these improvements will be constructed along with final design of drainage at this intersection. ← Identify the trigger for removal of the TSB.

Resolved
05/30/2018

Design Point 6 (Q_2 = 1 cfs Q_5 = 3 cfs, Q_{100} = 15 cfs) represents flows from Basins OS-2 and BS-3. These combined flows are collected at a low point where proposed dual 18" RCP culverts will be installed to replace the temporary sediment basin installed with early grading. (See Appendix for culvert design) These flows then combine with Basin BS-1B and travel in the existing natural channel towards the existing downstream 48" CMP culvert. **Design Point 3 (Q_2 = 1 cfs Q_5 = 6 cfs, Q_{100} = 39 cfs)** then represents the total flow from this site leaving the property at this location. The pre-development on-site flow at this location equals **Q_2 = 1 cfs Q_5 = 5 cfs, Q_{100} = 41 cfs.** Thus, the downstream facilities will not see a significant change in flows.

Design Point 7 (Q_2 = 2 cfs Q_5 = 8 cfs, Q_{100} = 38 cfs) represents existing and developed flows from Basins OS-3 and BS-5. These flows will travel as sheet flow towards the low point where dual 30" RCP culverts will installed under Stagecoach Road to replace the temporary sediment basin installed with early grading. (See Appendix for culvert design)

High Forest Ranch Detention Pond 16 outfalls onto the property just upstream of Design Point 8. These existing flows will continue to enter the site and travel through proposed triple 42" RCP culverts under Stagecoach Road. (See Appendix for culvert design) **Design Point 8 (Q_2 = 21 cfs Q_5 = 70 cfs, Q_{100} = 284 cfs)** represents the existing and developed flows exiting the property and continuing south within the natural channel on the Shamrock Ranch property. These flows remain consistent with the historic flows at this location.

Design Point 9 (Q_2 = 1 cfs Q_5 = 5 cfs, Q_{100} = 23 cfs) represents existing flows from Basins OS-7 and BS-12. These combined flows are collected at a low point where proposed dual 24" RCP culverts will be installed to replace the temporary sediment basin installed with early grading. (See Appendix for culvert design)

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Design Point 10 (Q_2 = 11 cfs Q_5 = 32 cfs, Q_{100} = 143 cfs) represents existing and developed flows from Basins OS-8, OS-10, OS-11, BS-13 and BS-14. These flows will travel to the low point at this location where dual 42" RCP culverts will be installed for the crossing of Stagecoach Road. (See Appendix for culvert design)

Design Point 11 (Q_2 = 5 cfs Q_5 = 12 cfs, Q_{100} = 36 cfs) represents developed flow from Basin BS-16. These flows will travel to the low point at this location where dual 24" RCP culverts will be installed for the crossing of the road. (See Appendix for culvert design)

Explain why the construction plans show a bend & manhole. Is there a future extension of the storm system?

Yes, 36" RCP will route these flows directly into pond 4.

Design Point 12 (Q_2 = 4 cfs Q_5 = 11 cfs, Q_{100} = 44 ...ed developed flow from Basins BS-16 and BS-15. These flows will travel to the ... where a 30" RCP culvert will be installed and directly enter Detention Pond 4 at the south end. (See Appendix for culvert design)

Resolved 05/30/2018

Update the 2yr and 5yr to match the ud-detention worksheet.

The total developed flows entering **Detention Facility 4**, including ...qual **(Q_2 = 3 cfs Q_5 = 52 cfs, Q_{100} = 217 cfs)**. The major flows enter the facility at the north end through a rock chute. (See Appendix for rock chute and pond design) The following describes the design of this facility:
(See Appendix for UD Detention pond design sheets):

Detention Pond 4 (Full Spectrum – see multiple storm release data below)
0.99 Ac.-ft. EURV required
1.05 Ac.-ft. EURV design with 4:1 max. slopes
5.06 Ac.-ft. 100-yr. Storage

Total In-flow:	**Q_2 = 18 cfs,**	**Q_5 = 52 cfs,**	**Q_{100} = 217 cfs**
Pond Design Release:	**Q_2 = 0.3 cfs,**	**Q_5 = 0.3 cfs,**	**Q_{100} = 142 cfs**
Pre-development Release:	**Q_2 = 1.5 cfs,**	**Q_5 = 2.5 cfs,**	**Q_{100} = 152cfs**

(Ownership and maintenance by the Flying Horse North HOA)

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Basin BS-24 (Q_2 = 0.6 cfs Q_5 = 3 cfs, Q_{100} = 18 cfs) represents sheet flow from three residential lots within Filing No. 1 that will continue to direct release off-site. However, portions of this historic basin area will be routed into Flying Horse North Pond 8, therefore the developed flows from this basin do not significantly change from the pre-development condition. The pre-development flows from the historic basin area equal **Q_2 = 0.2 cfs Q_5 = 2 cfs, Q_{100} = 18 cfs.** Also, given the lot size, no water quality is required.

FLYING HORSE NORTH FILING NO. 1

East Cherry Creek Drainage Basin

The following basins are still tributary to the Filing No. 1 platting area but are within the East Chery Creek Drainage Basin:

See revised

Design Point 24 (Q_2 = 2 cfs Q_5 = 8 cfs, Q_{100} = 45 cfs) represents developed flows from Basins CC-4C and CC-5. Basin CC-4C represents the f f this site a site specific detention/SWQ facil esign purposes this basin is assumed to releas esign Point 24 where a 36" RCP culvert is (See Appendix for culvert design)

If the concentrated flow from the future pond is directed to the residential lots, there will likely be drainage impacts.

Provide additional guidance in the narrative regarding the release location (consider the 100yr and emergency overflow path). Specific drainage easements may be required if draining through residential lots.

Resolved
05/30/2018

The total developed flows entering **Detention Facility 12**, including Basin CC-6 equals **(Q_2 = 4 cfs Q_5 = 17 cfs, Q_{100} = 86 cfs)**. The following describes the design of this facility:
(See Appendix for UD Detention pond design sheets):

Detention Pond 12 (Full Spectrum – see multiple storm release data below)
0.66 Ac.-ft. EURV required
0.75 Ac.-ft. EURV design with 4:1 max. slopes
2.69 Ac.-ft. 100-yr. Storage

Total In-flow:	**Q_2 = 4 cfs,**	**Q_5 = 17 cfs,**	**Q_{100} = 86 cfs**
Pond Design Release:	**Q_2 = 0.2 cfs,**	**Q_5 = 0.3 cfs,**	**Q_{100} = 45 cfs**
Pre-development Release:	**Q_2 = 0.5 cfs,**	**Q_5 = 0.9 cfs,**	**Q_{100} = 55 cfs**

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the low-point where a 24" RCP culvert is sized to handle the fully developed flows at this location. (See Appendix for culvert design) A downstream s e sediment control for the small developed roadway area.

Resolved 05/30/2018

Missing. Include the culvert design in the Appendix and the construction plans.

Now included

Design Point 31 (Q_2 = 0.7 cfs Q_5 = 3 cfs, Q the full build-out developed flows from Basin C elease fron resents future residential 5 ac. lots. The flows wil towards th e an existing 24" CMP culvert will adequately handle the fully developed flows at th

Unresolved. Culvert report for DP 31 is missing.

Include the supporting calculation. Verify there is sufficient drainage easement that covers the natural swale.

Added info. and grant.

Design Point 32 (Q_2 = 2 cfs Q_5 = 8 cfs, Q_{100} = 40 cfs) represents the full build-out developed flows from Basins OS-16 and CC-17. Basin CC-17 represents future residential lots and OS-16 unplatted, 5-ac. zoned residential property. These flows will continue to sheet flow towards the low-point where a 36" RCP culvert is sized to handle the fully developed flows at this location. (See Appendix for culvert design) A downstream sediment basin will be installed to provide sediment control for the small developed roadway area.

FLYING HORSE NORTH PRELIMINARY PLAN (Future Platting)

Black Squirrel Creek Drainage Basin

The following basins are in the Black Squirrel Creek Drainage Basin but are not a part of Filing 1 lot development. These areas will require future final drainage report(s) upon future lot development.

Design Point 18 (Q_2 = 5 cfs Q_5 = 22 cfs, Q_{100} = 115 cfs) represents developed flows from Basins BS-28, BS-29, BS-30 and OS-18. Portions of basins BS-28 and BS-29 include golf course development taking place with Filing No. 1. However, the majority of these basins include forested future residential lots with basin OS-18 being existing 2.5 ac. minimum lots. Future developed flows will be routed to this location where a future detention facility will be installed. This facility will be sized to meet EURV requirements and release pre-development flow quantities. In the interim, with only the golf course construction, a temporary sediment basin located withi ovide sediment control from the developed g

Elaborate on where this drains to. From the drainage map it seems to drain into the future detention facility at DP 19. If this is the case then identify that ponds DP18 and DP19 must be designed/analyzed as ponds in a series.

Identify and discuss the emergency overflow path for Pond 18. It appears that it would cross over residential lots and into Pond DP19

Resolved 05/30/2018

See additional text in report

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The following calculations are based on the 2017 drainage/bridge fees for the Black Squirrel Creek Drainage Basin:

Update the calculation based on the 2018 rates since the application was submitted in 2018.

Resolved 05/30/2018

FILING 1 FEE TOTALS:

Bridge Fees

$ 465.00 x 20.96 Impervious Ac. = $ 9,746.40

Drainage Fees

$ 7,387.00 x 20.96 Impervious Ac. = $ 154,831.52

Add a section listing the 4 step process (ECM Appendix I Section I.7.2) with narrative on how each step was considered and/or incorporated in the drainage design.

Resolved 05/30/2018 Added

SUMMARY

This proposed development remains consistent with the previously approved Flying Horse North MDDP and Preliminary Drainage Report for Flying Horse North (Golf Course grading and private access roads). The proposed storm facilities have been sized to adequately handle the 100-yr. developed flows. All proposed detention facilities meet current criteria and provide full spectrum design. Upon future development outside of Filing No. 1, final drainage reports will be required finalizing final design of the proposed future drainage facilities. The proposed development will not adversely impact surrounding developments.

PREPARED BY:

Classic Consulting Engineers & Surveyors, LLC

Marc A. Whorton, P.E.
Project Manager

Maw/109611/reports/109611PDR.doc

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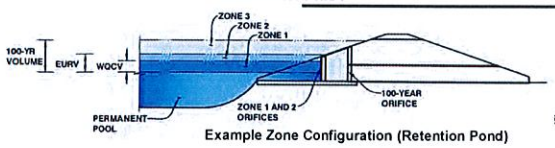
Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: Flying Horse North Filing No. 1

Basin ID: Pond 1



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.85	0.210	Orifice Plate
Zone 2 (EURV)	2.74	0.223	Orifice Plate
Zone 3 (100-year)	5.04	0.710	Weir&Pipe (Restrict)
		1.143	Total

0

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	2.75	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	11.00	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft^2
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft^2

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.90	1.80	2.70				
Orifice Area (sq. inches)	0.99	1.43	1.43	1.43				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft^2
Vertical Orifice Centroid =	N/A	N/A	feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	2.75	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	4.00	N/A	feet
Overflow Weir Slope =	3.00	N/A	H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	4.00	N/A	feet
Overflow Grate Open Area % =	75%	N/A	%, grate open area/total area
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	4.08	N/A	feet
Over Flow Weir Slope Length =	4.22	N/A	feet
Grate Open Area / 100-yr Orifice Area =	5.00	N/A	should be ≥ 4
Overflow Grate Open Area w/o Debris =	12.65	N/A	ft^2
Overflow Grate Open Area w/ Debris =	6.32	N/A	ft^2

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.33	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	24.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	18.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	2.53	N/A	ft^2
Outlet Orifice Centroid =	0.83	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	2.09	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	5.00	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	25.00	feet
Spillway End Slopes =	3.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

Spillway Design Flow Depth=	0.69	feet
Stage at Top of Freeboard =	6.69	feet
Basin Area at Top of Freeboard =	0.39	acres

Routed Hydrograph Results

Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.39
Calculated Runoff Volume (acre-ft) =	0.210	0.433	0.319	0.469	0.809	1.596	2.099	2.758	4.375
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.210	0.432	0.319	0.469	0.809	1.596	2.099	2.757	4.375
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.02	0.16	0.55	0.77	1.04	1.66
Predevelopment Peak Q (cfs) =	0.0	0.0	0.2	0.4	3.5	12.0	16.7	22.6	36.2
Peak Inflow Q (cfs) =	3.0	6.1	4.5	6.6	11.4	22.3	29.2	38.2	60.2
Peak Outflow Q (cfs) =	0.1	0.2	0.1	0.2	3.8	13.7	20.2	24.6	49.8
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.5	1.1	1.1	1.2	1.1	1.4
Structure Controlling Flow =	Plate	Plate	Plate	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	0.0	0.3	1.1	1.6	1.9	2.1
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	50	66	59	67	64	54	49	45	37
Time to Drain 99% of Inflow Volume (hours) =	53	71	63	73	72	68	66	63	56
Maximum Ponding Depth (ft) =	1.78	2.63	2.21	2.76	3.28	3.84	4.10	4.59	5.44
Area at Maximum Ponding Depth (acres) =	0.21	0.26	0.25	0.27	0.29	0.31	0.31	0.33	0.37
Maximum Volume Stored (acre-ft) =	0.194	0.403	0.295	0.438	0.582	0.750	0.831	0.986	1.283

Resolved
05/30/2018

0.9 for full spectrum detention. Comment applies for all Full Spectrum Pond design

revised

Resolved
05/30/2018

Runoff is now concentrated flow. Analyze and discuss the results/recommendations in the narrative. It seems that erosion will occur, a swale may be required within a drainage easement. For this particular location the swale would connect to the pond with a rundown and a trickle channel to the outlet structure.

Review all other culvert locations regarding the above comment.

Removed culverts @ DP 6 and now route d'tal around cul-de-sac. See revised th rundown and trickle channel. Added esmt.

Resolved
05/30/2018

Show the 100yr spread from DP3. Is the natural swale sufficient to keep the runoff from draining into Pond 1?

Now shown

Resolved
05/30/2018

Is this an existing culvert?

Yes added label

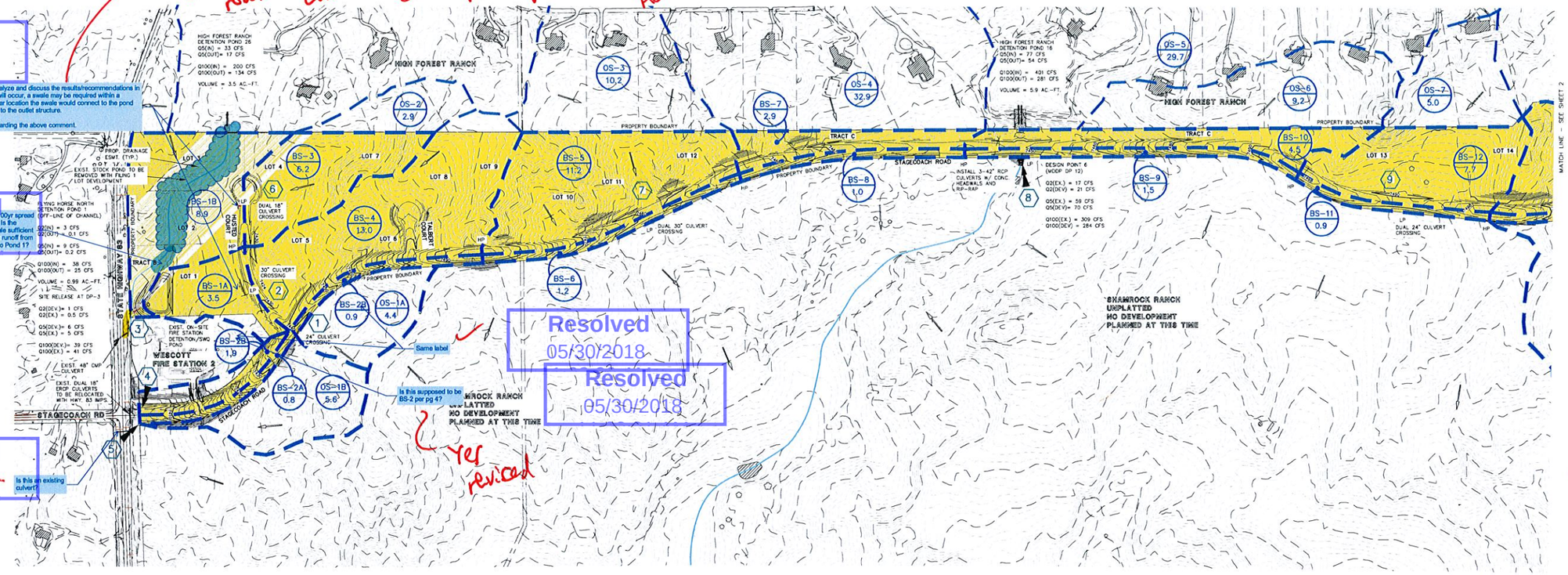
Resolved
05/30/2018

Same label

Is this supposed to be BS-2 per pg 4?

Yes revised

Resolved
05/30/2018



BASIN SUMMARY - DEVELOPED CONDITIONS

BASIN (label)	AREA (acres)	COMPOSITE CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
OS-1A	4.40	61.0	0.20	0.4	1.6	7.7
OS-1B	5.60	61.0	0.21	0.5	1.9	9.4
EX-DP-3 (Pre-Dev.)	36.00	60.0	0.25	0.5	4.8	41.3
OS-2	2.90	61.0	0.20	0.1	0.6	4.0
OS-3	10.20	65.0	0.19	1.0	3.8	17.9
OS-4	32.90	65.0	0.23	2.8	11.2	53.6
OS-5	29.70	65.0	0.39	1.9	7.1	37.0
OS-6	9.20	65.0	0.21	0.9	3.2	15.5
OS-7	5.00	65.0	0.18	0.5	2.0	9.0
BS-1A	3.50	65.0	0.17	0.4	1.4	6.3
BS-1B	8.90	65.0	0.20	0.4	2.4	13.8
BS-2	1.90	89.0	0.35	2.9	4.2	8.4
BS-2A	0.80	89.0	0.13	1.2	1.8	3.5
BS-2B	0.90	89.0	0.12	1.4	2.0	4.0
BS-3	6.20	65.0	0.20	0.6	2.3	10.8
BS-4	13.00	67.0	0.23	1.9	5.5	23.6
BS-5	11.20	65.0	0.18	1.1	4.4	20.1
BS-6	1.20	89.0	0.09	1.9	2.8	5.4
BS-7	2.90	65.0	0.13	4.4	6.4	12.8
BS-8	1.00	89.0	0.12	1.6	2.2	4.5
BS-9	1.50	89.0	0.13	2.3	3.3	6.6
BS-10	4.50	65.0	0.24	6.0	8.7	17.5
BS-11	0.90	89.0	0.08	1.5	2.1	4.1
BS-12	7.70	65.0	0.19	0.8	3.0	13.8

DESIGN POINTS SURFACE ROUTING SUMMARY - DEVELOPED CONDITIONS

Design Point (label)	Contributing Basins	Q 2 Yr. Q (cfs)	Q 5 Yr. Q (cfs)	Q 100 Yr. Q (cfs)
DP-1 DEV	OS-1A, BS-2B	1.6	3.4	11
DP-2 DEV	DP-1, BS-4	3.2	8.8	35
TOTAL INFLOW TO FHN POND 1	**DP-1, DP-2, BS-1A**	**3**	**9**	**38**
DP-3 DEV	**OS-2, BS-3, BS-1B, Release from FHN Pond 1**	**1**	**6**	**39**
DP-4 DEV	BS-2	2.9	4.2	8
DP-5 DEV	OS-1B, BS-2A	1.5	3.5	13
DP-6 DEV	OS-2, BS-3	0.6	2.8	15
DP-7 DEV	OS-3, BS-5	2.1	8.2	38
DP-8 DEV	OS-4, OS-5, OS-6, BS-7, BS-10, Release from Exist. HFR Pond 16	20.9	70.4	284
DP-9 DEV	OS-7, BS-12	1.3	5.0	23

LEGEND

DESCRIPTION	SYMBOL
EXISTING GROUND CONTOUR	6910
PROPOSED FINISHED CONTOUR	6910
BASIN BOUNDARY BLACK SQUIRREL	
DESIGN POINT	3
BASIN IDENTIFIER / AREA IN ACRES	BB 10.0
EXISTING DIRECTION OF FLOW	
PROPOSED DIRECTION OF FLOW	
STORM SEWER	
FILING NO. 1 PLAT AREA	

N

200 100 0 200 400

SCALE: 1" = 200'

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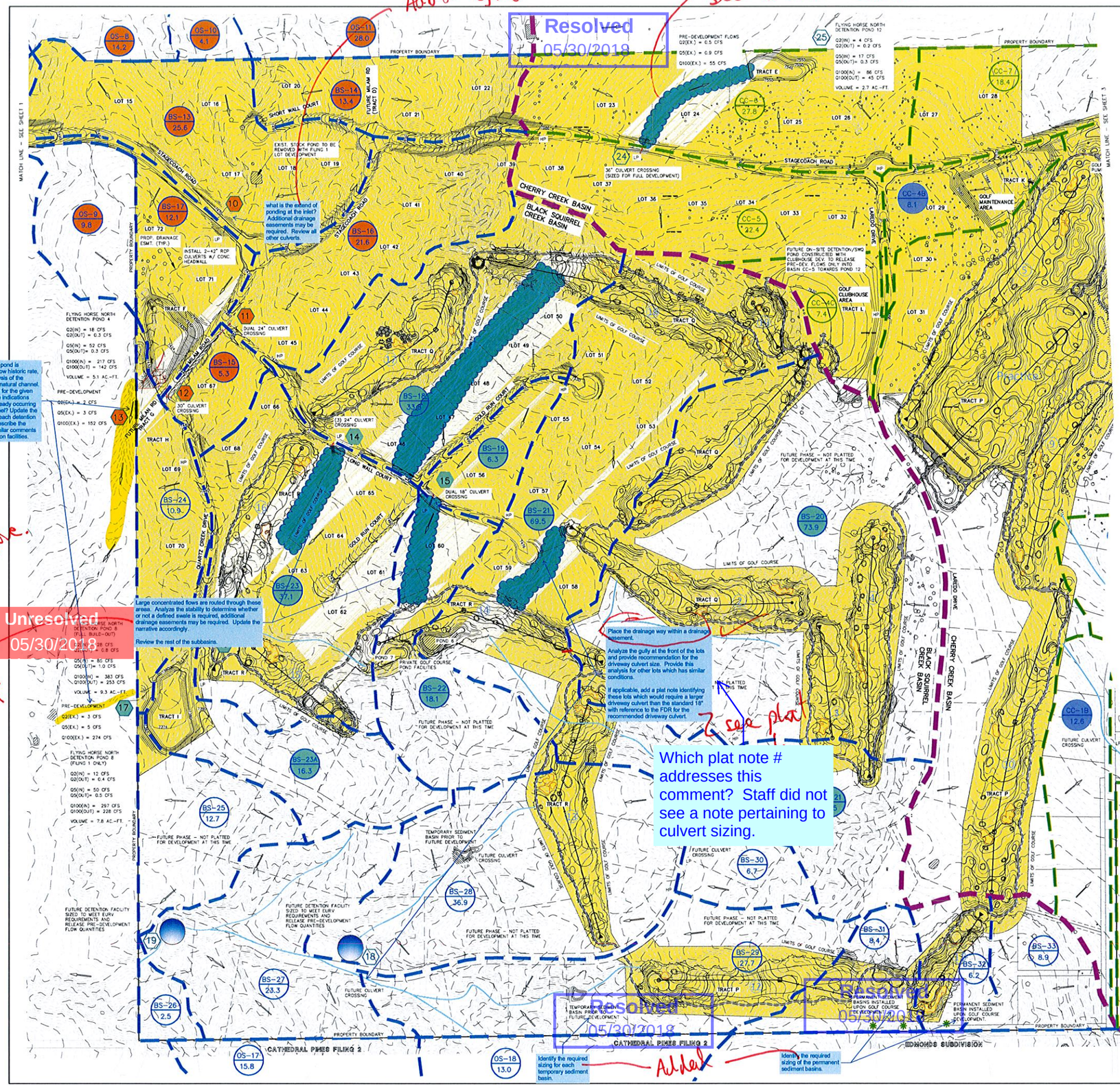
FLYING HORSE NORTH
PRELIMINARY/FINAL DRAINAGE REPORT
FILING NO. 1 DRAINAGE MAP

DESIGNED BY	MAW	SCALE		DATE	10-20-17
DRAWN BY	MAW	(H) 1"= 200'		SHEET	1 OF 4
CHECKED BY		(V) 1"= N/A		JOB NO.	1096.11

Addtl esmt.

See channel calcs.

Resolved
05/30/2018



Unresolved
05/31/2018

see revised narrative.

Unresolved
05/30/2018

See revised report

see plat

Which plat note # addresses this comment? Staff did not see a note pertaining to culvert sizing.

Resolved
05/30/2018

Resolved
05/30/2018

Addtl

BASIN SUMMARY - DEVELOPED CONDITIONS

BASIN (label)	AREA (acres)	COMPOSITE CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
OS-8	14.20	66.0	0.27	2.1	6.2	24.7
OS-9	9.80	60.0	0.37	0.1	1.0	9.1
OS-10	4.10	66.0	0.17	0.7	2.1	8.2
OS-11	28.00	65.0	0.36	2.4	8.2	38.7
OS-12	66.10	62.7	0.37	2.2	11.9	75.8
OS-13	36.90	63.0	0.33	1.4	7.4	45.0
OS-14	26.40	62.0	0.31	0.7	4.6	31.0
OS-15	70.80	63.9	0.36	3.3	14.8	84.2
OS-16	4.50	66.0	0.24	0.4	1.6	7.2
OS-17	15.80	65.0	0.19	1.6	5.9	27.7
OS-18	13.00	66.0	0.20	1.3	4.7	22.6
BS-13	25.60	66.0	0.23	3.7	10.2	40.7
BS-14	13.40	65.0	0.23	2.6	6.8	26.5
BS-15	5.30	66.0	0.18	1.6	3.7	12.2
BS-16	21.60	65.0	0.34	4.6	11.8	44.1
BS-17	12.10	66.0	0.21	3.1	7.7	26.7
BS-18	33.80	63.6	0.41	3.5	12.4	56.0
BS-19	6.30	66.0	0.18	2.1	4.6	16.0
BS-20	73.90	63.4	0.31	7.4	24.6	112.4
BS-21	69.50	64.3	0.35	7.8	23.9	103.0
BS-22	18.10	64.4	0.22	3.7	9.6	36.5
BS-23	37.10	63.3	0.33	4.5	13.6	58.7
BS-23A	16.30	64.4	0.29	5.5	12.0	38.3
BS-24	10.90	63.0	0.17	0.6	3.3	17.6
EX-24 (Pre-Dev.)	13.20	60.0	0.17	0.2	2.2	17.6
BS-25	12.70	63.0	0.23	0.4	2.7	17.3
BS-26	2.50	60.0	0.18	0.0	0.4	3.4
BS-27	23.30	65.0	0.22	2.1	8.0	38.8
BS-28	36.90	64.4	0.32	2.2	9.3	49.4
BS-29	27.70	64.0	0.33	1.4	8.5	35.9
BS-30	6.70	65.0	0.20	0.7	2.4	11.7
BS-31	8.40	62.5	0.23	0.8	1.9	11.8
BS-32	6.20	62.6	0.20	0.3	1.6	9.4
BS-33	8.90	64.7	0.19	0.8	3.2	15.3
CC-1A	9.80	65.0	0.25	0.8	3.3	16.0
CC-1B	12.60	64.8	0.25	1.0	4.0	19.4
CC-2A	11.00	65.0	0.22	1.0	3.8	18.3
CC-2B	20.80	65.0	0.22	1.9	7.1	34.6
CC-2C	6.40	65.0	0.18	0.7	2.5	11.5
CC-3	52.50	63.1	0.43	1.8	8.8	54.5
CC-4A	108.70	62.6	0.44	15.4	39.0	156.0
CC-4B	8.10	76.1	0.26	4.0	7.3	20.6
CC-4C (Pre-Dev.)	7.40	61.0	0.13	0.2	1.6	11.2
CC-5	22.40	65.0	0.26	1.8	7.1	34.3
CC-6	27.80	65.0	0.25	2.3	9.1	43.2
CC-7	18.40	65.0	0.29	1.4	5.4	27.0

DESIGN POINTS SURFACE ROUTING SUMMARY - DEVELOPED CONDITIONS

Design Point (label)	Contributing Basins	Q 2 Yr. Q (cfs)	Q 5 Yr. Q (cfs)	Q 100 Yr. Q (cfs)
DP-10 DEV	OS-8, OS-10, OS-11, BS-13, BS-14	10.7	32.0	143
DP-11 DEV	BS-16	4.6	11.8	36
DP-12 DEV	DP-11, BS-15	4.0	11.0	44
TOTAL INFLOW TO FHN POND 4	DP-10, DP-12, BS-17, OS-9	18	52	217
DP-13 DEV	Release from FHN Pond 4	0.3	0.3	142
DP-14 DEV	BS-18	3.5	12.4	56
DP-15 DEV	BS-19	2.1	4.6	15
DP-16 DEV	DP-14, DP-15, BS-20, BS-21, BS-22, BS-23	25.0	78.0	362
TOTAL INFLOW TO FHN POND 8 (Full Build-out)	DP-10, DP-12, BS-17, OS-8	28	85	383
DP-17 DEV (Full Build-out)	Release from FHN Pond 8	0.8	1.0	253
TOTAL INFLOW TO FHN POND 8 (Filing 1 Only)	DP-10, DP-12, BS-17, OS-8	12	50	297
DP-17 DEV (Filing 1 Only)	Release from FHN Pond 8	0.4	0.5	228
DP-18 DEV	BS-28, BS-29, BS-30, OS-18	5.0	21.6	115
DP-19 DEV	BS-27, OS-17, Release from DP-18	3.8	16.8	126
DP-20 DEV	CC-1A, OS-12	3.2	14.3	88
DP-21 DEV	CC-2A, OS-13	2.1	10.5	62
DP-22 DEV	CC-2B, Release from DP-21	3.7	15.6	92
DP-23 DEV	CC-3, OS-14	2.5	13.0	84
DP-24 DEV	CC-4C (Pre-Dev.) CC-5	1.9	8.4	45
TOTAL INFLOW TO FHN POND 12	CC-4C, CC-5, CC-6	4	17	86
DP-25 DEV	Release from FHN Pond 12	0.2	0.3	45

LEGEND

DESCRIPTION	SYMBOL
EXISTING GROUND CONTOUR	6910
PROPOSED FINISHED CONTOUR	6910
BASIN BOUNDARY EAST CHERRY CREEK	
MAJOR BASIN BOUNDARY	
BASIN BOUNDARY BLACK SQUIRREL	
DESIGN POINT	3
BASIN IDENTIFIER / AREA IN ACRES	BB 10.0
EXISTING DIRECTION OF FLOW	
PROPOSED DIRECTION OF FLOW	
STORM SEWER	
FILING NO. 1 PLAT AREA	

N

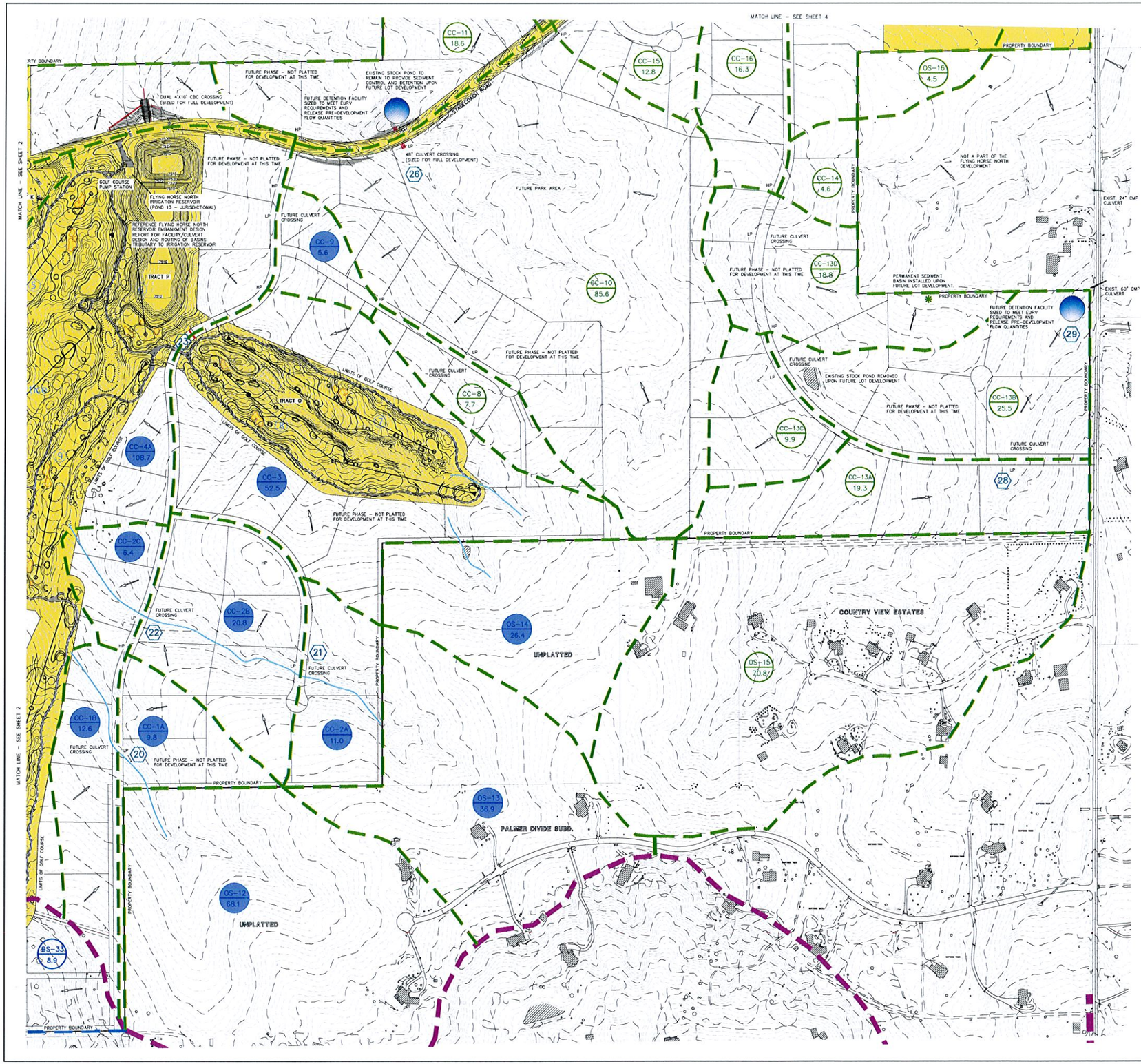
200 100 0 200 400

SCALE: 1" = 200'

CLASSIC CONSULTING ENGINEERS & SURVEYORS
619 N. Cascade Avenue, Suite 200 (719)785-0790
Colorado Springs, Colorado 80903 (719)785-0799 (Fax)

FLYING HORSE NORTH
PRELIMINARY/FINAL DRAINAGE REPORT
FILING NO. 1 DRAINAGE MAP

DESIGNED BY	MAA	SCALE	DATE	10-25-17
DRAWN BY	MAA	(H) 1" = 200'	SHEET 2	OF 4
CHECKED BY		(V) 1" = N/A	JOB NO.	1096.11



BASIN SUMMARY - DEVELOPED CONDITIONS

BASIN (label)	AREA (acres)	COMPOSITE CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
OS-8	14.20	65.0	0.27	2.1	6.2	24.7
OS-9	9.80	60.0	0.37	0.1	1.0	9.1
OS-10	4.10	65.0	0.17	0.7	2.1	8.2
OS-11	28.00	65.0	0.35	2.4	8.2	38.7
OS-12	68.10	62.7	0.37	2.2	11.9	75.8
OS-13	36.90	63.0	0.33	1.4	7.4	45.0
OS-14	26.40	62.0	0.31	0.7	4.6	31.0
OS-15	70.80	63.9	0.36	3.3	14.8	84.2
OS-16	4.50	65.0	0.24	0.4	1.5	7.2
OS-17	15.80	65.0	0.18	1.6	5.9	27.7
OS-18	13.00	65.0	0.20	1.3	4.7	22.6
CC-1A	6.80	65.0	0.23	0.8	3.3	16.0
CC-1B	12.60	64.8	0.25	1.0	4.6	19.4
CC-2A	11.00	65.0	0.22	1.0	3.8	18.3
CC-2B	20.80	65.0	0.22	1.9	7.1	34.6
CC-2C	6.40	65.0	0.18	0.7	2.5	11.5
CC-3	52.50	63.1	0.43	1.8	8.8	54.5
CC-4A	108.70	62.6	0.44	15.4	39.0	156.0
CC-4B	8.10	76.1	0.26	4.0	7.3	20.6
CC-4C (Pre-Dev.)	7.40	61.0	0.13	0.2	1.8	11.2
CC-5	22.40	65.0	0.26	1.8	7.1	34.3
CC-6	37.80	65.0	0.25	2.3	9.1	43.2
CC-7	18.40	65.0	0.29	1.4	5.4	27.0
CC-8	7.70	65.0	0.25	0.5	2.5	12.9
CC-9	5.60	65.0	0.19	0.6	2.1	9.5
CC-10	85.60	62.6	0.39	2.5	14.1	91.9
CC-11	18.60	63.1	0.31	0.9	5.0	26.1
CC-12	12.20	65.0	0.26	1.0	3.9	18.7
CC-13A	19.30	65.0	0.31	1.4	5.4	27.3
CC-13B	25.50	65.0	0.31	1.8	7.2	36.1
CC-13C	9.90	65.0	0.22	0.9	3.4	16.5
CC-13D	18.80	65.0	0.25	1.5	6.2	29.2
CC-14	4.60	65.0	0.21	0.4	1.6	7.8
CC-15	12.80	65.0	0.24	1.1	4.3	20.4
CC-16	16.30	65.0	0.30	1.2	4.8	23.6
CC-17	23.00	65.0	0.35	1.7	6.5	32.5
CC-18	5.80	65.0	0.30	0.4	1.7	8.4
CC-19	3.70	65.0	0.25	0.3	1.2	5.8
CC-20	39.30	65.0	0.25	3.2	12.9	61.0
CC-21	6.20	61.0	0.20	0.1	1.2	8.5
CC-22	13.80	65.0	0.25	1.1	4.5	21.4
CC-23	5.70	54.7	0.33	0.4	1.5	7.7
CC-24	39.60	65.0	0.25	3.3	13.0	61.5
CC-25	3.50	65.0	0.23	0.3	1.2	5.7
CC-26	16.70	65.0	0.26	1.4	5.3	25.5
CC-27	18.90	64.4	0.31	1.2	4.9	25.8
CC-28	154.80	64.4	0.63	6.5	24.7	136.3

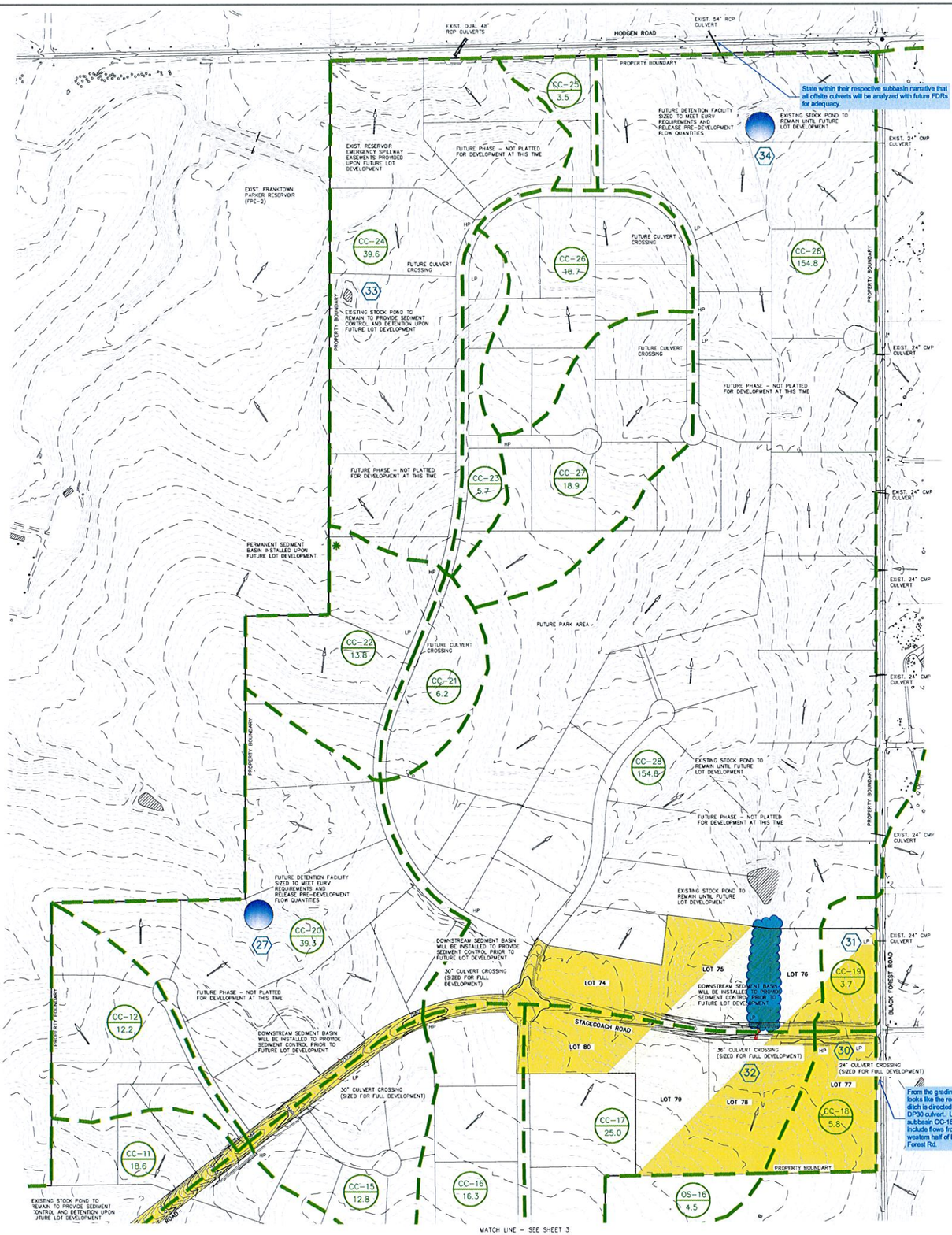
DESIGN POINTS SURFACE ROUTING SUMMARY - DEVELOPED CONDITIONS

Design Point (label)	Contributing Basins	Q 2 Yr. Q (cfs)	Q 5 Yr. Q (cfs)	Q 100 Yr. Q (cfs)
DP-20 DEV	CC-1A, OS-12	3.2	14.3	88
DP-21 DEV	CC-2A, OS-13	2.1	10.5	62
DP-22 DEV	CC-2B, Release from DP-21	3.7	16.6	92
DP-23 DEV	CC-3, OS-14	2.5	13.0	84
DP-24 DEV	CC-4C (Pre-Dev.), CC-5	1.9	8.4	45
TOTAL INFLOW TO FHN POND 12	**CC-4C, CC-8, CC-6**	**4**	**17**	**86**
DP-25 DEV	**Release from FHN Pond 12**	**0.2**	**0.3**	**45**
DP-26 DEV	CC-8, CC-10	3.0	15.9	102
DP-27 DEV	CC-15, CC-20	4.3	17.2	81
DP-28 DEV	CC-13A, OS-15	4.6	19.8	110
DP-29 DEV	CC-13B, CC-13C, Release from DP-28	5.8	26.6	155
DP-30 DEV	CC-18	0.4	1.7	8
DP-31 DEV	CC-19, Release from DP-30	0.7	2.7	14
DP-32 DEV	CC-17, OS-16	2.0	7.8	40
DP-33 DEV	CC-23, CC-24	3.6	14.4	69
DP-34 DEV	CC-16, CC-26, CC-27, CC-28, Release from DP-32	8.0	23.5	168

CLASSIC CONSULTING ENGINEERS & SURVEYORS
619 N. Cascade Avenue, Suite 200 (719)785-0790
Colorado Springs, Colorado 80903 (719)785-0799 (Fax)

FLYING HORSE NORTH
PRELIMINARY/FINAL DRAINAGE REPORT
FILING NO. 1 AND
PRELIMINARY PLAN DRAINAGE MAP

DESIGNED BY	MAW	SCALE		DATE	10-25-17
DRAWN BY	MAW	(H) 1"= 200'		SHEET	3 OF 4
CHECKED BY		(V) 1"= N/A		JOB NO.	1096.11



Added text in report

Resolved
05/30/2018

BASIN SUMMARY - DEVELOPED CONDITIONS

BASIN (label)	AREA (acres)	COMPOSITE CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
OS-16	4.50	65.0	0.24	0.4	1.5	7.2
OS-17	15.80	65.0	0.19	1.6	5.9	27.7
OS-18	13.00	65.0	0.20	1.3	4.7	22.6
CC-11	18.60	63.1	0.21	0.9	5.0	28.1
CC-12	12.20	65.0	0.26	1.0	3.9	18.7
CC-13A	19.30	65.0	0.31	1.4	5.4	27.3
CC-13B	25.50	65.0	0.31	1.8	7.2	36.1
CC-13C	9.90	65.0	0.22	0.9	3.4	16.5
CC-13D	18.80	65.0	0.25	1.5	6.2	29.2
CC-14	4.60	65.0	0.21	0.4	1.6	7.8
CC-15	12.80	65.0	0.24	1.1	4.3	20.4
CC-16	16.30	65.0	0.30	1.2	4.6	23.6
CC-17	25.00	65.0	0.35	1.7	6.5	32.8
CC-18	5.80	65.0	0.30	0.4	1.7	8.4
CC-19	3.70	65.0	0.25	0.3	1.2	5.8
CC-20	39.30	65.0	0.25	3.2	12.9	61.0
CC-21	6.20	61.0	0.20	0.1	1.2	8.5
CC-22	13.80	65.0	0.25	1.1	4.5	21.4
CC-23	5.70	64.7	0.33	0.4	1.5	7.7
CC-24	39.60	65.0	0.25	3.3	13.0	61.5
CC-25	3.50	65.0	0.23	0.3	1.2	5.7
CC-26	16.70	65.0	0.26	1.4	5.3	25.6
CC-27	18.90	64.4	0.31	1.2	4.9	25.8
CC-28	154.80	64.4	0.63	6.5	24.7	136.3

DESIGN POINTS SURFACE ROUTING SUMMARY - DEVELOPED CONDITIONS

Design Point (label)	Contributing Basins	Q 2 Yr. Q (cfs)	Q 5 Yr. Q (cfs)	Q 100 Yr. Q (cfs)
DP-27 DEV	CC-15, CC-20	4.3	17.2	81
DP-28 DEV	CC-13A, OS-15	4.6	19.8	110
DP-29 DEV	CC-13B, CC-13C, Release from DP-28	5.8	26.6	155
DP-30 DEV	CC-18	0.4	1.7	8
DP-31 DEV	CC-19, Release from DP-30	0.7	2.7	14
DP-32 DEV	CC-17, OS-16	2.0	7.8	40
DP-33 DEV	CC-23, CC-24	3.6	14.4	69
DP-34 DEV	CC-16, CC-26, CC-27, CC-28, Release from DP-32	6.0	23.5	168

Resolved
05/30/2018

LEGEND

DESCRIPTION	SYMBOL
EXISTING GROUND CONTOUR	6910
PROPOSED FINISHED CONTOUR	6910
BASIN BOUNDARY EAST CHERRY CREEK	
MAJOR BASIN BOUNDARY	
BASIN BOUNDARY BLACK SQUIRREL	
DESIGN POINT	3
BASIN IDENTIFIER / AREA IN ACRES	BB 10.0
EXISTING DIRECTION OF FLOW	
PROPOSED DIRECTION OF FLOW	
STORM SEWER	
FILING NO. 1 PLAT AREA	

N

200 100 0 200 400

SCALE: 1" = 200'

CLASSIC CONSULTING ENGINEERS & SURVEYORS
619 N. Cascade Avenue, Suite 200, Colorado Springs, Colorado 80903 (719)785-0790 (719)785-0799 (Fax)

FLYING HORSE NORTH
PRELIMINARY/FINAL DRAINAGE REPORT
FILING NO. 1 AND
PRELIMINARY PLAN DRAINAGE MAP

DESIGNED BY	MAA	SCALE		DATE	10-17-17
DRAWN BY	MAA	(H) 1"= 200'		SHEET	4 OF 4
CHECKED BY		(V) 1"= N/A		JOB NO.	1096.11

PRELIMINARY DRAINAGE REPORT
FOR
FLYING HORSE NORTH PRELIMINARY PLAN
AND
FINAL DRAINAGE REPORT
FOR
FLYING HORSE NORTH FILING NO. 1

NOVEMBER 2017
Revised April 2018

Prepared for:
PRI #2 LLC
6385 CORPORATE DRIVE SUITE 200
COLORADO SPRINGS CO 80919
(719) 592-9333

Prepared by:
CLASSIC CONSULTING ENGINEERS & SURVEYORS
619 N. CASCADE AVE SUITE 200
COLORADO SPRINGS CO 80903
(719) 785-0790

Job no. 1096.11
PCD File No. SF-18-001

Add PCD File No. SP-17-012 and SF-18-001
Unresolved

PRELIMINARY DRAINAGE REPORT
FLYING HORSE NORTH PRELIMINARY PLAN AND
FINAL DRAINAGE REPORT FOR FLYING HORSE NORTH FILING NO. 1

DRAINAGE REPORT STATEMENT

ENGINEER'S STATEMENT:
The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

_____ _____
Marc A. Whorton, P.E. Colorado P.E. #37155 Date

OWNER/DEVELOPER'S STATEMENT:
I, the owner/developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Business Name: PRI #2 LLC

By: _____

Title: _____

Address: 6385 Corporate Drive, Suite 200

Colorado Springs, CO 80919

EL PASO COUNTY:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

_____ _____
Jennifer Irvine, P.E. Date
County Engineer / ECM Administrator

Conditions:

CLASSIC SM
CONSULTING
ENGINEERS & SURVEYORS

PRELIMINARY DRAINAGE REPORT
FLYING HORSE NORTH PRELIMINARY PLAN AND
FINAL DRAINAGE REPORT FOR FLYING HORSE NORTH FILING NO. 1

TABLE OF CONTENTS:

APPENDICES

1. Provide road side ditch analysis with recommendations for ditch lining. Given the steep slopes on sections of the proposed road network, additional erosion protection. **Unresolved. Must be provided prior to final submittal. Depending on the analysis, it may impact other documents such as the construction drawings and Financial Assurance Estimate.**

CLASSIC CONSULTING ENGINEERS & SURVEYORS

PURPOSE

The purpose of this Drainage Report is two-fold: first to identify major drainage corridors within this area and recommend preliminary facilities based on the Preliminary Plan layout and secondly to provide specific final design for the necessary facilities within the Filing No. 1 Final Plat area. These proposed facilities will route all developed storm water runoff to adequate outfall facilities. The drainage improvements proposed in this report that are outside of Filing No. 1 are preliminary in nature and future final drainage reports will be required for these areas. The Filing No. 1 design area within the proposed golf course includes the design of a jurisdictional dam facility. This specific facility design is handled in a separate report and submittal package reviewed and approved by the Colorado Dam Safety Board. However, El Paso County Engineering Staff will have the opportunity to review and approved specific aspects of the facility as well.

GENERAL DESCRIPTION

Flying Horse North is a 1,418 acre site located in all of section 36, township 11 south, range 66 west of the sixth principal meridian, and a portion of sections 30 and 31 township 11 south, range 65 west of the sixth principal meridian. The site is bounded on the north by Hodgen Road and the High Forest Ranch Community, to the south by the Cathedral Pines Subdivision and unplatted county land, to the east by Black Forest Road, and to the west by the State Highway 83 and unplatted county land. The site stretches across 2 existing drainage basins, the Black Squirrel Creek Drainage Basin and East Cherry Creek Drainage Basin. Large lot single family residential and a golf course with a club house are included in the proposed Preliminary Plan for this site. A site specific PUD plan and early grading plan for the golf course and associated private access roads was previously approved in the Fall of 2016. Tree removal, grading and erosion control for the golf course and access roads is currently under construction based on this approval.

The average soil condition reflects Hydrologic Group "B" (Brussett Loam, Elbeth Sandy Loam, Kettle Gravelly Loamy Sand, Peyton Sandy Loam, Peyton Pring Complex, Pring Course Sandy Loam, and Tomah-Crowfoot Loamy Sand) as determined by the "Soil Survey of El Paso County Area," prepared by the Soil Conservation Service (see map in Appendix).

EXISTING DRAINAGE CONDITIONS

As described in the MDDP for Flying Horse North, this site sits in the upper reaches of both the Black Squirrel Creek and the East Cherry Creek Drainage Basins. There are approximately 540 acres in the Black Squirrel Creek Drainage Basin and 878 acres of area in the East Cherry Creek Drainage Basin. The majority of the Filing No. 1 area will be within the Black Squirrel Creek Basin, however, all required improvements for Filing No. 1 including the golf course within both basins will be discussed below.

Page 29 of the DBPS noted the upper reaches to remain as natural as possible except for the addition of grade control structures and riprap at sharp horizontal bends for the purpose of stabilizing the channel. Identify the applicable reach within the FHN and analyze.

Black Squirrel Creek Drainage Basin

The Flying Horse North property is located at the top of the Black Squirrel Creek Drainage Basin. Currently there are corrugated metal culverts within Hwy. 83 to convey flows across the existing roadway. Existing conditions in this basin are largely forested with rolling hills and natural valleys and swales draining offsite to the southwest. This basin has been previously studied in the "Black Squirrel Creek Drainage Basin Planning Study" prepared by URS Consultants, January 1989. All the runoff from the area located in the Black Squirrel Drainage Basin converges at the main channel of the Black Squirrel Creek located on the adjacent property. As a part of the MDDP for Flying Horse North, also prepared by Classic consulting, an existing drainage analysis was performed to confirm allowable release rates at key design points along the project boundary. Offsite flows were recreated from surrounding reports that were previously approved. Drainage Criteria has been updated since these reports have been approved. Flow differences will occur due to the updated drainage criteria. Using these previous reports contributing areas, CN values and time of concentrations, this report along with the MDDP have attempted to recreate offsite flows for use in calculating existing conditions. Currently in the Flying Horse North property boundary within the Black Squirrel Drainage basin there are existing stock ponds with berms that retain existing flows from reaching downstream channels until overtopping. There are no records or design plans for these stock ponds. For this existing condition analysis these ponds were removed from the project model.

Along the northern boundary of the property there are numerous locations where off-site flows come onto the site. The High Forest Ranch development has two detention facilities that release concentrated flows directly onto the property. The on-site downstream corridors from these facilities will remain natural to the greatest extent possible and where required improved to accommodate these existing flows. High Forest Ranch Pond 26 releases flows on-site at the northwest corner of the site that then travel through the site towards an existing 48" CMP at Hwy. 83. High Forest Ranch Pond 16 releases flows on-site approximately 4100 LF east of Hwy. 83. These flows cross the site and continue to travel in a southwest direction through the Shamrock Ranch property. This is the start of the headwaters of the Black Squirrel Creek Drainage Basin with the majority of the existing flows coming from the High Forest Ranch Development.

Section 36 lies approximately 1.5 miles east of Hwy. 83 and this is where the bulk of the Flying Horse North property ownership begins. Several major drainage corridors feeding the Black Squirrel Creek Basin traverse

Section 36 and travel in a southwest direction towards the west edge of the property. The MDDP designates four major design points along this boundary. Multiple detention/storm water quality facilities are planned for these corridors and will be constructed along with future land development. Portions of the Cathedral Pines Development to the south contributes developed flows to this property. These flows will be accommodated in the various on-site facility designs. A smaller on-site basin at the southeast corner of section 36 releases historic flows onto the Cathedral Pines and the Edmonds Subdivision. An on-site detention/storm water quality facility is planned in this corridor to help mitigate development.

East Cherry Creek Drainage Basin

The Palmer Divide traverses the eastern half of section 36 which defines the major basin line between the Black Squirrel Creek and the East Cherry Creek Basins. The vegetation also changes drastically in this area. The majority of the East Cherry Creek Basin contains very little trees and more grazing prairie land and meadows. This area defines the edge of Black Forest. In general, historic flow patterns in this basin travel in a northeasterly direction towards Hodgen Road. The MDDP designates several major design points along the north boundary. Again, multiple detention/storm water quality facilities are planned for these corridors and to be constructed along with future land development. Portions of the Palmer Divide Subdivision and multiple large unplatted properties the south contribute developed flows to this property. These flows will be accommodated in the various on-site facility designs.

PROPOSED DRAINAGE CONDITIONS

The proposed land development within the Flying Horse North Filing No. 1 and future development within the remaining portions of the Preliminary Plan will be 2.5-5 acre large lot residential with associated paved streets and roadside ditches. The 18-hole private Golf Course with a club house site, driving range and maintenance facility is also planned as a part of Filing No. 1. Based on the current El Paso County ECM Section I.7.1.B. and given the size of the lots within this entire development area, stormwater quality is not required to be provided. However, detention/EURV will still be provided in specific locations on-site to limit the on-site development flow release to remain consistent with pre-development conditions within the major drainage corridors. These proposed facilities will aide in limiting any detrimental effects on downstream corridors. At specific areas where the Filing No. 1 development creates concentrated flows into future development areas, temporary sediment basins will be constructed to minimize sediment transfer

downstream and off-site. The Filing No. 1 Final Drainage Report portion of this report will define the permanent facilities providing an Excess Urban Runoff Volume (EURV) in the lower portion of the facility storage volume with an outlet control device. Frequent and infrequent inflows are released at rates approximating undeveloped conditions. This concept provides some mitigation of increased runoff volume by releasing a portion of the increased runoff at a low rate over an extended period of time, up to 72 hours. This means that frequent storms, smaller than the 2 year event, will be reduced to very low flows near or below the sediment carrying threshold value for downstream drainage ways. Also, by incorporating an outlet structure that limits the 100-year runoff to the undeveloped condition rate, the discharge hydrograph for storms between the 2 year and the 100 year event will approximate the hydrograph for the undeveloped conditions and will help effectively mitigate the effects of this development. Again, prior to any land development beyond the Filing No. 1 Final Plat area, additional final drainage reports, final plats and construction plans will be required detailing this criteria.

The following hydrology descriptions will start at the western edge of the Flying Horse North property and move east into the East Cherry Creek Basin, describing the development within the Filing No. 1 area first.

FLYING HORSE NORTH FILING NO. 1

Black Squirrel Creek Drainage Basin

As mentioned previously, Flying Horse North is located in the upper region of the Black Squirrel Creek Drainage Basin. Per the approved DBPS for Black Squirrel Creek, the reaches in this area were proposed to remain as natural as possible. There were no recommendations for detention facilities within the area that is Flying Horse North, but due to current drainage criteria, detention/EURV facilities will be proposed with this development.

High Forest Ranch Detention Pond 26 outfalls onto the property at the very northwest corner of the site. These existing flows will continue to enter the site and travel within the natural channel towards the existing 48" CMP culvert crossing at Hwy. 83. Drainage easements across the proposed lots in this area will be provided on the final plat. The existing stock pond within lots 2 and 3 will be removed with grading of the road in this area. Tract B is platted in order to provide a detention/EURV facility for the lots and public road in this area. This facility will be constructed with Filing No. 1 with ownership and maintenance by the Flying Horse North HOA.

Design Point 1 (Q_2 = 2 cfs Q_5 = 3 cfs, Q_{100} = 11 cfs) represents the existing off-site and on-site developed flows from Basins OS-1A and BS-2B. The combined flow from these basins travel to a low point just east of Stagecoach Road where a proposed 24" RCP culvert will be installed to convey these flows under the road. (See Appendix for culvert design)

Design Point 2 (Q_2 = 3 cfs Q_5 = 9 cfs, Q_{100} = 35 cfs) represents flows from DP 1 and Basin BS-4. These combined flows are collected at a low point where a proposed 30" RCP culvert will be installed to replace the temporary sediment basin installed with early grading. (See Appendix for culvert design)

The total developed flows entering **Detention Facility 1** including Basin BS-1A equal **(Q_2 = 4 cfs Q_5 = 7 cfs, Q_{100} = 38 cfs)**. These combined flows will travel in the natural drainage corridor across lot 1 within a drainage easement and enter the detention facility. The following describes the design of this facility (See Appendix for UD Detention pond design sheets):

Detention Pond 1 (Full Spectrum – see multiple storm release data below)
0.43 Ac.-ft. EURV required
0.50 Ac.-ft. EURV design with 3:1 max. slopes
1.1 Ac.-ft. 100-yr. Storage

Total In-flow:	**Q_2 = 4 cfs,**	**Q_5 = 7 cfs,**	**Q_{100} = 38 cfs**
Pond Design Release:	**Q_2 = 0.1 cfs,**	**Q_5 = 0.2 cfs,**	**Q_{100} = 21 cfs**
Pre-development Release:	**Q_2 = 0.2 cfs,**	**Q_5 = 0.4 cfs,**	**Q_{100} = 23 cfs**

(Ownership and maintenance by the Flying Horse North HOA)

The downstream corridor from this proposed facility shows no indication of erosion at this time and is anticipated to continue to adequately handle the detained developed flows from this portion of the subdivision.

Design Point 4 (Q_2 = 3 cfs Q_5 = 4 cfs, Q_{100} = 8 cfs) represents existing and developed flows from Basin BS-2 (north side of Stagecoach Rd.) These flows will travel in a side road ditch towards Hwy. 83. A temporary sediment basin will be installed during construction of this portion of the roadway. This

development will be required to provide improvements to this intersection and Hwy. 83 per the site traffic study. Upon review/approval from CDOT, these improvements will be constructed along with final design of drainage at this intersection which will include the relocation of the dual 18" ERCP culverts and the removal of the temporary sediment basin.

Design Point 5 (Q_2 = 2 cfs Q_5 = 4 cfs, Q_{100} = 13 cfs) represents existing and developed flows from Basins OS-1B and BS-2A (south side of Stagecoach Rd.) These flows will travel in a side road ditch towards Hwy. 83. A temporary sediment basin will be installed during construction of this portion of the roadway. Upon review/approval from CDOT, these improvements will be constructed along with final design of drainage at this intersection and the removal of the temporary sediment basin at this location.

Design Point 6 (Q_2 = 1 cfs Q_5 = 3 cfs, Q_{100} = 15 cfs) represents flows from Basins OS-2 and BS-3. These combined flows travel via the side road ditch along the east side of the road and then around the cul-de-sac, through lot 3 within a drainage easement towards the existing natural channel to the west. These flows then combine with Basin BS-1B and continue to travel in the existing natural channel towards the existing downstream 48" CMP culvert. **Design Point 3 (Q_2 = 1 cfs Q_5 = 6 cfs, Q_{100} = 39 cfs)** then represents the total flow from this site leaving the property at this location. The pre-development on-site flow at this location equals **Q_2 = 1 cfs Q_5 = 5 cfs, Q_{100} = 41 cfs.** Thus, the downstream facilities will not see a significant change in flows.

Design Point 7 (Q_2 = 2 cfs Q_5 = 8 cfs, Q_{100} = 38 cfs) represents existing and developed flows from Basins OS-3 and BS-5. These flows will travel as sheet flow towards the low point where dual 30" RCP culverts will installed under Stagecoach Road to replace the temporary sediment basin installed with early grading. (See Appendix for culvert design)

High Forest Ranch Detention Pond 16 outfalls onto the property just upstream of Design Point 8. These existing flows will continue to enter the site and travel through proposed triple 48" RCP culverts under Stagecoach Road. (See Appendix for culvert design) **Design Point 8 (Q_2 = 21 cfs Q_5 = 70 cfs, Q_{100} = 284 cfs)** represents the existing and developed flows exiting the property and continuing south within the natural channel on the Shamrock Ranch property. These flows remain consistent with the historic flows at this location.

Design Point 9 (Q_2 = 1 cfs Q_5 = 5 cfs, Q_{100} = 23 cfs) represents existing flows from Basins OS-7 and BS-12. These combined flows are collected at a low point where proposed dual 24" RCP culverts will be installed to replace the temporary sediment basin installed with early grading. (See Appendix for culvert design) **Design Point 10 (Q_2 = 11 cfs Q_5 = 32 cfs, Q_{100} = 143 cfs)** represents existing and developed flows from Basins OS-8, OS-10, OS-11, BS-13 and BS-14. These flows will travel to the low point at this location where dual 42" RCP culverts will be installed for the crossing of Stagecoach Road. (See Appendix for culvert design)

Design Point 11 (Q_2 = 5 cfs Q_5 = 12 cfs, Q_{100} = 36 cfs) represents developed flow from Basin BS-16. These flows will travel to the low point at this location where dual 24" RCP culverts will be installed for the crossing of the road. (See Appendix for culvert design) **Design Point 12 (Q_2 = 4 cfs Q_5 = 11 cfs, Q_{100} = 44 cfs)** represents the combined developed flow from Basins BS-16 and BS-15. These flows will travel to the low point at this location where a 36" RCP culvert and storm system will be installed to route the collected flows directly into Detention Pond 4 at the south end. (See Appendix for culvert design)

The total developed flows entering **Detention Facility 4**, including Basins OS-9 and BS-17 equal **(Q_2 = 10 cfs Q_5 = 16 cfs, Q_{100} = 217 cfs)**. The major flows enter the facility at the north end through a rock chute. (See Appendix for rock chute and pond design) The following describes the design of this facility:
(See Appendix for UD Detention pond design sheets):

Detention Pond 4 (Full Spectrum – see multiple storm release data below)
0.99 Ac.-ft. EURV required
1.05 Ac.-ft. EURV design with 4:1 max. slopes
5.1 Ac.-ft. 100-yr. Storage

Total In-flow:	**Q_2 = 10 cfs,**	**Q_5 = 16 cfs,**	**Q_{100} = 217 cfs**
Pond Design Release:	**Q_2 = 0.3 cfs,**	**Q_5 = 0.4 cfs,**	**Q_{100} = 142 cfs**
Pre-development Release:	**Q_2 = 1.5 cfs,**	**Q_5 = 2.5 cfs,**	**Q_{100} = 152cfs**

(Ownership and maintenance by the Flying Horse North HOA)

The downstream corridor from this proposed facility shows little indication of erosion at this time and is anticipated to continue to adequately handle the detained developed flows from this portion of the subdivision. In addition, we have been coordinating with the adjacent property owner and his engineering

consultant on this specific corridor and will continue to do so until the on-site detention facility construction is complete and all disturbed areas are re-established.

Design Point 14 (Q_2 = 4 cfs Q_5 = 12 cfs, Q_{100} = 56 cfs) represents the developed flow from Basin BS-18. These flows will travel to the low point at this location where three 24" RCP culverts will be installed to cross under the road. (See Appendix for culvert design) These flows then enter Basin OS-23 through a drainage easement on the rear of lot 65 and continue to travel towards DP-16. **Design Point 15 (Q_2 = 2 cfs Q_5 = 5 cfs, Q_{100} = 15 cfs)** represents the developed flow from Basin BS-19. These flows will travel to the low point at this location where dual 18" RCP culverts will be installed to cross under the road. (See Appendix for culvert design) These flows then enter Basin OS-22 through a drainage easement across lots 60 and 61 and continue to travel towards the golf course.

Basins BS-20, BS-21, BS-22, BS-23 and BS-23A are relatively large basins that contain both Filing 1 lots, much of the golf course but also future lots that will remain undeveloped at this time. However, these basins all ultimately travel in a southwesterly direction towards the proposed Detention Facility 8. This report analyzes both the "Filing 1 Only" condition as well as the "full build-out condition" in the design of this detention facility. With the development of Filing 1, Detention Facility 8 will be sized and graded for the ultimate design accounting for the future lot development. The outlet structure and emergency overflow weir will also be designed for the ultimate condition. However, we will provide two orifice plate designs for the outlet box. The initial plate will be constructed that will handle the proper release for the Filing 1 development only. Upon the next phase of lot development within these basins (BS-20, BS-21, BS-22, BS-23 and BS-23A) the ultimate plate will be installed to replace this initial plate design. No further changes to the outlet structure or pond will need to take place. Thus, the following describes the two scenarios:

Full Build-out Design (accounting for future lot development)

Design Point 16 (Q_2 = 25 cfs Q_5 = 78 cfs, Q_{100} = 362 cfs) represents the total developed flows from Basins BS-18 thru BS-23 with the fully developed golf course and lots in Filing 1 and the future phases. These flows travel to the low point at this location where dual 60" RCP culverts will be installed to cross under the road. (See Appendix for culvert design) These flows represent the major portion of the flows entering Detention Facility 8 with the remaining flows coming from Basin OS-23A.

The total developed flow entering **Detention Facility 4** includes Basin BS-23A. The following describes the design of this facility: (See Appendix for UD Detention pond – **Full Build-out** design sheets):

Detention Pond 8 (Full Spectrum – see multiple storm release data below)
2.40 Ac.-ft. EURV required
2.45 Ac.-ft. EURV design with 4:1 max. slopes
9.32 Ac.-ft. 100-yr. Storage
Total In-flow: Q_2 **= 28 cfs,** Q_5 **= 85 cfs,** Q_{100} **= 383 cfs**
Pond Design Release: Q_2 **= 0.8 cfs,** Q_5 **= 1.0 cfs,** Q_{100} **= 253 cfs**
Pre-development Release: Q_2 **= 2.6 cfs,** Q_5 **= 4.5 cfs,** Q_{100} **= 274cfs**
(Ownership and maintenance by the Flying Horse North HOA)

Filing 1 Only Design (accounting for golf course and Filing 1 lot development)

Under this scenario, only the golf course and Filing 1 lots are developed and Basins BS-20, BS-21, BS-22, BS-23 and BS-23A have been adjusted to account for only this initial phase of development. The following describes the facility requirements for this design: (See Appendix for UD Detention pond – **Filing 1 Only** design sheets):

****Detention Pond 8 (Full Spectrum – see multiple storm release data below)**
1.13 Ac.-ft. EURV required
2.45 Ac.-ft. EURV design with 4:1 max. slopes
7.76 Ac.-ft. 100-yr. Storage
Total In-flow: Q_2 **= 9 cfs,** Q_5 **= 14 cfs,** Q_{100} **= 301 cfs**
Pond Design Release: Q_2 **= 0.4 cfs,** Q_5 **= 0.5 cfs,** Q_{100} **= 219 cfs**
Pre-development Release: Q_2 **= 2.2 cfs,** Q_5 **= 3.9 cfs,** Q_{100} **= 237cfs**
(Ownership and maintenance by the Flying Horse North HOA)

**Please note that all facility design remains the same as the Full Build-out scenario except for the different orifice plate.

The downstream corridor from this proposed facility shows little indication of erosion at this time and is anticipated to continue to adequately handle the detained developed flows from this portion of the subdivision. In addition, we have been coordinating with the adjacent property owner and his engineering consultant on this specific corridor and will continue to do so until the on-site detention facility construction is complete and all disturbed areas are re-established.

Basin BS-24 (Q_2 = 0.6 cfs Q_5 = 3 cfs, Q_{100} = 18 cfs) represents sheet flow from three residential lots within Filing No. 1 that will continue to direct release off-site. However, portions of this historic basin area will be routed into Flying Horse North Pond 8, therefore the developed flows from this basin do not significantly change from the pre-development condition. The pre-development flows from the historic basin area equal **Q_2 = 0.2 cfs Q_5 = 2 cfs, Q_{100} = 18 cfs.** Also, given the lot size, no water quality is required.

FLYING HORSE NORTH FILING NO. 1

East Cherry Creek Drainage Basin

The following basins are still tributary to the Filing No. 1 platting area but are within the East Chery Creek Drainage Basin:

Design Point 24 (Q_2 = 2 cfs Q_5 = 8 cfs, Q_{100} = 45 cfs) represents developed flows from Basins CC-4C and CC-5. Basin CC-4C represents the future golf course clubhouse site. Upon future development of this site a site specific detention/SWQ facility will be installed. This future facility will release into the side road ditch on the west side of Allen Ranch Road and travel in a northerly direction. The side road ditch along this stretch of Allen Ranch Road and the south side of Old Stagecoach Road will be sized to handle these flows. The 100-yr. emergency overflow from this future facility will also be into the side road ditch of Allen Ranch Road and not towards any residential lots. For ultimate downstream design purposes this basin is assumed to release pre-development flows. These flows will travel towards Design Point 24 where a 36" RCP culvert is sized to handle the fully developed flows at this location. (See Appendix for culvert design)

The total developed flows entering **Detention Facility 12**, including Basin CC-6 equals **(Q_2 = 6 cfs Q_5 = 9 cfs, Q_{100} = 85 cfs)**. The following describes the design of this facility:
(See Appendix for UD Detention pond design sheets):

Detention Pond 12 (Full Spectrum – see multiple storm release data below)
0.66 Ac.-ft. EURV required
0.75 Ac.-ft. EURV design with 4:1 max. slopes
2.69 Ac.-ft. 100-yr. Storage

Total In-flow:	**Q_2 = 6 cfs, Q_5 = 9 cfs, Q_{100} = 85 cfs**
Pond Design Release:	**Q_2 = 0.2 cfs, Q_5 = 0.3 cfs, Q_{100} = 45 cfs**
Pre-development Release:	**Q_2 = 0.5 cfs, Q_5 = 0.9 cfs, Q_{100} = 55 cfs**

(Ownership and maintenance by the Flying Horse North HOA)

The downstream corridor from this proposed facility shows no indication of erosion at this time and is anticipated to continue to adequately handle the detained developed flows from this portion of the subdivision.

Design Point 25 (Q_2 = 0.2 cfs Q_5 = 0.3cfs, Q_{100} = 45 cfs) then represents the total flow leaving the site at this location. The pre-development flow at this location equals **Q_2 = 0.5 cfs Q_5 = 0.9 cfs, Q_{100} = 55 cfs.** Thus, the downstream facilities will not see a significant change in flows.

Basins OS-12, OS-13, OS-14, CC-1A, CC-1B, CC-2A, CC-2B, CC-2C, CC-3, CC-4A, CC-4B and CC-9 are all tributary to the proposed Flying Horse North Pond 13. Nearly all the proposed residential lots within these basins are part of future development outside of Filing No. 1 platting. The only structure associated with Filing No. 1 development is the pond embankment/outlet structure crossing Stagecoach Road. However, this facility has been classified by the Colorado Dam Safety Branch (DSB) as a low-hazard, jurisdictional facility. As such, a separate Design Report including hydrology/hydraulic design and embankment/structure design has been prepared for DSB and El Paso County review and approval. Please reference this report for the required detention/SWQ design for this facility.

Design Point 26 (Q_2 = 3 cfs Q_5 = 16 cfs, Q_{100} = 102 cfs) represents the full build-out developed flows from Basins CC-8 and CC-10. Basin CC-8 represents future residential lots and CC-10 mostly future

passive park area. These flows will continue to sheet flow towards the low-point where a 48" RCP culvert is sized to handle the fully developed flows at this location. (See Appendix for culvert design) After crossing Stagecoach Road, these flows will continue to flow directly into the existing stock pond just north of the roadway. This facility will provided sediment control for the small developed roadway area. Upon future development and plating of the lots planned within these basins, this stock pond will be formally designed into a detention facility.

Basin CC-15 (Q_2 = 1 cfs Q_5 = 4 cfs, Q_{100} = 20 cfs) represents the full build-out developed flows from the future residential lots tributary to this basin. These flows will continue to sheet flow towards the low-point where a 30" RCP culvert is sized to handle the fully developed flows at this location. (See Appendix for culvert design) A downstream sediment basin will be installed to provide sediment control for the small developed roadway area.

Basin CC-16 (Q_2 = 1 cfs Q_5 = 5 cfs, Q_{100} = 24 cfs) represents the full build-out developed flows from the future residential lots tributary to this basin. These flows will continue to sheet flow towards the low-point at the southwest corner of Old Stagecoach Road and Rubble Drive where a 24" RCP culvert is sized to handle the fully developed flows at this location. (See Appendix for culvert design) A downstream sediment basin will be installed to provide sediment control for the small developed roadway area.

Design Point 30 (Q_2 = 0.7 cfs Q_5 = 2 cfs, Q_{100} = 10 cfs) represents the full build-out developed flows from Basin CC-18. This Basin represents future residential lots. The flows will continue to sheet flow towards the low-point where a 24" RCP culvert is sized to handle the fully developed flows at this location. (See Appendix for culvert design) A downstream sediment basin will be installed to provide sediment control for the small developed roadway area.

Design Point 31 (Q_2 = 0.9 cfs Q_5 = 3 cfs, Q_{100} = 15 cfs) represents the full build-out developed flows from Basin CC-19 and the upstream release from DP-30. This Basin represents future residential 5 ac. lots. The flows will continue to sheet flow within a proposed drainage easement towards the existing low-point where an existing 24" CMP culvert will adequately handle the fully developed flows at this location.

Design Point 32 (Q_2 = 2 cfs Q_5 = 8 cfs, Q_{100} = 40 cfs) represents the full build-out developed flows from Basins OS-16 and CC-17. Basin CC-17 represents future residential lots and OS-16 unplatted, 5-ac. zoned residential property. These flows will continue to sheet flow towards the low-point where a 36" RCP culvert is sized to handle the fully developed flows at this location. (See Appendix for culvert design) A downstream sediment basin will be installed to provide sediment control for the small developed roadway area.

FLYING HORSE NORTH PRELIMINARY PLAN (Future Platting)

Black Squirrel Creek Drainage Basin

The following basins are in the Black Squirrel Creek Drainage Basin but are not a part of Filing 1 lot development. These areas will require future final drainage report(s) upon future lot development.

Design Point 18 (Q_2 = 5 cfs Q_5 = 22 cfs, Q_{100} = 115 cfs) represents developed flows from Basins BS-28, BS-29, BS-30 and OS-18. Portions of basins BS-28 and BS-29 include golf course development taking place with Filing No. 1. However, the majority of these basins include forested future residential lots with basin OS-18 being existing 2.5 ac. minimum lots. Future developed flows will be routed to this location where a future detention facility will be installed. This facility will be sized to meet EURV requirements and release pre-development flow quantities into the future side road ditch and through future drainage easements towards the future detention facility at Design Point 19. These future facilities will be further analyzed as ponds in series and emergency overflow paths well defined with the future final drainage report. In the interim, with only the golf course construction, a temporary sediment basin located within the future roadway in basin BS-28 will be installed to provide sediment control from the developed golf course area.

Design Point 19 (Q_2 = 4 cfs Q_5 = 17 cfs, Q_{100} = 126 cfs) represents developed flows from Basins BS-27 and OS-17. These basins include forested future residential lots with basin OS-17 being existing 2.5 ac. minimum lots. Future developed flows will be routed to this location where a future detention facility will be installed to meet EURV requirements and release pre-development flow quantities. Both of these future facilities will be constructed in tracts with ownership and maintenance by the Flying Horse North HOA.

Basin BS-26 (Q_2 = 0.04 cfs Q_5 = 0.4 cfs, Q_{100} = 3 cfs) represents sheet flow from the extreme rear portion of a future residential lot. This area of the lot will likely not be built upon, therefore not significantly changing the drainage conditions from the pre-development condition. The pre-development flow from the historic basin area equals **Q_2 = 0.04 cfs Q_5 = 0.4 cfs, Q_{100} = 3 cfs.** Also, given the lot size, no water quality is required.

Basins BS-31 (Q_2 = 0.3 cfs Q_5 = 2 cfs, Q_{100} = 12 cfs), BS-32 (Q_2 = 0.3 cfs Q_5 = 2 cfs, Q_{100} = 9 cfs) and BS-33 (Q_2 = 0.8 cfs Q_5 = 3 cfs, Q_{100} = 15 cfs) represent smaller basins that will continue to sheet flow off-site to the south. These basins represent some golf course development and multiple future residential lots. Given the lot size, no water quality is required. However, permanent sediment basins will be installed downstream of the golf course development to provide sediment control. Developed flows released from these basins will not be significantly different than the pre-development flows.

East Cherry Creek Drainage Basin

The following basins are not tributary to the Filing No. 1 platting area but are within the East Chery Creek Drainage Basin and planned for future residential lot development.

Design Point 28 (Q_2 = 5 cfs Q_5 = 20 cfs, Q_{100} = 110 cfs) represents the full build-out developed flows from Basins OS-13 and CC-13A. Basin CC-13A represents future residential lots and OS-13 platted, 5-ac. zoned residential property. These flows will continue to sheet flow towards the low-point where a future culvert will be installed to handle the fully developed flows at this location. The flows are then conveyed in the natural channel towards Design Point 29.

Design Point 29 (Q_2 = 6 cfs Q_5 = 27 cfs, Q_{100} = 155 cfs) represents the full build-out developed flows from Basins CC-13B, CC-13C and release from DP-28. These basins represent future residential lots. At this location, a future detention facility will be installed to meet EURV requirements and release pre-development flow quantities. This future facility will be constructed in a tract with ownership and maintenance by the Flying Horse North HOA.

Basin CC-13D (Q_2 = 2 cfs Q_5 = 6 cfs, Q_{100} = 29 cfs) represents future residential lots that will continue to sheet flow off-site. Given the lot size, no water quality is required. However, a permanent sediment basin will be installed just prior to release off-site to provide sediment control. Developed flows released from this basin will not be significantly different than the pre-development flows.

Basin CC-14 (Q_2 = 0.4 cfs Q_5 = 2 cfs, Q_{100} = 8 cfs) represents sheet flow from the rear portion of two future residential lots. The majority of this area is not anticipated to be developed, therefore not significantly changing the drainage conditions from the pre-development condition. Also, given the lot size, no water quality is required.

Design Point 27 (Q_2 = 4 cfs Q_5 = 17 cfs, Q_{100} = 81 cfs) represents the full build-out developed flows from the previously described basin CC-15 and CC-20. These basins represent future residential lots. At this location, a future detention facility will be installed to meet EURV requirements and release pre-development flow quantities. This future facility will be constructed in a tract with ownership and maintenance by the Flying Horse North HOA.

Basins CC-21 (Q_2 = 0.1 cfs Q_5 = 1 cfs, Q_{100} = 9 cfs) and CC-22 (Q_2 = 1 cfs Q_5 = 5 cfs, Q_{100} = 21 cfs) represent future residential 5 ac. lots and park area that will continue to sheet flow off-site. Given the lot size, no water quality is required. However, a permanent sediment basin will be installed just prior to release off-site to provide sediment control. Developed flows released from this basin will not be significantly different than the pre-development flows.

Basins CC-23 (Q_2 = 0.4 cfs Q_5 = 1 cfs, Q_{100} = 8 cfs) and CC-24 (Q_2 = 3 cfs Q_5 = 13 cfs, Q_{100} = 62 cfs) represent future 5 ac. residential lots that will continue to sheet flow off-site. Given the lot size, no water quality is required. Given that the proposed lots are planned for 5 ac. residential, the developed flows released from this basin will not be significantly different than the pre-development flows. However, multiple permanent sediment basins may be installed just prior to release off-site to provide sediment control. This basin also contains a portion of the adjacent Franktown/Parker Reservoir emergency spillway crossing two proposed lots. This existing facility, which doesn't appear to be within any existing easement, will be further analyzed with a final drainage report for this area. Appropriate drainage easements may be provided at time of final plating.

Basin CC-25 (Q_2 = 0.3 cfs Q_5 = 1 cfs, Q_{100} = 6 cfs) represents a small portion of two future residential 5 ac. lots that will continue to sheet flow off-site. Given that the proposed lots are planned for 5 ac. residential, the developed flows released from this basin will not be significantly different than the pre-development flows.

Design Point 34 (Q_2 = 6 cfs Q_5 = 24 cfs, Q_{100} = 168 cfs) represents the full build-out developed flows from Basins CC-26, CC-27, CC-28, release from CC-16 and release from DP-32. These basins represent future residential lots and park area. At this location, a future detention facility will be installed and likely replace the existing stock pond to meet EURV requirements and release pre-development flow quantities. The downstream existing culvert under Hodgen Road will be further analyzed with future final drainage reports. This future facility will be constructed in a tract with ownership and maintenance by the Flying Horse North HOA.

FACILITY MAINTENANCE

All proposed drainage structures within the platted County ROW will be owned and maintained by El Paso County. All proposed drainage structures within easements or tracts will be owned and maintained by the Flying Horse North HOA of Golf Course owner.

DRAINAGE CRITERIA

Hydrologic calculations were performed using the City of Colorado Springs/El Paso County Drainage Criteria Manual, revised in November 1991 and October 1994 with County adopted Chapter 6 and Section 3.2.1 of Chapter 13 of the City of Colorado Springs/El Paso County Drainage Criteria Manual as revised in May 2014. Detention storage and storm sewer conveyance to Black Squirrel Creek Drainage Basin was established with the Black Squirrel DBPS, previously referenced. The IDF curves from Figure 6-5 of the City of Colorado Springs/El Paso County DCM was used to estimate storm water runoff anticipated from design storms for the 2 year, 5 year and 100 year recurrence interval. (See Appendix)

Revise based on the 4-step process defined in Appendix I Section I.7.2.
Step 1: Employ Runoff Reduction Practices
Step 2: Stabilized Drainageways
Step 3: Provide WQCV
Step 4: Consider Need for Industrial and Commercial BMPs

County DCM requires the Four Step Process for receiving water off volumes, treating the water quality capture volume (WQCV), ing long-term source controls. The Four Step Process pertains to management of smaller, frequently occurring storm events, as opposed to larger storms for which drainage and flood control infrastructure are sized. Implementation of these four steps helps to achieve storm water permit requirements. This site adheres to this **Four Step Process** as follows:

1. **Employ Runoff Reduction Practices:** Development of project site is proposed large lot single family residential (2.5 ac. min.) with homes and associated landscaping along with a private golf course. Proposed impervious areas (roof tops, patios) will sheet flow across landscaped ground, through open space areas and across the golf course to slow runoff and increase time of concentration prior to being conveyed to the proposed public streets. This will minimize directly connected impervious areas within the project site.

2. **Implement BMP's that provide a Water Quality Capture Volume with slow release:** Runoff from this development will be treated through capture and slow release of the WQCV in multiple permanent Extended Detention Basins designed per current El Paso County drainage criteria.

3. **Stabilize Drainageways:** This site will utilize roadside ditches and culvert crossings throughout the site. These facilities will then direct the on-site development flows to the multiple detention/SWQ ponds mentioned above, designed to release at or below historic rates into Black Squirrel and East Cherry Creek. Based upon the proposed reduction in released flows compared to the pre-developed flows, no impact to downstream drainageways is anticipated.

4. **Implement Site Specific and Other Source Control BMP's**: A site specific storm water quality and erosion control plan and narrative was previously approved for this development in October 2016 (PUD-16-002). Details such as site specific source control construction BMP's as well as permanent BMP's were detailed in this plan and narrative to protect receiving waters. Much of these BMP's are currently constructed and being maintained as the majority of the development has been graded and erosion control methods employed.

FLOODPLAIN STATEMENT

A small portion of the Preliminary Plan (future lots not platted at this time) is located within a floodplain as determined by the Flood Insurance Rate Maps (F.I.R.M.) Map Number 08041C 0295F, 0841C 0315F, 04081C 0325F effective date, March 17, 1997 (See Appendix). However, no portion of property proposed to be platted with Filing No. 1 is within the floodplain.

DRAINAGE AND BRIDGE FEES

FLYING HORSE NORTH FILING NO. 1

The East Cherry Creek Basin does not currently have a Drainage Basin Fee. However, the following fees for the Filing No. 1 platted area within the Black Squirrel Creek Basin are due prior to platting:

The fees are calculated using the following impervious acreage method approved by El Paso County. The acreage for Flying Horse Filing No. 1 within the Black Squirrel Creek Basin is 342.7 acres. This total area is broken into two uses: 2.5 ac. lots (including roads and tracts) and golf course. The 2.5 ac. lot area equals 234.4 acres and the golf course area equals 108.3 acres. Thus, the percent imperviousness for this subdivision is calculated as follows (See Figure 1.1 for Basin Area Exhibit):

2.5 ac. lots (incl. roads and tracts)

(Per El Paso County Percent Impervious Chart: 11%)

234.4 Ac. x 11% = **25.78 Impervious Ac.**

25% reduction for 2.5 ac. lots

25.78 Imp. ac. x .75% **= 19.34 Impervious Ac.**

Golf Course Development

(Per El Paso County Percent Impervious Chart for greenbelts: 2%)

108.3 Ac. x 2% = **2.17 Impervious Ac.**

25% reduction for golf course development

2.17 Imp. ac. x .75% **= 1.62 Impervious Ac.**

Total Impervious Acreage for Filing 1: 20.96 Imp. Ac.

Since the applicant is requesting the reimbursement of 50% of construction costs, remove the 25% reduction.
Per ECM Appendix L Section 3.10.4a, both the 50% reimbursement and the 25% reduction cannot both be applied to the same development.

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The following calculations are based on the 2018 drainage/bridge fees for the Black Squirrel Creek Drainage Basin:

FILING 1 FEE TOTALS (prior to reduction):

Bridge Fees

$ 492.00 x 20.96 Impervious Ac. = $ 10,312.32

Drainage Fees

$ 7,808.00 x 20.96 Impervious Ac. = $ 163,655.68

Per the ECM 3.10.4a, this development requests a reduction of drainage fees based on the three on-site full spectrum detention/SWQ facilities proposed to be constructed with Filing 1. The following facilities within the Black Squirrel Creek basin seem to meet the criteria for this reduction:

Detention Pond 1	1.1 ac-ft. full spectrum	$ 24,448 x 50% =	$ 12,224.00
Detention Pond 4	4.5 ac-ft. full spectrum	$ 130,270 x 50% =	$ 65,135.00
Detention Pond 8	9.4 ac-ft. full spectrum	$ 111,320 x 50% =	$ 55,660.00
Total Reduction			$ 133,019.00

FILING 1 FEE TOTALS:

Bridge Fees

$ 492.00 x 20.96 Impervious Ac. = $ 10,312.32

Drainage Fees

$ 163,655.68 - 133,019.00 = $ 30,636.68

SUMMARY

This proposed development remains consistent with the previously approved Flying Horse North MDDP and Preliminary Drainage Report for Flying Horse North (Golf Course grading and private access roads). The proposed storm facilities have been sized to adequately handle the 100-yr. developed flows. All proposed detention facilities meet current criteria and provide full spectrum design. Upon future development outside of Filing No. 1, final drainage reports will be required finalizing final design of the proposed future drainage facilities. The proposed development will not adversely impact surrounding developments.

PREPARED BY:

Classic Consulting Engineers & Surveyors, LLC

Marc A. Whorton, P.E.
Project Manager

Maw/109611/reports/109611PDR.doc

REFERENCES

1. City of Colorado Springs/County of El Paso Drainage Criteria Manual, as revised in November 1991 and 1994 with County adopted Chapter 6 and Section 3.2.1 of Chapter 13 of the City of Colorado Springs/El Paso County Drainage Criteria Manual as revised in May 2014.

2. "Master Development Drainage Plan for Flying Horse North", Classic Consulting, dated September 2016.

3. "Preliminary Drainage Report for Flying Horse North (Golf Course Grading and Private Access Roads)", Classic Consulting, dated September 2016.

4. "Final Drainage Report High Forest Ranch Filing No. 1" JR Engineering, dated March 2001.

5. "Final Drainage Report for High Forest Ranch Filing No. 2 and High Forest Ranch Filing No. 3" Classic Consulting Engineers and Surveyors dated May 2001.

6. "Final Drainage Report and Plan for Cathedral Pines Subdivision Filing No. 2," Leigh Whitehead & Associates, dated March 2005.

7. "Final Drainage Report and Plan for Cathedral Pines Subdivision Filing No. 3," Stillwater Engineering, dated July 2006.

8. "Black Squirrel Creek Drainage Basin Planning Study," URS Corporation, dated August 1987.

9. "Final Drainage Report for Country View Estates" Associated Design Professionals Inc, dated October 1998.

10. "Urban Storm Drainage Criteria Manual Volume 1, 2 & 3" Urban Drainage and Flood Control District, dated January 2016.

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APPENDIX

VICINITY MAP

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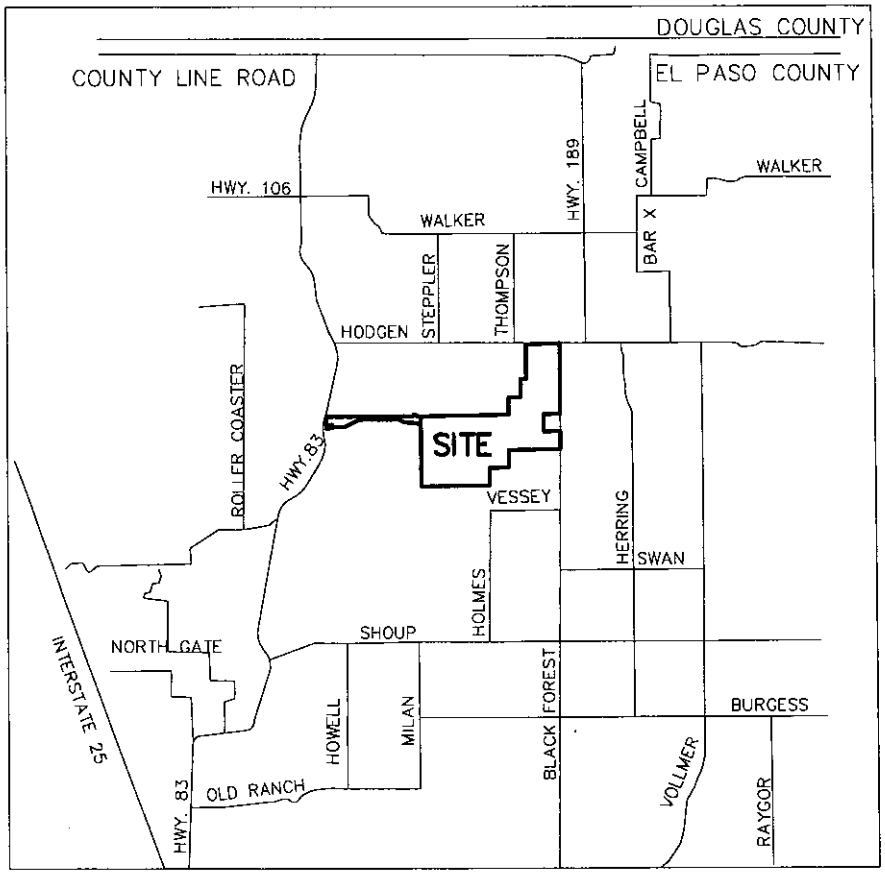

VICINITY MAP

NTS

SOILS MAP (WEB SOIL SURVEY)

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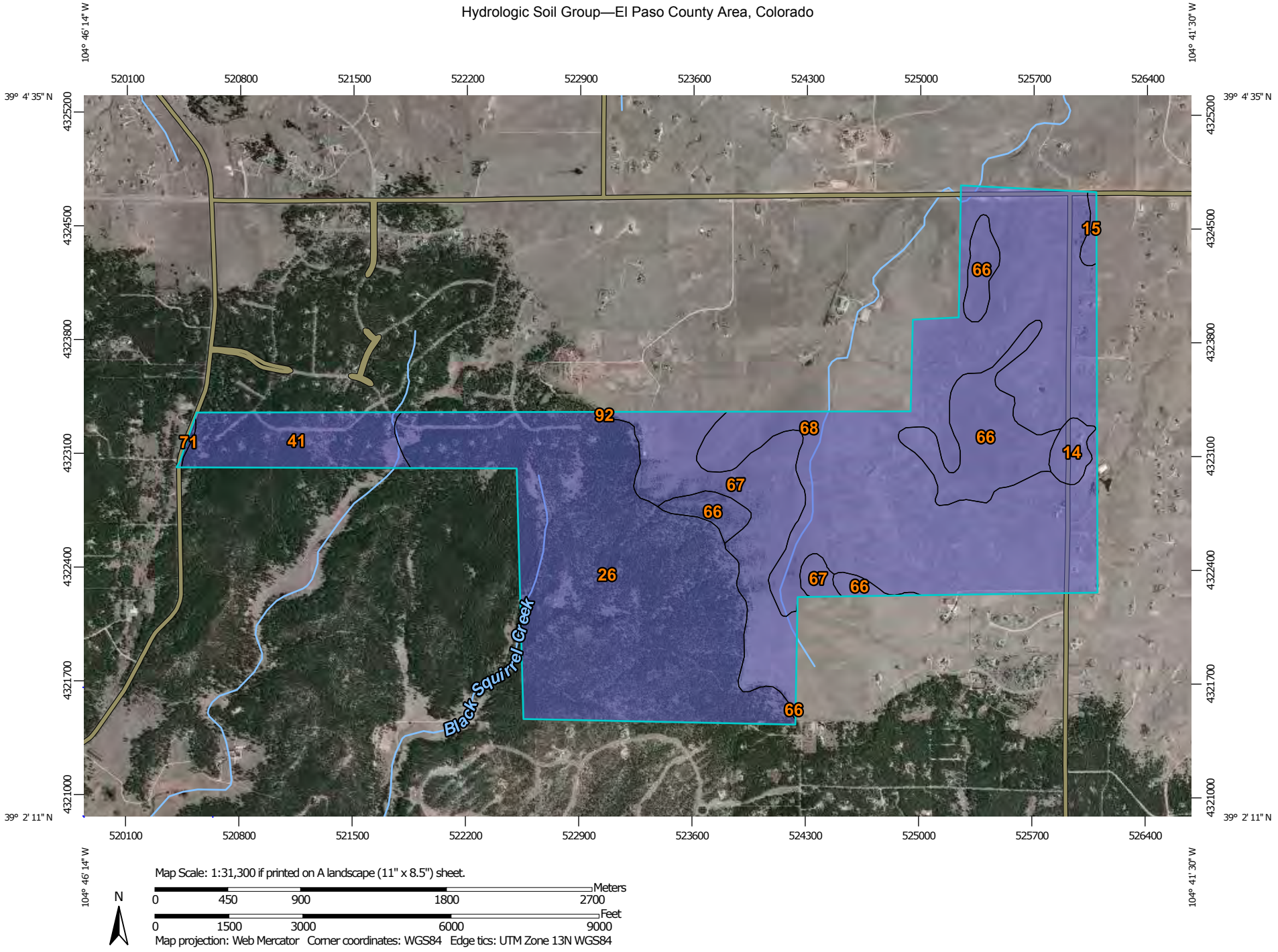
104° 46' 14" W
104° 41' 30" W
39° 4' 35" N
39° 2' 11" N
520100
520800
521500
522200
522900
523600
524300
525000
525700
526400
4325200
4324500
4323800
4323100
4322400
4321700
4321000
92
71
41
68
66
15
14
67
26
Black Squirrel Creek
Map Scale: 1:31,300 if printed on A landscape (11" x 8.5") sheet.
N
Meters
0 450 900 1800 2700
Feet
0 1500 3000 6000 9000
Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

Soil Rating Polygons

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
Survey Area Data: Version 13, Sep 22, 2015

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Apr 15, 2011—Sep 22, 2011

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Hydrologic Soil Group— Summary by Map Unit — El Paso County Area, Colorado (CO625)				
Map unit symbol	**Map unit name**	**Rating**	**Acres in AOI**	**Percent of AOI**
14	Brussett loam, 1 to 3 percent slopes	B	19.6	1.1%
15	Brussett loam, 3 to 5 percent slopes	B	7.0	0.4%
26	Elbeth sandy loam, 8 to 15 percent slopes	B	615.7	33.6%
41	Kettle gravelly loamy sand, 8 to 40 percent slopes	B	109.3	6.0%
66	Peyton sandy loam, 1 to 5 percent slopes	B	160.6	8.8%
67	Peyton sandy loam, 5 to 9 percent slopes	B	198.8	10.8%
68	Peyton-Pring complex, 3 to 8 percent slopes	B	719.7	39.3%
71	Pring coarse sandy loam, 3 to 8 percent slopes	B	1.7	0.1%
92	Tomah-Crowfoot loamy sands, 3 to 8 percent slopes	B	0.8	0.0%
Totals for Area of Interest			**1,833.2**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: *None Specified*

Tie-break Rule: Higher

F.E.M.A. MAP

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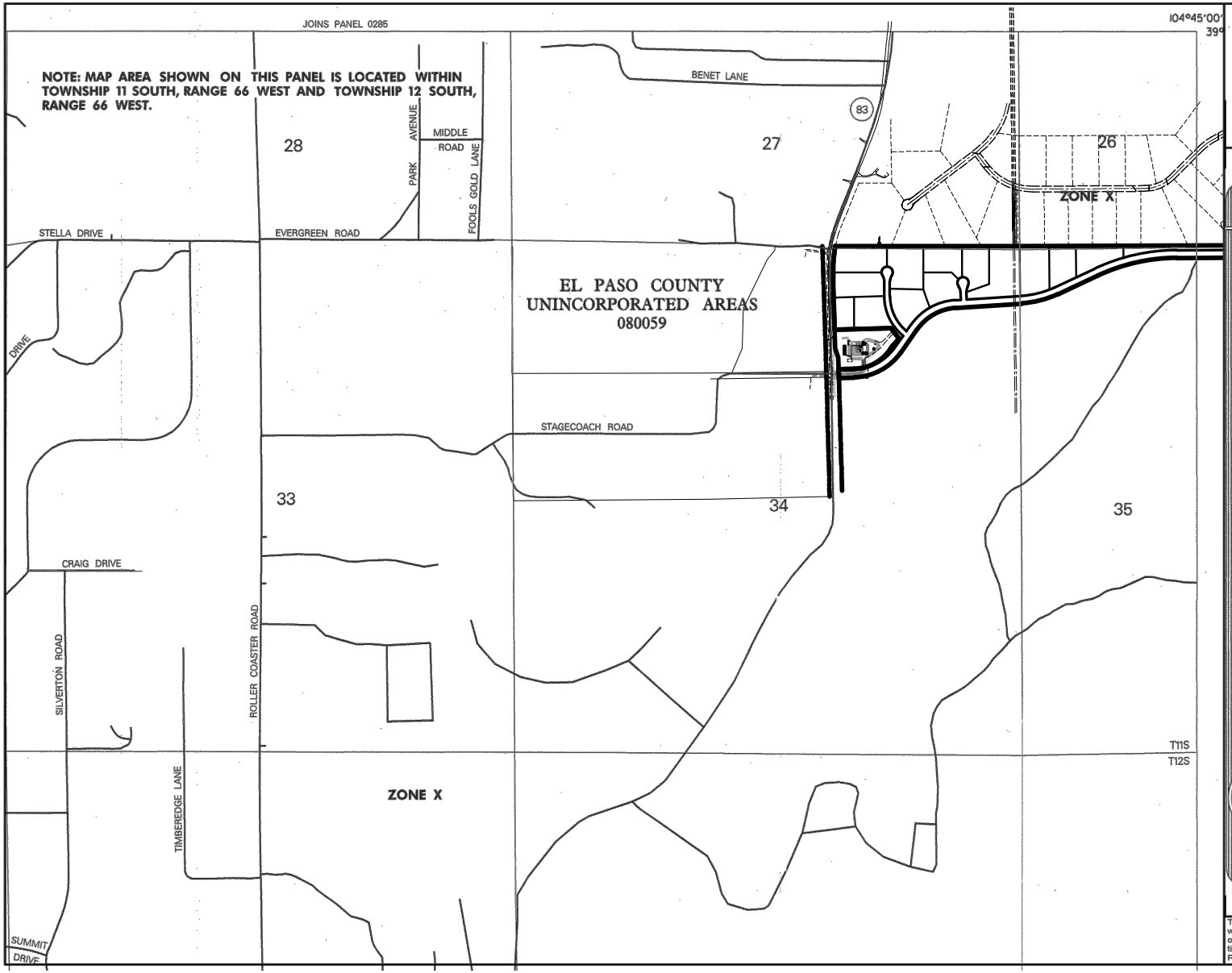


APPROXIMATE SCALE IN FEET
1000 0 1000

NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP

EL PASO COUNTY, COLORADO AND INCORPORATED AREAS

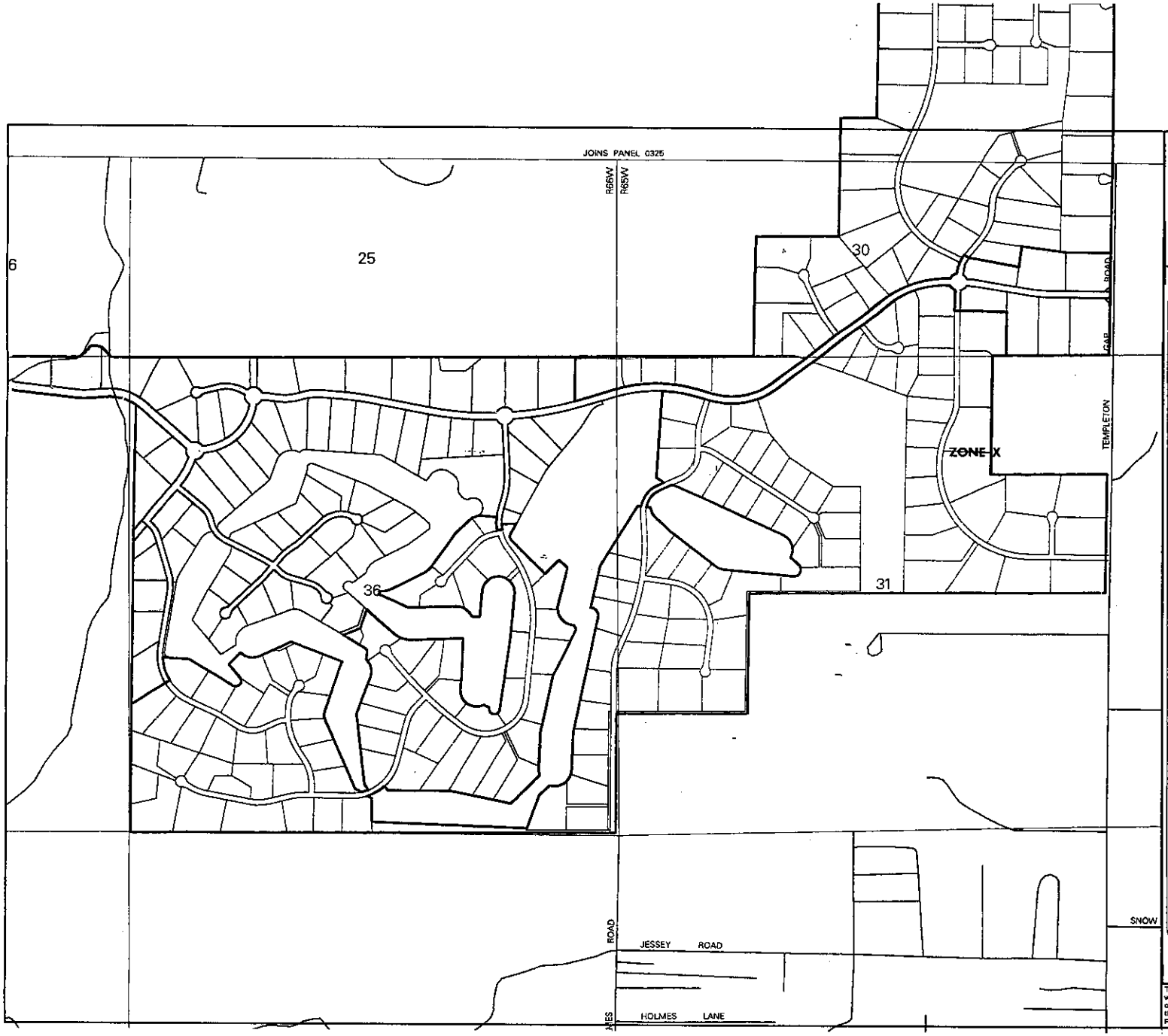
PANEL 295 OF 1300
(SEE MAP INDEX FOR PANELS NOT PRINTED)

CONTAINS: COMMUNITY	NUMBER	PANEL	SUFFIX
COLORADO SPRINGS, CITY OF	080060	0295	F
EL PASO COUNTY, UNINCORPORATED AREAS	080059	0295	F

MAP NUMBER
08041C0295 F

EFFECTIVE DATE:
MARCH 17, 1997

Federal Emergency Management Agency



APPROXIMATE SCALE IN FEET
1000 0 1000

NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP

EL PASO COUNTY,
COLORADO AND
INCORPORATED AREAS

PANEL 315 OF 1300
(SEE MAP INDEX FOR PANELS NOT PRINTED)

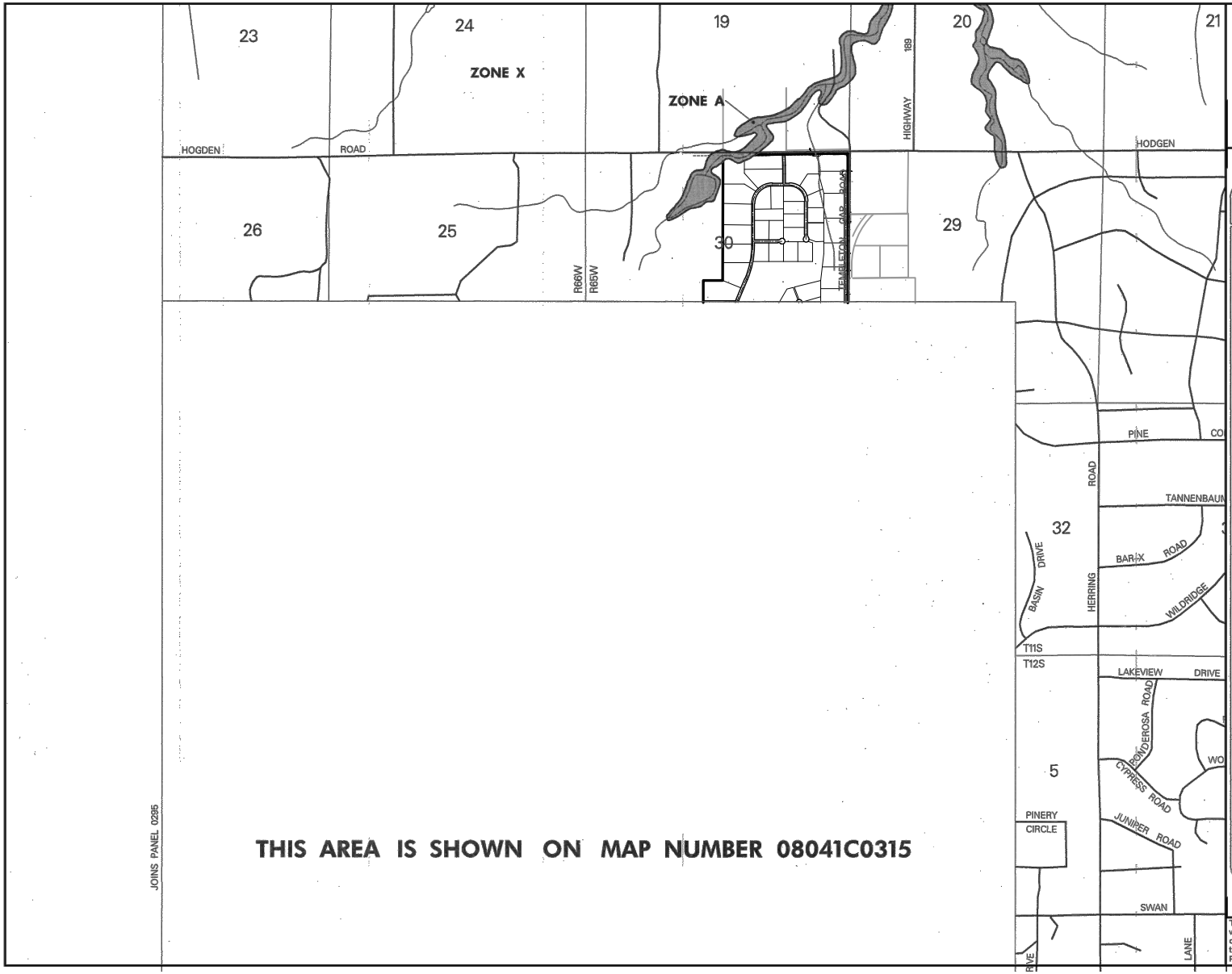
CONTAINS:
COMMUNITY NUMBER PANEL SUFFIX

EL PASO COUNTY, UNINCORPORATED AREAS 080059 0315 F

MAP NUMBER
08041C0315 F

EFFECTIVE DATE:
MARCH 17, 1997

Federal Emergency Management Agency



APPROXIMATE SCALE IN FEET

2000 0 2000

NATIONAL FLOOD INSURANCE PROGRAM

FIRM

FLOOD INSURANCE RATE MAP

EL PASO COUNTY, COLORADO AND INCORPORATED AREAS

PANEL 325 OF 1300

(SEE MAP INDEX FOR PANELS NOT PRINTED)

CONTAINS: COMMUNITY	NUMBER	PANEL	SUFFIX
EL PASO COUNTY, UNINCORPORATED AREAS	080059	0325	F

MAP NUMBER 08041C0325 F

EFFECTIVE DATE: MARCH 17, 1997

Federal Emergency Management Agency

HYDROLOGY / HYDRAULIC CALCULATIONS

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ALL LAND ASSUMED 2 ACRE RESIDENTIAL LOTS, UNDEVELOPED WOODS OR
GOOD CONDITION OPEN SPACE (LAWNS, PARKS GOLF COURSES, CEMETARIES ETC.)

C_N VALUES - DEVELOPED CONDITIONS

BASIN (label)	BASIN AREA (Ac)	**GOLF COURSE / WOODS (B)**		**2 AC. RESIDENTIAL (B)**		**COMPOSITE C_N**
		CN	AREA (Ac.)	CN	AREA (Ac.)	
OS-1A	4.4	61	4.4	65	0.0	**61.0**
OS-1B	5.6	61	5.6	65	0.0	**61.0**
EX-DP-3 (Pre-Dev.)	36.0	60	36.0	65	0.0	**60.0**
OS-2	2.9	61	2.9	65	0.0	**61.0**
OS-3	10.2	61	0.0	65	10.2	**65.0**
OS-4	32.9	61	0.0	65	32.9	**65.0**
OS-5	29.7	61	0.0	65	29.7	**65.0**
OS-6	9.2	61	0.0	65	9.2	**65.0**
OS-7	5.0	61	0.0	65	5.0	**65.0**
OS-8	14.2	61	0.0	65	14.2	**65.0**
OS-9	9.8	60	9.8	65	0.0	**60.0**
OS-10	4.1	61	0.0	65	4.1	**65.0**
OS-11	28.0	61	0.0	65	28.0	**65.0**
OS-12	68.1	61	40.0	65	28.1	**62.7**
OS-13	36.9	61	18.0	65	18.9	**63.0**
OS-14	26.4	61	20.0	65	6.4	**62.0**
OS-15	70.8	61	20.0	65	50.8	**63.9**
OS-16	4.5	61	0.0	65	4.5	**65.0**
OS-17	15.8	61	0.0	65	15.8	**65.0**
OS-18	13.0	61	0.0	65	13.0	**65.0**

ALL LAND ASSUMED 2 ACRE RESIDENTIAL LOTS, UNDEVELOPED WOODS OR
GOOD CONDITION OPEN SPACE (LAWNS, PARKS GOLF COURSES, CEMETARIES ETC.)

C_N VALUES - DEVELOPED CONDITIONS

BASIN (label)	BASIN AREA (Ac)	**GOLF COURSE / WOODS (B)**		**2 AC. RESIDENTIAL (B)**		**COMPOSITE C_N**
		CN	AREA (Ac.)	CN	AREA (Ac.)	
BS-1A	3.5	61	0.0	65	3.5	**65.0**
BS-1B	8.9	61	0.0	65	8.9	**65.0**
BS-2	1.9	61	0.0	89	1.9	**89.0**
BS-2A	0.8	61	0.0	89	0.8	**89.0**
BS-2B	0.9	61	0.0	89	0.9	**89.0**
BS-3	6.2	61	0.0	65	6.2	**65.0**
BS-4	13.0	61	0.0	67	13.0	**67.0**
BS-5	11.2	61	0.0	65	11.2	**65.0**
BS-6	1.2	61	0.0	89	1.2	**89.0**
BS-7	2.9	61	0.0	65	2.9	**65.0**
BS-8	1.0	61	0.0	89	1.0	**89.0**
BS-9	1.5	61	0.0	89	1.5	**89.0**
BS-10	4.5	61	0.0	65	4.5	**65.0**
BS-11	0.9	61	0.0	89	0.9	**89.0**
BS-12	7.7	61	0.0	65	7.7	**65.0**
BS-13	25.6	61	0.0	65	25.6	**65.0**
BS-14	13.4	61	0.0	65	13.4	**65.0**
BS-15	5.3	61	0.0	65	5.3	**65.0**
BS-16	21.6	61	0.0	65	21.6	**65.0**
BS-17	12.1	61	0.0	65	12.1	**65.0**
BS-18	33.8	61	12.1	65	21.7	**63.6**
BS-19	6.3	61	0.0	65	6.3	**65.0**
BS-20	73.9	61	30.2	65	43.7	**63.4**
BS-21	69.5	61	12.1	65	57.4	**64.3**
BS-22	18.1	61	2.5	65	15.6	**64.4**
BS-23	37.1	61	15.4	65	21.7	**63.3**
BS-23A	16.3	61	2.5	65	13.8	**64.4**
BS-24	10.9	60	4.3	65	6.6	**63.0**
EX-24 (Pre-Dev.)	13.2	60	13.2	65	0.0	**60.0**
BS-25	12.7	60	5.0	65	7.7	**63.0**
BS-26	2.5	60	2.5	65	0.0	**60.0**
BS-27	23.3	61	0.0	65	23.3	**65.0**
BS-28	36.9	61	5.6	65	31.3	**64.4**
BS-29	27.7	61	7.2	65	20.5	**64.0**
BS-30	6.7	61	0.0	65	6.7	**65.0**
BS-31	8.4	60	4.2	65	4.2	**62.5**
BS-32	6.2	60	3.0	65	3.2	**62.6**
BS-33	8.9	60	0.6	65	8.3	**64.7**

ALL LAND ASSUMED 2 ACRE RESIDENTIAL LOTS, UNDEVELOPED WOODS OR
GOOD CONDITION OPEN SPACE (LAWNS, PARKS GOLF COURSES, CEMETARIES ETC.)

C_N VALUES - DEVELOPED CONDITIONS

BASIN (label)	BASIN AREA (Ac)	**GOLF COURSE / WOODS (B)**		**2 AC. RESIDENTIAL (B)**		**COMPOSITE C_N**
		CN	AREA (Ac.)	CN	AREA (Ac.)	
CC-1A	9.8	61	0.0	65	9.8	**65.0**
CC-1B	12.6	61	0.5	65	12.1	**64.8**
CC-2A	11.0	61	0.0	65	11.0	**65.0**
CC-2B	20.8	61	0.0	65	20.8	**65.0**
CC-2C	6.4	61	0.0	65	6.4	**65.0**
CC-3	52.5	61	25.0	65	27.5	**63.1**
CC-4A	108.7	61	65.0	65	43.7	**62.6**
CC-4B	8.1	85	4.5	65	3.6	**76.1**
CC-4C (Pre-Dev.)	7.4	61	7.4	65	0.0	**61.0**
CC-5	22.4	61	0.0	65	22.4	**65.0**
CC-6	27.8	61	0.0	65	27.8	**65.0**
CC-7	18.4	61	0.0	65	18.4	**65.0**
CC-8	7.7	61	0.0	65	7.7	**65.0**
CC-9	5.6	61	0.0	65	5.6	**65.0**
CC-10	85.6	61	51.0	65	34.6	**62.6**
CC-11	18.6	61	9.0	65	9.6	**63.1**
CC-12	12.2	61	0.0	65	12.2	**65.0**
CC-13A	19.3	61	0.0	65	19.3	**65.0**
CC-13B	25.5	61	0.0	65	25.5	**65.0**
CC-13C	9.9	61	0.0	65	9.9	**65.0**
CC-13D	18.8	61	0.0	65	18.8	**65.0**
CC-14	4.6	61	0.0	65	4.6	**65.0**
CC-15	12.8	61	0.0	65	12.8	**65.0**
CC-16	16.3	61	0.0	65	16.3	**65.0**
CC-17	25.0	61	0.0	65	25.0	**65.0**
CC-18	6.2	65	5.8	89	0.4	**66.5**
CC-19	3.7	61	0.0	65	3.7	**65.0**
CC-20	39.3	61	0.0	65	39.3	**65.0**
CC-21	6.2	61	6.2	65	0.0	**61.0**
CC-22	13.8	61	0.0	65	13.8	**65.0**
CC-23	5.7	61	0.4	65	5.3	**64.7**
CC-24	39.6	61	0.0	65	39.6	**65.0**
CC-25	3.5	61	0.0	65	3.5	**65.0**
CC-26	16.7	61	0.0	65	16.7	**65.0**
CC-27	18.9	61	3.0	65	15.9	**64.4**
CC-28	154.8	61	23.0	65	131.8	**64.4**

TIME OF CONCENTRATION - DEVELOPED

BASIN	COMPOSITE Cn	C(5)	OVERLAND Length (ft)	OVERLAND Height (ft)	OVERLAND Tc (min)	STREET / CHANNEL FLOW (DCM Vol. 1 Fig. 6-25) Length (ft)	Slope (%)	Velocity (fps)	Tc (min)	Tc TOTAL (min)	Tc LAG (0.6tc) (min)	Tc LAG (0.6tc) (hr)
OS-1A	61.0	0.08	300	20	17.1	150	4.0%	1.0	2.5	19.6	11.7	0.20
OS-1B	61.0	0.08	300	20	17.1	300	8.0%	1.4	3.6	20.6	12.4	0.21
EX-DP-3 (Pre-Dev.)	60.0	0.08	300	20	17.1	900	5.0%	1.9	7.9	25.0	15.0	0.25
OS-2	61.0	0.08	300	20	17.1	300	6.0%	2.0	2.5	19.6	11.7	0.20
OS-3	65.0	0.08	300	22	16.5	275	6.2%	2.0	2.3	18.8	11.3	0.19
OS-4	65.0	0.08	300	18	17.7	420	4.3%	1.3	5.4	23.0	13.8	0.23
OS-5	65.0	0.08	300	12	20.2	1200	2.5%	1.1	19.0	39.2	23.5	0.39
OS-6	65.0	0.08	300	17	18.0	300	5.5%	1.9	2.6	20.6	12.4	0.21
OS-7	65.0	0.08	300	20	17.1	180	6.5%	2.1	1.4	18.5	11.1	0.18
OS-8	65.0	0.08	300	14	19.2	260	5.5%	0.6	7.5	26.7	16.0	0.27
OS-9	60.0	0.08	300	12	20.2	500	3.5%	0.5	16.7	36.9	22.1	0.37
OS-10	65.0	0.08	300	19	17.3					17.3	10.4	0.17
OS-11	65.0	0.08	300	14	19.2	600	6.5%	0.7	15.4	34.6	20.7	0.35
OS-12	62.7	0.08	300	10	21.4	1400	2.5%	1.5	15.6	37.0	22.2	0.37
OS-13	63.0	0.08	300	10	21.4	1000	3.0%	1.5	11.1	32.6	19.5	0.33
OS-14	62.0	0.08	300	8	23.1	1000	5.0%	2.1	7.9	31.0	18.6	0.31
OS-15	63.9	0.08	300	16	18.4	2200	4.0%	1.9	19.3	37.7	22.6	0.38
OS-16	65.0	0.08	300	7	24.1					24.1	14.5	0.24
OS-17	65.0	0.08	300	20	17.1	350	6.0%	2.5	2.3	19.4	11.6	0.19
OS-18	65.0	0.08	300	18	17.7	300	6.0%	2.5	2.0	19.7	11.8	0.20
BS-1A	65.0	0.08	300	19	17.3					17.3	10.4	0.17
BS-1B	65.0	0.08	300	18	17.7	200	2.5%	1.2	2.8	20.4	12.3	0.20
BS-2	89.0	0.08	300	16	18.4	630	7.0%	0.7	16.2	34.5	20.7	0.35
BS-2A	89.0	0.08	30	1.5	5.9	700	6.5%	1.7	6.9	12.8	7.7	0.13
BS-2B	89.0	0.08	30	1.5	5.9	800	6.5%	2.2	6.1	12.0	7.2	0.12
BS-3	65.0	0.08	300	18	17.7	300	5.3%	2.2	2.3	19.9	12.0	0.20
BS-4	67.0	0.08	300	22	16.5	960	7.0%	2.4	6.7	23.2	13.9	0.23
BS-5	65.0	0.08	300	20	17.1	150	7.0%	2.4	1.0	18.1	10.9	0.18
BS-6	89.0	0.08	10	0.2	4.6	700	7.0%	2.4	4.9	9.5	5.7	0.09

TIME OF CONCENTRATION - DEVELOPED

BASIN	COMPOSITE Cn	C(5)	OVERLAND Length (ft)	OVERLAND Height (ft)	OVERLAND Tc (min)	STREET / CHANNEL FLOW (DCM Vol. 1 Fig. 6-25) Length (ft)	Slope (%)	Velocity (fps)	Tc (min)	Tc TOTAL (min)	Tc LAG (0.6tc) (min)	Tc LAG (0.6tc) (hr)
BS-7	65.0	0.08	90	6	9.3	400	2.0%	2.0	3.3	12.7	7.6	0.13
BS-8	89.0	0.08	10	0.2	4.6	960	7.0%	2.3	7.0	11.6	7.0	0.12
BS-9	89.0	0.08	10	0.2	4.6	1100	5.0%	2.2	8.3	13.0	7.8	0.13
BS-10	65.0	0.08	300	16	18.4	800	5.0%	2.2	6.1	24.4	14.7	0.24
BS-11	89.0	0.08	10	0.2	4.6	300	3.0%	1.6	3.1	7.8	4.7	0.08
BS-12	65.0	0.08	300	18	17.7	180	3.0%	1.8	1.7	19.3	11.6	0.19
BS-13	65.0	0.08	300	16	18.4	630	7.0%	2.5	4.2	22.6	13.5	0.23
BS-14	65.0	0.08	300	14	19.2	700	8.0%	2.8	4.2	23.4	14.0	0.23
BS-15	65.0	0.08	300	26	15.6	250	5.0%	2.0	2.1	17.7	10.6	0.18
BS-16	65.0	0.08	300	18	17.7	1500	3.0%	1.5	16.7	34.3	20.6	0.34
BS-17	65.0	0.08	300	14	19.2	250	5.0%	2.0	2.1	21.3	12.8	0.21
BS-18	63.6	0.08	300	26	15.6	1800	3.0%	1.2	25.0	40.6	24.4	0.41
BS-19	65.0	0.08	300	16	18.4					18.4	11.0	0.18
BS-20	63.4	0.08	300	18	17.7	1650	6.5%	2.0	13.8	31.4	18.8	0.31
BS-21	64.3	0.08	300	30	14.9	2000	5.0%	1.7	19.6	34.5	20.7	0.35
BS-22	64.4	0.08	300	21	16.8	500	4.0%	1.5	5.6	22.3	13.4	0.22
BS-23	63.3	0.08	300	14	19.2	800	4.0%	1.0	13.3	32.5	19.5	0.33
BS-23A	64.4	0.08	300	14	19.2	1200	4.0%	2.0	10.0	29.2	17.5	0.29
BS-24	63.0	0.08	300	22	16.5					16.5	9.9	0.17
EX-24 (Pre-Dev.)	60.0	0.08	300	22	16.5					16.5	9.9	0.17
BS-25	63.0	0.08	300	10	21.4	200	5.0%	2.1	1.6	23.0	13.8	0.23
BS-26	60.0	0.08	300	20	17.1	100	4.0%	2.0	0.8	17.9	10.7	0.18
BS-27	65.0	0.08	300	22	16.5	900	8.0%	2.8	5.4	21.9	13.1	0.22
BS-28	64.4	0.08	300	16	18.4	1500	3.0%	1.8	13.9	32.2	19.3	0.32
BS-29	64.0	0.08	300	18	17.7	1900	4.5%	2.1	15.1	32.7	19.6	0.33
BS-30	65.0	0.08	300	20	17.1	400	6.0%	2.3	2.9	20.0	12.0	0.20
BS-31	62.5	0.08	300	12	20.2	300	4.0%	2.0	2.5	22.7	13.6	0.23
BS-32	62.6	0.08	300	18	17.7	200	5.0%	1.7	2.0	19.6	11.8	0.20
BS-33	64.7	0.08	300	22	16.5	350	6.0%	2.3	2.5	19.1	11.4	0.19

TIME OF CONCENTRATION - DEVELOPED

BASIN	COMPOSITE Cn	C(5)	OVERLAND Length (ft)	OVERLAND Height (ft)	OVERLAND Tc (min)	STREET / CHANNEL FLOW (DCM Vol. 1 Fig. 6-25) Length (ft)	Slope (%)	Velocity (fps)	Tc (min)	Tc TOTAL (min)	Tc LAG (0.6tc) (min)	Tc LAG (0.6tc) (hr)
CC-1A	65.0	0.08	300	16	18.4	500	5.0%	1.7	4.9	23.3	14.0	0.23
CC-1B	64.8	0.08	300	14	19.2	700	4.0%	2.0	5.8	25.0	15.0	0.25
CC-2A	65.0	0.08	300	14	19.2	250	3.0%	1.5	2.8	22.0	13.2	0.22
CC-2B	65.0	0.08	300	14	19.2	280	3.0%	1.5	3.1	22.3	13.4	0.22
CC-2C	65.0	0.08	300	18	17.7					17.7	10.6	0.18
CC-3	63.1	0.08	300	18	17.7	2300	3.0%	1.5	25.6	43.2	25.9	0.43
CC-4A	62.6	0.08	300	14	19.2	2700	2.0%	1.8	25.0	44.2	26.5	0.44
CC-4B	76.1	0.08	300	12	20.2	600	3.0%	1.6	6.3	26.4	15.9	0.26
CC-4C (Pre-Dev.)	61.0	0.08	40	0.8	9.3	350	3.0%	1.5	3.9	13.2	7.9	0.13
CC-5	65.0	0.08	300	18	17.7	1000	4.0%	2.0	8.3	26.0	15.6	0.26
CC-6	65.0	0.08	300	14	19.2	550	2.5%	1.6	5.7	24.9	14.9	0.25
CC-7	65.0	0.08	300	16	18.4	1000	3.0%	1.6	10.4	28.8	17.3	0.29
CC-8	65.0	0.08	300	10	21.4	250	2.0%	1.2	3.5	24.9	14.9	0.25
CC-9	65.0	0.08	300	18	17.7	100	2.0%	1.2	1.4	19.0	11.4	0.19
CC-10	62.6	0.08	300	22	16.5	2400	3.0%	1.8	22.2	38.7	23.2	0.39
CC-11	63.1	0.08	300	18	17.7	450	5.0%	2.1	3.6	21.2	12.7	0.21
CC-12	65.0	0.08	300	11	20.8	650	4.0%	2.0	5.4	26.2	15.7	0.26
CC-13A	65.0	0.08	300	14	19.2	1400	4.0%	2.0	11.7	30.9	18.5	0.31
CC-13B	65.0	0.08	300	18	17.7	1300	3.0%	1.6	13.5	31.2	18.7	0.31
CC-13C	65.0	0.08	300	14	19.2	350	4.0%	2.0	2.9	22.1	13.3	0.22
CC-13D	65.0	0.08	300	20	17.1	900	4.0%	2.0	7.5	24.6	14.7	0.25
CC-14	65.0	0.08	300	10	21.4					21.4	12.9	0.21
CC-15	65.0	0.08	300	14	19.2	550	3.0%	1.8	5.1	24.3	14.6	0.24
CC-16	65.0	0.08	300	10	21.4	650	2.5%	1.3	8.3	29.8	17.9	0.30
CC-17	65.0	0.08	300	9	22.2	950	2.0%	1.2	13.2	35.4	21.2	0.35
CC-18	66.5	0.08	300	7	24.1	400	2.0%	1.2	5.6	29.7	17.8	0.30
CC-19	65.0	0.08	300	8	23.1	100	2.0%	1.0	1.7	24.7	14.8	0.25
CC-20	65.0	0.08	300	9	22.2	350	6.0%	2.2	2.7	24.8	14.9	0.25
CC-21	61.0	0.08	300	18	17.7	200	3.0%	1.8	1.9	19.5	11.7	0.20
CC-22	65.0	0.08	300	14	19.2	700	4.0%	2.0	5.8	25.0	15.0	0.25
CC-23	64.7	0.08	300	10	21.4	850	2.0%	1.2	11.8	33.2	19.9	0.33
CC-24	65.0	0.08	300	20	17.1	900	4.0%	1.9	7.9	25.0	15.0	0.25
CC-25	65.0	0.08	300	16	18.4	500	3.0%	1.8	4.6	23.0	13.8	0.23
CC-26	65.0	0.08	300	14	19.2	900	5.0%	2.1	7.1	26.3	15.8	0.26
CC-27	64.4	0.08	300	14	19.2	1300	3.0%	1.8	12.0	31.2	18.7	0.31
CC-28	64.4	0.08	300	14	19.2	4700	3.0%	1.8	43.5	62.7	37.6	0.63

BASIN SUMMARY - DEVELOPED CONDITIONS

BASIN (label)	AREA (acres)	COMPOSITE CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
OS-1A	4.40	61.0	0.20	0.4	1.6	7.7
OS-1B	5.60	61.0	0.21	0.5	1.9	9.4
EX-DP-3 (Pre-Dev.)	36.00	60.0	0.25	0.5	4.8	41.3
OS-2	2.90	61.0	0.20	0.1	0.6	4.0
OS-3	10.20	65.0	0.19	1.0	3.8	17.9
OS-4	32.90	65.0	0.23	2.8	11.2	53.6
OS-5	29.70	65.0	0.39	1.9	7.1	37.0
OS-6	9.20	65.0	0.21	0.9	3.2	15.5
OS-7	5.00	65.0	0.18	0.5	2.0	9.0
OS-8	14.20	65.0	0.27	2.1	6.2	24.7
OS-9	9.80	60.0	0.37	0.1	1.0	9.1
OS-10	4.10	65.0	0.17	0.7	2.1	8.2
OS-11	28.00	65.0	0.35	2.4	8.2	38.7
OS-12	68.10	62.7	0.37	2.2	11.9	75.8
OS-13	36.90	63.0	0.33	1.4	7.4	45.0
OS-14	26.40	62.0	0.31	0.7	4.6	31.0
OS-15	70.80	63.9	0.38	3.3	14.8	84.2
OS-16	4.50	65.0	0.24	0.4	1.5	7.2
OS-17	15.80	65.0	0.19	1.6	5.9	27.7
OS-18	13.00	65.0	0.20	1.3	4.7	22.6

BASIN SUMMARY - DEVELOPED CONDITIONS

BASIN (label)	AREA (acres)	COMPOSITE CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
BS-1A	3.50	65.0	0.17	0.4	1.4	6.3
BS-1B	8.90	65.0	0.20	0.4	2.4	13.8
BS-2	1.90	89.0	0.35	2.9	4.2	8.4
BS-2A	0.80	89.0	0.13	1.2	1.8	3.5
BS-2B	0.90	89.0	0.12	1.4	2.0	4.0
BS-3	6.20	65.0	0.20	0.6	2.3	10.8
BS-4	13.00	67.0	0.23	1.9	5.5	23.6
BS-5	11.20	65.0	0.18	1.1	4.4	20.1
BS-6	1.20	89.0	0.09	1.9	2.8	5.4
BS-7	2.90	65.0	0.13	4.4	6.4	12.8
BS-8	1.00	89.0	0.12	1.6	2.2	4.5
BS-9	1.50	89.0	0.13	2.3	3.3	6.6
BS-10	4.50	65.0	0.24	6.0	8.7	17.5
BS-11	0.90	89.0	0.08	1.5	2.1	4.1
BS-12	7.70	65.0	0.19	0.8	3.0	13.8
BS-13	25.60	65.0	0.23	3.7	10.2	40.7
BS-14	13.40	65.0	0.23	2.6	6.8	26.5
BS-15	5.30	65.0	0.18	1.6	3.7	12.2
BS-16	21.60	65.0	0.34	4.6	11.8	44.1
BS-17	12.10	65.0	0.21	3.1	7.7	26.7
BS-18	33.80	63.6	0.41	3.5	12.4	56.0
BS-19	6.30	65.0	0.18	2.1	4.6	15.0
BS-20	73.90	63.4	0.31	7.4	24.6	112.4
BS-21	69.50	64.3	0.35	7.8	23.9	103.0
BS-22	18.10	64.4	0.22	3.7	9.6	36.5
BS-23	37.10	63.3	0.33	4.5	13.6	58.2
BS-23A	16.30	64.4	0.29	5.5	12.0	38.3
BS-24	10.90	63.0	0.17	0.6	3.3	17.6
EX-24 (Pre-Dev.)	13.20	60.0	0.17	0.2	2.2	17.8
BS-25	12.70	63.0	0.23	0.4	2.7	17.3
BS-26	2.50	60.0	0.18	0.0	0.4	3.4
BS-27	23.30	65.0	0.22	2.1	8.0	38.8
BS-28	36.90	64.4	0.32	2.2	9.3	49.4
BS-29	27.70	64.0	0.33	1.4	6.5	35.9
BS-30	6.70	65.0	0.20	0.7	2.4	11.7
BS-31	8.40	62.5	0.23	0.3	1.9	11.8
BS-32	6.20	62.6	0.20	0.3	1.6	9.4
BS-33	8.90	64.7	0.19	0.8	3.2	15.3

BASIN SUMMARY - DEVELOPED CONDITIONS

BASIN (label)	AREA (acres)	COMPOSITE CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
CC-1A	9.80	65.0	0.23	0.8	3.3	16.0
CC-1B	12.60	64.8	0.25	1.0	4.0	19.4
CC-2A	11.00	65.0	0.22	1.0	3.8	18.3
CC-2B	20.80	65.0	0.22	1.9	7.1	34.6
CC-2C	6.40	65.0	0.18	0.7	2.5	11.5
CC-3	52.50	63.1	0.43	1.8	8.8	54.5
CC-4A	108.70	62.6	0.44	15.4	39.0	156.0
CC-4B	8.10	76.1	0.26	4.0	7.3	20.6
CC-4C (Pre-Dev.)	7.40	61.0	0.13	0.2	1.8	11.2
CC-5	22.40	65.0	0.26	1.8	7.1	34.3
CC-6	27.80	65.0	0.25	2.3	9.1	43.2
CC-7	18.40	65.0	0.29	1.4	5.4	27.0
CC-8	7.70	65.0	0.25	0.6	2.5	12.0
CC-9	5.60	65.0	0.19	0.6	2.1	9.8
CC-10	85.60	62.6	0.39	2.6	14.1	91.9
CC-11	18.60	63.1	0.21	0.9	5.0	28.1
CC-12	12.20	65.0	0.26	1.0	3.9	18.7
CC-13A	19.30	65.0	0.31	1.4	5.4	27.3
CC-13B	25.50	65.0	0.31	1.8	7.2	36.1
CC-13C	9.90	65.0	0.22	0.9	3.4	16.5
CC-13D	18.80	65.0	0.25	1.5	6.2	29.2
CC-14	4.60	65.0	0.21	0.4	1.6	7.8
CC-15	12.80	65.0	0.24	1.1	4.3	20.4
CC-16	16.30	65.0	0.30	1.2	4.6	23.6
CC-17	25.00	65.0	0.35	1.7	6.5	32.8
CC-18	6.20	66.5	0.30	0.7	2.2	9.7
CC-19	3.70	65.0	0.25	0.3	1.2	5.8
CC-20	39.30	65.0	0.25	3.2	12.9	61.0
CC-21	6.20	61.0	0.20	0.1	1.2	8.5
CC-22	13.80	65.0	0.25	1.1	4.5	21.4
CC-23	5.70	64.7	0.33	0.4	1.5	7.7
CC-24	39.60	65.0	0.25	3.3	13.0	61.5
CC-25	3.50	65.0	0.23	0.3	1.2	5.7
CC-26	16.70	65.0	0.26	1.4	5.3	25.6
CC-27	18.90	64.4	0.31	1.2	4.9	25.8
CC-28	154.80	64.4	0.63	6.5	24.7	136.3

DESIGN POINTS SURFACE ROUTING SUMMARY - DEVELOPED CONDITIONS

Design Point (label)	**Contributing Basins**	**Q 2 Yr.** Q (cfs)	**Q 5 Yr.** Q (cfs)	**Q 100 Yr.** Q (cfs)
DP-1 DEV	OS-1A, BS-2B	1.6	3.4	11
DP-2 DEV	DP-1, BS-4	3.2	8.8	35
TOTAL INFLOW TO POND 1 (UD Detention hydrograph)	**DP-1, DP-2, BS-1A**	**4**	**7**	**38**
DP-3 DEV (Pond Pack routing)	**OS-2, BS-3, BS-1B, Release from FHN Pond 1**	**1**	**6**	**39**
DP-4 DEV	BS-2	2.9	4.2	8
DP-5 DEV	OS-1B, BS-2A	1.5	3.5	13
DP-6 DEV	OS-2, BS-3	0.6	2.8	15
DP-7 DEV	OS-3, BS-5	2.1	8.2	38
DP-8 DEV	OS-4, OS-5, OS-6, BS-7, BS-10, Release from Exist. HFR Pond 16	20.9	70.4	284
DP-9 DEV	OS-7, BS-12	1.3	5.0	23
DP-10 DEV	OS-8, OS-10, OS-11, BS-13, BS-14	10.7	32.0	143
DP-11 DEV	BS-16	4.6	11.8	36
DP-12 DEV	DP-11, BS-15	4.0	11.0	44
TOTAL INFLOW TO POND 4 (UD Detention hydrograph)	**DP-10, DP-12, BS-17, OS-9**	**10**	**16**	**217**
DP-13 DEV	**Release from FHN Pond 4**	**0.3**	**0.3**	**142**
DP-14 DEV	BS-18	3.5	12.4	56
DP-15 DEV	BS-19	2.1	4.6	15
DP-16 DEV	DP-14, DP-15, BS-20, BS-21, BS-22, BS-23	25.0	78.0	362
TOTAL INFLOW TO FHN POND 8 (Full Build-out) (UD Detention hydrograph)	**DP-10, DP-12, BS-17, OS-9**	**24**	**37**	**390**
DP-17 DEV (Full Build-out)	**Release from FHN Pond 8**	**0.8**	**1.0**	**253**
TOTAL INFLOW TO FHN POND 8 (Filing 1 Only) (UD Detention hydrograph)	**DP-10, DP-12, BS-17, OS-9**	**9**	**14**	**301**
DP-17 DEV (Filing 1 Only)	**Release from FHN Pond 8**	**0.4**	**0.5**	**219**

DESIGN POINTS SURFACE ROUTING SUMMARY - DEVELOPED CONDITIONS

Design Point (label)	**Contributing Basins**	**Q 2 Yr.** Q (cfs)	**Q 5 Yr.** Q (cfs)	**Q 100 Yr.** Q (cfs)
DP-18 DEV	BS-28, BS-29, BS-30, OS-18	5.0	21.6	115
DP-19 DEV	BS-27, OS-17, Release from DP-18	3.8	16.8	126
DP-20 DEV	CC-1A, OS-12	3.2	14.3	88
DP-21 DEV	CC-2A, OS-13	2.1	10.5	62
DP-22 DEV	CC-2B, Release from DP-21	3.7	16.6	92
DP-23 DEV	CC-3, OS-14	2.5	13.0	84
DP-24 DEV	CC-4C (Pre-Dev.), CC-5	1.9	8.4	45
TOTAL INFLOW TO POND 12 (UD Detention hydrograph)	**CC-4C, CC-5, CC-6**	**6**	**9**	**85**
DP-25 DEV	**Release from FHN Pond 12**	**0.2**	**0.3**	**45**
DP-26 DEV	CC-8, CC-10	3.0	15.9	102
DP-27 DEV	CC-15, CC-20	4.3	17.2	81
DP-28 DEV	CC-13A, OS-15	4.6	19.8	110
DP-29 DEV	CC-13B, CC-13C, Release from DP-28	5.8	26.6	155
DP-30 DEV	CC-18	0.7	2.2	10
DP-31 DEV	CC-19, Release from DP-30	0.9	3.2	15
DP-32 DEV	CC-17, OS-16	2.0	7.8	40
DP-33 DEV	CC-23, CC-24	3.6	14.4	69
DP-34 DEV	CC-26, CC-27, CC-28 and Release from CC-16 & DP-32	6.0	23.5	168

ALL LAND ASSUMED 2 ACRE RESIDENTIAL LOTS, UNDEVELOPED WOODS (FUTURE FILING) OR
GOOD CONDITION OPEN SPACE (LAWNS, PARKS GOLF COURSES, CEMETARIES ETC.)

C_N VALUES - DEVELOPED CONDITIONS (FILING 1 ONLY)

BASIN (label)	BASIN AREA (Ac)	**GOLF COURSE (B)**		**2 AC. RESIDENTIAL (B)**		**UNDEVELOPED WOODS (B)**		**COMPOSITE C_N**
		CN	AREA (Ac.)	CN	AREA (Ac.)	CN	AREA (Ac.)	
BS-20	73.9	61	30.2	60	1.0	60	42.7	**60.4**
BS-21	69.5	61	12.1	65	34.4	60	23.0	**62.6**
BS-22	18.1	61	2.5	65	5.1	60	10.5	**61.5**
BS-23	37.1	61	15.4	65	20.2	60	1.5	**63.1**
BS-23A	16.3	61	0.0	65	2.5	60	13.8	**60.8**

TIME OF CONCENTRATION DEVELOPED (FILING 1 ONLY)

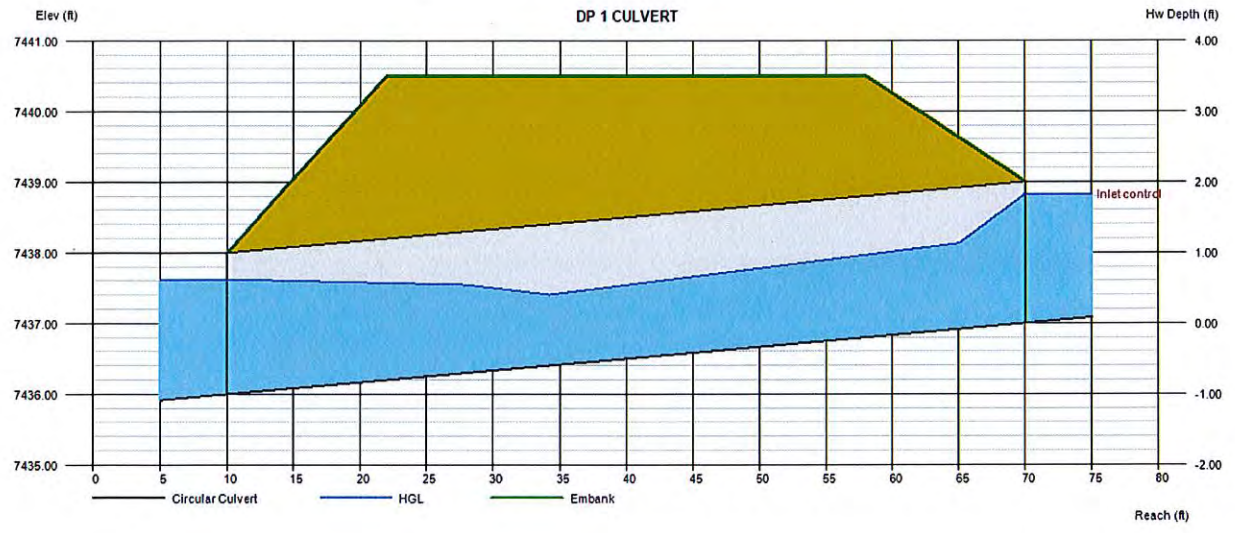
BASIN	COMPOSITE Cn	C(5)	Length *(ft)*	OVERLAND Height *(ft)*	Tc *(min)*	STREET / CHANNEL FLOW (DCM Vol. 1 Fig. 6-25) Length *(ft)*	Slope *(%)*	Velocity *(fps)*	Tc *(min)*	Tc TOTAL *(min)*	Tc LAG (0.6tc) *(min)*	Tc LAG (0.6tc) *(hr)*
BS-20	60.4	0.08	1000	60	32.2					32.2	19.3	0.32
BS-21	62.6	0.08	1000	30	40.5	500	4.0%	1.7	4.9	45.4	27.3	0.45
BS-22	61.5	0.08	300	21	16.8	500	4.0%	1.5	5.6	22.3	13.4	0.22
BS-23	63.1	0.08	300	14	19.2	800	4.0%	1.0	13.3	32.5	19.5	0.33
BS-23A	60.8	0.08	1000	64	31.6	200	2.0%	1.5	2.2	33.8	20.3	0.34

Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Monday, Nov 20 2017

DP 1 CULVERT

Invert Elev Dn (ft)	= 7436.00	**Calculations**	
Pipe Length (ft)	= 60.00	Qmin (cfs)	= 0.00
Slope (%)	= 1.67	Qmax (cfs)	= 12.00
Invert Elev Up (ft)	= 7437.00	Tailwater Elev (ft)	= (dc+D)/2
Rise (in)	= 24.0		
Shape	= Circular	**Highlighted**	
Span (in)	= 24.0	Qtotal (cfs)	= 12.00
No. Barrels	= 1	Qpipe (cfs)	= 12.00
n-Value	= 0.013	Qovertop (cfs)	= 0.00
Culvert Type	= Circular Concrete	Veloc Dn (ft/s)	= 4.40
Culvert Entrance	= Groove end projecting (C)	Veloc Up (ft/s)	= 5.85
Coeff. K,M,c,Y,k	= 0.0045, 2, 0.0317, 0.69, 0.2	HGL Dn (ft)	= 7437.62
		HGL Up (ft)	= 7438.24
Embankment		Hw Elev (ft)	= 7438.82
Top Elevation (ft)	= 7440.50	Hw/D (ft)	= 0.91
Top Width (ft)	= 36.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 60.00		



Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Mar 29 2018

DP 2 CULVERT

Invert Elev Dn (ft)	=	7417.60
Pipe Length (ft)	=	90.00
Slope (%)	=	6.00
Invert Elev Up (ft)	=	7423.00
Rise (in)	=	30.0
Shape	=	Circular
Span (in)	=	30.0
No. Barrels	=	1
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Groove end projecting (C)
Coeff. K,M,c,Y,k	=	0.0045, 2, 0.0317, 0.69, 0.2

Embankment

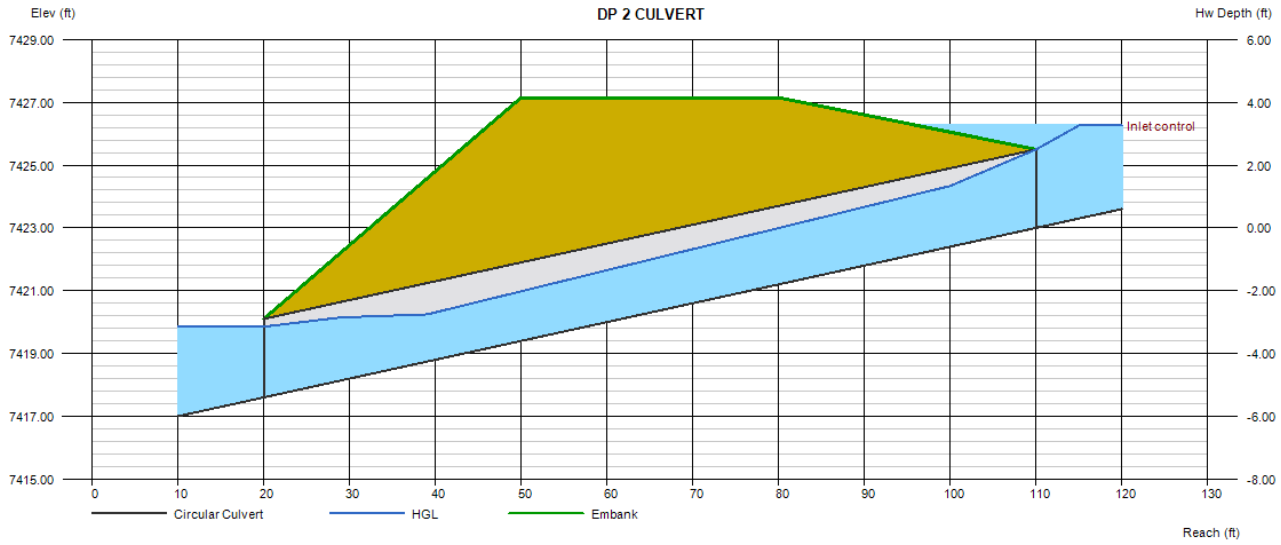
Top Elevation (ft)	=	7427.15
Top Width (ft)	=	30.00
Crest Width (ft)	=	60.00

Calculations

Qmin (cfs)	=	0.00
Qmax (cfs)	=	35.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	35.00
Qpipe (cfs)	=	35.00
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	7.51
Veloc Up (ft/s)	=	8.28
HGL Dn (ft)	=	7419.85
HGL Up (ft)	=	7425.01
Hw Elev (ft)	=	7426.26
Hw/D (ft)	=	1.30
Flow Regime	=	Inlet Control



Culvert Report

CULVERT @ FIRE STATION DRIVEWAY

Invert Elev Dn (ft)	=	7412.00
Pipe Length (ft)	=	60.00
Slope (%)	=	8.33
Invert Elev Up (ft)	=	7417.00
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	1
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Groove end projecting (C)
Coeff. K,M,c,Y,k	=	0.0045, 2, 0.0317, 0.69, 0.2

Embankment

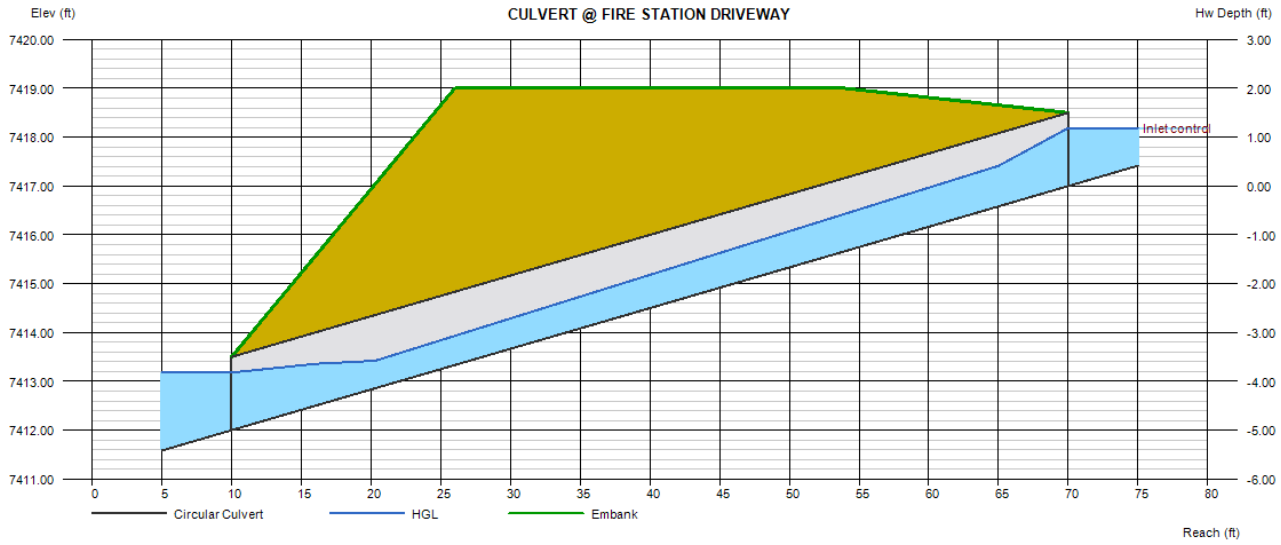
Top Elevation (ft)	=	7419.00
Top Width (ft)	=	28.00
Crest Width (ft)	=	60.00

Calculations

Qmin (cfs)	=	0.00
Qmax (cfs)	=	5.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	5.00
Qpipe (cfs)	=	5.00
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	3.35
Veloc Up (ft/s)	=	4.77
HGL Dn (ft)	=	7413.18
HGL Up (ft)	=	7417.86
Hw Elev (ft)	=	7418.19
Hw/D (ft)	=	0.79
Flow Regime	=	Inlet Control



Culvert Report

DP 7 CULVERT

Invert Elev Dn (ft)	=	7453.00
Pipe Length (ft)	=	66.00
Slope (%)	=	4.00
Invert Elev Up (ft)	=	7455.64
Rise (in)	=	30.0
Shape	=	Circular
Span (in)	=	30.0
No. Barrels	=	2
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Groove end projecting (C)
Coeff. K,M,c,Y,k	=	0.0045, 2, 0.0317, 0.69, 0.2

Embankment

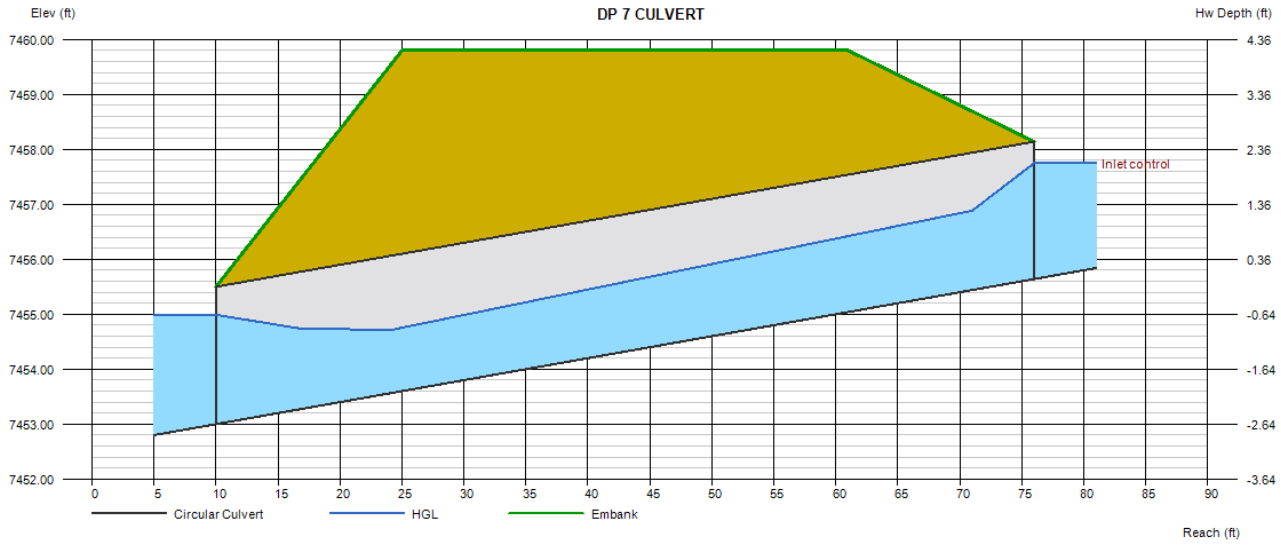
Top Elevation (ft)	=	7459.80
Top Width (ft)	=	36.00
Crest Width (ft)	=	60.00

Calculations

Qmin (cfs)	=	0.00
Qmax (cfs)	=	38.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	38.00
Qpipe (cfs)	=	38.00
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	4.54
Veloc Up (ft/s)	=	6.30
HGL Dn (ft)	=	7454.99
HGL Up (ft)	=	7457.12
Hw Elev (ft)	=	7457.75
Hw/D (ft)	=	0.84
Flow Regime	=	Inlet Control



Culvert Report

DP 8 CULVERTS

Invert Elev Dn (ft)	=	7438.30
Pipe Length (ft)	=	100.00
Slope (%)	=	5.70
Invert Elev Up (ft)	=	7444.00
Rise (in)	=	48.0
Shape	=	Circular
Span (in)	=	48.0
No. Barrels	=	3
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment

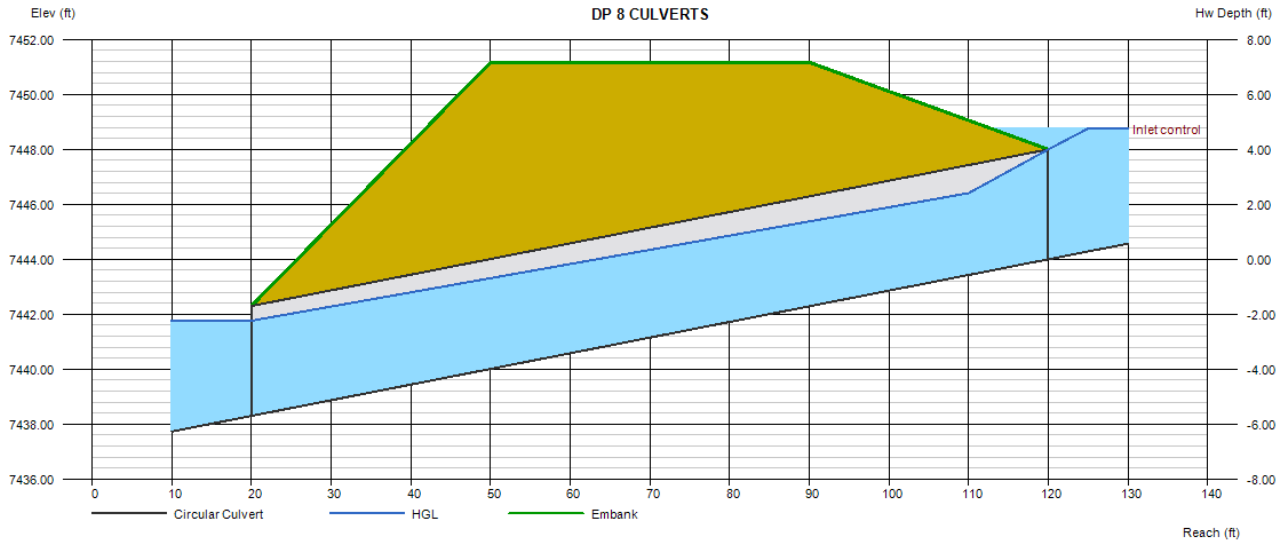
Top Elevation (ft)	=	7451.17
Top Width (ft)	=	40.00
Crest Width (ft)	=	50.00

Calculations

Qmin (cfs)	=	0.00
Qmax (cfs)	=	280.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	280.00
Qpipe (cfs)	=	280.00
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	8.07
Veloc Up (ft/s)	=	9.47
HGL Dn (ft)	=	7441.76
HGL Up (ft)	=	7446.93
Hw Elev (ft)	=	7448.76
Hw/D (ft)	=	1.19
Flow Regime	=	Inlet Control



Culvert Report

DP 9 CULVERTS

Invert Elev Dn (ft)	=	7486.10
Pipe Length (ft)	=	60.00
Slope (%)	=	2.30
Invert Elev Up (ft)	=	7487.48
Rise (in)	=	24.0
Shape	=	Circular
Span (in)	=	24.0
No. Barrels	=	2
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Groove end projecting (C)
Coeff. K,M,c,Y,k	=	0.0045, 2, 0.0317, 0.69, 0.2

Embankment

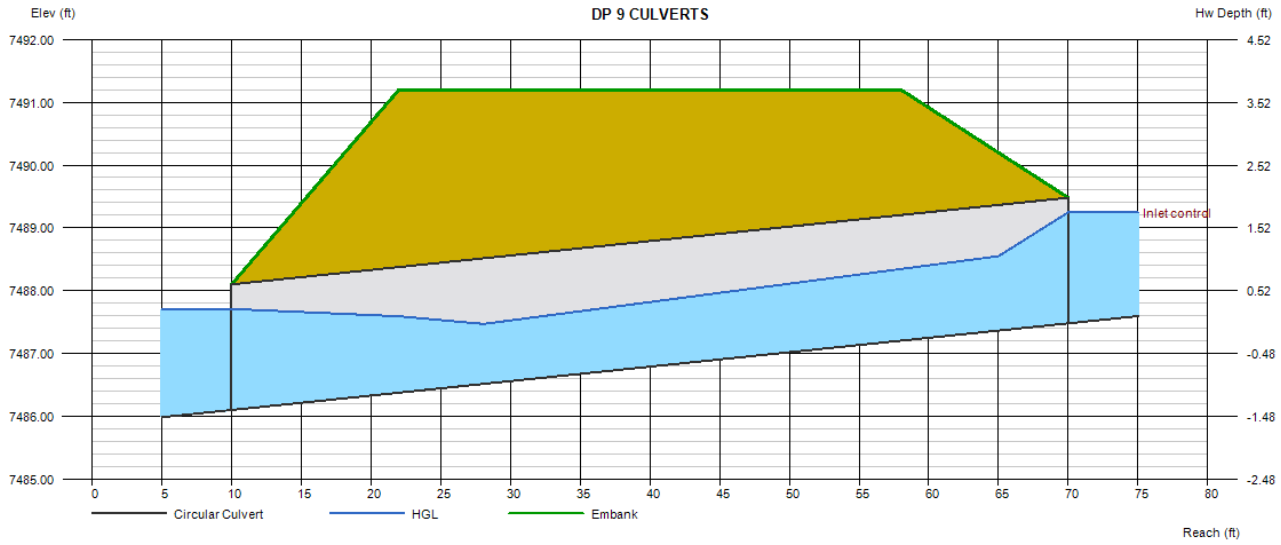
Top Elevation (ft)	=	7491.20
Top Width (ft)	=	36.00
Crest Width (ft)	=	60.00

Calculations

Qmin (cfs)	=	0.00
Qmax (cfs)	=	23.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	23.00
Qpipe (cfs)	=	23.00
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	4.25
Veloc Up (ft/s)	=	5.75
HGL Dn (ft)	=	7487.71
HGL Up (ft)	=	7488.70
Hw Elev (ft)	=	7489.25
Hw/D (ft)	=	0.88
Flow Regime	=	Inlet Control



Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Nov 30 2017

DP 10 CULVERT

Invert Elev Dn (ft)	= 7455.80
Pipe Length (ft)	= 112.07
Slope (%)	= 4.00
Invert Elev Up (ft)	= 7460.28
Rise (in)	= 42.0
Shape	= Circular
Span (in)	= 42.0
No. Barrels	= 2
n-Value	= 0.013
Culvert Type	= Circular Concrete
Culvert Entrance	= Square edge w/headwall (C)
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5

Embankment

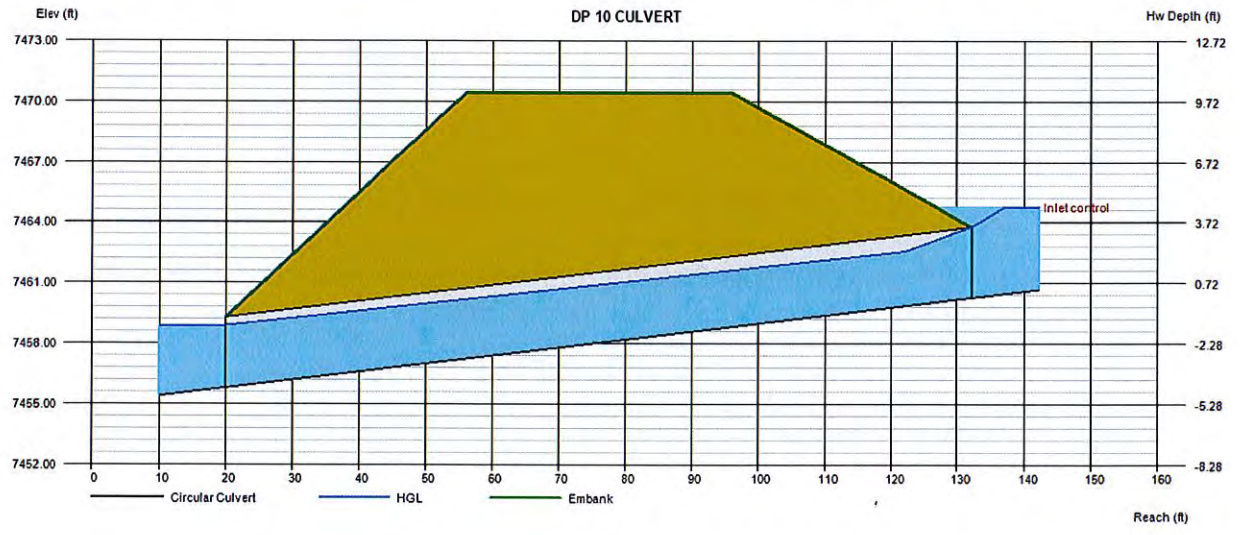
Top Elevation (ft)	= 7470.42
Top Width (ft)	= 40.00
Crest Width (ft)	= 50.00

Calculations

Qmin (cfs)	= 0.00
Qmax (cfs)	= 144.00
Tailwater Elev (ft)	= (dc+D)/2

Highlighted

Qtotal (cfs)	= 144.00
Qpipe (cfs)	= 144.00
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 8.03
Veloc Up (ft/s)	= 9.19
HGL Dn (ft)	= 7458.88
HGL Up (ft)	= 7462.94
Hw Elev (ft)	= 7464.78
Hw/D (ft)	= 1.29
Flow Regime	= Inlet Control

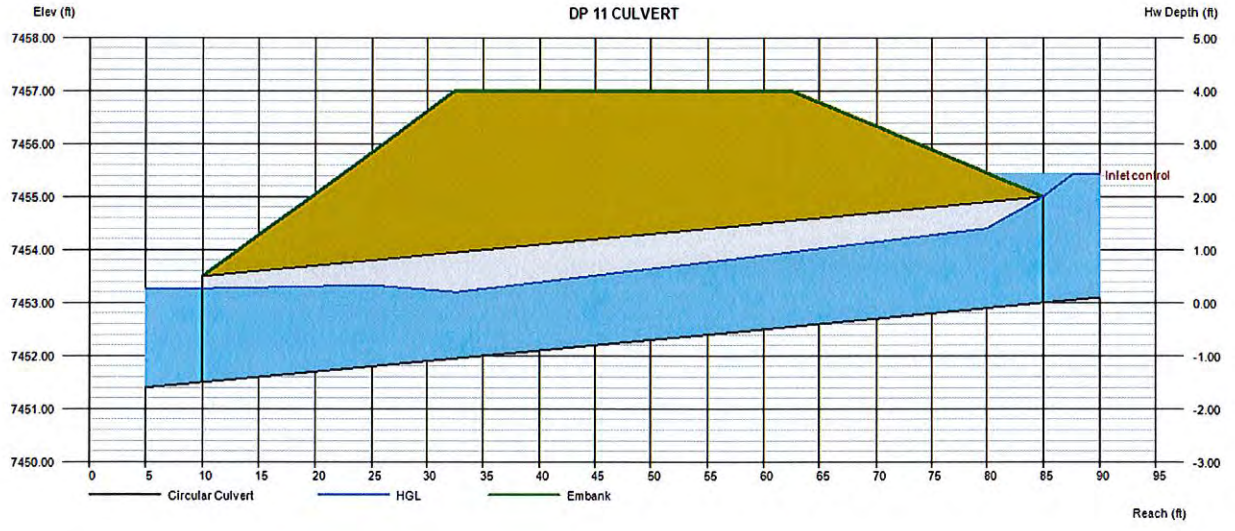


Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Nov 30 2017

DP 11 CULVERT

Invert Elev Dn (ft)	= 7451.50	**Calculations**	
Pipe Length (ft)	= 75.00	Qmin (cfs)	= 0.00
Slope (%)	= 2.00	Qmax (cfs)	= 36.00
Invert Elev Up (ft)	= 7453.00	Tailwater Elev (ft)	= (dc+D)/2
Rise (in)	= 24.0		
Shape	= Circular	**Highlighted**	
Span (in)	= 24.0	Qtotal (cfs)	= 36.00
No. Barrels	= 2	Qpipe (cfs)	= 36.00
n-Value	= 0.013	Qovertop (cfs)	= 0.00
Culvert Type	= Circular Concrete	Veloc Dn (ft/s)	= 6.14
Culvert Entrance	= Groove end projecting (C)	Veloc Up (ft/s)	= 6.99
Coeff. K,M,c,Y,k	= 0.0045, 2, 0.0317, 0.69, 0.2	HGL Dn (ft)	= 7453.26
		HGL Up (ft)	= 7454.53
Embankment		Hw Elev (ft)	= 7455.42
Top Elevation (ft)	= 7457.00	Hw/D (ft)	= 1.21
Top Width (ft)	= 30.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 20.00		



Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Mar 29 2018

DP 12 CULVERT

Invert Elev Dn (ft)	=	7426.00
Pipe Length (ft)	=	130.00
Slope (%)	=	1.15
Invert Elev Up (ft)	=	7427.50
Rise (in)	=	36.0
Shape	=	Circular
Span (in)	=	36.0
No. Barrels	=	1
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Groove end projecting (C)
Coeff. K,M,c,Y,k	=	0.0045, 2, 0.0317, 0.69, 0.2

Embankment

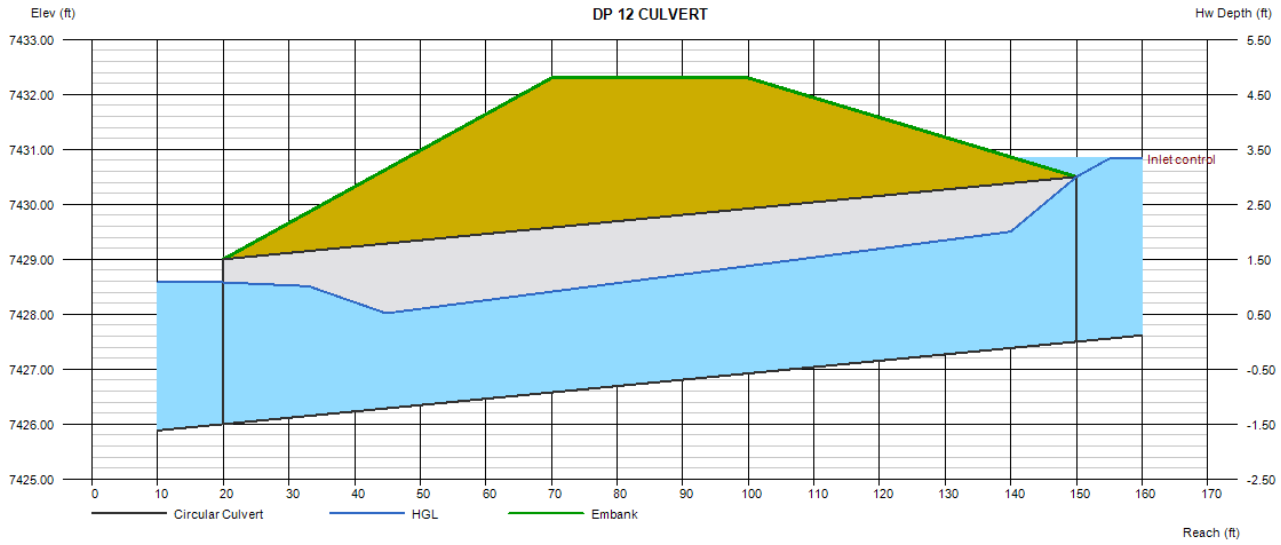
Top Elevation (ft)	=	7432.30
Top Width (ft)	=	30.00
Crest Width (ft)	=	20.00

Calculations

Qmin (cfs)	=	0.00
Qmax (cfs)	=	44.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	44.00
Qpipe (cfs)	=	44.00
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	6.80
Veloc Up (ft/s)	=	8.08
HGL Dn (ft)	=	7428.58
HGL Up (ft)	=	7429.66
Hw Elev (ft)	=	7430.83
Hw/D (ft)	=	1.11
Flow Regime	=	Inlet Control

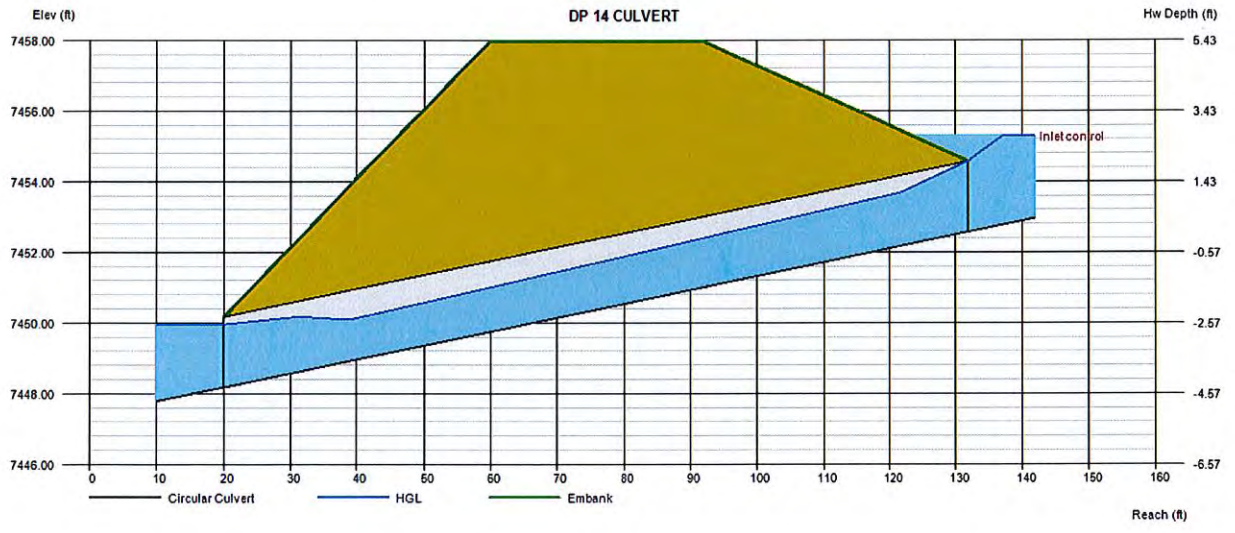


Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Nov 30 2017

DP 14 CULVERT

Invert Elev Dn (ft)	= 7448.18	**Calculations**	
Pipe Length (ft)	= 111.84	Qmin (cfs)	= 0.00
Slope (%)	= 3.92	Qmax (cfs)	= 56.00
Invert Elev Up (ft)	= 7452.57	Tailwater Elev (ft)	= (dc+D)/2
Rise (in)	= 24.0		
Shape	= Circular	**Highlighted**	
Span (in)	= 24.0	Qtotal (cfs)	= 56.00
No. Barrels	= 3	Qpipe (cfs)	= 56.00
n-Value	= 0.012	Qovertop (cfs)	= 0.00
Culvert Type	= Circular Concrete	Veloc Dn (ft/s)	= 6.33
Culvert Entrance	= Square edge w/headwall (C)	Veloc Up (ft/s)	= 7.13
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 7449.96
		HGL Up (ft)	= 7454.12
Embankment		Hw Elev (ft)	= 7455.28
Top Elevation (ft)	= 7457.97	Hw/D (ft)	= 1.35
Top Width (ft)	= 32.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 40.00		

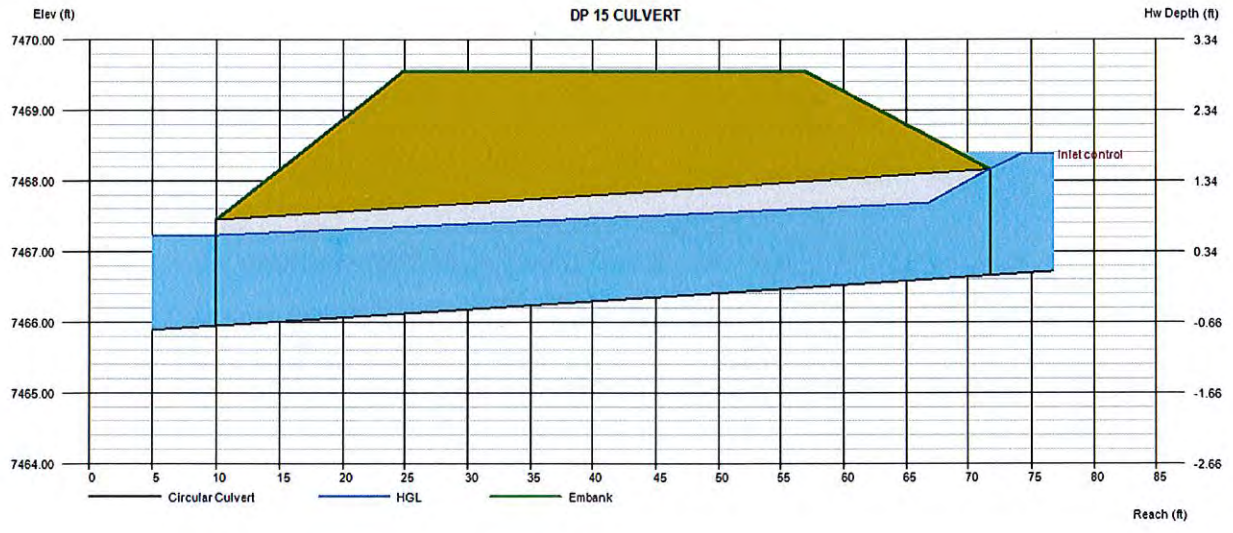


Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Nov 30 2017

DP 15 CULVERT

Invert Elev Dn (ft)	= 7465.95	**Calculations**	
Pipe Length (ft)	= 61.74	Qmin (cfs)	= 0.00
Slope (%)	= 1.15	Qmax (cfs)	= 15.00
Invert Elev Up (ft)	= 7466.66	Tailwater Elev (ft)	= (dc+D)/2
Rise (in)	= 18.0		
Shape	= Circular	**Highlighted**	
Span (in)	= 18.0	Qtotal (cfs)	= 15.00
No. Barrels	= 2	Qpipe (cfs)	= 15.00
n-Value	= 0.013	Qovertop (cfs)	= 0.00
Culvert Type	= Circular Concrete	Veloc Dn (ft/s)	= 4.67
Culvert Entrance	= Square edge w/headwall (C)	Veloc Up (ft/s)	= 5.62
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 7467.23
		HGL Up (ft)	= 7467.72
Embankment		Hw Elev (ft)	= 7468.38
Top Elevation (ft)	= 7469.55	Hw/D (ft)	= 1.15
Top Width (ft)	= 32.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 40.00		



Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Nov 30 2017

DP 16 CULVERT

Invert Elev Dn (ft)	=	7373.00
Pipe Length (ft)	=	200.00
Slope (%)	=	6.00
Invert Elev Up (ft)	=	7385.00
Rise (in)	=	60.0
Shape	=	Circular
Span (in)	=	60.0
No. Barrels	=	2
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Groove end w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0018, 2, 0.0292, 0.74, 0.2

Embankment

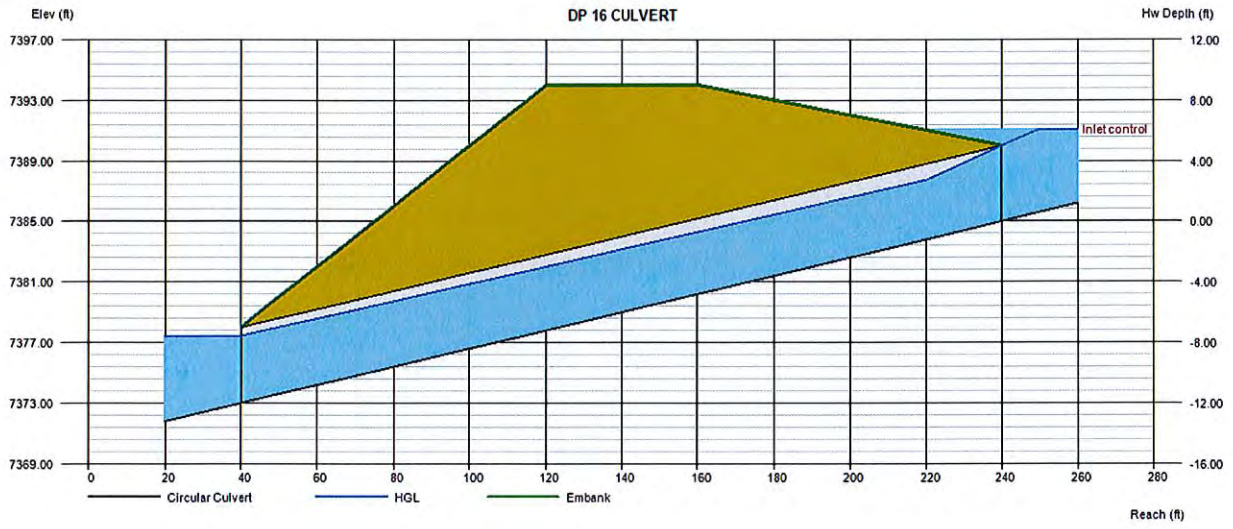
Top Elevation (ft)	=	7394.00
Top Width (ft)	=	40.00
Crest Width (ft)	=	80.00

Calculations

Qmin (cfs)	=	0.00
Qmax (cfs)	=	365.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	365.00
Qpipe (cfs)	=	365.00
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	9.92
Veloc Up (ft/s)	=	11.21
HGL Dn (ft)	=	7377.43
HGL Up (ft)	=	7388.87
Hw Elev (ft)	=	7391.07
Hw/D (ft)	=	1.21
Flow Regime	=	Inlet Control

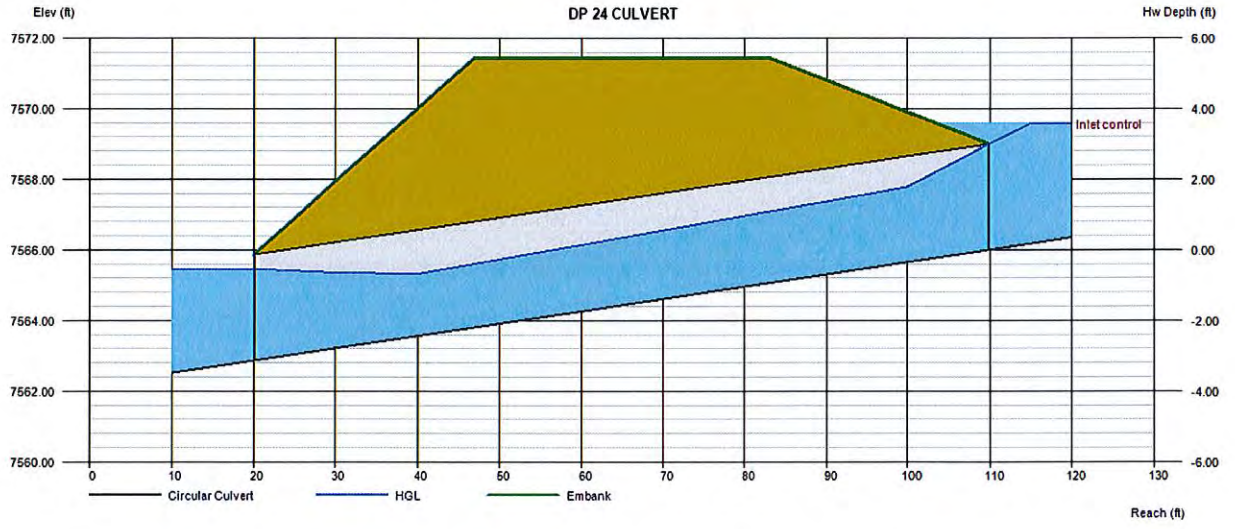


Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Nov 30 2017

DP 24 CULVERT

Invert Elev Dn (ft)	= 7562.87	**Calculations**	
Pipe Length (ft)	= 89.81	Qmin (cfs)	= 0.00
Slope (%)	= 3.49	Qmax (cfs)	= 45.00
Invert Elev Up (ft)	= 7566.00	Tailwater Elev (ft)	= (dc+D)/2
Rise (in)	= 36.0		
Shape	= Circular	**Highlighted**	
Span (in)	= 36.0	Qtotal (cfs)	= 45.00
No. Barrels	= 1	Qpipe (cfs)	= 45.00
n-Value	= 0.012	Qovertop (cfs)	= 0.00
Culvert Type	= Circular Concrete	Veloc Dn (ft/s)	= 6.93
Culvert Entrance	= Square edge w/headwall (C)	Veloc Up (ft/s)	= 8.16
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 7565.46
		HGL Up (ft)	= 7568.18
Embankment		Hw Elev (ft)	= 7569.57
Top Elevation (ft)	= 7571.44	Hw/D (ft)	= 1.19
Top Width (ft)	= 36.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 40.00		

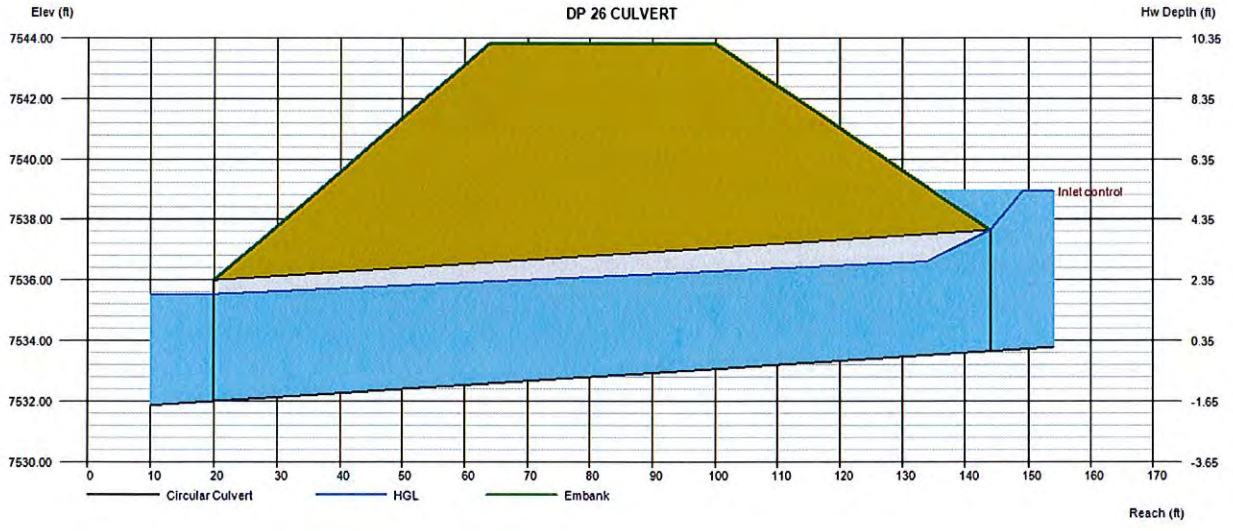


Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Nov 30 2017

DP 26 CULVERT

Invert Elev Dn (ft)	= 7532.00	**Calculations**	
Pipe Length (ft)	= 123.98	Qmin (cfs)	= 0.00
Slope (%)	= 1.33	Qmax (cfs)	= 102.00
Invert Elev Up (ft)	= 7533.65	Tailwater Elev (ft)	= (dc+D)/2
Rise (in)	= 48.0		
Shape	= Circular	**Highlighted**	
Span (in)	= 48.0	Qtotal (cfs)	= 102.00
No. Barrels	= 1	Qpipe (cfs)	= 102.00
n-Value	= 0.012	Qovertop (cfs)	= 0.00
Culvert Type	= Circular Concrete	Veloc Dn (ft/s)	= 8.69
Culvert Entrance	= Square edge w/headwall (C)	Veloc Up (ft/s)	= 9.90
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 7535.53
		HGL Up (ft)	= 7536.71
Embankment		Hw Elev (ft)	= 7538.93
Top Elevation (ft)	= 7543.81	Hw/D (ft)	= 1.32
Top Width (ft)	= 36.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 50.00		

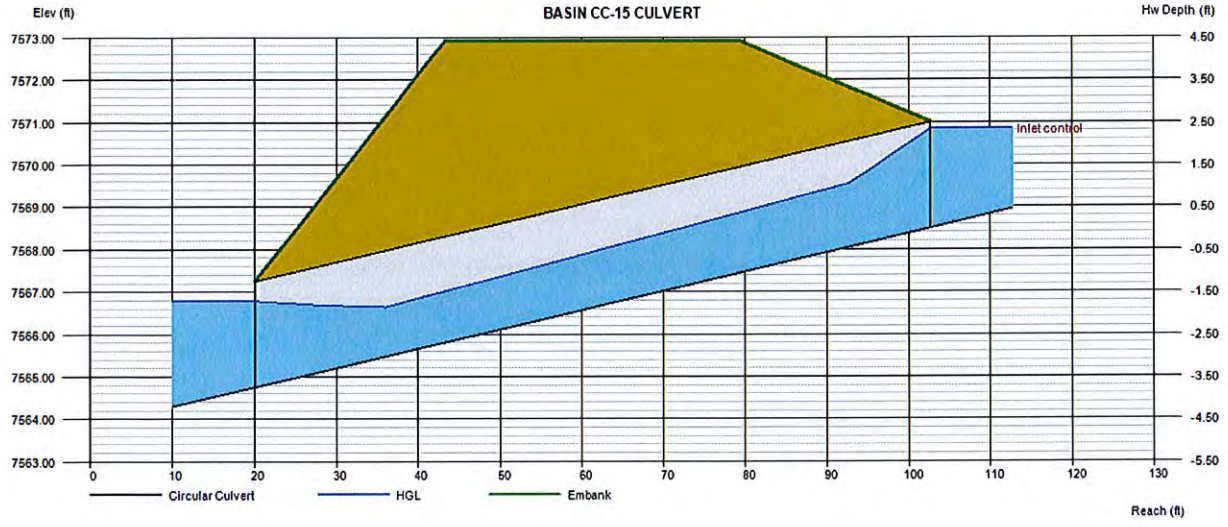


Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Nov 30 2017

BASIN CC-15 CULVERT

Invert Elev Dn (ft)	= 7564.75	**Calculations**	
Pipe Length (ft)	= 82.63	Qmin (cfs)	= 0.00
Slope (%)	= 4.54	Qmax (cfs)	= 21.00
Invert Elev Up (ft)	= 7568.50	Tailwater Elev (ft)	= (dc+D)/2
Rise (in)	= 30.0		
Shape	= Circular	**Highlighted**	
Span (in)	= 30.0	Qtotal (cfs)	= 21.00
No. Barrels	= 1	Qpipe (cfs)	= 21.00
n-Value	= 0.012	Qovertop (cfs)	= 0.00
Culvert Type	= Circular Concrete	Veloc Dn (ft/s)	= 4.92
Culvert Entrance	= Square edge w/headwall (C)	Veloc Up (ft/s)	= 6.54
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 7566.78
		HGL Up (ft)	= 7570.06
Embankment		Hw Elev (ft)	= 7570.84
Top Elevation (ft)	= 7572.92	Hw/D (ft)	= 0.94
Top Width (ft)	= 36.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 50.00		



Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Wednesday, Apr 4 2018

BASIN CC-16 CULVERT

Invert Elev Dn (ft)	=	7568.00
Pipe Length (ft)	=	100.00
Slope (%)	=	1.00
Invert Elev Up (ft)	=	7569.00
Rise (in)	=	30.0
Shape	=	Circular
Span (in)	=	30.0
No. Barrels	=	1
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment

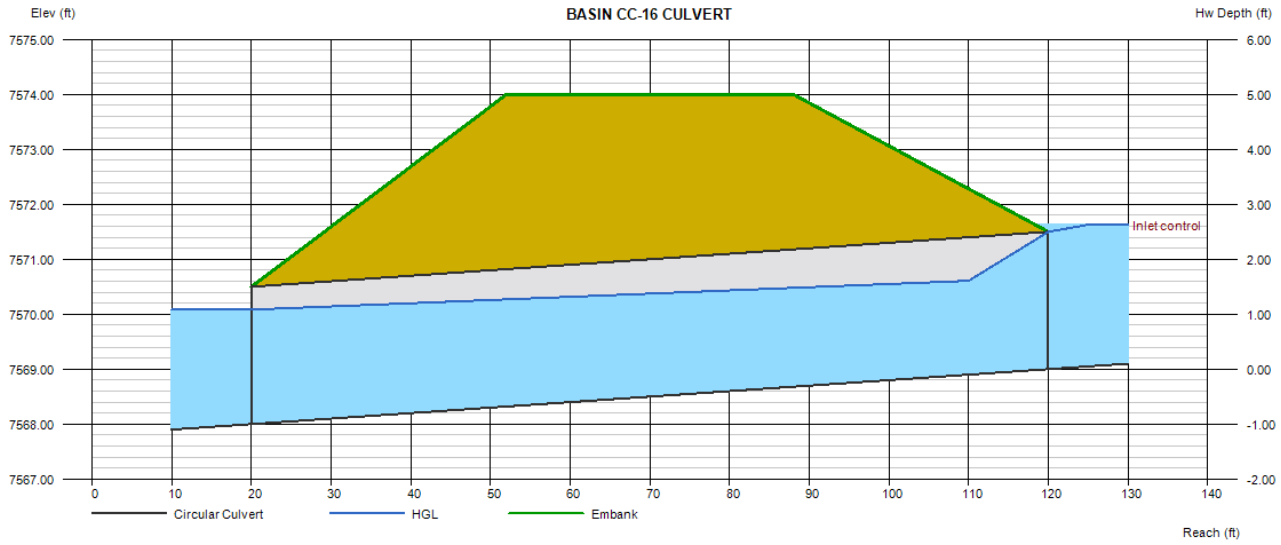
Top Elevation (ft)	=	7574.00
Top Width (ft)	=	36.00
Crest Width (ft)	=	50.00

Calculations

Qmin (cfs)	=	0.00
Qmax (cfs)	=	24.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	24.00
Qpipe (cfs)	=	24.00
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	5.49
Veloc Up (ft/s)	=	6.90
HGL Dn (ft)	=	7570.08
HGL Up (ft)	=	7570.67
Hw Elev (ft)	=	7571.63
Hw/D (ft)	=	1.05
Flow Regime	=	Inlet Control



Culvert Report

DP 30 CULVERT

Invert Elev Dn (ft)	=	7565.50
Pipe Length (ft)	=	100.00
Slope (%)	=	0.50
Invert Elev Up (ft)	=	7566.00
Rise (in)	=	24.0
Shape	=	Circular
Span (in)	=	24.0
No. Barrels	=	1
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment

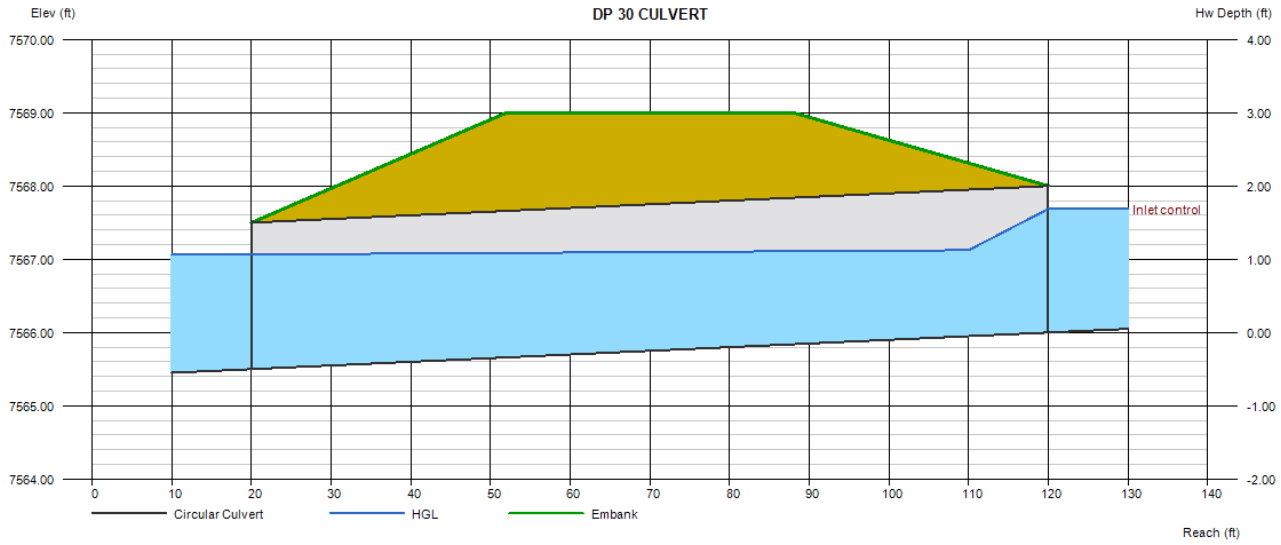
Top Elevation (ft)	=	7569.00
Top Width (ft)	=	36.00
Crest Width (ft)	=	40.00

Calculations

Qmin (cfs)	=	0.00
Qmax (cfs)	=	10.00
Tailwater Elev (ft)	=	(dc+D)/2

Highlighted

Qtotal (cfs)	=	10.00
Qpipe (cfs)	=	10.00
Qovertop (cfs)	=	0.00
Veloc Dn (ft/s)	=	3.79
Veloc Up (ft/s)	=	5.46
HGL Dn (ft)	=	7567.07
HGL Up (ft)	=	7567.13
Hw Elev (ft)	=	7567.69
Hw/D (ft)	=	0.84
Flow Regime	=	Inlet Control

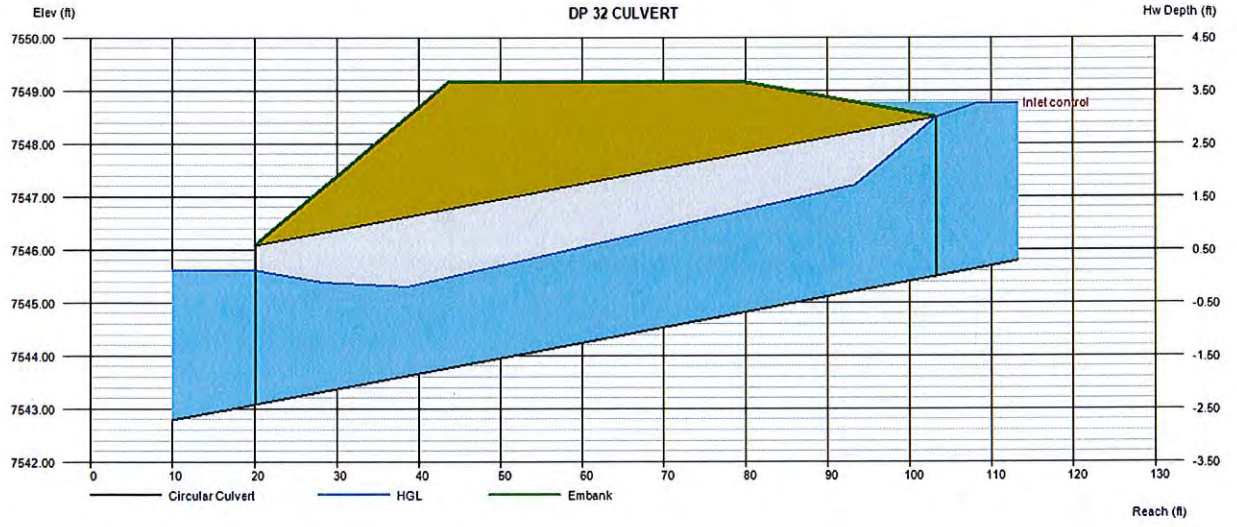


Culvert Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Thursday, Nov 30 2017

DP 32 CULVERT

Invert Elev Dn (ft)	= 7543.08	**Calculations**	
Pipe Length (ft)	= 83.29	Qmin (cfs)	= 0.00
Slope (%)	= 2.91	Qmax (cfs)	= 40.00
Invert Elev Up (ft)	= 7545.50	Tailwater Elev (ft)	= (dc+D)/2
Rise (in)	= 36.0		
Shape	= Circular	**Highlighted**	
Span (in)	= 36.0	Qtotal (cfs)	= 40.00
No. Barrels	= 1	Qpipe (cfs)	= 40.00
n-Value	= 0.012	Qovertop (cfs)	= 0.00
Culvert Type	= Circular Concrete	Veloc Dn (ft/s)	= 6.29
Culvert Entrance	= Square edge w/headwall (C)	Veloc Up (ft/s)	= 7.74
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5	HGL Dn (ft)	= 7545.61
		HGL Up (ft)	= 7547.56
Embankment		Hw Elev (ft)	= 7548.76
Top Elevation (ft)	= 7549.16	Hw/D (ft)	= 1.09
Top Width (ft)	= 36.00	Flow Regime	= Inlet Control
Crest Width (ft)	= 40.00		



Rock Chute Design Data

(Version 4.01 - 04/23/03, Based on Design of Rock Chutes by Robinson, Rice, Kadavy, ASAE, 1998)

Project: Flying Horse Filing No. 1 (Pond 4) **County:** EL Paso
Designer: Marc Whorton **Checked by:**
Date: 11/30/2017 **Date:**

Input Channel Geometry

→ Inlet Channel	→ Chute	→ Outlet Channel
Bw = 20.0 ft.	Bw = 15.0 ft.	Bw = 18.0 ft.
Side slopes = 4.0 (m:1)	Factor of safety = 1.20 (F_s)	Side slopes = 4.0 (m:1)
n-value = 0.035	Side slopes = 4.0 (m:1) → 2.0:1 max.	n-value = 0.035
Bed slope = 0.0100 ft./ft.	Bed slope (4:1) = 0.250 ft./ft. → 2.5:1 max.	Bed slope = 0.0100 ft./ft.
Freeboard = 2.0 ft.	Outlet apron depth, d = 1.5 ft.	Base flow = 40.0 cfs

Design Storm Data (Table 2, NHCP, NRCS Grade Stabilization Structure No. 410)

Drainage area = acres Rainfall = ○ 0 - 3 in. ◉ 3 - 5 in. ○ 5+ in.

Apron elev. --- Inlet = 7436.0 ft. --- Outlet = 7426.0 ft. --- (H_{drop} = 8.5 ft.)

Chute capacity = Q10-year
Total capacity = Q25-year
Minimum capacity (based on a 5-year, 24-hour storm with a 3 - 5 inch rainfall)

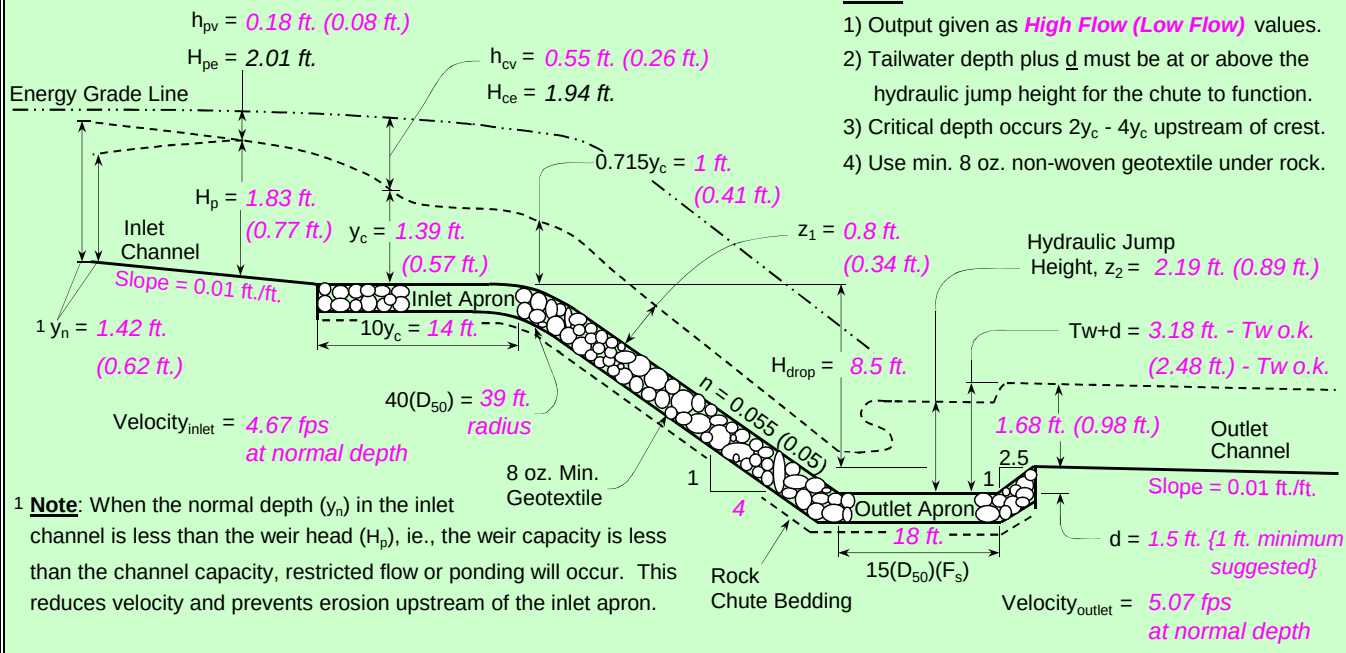
Q_{high} = 170.0 cfs High flow storm through chute → Tw (ft.) = Program

Q_{low} = 40.0 cfs Low flow storm through chute → Tw (ft.) = Program

***Note**: The total required capacity is routed through the chute (principal spillway) or in combination with an auxiliary spillway.*
***Input tailwater (Tw)**:*

0.25

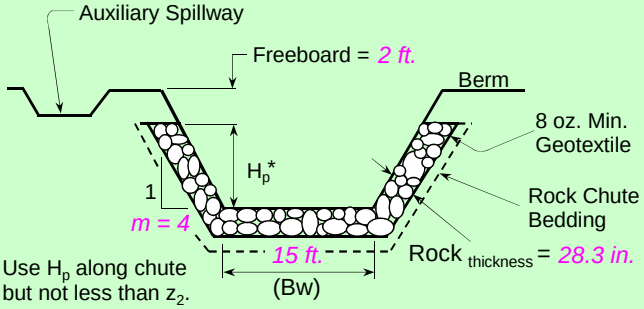
Profile and Cross Section (Output)



Notes:
1) Output given as ***High Flow (Low Flow)*** values.
2) Tailwater depth plus d must be at or above the hydraulic jump height for the chute to function.
3) Critical depth occurs 2y_c - 4y_c upstream of crest.
4) Use min. 8 oz. non-woven geotextile under rock.

[1] **Note**: When the normal depth (y_n) in the inlet channel is less than the weir head (H_p), ie., the weir capacity is less than the channel capacity, restricted flow or ponding will occur. This reduces velocity and prevents erosion upstream of the inlet apron.

Profile Along Centerline of Chute



Typical Cross Section

q_t =	9.32 cfs/ft.	Equivalent unit discharge
F_S =	1.20	Factor of safety (multiplier)
z_1 =	0.8 ft.	Normal depth in chute
n-value =	0.055	Manning's roughness coefficient
$D_{50}(F_s)$ =	14.2 in. (207 lbs. - 50% round / 50% angular)	
2(D_{50})(F_s) =	28.3 in.	Rock chute thickness
Tw + d =	3.18 ft.	Tailwater above outlet apron
z_2 =	2.19 ft.	Hydraulic jump height
*** ***The outlet***	***will***	***function adequately***

High Flow Storm Information

Rock Chute Design - Plan Sheet

(Version 4.0 - 07/10/00, Based on Design of Rock Chutes by Robinson, Rice, Kadavy, ASAE, 1998)

Project: Flying Horse Filing No. 1 (Pond 4) **County:** EL Paso
Designer: Marc Whorton **Checked by:**
Date: 11/30/2017 **Date:**

Design Values

Angular D_{50} dia. = 14.2 in.
$Rock_{chute}$ thickness = 28.3 in.
Inlet apron length = 14 ft.
Outlet apron length = 18 ft.
Radius = 39 ft.
Will bedding be used? Yes

Rock Gradation Envelope

% Passing	Diameter, in. (weight, lbs.)
D_{100}	21 - 28 (704 - 1669)
D_{85}	18 - 26 (458 - 1217)
D_{50}	14 - 21 (209 - 704)
D_{10}	11 - 18 (107 - 458)

Depth (in.) = 6.0

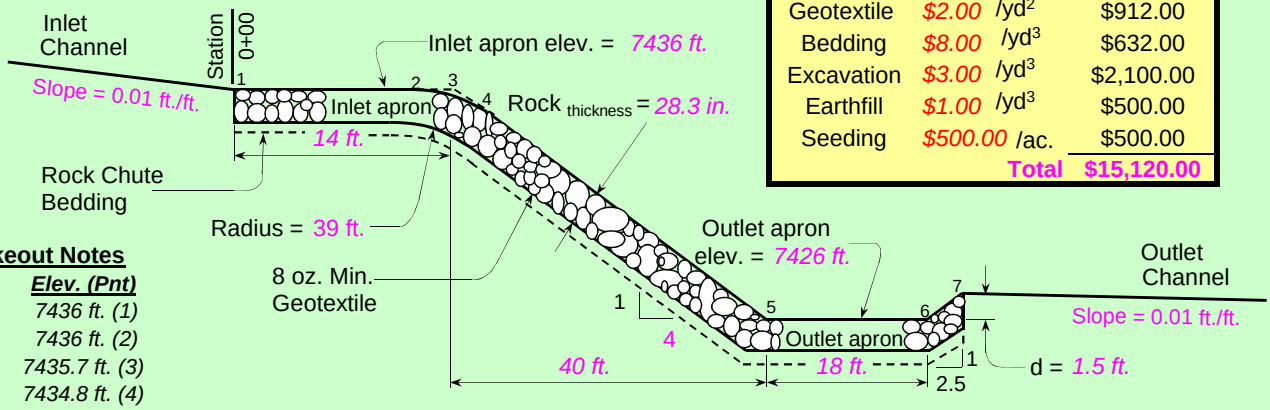
Quantities [a]

Angular Rock = 291 yd^3
Geotextile (8 oz.)[b] = 456 yd^2
Bedding (6 in.) = 79 yd^3
Excavation = 700 yd^3
Earthfill = 500 yd^3
Seeding = 1.0 acres

Notes: [a] *Rock, bedding, and geotextile quantities are determined from the x-section below (neglect radius).*
[b] *Geotextile shall be overlapped (18-in. min.) and anchored (18-in. min. along sides and 24-in. min. on the ends).*

Rock Chute Cost Estimate

Unit	Unit Cost	Cost
Rock	$36.00 /$yd^3$	$10,476.00
Geotextile	$2.00 /$yd^2$	$912.00
Bedding	$8.00 /$yd^3$	$632.00
Excavation	$3.00 /$yd^3$	$2,100.00
Earthfill	$1.00 /$yd^3$	$500.00
Seeding	$500.00 /ac.	$500.00
	Total	**$15,120.00**

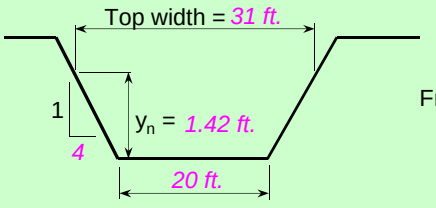


Stakeout Notes

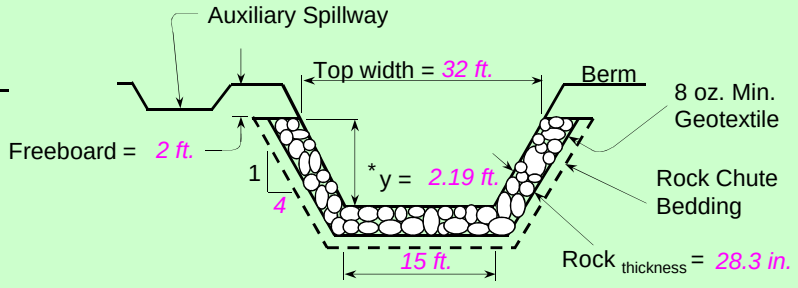
Sta.	Elev. (Pnt)
0+00	7436 ft. (1)
0+9.2	7436 ft. (2)
0+14	7435.7 ft. (3)
0+18.7	7434.8 ft. (4)
0+54	7426 ft. (5)
0+72	7426 ft. (6)
0+75.8	7427.5 ft. (7)

Profile Along Centerline of Rock Chute

****Note**: *The outlet will function adequately*

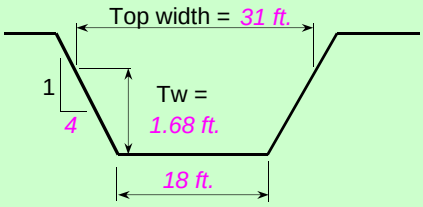


Inlet Channel Cross Section



Rock Chute Cross Section

* Use H_p throughout chute but not less than z_2.



Outlet Channel Cross Section

Profile, Cross Sections, and Quantities

Project: Flying Horse Filing No. 1 (Pond 4)
Location: EL Paso County

U.S. Department of Agriculture
Natural Resources Conservation Service

Designed: Marc Whorton
Drawn: NRCS Standard Dwg.
Traced:
Checked:
Approved by:
Title:
Title:
Sheet No.
of
Drawing No.

Publication No. FHWA-NHI-05-114
September 2005

U.S. Department of Transportation

Federal Highway Administration

Hydraulic Engineering Circular No. 15, Third Edition

Design of Roadside Channels with Flexible Linings

National Highway Institute

Table 2.1. Typical Roughness Coefficients for Selected Linings

Lining Category	Lining Type	Manning's n[1]		
		Maximum	Typical	Minimum
Rigid	Concrete	0.015	0.013	0.011
	Grouted Riprap	0.040	0.030	0.028
	Stone Masonry	0.042	0.032	0.030
	Soil Cement	0.025	0.022	0.020
	Asphalt	0.018	0.016	0.016
Unlined	Bare Soil[2]	0.025	0.020	0.016
	Rock Cut (smooth, uniform)	0.045	0.035	0.025
RECP	Open-weave textile	0.028	0.025	0.022
	Erosion control blankets	0.045	0.035	0.028
	Turf reinforcement mat	0.036	0.030	0.024

[1]Based on data from Kouwen, et al. (1980), Cox, et al. (1970), McWhorter, et al. (1968) and Thibodeaux (1968).
[2]Minimum value accounts for grain roughness. Typical and maximum values incorporate varying degrees of form roughness.

Table 2.2. Typical Roughness Coefficients for Riprap, Cobble, and Gravel Linings

Lining Category	Lining Type	Manning's n for Selected Flow Depths[1]		
		0.15 m (0.5 ft)	0.50 m (1.6 ft)	1.0 m (3.3 ft)
Gravel Mulch	D_{50} = 25 mm (1 in.)	0.040	0.033	0.031
	D_{50} = 50 mm (2 in.)	0.056	0.042	0.038
Cobbles	D_{50} = 0.10 m (0.33 ft)	--[2]	0.055	0.047
Rock Riprap	D_{50} = 0.15 m (0.5 ft)	--[2]	0.069	0.056
	D_{50} = 0.30 m (1.0 ft)	--[2]	--[2]	0.080

[1]Based on Equation 6.1 (Blodgett and McConaughy, 1985). Manning's n estimated assuming a trapezoidal channel with 1:3 side slopes and 0.6 m (2 ft) bottom width.
[2]Shallow relative depth (average depth to D_{50} ratio less than 1.5) requires use of Equation 6.2 (Bathurst, et al., 1981) and is slope-dependent. See Section 6.1.

2.2 SHEAR STRESS

2.2.1 Equilibrium Concepts

Most highway drainage channels cannot tolerate bank instability and possible lateral migration. Stable channel design concepts focus on evaluating and defining a channel configuration that will perform within acceptable limits of stability. Methods for evaluation and definition of a stable configuration depend on whether the channel boundaries can be viewed as:

- essentially rigid (static)
- movable (dynamic).

In the first case, stability is achieved when the material forming the channel boundary effectively resists the erosive forces of the flow. Under such conditions the channel bed and banks are in

protected. Therefore permissible shear stress is not significantly affected by the erodibility of the underlying soil. However, if the lining moves, the underlying soil will be exposed to the erosive force of the flow.

Table 2.3 provides typical examples of permissible shear stress for selected lining types. Representative values for different soil types are based on the methods found in Chapter 4 while those for gravel mulch and riprap are based on methods found in Chapter 7. Vegetative and RECP lining performance relates to how well they protect the underlying soil from shear stresses so these linings do not have permissible shear stresses independent of soil types. Chapters 4 (vegetation) and 5 (RECPs) describe the methods for analyzing these linings. Permissible shear stress for gabion mattresses depends on rock size and mattress thickness as is described in Section 7.2.

Table 2.3. Typical Permissible Shear Stresses for Bare Soil and Stone Linings

		Permissible Shear Stress	
Lining Category	Lining Type	N/m^2	lb/ft^2
Bare Soil[1] Cohesive (PI = 10)	Clayey sands	1.8-4.5	0.037-0.095
	Inorganic silts	1.1-4.0	0.027-0.11
	Silty sands	1.1-3.4	0.024-0.072
Bare Soil[1] Cohesive (PI $\geq$ 20)	Clayey sands	4.5	0.094
	Inorganic silts	4.0	0.083
	Silty sands	3.5	0.072
	Inorganic clays	6.6	0.14
Bare Soil[2] Non-cohesive (PI < 10)	Finer than coarse sand D_{75}<1.3 mm (0.05 in)	1.0	0.02
	Fine gravel D_{75}=7.5 mm (0.3 in)	5.6	0.12
	Gravel D_{75}=15 mm (0.6 in)	11	0.24
Gravel Mulch[3]	Coarse gravel D_{50} = 25 mm (1 in)	19	0.4
	Very coarse gravel D_{50} = 50 mm (2 in)	38	0.8
Rock Riprap[3]	D_{50} = 0.15 m (0.5 ft)	113	2.4
	D_{50} = 0.30 m (1.0 ft)	227	4.8

[1]Based on Equation 4.6 assuming a soil void ratio of 0.5 (USDA, 1987).
[2]Based on Equation 4.5 derived from USDA (1987)
[3]Based on Equation 6.7 with Shield's parameter equal to 0.047.

2.3 DESIGN PARAMETERS

2.3.1 Design Discharge Frequency

Design flow rates for permanent roadside and median drainage channel linings usually have a 5 or 10-year return period. A lower return period flow is allowable if a transitional lining is to be used, typically the mean annual storm (approximately a 2-year return period, i.e., 50 percent probability of occurrence in a year). Transitional channel linings are often used during the establishment of vegetation. The probability of damage during this relatively short time is low,

- significant uncertainty regarding the design discharge
- consequences of failure are high

The basic procedure for flexible lining design consists of the following steps and is summarized in Figure 3.1. (An alternative process for determining an allowable discharge given slope and shape is presented in Section 3.6.)

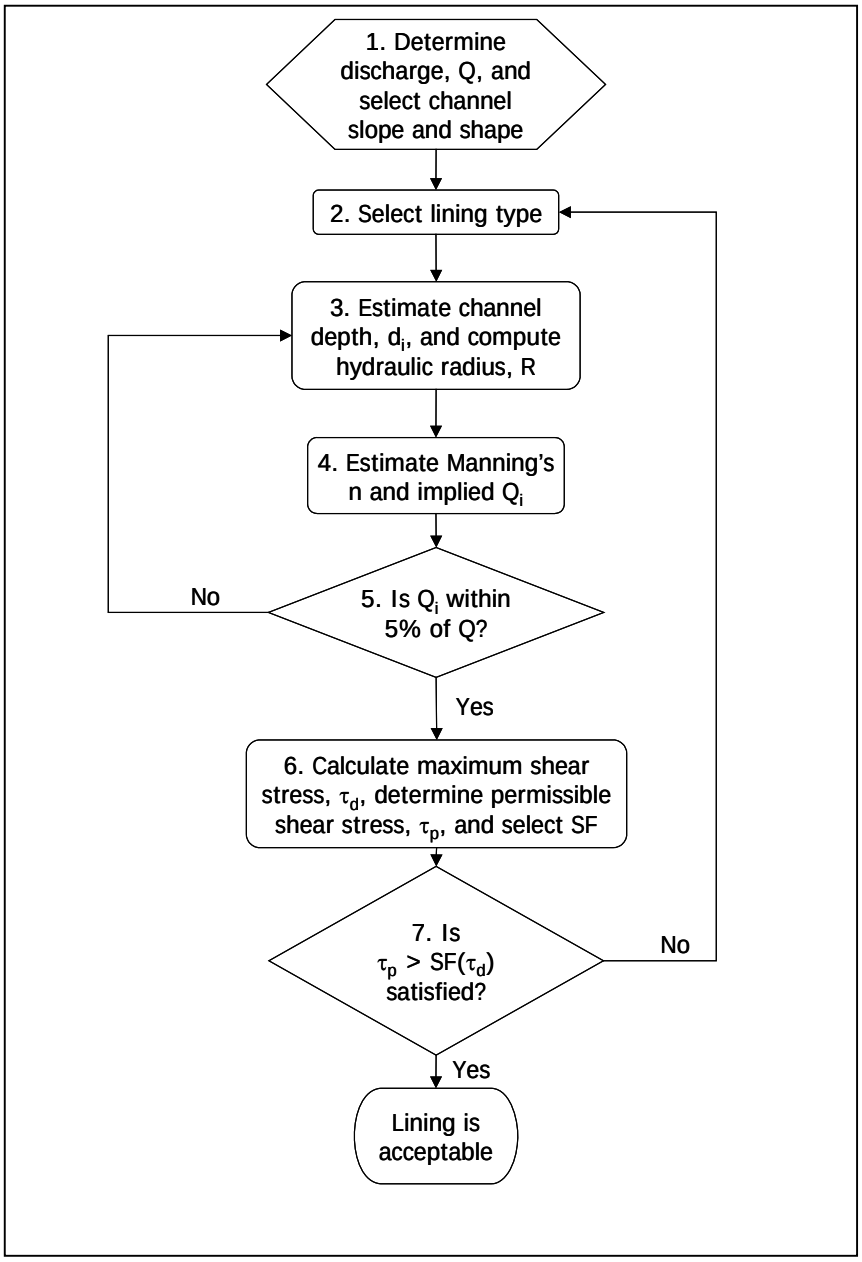
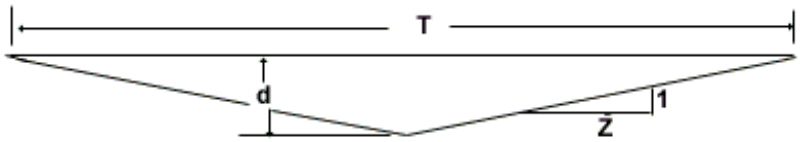


Figure 3.1. Flexible Channel Lining Design Flow Chart

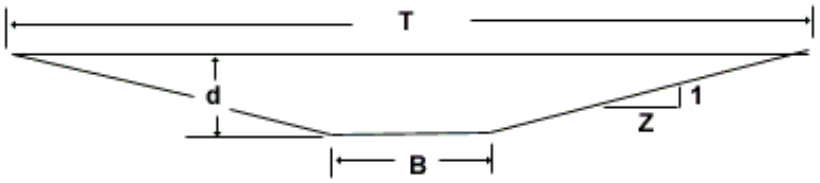
APPENDIX B: CHANNEL GEOMETRY EQUATIONS

V- SHAPE



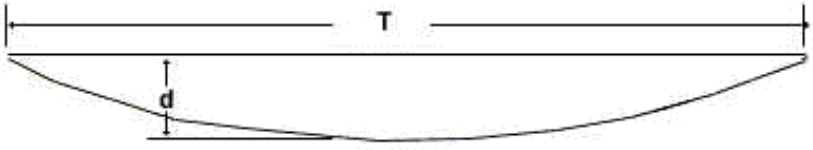
$$A = Zd^2$$
$$p = 2d\sqrt{Z^2 + 1}$$
$$T = 2dZ$$

TRAPEZOIDAL



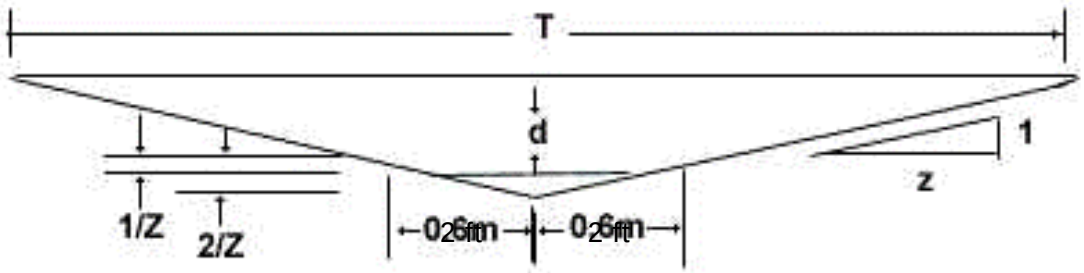
$$A = Bd + Zd^2$$
$$P = B + 2d\sqrt{z^2 + 1}$$
$$T = B + 2dZ$$

PARABOLIC



$$A = \frac{2}{3}Td$$
$$P = \frac{1}{2}\sqrt{16d^2 + T^2} + \left(\frac{T^2}{8d}\right)\ln_e\left(\frac{4d + \sqrt{16d^2 + T^2}}{T}\right)$$
$$T = 1.5\frac{A}{d}$$

V-SHAPE WITH ROUNDED BOTTOM



2 CASES

No. 1

If d ≤ 1/Z, then:

$$A = \frac{8}{3} d\sqrt{dZ}$$

$$P = 2Z\ln_e\left(\sqrt{\frac{d}{Z}} + \sqrt{1+\frac{d}{Z}}\right) + 2\sqrt{d^2 + dZ}$$

$$T = 4\sqrt{dZ}$$

No. 2

If d > 1/Z, then:

$$A = \frac{8}{3} d + 4\left(d - \frac{1}{Z}\right) + Z\left(d - \frac{1}{Z}\right)^2$$

$$P = 2Z\ln_e\left(\frac{1+\sqrt{Z^2+1}}{Z}\right) + 2\frac{\sqrt{Z^2+1}}{Z} + 2\left(d - \frac{1}{Z}\right)\sqrt{1+Z^2}$$

$$T = 4 + 2Z\left(d - \frac{1}{Z}\right)$$

Note: The equations for V-shape with rounded bottom only apply in customary units for a channel with a 4 ft wide rounded bottom.

Channel Report

Natural Channel to Pond 12 (channel approaching pond)

Trapezoidal

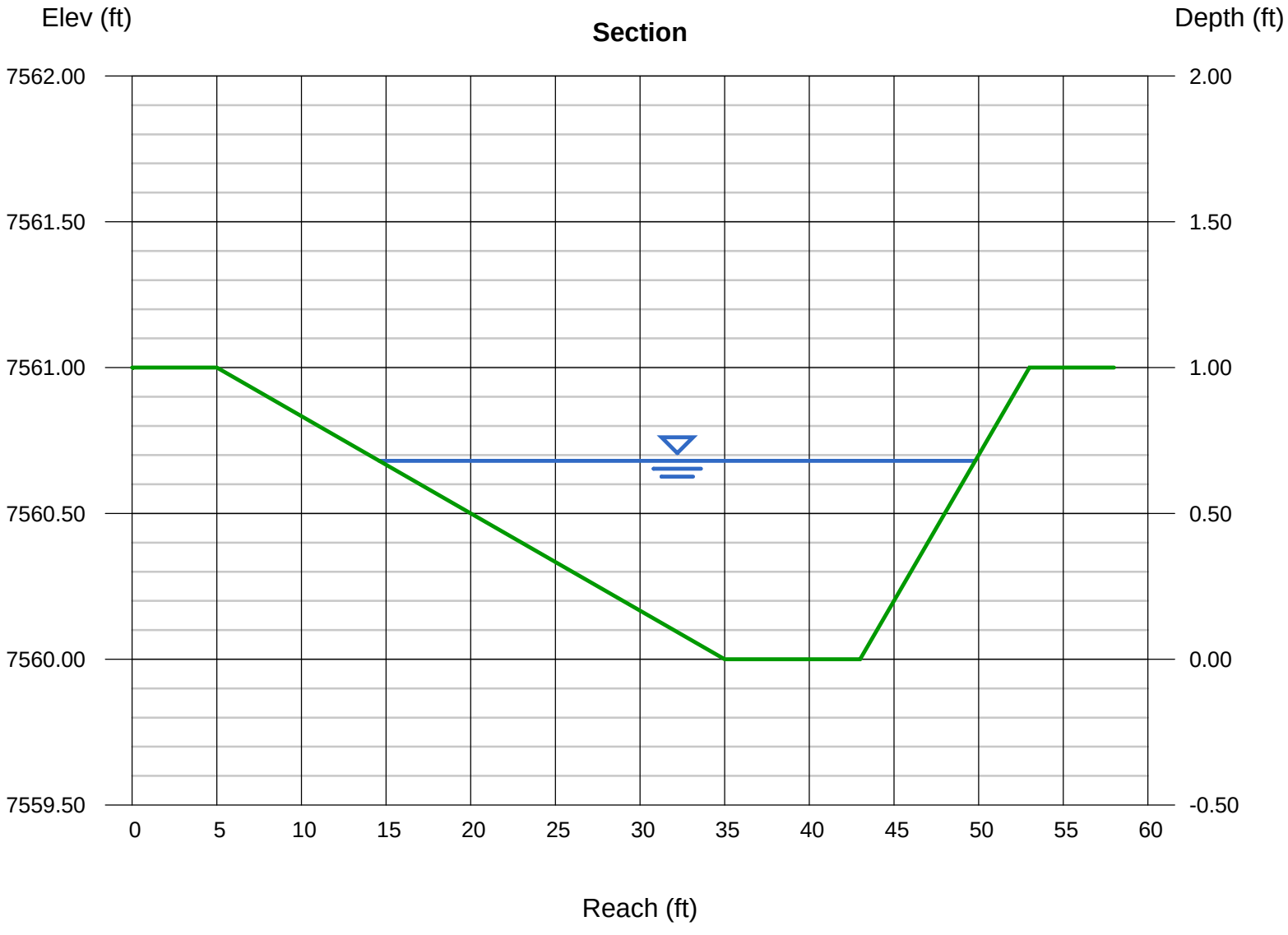
Bottom Width (ft)	= 8.00
Side Slopes (z:1)	= 30.00, 10.00
Total Depth (ft)	= 1.00
Invert Elev (ft)	= 7560.00
Slope (%)	= 2.70
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 65.00

Highlighted

Depth (ft)	= 0.68
Q (cfs)	= 65.00
Area (sqft)	= 14.69
Velocity (ft/s)	= 4.43
Wetted Perim (ft)	= 35.25
Crit Depth, Yc (ft)	= 0.75
Top Width (ft)	= 35.20
EGL (ft)	= 0.98



Channel Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Wednesday, Apr 4 2018

Natural Channel to Pond 12 (Just north of Stagecoach Rd.)

Trapezoidal

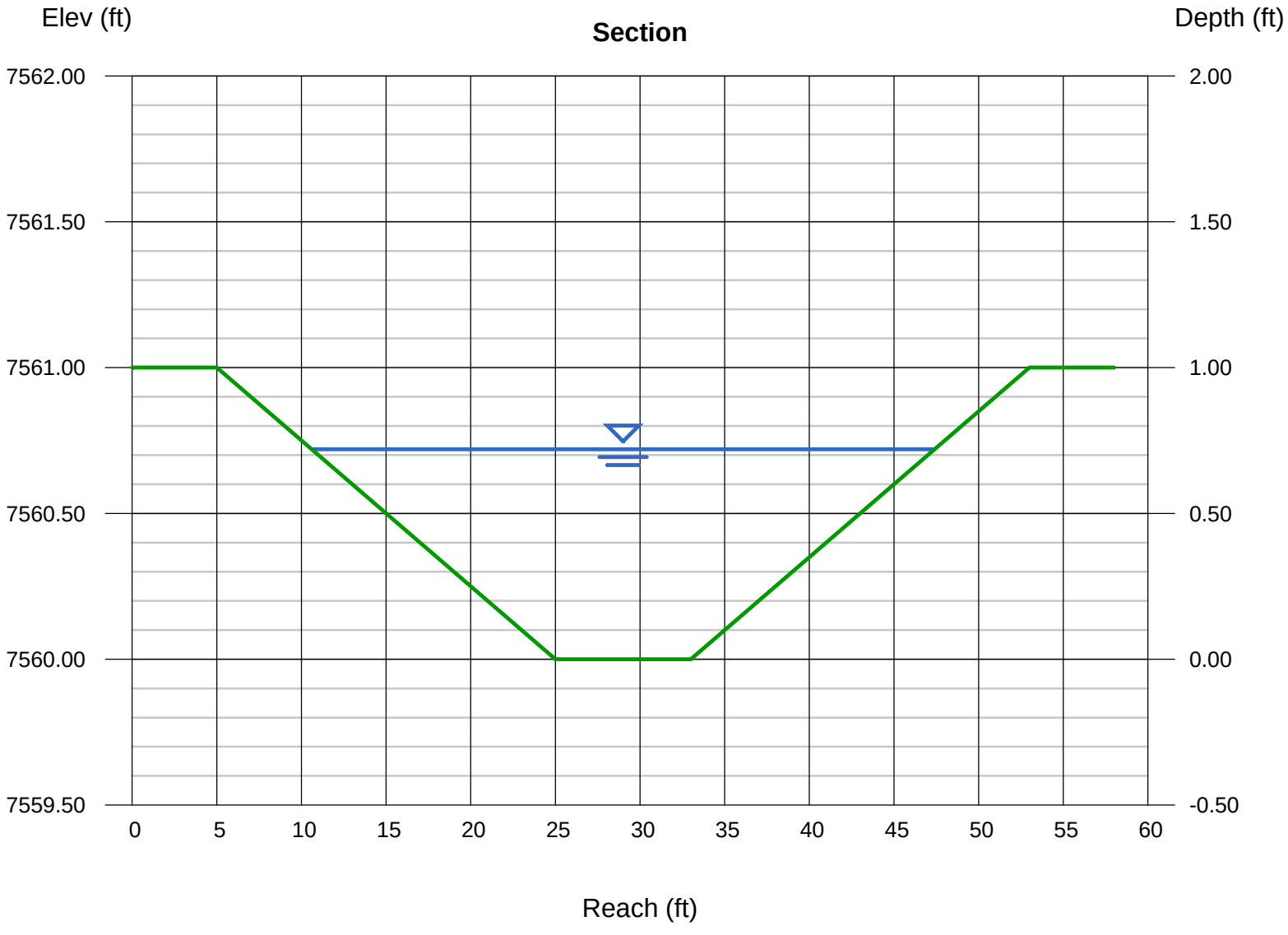
Bottom Width (ft)	= 8.00
Side Slopes (z:1)	= 20.00, 20.00
Total Depth (ft)	= 1.00
Invert Elev (ft)	= 7560.00
Slope (%)	= 1.00
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 45.00

Highlighted

Depth (ft)	= 0.72
Q (cfs)	= 45.00
Area (sqft)	= 16.13
Velocity (ft/s)	= 2.79
Wetted Perim (ft)	= 36.84
Crit Depth, Yc (ft)	= 0.63
Top Width (ft)	= 36.80
EGL (ft)	= 0.84



Channel Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Wednesday, Apr 4 2018

Natural Channel running through lots 58-59

Trapezoidal

Bottom Width (ft)	= 5.00
Side Slopes (z:1)	= 6.00, 6.00
Total Depth (ft)	= 1.00
Invert Elev (ft)	= 7600.00
Slope (%)	= 5.00
N-Value	= 0.030

Calculations

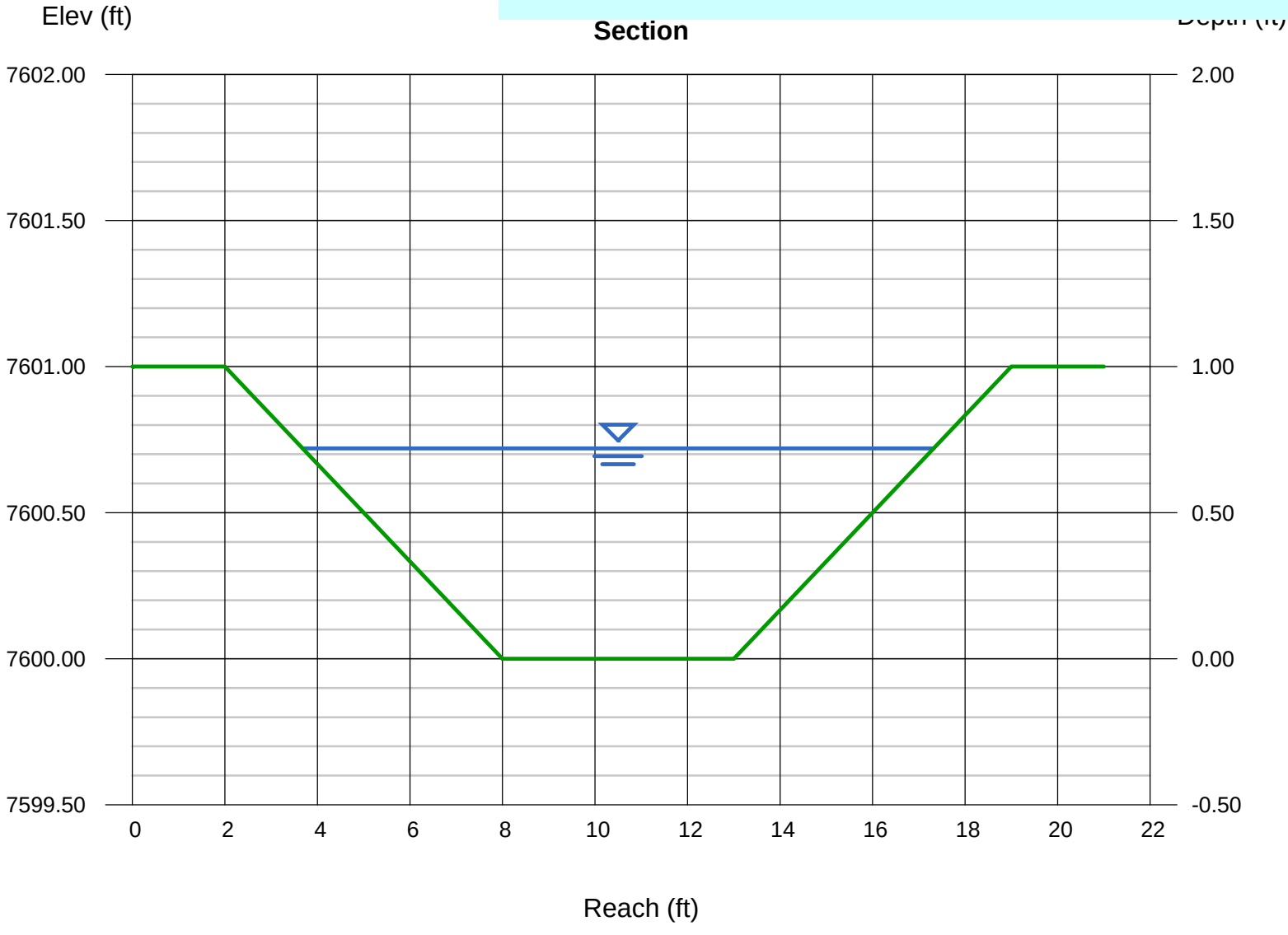
Compute by:	Known Q
Known Q (cfs)	= 46.00

Highlighted

Depth (ft)	= 0.72
Q (cfs)	= 46.00
Area (sqft)	= 6.71
Velocity (ft/s)	= 6.86
Wetted Perim (ft)	= 13.76
Crit Depth, Yc (ft)	= 0.96
Top Width (ft)	= 13.64
EGL (ft)	= 1.45

The velocity appears to be greater than permissible velocities for native grass (typically 4 fps for short native grass). Improvements appears to be required for erosion protection. Check the other channel reports and update narrative to discuss the findings.

Include the Froude number in all the channel reports. See DCM 6.5.2



Channel Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Wednesday, Apr 4 2018

Natural Channel running through lots 46-50

Trapezoidal

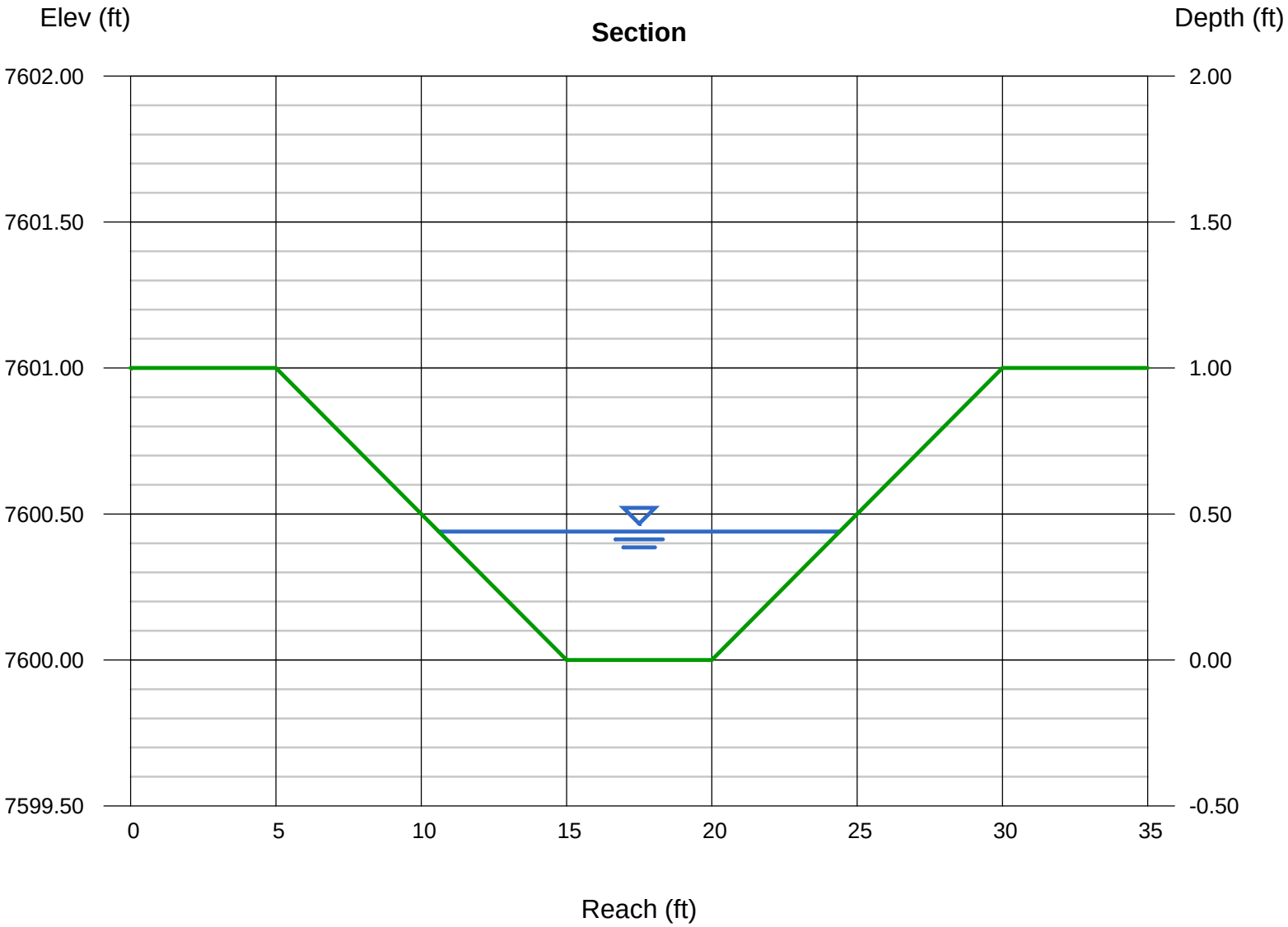
Bottom Width (ft)	= 5.00
Side Slopes (z:1)	= 10.00, 10.00
Total Depth (ft)	= 1.00
Invert Elev (ft)	= 7600.00
Slope (%)	= 6.00
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 22.00

Highlighted

Depth (ft)	= 0.44
Q (cfs)	= 22.00
Area (sqft)	= 4.14
Velocity (ft/s)	= 5.32
Wetted Perim (ft)	= 13.84
Crit Depth, Yc (ft)	= 0.59
Top Width (ft)	= 13.80
EGL (ft)	= 0.88



Channel Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Wednesday, Apr 4 2018

Natural Channel from DP 15

Trapezoidal

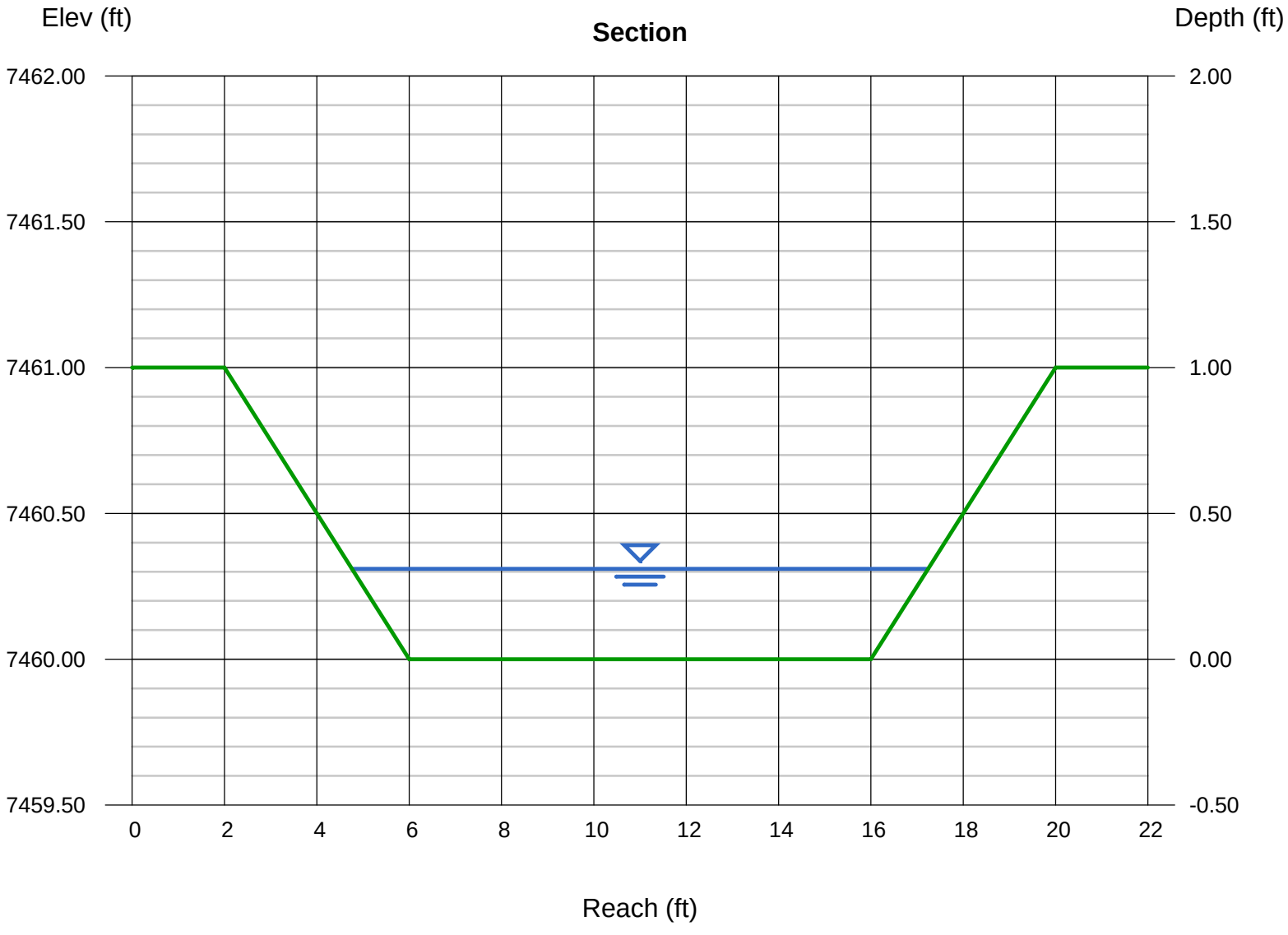
Bottom Width (ft)	= 10.00
Side Slopes (z:1)	= 4.00, 4.00
Total Depth (ft)	= 1.00
Invert Elev (ft)	= 7460.00
Slope (%)	= 4.50
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 15.00

Highlighted

Depth (ft)	= 0.31
Q (cfs)	= 15.00
Area (sqft)	= 3.48
Velocity (ft/s)	= 4.30
Wetted Perim (ft)	= 12.56
Crit Depth, Yc (ft)	= 0.40
Top Width (ft)	= 12.48
EGL (ft)	= 0.60



Channel Report

Hydraflow Express Extension for Autodesk® AutoCAD® Civil 3D® by Autodesk, Inc. Wednesday, Apr 4 2018

Natural Channel from DP 14

Trapezoidal

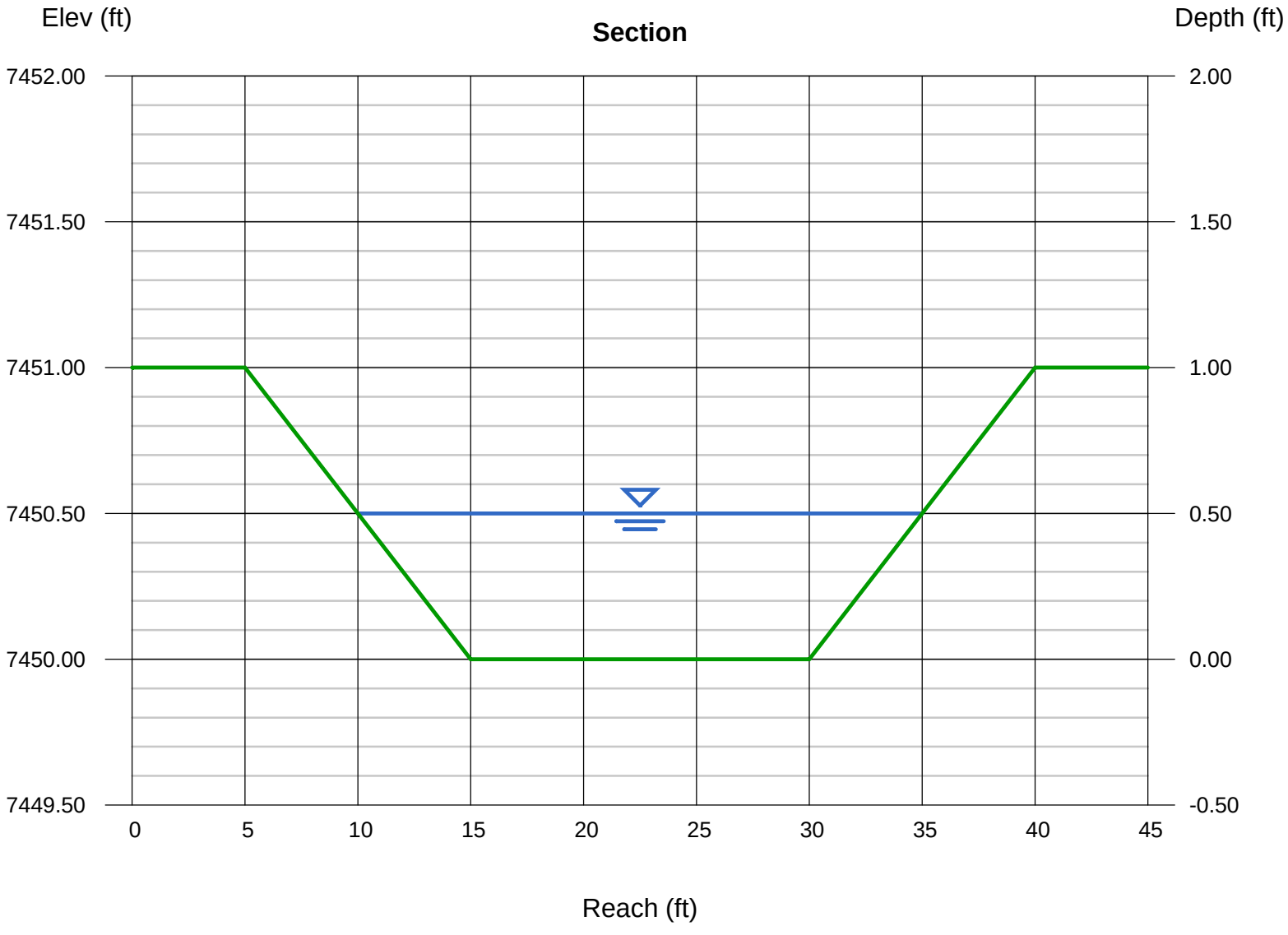
Bottom Width (ft)	= 15.00
Side Slopes (z:1)	= 10.00, 10.00
Total Depth (ft)	= 1.00
Invert Elev (ft)	= 7450.00
Slope (%)	= 4.50
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 56.00

Highlighted

Depth (ft)	= 0.50
Q (cfs)	= 56.00
Area (sqft)	= 10.00
Velocity (ft/s)	= 5.60
Wetted Perim (ft)	= 25.05
Crit Depth, Yc (ft)	= 0.66
Top Width (ft)	= 25.00
EGL (ft)	= 0.99



Channel Report

Natural Channel from DP 2

Trapezoidal

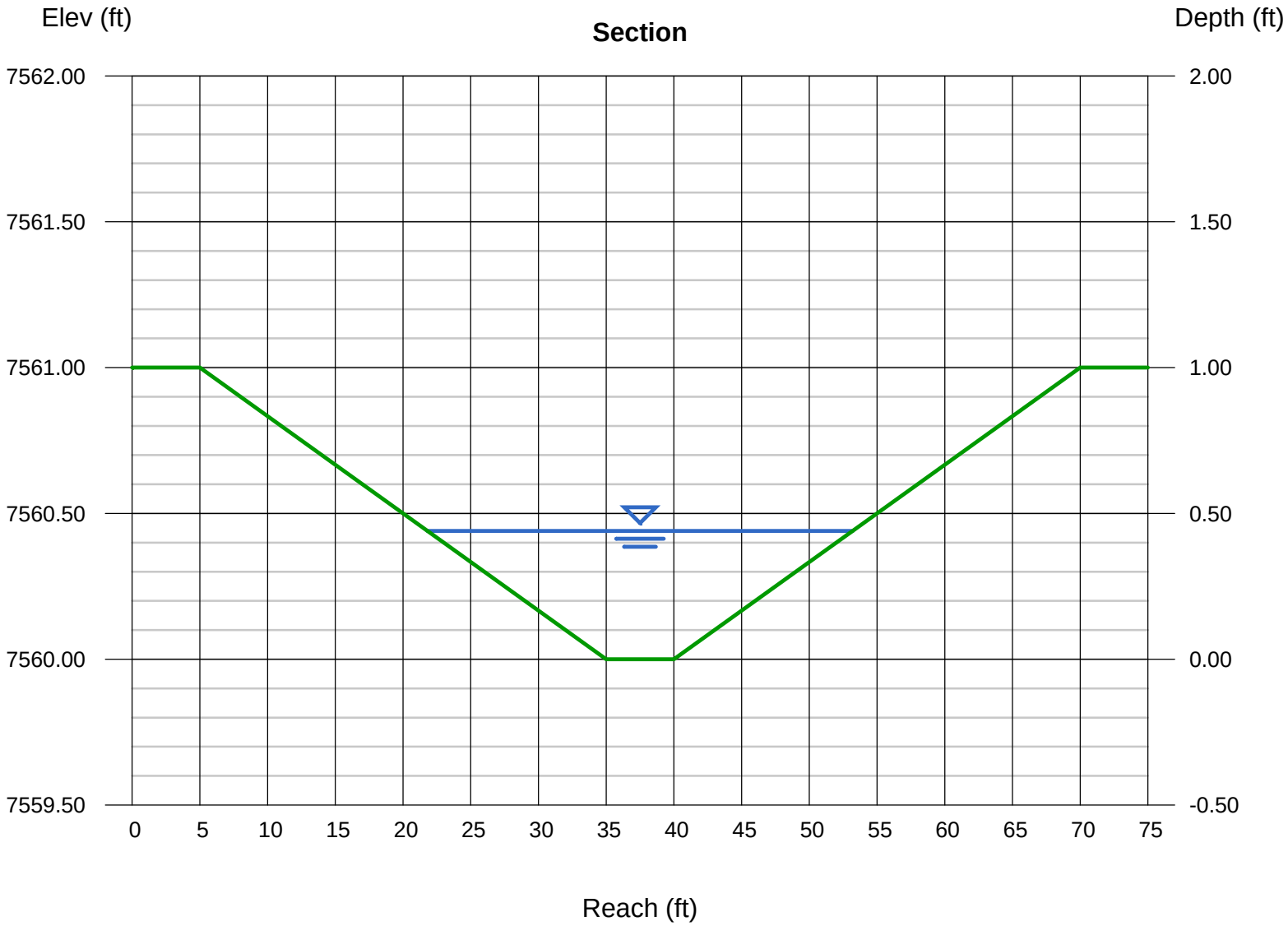
Bottom Width (ft)	= 5.00
Side Slopes (z:1)	= 30.00, 30.00
Total Depth (ft)	= 1.00
Invert Elev (ft)	= 7560.00
Slope (%)	= 5.00
N-Value	= 0.030

Calculations

Compute by:	Known Q
Known Q (cfs)	= 35.00

Highlighted

Depth (ft)	= 0.44
Q (cfs)	= 35.00
Area (sqft)	= 8.01
Velocity (ft/s)	= 4.37
Wetted Perim (ft)	= 31.41
Crit Depth, Yc (ft)	= 0.54
Top Width (ft)	= 31.40
EGL (ft)	= 0.74



$$H_a = \frac{(H + Y_n)}{2}$$ Equation 9-19

Where the maximum value of H_a shall not exceed H, and:

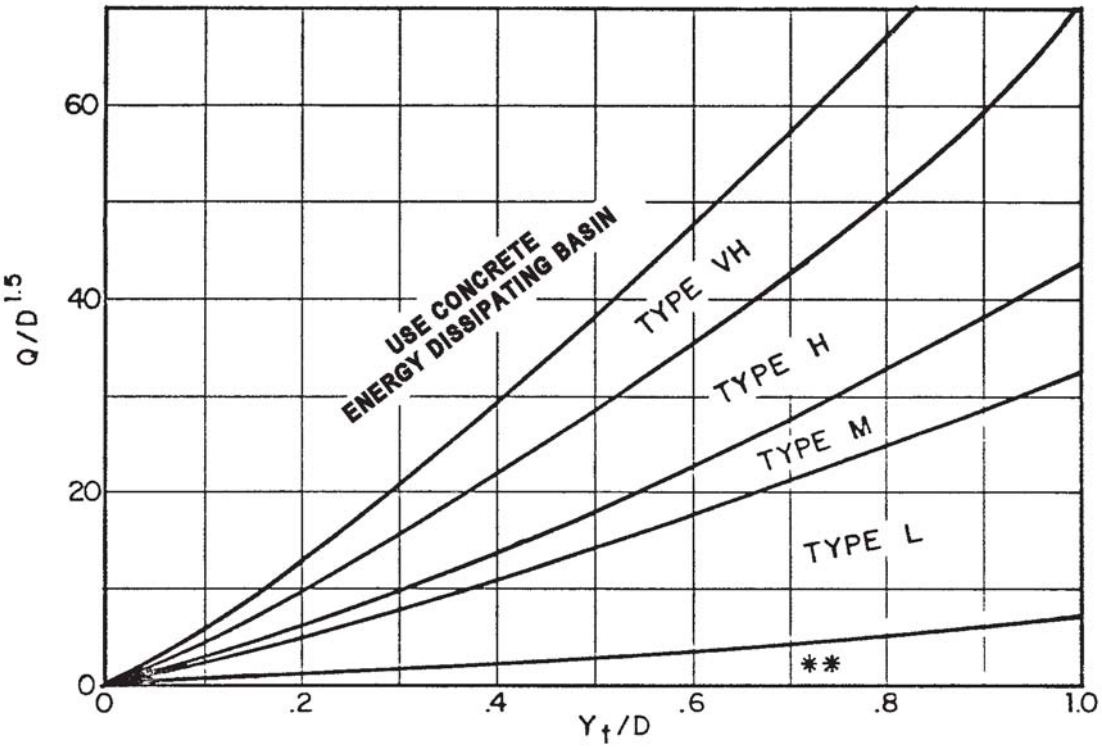
D_a = parameter to use in place of D in Figure 9-38 when flow is supercritical (ft)

D_c = diameter of circular culvert (ft)

H_a = parameter to use in place of H in Figure 9-39 when flow is supercritical (ft)

H = height of rectangular culvert (ft)

Y_n = normal depth of supercritical flow in the culvert (ft)



Use D_a instead of D whenever flow is supercritical in the barrel.
** Use Type L for a distance of 3D downstream.

Figure 9-38. Riprap erosion protection at circular conduit outlet (valid for Q/D2.5 ≤ 6.0)

RIP-RAP CALCULATIONS

Design Point	Flow (cfs)	Tailwater Depth (ft.) (See Culvert Reports)	Pipe Diameter (ft.)	$Q / D^{1.5}$	Y_t / D	Rock Type (See Fig. 9-38)	Rock Size (d50) (in.)
DP-1	11	1.62	2.0	3.9	0.8	Type L	9"
DP-2	35	2.25	2.5	8.9	0.9	Type L	9"
Fire Station Dwy.	5	1.18	1.5	2.7	0.8	Type L	9"
DP-4	11	1.40	1.5	6.0	0.9	Type L	9"
DP-5	15	1.45	1.5	8.2	1.0	Type L	9"
DP-7	38	1.99	2.5	9.6	0.8	Type L	9"
DP-8	284	3.46	4.0	35.5	0.9	Type M	12"
DP-9	23	1.61	2.0	8.1	0.8	Type L	9"
DP-10	144	3.08	3.5	22.0	0.9	Type L	9"
DP-11	36	1.76	2.0	12.7	0.9	Type L	9"
DP-12	44	2.58	3.0	8.5	0.9	Type L	9"
DP-14	56	1.78	2.0	19.8	0.9	Type L	9"
DP-15	15	1.28	1.5	8.2	0.9	Type L	9"
DP-24	45	2.59	3.0	8.7	0.9	Type L	9"
DP-26	102	3.53	4.0	12.8	0.9	Type L	9"
Basin CC-15	21	2.03	2.5	5.3	0.8	Type L	9"
Basin CC-16	24	2.08	2.5	6.1	0.8	Type L	9"
DP-30	10	1.57	2.0	3.5	0.8	Type L	9"
DP-32	40	2.53	3.0	7.7	0.8	Type L	9"

DETENTION FACILITY CALCULATIONS

CLASSIC SM
CONSULTING
ENGINEERS & SURVEYORS

Design Procedure Form: Extended Detention Basin (EDB)

UD-BMP (Version 3.06, November 2016)

Designer: Marc A. Whorton, P.E.
Company: Classic Consulting
Date: November 27, 2017
Project: Flying Horse North Filing No. 1
Location: Pond 1

1. Basin Storage Volume	
A) Effective Imperviousness of Tributary Area, I_a	I_a = 20.0 %
B) Tributary Area's Imperviousness Ratio ($i = I_a / 100$)	i = 0.200
C) Contributing Watershed Area	Area = 21.800 ac
D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm	d_6 = 0.42 in
E) Design Concept (Select EURV when also designing for flood control)	Choose One: ○ Water Quality Capture Volume (WQCV) ◉ Excess Urban Runoff Volume (EURV)
F) Design Volume (WQCV) Based on 40-hour Drain Time ($V_{DESIGN} = (1.0 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i) / 12 * Area$)	V_{DESIGN}= 0.210 ac-ft
G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume ($V_{WQCV\ OTHER} = (d_6 * (V_{DESIGN}/0.43))$)	$V_{DESIGN\ OTHER}$= 0.205 ac-ft
H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired)	$V_{DESIGN\ USER}$= ac-ft
I) Predominant Watershed NRCS Soil Group	Choose One: ○ A ◉ B ○ C / D
J) Excess Urban Runoff Volume (EURV) Design Volume For HSG A: $EURV_A = 1.68 * i^{1.28}$ For HSG B: $EURV_B = 1.36 * i^{1.08}$ For HSG C/D: $EURV_{C/D} = 1.20 * i^{1.08}$	EURV = 0.434 ac-f t
2. Basin Shape: Length to Width Ratio (A basin length to width ratio of at least 2:1 will improve TSS reduction.)	L : W = 2.0 : 1
3. Basin Side Slopes	
A) Basin Maximum Side Slopes (Horizontal distance per unit vertical, 4:1 or flatter preferred)	Z = 4.00 ft / ft
4. Inlet	
A) Describe means of providing energy dissipation at concentrated inflow locations:	No concentrated inflow. Flows will enter as sheet flow

Design Procedure Form: Extended Detention Basin (EDB)

Designer: Marc A. Whorton, P.E.
Company: Classic Consulting
Date: November 27, 2017
Project: Flying Horse North Filing No. 1
Location: Pond 1

5. Forebay

A) Minimum Forebay Volume (V_{FMIN} = 2% of the WQCV) — V_{FMIN} = 0.004 ac-ft

B) Actual Forebay Volume — V_F = 0.005 ac-ft

C) Forebay Depth (D_F = 18 inch maximum) — D_F = 12.0 in

D) Forebay Discharge

i) Undetained 100-year Peak Discharge — Q_{100} = 38.00 cfs

ii) Forebay Discharge Design Flow ($Q_F = 0.02 * Q_{100}$) — Q_F = 0.76 cfs

E) Forebay Discharge Design

Choose One
- ○ Berm With Pipe
- ◉ Wall with Rect. Notch
- ○ Wall with V-Notch Weir

(flow too small for berm w/ pipe)

F) Discharge Pipe Size (minimum 8-inches) — Calculated D_P = in

G) Rectangular Notch Width — Calculated W_N = 5.1 in

6. Trickle Channel

A) Type of Trickle Channel

Choose One
- ○ Concrete
- ◉ Soft Bottom

PROVIDE A CONSISTENT LONGITUDINAL SLOPE FROM FOREBAY TO MICROPOOL WITH NO MEANDERING. RIPRAP AND SOIL RIPRAP LINED CHANNELS ARE NOT RECOMMENDED. MINIMUM DEPTH OF 1.5 FEET

F) Slope of Trickle Channel — S = 0.0100 ft / ft

7. Micropool and Outlet Structure

A) Depth of Micropool (2.5-feet minimum) — D_M = 2.5 ft

B) Surface Area of Micropool (10 ft^2 minimum) — A_M = 50 sq ft

C) Outlet Type

Choose One
- ◉ Orifice Plate
- ○ Other (Describe):

D) Smallest Dimension of Orifice Opening Based on Hydrograph Routing (Use UD-Detention) — $D_{orifice}$ = 1.13 inches

E) Total Outlet Area — A_{ot} = 5.16 square inches

Design Procedure Form: Extended Detention Basin (EDB)

Designer: Marc A. Whorton, P.E.
Company: Classic Consulting
Date: November 27, 2017
Project: Flying Horse North Filing No. 1
Location: Pond 1

8. Initial Surcharge Volume	
A) Depth of Initial Surcharge Volume (Minimum recommended depth is 4 inches)	D_{IS} = 6 in
B) Minimum Initial Surcharge Volume (Minimum volume of 0.3% of the WQCV)	V_{IS} = cu ft
C) Initial Surcharge Provided Above Micropool	V_s= 25.0 cu ft
9. Trash Rack	
A) Water Quality Screen Open Area: $A_t = A_{ot} * 38.5*(e^{-0.095D})$	A_t = 179 square inches
B) Type of Screen (If specifying an alternative to the materials recommended in the USDCM, indicate "other" and enter the ratio of the total open are to the total screen are for the material specified.)	*S.S. Well Screen with 60% Open Area*
Other (Y/N): N	
C) Ratio of Total Open Area to Total Area (only for type 'Other')	User Ratio =
D) Total Water Quality Screen Area (based on screen type)	A_{total} = 298 sq. in.
E) Depth of Design Volume (EURV or WQCV) (Based on design concept chosen under 1E)	H= 2.75 feet
F) Height of Water Quality Screen (H_{TR})	H_{TR}= 61 inches
G) Width of Water Quality Screen Opening ($W_{opening}$) (Minimum of 12 inches is recommended)	$W_{opening}$ = 12.0 inches

Design Procedure Form: Extended Detention Basin (EDB)

Designer: Marc A. Whorton, P.E.
Company: Classic Consulting
Date: November 27, 2017
Project: Flying Horse North Filing No. 1
Location: Pond 1

10. Overflow Embankment	
A) Describe embankment protection for 100-year and greater overtopping:	Soil Rip-Rap
B) Slope of Overflow Embankment (Horizontal distance per unit vertical, 4:1 or flatter preferred)	4.00
11. Vegetation	Choose One: ○ Irrigated ◉ Not Irrigated
12. Access	
A) Describe Sediment Removal Procedures	

Notes:

Site-Level Low Impact Development (LID) Design Effective Impervious Calculator

LID Credit by Impervious Reduction Factor (IRF) Method

UD-BMP (Version 3.06, November 2016)

User Input

Calculated cells

***Design Storm: 1-Hour Rain Depth	WQCV Event	0.42	inches
***Minor Storm: 1-Hour Rain Depth	5-Year Event	1.50	inches
***Major Storm: 1-Hour Rain Depth	100-Year Event	2.52	inches
Optional User Defined Storm	CUHP		
(CUHP) NOAA 1 Hour Rainfall Depth and Frequency for User Defined Storm	100-Year Event	2.52	
Max Intensity for Optional User Defined Storm	2.51496		

Designer: Marc A. Whorton, P.E.
Company: Classic Consulting
Date: November 30, 2017
Project: Flying Horse North (Trib. Basins to Pond 4)
Location: Black Forest, CO

SITE INFORMATION (USER-INPUT)

Sub-basin Identifier	OS-8	OS-9	OS-10	OS-11	BS-13	BS-14	BS-15	BS-16	BS-17					
Receiving Pervious Area Soil Type	Loamy Sand	Loamy Sand	Loamy Sand	Loamy Sand	Loamy Sand	Loamy Sand	Loamy Sand	Loamy Sand	Loamy Sand					
Total Area (ac., Sum of DCIA, UIA, RPA, & SPA)	14.200	9.800	4.100	28.000	25.600	13.400	5.300	21.600	12.100					
Directly Connected Impervious Area (DCIA, acres)	0.950	0.000	0.180	0.500	2.200	1.100	0.760	1.900	1.600					
Unconnected Impervious Area (UIA, acres)	0.520	0.000	0.200	0.500	1.000	0.500	0.200	1.200	0.400					
Receiving Pervious Area (RPA, acres)	3.000	0.000	1.200	6.300	5.000	3.000	1.000	6.000	2.000					
Separate Pervious Area (SPA, acres)	9.730	9.800	2.520	20.700	17.400	8.800	3.340	12.500	8.100					
RPA Treatment Type: Conveyance (C), Volume (V), or Permeable Pavement (PP)	C	C	C	C	C	C	C	C	C					

CALCULATED RESULTS (OUTPUT)

Total Calculated Area (ac, check against input)	14.200	9.800	4.100	28.000	25.600	13.400	5.300	21.600	12.100					
Directly Connected Impervious Area (DCIA, %)	6.7%	0.0%	4.4%	1.8%	8.6%	8.2%	14.3%	8.8%	13.2%					
Unconnected Impervious Area (UIA, %)	3.7%	0.0%	4.9%	1.8%	3.9%	3.7%	3.8%	5.6%	3.3%					
Receiving Pervious Area (RPA, %)	21.1%	0.0%	29.3%	22.5%	19.5%	22.4%	18.9%	27.8%	16.5%					
Separate Pervious Area (SPA, %)	68.5%	100.0%	61.5%	73.9%	68.0%	65.7%	63.0%	57.9%	66.9%					
A_R (RPA / UIA)	5.769	0.000	6.000	12.600	5.000	6.000	5.000	5.000	5.000					
I_a Check	0.150	1.000	0.140	0.070	0.170	0.140	0.170	0.170	0.170					
f / I for WQCV Event:	4.6	4.6	4.6	4.6	4.6	4.6	4.6	4.6	4.6					
f / I for 5-Year Event:	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5	0.5					
f / I for 100-Year Event:	0.4	0.4	0.4	0.4	0.4	0.4	0.4	0.4	0.4					
f / I for Optional User Defined Storm CUHP:	0.39	0.39	0.39	0.39	0.39	0.39	0.39	0.39	0.39					
IRF for WQCV Event:	0.32	1.00	0.30	0.15	0.37	0.30	0.37	0.37	0.37					
IRF for 5-Year Event:	0.63	1.00	0.59	0.29	0.71	0.59	0.71	0.71	0.71					
IRF for 100-Year Event:	0.65	1.00	0.61	0.30	0.73	0.61	0.73	0.73	0.73					
IRF for Optional User Defined Storm CUHP:	0.65	1.00	0.61	0.30	0.73	0.61	0.73	0.73	0.73					
Total Site Imperviousness: I_{total}	10.4%	0.0%	9.3%	3.6%	12.5%	11.9%	18.1%	14.4%	16.5%					
Effective Imperviousness for WQCV Event:	7.9%	0.0%	5.9%	2.1%	10.0%	9.3%	15.7%	10.8%	14.4%					
Effective Imperviousness for 5-Year Event:	9.0%	0.0%	7.3%	2.3%	11.4%	10.4%	17.0%	12.8%	15.6%					
Effective Imperviousness for 100-Year Event:	9.1%	0.0%	7.3%	2.3%	11.5%	10.5%	17.1%	12.9%	15.7%					
Effective Imperviousness for Optional User Defined Storm CUHP:	9.1%	0.0%	7.3%	2.3%	11.5%	10.5%	17.1%	12.9%	15.7%					

LID / EFFECTIVE IMPERVIOUSNESS CREDITS

WQCV Event CREDIT: Reduce Detention By:	21.0%	N/A	33.4%	41.1%	16.8%	18.7%	10.1%	20.5%	9.9%	N/A	N/A	N/A	N/A	N/A
This line only for 10-Year Event	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
100-Year Event CREDIT**: Reduce Detention By:	15.3%	N/A	26.4%	79.5%	9.8%	14.7%	6.1%	11.8%	6.0%	N/A	N/A	N/A	N/A	N/A
User Defined CUHP CREDIT: Reduce Detention By:	7.4%	0.0%	12.1%	13.6%	5.1%	7.6%	3.6%	6.5%	3.4%					

Total Site Imperviousness:	**10.2%**
Total Site Effective Imperviousness for WQCV Event:	**8.0%**
Total Site Effective Imperviousness for 5-Year Event:	**9.0%**
Total Site Effective Imperviousness for 100-Year Event:	**9.1%**
Total Site Effective Imperviousness for Optional User Defined Storm CUHP:	**9.1%**

Notes:

* Use Green-Ampt average infiltration rate values from Table 3-3.

** Flood control detention volume credits based on empirical equations from Storage Chapter of USDCM.

*** Method assumes that 1-hour rainfall depth is equivalent to 1-hour intensity for calculation purposed

Design Procedure Form: Extended Detention Basin (EDB)

UD-BMP (Version 3.06, November 2016)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 27, 2017
Project: Flying Horse North Filing 1 (Pond 4)
Location: Black Forest, CO

1. Basin Storage Volume

A) Effective Imperviousness of Tributary Area, I_a — I_a = 8.0 %

B) Tributary Area's Imperviousness Ratio ($i = I_a / 100$) — i = 0.080

C) Contributing Watershed Area — Area = 134.100 ac

D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm — d_6 = 0.42 in

E) Design Concept (Select EURV when also designing for flood control)

Choose One
- ○ Water Quality Capture Volume (WQCV)
- ◉ Excess Urban Runoff Volume (EURV)

F) Design Volume (WQCV) Based on 40-hour Drain Time
($V_{DESIGN} = (1.0 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i) / 12 * Area$)
V_{DESIGN} = 0.617 ac-ft

G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume
($V_{WQCV\ OTHER} = (d_6 * (V_{DESIGN} / 0.43))$)
$V_{DESIGN\ OTHER}$ = 0.603 ac-ft

H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired)
$V_{DESIGN\ USER}$ = _____ ac-ft

I) Predominant Watershed NRCS Soil Group

Choose One
- ○ A
- ◉ B
- ○ C / D

J) Excess Urban Runoff Volume (EURV) Design Volume
For HSG A: $EURV_A = 1.68 * i^{1.28}$
For HSG B: $EURV_B = 1.36 * i^{1.08}$
For HSG C/D: $EURV_{C/D} = 1.20 * i^{1.08}$
EURV = 0.993 ac-f t

2. Basin Shape: Length to Width Ratio
(A basin length to width ratio of at least 2:1 will improve TSS reduction.)
L : W = 2.0 : 1

3. Basin Side Slopes

A) Basin Maximum Side Slopes
(Horizontal distance per unit vertical, 4:1 or flatter preferred)
Z = 4.00 ft / ft

4. Inlet

A) Describe means of providing energy dissipation at concentrated inflow locations:

Design Procedure Form: Extended Detention Basin (EDB)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 27, 2017
Project: Flying Horse North Filing 1 (Pond 4)
Location: Black Forest, CO

5. Forebay	
A) Minimum Forebay Volume (V_{FMIN} = 3% of the WQCV)	V_{FMIN} = 0.018 ac-ft
B) Actual Forebay Volume	V_F = 0.019 ac-ft
C) Forebay Depth (D_F = 18 inch maximum)	D_F = 12.0 in
D) Forebay Discharge	
i) Undetained 100-year Peak Discharge	Q_{100} = 217.00 cfs
ii) Forebay Discharge Design Flow ($Q_F = 0.02 * Q_{100}$)	Q_F = 4.34 cfs
E) Forebay Discharge Design	Choose One: ○ Berm With Pipe ◉ Wall with Rect. Notch ○ Wall with V-Notch Weir **(flow too small for berm w/ pipe)**
F) Discharge Pipe Size (minimum 8-inches)	Calculated D_P = in
G) Rectangular Notch Width	Calculated W_N = 18.0 in
6. Trickle Channel	
A) Type of Trickle Channel	Choose One: ◉ Concrete ○ Soft Bottom
F) Slope of Trickle Channel	S = 0.0100 ft / ft
7. Micropool and Outlet Structure	
A) Depth of Micropool (2.5-feet minimum)	D_M = 2.5 ft
B) Surface Area of Micropool (10 ft^2 minimum)	A_M = 160 sq ft
C) Outlet Type	Choose One: ◉ Orifice Plate ○ Other (Describe):
D) Smallest Dimension of Orifice Opening Based on Hydrograph Routing (Use UD-Detention)	$D_{orifice}$ = 1.88 inches
E) Total Outlet Area	A_{ot} = 8.58 square inches

Design Procedure Form: Extended Detention Basin (EDB)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 27, 2017
Project: Flying Horse North Filing 1 (Pond 4)
Location: Black Forest, CO

8. Initial Surcharge Volume	
A) Depth of Initial Surcharge Volume (Minimum recommended depth is 4 inches)	D_{IS} = 6 in
B) Minimum Initial Surcharge Volume (Minimum volume of 0.3% of the WQCV)	V_{IS} = 78.8 cu ft
C) Initial Surcharge Provided Above Micropool	V_s= 80.0 cu ft
9. Trash Rack	
A) Water Quality Screen Open Area: $A_t = A_{ot} * 38.5*(e^{-0.095D})$	A_t = 276 square inches
B) Type of Screen (If specifying an alternative to the materials recommended in the USDCM, indicate "other" and enter the ratio of the total open are to the total screen are for the material specified.)	*Aluminum Amico-Klemp SR Series with Cross Rods 2" O.C.*
Other (Y/N): N	
C) Ratio of Total Open Area to Total Area (only for type 'Other')	User Ratio =
D) Total Water Quality Screen Area (based on screen type)	A_{total} = 389 sq. in.
E) Depth of Design Volume (EURV or WQCV) (Based on design concept chosen under 1E)	H= 4 feet
F) Height of Water Quality Screen (H_{TR})	H_{TR}= 76 inches
G) Width of Water Quality Screen Opening ($W_{opening}$) (Minimum of 12 inches is recommended)	$W_{opening}$ = 12.0 inches

Design Procedure Form: Extended Detention Basin (EDB)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 27, 2017
Project: Flying Horse North Filing 1 (Pond 4)
Location: Black Forest, CO

10. Overflow Embankment	
A) Describe embankment protection for 100-year and greater overtopping:	Soil Rip-Rap
B) Slope of Overflow Embankment (Horizontal distance per unit vertical, 4:1 or flatter preferred)	4.00
11. Vegetation	Choose One: ○ Irrigated ◉ Not Irrigated
12. Access	
A) Describe Sediment Removal Procedures	

Notes:

Design Procedure Form: Extended Detention Basin (EDB)

UD-BMP (Version 3.06, November 2016)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 30, 2017
Project: Flying Horse North Filing 1 (Pond 4 - North Forbay Design)
Location: Black Forest, CO

1. Basin Storage Volume	
A) Effective Imperviousness of Tributary Area, I_a	I_a = 8.0 %
B) Tributary Area's Imperviousness Ratio ($i = I_a / 100$)	i = 0.080
C) Contributing Watershed Area	Area = 107.200 ac
D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm	d_6 = 0.42 in
E) Design Concept (Select EURV when also designing for flood control)	Choose One: ○ Water Quality Capture Volume (WQCV) ◉ Excess Urban Runoff Volume (EURV)
F) Design Volume (WQCV) Based on 40-hour Drain Time ($V_{DESIGN} = (1.0 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i) / 12 * Area$)	V_{DESIGN}= 0.494 ac-ft
G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume ($V_{WQCV\ OTHER} = (d_6 * (V_{DESIGN}/0.43))$	$V_{DESIGN\ OTHER}$= 0.482 ac-ft
H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired)	$V_{DESIGN\ USER}$= ac-ft
I) Predominant Watershed NRCS Soil Group	Choose One: ○ A ◉ B ○ C / D
J) Excess Urban Runoff Volume (EURV) Design Volume For HSG A: $EURV_A = 1.68 * i^{1.28}$ For HSG B: $EURV_B = 1.36 * i^{1.08}$ For HSG C/D: $EURV_{C/D} = 1.20 * i^{1.08}$	EURV = 0.794 ac-f t
2. Basin Shape: Length to Width Ratio (A basin length to width ratio of at least 2:1 will improve TSS reduction.)	L : W = 2.0 : 1
3. Basin Side Slopes	
A) Basin Maximum Side Slopes (Horizontal distance per unit vertical, 4:1 or flatter preferred)	Z = 4.00 ft / ft
4. Inlet	
A) Describe means of providing energy dissipation at concentrated inflow locations:	

Design Procedure Form: Extended Detention Basin (EDB)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 30, 2017
Project: Flying Horse North Filing 1 (Pond 4 - North Forbay Design)
Location: Black Forest, CO

5. Forebay

A) Minimum Forebay Volume (V_{FMIN} = 3% of the WQCV) — V_{FMIN} = 0.015 ac-ft

B) Actual Forebay Volume — V_F = 0.015 ac-ft

C) Forebay Depth (D_F = 18 inch maximum) — D_F = 12.0 in

D) Forebay Discharge

i) Undetained 100-year Peak Discharge — Q_{100} = 170.00 cfs

ii) Forebay Discharge Design Flow ($Q_F = 0.02 * Q_{100}$) — Q_F = 3.40 cfs

E) Forebay Discharge Design

Choose One
- ○ Berm With Pipe
- ◉ Wall with Rect. Notch
- ○ Wall with V-Notch Weir

(flow too small for berm w/ pipe)

F) Discharge Pipe Size (minimum 8-inches) — Calculated D_P = in

G) Rectangular Notch Width — Calculated W_N = 14.7 in

6. Trickle Channel

A) Type of Trickle Channel

Choose One
- ◉ Concrete
- ○ Soft Bottom

F) Slope of Trickle Channel — S = 0.0100 ft / ft

7. Micropool and Outlet Structure

A) Depth of Micropool (2.5-feet minimum) — D_M = 2.5 ft

B) Surface Area of Micropool (10 ft^2 minimum) — A_M = 160 sq ft

C) Outlet Type

Choose One
- ◉ Orifice Plate
- ○ Other (Describe):

D) Smallest Dimension of Orifice Opening Based on Hydrograph Routing (Use UD-Detention) — $D_{orifice}$ = 1.88 inches

E) Total Outlet Area — A_{ot} = 8.58 square inches

Design Procedure Form: Extended Detention Basin (EDB)

UD-BMP (Version 3.06, November 2016)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 30, 2017
Project: Flying Horse North Filing 1 (Pond 4 - South Forbay Design)
Location: Black Forest, CO

1. Basin Storage Volume

A) Effective Imperviousness of Tributary Area, I_a — I_a = 8.0 %

B) Tributary Area's Imperviousness Ratio ($i = I_a / 100$) — i = 0.080

C) Contributing Watershed Area — Area = 26.900 ac

D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm — d_6 = 0.42 in

E) Design Concept (Select EURV when also designing for flood control)

Choose One
- ○ Water Quality Capture Volume (WQCV)
- ◉ Excess Urban Runoff Volume (EURV)

F) Design Volume (WQCV) Based on 40-hour Drain Time
($V_{DESIGN} = (1.0 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i) / 12 * Area$)
V_{DESIGN} = 0.124 ac-ft

G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume
($V_{WQCV\ OTHER} = (d_6 * (V_{DESIGN} / 0.43))$)
$V_{DESIGN\ OTHER}$ = 0.121 ac-ft

H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired)
$V_{DESIGN\ USER}$ = _____ ac-ft

I) Predominant Watershed NRCS Soil Group

Choose One
- ○ A
- ◉ B
- ○ C / D

J) Excess Urban Runoff Volume (EURV) Design Volume
For HSG A: $EURV_A = 1.68 * i^{1.28}$
For HSG B: $EURV_B = 1.36 * i^{1.08}$
For HSG C/D: $EURV_{C/D} = 1.20 * i^{1.08}$
EURV = 0.199 ac-f t

2. Basin Shape: Length to Width Ratio
(A basin length to width ratio of at least 2:1 will improve TSS reduction.)
L : W = 2.0 : 1

3. Basin Side Slopes

A) Basin Maximum Side Slopes
(Horizontal distance per unit vertical, 4:1 or flatter preferred)
Z = 4.00 ft / ft

4. Inlet

A) Describe means of providing energy dissipation at concentrated inflow locations:

Design Procedure Form: Extended Detention Basin (EDB)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 30, 2017
Project: Flying Horse North Filing 1 (Pond 4 - South Forbay Design)
Location: Black Forest, CO

5. Forebay

A) Minimum Forebay Volume (V_{FMIN} = 2% of the WQCV) — V_{FMIN} = 0.002 ac-ft

B) Actual Forebay Volume — V_F = 0.002 ac-ft

C) Forebay Depth (D_F = 18 inch maximum) — D_F = 8.0 in

D) Forebay Discharge

i) Undetained 100-year Peak Discharge — Q_{100} = 56.00 cfs

ii) Forebay Discharge Design Flow ($Q_F = 0.02 * Q_{100}$) — Q_F = 1.12 cfs

E) Forebay Discharge Design

Choose One
- ○ Berm With Pipe
- ◉ Wall with Rect. Notch
- ○ Wall with V-Notch Weir

(flow too small for berm w/ pipe)

F) Discharge Pipe Size (minimum 8-inches) — Calculated D_P = in

G) Rectangular Notch Width — Calculated W_N = 9.0 in

6. Trickle Channel

A) Type of Trickle Channel

Choose One
- ◉ Concrete
- ○ Soft Bottom

F) Slope of Trickle Channel — S = 0.0100 ft / ft

7. Micropool and Outlet Structure

A) Depth of Micropool (2.5-feet minimum) — D_M = 2.5 ft

B) Surface Area of Micropool (10 ft^2 minimum) — A_M = 160 sq ft

C) Outlet Type

Choose One
- ◉ Orifice Plate
- ○ Other (Describe):

D) Smallest Dimension of Orifice Opening Based on Hydrograph Routing (Use UD-Detention) — $D_{orifice}$ = 1.88 inches

E) Total Outlet Area — A_{ot} = 8.58 square inches

Site-Level Low Impact Development (LID) Design Effective Impervious Calculator

LID Credit by Impervious Reduction Factor (IRF) Method

UD-BMP (Version 3.06, November 2016)

User Input

Calculated cells

***Design Storm: 1-Hour Rain Depth	WQCV Event	0.42	inches
***Minor Storm: 1-Hour Rain Depth	5-Year Event	1.50	inches
***Major Storm: 1-Hour Rain Depth	100-Year Event	2.52	inches
Optional User Defined Storm	CUHP		
(CUHP) NOAA 1 Hour Rainfall Depth and Frequency for User Defined Storm	100-Year Event	2.52	
Max Intensity for Optional User Defined Storm	2.51496		

Designer: Marc A. Whorton, P.E.
Company: Classic Consulting
Date: November 30, 2017
Project: Flying Horse North (Trib. Basins to Pond 8)
Location: Black Forest, CO

SITE INFORMATION (USER-INPUT)

Sub-basin Identifier	BS-18	BS-19	BS-20	BS-21	BS-22	BS-23	BS-23A								
Receiving Pervious Area Soil Type	Loamy Sand	Loamy Sand	Loamy Sand	Loamy Sand	Loamy Sand	Loamy Sand	Loamy Sand								
Total Area (ac., Sum of DCIA, UIA, RPA, & SPA)	33.800	6.300	73.900	69.500	18.100	37.100	16.300								
Directly Connected Impervious Area (DCIA, acres)	1.500	1.000	4.500	4.200	1.500	3.200	2.900								
Unconnected Impervious Area (UIA, acres)	1.200	0.300	3.400	3.500	1.700	2.300	1.800								
Receiving Pervious Area (RPA, acres)	5.200	0.700	11.500	10.100	4.000	5.500	2.500								
Separate Pervious Area (SPA, acres)	25.900	4.300	54.500	51.700	10.900	26.100	9.100								
RPA Treatment Type: Conveyance (C), Volume (V), or Permeable Pavement (PP)	C	C	C	C	C	C	C								

CALCULATED RESULTS (OUTPUT)

Total Calculated Area (ac, check against input)	33.800	6.300	73.900	69.500	18.100	37.100	16.300								
Directly Connected Impervious Area (DCIA, %)	4.4%	15.9%	6.1%	6.0%	8.3%	8.6%	17.8%								
Unconnected Impervious Area (UIA, %)	3.6%	4.8%	4.6%	5.0%	9.4%	6.2%	11.0%								
Receiving Pervious Area (RPA, %)	15.4%	11.1%	15.6%	14.5%	22.1%	14.8%	15.3%								
Separate Pervious Area (SPA, %)	76.6%	68.3%	73.7%	74.4%	60.2%	70.4%	55.8%								
A_R (RPA / UIA)	4.333	2.333	3.382	2.886	2.353	2.391	1.389								
I_a Check	0.190	0.300	0.230	0.260	0.300	0.290	0.420								
f / I for WQCV Event:	4.6	4.6	4.6	4.6	4.6	4.6	4.6								
f / I for 5-Year Event:	0.5	0.5	0.5	0.5	0.5	0.5	0.5								
f / I for 100-Year Event:	0.4	0.4	0.4	0.4	0.4	0.4	0.4								
f / I for Optional User Defined Storm CUHP:	0.39	0.39	0.39	0.39	0.39	0.39	0.39								
IRF for WQCV Event:	0.41	0.49	0.45	0.47	0.49	0.49	0.56								
IRF for 5-Year Event:	0.80	0.86	0.84	0.85	0.86	0.85	0.88								
IRF for 100-Year Event:	0.82	0.88	0.87	0.88	0.88	0.88	0.90								
IRF for Optional User Defined Storm CUHP:	0.82	0.88	0.87	0.88	0.88	0.88	0.90								
Total Site Imperviousness: I_{total}	8.0%	20.6%	10.7%	11.1%	17.7%	14.8%	28.8%								
Effective Imperviousness for WQCV Event:	5.9%	18.2%	8.2%	8.4%	12.9%	11.6%	24.0%								
Effective Imperviousness for 5-Year Event:	7.3%	19.9%	10.0%	10.3%	16.3%	13.9%	27.5%								
Effective Imperviousness for 100-Year Event:	7.4%	20.1%	10.1%	10.4%	16.6%	14.1%	27.7%								
Effective Imperviousness for Optional User Defined Storm CUHP:	7.4%	20.1%	10.1%	10.4%	16.6%	14.1%	27.7%								

LID / EFFECTIVE IMPERVIOUSNESS CREDITS

WQCV Event CREDIT: Reduce Detention By:	23.8%	8.6%	20.6%	21.0%	21.7%	17.7%	11.3%	N/A	N/A	N/A	N/A	N/A	N/A	N/A	
This line only for 10-Year Event	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	
100-Year Event CREDIT**: Reduce Detention By:	10.5%	3.0%	6.8%	6.9%	7.0%	5.7%	4.0%	N/A	N/A	N/A	N/A	N/A	N/A	N/A	
User Defined CUHP CREDIT: Reduce Detention By:	4.4%	1.8%	3.3%	3.4%	4.1%	3.2%	2.7%								

Total Site Imperviousness:	**12.9%**
Total Site Effective Imperviousness for WQCV Event:	**10.0%**
Total Site Effective Imperviousness for 5-Year Event:	**12.1%**
Total Site Effective Imperviousness for 100-Year Event:	**12.2%**
Total Site Effective Imperviousness for Optional User Defined Storm CUHP:	**12.2%**

Notes:

* Use Green-Ampt average infiltration rate values from Table 3-3.
** Flood control detention volume credits based on empirical equations from Storage Chapter of USDCM.
*** Method assumes that 1-hour rainfall depth is equivalent to 1-hour intensity for calculation purposed

Design Procedure Form: Extended Detention Basin (EDB)

UD-BMP (Version 3.06, November 2016)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 30, 2017
Project: Flying Horse North Filing 1 (Pond 8) (Ultimate Build-out)
Location: Black Forest, CO

1. Basin Storage Volume

A) Effective Imperviousness of Tributary Area, I_a — I_a = 10.0 %

B) Tributary Area's Imperviousness Ratio ($i = I_a / 100$) — i = 0.100

C) Contributing Watershed Area — Area = 255.000 ac

D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm — d_6 = 0.42 in

E) Design Concept (Select EURV when also designing for flood control)

Choose One
- ○ Water Quality Capture Volume (WQCV)
- ◉ Excess Urban Runoff Volume (EURV)

F) Design Volume (WQCV) Based on 40-hour Drain Time
($V_{DESIGN} = (1.0 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i) / 12 * Area$)
V_{DESIGN} = 1.424 ac-ft

G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume
($V_{WQCV\ OTHER} = (d_6 * (V_{DESIGN} / 0.43))$)
$V_{DESIGN\ OTHER}$ = 1.391 ac-ft

H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired)
$V_{DESIGN\ USER}$ = _____ ac-ft

I) Predominant Watershed NRCS Soil Group

Choose One
- ○ A
- ◉ B
- ○ C / D

J) Excess Urban Runoff Volume (EURV) Design Volume
For HSG A: $EURV_A = 1.68 * i^{1.28}$
For HSG B: $EURV_B = 1.36 * i^{1.08}$
For HSG C/D: $EURV_{C/D} = 1.20 * i^{1.08}$
EURV = 2.404 ac-f t

2. Basin Shape: Length to Width Ratio
(A basin length to width ratio of at least 2:1 will improve TSS reduction.)
L : W = 2.0 : 1

3. Basin Side Slopes

A) Basin Maximum Side Slopes
(Horizontal distance per unit vertical, 4:1 or flatter preferred)
Z = 4.00 ft / ft

4. Inlet

A) Describe means of providing energy dissipation at concentrated inflow locations:

Design Procedure Form: Extended Detention Basin (EDB)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 30, 2017
Project: Flying Horse North Filing 1 (Pond 8) (Ultimate Build-out)
Location: Black Forest, CO

5. Forebay

A) Minimum Forebay Volume (V_{FMIN} = 3% of the WQCV) — V_{FMIN} = 0.042 ac-ft

B) Actual Forebay Volume — V_F = 0.042 ac-ft

C) Forebay Depth (D_F = 30 inch maximum) — D_F = 18.0 in

D) Forebay Discharge

i) Undetained 100-year Peak Discharge — Q_{100} = 390.00 cfs

ii) Forebay Discharge Design Flow ($Q_F = 0.02 * Q_{100}$) — Q_F = 7.80 cfs

E) Forebay Discharge Design

Choose One
- ○ Berm With Pipe
- ◉ Wall with Rect. Notch
- ○ Wall with V-Notch Weir

F) Discharge Pipe Size (minimum 8-inches) — Calculated D_P = in

G) Rectangular Notch Width — Calculated W_N = 18.9 in

6. Trickle Channel

A) Type of Trickle Channel

Choose One
- ◉ Concrete
- ○ Soft Bottom

F) Slope of Trickle Channel — S = 0.0100 ft / ft

7. Micropool and Outlet Structure

A) Depth of Micropool (2.5-feet minimum) — D_M = 2.5 ft

B) Surface Area of Micropool (10 ft^2 minimum) — A_M = 384 sq ft

C) Outlet Type

Choose One
- ◉ Orifice Plate
- ○ Other (Describe):

D) Smallest Dimension of Orifice Opening Based on Hydrograph Routing (Use UD-Detention) — $D_{orifice}$ = 2.61 inches

E) Total Outlet Area — A_{ot} = 16.08 square inches

Design Procedure Form: Extended Detention Basin (EDB)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 30, 2017
Project: Flying Horse North Filing 1 (Pond 8) (Ultimate Build-out)
Location: Black Forest, CO

8. Initial Surcharge Volume	
A) Depth of Initial Surcharge Volume (Minimum recommended depth is 4 inches)	D_{IS} = 6 in
B) Minimum Initial Surcharge Volume (Minimum volume of 0.3% of the WQCV)	V_{IS} = 181.8 cu ft
C) Initial Surcharge Provided Above Micropool	V_s= 192.0 cu ft
9. Trash Rack	
A) Water Quality Screen Open Area: $A_t = A_{ot} * 38.5*(e^{-0.095D})$	A_t = 483 square inches
B) Type of Screen (If specifying an alternative to the materials recommended in the USDCM, indicate "other" and enter the ratio of the total open are to the total screen are for the material specified.)	*Aluminum Amico-Klemp SR Series with Cross Rods 2" O.C.*
Other (Y/N): N	
C) Ratio of Total Open Area to Total Area (only for type 'Other')	User Ratio =
D) Total Water Quality Screen Area (based on screen type)	A_{total} = 680 sq. in.
E) Depth of Design Volume (EURV or WQCV) (Based on design concept chosen under 1E)	H= 5.25 feet
F) Height of Water Quality Screen (H_{TR})	H_{TR}= 91 inches
G) Width of Water Quality Screen Opening ($W_{opening}$) (Minimum of 12 inches is recommended)	$W_{opening}$ = 12.0 inches

Design Procedure Form: Extended Detention Basin (EDB)

Designer: Marc A. Whorton
Company: Classic Consutling
Date: November 30, 2017
Project: Flying Horse North Filing 1 (Pond 8) (Ultimate Build-out)
Location: Black Forest, CO

10. Overflow Embankment	
A) Describe embankment protection for 100-year and greater overtopping:	Soil Rip-Rap
B) Slope of Overflow Embankment (Horizontal distance per unit vertical, 4:1 or flatter preferred)	4.00
11. Vegetation	Choose One: ○ Irrigated ◉ Not Irrigated
12. Access	
A) Describe Sediment Removal Procedures	

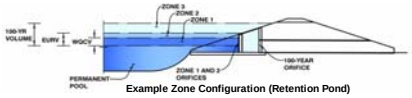
Notes:

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: Flying Horse North Filing No. 1

Basin ID: Pond 1



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type =	EDB	
Watershed Area =	21.80	acres
Watershed Length =	1,500	ft
Watershed Slope =	0.010	ft/ft
Watershed Imperviousness =	20.00%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Desired WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	
Water Quality Capture Volume (WQCV) =	0.210	acre-feet
Excess Urban Runoff Volume (EURV) =	0.433	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.319	acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	0.469	acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	0.809	acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	1.596	acre-feet
50-yr Runoff Volume (P1 = 2.25 in.) =	2.099	acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	2.758	acre-feet
500-yr Runoff Volume (P1 = 3.39 in.) =	4.375	acre-feet
Approximate 2-yr Detention Volume =	0.297	acre-feet
Approximate 5-yr Detention Volume =	0.440	acre-feet
Approximate 10-yr Detention Volume =	0.711	acre-feet
Approximate 25-yr Detention Volume =	0.880	acre-feet
Approximate 50-yr Detention Volume =	0.930	acre-feet
Approximate 100-yr Detention Volume =	1.143	acre-feet

Optional User Override 1-hr Precipitation	
1.19	inches
1.50	inches
1.75	inches
2.00	inches
2.25	inches
2.52	inches
3.39	inches

Stage-Storage Calculation

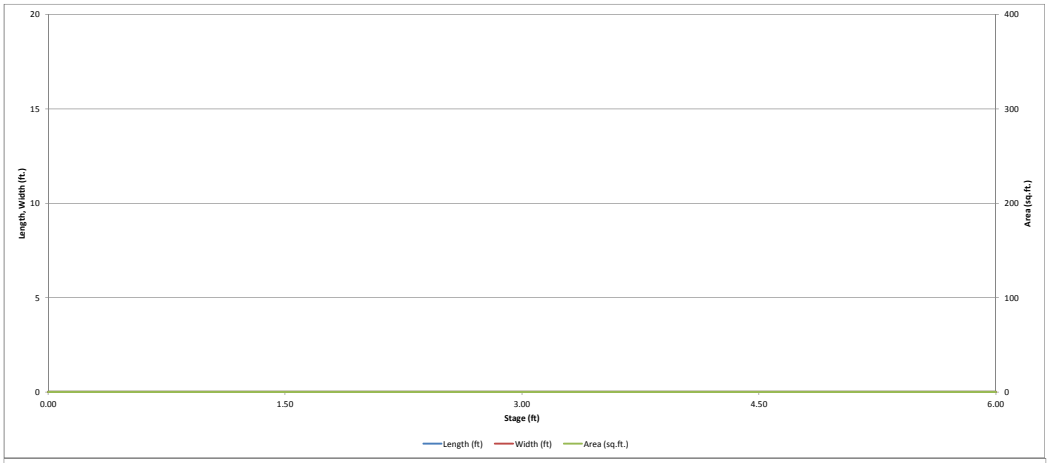
Zone 1 Volume (WQCV) =	0.210	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.223	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	0.710	acre-feet
Total Detention Basin Volume =	1.143	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	**user**	acre-feet

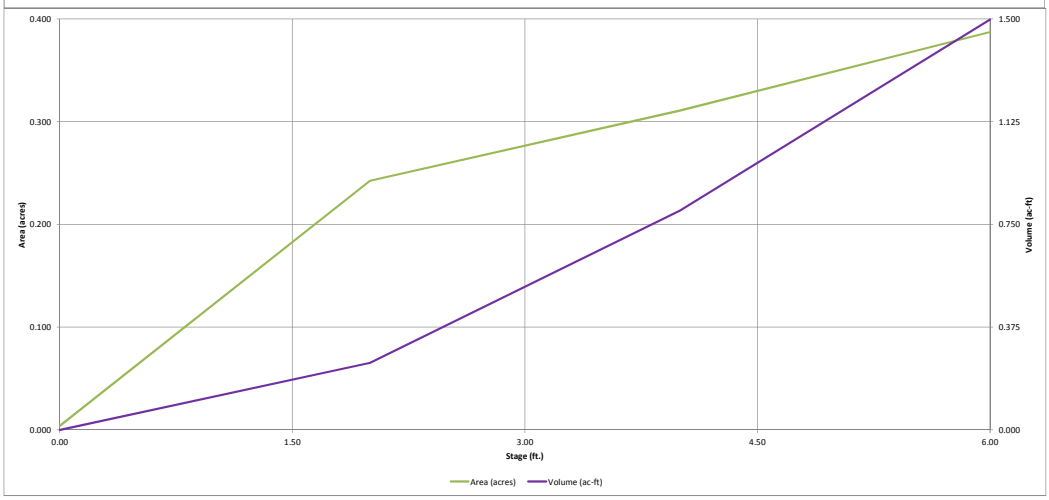
Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	168	0.004		
7392	--	2.00	--	--	--	10,560	0.242	10,623	0.244
7394	--	4.00	--	--	--	13,538	0.311	34,826	0.799
7396	--	6.00	--	--	--	16,868	0.387	65,232	1.498

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)



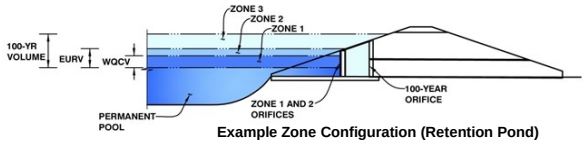


Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: Flying Horse North Filing No. 1

Basin ID: Pond 1



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.85	0.210	Orifice Plate
Zone 2 (EURV)	2.74	0.223	Orifice Plate
Zone 3 (100-year)	5.04	0.710	Weir&Pipe (Restrict)
		1.143	Total

0

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

Calculated Parameters for Underdrain

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	N/A	ft^2
Depth at top of Zone using Orifice Plate =	2.75	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	11.00	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	N/A	inches	Elliptical Slot Area =	N/A	ft^2

Calculated Parameters for Plate

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.90	1.80	2.70				
Orifice Area (sq. inches)	1.30	1.43	1.43	1.43				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected			Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	N/A	N/A	ft^2
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	N/A	N/A	feet
Vertical Orifice Diameter =	N/A	N/A	inches				

Calculated Parameters for Vertical Orifice

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 3 Weir	Not Selected			Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	2.75	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	4.08	N/A	feet
Overflow Weir Front Edge Length =	4.00	N/A	feet	Over Flow Weir Slope Length =	4.22	N/A	feet
Overflow Weir Slope =	3.00	N/A	H:V (enter zero for flat grate)	Grate Open Area / 100-yr Orifice Area =	6.12	N/A	should be ≥ 4
Horiz. Length of Weir Sides =	4.00	N/A	feet	Overflow Grate Open Area w/o Debris =	12.65	N/A	ft^2
Overflow Grate Open Area % =	75%	N/A	%, grate open area/total area	Overflow Grate Open Area w/ Debris =	6.32	N/A	ft^2
Debris Clogging % =	50%	N/A	%				

Calculated Parameters for Overflow Weir

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected			Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.33	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	2.07	N/A	ft^2
Outlet Pipe Diameter =	24.00	N/A	inches	Outlet Orifice Centroid =	0.71	N/A	feet
Restrictor Plate Height Above Pipe Invert =	15.00		inches	Half-Central Angle of Restrictor Plate on Pipe =	1.82	N/A	radians

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	5.00	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	0.69	feet
Spillway Crest Length =	25.00	feet	Stage at Top of Freeboard =	6.69	feet
Spillway End Slopes =	3.00	H:V	Basin Area at Top of Freeboard =	0.39	acres
Freeboard above Max Water Surface =	1.00	feet			

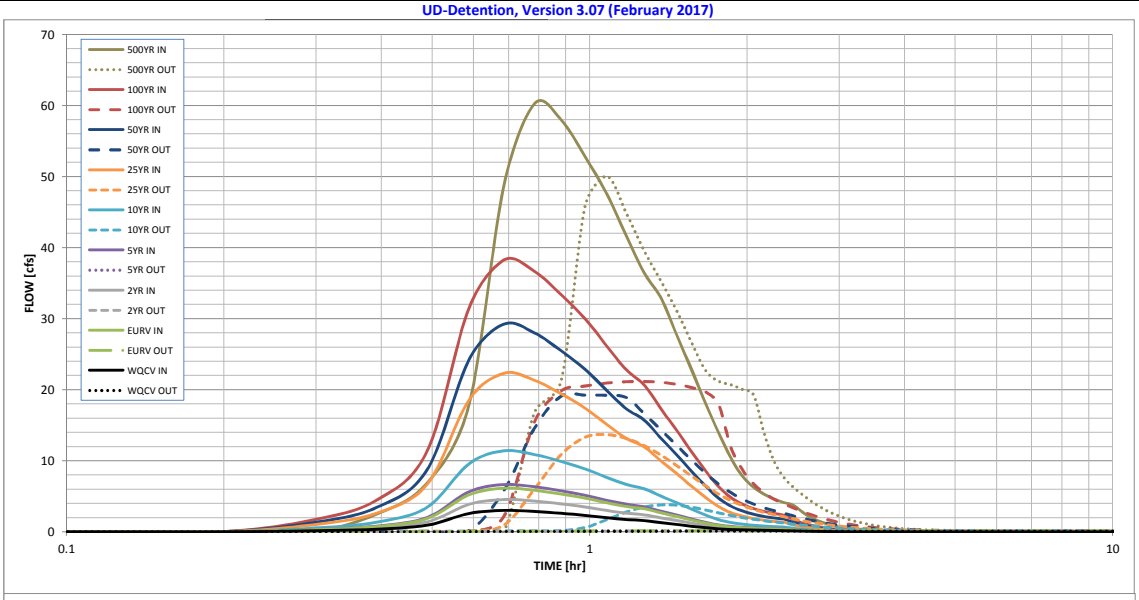
Calculated Parameters for Spillway

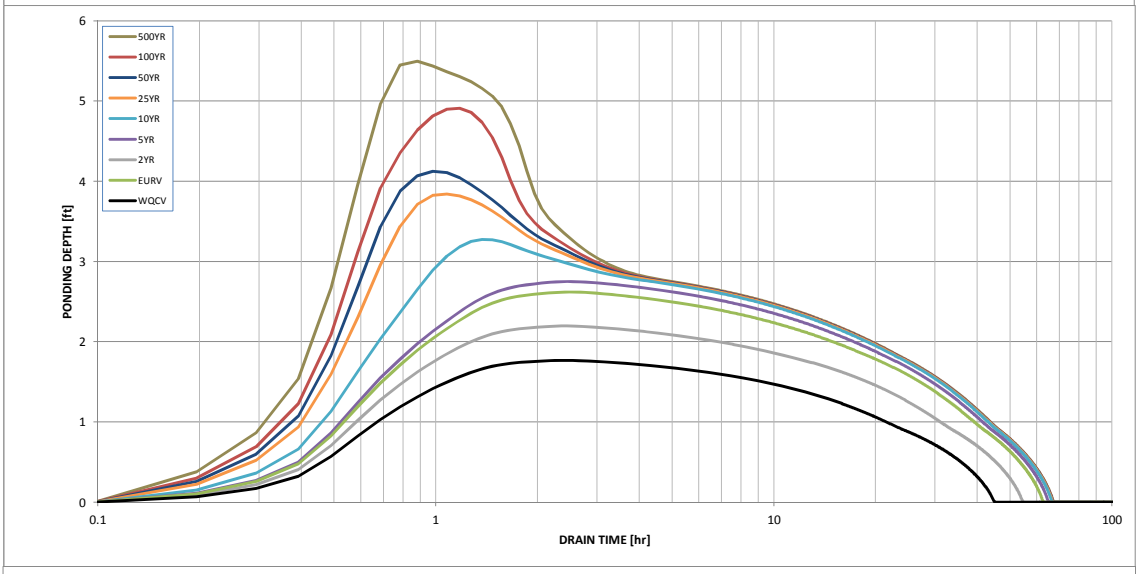
Routed Hydrograph Results

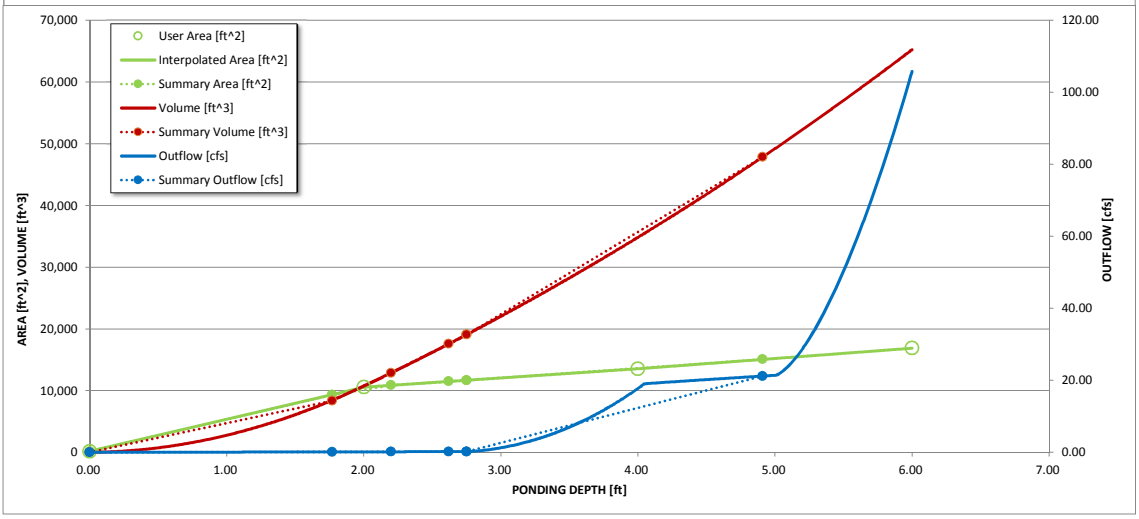
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.39
Calculated Runoff Volume (acre-ft) =	0.210	0.433	0.319	0.469	0.809	1.596	2.099	2.758	4.375
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.210	0.432	0.319	0.469	0.809	1.596	2.099	2.757	4.375
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.02	0.16	0.55	0.77	1.04	1.66
Predevelopment Peak Q (cfs) =	0.0	0.0	0.2	0.4	3.5	12.0	16.7	22.6	36.2
Peak Inflow Q (cfs) =	3.0	6.1	4.5	6.6	11.4	22.3	29.2	38.2	60.2
Peak Outflow Q (cfs) =	0.1	0.2	0.1	0.2	3.8	13.7	19.3	21.2	50.1
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.5	1.1	1.1	1.2	0.9	1.4
Structure Controlling Flow =	Plate	Plate	Plate	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Outlet Plate 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	0.0	0.3	1.1	1.5	1.6	1.7
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	41	55	49	57	55	47	44	40	33
Time to Drain 99% of Inflow Volume (hours) =	**43**	59	52	61	61	58	56	54	49
Maximum Ponding Depth (ft) =	1.77	2.62	2.20	2.75	3.28	3.84	4.13	4.91	5.50
Area at Maximum Ponding Depth (acres) =	0.21	0.26	0.25	0.27	0.29	0.30	0.32	0.35	0.37
Maximum Volume Stored (acre-ft) =	0.192	0.401	0.293	0.438	0.582	0.747	0.837	1.095	1.305

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename:

Storm Inflow Hydrographs **UD-Detention, Version 3.07 (February 2017)**

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.88 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:53	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Hydrograph	0:11:46	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Constant	0:17:38	0.14	0.27	0.20	0.30	0.50	0.96	1.25	1.63	2.49
0.850	0:23:31	0.36	0.74	0.55	0.80	1.36	2.63	3.43	4.47	6.93
	0:29:24	0.93	1.89	1.40	2.05	3.48	6.75	8.81	11.46	17.79
	0:35:17	2.57	5.19	3.86	5.62	9.57	18.53	24.17	31.46	48.76
	0:41:10	3.01	6.13	4.54	6.65	11.39	22.29	29.23	38.24	60.21
	0:47:02	2.86	5.85	4.32	6.34	10.88	21.33	27.99	36.67	57.98
	0:52:55	2.60	5.32	3.93	5.77	9.90	19.42	25.48	33.37	52.82
	0:58:48	2.31	4.74	3.50	5.14	8.85	17.42	22.88	30.01	47.59
	1:04:41	1.98	4.08	3.01	4.43	7.65	15.12	19.90	26.16	41.65
	1:10:34	1.73	3.56	2.62	3.86	6.67	13.14	17.30	22.78	36.37
	1:16:26	1.56	3.22	2.37	3.50	6.04	11.92	15.68	20.62	32.82
	1:22:19	1.28	2.65	1.95	2.88	5.00	9.92	13.08	17.23	27.52
	1:28:12	1.03	2.16	1.58	2.34	4.09	8.16	10.79	14.24	22.81
	1:34:05	0.78	1.65	1.20	1.80	3.16	6.38	8.47	11.22	18.09
	1:39:58	0.57	1.22	0.88	1.33	2.36	4.84	6.46	8.60	13.98
	1:45:50	0.42	0.89	0.65	0.97	1.71	3.54	4.76	6.37	10.45
	1:51:43	0.33	0.69	0.50	0.75	1.32	2.70	3.61	4.81	7.83
	1:57:36	0.27	0.57	0.41	0.62	1.08	2.20	2.93	3.89	6.29
	2:03:29	0.23	0.48	0.35	0.53	0.92	1.86	2.47	3.28	5.29
	2:09:22	0.20	0.42	0.31	0.46	0.81	1.63	2.16	2.86	4.61
	2:15:14	0.18	0.38	0.28	0.42	0.73	1.46	1.94	2.56	4.12
	2:21:07	0.17	0.35	0.26	0.39	0.67	1.34	1.78	2.35	3.77
	2:27:00	0.12	0.26	0.19	0.28	0.49	0.99	1.31	1.74	2.81
	2:32:53	0.09	0.19	0.14	0.21	0.36	0.72	0.96	1.26	2.04
	2:38:46	0.07	0.14	0.10	0.15	0.26	0.53	0.70	0.93	1.51
	2:44:38	0.05	0.10	0.07	0.11	0.19	0.39	0.52	0.69	1.12
	2:50:31	0.03	0.07	0.05	0.08	0.14	0.28	0.38	0.50	0.82
	2:56:24	0.02	0.05	0.04	0.06	0.10	0.20	0.27	0.36	0.59
	3:02:17	0.02	0.04	0.03	0.04	0.07	0.15	0.20	0.26	0.43
	3:08:10	0.01	0.02	0.02	0.03	0.05	0.10	0.13	0.18	0.30
	3:14:02	0.01	0.01	0.01	0.02	0.03	0.06	0.08	0.12	0.19
	3:19:55	0.00	0.01	0.00	0.01	0.01	0.03	0.05	0.06	0.11
	3:25:48	0.00	0.00	0.00	0.00	0.00	0.01	0.02	0.03	0.05
	3:31:41	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.01
	3:37:34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:43:26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:49:19	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:01:05	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:06:58	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:12:50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:18:43	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:24:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:36:22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:42:14	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:48:07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:54:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:59:53	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:46	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:11:38	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:17:31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:23:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:29:17	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:41:02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:46:55	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:52:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:58:41	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:04:34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:10:26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:16:19	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:22:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:28:05	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:33:58	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:39:50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:45:43	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:51:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:57:29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:03:22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]
TOP OF MICROPOOL	0.00	168	0.004	0	0.000	0.00
WQCV	1.77	9,313	0.214	8,343	0.192	0.10
2-YR. WSE	2.20	10,858	0.249	12,870	0.295	0.15
EURV	2.62	11,483	0.264	17,561	0.403	0.18
5-YR. WSE	2.75	11,677	0.268	19,067	0.438	0.19
TOP OF BOX	2.75	11,677	0.268	19,067	0.438	0.19
100 YR. WSE	4.91	15,053	0.346	47,835	1.098	21.17

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

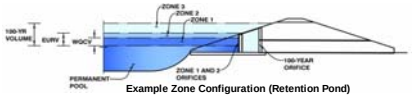
Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: FLYING HORSE NORTH FILING 1

Basin ID: POND 4



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type =	EDB	
Watershed Area =	134.10	acres
Watershed Length =	3,900	ft
Watershed Slope =	0.021	ft/ft
Watershed Imperviousness =	8.00%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Desired WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	
Water Quality Capture Volume (WQCV) =	0.617	acre-feet
Excess Urban Runoff Volume (EURV) =	0.990	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.666	acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	1.055	acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	2.694	acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	7.789	acre-feet
50-yr Runoff Volume (P1 = 2.25 in.) =	10.959	acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	15.099	acre-feet
500-yr Runoff Volume (P1 = 3.85 in.) =	28.334	acre-feet
Approximate 2-yr Detention Volume =	0.618	acre-feet
Approximate 5-yr Detention Volume =	0.988	acre-feet
Approximate 10-yr Detention Volume =	2.274	acre-feet
Approximate 25-yr Detention Volume =	3.285	acre-feet
Approximate 50-yr Detention Volume =	3.412	acre-feet
Approximate 100-yr Detention Volume =	4.464	acre-feet

Optional User Override 1-hr Precipitation

1.19	inches
1.50	inches
1.75	inches
2.00	inches
2.25	inches
2.52	inches
3.85	inches

Stage-Storage Calculation

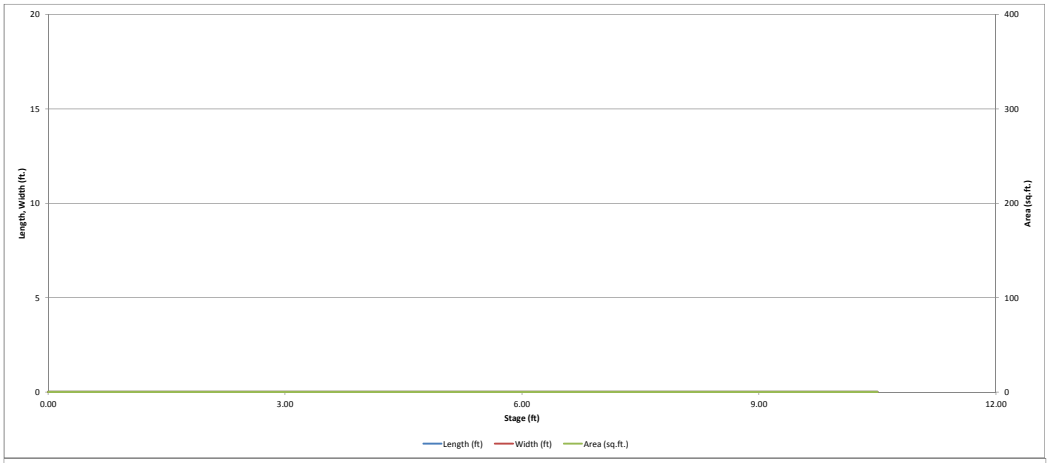
Zone 1 Volume (WQCV) =	0.617	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.373	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	3.474	acre-feet
Total Detention Basin Volume =	4.464	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	user	acre-feet

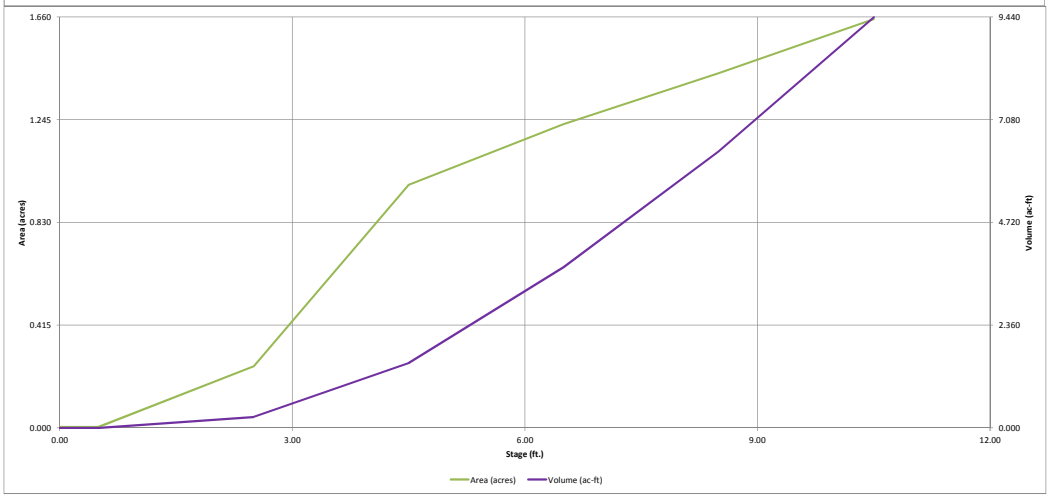
Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	160	0.004		
7422	--	0.50	--	--	--	160	0.004	78	0.002
7424	--	2.50	--	--	--	10,810	0.248	11,050	0.254
7426	--	4.50	--	--	--	42,734	0.981	64,594	1.483
7428	--	6.50	--	--	--	53,403	1.226	160,731	3.690
7430	--	8.50	--	--	--	62,379	1.432	276,513	6.348
7432	--	10.50	--	--	--	71,913	1.651	410,805	9.431

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)



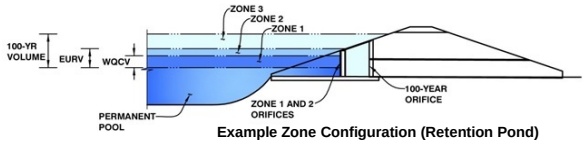


Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: FLYING HORSE NORTH FILING 1

Basin ID: POND 4



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.39	0.617	Orifice Plate
Zone 2 (EURV)	3.94	0.373	Orifice Plate
Zone 3 (100-year)	7.12	3.474	Weir&Pipe (Restrict)
		4.464	Total

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	4.00	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	16.00	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft^2
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft^2

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.30	2.60	3.90				
Orifice Area (sq. inches)	2.00	2.19	2.19	2.19				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft^2
Vertical Orifice Centroid =	N/A	N/A	feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	4.00	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	10.00	N/A	feet
Overflow Weir Slope =	4.00	N/A	H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	4.00		feet
Overflow Grate Open Area % =	75%	N/A	%, grate open area/total area
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	5.00	N/A	feet
Over Flow Weir Slope Length =	4.12	N/A	feet
Grate Open Area / 100-yr Orifice Area =	2.46	N/A	should be ≥ 4
Overflow Grate Open Area w/o Debris =	30.92	N/A	ft^2
Overflow Grate Open Area w/ Debris =	15.46	N/A	ft^2

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	2.50	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	48.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	48.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	12.57	N/A	ft^2
Outlet Orifice Centroid =	2.00	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	3.14	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	8.00	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	40.00	feet
Spillway End Slopes =	6.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

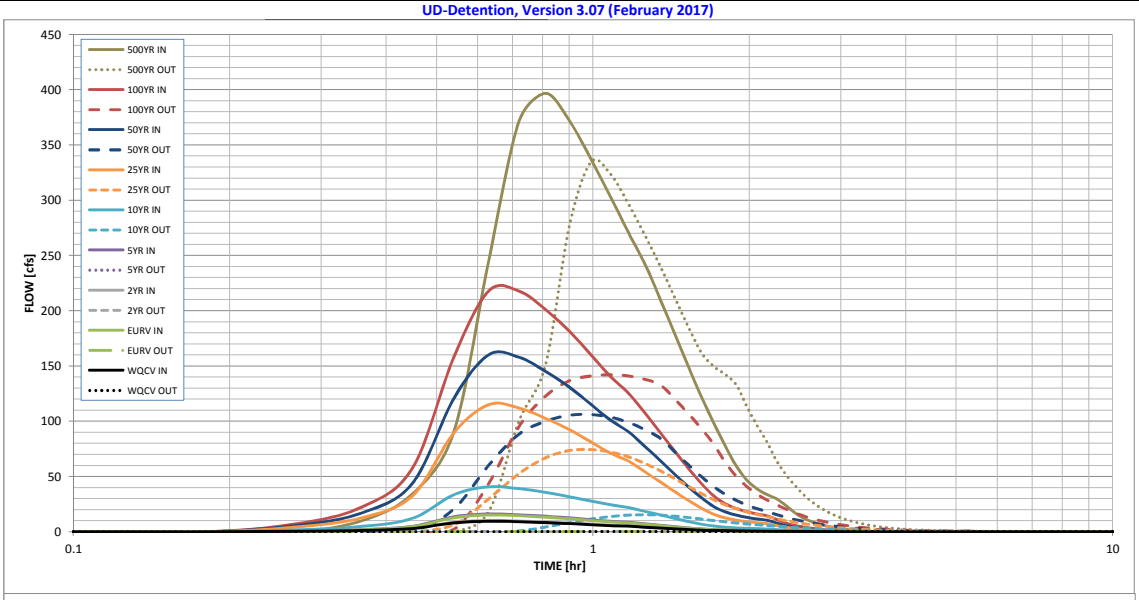
Spillway Design Flow Depth=	1.34	feet
Stage at Top of Freeboard =	10.34	feet
Basin Area at Top of Freeboard =	1.63	acres

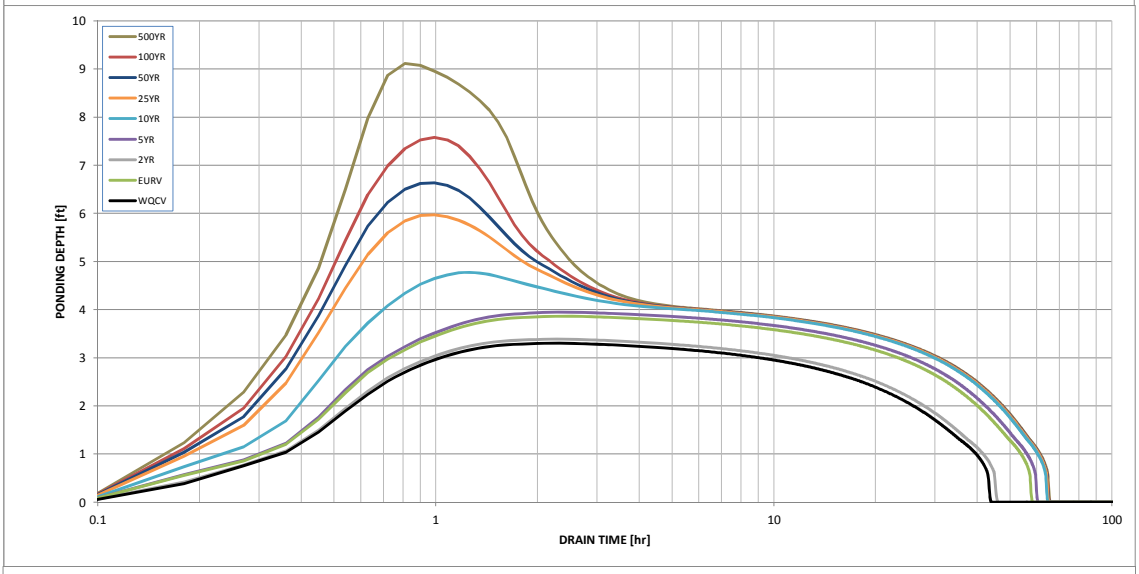
Routed Hydrograph Results

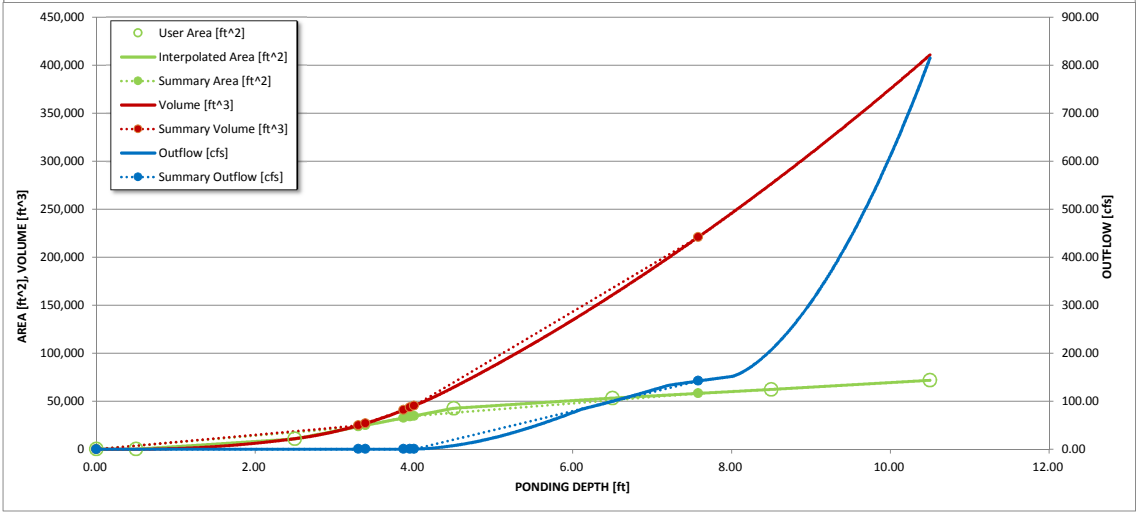
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.85
Calculated Runoff Volume (acre-ft) =	0.617	0.990	0.666	1.055	2.694	7.789	10.959	15.099	28.334
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.617	0.989	0.665	1.054	2.692	7.773	10.944	15.081	28.305
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.02	0.18	0.60	0.84	1.13	2.05
Predevelopment Peak Q (cfs) =	0.0	0.0	1.5	2.5	24.1	81.1	112.3	151.7	275.3
Peak Inflow Q (cfs) =	9.5	15.1	10.2	16.1	40.7	114.9	160.1	217.9	396.7
Peak Outflow Q (cfs) =	0.3	0.3	0.3	0.4	15.5	74.5	106.2	142.2	334.4
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.1	0.6	0.9	0.9	0.9	1.2
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	0.5	2.4	3.4	4.6	5.6
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	40	52	42	54	52	41	35	29	13
Time to Drain 99% of Inflow Volume (hours) =	**42**	56	44	58	59	53	50	47	38
Maximum Ponding Depth (ft) =	3.30	3.86	3.39	3.95	4.77	5.97	6.64	7.57	9.11
Area at Maximum Ponding Depth (acres) =	0.54	0.75	0.57	0.78	1.01	1.16	1.24	1.34	1.50
Maximum Volume Stored (acre-ft) =	0.569	0.930	0.614	0.991	1.752	3.046	3.850	5.061	7.242

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename:

Storm Inflow Hydrographs **UD-Detention, Version 3.07 (February 2017)**

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.40 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Hydrograph	0:10:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Constant	0:16:12	0.42	0.66	0.45	0.71	1.73	4.42	5.77	7.29	10.17
0.925	0:21:36	1.13	1.80	1.22	1.91	4.75	12.73	17.17	22.53	34.50
	0:27:00	2.91	4.61	3.13	4.91	12.20	32.70	44.13	57.96	90.44
	0:32:24	8.00	12.68	8.61	13.48	33.46	89.47	120.54	157.99	244.93
	0:37:48	9.48	15.14	10.22	16.11	40.66	114.89	160.07	217.88	370.89
	0:43:12	9.05	14.46	9.76	15.40	38.98	112.05	157.87	217.76	396.70
	0:48:36	8.24	13.17	8.88	14.02	35.48	102.62	145.16	201.10	372.27
	0:54:00	7.36	11.79	7.93	12.55	31.90	92.56	131.02	181.62	337.53
	0:59:24	6.35	10.20	6.85	10.87	27.81	81.35	115.46	160.46	303.55
	1:04:48	5.53	8.88	5.97	9.46	24.21	71.19	101.13	140.64	270.81
	1:10:12	5.01	8.05	5.41	8.57	21.92	63.93	90.52	125.45	241.80
	1:15:36	4.13	6.67	4.46	7.11	18.31	54.03	76.96	107.31	209.41
	1:21:00	3.38	5.47	3.65	5.83	15.13	44.91	64.06	89.43	177.93
	1:26:24	2.60	4.24	2.81	4.53	11.91	35.94	51.53	72.33	148.32
	1:31:48	1.93	3.18	2.09	3.40	9.13	27.99	40.28	56.74	121.33
	1:37:12	1.40	2.30	1.52	2.47	6.76	21.12	30.55	43.29	97.87
	1:42:36	1.09	1.77	1.17	1.90	5.11	15.68	22.55	32.04	76.98
	1:48:00	0.89	1.46	0.97	1.56	4.13	12.48	17.85	25.15	59.05
	1:53:24	0.76	1.23	0.82	1.32	3.49	10.47	14.94	20.95	46.42
	1:58:48	0.67	1.08	0.72	1.15	3.04	9.07	12.91	18.05	38.79
	2:04:12	0.60	0.97	0.65	1.04	2.72	8.09	11.49	16.03	33.72
	2:09:36	0.55	0.90	0.60	0.96	2.50	7.39	10.48	14.59	30.13
	2:15:00	0.41	0.66	0.44	0.70	1.85	5.62	8.09	11.44	24.60
	2:20:24	0.30	0.48	0.32	0.51	1.34	4.05	5.83	8.25	18.28
	2:25:48	0.22	0.35	0.24	0.38	0.99	3.01	4.33	6.11	13.29
	2:31:12	0.16	0.26	0.17	0.28	0.74	2.23	3.21	4.54	9.86
	2:36:36	0.11	0.19	0.12	0.20	0.54	1.65	2.38	3.36	7.46
	2:42:00	0.08	0.13	0.09	0.14	0.38	1.19	1.72	2.44	5.57
	2:47:24	0.06	0.10	0.06	0.10	0.28	0.86	1.24	1.76	4.14
	2:52:48	0.04	0.06	0.04	0.07	0.19	0.61	0.89	1.27	3.08
	2:58:12	0.02	0.04	0.02	0.04	0.12	0.40	0.59	0.85	2.26
	3:03:36	0.01	0.02	0.01	0.02	0.07	0.24	0.35	0.52	1.57
	3:09:00	0.00	0.01	0.00	0.01	0.03	0.11	0.18	0.27	1.00
	3:14:24	0.00	0.00	0.00	0.00	0.01	0.04	0.06	0.10	0.56
	3:19:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.24
	3:25:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.06
	3:30:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:36:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:41:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:46:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:52:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:57:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:03:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:08:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:13:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:19:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:24:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:46:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:51:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:57:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:02:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:07:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:13:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:18:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:24:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:29:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:34:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:51:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:56:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:01:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:07:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:12:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:18:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:23:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:28:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]
TOP OF MICROPOOL	0.00	160	0.004	0	0.000	0.00
WQCV	3.30	23,580	0.541	24,806	0.569	0.29
2 YR. WSE	3.39	25,016	0.574	26,993	0.620	0.29
EURV	3.87	32,678	0.750	40,839	0.938	0.33
5 YR. WSE	3.95	33,955	0.779	43,504	0.999	0.35
TOP OF BOX	4.00	34,753	0.798	45,222	1.038	0.36
100 YR. WSE	7.58	58,250	1.337	221,023	5.074	142.28

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

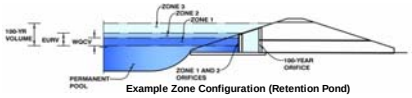
Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: FLYING HORSE NORTH FILING 1

Basin ID: POND 8 (FULL BUILD-OUT)



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type =	EDB	
Watershed Area =	255.00	acres
Watershed Length =	5,500	ft
Watershed Slope =	0.017	ft/ft
Watershed Imperviousness =	10.00%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Desired WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	
Water Quality Capture Volume (WQCV) =	1.424	acre-feet
Excess Urban Runoff Volume (EURV) =	2.397	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	1.648	acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	2.564	acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	5.846	acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	15.453	acre-feet
50-yr Runoff Volume (P1 = 2.25 in.) =	21.458	acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	29.303	acre-feet
500-yr Runoff Volume (P1 = 3.85 in.) =	54.585	acre-feet
Approximate 2-yr Detention Volume =	1.530	acre-feet
Approximate 5-yr Detention Volume =	2.400	acre-feet
Approximate 10-yr Detention Volume =	4.982	acre-feet
Approximate 25-yr Detention Volume =	6.957	acre-feet
Approximate 50-yr Detention Volume =	7.278	acre-feet
Approximate 100-yr Detention Volume =	9.408	acre-feet

Optional User Override 1-hr Precipitation

1.19	inches
1.50	inches
1.75	inches
2.00	inches
2.25	inches
2.52	inches
3.85	inches

Stage-Storage Calculation

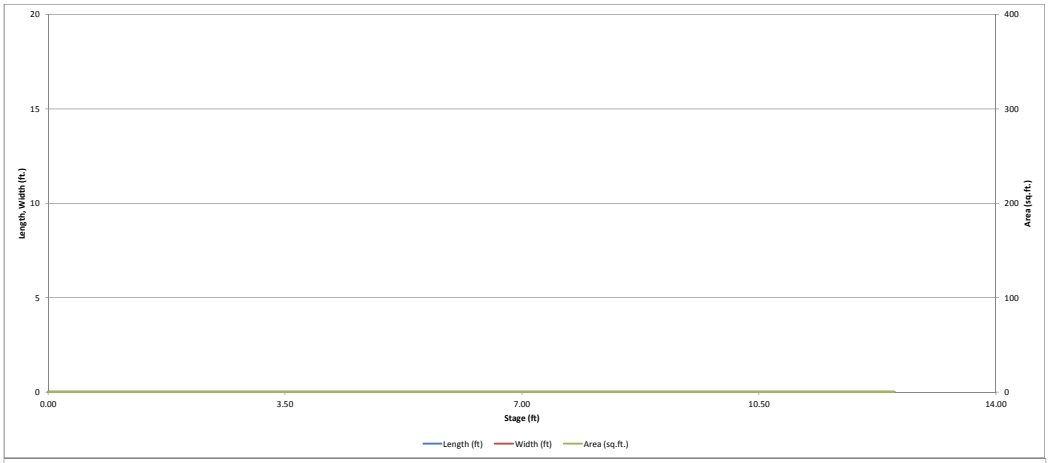
Zone 1 Volume (WQCV) =	1.424	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.973	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	7.011	acre-feet
Total Detention Basin Volume =	9.408	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	user	acre-feet

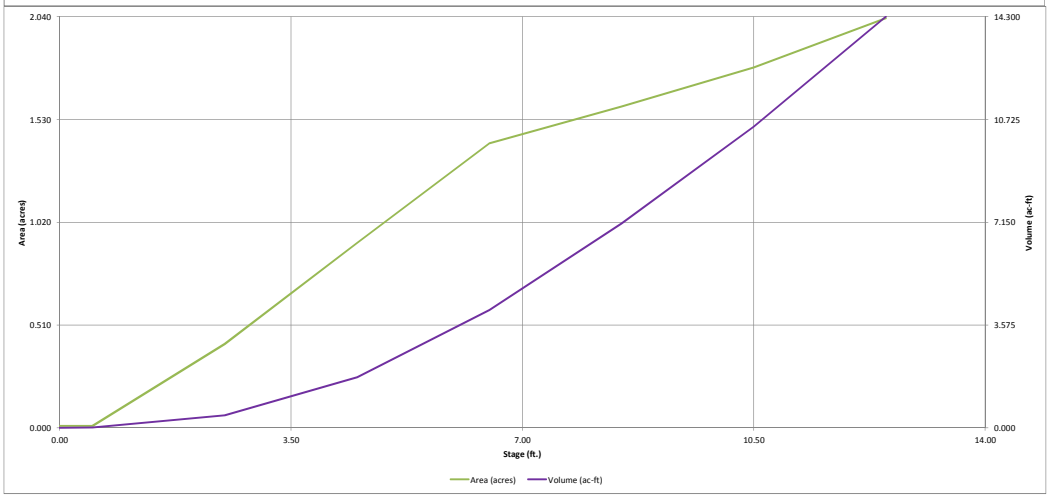
Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	384	0.009		
7370	--	0.50	--	--	--	384	0.009	188	0.004
7372	--	2.50	--	--	--	18,082	0.415	18,658	0.428
7374	--	4.50	--	--	--	40,004	0.918	76,744	1.762
7376	--	6.50	--	--	--	61,465	1.411	178,213	4.091
7378	--	8.50	--	--	--	69,433	1.594	309,111	7.096
7380	--	10.50	--	--	--	77,895	1.788	456,439	10.478
7382	--	12.50	--	--	--	88,522	2.032	622,856	14.299

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)



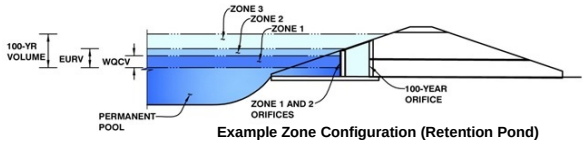


Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: FLYING HORSE NORTH FILING 1

Basin ID: POND 8 (FULL BUILD-OUT)



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	4.12	1.424	Orifice Plate
Zone 2 (EURV)	5.14	0.973	Orifice Plate
Zone 3 (100-year)	9.90	7.011	Weir&Pipe (Restrict)
		9.408	Total

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)	Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Diameter =	N/A	inches	Underdrain Orifice Centroid =	N/A	feet

Calculated Parameters for Underdrain

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)	WQ Orifice Area per Row =	N/A	ft^2
Depth at top of Zone using Orifice Plate =	5.25	ft (relative to basin bottom at Stage = 0 ft)	Elliptical Half-Width =	N/A	feet
Orifice Plate: Orifice Vertical Spacing =	21.00	inches	Elliptical Slot Centroid =	N/A	feet
Orifice Plate: Orifice Area per Row =	N/A	inches	Elliptical Slot Area =	N/A	ft^2

Calculated Parameters for Plate

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.80	3.60					
Orifice Area (sq. inches)	5.35	5.35	5.35					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected			Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Area =	N/A	N/A	ft^2
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)	Vertical Orifice Centroid =	N/A	N/A	feet
Vertical Orifice Diameter =	N/A	N/A	inches				

Calculated Parameters for Vertical Orifice

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 3 Weir	Not Selected			Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	5.25	N/A	ft (relative to basin bottom at Stage = 0 ft)	Height of Grate Upper Edge, H_t =	6.25	N/A	feet
Overflow Weir Front Edge Length =	16.00	N/A	feet	Over Flow Weir Slope Length =	4.12	N/A	feet
Overflow Weir Slope =	4.00	N/A	H:V (enter zero for flat grate)	Grate Open Area / 100-yr Orifice Area =	2.66	N/A	should be ≥ 4
Horiz. Length of Weir Sides =	4.00		feet	Overflow Grate Open Area w/o Debris =	49.48	N/A	ft^2
Overflow Grate Open Area % =	75%	N/A	%, grate open area/total area	Overflow Grate Open Area w/ Debris =	24.74	N/A	ft^2
Debris Clogging % =	50%	N/A	%				

Calculated Parameters for Overflow Weir

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected			Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.50	N/A	ft (distance below basin bottom at Stage = 0 ft)	Outlet Orifice Area =	18.61	N/A	ft^2
Outlet Pipe Diameter =	60.00	N/A	inches	Outlet Orifice Centroid =	2.38	N/A	feet
Restrictor Plate Height Above Pipe Invert =	54.00		inches	Half-Central Angle of Restrictor Plate on Pipe =	2.50	N/A	radians

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	10.00	ft (relative to basin bottom at Stage = 0 ft)	Spillway Design Flow Depth=	1.38	feet
Spillway Crest Length =	75.00	feet	Stage at Top of Freeboard =	12.38	feet
Spillway End Slopes =	4.00	H:V	Basin Area at Top of Freeboard =	2.02	acres
Freeboard above Max Water Surface =	1.00	feet			

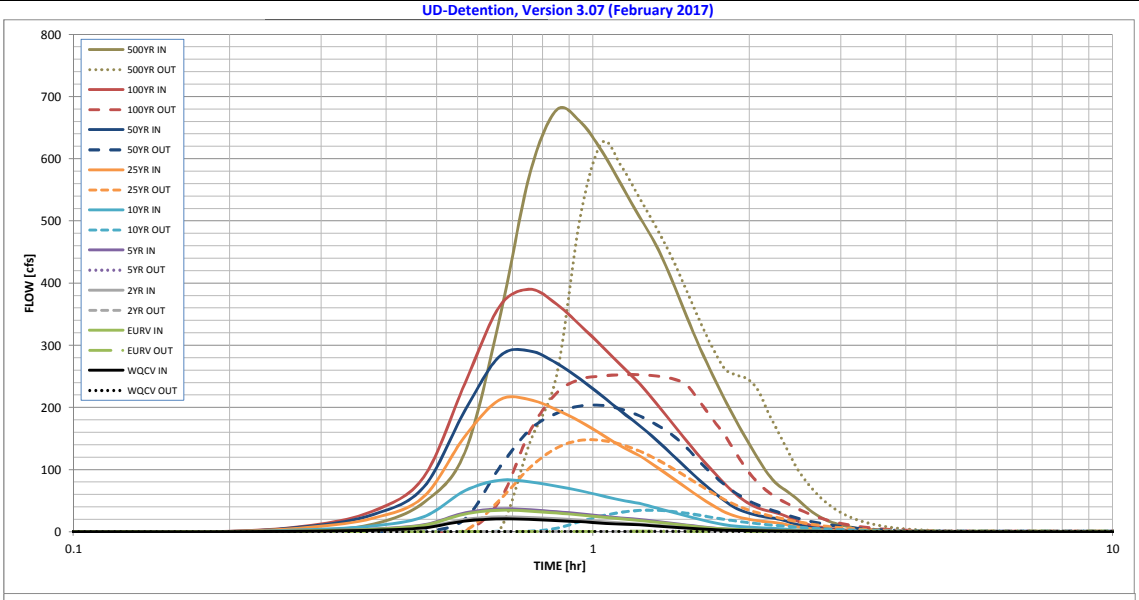
Calculated Parameters for Spillway

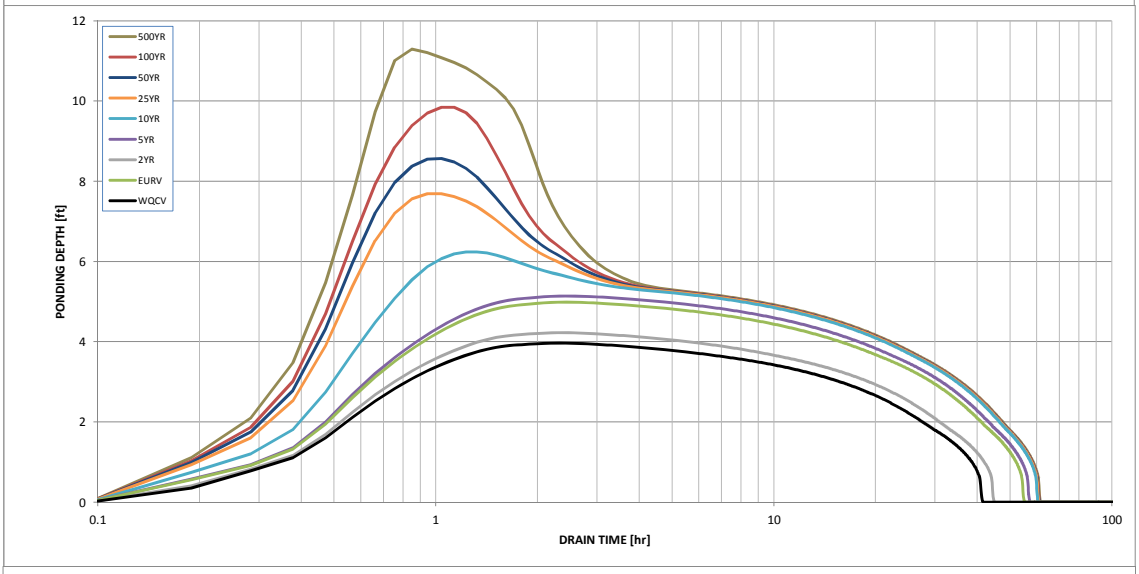
Routed Hydrograph Results

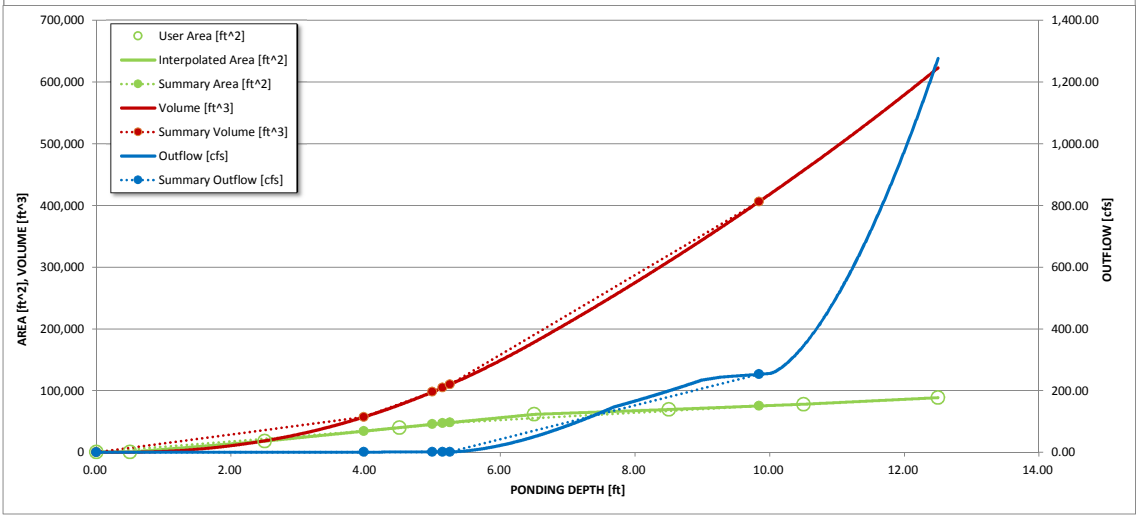
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.85
Calculated Runoff Volume (acre-ft) =	1.424	2.397	1.648	2.564	5.846	15.453	21.458	29.303	54.585
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	1.423	2.395	1.646	2.562	5.839	15.442	21.440	29.281	54.548
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.02	0.17	0.57	0.79	1.07	1.95
Predevelopment Peak Q (cfs) =	0.0	0.0	2.6	4.5	42.9	145.9	202.2	273.7	497.4
Peak Inflow Q (cfs) =	20.6	34.5	23.8	36.9	82.8	212.4	290.8	390.0	677.9
Peak Outflow Q (cfs) =	0.7	0.9	0.8	1.0	34.7	147.2	203.6	252.9	625.6
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.2	0.8	1.0	1.0	0.9	1.3
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	0.7	2.9	4.1	5.1	5.5
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	50	41	52	50	40	36	31	18
Time to Drain 99% of Inflow Volume (hours) =	**40**	53	43	55	56	51	48	46	39
Maximum Ponding Depth (ft) =	3.97	4.99	4.22	5.14	6.24	7.69	8.57	9.84	11.30
Area at Maximum Ponding Depth (acres) =	0.78	1.04	0.85	1.07	1.35	1.52	1.60	1.72	1.88
Maximum Volume Stored (acre-ft) =	1.303	2.231	1.515	2.389	3.733	5.835	7.192	9.319	11.929

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename:

Storm Inflow Hydrographs **UD-Detention, Version 3.07 (February 2017)**

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.67 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:40	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Hydrograph	0:11:20	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Constant	0:17:01	0.90	1.47	1.03	1.57	3.32	7.06	8.52	9.82	12.32
0.881	0:22:41	2.44	4.04	2.81	4.31	9.37	21.88	27.67	33.54	46.83
	0:28:21	6.26	10.37	7.21	11.07	24.07	56.31	71.67	88.06	127.54
	0:34:01	17.19	28.47	19.80	30.39	65.93	153.46	194.79	238.36	343.20
	0:39:41	20.64	34.51	23.84	36.89	82.80	212.27	282.31	362.92	575.41
	0:45:22	19.75	33.07	22.82	35.36	80.17	212.38	290.77	389.96	677.94
	0:51:02	17.98	30.10	20.77	32.18	73.22	196.20	271.27	366.28	659.59
	0:56:42	16.12	27.05	18.63	28.93	66.00	177.21	245.63	332.17	612.74
	1:02:22	13.98	23.55	16.18	25.20	57.88	156.59	218.92	299.06	557.82
	1:08:02	12.15	20.49	14.05	21.94	50.60	137.25	193.21	267.13	506.91
	1:13:43	11.02	18.56	12.75	19.86	45.56	122.39	171.19	238.73	460.88
	1:19:23	9.16	15.49	10.61	16.59	38.34	104.74	148.01	206.78	406.08
	1:25:03	7.54	12.79	8.74	13.70	31.83	87.30	124.78	175.85	347.82
	1:30:43	5.88	10.06	6.83	10.78	25.35	70.63	102.70	146.80	295.93
	1:36:23	4.45	7.69	5.19	8.26	19.67	55.42	82.28	120.36	251.22
	1:42:04	3.25	5.68	3.80	6.11	14.78	42.31	64.67	97.36	211.51
	1:47:44	2.48	4.30	2.89	4.62	11.02	31.33	49.21	76.89	177.08
	1:53:24	2.03	3.49	2.36	3.74	8.82	24.57	37.34	59.19	145.61
	1:59:04	1.71	2.94	1.99	3.15	7.41	20.46	30.48	46.30	117.11
	2:04:44	1.50	2.57	1.74	2.75	6.43	17.62	25.97	38.58	92.60
	2:10:25	1.35	2.30	1.57	2.47	5.74	15.65	22.87	33.48	75.52
	2:16:05	1.24	2.11	1.44	2.26	5.25	14.24	20.63	29.88	64.95
	2:21:45	0.91	1.56	1.06	1.67	3.95	11.18	16.56	24.44	54.24
	2:27:25	0.67	1.14	0.77	1.22	2.86	8.06	12.07	18.21	42.19
	2:33:05	0.49	0.84	0.57	0.90	2.12	5.97	8.83	13.23	31.42
	2:38:46	0.36	0.62	0.42	0.67	1.57	4.43	6.59	9.80	23.43
	2:44:26	0.26	0.45	0.30	0.48	1.16	3.28	4.92	7.43	17.42
	2:50:06	0.19	0.32	0.22	0.35	0.83	2.39	3.63	5.56	13.19
	2:55:46	0.13	0.23	0.16	0.25	0.60	1.72	2.65	4.13	10.01
	3:01:26	0.09	0.16	0.11	0.17	0.43	1.24	1.96	3.08	7.60
	3:07:07	0.06	0.10	0.07	0.11	0.28	0.83	1.38	2.27	5.82
	3:12:47	0.03	0.06	0.04	0.06	0.16	0.51	0.90	1.58	4.44
	3:18:27	0.01	0.02	0.01	0.03	0.08	0.26	0.52	1.02	3.25
	3:24:07	0.00	0.01	0.00	0.01	0.02	0.10	0.25	0.58	2.24
	3:29:47	0.00	0.00	0.00	0.00	0.00	0.01	0.07	0.26	1.42
	3:35:28	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.07	0.78
	3:41:08	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.34
	3:46:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.07
	3:52:28	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:58:08	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:03:49	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:09:29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:09	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:49	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:26:29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:32:10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:37:50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:43:30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:49:10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:54:50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:06:11	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:11:51	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:17:31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:23:11	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:28:52	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:34:32	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:52	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:51:32	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:57:13	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:02:53	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:08:33	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:14:13	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:19:53	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:25:34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:31:14	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:36:54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:42:34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:48:14	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]
TOP OF MICROPOOL	0.00	384	0.009	0	0.000	0.00
WQCV	3.97	34,195	0.785	57,081	1.310	0.73
EURV	4.99	45,262	1.039	97,634	2.241	0.93
5 YR. WSE	5.14	46,872	1.076	104,544	2.400	0.95
TOP OF BOX	5.25	48,052	1.103	109,765	2.520	0.97
100 YR. WSE	9.84	75,103	1.724	405,950	9.319	252.87

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

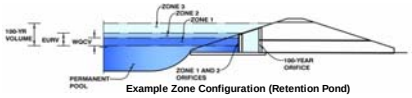
Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: FLYING HORSE NORTH FILING 1

Basin ID: POND 8 (FILING 1 ONLY INCL. GOLF COURSE)



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type =	EDB	
Watershed Area =	255.00	acres
Watershed Length =	6,000	ft
Watershed Slope =	0.006	ft/ft
Watershed Imperviousness =	5.00%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Desired WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	
Water Quality Capture Volume (WQCV) =	0.768	acre-feet
Excess Urban Runoff Volume (EURV) =	1.134	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.728	acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	1.198	acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	4.039	acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	13.847	acre-feet
50-yr Runoff Volume (P1 = 2.25 in.) =	19.909	acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	27.825	acre-feet
500-yr Runoff Volume (P1 = 3.85 in.) =	52.818	acre-feet
Approximate 2-yr Detention Volume =	0.673	acre-feet
Approximate 5-yr Detention Volume =	1.120	acre-feet
Approximate 10-yr Detention Volume =	3.339	acre-feet
Approximate 25-yr Detention Volume =	5.092	acre-feet
Approximate 50-yr Detention Volume =	5.177	acre-feet
Approximate 100-yr Detention Volume =	6.915	acre-feet

Optional User Override 1-hr Precipitation

1.19	inches
1.50	inches
1.75	inches
2.00	inches
2.25	inches
2.52	inches
3.85	inches

Stage-Storage Calculation

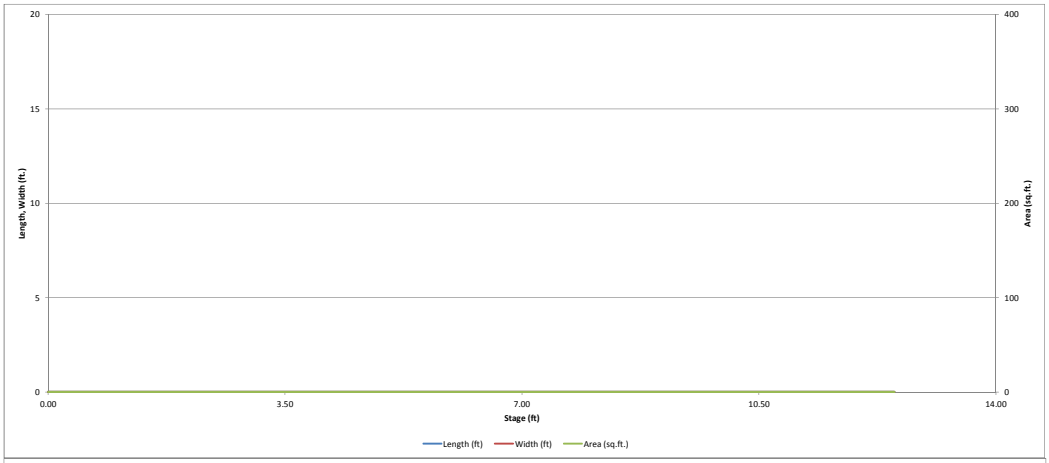
Zone 1 Volume (WQCV) =	0.768	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.366	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	5.782	acre-feet
Total Detention Basin Volume =	6.915	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	user	acre-feet

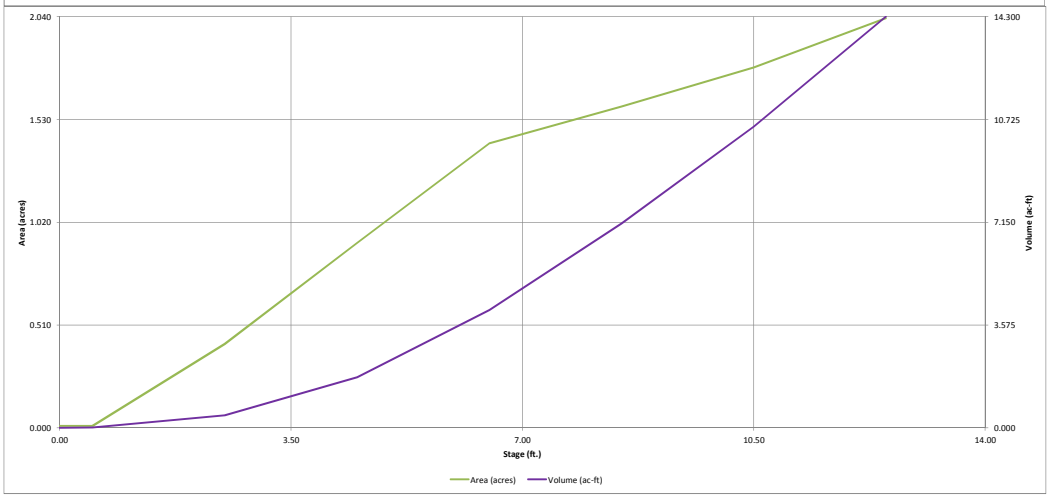
Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	384	0.009		
7370	--	0.50	--	--	--	384	0.009	188	0.004
7372	--	2.50	--	--	--	18,082	0.415	18,658	0.428
7374	--	4.50	--	--	--	40,004	0.918	76,744	1.762
7376	--	6.50	--	--	--	61,465	1.411	178,213	4.091
7378	--	8.50	--	--	--	69,433	1.594	309,111	7.096
7380	--	10.50	--	--	--	77,895	1.788	456,439	10.478
7382	--	12.50	--	--	--	88,522	2.032	622,856	14.299

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)



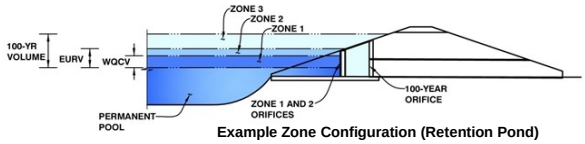


Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: FLYING HORSE NORTH FILING 1

Basin ID: POND 8 (FILING 1 ONLY INCL. GOLF COURSE)



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.18	0.768	Orifice Plate
Zone 2 (EURV)	3.74	0.366	Orifice Plate
Zone 3 (100-year)	8.39	5.782	Weir&Pipe (Restrict)
		6.915	Total

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	5.25	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	21.00	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft^2
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft^2

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.80	3.60					
Orifice Area (sq. inches)	3.75	3.75	3.75					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft^2
Vertical Orifice Centroid =	N/A	N/A	feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	5.25	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	16.00	N/A	feet
Overflow Weir Slope =	4.00	N/A	H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	4.00		feet
Overflow Grate Open Area % =	75%	N/A	%, grate open area/total area
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	6.25	N/A	feet
Over Flow Weir Slope Length =	4.12	N/A	feet
Grate Open Area / 100-yr Orifice Area =	2.94	N/A	should be ≥ 4
Overflow Grate Open Area w/o Debris =	49.48	N/A	ft^2
Overflow Grate Open Area w/ Debris =	24.74	N/A	ft^2

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.50	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	60.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	48.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	16.84	N/A	ft^2
Outlet Orifice Centroid =	2.18	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	2.21	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	10.00	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	75.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

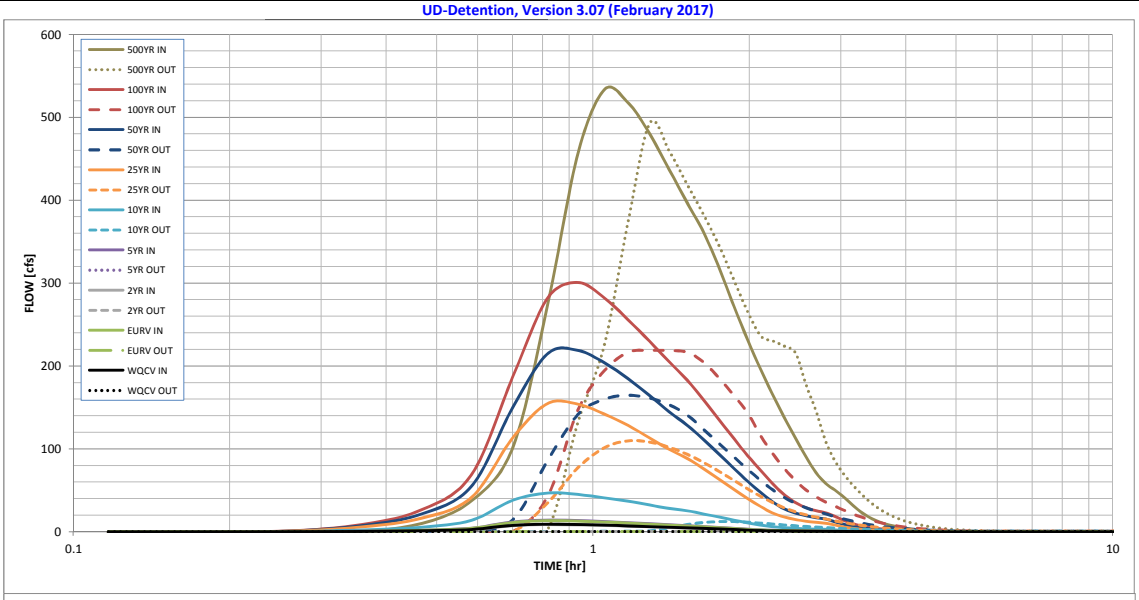
Spillway Design Flow Depth=	1.38	feet
Stage at Top of Freeboard =	12.38	feet
Basin Area at Top of Freeboard =	2.02	acres

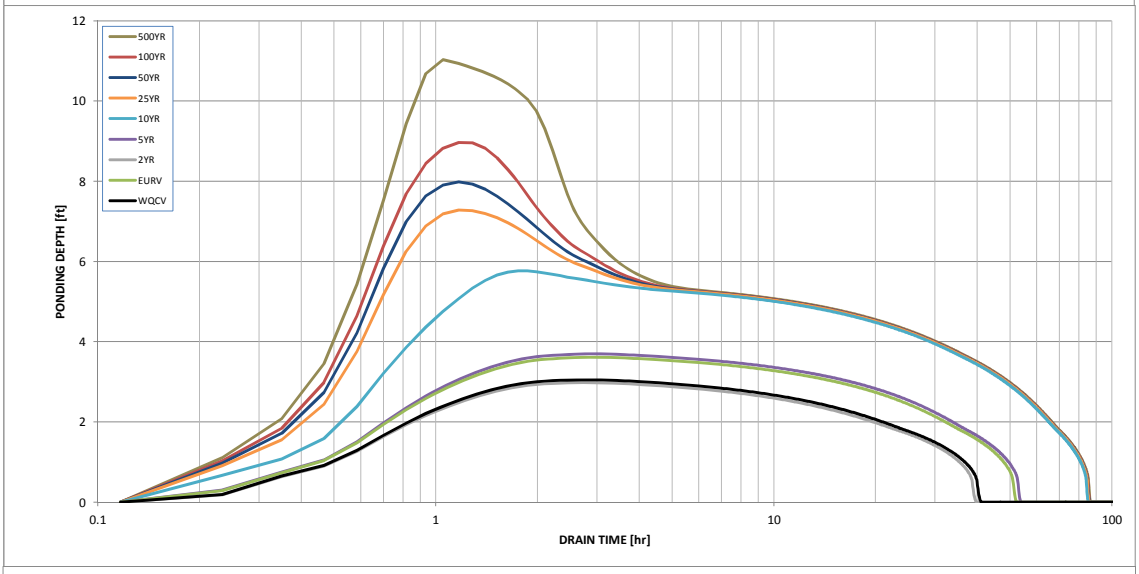
Routed Hydrograph Results

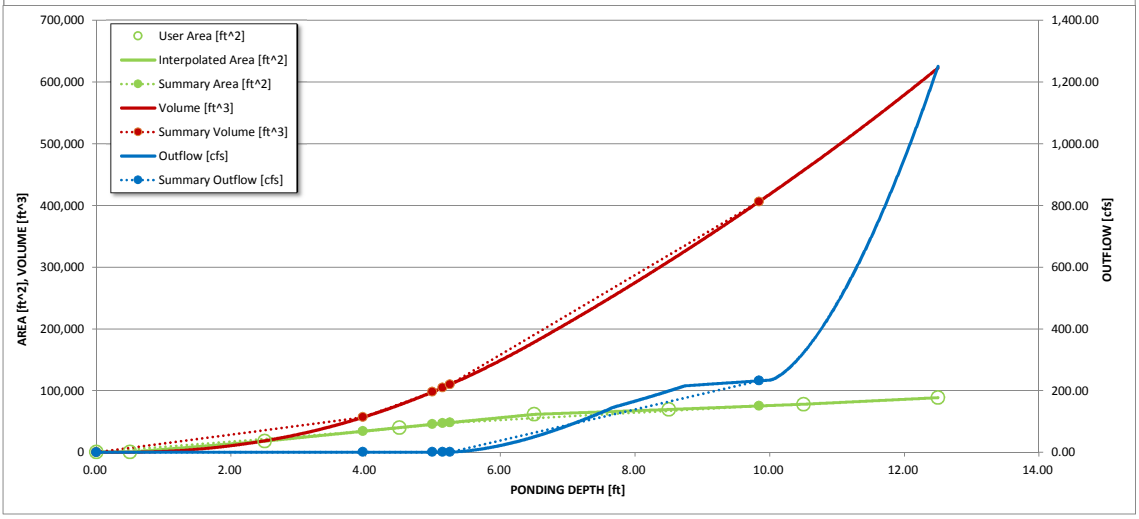
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.85
Calculated Runoff Volume (acre-ft) =	0.768	1.134	0.728	1.198	4.039	13.847	19.909	27.825	52.818
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.768	1.133	0.728	1.197	4.038	13.842	19.906	27.823	52.817
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.02	0.14	0.49	0.68	0.93	1.69
Predevelopment Peak Q (cfs) =	0.0	0.0	2.2	3.9	35.8	125.2	173.8	236.6	431.3
Peak Inflow Q (cfs) =	9.1	13.3	8.6	14.1	46.7	154.6	218.8	300.7	533.2
Peak Outflow Q (cfs) =	0.4	0.4	0.4	0.5	12.4	109.9	164.8	218.9	492.9
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.1	0.3	0.9	0.9	0.9	1.1
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	0.2	2.2	3.3	4.4	5.0
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	48	37	49	74	58	52	44	25
Time to Drain 99% of Inflow Volume (hours) =	**40**	50	38	52	80	73	69	65	55
Maximum Ponding Depth (ft) =	3.05	3.61	2.98	3.70	5.77	7.28	7.98	8.97	11.03
Area at Maximum Ponding Depth (acres) =	0.55	0.69	0.53	0.71	1.23	1.48	1.55	1.64	1.85
Maximum Volume Stored (acre-ft) =	0.689	1.044	0.651	1.100	3.114	5.220	6.280	7.840	11.425

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename:

Storm Inflow Hydrographs **UD-Detention, Version 3.07 (February 2017)**

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
7.01 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:07:01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Hydrograph	0:14:01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Constant	0:21:02	0.40	0.58	0.38	0.61	1.94	5.29	6.64	7.78	9.87
0.713	0:28:02	1.08	1.58	1.03	1.67	5.39	16.16	21.33	26.31	37.31
	0:35:03	2.78	4.06	2.63	4.28	13.84	41.55	55.07	68.92	101.34
	0:42:04	7.63	11.14	7.24	11.76	37.95	113.33	149.82	186.68	272.77
	0:49:04	9.07	13.34	8.61	14.08	46.69	154.61	214.68	281.86	455.14
	0:56:05	8.66	12.75	8.22	13.46	44.90	153.93	218.83	300.74	533.24
	1:03:05	7.89	11.61	7.48	12.25	40.89	141.97	203.92	282.08	517.64
	1:10:06	7.05	10.39	6.69	10.98	36.83	128.20	184.68	255.72	480.21
	1:17:07	6.09	9.00	5.78	9.51	32.21	113.19	164.23	229.85	436.94
	1:24:07	5.31	7.83	5.03	8.27	28.12	99.19	144.66	204.92	396.74
	1:31:08	4.81	7.10	4.56	7.50	25.39	88.56	128.40	182.88	360.38
	1:38:08	3.98	5.89	3.77	6.23	21.27	75.63	110.72	158.37	317.03
	1:45:09	3.25	4.84	3.08	5.11	17.62	63.01	93.00	134.50	271.55
	1:52:10	2.51	3.76	2.38	3.98	13.96	50.88	76.23	112.03	230.93
	1:59:10	1.88	2.83	1.77	3.00	10.78	39.88	60.70	91.53	195.76
	2:06:11	1.36	2.05	1.28	2.18	8.04	30.37	47.33	73.71	164.57
	2:13:11	1.05	1.58	0.99	1.67	6.04	22.45	35.75	57.86	137.44
	2:20:12	0.86	1.29	0.82	1.37	4.86	17.67	27.36	44.29	112.68
	2:27:13	0.73	1.09	0.69	1.16	4.09	14.74	22.45	34.91	90.29
	2:34:13	0.64	0.96	0.61	1.02	3.56	12.71	19.19	29.22	71.20
	2:41:14	0.58	0.86	0.55	0.91	3.18	11.29	16.93	25.42	58.37
	2:48:14	0.53	0.79	0.50	0.84	2.91	10.28	15.31	22.73	50.37
	2:55:15	0.39	0.58	0.37	0.62	2.17	8.03	12.22	18.54	42.05
	3:02:16	0.29	0.43	0.27	0.45	1.57	5.79	8.87	13.76	32.64
	3:09:16	0.21	0.31	0.20	0.33	1.16	4.29	6.53	10.00	24.27
	3:16:17	0.15	0.23	0.15	0.25	0.86	3.18	4.86	7.43	18.07
	3:23:17	0.11	0.17	0.11	0.18	0.63	2.36	3.62	5.61	13.43
	3:30:18	0.08	0.12	0.07	0.13	0.45	1.71	2.66	4.19	10.19
	3:37:19	0.06	0.09	0.05	0.09	0.33	1.24	1.93	3.11	7.72
	3:44:19	0.04	0.06	0.04	0.06	0.23	0.89	1.42	2.32	5.86
	3:51:20	0.02	0.03	0.02	0.04	0.15	0.59	0.99	1.69	4.49
	3:58:20	0.01	0.02	0.01	0.02	0.08	0.36	0.64	1.17	3.41
	4:05:21	0.00	0.01	0.00	0.01	0.04	0.18	0.36	0.74	2.48
	4:12:22	0.00	0.00	0.00	0.00	0.01	0.06	0.16	0.41	1.70
	4:19:22	0.00	0.00	0.00	0.00	0.00	0.00	0.04	0.18	1.06
	4:26:23	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.04	0.58
	4:33:23	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.24
	4:40:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.04
	4:47:25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:54:25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:01:26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:08:26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:27	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:22:28	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:29:28	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:36:29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:43:29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:57:31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:04:31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:11:32	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:18:32	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:25:33	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:32:34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:39:34	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:46:35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:53:35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:00:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:07:37	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:14:37	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:21:38	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:28:38	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:35:39	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:42:40	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:49:40	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:56:41	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	8:03:41	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	8:10:42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	8:17:43	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	8:24:43	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]
TOP OF MICROPOOL	0.00	384	0.009	0	0.000	0.00
WQCV	3.96	34,085	0.782	56,740	1.303	0.51
EURV	4.99	45,262	1.039	97,634	2.241	0.65
5 YR. WSE	5.14	46,872	1.076	104,544	2.400	0.67
TOP OF BOX	5.25	48,052	1.103	109,765	2.520	0.68
100 YR. WSE	9.84	75,103	1.724	405,950	9.319	231.57

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

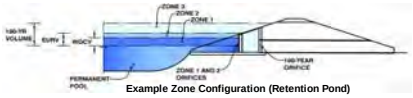
Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: Flying Horse North Filing No. 1

Basin ID: Pond 12



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type =	EDB	
Watershed Area =	57.60	acres
Watershed Length =	3,000	ft
Watershed Slope =	0.014	ft/ft
Watershed Imperviousness =	12.00%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Desired WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths =	User Input	
Water Quality Capture Volume (WQCV) =	0.375	acre-feet
Excess Urban Runoff Volume (EURV) =	0.659	acre-feet
2-yr Runoff Volume (P1 = 1.19 in.) =	0.461	acre-feet
5-yr Runoff Volume (P1 = 1.5 in.) =	0.708	acre-feet
10-yr Runoff Volume (P1 = 1.75 in.) =	1.484	acre-feet
25-yr Runoff Volume (P1 = 2 in.) =	3.636	acre-feet
50-yr Runoff Volume (P1 = 2.25 in.) =	4.987	acre-feet
100-yr Runoff Volume (P1 = 2.52 in.) =	6.752	acre-feet
500-yr Runoff Volume (P1 = 3.39 in.) =	10.997	acre-feet
Approximate 2-yr Detention Volume =	0.429	acre-feet
Approximate 5-yr Detention Volume =	0.663	acre-feet
Approximate 10-yr Detention Volume =	1.275	acre-feet
Approximate 25-yr Detention Volume =	1.727	acre-feet
Approximate 50-yr Detention Volume =	1.814	acre-feet
Approximate 100-yr Detention Volume =	2.319	acre-feet

Optional User Override 1-hr Precipitation	
1.19	inches
1.50	inches
1.75	inches
2.00	inches
2.25	inches
2.52	inches
3.39	inches

Stage-Storage Calculation

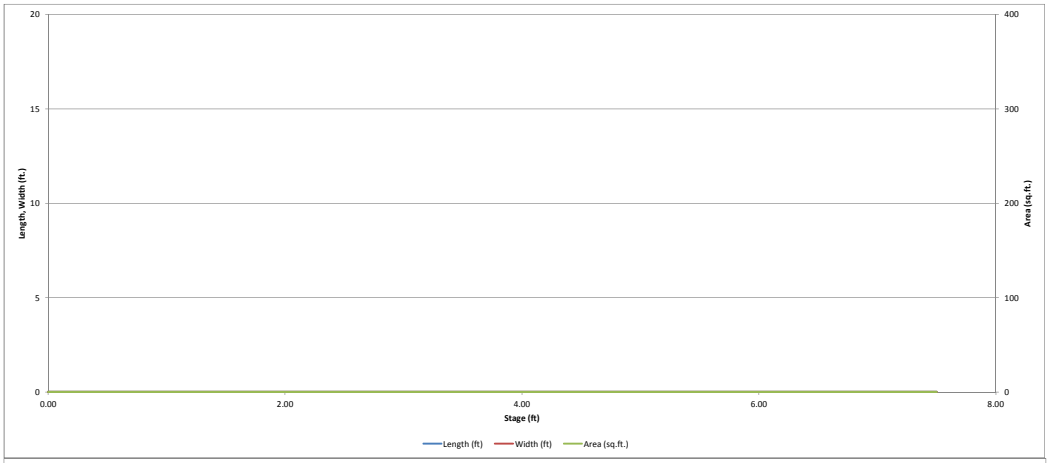
Zone 1 Volume (WQCV) =	0.375	acre-feet
Zone 2 Volume (EURV - Zone 1) =	0.285	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	1.660	acre-feet
Total Detention Basin Volume =	2.319	acre-feet
Initial Surcharge Volume (ISV) =	user	ft^3
Initial Surcharge Depth (ISD) =	user	ft
Total Available Detention Depth (H_{total}) =	user	ft
Depth of Trickle Channel (H_{TC}) =	user	ft
Slope of Trickle Channel (S_{TC}) =	user	ft/ft
Slopes of Main Basin Sides (S_{main}) =	user	H:V
Basin Length-to-Width Ratio ($R_{L/W}$) =	user	
Initial Surcharge Area (A_{ISV}) =	user	ft^2
Surcharge Volume Length (L_{ISV}) =	user	ft
Surcharge Volume Width (W_{ISV}) =	user	ft
Depth of Basin Floor (H_{FLOOR}) =	user	ft
Length of Basin Floor (L_{FLOOR}) =	user	ft
Width of Basin Floor (W_{FLOOR}) =	user	ft
Area of Basin Floor (A_{FLOOR}) =	user	ft^2
Volume of Basin Floor (V_{FLOOR}) =	user	ft^3
Depth of Main Basin (H_{MAIN}) =	user	ft
Length of Main Basin (L_{MAIN}) =	user	ft
Width of Main Basin (W_{MAIN}) =	user	ft
Area of Main Basin (A_{MAIN}) =	user	ft^2
Volume of Main Basin (V_{MAIN}) =	user	ft^3
Calculated Total Basin Volume (V_{total}) =	user	acre-feet

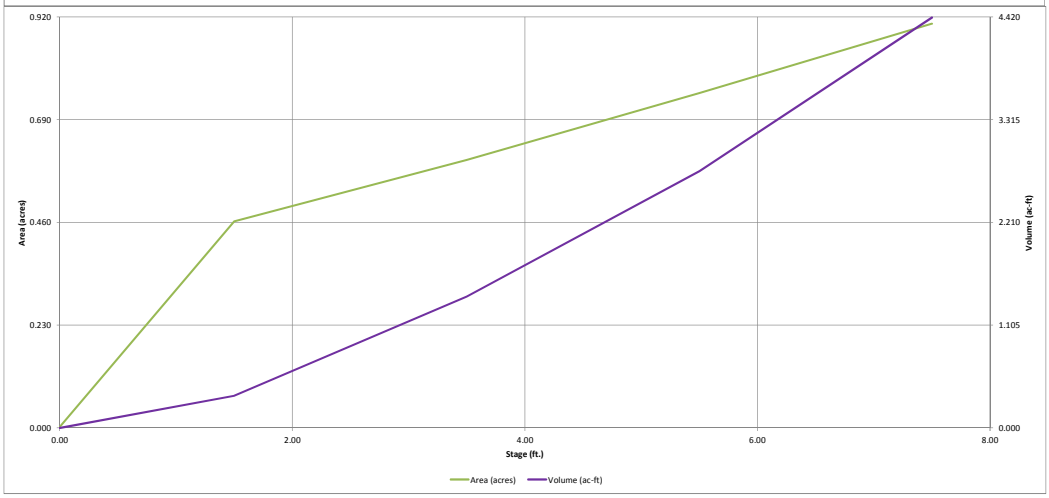
Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	100	0.002		
7546	--	1.50	--	--	--	20,138	0.462	14,977	0.344
7548	--	3.50	--	--	--	26,118	0.600	61,434	1.410
7550	--	5.50	--	--	--	32,634	0.749	120,186	2.759
7552	--	7.50	--	--	--	39,405	0.905	192,225	4.413

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)



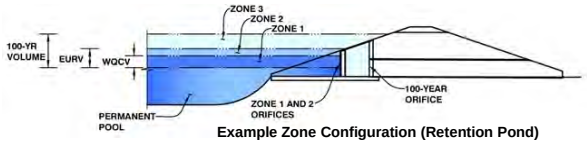


Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: Flying Horse North Filing No. 1

Basin ID: Pond 12



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.56	0.375	Orifice Plate
Zone 2 (EURV)	2.15	0.285	Orifice Plate
Zone 3 (100-year)	4.90	1.660	Weir&Pipe (Restrict)
		2.319	Total

0

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	2.25	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	9.00	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft^2
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft^2

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.80	1.60					
Orifice Area (sq. inches)	2.41	2.79	2.79					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft^2
Vertical Orifice Centroid =	N/A	N/A	feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	2.25	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	6.00	N/A	feet
Overflow Weir Slope =	4.00	N/A	H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	4.00	N/A	feet
Overflow Grate Open Area % =	75%	N/A	%, grate open area/total area
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	3.25	N/A	feet
Over Flow Weir Slope Length =	4.12	N/A	feet
Grate Open Area / 100-yr Orifice Area =	4.41	N/A	should be ≥ 4
Overflow Grate Open Area w/o Debris =	18.55	N/A	ft^2
Overflow Grate Open Area w/ Debris =	9.28	N/A	ft^2

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.50	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	30.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	24.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	4.21	N/A	ft^2
Outlet Orifice Centroid =	1.09	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	2.21	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	5.50	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	20.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

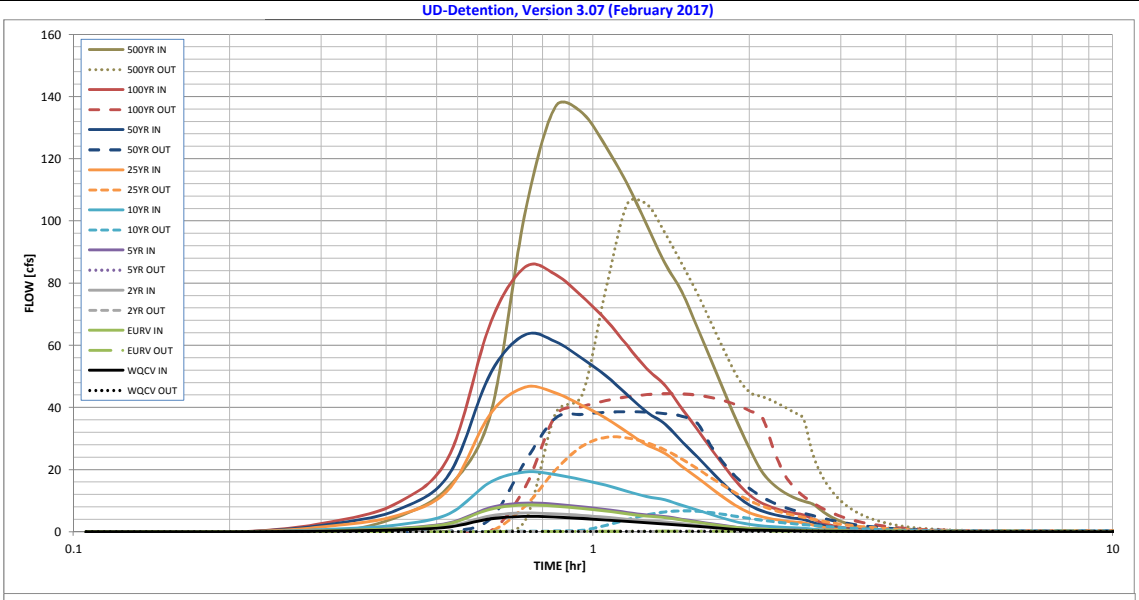
Spillway Design Flow Depth=	1.13	feet
Stage at Top of Freeboard =	7.63	feet
Basin Area at Top of Freeboard =	0.90	acres

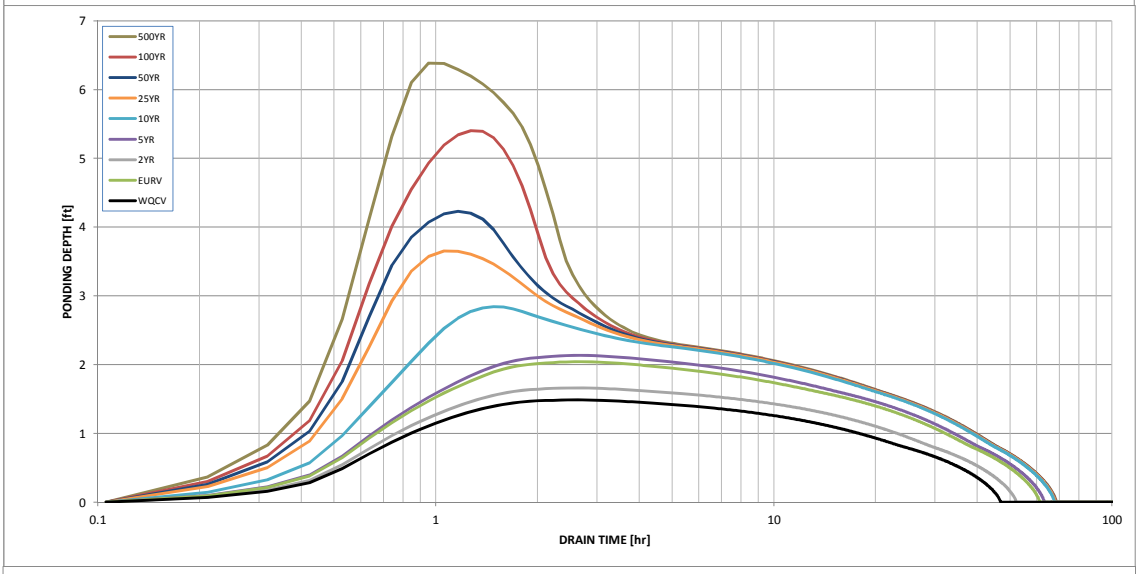
Routed Hydrograph Results

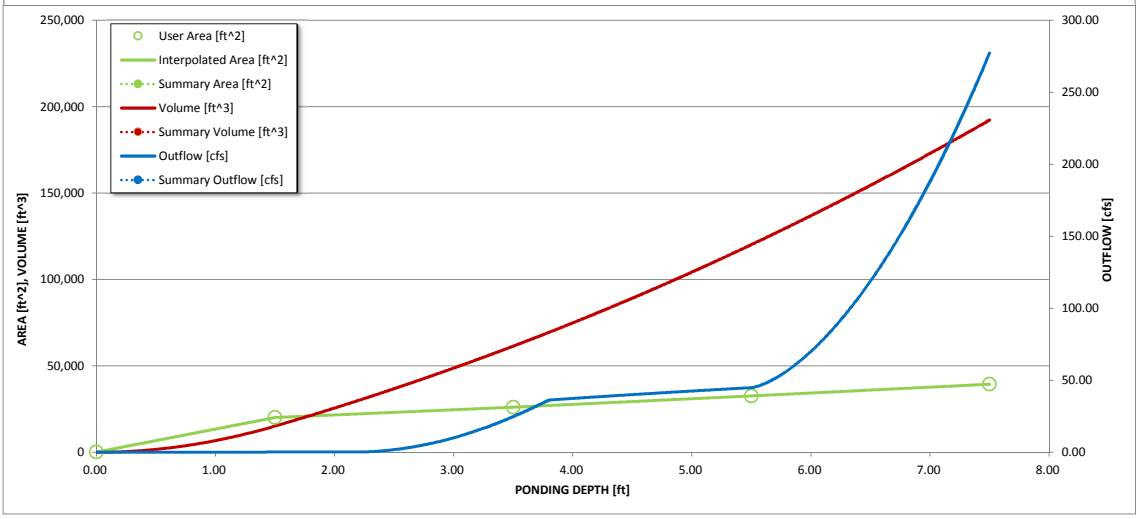
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.39
Calculated Runoff Volume (acre-ft) =	0.375	0.659	0.461	0.708	1.484	3.636	4.987	6.752	10.997
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.374	0.659	0.461	0.707	1.483	3.636	4.986	6.752	10.992
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.02	0.14	0.50	0.70	0.95	1.52
Predevelopment Peak Q (cfs) =	0.0	0.0	0.5	0.9	8.3	28.9	40.1	54.6	87.5
Peak Inflow Q (cfs) =	4.9	8.6	6.1	9.2	19.2	46.5	63.4	85.2	136.7
Peak Outflow Q (cfs) =	0.2	0.3	0.2	0.3	6.8	30.4	38.7	44.5	105.6
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.3	0.8	1.1	1.0	0.8	1.2
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Overflow Grate 1	Overflow Grate 1	Outlet Plate 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	0.3	1.6	2.1	2.4	2.6
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	43	55	47	56	56	44	40	35	26
Time to Drain 99% of Inflow Volume (hours) =	**45**	58	50	60	62	58	55	52	45
Maximum Ponding Depth (ft) =	1.49	2.04	1.66	2.13	2.84	3.65	4.23	5.40	6.38
Area at Maximum Ponding Depth (acres) =	0.46	0.50	0.47	0.51	0.55	0.61	0.65	0.74	0.82
Maximum Volume Stored (acre-ft) =	0.339	0.608	0.423	0.653	1.030	1.501	1.861	2.685	3.448

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename:

Storm Inflow Hydrographs **UD-Detention, Version 3.07 (February 2017)**

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
6.35 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:06:21	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Hydrograph	0:12:42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Constant	0:19:03	0.22	0.38	0.27	0.41	0.83	1.95	2.59	3.35	4.93
0.787	0:25:24	0.59	1.03	0.73	1.10	2.27	5.39	7.24	9.55	14.66
	0:31:45	1.52	2.64	1.86	2.83	5.82	13.84	18.60	24.51	37.68
	0:38:06	4.18	7.25	5.12	7.77	15.98	37.96	50.96	67.11	102.91
	0:44:27	4.93	8.61	6.05	9.24	19.20	46.50	63.37	85.18	136.71
	0:50:48	4.69	8.22	5.77	8.82	18.37	44.66	61.16	82.76	134.85
	0:57:09	4.27	7.48	5.25	8.03	16.72	40.65	55.78	75.69	123.99
	1:03:30	3.80	6.68	4.68	7.17	14.99	36.60	50.26	68.25	111.92
	1:09:51	3.27	5.77	4.03	6.20	13.01	31.99	44.03	59.91	98.63
	1:16:12	2.86	5.03	3.52	5.40	11.31	27.91	38.47	52.41	86.39
	1:22:33	2.59	4.56	3.19	4.89	10.25	25.22	34.68	47.13	77.33
	1:28:54	2.12	3.76	2.62	4.04	8.53	21.11	29.12	39.74	65.74
	1:35:15	1.72	3.07	2.13	3.30	7.02	17.48	24.16	33.01	54.72
	1:41:36	1.32	2.37	1.63	2.55	5.48	13.82	19.19	26.36	44.03
	1:47:57	0.97	1.76	1.21	1.90	4.15	10.65	14.86	20.49	34.41
	1:54:18	0.71	1.28	0.88	1.37	3.03	7.94	11.14	15.43	26.10
	2:00:39	0.55	0.99	0.68	1.06	2.31	5.96	8.33	11.48	19.26
	2:07:00	0.45	0.81	0.56	0.87	1.89	4.81	6.68	9.16	15.25
	2:13:21	0.39	0.69	0.48	0.74	1.60	4.05	5.61	7.69	12.76
	2:19:42	0.34	0.61	0.42	0.65	1.40	3.52	4.88	6.67	11.03
	2:26:03	0.31	0.55	0.38	0.59	1.26	3.15	4.36	5.95	9.82
	2:32:24	0.28	0.50	0.35	0.54	1.15	2.89	3.99	5.44	8.95
	2:38:45	0.21	0.37	0.26	0.40	0.85	2.15	2.99	4.12	6.91
	2:45:06	0.15	0.27	0.19	0.29	0.62	1.56	2.16	2.97	4.98
	2:51:27	0.11	0.20	0.14	0.21	0.46	1.15	1.60	2.20	3.70
	2:57:48	0.08	0.15	0.10	0.16	0.34	0.86	1.19	1.64	2.74
	3:04:09	0.06	0.10	0.07	0.11	0.24	0.63	0.87	1.21	2.03
	3:10:30	0.04	0.07	0.05	0.08	0.17	0.45	0.63	0.87	1.47
	3:16:51	0.03	0.05	0.04	0.06	0.13	0.33	0.46	0.63	1.06
	3:23:12	0.02	0.03	0.02	0.04	0.09	0.23	0.32	0.44	0.76
	3:29:33	0.01	0.02	0.01	0.02	0.05	0.15	0.21	0.29	0.50
	3:35:54	0.01	0.01	0.01	0.01	0.03	0.08	0.12	0.17	0.30
	3:42:15	0.00	0.00	0.00	0.00	0.01	0.04	0.06	0.08	0.15
	3:48:36	0.00	0.00	0.00	0.00	0.00	0.01	0.02	0.02	0.05
	3:54:57	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:01:18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:07:39	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:14:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:21	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:26:42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:33:03	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:39:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:45	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:52:06	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:58:27	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:04:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:11:09	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:17:30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:23:51	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:36:33	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:42:54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:49:15	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:36	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:01:57	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:08:18	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:14:39	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:21:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:27:21	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:33:42	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:40:03	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:46:24	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:52:45	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:59:06	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:05:27	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:11:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:18:09	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:24:30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:30:51	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	7:37:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Summary Stage-Area-Volume-Discharge Relationships

The user can create a summary S-A-V-D by entering the desired stage increments and the remainder of the table will populate automatically.

The user should graphically compare the summary S-A-V-D table to the full S-A-V-D table in the chart to confirm it captures all key transition points.

Stage - Storage Description	Stage [ft]	Area [ft^2]	Area [acres]	Volume [ft^3]	Volume [ac-ft]	Total Outflow [cfs]

For best results, include the stages of all grade slope changes (e.g. ISV and Floor) from the S-A-V table on Sheet 'Basin'.

Also include the inverts of all outlets (e.g. vertical orifice, overflow grate, and spillway, where applicable).

Project Summary	
Title	Flying Horse North Filing No.1
Engineer	MAW
Company	CCES
Date	4/4/2018

Notes	2 Year

Table of Contents

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
Area to HFR Pond 16	Post-Development 2 YR	2	4.331	12.250	20.25
Area to Pond 1	Post-Development 2 YR	2	0.379	12.100	3.10
BS-10	Post-Development 2 YR	2	0.416	12.050	5.95
BS-11	Post-Development 2 YR	2	0.083	12.000	1.45
BS-12	Post-Development 2 YR	2	0.104	12.100	0.78
BS-13	Post-Development 2 YR	2	0.511	12.100	4.88
BS-14	Post-Development 2 YR	2	0.263	12.100	2.48
BS-15	Post-Development 2 YR	2	0.127	12.050	1.58
BS-16	Post-Development 2 YR	2	0.445	12.150	3.42
BS-17	Post-Development 2 YR	2	0.278	12.100	3.02
BS-18	Post-Development 2 YR	2	0.499	12.100	3.50
BS-19	Post-Development 2 YR	2	0.161	12.050	2.08
BS-1A	Post-Development 2 YR	2	0.047	12.100	0.36
BS-1B	Post-Development 2 YR	2	0.093	12.100	0.43
BS-2	Post-Development 2 YR	2	0.176	12.000	2.90
BS-20	Post-Development 2 YR	2	1.164	12.150	7.43
BS-21	Post-Development 2 YR	2	1.202	12.200	7.78
BS-22	Post-Development 2 YR	2	0.372	12.100	3.72
BS-23	Post-Development 2 YR	2	0.657	12.150	4.49
BS-23A	Post-Development 2 YR	2	0.463	12.050	5.48
BS-24	Post-Development 2 YR	2	0.114	12.100	0.59
BS-25	Post-Development 2 YR	2	0.115	12.150	0.36
BS-26	Post-Development 2 YR	2	0.016	14.250	0.04
BS-27	Post-Development 2 YR	2	0.315	12.100	2.10

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
BS-28	Post-Development 2 YR	2	0.462	12.200	2.21
BS-29	Post-Development 2 YR	2	0.329	12.200	1.43
BS-2A	Post-Development 2 YR	2	0.074	12.000	1.22
BS-2B	Post-Development 2 YR	2	0.083	12.000	1.40
BS-3	Post-Development 2 YR	2	0.084	12.100	0.60
BS-30	Post-Development 2 YR	2	0.091	12.100	0.65
BS-31	Post-Development 2 YR	2	0.082	12.150	0.31
BS-32	Post-Development 2 YR	2	0.061	12.100	0.25
BS-33	Post-Development 2 YR	2	0.116	12.100	0.81
BS-4	Post-Development 2 YR	2	0.222	12.100	1.86
BS-5	Post-Development 2 YR	2	0.152	12.100	1.13
BS-6	Post-Development 2 YR	2	0.111	12.000	1.93
BS-7	Post-Development 2 YR	2	0.268	12.000	4.43
BS-8	Post-Development 2 YR	2	0.093	12.000	1.55
BS-9	Post-Development 2 YR	2	0.139	12.000	2.29
CC-10	Post-Development 2 YR	2	0.839	12.350	2.56
CC-11	Post-Development 2 YR	2	0.197	12.100	0.90
CC-12	Post-Development 2 YR	2	0.165	12.150	0.98
CC-13A	Post-Development 2 YR	2	0.261	12.200	1.39
CC-13B	Post-Development 2 YR	2	0.344	12.200	1.84
CC-13C	Post-Development 2 YR	2	0.134	12.100	0.89
CC-13D	Post-Development 2 YR	2	0.254	12.150	1.54
CC-14	Post-Development 2 YR	2	0.062	12.100	0.43
CC-15	Post-Development 2 YR	2	0.173	12.100	1.08

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
CC-16	Post-Development 2 YR	2	0.220	12.150	1.19
CC-17	Post-Development 2 YR	2	0.337	12.200	1.66
CC-18	Post-Development 2 YR	2	0.100	12.150	0.67
CC-19	Post-Development 2 YR	2	0.050	12.150	0.30
CC-1A	Post-Development 2 YR	2	0.133	12.100	0.84
CC-1B	Post-Development 2 YR	2	0.166	12.150	0.98
CC-20	Post-Development 2 YR	2	0.532	12.150	3.23
CC-21	Post-Development 2 YR	2	0.048	12.300	0.12
CC-22	Post-Development 2 YR	2	0.187	12.150	1.13
CC-23	Post-Development 2 YR	2	0.074	12.200	0.37
CC-24	Post-Development 2 YR	2	0.536	12.150	3.25
CC-25	Post-Development 2 YR	2	0.047	12.100	0.30
CC-26	Post-Development 2 YR	2	0.226	12.150	1.35
CC-27	Post-Development 2 YR	2	0.237	12.200	1.15
CC-28	Post-Development 2 YR	2	1.917	12.500	6.47
CC-2A	Post-Development 2 YR	2	0.149	12.100	0.99
CC-2B	Post-Development 2 YR	2	0.282	12.100	1.87
CC-2C	Post-Development 2 YR	2	0.087	12.100	0.65
CC-3	Post-Development 2 YR	2	0.551	12.350	1.80
CC-4A	Post-Development 2 YR	2	2.341	12.200	15.39
CC-4B	Post-Development 2 YR	2	0.316	12.100	3.95
CC-4C (Pre-Development)	Post-Development 2 YR	2	0.057	12.100	0.15
CC-5	Post-Development 2 YR	2	0.303	12.150	1.81
CC-6	Post-Development 2 YR	2	0.376	12.150	2.28

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
CC-7	Post-Development 2 YR	2	0.249	12.150	1.38
CC-8	Post-Development 2 YR	2	0.104	12.150	0.63
CC-9	Post-Development 2 YR	2	0.076	12.100	0.55
EX-24	Post-Development 2 YR	2	0.086	14.250	0.21
EX-DP-3 (Pre-Dev.)	Post-Development 2 YR	2	0.235	14.300	0.54
OS-10	Post-Development 2 YR	2	0.072	12.050	0.73
OS-11	Post-Development 2 YR	2	0.420	12.200	2.36
OS-12	Post-Development 2 YR	2	0.678	12.300	2.17
OS-13	Post-Development 2 YR	2	0.384	12.250	1.37
OS-14	Post-Development 2 YR	2	0.238	12.250	0.68
OS-15	Post-Development 2 YR	2	0.829	12.250	3.30
OS-16	Post-Development 2 YR	2	0.061	12.100	0.38
OS-17	Post-Development 2 YR	2	0.214	12.100	1.56
OS-18	Post-Development 2 YR	2	0.176	12.100	1.26
OS-1A	Post-Development 2 YR	2	0.060	12.100	0.43
OS-1B	Post-Development 2 YR	2	0.076	12.100	0.52
OS-2	Post-Development 2 YR	2	0.022	12.300	0.05
OS-3	Post-Development 2 YR	2	0.138	12.100	1.01
OS-4	Post-Development 2 YR	2	0.445	12.100	2.84
OS-5	Post-Development 2 YR	2	0.400	12.250	1.87
OS-6	Post-Development 2 YR	2	0.125	12.100	0.86
OS-7	Post-Development 2 YR	2	0.068	12.100	0.51
OS-8	Post-Development 2 YR	2	0.264	12.100	2.12
OS-9	Post-Development 2 YR	2	0.064	14.350	0.13

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
DP-1	Post-Development 2 YR	2	0.143	12.050	1.61
DP-10	Post-Development 2 YR	2	1.530	12.100	11.95
DP-11	Post-Development 2 YR	2	0.445	12.150	3.42
DP-12	Post-Development 2 YR	2	0.572	12.150	4.24
DP-13	Post-Development 2 YR	2	1.520	14.500	3.35
DP-16	Post-Development 2 YR	2	4.050	12.200	24.96
DP-17	Post-Development 2 YR	2	2.080	16.550	3.36
DP-18	Post-Development 2 YR	2	1.058	12.150	5.03
DP-19	Post-Development 2 YR	2	1.520	12.100	3.82
DP-2	Post-Development 2 YR	2	0.365	12.100	3.23
DP-20	Post-Development 2 YR	2	0.810	12.300	2.70
DP-21	Post-Development 2 YR	2	0.532	12.150	2.10
DP-22	Post-Development 2 YR	2	0.814	12.150	3.73
DP-23	Post-Development 2 YR	2	0.789	12.350	2.47
DP-24	Post-Development 2 YR	2	0.360	12.150	1.94
DP-25	Post-Development 2 YR	2	0.313	23.950	0.35
DP-26	Post-Development 2 YR	2	0.943	12.300	2.98
DP-27	Post-Development 2 YR	2	0.705	12.150	4.29
DP-28	Post-Development 2 YR	2	1.090	12.250	4.59
DP-29	Post-Development 2 YR	2	1.552	12.350	5.78
DP-3	Post-Development 2 YR	2	0.572	12.150	1.43
DP-30	Post-Development 2 YR	2	0.100	12.150	0.67
DP-31	Post-Development 2 YR	2	0.150	12.150	0.94
DP-32	Post-Development 2 YR	2	0.398	12.200	1.99

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
DP-33	Post-Development 2 YR	2	0.610	12.150	3.60
DP-34	Post-Development 2 YR	2	2.819	13.250	5.98
DP-4	Post-Development 2 YR	2	0.176	12.000	2.90
DP-5	Post-Development 2 YR	2	0.150	12.050	1.54
DP-6	Post-Development 2 YR	2	0.106	12.100	0.64
DP-7	Post-Development 2 YR	2	0.290	12.100	2.14
DP-8	Post-Development 2 YR	2	5.527	12.350	20.94
DP-9	Post-Development 2 YR	2	0.172	12.100	1.29
J-75	Post-Development 2 YR	2	2.535	12.450	8.58
O-100	Post-Development 2 YR	2	0.197	12.100	0.90
O-101	Post-Development 2 YR	2	0.254	12.150	1.54
O-102	Post-Development 2 YR	2	0.062	12.100	0.43
O-108	Post-Development 2 YR	2	0.235	12.150	1.24
O-110	Post-Development 2 YR	2	0.047	12.100	0.30
O-122	Post-Development 2 YR	2	0.047	12.100	0.36
O-125	Post-Development 2 YR	2	0.165	12.150	0.98
O-126	Post-Development 2 YR	2	2.733	12.200	18.53
O-127	Post-Development 2 YR	2	0.426	12.000	7.22
O-129	Post-Development 2 YR	2	0.400	12.250	1.87
O-137	Post-Development 2 YR	2	0.086	14.250	0.21
O-138	Post-Development 2 YR	2	0.235	14.300	0.54
O-73	Post-Development 2 YR	2	0.114	12.100	0.59
O-74	Post-Development 2 YR	2	0.115	12.150	0.36
O-75	Post-Development 2 YR	2	0.016	14.250	0.04

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
O-80	Post-Development 2 YR	2	0.082	12.150	0.31
O-81	Post-Development 2 YR	2	0.061	12.100	0.25
O-82	Post-Development 2 YR	2	0.116	12.100	0.81
O-86	Post-Development 2 YR	2	0.166	12.150	0.98
O-96	Post-Development 2 YR	2	0.087	12.100	0.65
O-98	Post-Development 2 YR	2	0.249	12.150	1.38

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
Exist. HFR Pond 16 (IN)	Post-Development 2 YR	2	4.331	12.250	20.25	(N/A)	(N/A)
Exist. HFR Pond 16 (OUT)	Post-Development 2 YR	2	4.273	12.400	17.00	7,455.39	0.222
FH North Pond 1 (IN)	Post-Development 2 YR	2	0.379	12.100	3.10	(N/A)	(N/A)
FH North Pond 1 (OUT)	Post-Development 2 YR	2	0.373	13.350	0.62	7,391.48	0.086
FH North Pond 12 (IN)	Post-Development 2 YR	2	0.736	12.150	4.19	(N/A)	(N/A)
FH North Pond 12 (OUT)	Post-Development 2 YR	2	0.313	23.950	0.35	7,546.37	0.421
FH North Pond 4 (IN)	Post-Development 2 YR	2	2.442	12.150	18.30	(N/A)	(N/A)
FH North Pond 4 (OUT)	Post-Development 2 YR	2	1.520	14.500	3.35	7,425.62	1.002
FH North Pond 8 (IN)	Post-Development 2 YR	2	4.509	12.200	27.90	(N/A)	(N/A)

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
FH North Pond 8 (OUT)	Post-Development 2 YR	2	2.080	16.550	3.36	7,374.84	2.484
Golf Course Pond 6 (IN)	Post-Development 2 YR	2	2.366	12.200	15.07	(N/A)	(N/A)
Golf Course Pond 6 (OUT)	Post-Development 2 YR	2	2.363	12.200	14.50	7,436.08	2.505
Golf Course Pond 7 (IN)	Post-Development 2 YR	2	2.896	12.200	17.89	(N/A)	(N/A)
Golf Course Pond 7 (OUT)	Post-Development 2 YR	2	2.896	12.200	17.64	7,424.09	1.477

Time-Depth Curve: TYPE II 24 HOUR	
Label	TYPE II 24 HOUR
Start Time	0.000 hours
Increment	0.250 hours
End Time	24.000 hours
Return Event	2 years

CUMULATIVE RAINFALL (in)
Output Time Increment = 0.250 hours
Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
1.250	0.0	0.0	0.0	0.0	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.750	0.1	0.1	0.1	0.1	0.1
5.000	0.1	0.1	0.2	0.2	0.2
6.250	0.2	0.2	0.2	0.2	0.2
7.500	0.2	0.2	0.3	0.3	0.3
8.750	0.3	0.3	0.3	0.3	0.4
10.000	0.4	0.4	0.4	0.5	0.5
11.250	0.5	0.6	0.8	1.4	1.5
12.500	1.5	1.6	1.6	1.7	1.7
13.750	1.7	1.7	1.8	1.8	1.8
15.000	1.8	1.8	1.8	1.9	1.9
16.250	1.9	1.9	1.9	1.9	1.9
17.500	1.9	1.9	1.9	1.9	2.0
18.750	2.0	2.0	2.0	2.0	2.0
20.000	2.0	2.0	2.0	2.0	2.0
21.250	2.0	2.0	2.0	2.1	2.1
22.500	2.1	2.1	2.1	2.1	2.1
23.750	2.1	2.1	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 1

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,390.00	0.0000	0.004	0.000	0.000	0.000
7,392.00	0.0000	0.242	0.277	0.185	0.185
7,394.00	0.0000	0.311	0.827	0.552	0.736
7,396.00	0.0000	0.387	1.045	0.697	1.433

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 12

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft^2)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,544.50	0.0000	0.002	0.000	0.000	0.000
7,546.00	0.0000	0.462	0.494	0.247	0.247
7,548.00	0.0000	0.600	1.588	1.059	1.306
7,550.00	0.0000	0.749	2.019	1.346	2.652
7,552.00	0.0000	0.905	2.477	1.652	4.304

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 4

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft^2)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,421.50	0.0000	0.004	0.000	0.000	0.000
7,422.00	0.0000	0.004	0.012	0.002	0.002
7,424.00	0.0000	0.248	0.283	0.189	0.191
7,426.00	0.0000	0.981	1.722	1.148	1.339
7,428.00	0.0000	1.226	3.304	2.202	3.542
7,430.00	0.0000	1.432	3.983	2.655	6.197
7,432.00	0.0000	1.651	4.621	3.080	9.277

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 8

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft^2)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,369.00	0.0000	0.009	0.000	0.000	0.000
7,370.00	0.0000	0.009	0.027	0.009	0.009
7,372.00	0.0000	0.415	0.485	0.323	0.332
7,374.00	0.0000	0.918	1.950	1.300	1.633
7,376.00	0.0000	1.411	3.467	2.311	3.944
7,378.00	0.0000	1.594	4.505	3.003	6.947
7,380.00	0.0000	1.788	5.070	3.380	10.327
7,382.00	0.0000	2.032	5.726	3.817	14.145

Subsection: Composite Rating Curve
Label: FH North Pond 1

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,390.00	0.00	(N/A)	0.00
7,390.50	0.21	(N/A)	0.00
7,391.00	0.42	(N/A)	0.00
7,391.50	0.63	(N/A)	0.00
7,392.00	0.84	(N/A)	0.00
7,392.50	1.05	(N/A)	0.00
7,392.75	1.15	(N/A)	0.00
7,393.00	2.70	(N/A)	0.00
7,393.50	9.09	(N/A)	0.00
7,394.00	18.16	(N/A)	0.00
7,394.50	29.05	(N/A)	0.00
7,395.00	41.22	(N/A)	0.00
7,395.50	47.14	(N/A)	0.00
7,396.00	48.76	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 12

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,544.50	0.00	(N/A)	0.00
7,545.00	0.09	(N/A)	0.00
7,545.50	0.19	(N/A)	0.00
7,546.00	0.28	(N/A)	0.00
7,546.50	0.37	(N/A)	0.00
7,546.75	0.42	(N/A)	0.00
7,547.00	1.93	(N/A)	0.00
7,547.50	8.19	(N/A)	0.00
7,548.00	17.12	(N/A)	0.00
7,548.50	27.99	(N/A)	0.00
7,549.00	33.62	(N/A)	0.00
7,549.50	35.86	(N/A)	0.00
7,550.00	37.97	(N/A)	0.00
7,550.50	39.96	(N/A)	0.00
7,551.00	41.87	(N/A)	0.00
7,551.50	43.69	(N/A)	0.00
7,552.00	45.43	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 4

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,421.50	0.00	(N/A)	0.00
7,422.00	0.09	(N/A)	0.00
7,422.50	0.19	(N/A)	0.00
7,423.00	0.29	(N/A)	0.00
7,423.50	0.38	(N/A)	0.00
7,424.00	0.48	(N/A)	0.00
7,424.50	0.57	(N/A)	0.00
7,425.00	0.67	(N/A)	0.00
7,425.50	0.77	(N/A)	0.00
7,426.00	11.42	(N/A)	0.00
7,426.50	30.84	(N/A)	0.00
7,427.00	55.96	(N/A)	0.00
7,427.50	85.67	(N/A)	0.00
7,428.00	119.35	(N/A)	0.00
7,428.50	156.50	(N/A)	0.00
7,429.00	196.62	(N/A)	0.00
7,429.50	205.78	(N/A)	0.00
7,430.00	211.74	(N/A)	0.00
7,430.50	217.53	(N/A)	0.00
7,431.00	223.18	(N/A)	0.00
7,431.50	228.71	(N/A)	0.00
7,432.00	234.08	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 4

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Contributing Structures
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 8

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)	Contributing Structures
7,369.00	0.00	(N/A)	0.00	(no Q: Riser - 1,Orifice - 1,Culvert - 1)
7,369.25	0.00	(N/A)	0.00	(no Q: Riser - 1,Orifice - 1,Culvert - 1)
7,369.50	0.00	(N/A)	0.00	(no Q: Riser - 1,Orifice - 1,Culvert - 1)
7,370.00	0.12	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,370.50	0.23	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,371.00	0.35	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,371.50	0.47	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,372.00	0.58	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,372.50	0.69	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,373.00	0.81	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,373.50	0.92	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,374.00	1.04	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,374.50	1.15	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,374.75	1.21	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,375.00	7.17	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,375.50	32.24	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,376.00	68.01	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,376.50	112.00	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,377.00	162.75	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,377.50	219.40	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,378.00	254.44	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,378.50	266.12	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,379.00	277.33	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,379.50	288.06	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,380.00	298.45	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,380.50	308.47	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,381.00	318.15	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,381.50	327.59	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,382.00	336.74	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 8

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Contributing Structures
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,390.00 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,390.00	0.00	0.000	0.004	0.00	0.00	0.00
7,390.50	0.21	0.007	0.029	0.00	0.21	3.74
7,391.00	0.42	0.033	0.077	0.00	0.42	16.33
7,391.50	0.63	0.088	0.148	0.00	0.63	43.31
7,392.00	0.84	0.185	0.242	0.00	0.84	90.25
7,392.50	1.05	0.310	0.258	0.00	1.05	151.00
7,392.75	1.15	0.375	0.267	0.00	1.15	182.89
7,393.00	2.70	0.443	0.275	0.00	2.70	217.24
7,393.50	9.09	0.585	0.293	0.00	9.09	292.39
7,394.00	18.16	0.736	0.311	0.00	18.16	374.53
7,394.50	29.05	0.896	0.329	0.00	29.05	462.88
7,395.00	41.22	1.066	0.348	0.00	41.22	556.98
7,395.50	47.14	1.244	0.367	0.00	47.14	649.42
7,396.00	48.76	1.433	0.387	0.00	48.76	742.29

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 12

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,544.50 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,544.50	0.00	0.000	0.002	0.00	0.00	0.00
7,545.00	0.09	0.013	0.066	0.00	0.09	6.48
7,545.50	0.19	0.081	0.219	0.00	0.19	39.23
7,546.00	0.28	0.247	0.462	0.00	0.28	119.92
7,546.50	0.37	0.486	0.495	0.00	0.37	235.77
7,546.75	0.42	0.612	0.512	0.00	0.42	296.70
7,547.00	1.93	0.742	0.529	0.00	1.93	361.15
7,547.50	8.19	1.015	0.564	0.00	8.19	499.59
7,548.00	17.12	1.306	0.600	0.00	17.12	649.32
7,548.50	27.99	1.615	0.636	0.00	27.99	809.69
7,549.00	33.62	1.942	0.672	0.00	33.62	973.58
7,549.50	35.86	2.288	0.710	0.00	35.86	1,143.10
7,550.00	37.97	2.652	0.749	0.00	37.97	1,321.75
7,550.50	39.96	3.036	0.787	0.00	39.96	1,509.54
7,551.00	41.87	3.439	0.825	0.00	41.87	1,706.45
7,551.50	43.69	3.862	0.865	0.00	43.69	1,912.72
7,552.00	45.43	4.304	0.905	0.00	45.43	2,128.56

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,421.50 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,421.50	0.00	0.000	0.004	0.00	0.00	0.00
7,422.00	0.09	0.002	0.004	0.00	0.09	1.06
7,422.50	0.19	0.009	0.030	0.00	0.19	4.74
7,423.00	0.29	0.035	0.079	0.00	0.29	17.47
7,423.50	0.38	0.092	0.152	0.00	0.38	44.96
7,424.00	0.48	0.191	0.248	0.00	0.48	92.92
7,424.50	0.57	0.348	0.386	0.00	0.57	169.09
7,425.00	0.67	0.582	0.554	0.00	0.67	282.28
7,425.50	0.77	0.907	0.752	0.00	0.77	439.80
7,426.00	11.42	1.339	0.981	0.00	11.42	659.57
7,426.50	30.84	1.844	1.040	0.00	30.84	923.46
7,427.00	55.96	2.379	1.100	0.00	55.96	1,207.47
7,427.50	85.67	2.945	1.162	0.00	85.67	1,510.87
7,428.00	119.35	3.542	1.226	0.00	119.35	1,833.49
7,428.50	156.50	4.167	1.276	0.00	156.50	2,173.36
7,429.00	196.62	4.818	1.327	0.00	196.62	2,528.43
7,429.50	205.78	5.494	1.379	0.00	205.78	2,864.99
7,430.00	211.74	6.197	1.432	0.00	211.74	3,211.06
7,430.50	217.53	6.926	1.485	0.00	217.53	3,569.83
7,431.00	223.18	7.682	1.540	0.00	223.18	3,941.46
7,431.50	228.71	8.466	1.595	0.00	228.71	4,326.23
7,432.00	234.08	9.277	1.651	0.00	234.08	4,724.32

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 8

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,369.00 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,369.00	0.00	0.000	0.009	0.00	0.00	0.00
7,369.25	0.00	0.002	0.009	0.00	0.00	1.09
7,369.50	0.00	0.004	0.009	0.00	0.00	2.18
7,370.00	0.12	0.009	0.009	0.00	0.12	4.47
7,370.50	0.23	0.023	0.054	0.00	0.23	11.44
7,371.00	0.35	0.069	0.137	0.00	0.35	33.85
7,371.50	0.47	0.166	0.257	0.00	0.47	80.81
7,372.00	0.58	0.332	0.415	0.00	0.58	161.47
7,372.50	0.69	0.566	0.522	0.00	0.69	274.74
7,373.00	0.81	0.857	0.642	0.00	0.81	415.47
7,373.50	0.92	1.210	0.774	0.00	0.92	586.62
7,374.00	1.04	1.633	0.918	0.00	1.04	791.20
7,374.50	1.15	2.120	1.031	0.00	1.15	1,027.05
7,374.75	1.21	2.385	1.091	0.00	1.21	1,155.47
7,375.00	7.17	2.665	1.151	0.00	7.17	1,297.03
7,375.50	32.24	3.272	1.278	0.00	32.24	1,615.90
7,376.00	68.01	3.944	1.411	0.00	68.01	1,976.89
7,376.50	112.00	4.661	1.456	0.00	112.00	2,367.74
7,377.00	162.75	5.400	1.501	0.00	162.75	2,776.25
7,377.50	219.40	6.162	1.547	0.00	219.40	3,201.72
7,378.00	254.44	6.947	1.594	0.00	254.44	3,616.84
7,378.50	266.12	7.756	1.641	0.00	266.12	4,020.00
7,379.00	277.33	8.589	1.690	0.00	277.33	4,434.25
7,379.50	288.06	9.446	1.738	0.00	288.06	4,859.76
7,380.00	298.45	10.327	1.788	0.00	298.45	5,296.84
7,380.50	308.47	11.236	1.848	0.00	308.47	5,746.74
7,381.00	318.15	12.175	1.908	0.00	318.15	6,210.82
7,381.50	327.59	13.144	1.970	0.00	327.59	6,689.43
7,382.00	336.74	14.145	2.032	0.00	336.74	7,182.75

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Project Summary	
Title	Flying Horse North Filing No.1
Engineer	MAW
Company	CCES
Date	4/4/2018

Notes	5 Year

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Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
Area to HFR Pond 16	Post-Development 5 YR	5	9.990	12.200	77.31
Area to Pond 1	Post-Development 5 YR	5	0.813	12.100	9.36
BS-10	Post-Development 5 YR	5	0.610	12.050	8.66
BS-11	Post-Development 5 YR	5	0.122	12.000	2.08
BS-12	Post-Development 5 YR	5	0.240	12.050	3.01
BS-13	Post-Development 5 YR	5	1.054	12.050	12.78
BS-14	Post-Development 5 YR	5	0.545	12.050	6.57
BS-15	Post-Development 5 YR	5	0.250	12.050	3.66
BS-16	Post-Development 5 YR	5	0.912	12.150	9.19
BS-17	Post-Development 5 YR	5	0.553	12.050	7.36
BS-18	Post-Development 5 YR	5	1.121	12.100	12.38
BS-19	Post-Development 5 YR	5	0.312	12.050	4.62
BS-1A	Post-Development 5 YR	5	0.109	12.050	1.39
BS-1B	Post-Development 5 YR	5	0.232	12.050	2.37
BS-2	Post-Development 5 YR	5	0.258	12.000	4.20
BS-20	Post-Development 5 YR	5	2.565	12.150	24.57
BS-21	Post-Development 5 YR	5	2.579	12.150	23.92
BS-22	Post-Development 5 YR	5	0.762	12.050	9.63
BS-23	Post-Development 5 YR	5	1.401	12.150	13.59
BS-23A	Post-Development 5 YR	5	0.874	12.050	11.98
BS-24	Post-Development 5 YR	5	0.285	12.050	3.25
BS-25	Post-Development 5 YR	5	0.301	12.100	2.68
BS-26	Post-Development 5 YR	5	0.048	12.100	0.40
BS-27	Post-Development 5 YR	5	0.727	12.100	7.95

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
BS-28	Post-Development 5 YR	5	1.090	12.150	9.31
BS-29	Post-Development 5 YR	5	0.790	12.150	6.49
BS-2A	Post-Development 5 YR	5	0.109	12.000	1.77
BS-2B	Post-Development 5 YR	5	0.122	12.000	2.02
BS-3	Post-Development 5 YR	5	0.194	12.050	2.25
BS-30	Post-Development 5 YR	5	0.209	12.050	2.43
BS-31	Post-Development 5 YR	5	0.209	12.100	1.94
BS-32	Post-Development 5 YR	5	0.156	12.100	1.55
BS-33	Post-Development 5 YR	5	0.271	12.050	3.18
BS-4	Post-Development 5 YR	5	0.478	12.100	5.54
BS-5	Post-Development 5 YR	5	0.350	12.050	4.38
BS-6	Post-Development 5 YR	5	0.163	12.000	2.76
BS-7	Post-Development 5 YR	5	0.394	12.000	6.40
BS-8	Post-Development 5 YR	5	0.136	12.000	2.24
BS-9	Post-Development 5 YR	5	0.204	12.000	3.31
CC-10	Post-Development 5 YR	5	2.140	12.200	14.13
CC-11	Post-Development 5 YR	5	0.490	12.100	4.95
CC-12	Post-Development 5 YR	5	0.380	12.100	3.88
CC-13A	Post-Development 5 YR	5	0.601	12.150	5.42
CC-13B	Post-Development 5 YR	5	0.794	12.150	7.17
CC-13C	Post-Development 5 YR	5	0.309	12.100	3.38
CC-13D	Post-Development 5 YR	5	0.586	12.100	6.17
CC-14	Post-Development 5 YR	5	0.144	12.100	1.59
CC-15	Post-Development 5 YR	5	0.399	12.100	4.27

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
CC-16	Post-Development 5 YR	5	0.507	12.150	4.64
CC-17	Post-Development 5 YR	5	0.777	12.150	6.47
CC-18	Post-Development 5 YR	5	0.219	12.100	2.15
CC-19	Post-Development 5 YR	5	0.115	12.100	1.21
CC-1A	Post-Development 5 YR	5	0.306	12.100	3.28
CC-1B	Post-Development 5 YR	5	0.386	12.100	4.02
CC-20	Post-Development 5 YR	5	1.225	12.100	12.89
CC-21	Post-Development 5 YR	5	0.133	12.100	1.17
CC-22	Post-Development 5 YR	5	0.430	12.100	4.53
CC-23	Post-Development 5 YR	5	0.173	12.150	1.48
CC-24	Post-Development 5 YR	5	1.235	12.100	12.99
CC-25	Post-Development 5 YR	5	0.109	12.100	1.17
CC-26	Post-Development 5 YR	5	0.521	12.100	5.31
CC-27	Post-Development 5 YR	5	0.558	12.150	4.88
CC-28	Post-Development 5 YR	5	4.534	12.350	24.70
CC-2A	Post-Development 5 YR	5	0.343	12.100	3.76
CC-2B	Post-Development 5 YR	5	0.649	12.100	7.10
CC-2C	Post-Development 5 YR	5	0.200	12.050	2.50
CC-3	Post-Development 5 YR	5	1.375	12.250	8.79
CC-4A	Post-Development 5 YR	5	4.709	12.200	38.97
CC-4B	Post-Development 5 YR	5	0.554	12.100	7.31
CC-4C (Pre-Development)	Post-Development 5 YR	5	0.159	12.050	1.81
CC-5	Post-Development 5 YR	5	0.698	12.100	7.13
CC-6	Post-Development 5 YR	5	0.867	12.100	9.12

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
CC-7	Post-Development 5 YR	5	0.573	12.100	5.35
CC-8	Post-Development 5 YR	5	0.240	12.100	2.53
CC-9	Post-Development 5 YR	5	0.175	12.050	2.08
EX-24	Post-Development 5 YR	5	0.255	12.050	2.17
EX-DP-3 (Pre-Dev.)	Post-Development 5 YR	5	0.693	12.100	4.76
OS-10	Post-Development 5 YR	5	0.154	12.050	2.13
OS-11	Post-Development 5 YR	5	0.938	12.150	8.20
OS-12	Post-Development 5 YR	5	1.720	12.200	11.85
OS-13	Post-Development 5 YR	5	0.961	12.150	7.35
OS-14	Post-Development 5 YR	5	0.624	12.150	4.61
OS-15	Post-Development 5 YR	5	1.997	12.200	14.76
OS-16	Post-Development 5 YR	5	0.140	12.100	1.50
OS-17	Post-Development 5 YR	5	0.493	12.050	5.88
OS-18	Post-Development 5 YR	5	0.406	12.050	4.72
OS-1A	Post-Development 5 YR	5	0.137	12.050	1.60
OS-1B	Post-Development 5 YR	5	0.175	12.100	1.94
OS-2	Post-Development 5 YR	5	0.062	12.100	0.55
OS-3	Post-Development 5 YR	5	0.318	12.050	3.80
OS-4	Post-Development 5 YR	5	1.026	12.100	11.02
OS-5	Post-Development 5 YR	5	0.922	12.200	7.14
OS-6	Post-Development 5 YR	5	0.287	12.100	3.18
OS-7	Post-Development 5 YR	5	0.156	12.050	1.96
OS-8	Post-Development 5 YR	5	0.556	12.100	6.22
OS-9	Post-Development 5 YR	5	0.188	12.200	0.99

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
DP-1	Post-Development 5 YR	5	0.260	12.050	3.37
DP-10	Post-Development 5 YR	5	3.247	12.100	34.85
DP-11	Post-Development 5 YR	5	0.912	12.150	9.19
DP-12	Post-Development 5 YR	5	1.161	12.100	11.23
DP-13	Post-Development 5 YR	5	4.192	12.600	15.02
DP-16	Post-Development 5 YR	5	8.729	12.150	78.46
DP-17	Post-Development 5 YR	5	7.095	12.800	23.12
DP-18	Post-Development 5 YR	5	2.495	12.100	21.62
DP-19	Post-Development 5 YR	5	3.616	12.100	16.76
DP-2	Post-Development 5 YR	5	0.737	12.050	8.76
DP-20	Post-Development 5 YR	5	2.026	12.150	14.26
DP-21	Post-Development 5 YR	5	1.304	12.150	10.47
DP-22	Post-Development 5 YR	5	1.951	12.100	16.60
DP-23	Post-Development 5 YR	5	1.999	12.200	13.04
DP-24	Post-Development 5 YR	5	0.857	12.100	8.41
DP-25	Post-Development 5 YR	5	1.084	14.200	1.85
DP-26	Post-Development 5 YR	5	2.380	12.200	15.93
DP-27	Post-Development 5 YR	5	1.624	12.100	17.16
DP-28	Post-Development 5 YR	5	2.598	12.150	19.79
DP-29	Post-Development 5 YR	5	3.678	12.200	26.59
DP-3	Post-Development 5 YR	5	1.262	12.100	5.77
DP-30	Post-Development 5 YR	5	0.219	12.100	2.15
DP-31	Post-Development 5 YR	5	0.334	12.150	3.22
DP-32	Post-Development 5 YR	5	0.917	12.150	7.77

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
DP-33	Post-Development 5 YR	5	1.407	12.100	14.38
DP-34	Post-Development 5 YR	5	6.776	12.650	23.53
DP-4	Post-Development 5 YR	5	0.258	12.000	4.20
DP-5	Post-Development 5 YR	5	0.283	12.050	3.52
DP-6	Post-Development 5 YR	5	0.256	12.050	2.77
DP-7	Post-Development 5 YR	5	0.668	12.050	8.18
DP-8	Post-Development 5 YR	5	12.227	12.300	70.39
DP-9	Post-Development 5 YR	5	0.397	12.050	4.97
J-75	Post-Development 5 YR	5	5.958	12.300	32.44
O-100	Post-Development 5 YR	5	0.490	12.100	4.95
O-101	Post-Development 5 YR	5	0.586	12.100	6.17
O-102	Post-Development 5 YR	5	0.144	12.100	1.59
O-108	Post-Development 5 YR	5	0.563	12.100	5.70
O-110	Post-Development 5 YR	5	0.109	12.100	1.17
O-122	Post-Development 5 YR	5	0.109	12.050	1.39
O-125	Post-Development 5 YR	5	0.380	12.100	3.88
O-126	Post-Development 5 YR	5	5.438	12.150	44.99
O-127	Post-Development 5 YR	5	0.625	12.000	10.39
O-129	Post-Development 5 YR	5	0.922	12.200	7.14
O-137	Post-Development 5 YR	5	0.255	12.050	2.17
O-138	Post-Development 5 YR	5	0.693	12.100	4.76
O-73	Post-Development 5 YR	5	0.285	12.050	3.25
O-74	Post-Development 5 YR	5	0.301	12.100	2.68
O-75	Post-Development 5 YR	5	0.048	12.100	0.40

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
O-80	Post-Development 5 YR	5	0.209	12.100	1.94
O-81	Post-Development 5 YR	5	0.156	12.100	1.55
O-82	Post-Development 5 YR	5	0.271	12.050	3.18
O-86	Post-Development 5 YR	5	0.386	12.100	4.02
O-96	Post-Development 5 YR	5	0.200	12.050	2.50
O-98	Post-Development 5 YR	5	0.573	12.100	5.35

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
Exist. HFR Pond 16 (IN)	Post-Development 5 YR	5	9.990	12.200	77.31	(N/A)	(N/A)
Exist. HFR Pond 16 (OUT)	Post-Development 5 YR	5	9.910	12.350	60.95	7,456.98	0.761
FH North Pond 1 (IN)	Post-Development 5 YR	5	0.813	12.100	9.36	(N/A)	(N/A)
FH North Pond 1 (OUT)	Post-Development 5 YR	5	0.774	13.700	1.01	7,392.43	0.291
FH North Pond 12 (IN)	Post-Development 5 YR	5	1.723	12.100	17.34	(N/A)	(N/A)
FH North Pond 12 (OUT)	Post-Development 5 YR	5	1.084	14.200	1.85	7,546.99	0.735
FH North Pond 4 (IN)	Post-Development 5 YR	5	5.146	12.100	52.30	(N/A)	(N/A)
FH North Pond 4 (OUT)	Post-Development 5 YR	5	4.192	12.600	15.02	7,426.09	1.431
FH North Pond 8 (IN)	Post-Development 5 YR	5	9.596	12.150	84.97	(N/A)	(N/A)

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
FH North Pond 8 (OUT)	Post-Development 5 YR	5	7.095	12.800	23.12	7,375.32	3.044
Golf Course Pond 6 (IN)	Post-Development 5 YR	5	5.144	12.150	48.48	(N/A)	(N/A)
Golf Course Pond 6 (OUT)	Post-Development 5 YR	5	5.138	12.200	46.40	7,436.24	2.611
Golf Course Pond 7 (IN)	Post-Development 5 YR	5	6.211	12.150	55.75	(N/A)	(N/A)
Golf Course Pond 7 (OUT)	Post-Development 5 YR	5	6.211	12.200	54.75	7,424.29	1.551

Time-Depth Curve: TYPE II 24 HOUR	
Label	TYPE II 24 HOUR
Start Time	0.000 hours
Increment	0.250 hours
End Time	24.000 hours
Return Event	5 years

CUMULATIVE RAINFALL (in)
Output Time Increment = 0.250 hours
Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
1.250	0.0	0.0	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.750	0.1	0.1	0.1	0.2	0.2
5.000	0.2	0.2	0.2	0.2	0.2
6.250	0.2	0.2	0.3	0.3	0.3
7.500	0.3	0.3	0.3	0.3	0.4
8.750	0.4	0.4	0.4	0.4	0.5
10.000	0.5	0.5	0.5	0.6	0.6
11.250	0.7	0.8	1.0	1.8	1.9
12.500	2.0	2.0	2.1	2.1	2.2
13.750	2.2	2.2	2.3	2.3	2.3
15.000	2.3	2.3	2.3	2.4	2.4
16.250	2.4	2.4	2.4	2.4	2.5
17.500	2.5	2.5	2.5	2.5	2.5
18.750	2.5	2.5	2.5	2.6	2.6
20.000	2.6	2.6	2.6	2.6	2.6
21.250	2.6	2.6	2.6	2.6	2.6
22.500	2.7	2.7	2.7	2.7	2.7
23.750	2.7	2.7	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 1

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,390.00	0.0000	0.004	0.000	0.000	0.000
7,392.00	0.0000	0.242	0.277	0.185	0.185
7,394.00	0.0000	0.311	0.827	0.552	0.736
7,396.00	0.0000	0.387	1.045	0.697	1.433

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 12

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft^2)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,544.50	0.0000	0.002	0.000	0.000	0.000
7,546.00	0.0000	0.462	0.494	0.247	0.247
7,548.00	0.0000	0.600	1.588	1.059	1.306
7,550.00	0.0000	0.749	2.019	1.346	2.652
7,552.00	0.0000	0.905	2.477	1.652	4.304

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 4

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft^2)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,421.50	0.0000	0.004	0.000	0.000	0.000
7,422.00	0.0000	0.004	0.012	0.002	0.002
7,424.00	0.0000	0.248	0.283	0.189	0.191
7,426.00	0.0000	0.981	1.722	1.148	1.339
7,428.00	0.0000	1.226	3.304	2.202	3.542
7,430.00	0.0000	1.432	3.983	2.655	6.197
7,432.00	0.0000	1.651	4.621	3.080	9.277

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 8

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,369.00	0.0000	0.009	0.000	0.000	0.000
7,370.00	0.0000	0.009	0.027	0.009	0.009
7,372.00	0.0000	0.415	0.485	0.323	0.332
7,374.00	0.0000	0.918	1.950	1.300	1.633
7,376.00	0.0000	1.411	3.467	2.311	3.944
7,378.00	0.0000	1.594	4.505	3.003	6.947
7,380.00	0.0000	1.788	5.070	3.380	10.327
7,382.00	0.0000	2.032	5.726	3.817	14.145

Subsection: Composite Rating Curve
Label: FH North Pond 1

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,390.00	0.00	(N/A)	0.00
7,390.50	0.21	(N/A)	0.00
7,391.00	0.42	(N/A)	0.00
7,391.50	0.63	(N/A)	0.00
7,392.00	0.84	(N/A)	0.00
7,392.50	1.05	(N/A)	0.00
7,392.75	1.15	(N/A)	0.00
7,393.00	2.70	(N/A)	0.00
7,393.50	9.09	(N/A)	0.00
7,394.00	18.16	(N/A)	0.00
7,394.50	29.05	(N/A)	0.00
7,395.00	41.22	(N/A)	0.00
7,395.50	47.14	(N/A)	0.00
7,396.00	48.76	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 12

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,544.50	0.00	(N/A)	0.00
7,545.00	0.09	(N/A)	0.00
7,545.50	0.19	(N/A)	0.00
7,546.00	0.28	(N/A)	0.00
7,546.50	0.37	(N/A)	0.00
7,546.75	0.42	(N/A)	0.00
7,547.00	1.93	(N/A)	0.00
7,547.50	8.19	(N/A)	0.00
7,548.00	17.12	(N/A)	0.00
7,548.50	27.99	(N/A)	0.00
7,549.00	33.62	(N/A)	0.00
7,549.50	35.86	(N/A)	0.00
7,550.00	37.97	(N/A)	0.00
7,550.50	39.96	(N/A)	0.00
7,551.00	41.87	(N/A)	0.00
7,551.50	43.69	(N/A)	0.00
7,552.00	45.43	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 4

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,421.50	0.00	(N/A)	0.00
7,422.00	0.09	(N/A)	0.00
7,422.50	0.19	(N/A)	0.00
7,423.00	0.29	(N/A)	0.00
7,423.50	0.38	(N/A)	0.00
7,424.00	0.48	(N/A)	0.00
7,424.50	0.57	(N/A)	0.00
7,425.00	0.67	(N/A)	0.00
7,425.50	0.77	(N/A)	0.00
7,426.00	11.42	(N/A)	0.00
7,426.50	30.84	(N/A)	0.00
7,427.00	55.96	(N/A)	0.00
7,427.50	85.67	(N/A)	0.00
7,428.00	119.35	(N/A)	0.00
7,428.50	156.50	(N/A)	0.00
7,429.00	196.62	(N/A)	0.00
7,429.50	205.78	(N/A)	0.00
7,430.00	211.74	(N/A)	0.00
7,430.50	217.53	(N/A)	0.00
7,431.00	223.18	(N/A)	0.00
7,431.50	228.71	(N/A)	0.00
7,432.00	234.08	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 4

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Contributing Structures
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 8

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)	Contributing Structures
7,369.00	0.00	(N/A)	0.00	(no Q: Riser - 1,Orifice - 1,Culvert - 1)
7,369.25	0.00	(N/A)	0.00	(no Q: Riser - 1,Orifice - 1,Culvert - 1)
7,369.50	0.00	(N/A)	0.00	(no Q: Riser - 1,Orifice - 1,Culvert - 1)
7,370.00	0.12	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,370.50	0.23	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,371.00	0.35	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,371.50	0.47	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,372.00	0.58	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,372.50	0.69	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,373.00	0.81	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,373.50	0.92	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,374.00	1.04	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,374.50	1.15	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,374.75	1.21	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,375.00	7.17	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,375.50	32.24	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,376.00	68.01	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,376.50	112.00	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,377.00	162.75	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,377.50	219.40	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,378.00	254.44	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,378.50	266.12	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,379.00	277.33	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,379.50	288.06	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,380.00	298.45	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,380.50	308.47	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,381.00	318.15	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,381.50	327.59	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,382.00	336.74	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 8

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Contributing Structures
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 1

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,390.00 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,390.00	0.00	0.000	0.004	0.00	0.00	0.00
7,390.50	0.21	0.007	0.029	0.00	0.21	3.74
7,391.00	0.42	0.033	0.077	0.00	0.42	16.33
7,391.50	0.63	0.088	0.148	0.00	0.63	43.31
7,392.00	0.84	0.185	0.242	0.00	0.84	90.25
7,392.50	1.05	0.310	0.258	0.00	1.05	151.00
7,392.75	1.15	0.375	0.267	0.00	1.15	182.89
7,393.00	2.70	0.443	0.275	0.00	2.70	217.24
7,393.50	9.09	0.585	0.293	0.00	9.09	292.39
7,394.00	18.16	0.736	0.311	0.00	18.16	374.53
7,394.50	29.05	0.896	0.329	0.00	29.05	462.88
7,395.00	41.22	1.066	0.348	0.00	41.22	556.98
7,395.50	47.14	1.244	0.367	0.00	47.14	649.42
7,396.00	48.76	1.433	0.387	0.00	48.76	742.29

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 12

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,544.50 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,544.50	0.00	0.000	0.002	0.00	0.00	0.00
7,545.00	0.09	0.013	0.066	0.00	0.09	6.48
7,545.50	0.19	0.081	0.219	0.00	0.19	39.23
7,546.00	0.28	0.247	0.462	0.00	0.28	119.92
7,546.50	0.37	0.486	0.495	0.00	0.37	235.77
7,546.75	0.42	0.612	0.512	0.00	0.42	296.70
7,547.00	1.93	0.742	0.529	0.00	1.93	361.15
7,547.50	8.19	1.015	0.564	0.00	8.19	499.59
7,548.00	17.12	1.306	0.600	0.00	17.12	649.32
7,548.50	27.99	1.615	0.636	0.00	27.99	809.69
7,549.00	33.62	1.942	0.672	0.00	33.62	973.58
7,549.50	35.86	2.288	0.710	0.00	35.86	1,143.10
7,550.00	37.97	2.652	0.749	0.00	37.97	1,321.75
7,550.50	39.96	3.036	0.787	0.00	39.96	1,509.54
7,551.00	41.87	3.439	0.825	0.00	41.87	1,706.45
7,551.50	43.69	3.862	0.865	0.00	43.69	1,912.72
7,552.00	45.43	4.304	0.905	0.00	45.43	2,128.56

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 4

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,421.50 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,421.50	0.00	0.000	0.004	0.00	0.00	0.00
7,422.00	0.09	0.002	0.004	0.00	0.09	1.06
7,422.50	0.19	0.009	0.030	0.00	0.19	4.74
7,423.00	0.29	0.035	0.079	0.00	0.29	17.47
7,423.50	0.38	0.092	0.152	0.00	0.38	44.96
7,424.00	0.48	0.191	0.248	0.00	0.48	92.92
7,424.50	0.57	0.348	0.386	0.00	0.57	169.09
7,425.00	0.67	0.582	0.554	0.00	0.67	282.28
7,425.50	0.77	0.907	0.752	0.00	0.77	439.80
7,426.00	11.42	1.339	0.981	0.00	11.42	659.57
7,426.50	30.84	1.844	1.040	0.00	30.84	923.46
7,427.00	55.96	2.379	1.100	0.00	55.96	1,207.47
7,427.50	85.67	2.945	1.162	0.00	85.67	1,510.87
7,428.00	119.35	3.542	1.226	0.00	119.35	1,833.49
7,428.50	156.50	4.167	1.276	0.00	156.50	2,173.36
7,429.00	196.62	4.818	1.327	0.00	196.62	2,528.43
7,429.50	205.78	5.494	1.379	0.00	205.78	2,864.99
7,430.00	211.74	6.197	1.432	0.00	211.74	3,211.06
7,430.50	217.53	6.926	1.485	0.00	217.53	3,569.83
7,431.00	223.18	7.682	1.540	0.00	223.18	3,941.46
7,431.50	228.71	8.466	1.595	0.00	228.71	4,326.23
7,432.00	234.08	9.277	1.651	0.00	234.08	4,724.32

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 8

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,369.00 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,369.00	0.00	0.000	0.009	0.00	0.00	0.00
7,369.25	0.00	0.002	0.009	0.00	0.00	1.09
7,369.50	0.00	0.004	0.009	0.00	0.00	2.18
7,370.00	0.12	0.009	0.009	0.00	0.12	4.47
7,370.50	0.23	0.023	0.054	0.00	0.23	11.44
7,371.00	0.35	0.069	0.137	0.00	0.35	33.85
7,371.50	0.47	0.166	0.257	0.00	0.47	80.81
7,372.00	0.58	0.332	0.415	0.00	0.58	161.47
7,372.50	0.69	0.566	0.522	0.00	0.69	274.74
7,373.00	0.81	0.857	0.642	0.00	0.81	415.47
7,373.50	0.92	1.210	0.774	0.00	0.92	586.62
7,374.00	1.04	1.633	0.918	0.00	1.04	791.20
7,374.50	1.15	2.120	1.031	0.00	1.15	1,027.05
7,374.75	1.21	2.385	1.091	0.00	1.21	1,155.47
7,375.00	7.17	2.665	1.151	0.00	7.17	1,297.03
7,375.50	32.24	3.272	1.278	0.00	32.24	1,615.90
7,376.00	68.01	3.944	1.411	0.00	68.01	1,976.89
7,376.50	112.00	4.661	1.456	0.00	112.00	2,367.74
7,377.00	162.75	5.400	1.501	0.00	162.75	2,776.25
7,377.50	219.40	6.162	1.547	0.00	219.40	3,201.72
7,378.00	254.44	6.947	1.594	0.00	254.44	3,616.84
7,378.50	266.12	7.756	1.641	0.00	266.12	4,020.00
7,379.00	277.33	8.589	1.690	0.00	277.33	4,434.25
7,379.50	288.06	9.446	1.738	0.00	288.06	4,859.76
7,380.00	298.45	10.327	1.788	0.00	298.45	5,296.84
7,380.50	308.47	11.236	1.848	0.00	308.47	5,746.74
7,381.00	318.15	12.175	1.908	0.00	318.15	6,210.82
7,381.50	327.59	13.144	1.970	0.00	327.59	6,689.43
7,382.00	336.74	14.145	2.032	0.00	336.74	7,182.75

Index

Project Summary	
Title	Flying Horse North Filing No.1
Engineer	MAW
Company	CCES
Date	4/4/2018

Notes	100 Year

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Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
Area to HFR Pond 16	Post-Development 100 YR	100	37.106	12.150	400.62
Area to Pond 1	Post-Development 100 YR	100	2.793	12.050	38.16
BS-10	Post-Development 100 YR	100	1.269	12.050	17.46
BS-11	Post-Development 100 YR	100	0.254	12.000	4.09
BS-12	Post-Development 100 YR	100	0.892	12.050	13.83
BS-13	Post-Development 100 YR	100	3.477	12.050	49.97
BS-14	Post-Development 100 YR	100	1.807	12.050	25.95
BS-15	Post-Development 100 YR	100	0.781	12.050	12.22
BS-16	Post-Development 100 YR	100	2.975	12.100	35.70
BS-17	Post-Development 100 YR	100	1.747	12.050	25.96
BS-18	Post-Development 100 YR	100	4.050	12.050	56.04
BS-19	Post-Development 100 YR	100	0.955	12.050	14.95
BS-1A	Post-Development 100 YR	100	0.405	12.050	6.29
BS-1B	Post-Development 100 YR	100	0.933	12.050	13.83
BS-2	Post-Development 100 YR	100	0.536	12.000	8.36
BS-20	Post-Development 100 YR	100	9.086	12.100	112.39
BS-21	Post-Development 100 YR	100	8.877	12.100	103.01
BS-22	Post-Development 100 YR	100	2.489	12.050	36.51
BS-23	Post-Development 100 YR	100	4.789	12.100	58.24
BS-23A	Post-Development 100 YR	100	2.590	12.050	38.30
BS-24	Post-Development 100 YR	100	1.143	12.050	17.59
BS-25	Post-Development 100 YR	100	1.263	12.050	17.27
BS-26	Post-Development 100 YR	100	0.223	12.050	3.35
BS-27	Post-Development 100 YR	100	2.696	12.050	38.80

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
BS-28	Post-Development 100 YR	100	4.139	12.100	49.40
BS-29	Post-Development 100 YR	100	3.050	12.100	35.94
BS-2A	Post-Development 100 YR	100	0.226	12.000	3.52
BS-2B	Post-Development 100 YR	100	0.254	12.000	4.00
BS-3	Post-Development 100 YR	100	0.718	12.050	10.78
BS-30	Post-Development 100 YR	100	0.776	12.050	11.65
BS-31	Post-Development 100 YR	100	0.858	12.050	11.80
BS-32	Post-Development 100 YR	100	0.637	12.050	9.41
BS-33	Post-Development 100 YR	100	1.016	12.050	15.33
BS-4	Post-Development 100 YR	100	1.652	12.050	23.58
BS-5	Post-Development 100 YR	100	1.298	12.050	20.12
BS-6	Post-Development 100 YR	100	0.339	12.000	5.44
BS-7	Post-Development 100 YR	100	0.818	12.000	12.76
BS-8	Post-Development 100 YR	100	0.282	12.000	4.45
BS-9	Post-Development 100 YR	100	0.423	12.000	6.60
CC-10	Post-Development 100 YR	100	8.756	12.150	91.86
CC-11	Post-Development 100 YR	100	1.960	12.050	28.14
CC-12	Post-Development 100 YR	100	1.411	12.100	18.67
CC-13A	Post-Development 100 YR	100	2.230	12.100	27.30
CC-13B	Post-Development 100 YR	100	2.947	12.100	36.07
CC-13C	Post-Development 100 YR	100	1.146	12.050	16.49
CC-13D	Post-Development 100 YR	100	2.175	12.050	29.20
CC-14	Post-Development 100 YR	100	0.532	12.050	7.76
CC-15	Post-Development 100 YR	100	1.481	12.050	20.37

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
CC-16	Post-Development 100 YR	100	1.884	12.100	23.58
CC-17	Post-Development 100 YR	100	2.885	12.100	32.77
CC-18	Post-Development 100 YR	100	0.769	12.100	9.73
CC-19	Post-Development 100 YR	100	0.428	12.050	5.75
CC-1A	Post-Development 100 YR	100	1.134	12.050	15.97
CC-1B	Post-Development 100 YR	100	1.444	12.050	19.35
CC-20	Post-Development 100 YR	100	4.547	12.050	61.04
CC-21	Post-Development 100 YR	100	0.585	12.050	8.51
CC-22	Post-Development 100 YR	100	1.597	12.050	21.44
CC-23	Post-Development 100 YR	100	0.650	12.100	7.71
CC-24	Post-Development 100 YR	100	4.581	12.050	61.51
CC-25	Post-Development 100 YR	100	0.405	12.050	5.70
CC-26	Post-Development 100 YR	100	1.932	12.100	25.56
CC-27	Post-Development 100 YR	100	2.121	12.100	25.82
CC-28	Post-Development 100 YR	100	17.270	12.300	136.30
CC-2A	Post-Development 100 YR	100	1.273	12.050	18.32
CC-2B	Post-Development 100 YR	100	2.407	12.050	34.64
CC-2C	Post-Development 100 YR	100	0.742	12.050	11.50
CC-3	Post-Development 100 YR	100	5.511	12.150	54.50
CC-4A	Post-Development 100 YR	100	15.367	12.150	155.93
CC-4B	Post-Development 100 YR	100	1.493	12.050	20.60
CC-4C (Pre-Development)	Post-Development 100 YR	100	0.699	12.000	11.16
CC-5	Post-Development 100 YR	100	2.591	12.100	34.28
CC-6	Post-Development 100 YR	100	3.216	12.050	43.18

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
CC-7	Post-Development 100 YR	100	2.127	12.100	27.04
CC-8	Post-Development 100 YR	100	0.891	12.050	11.96
CC-9	Post-Development 100 YR	100	0.648	12.050	9.80
EX-24	Post-Development 100 YR	100	1.178	12.050	17.75
EX-DP-3 (Pre-Dev.)	Post-Development 100 YR	100	3.210	12.100	41.28
OS-10	Post-Development 100 YR	100	0.528	12.050	8.23
OS-11	Post-Development 100 YR	100	3.372	12.100	38.66
OS-12	Post-Development 100 YR	100	7.008	12.150	75.81
OS-13	Post-Development 100 YR	100	3.863	12.100	44.96
OS-14	Post-Development 100 YR	100	2.622	12.100	31.03
OS-15	Post-Development 100 YR	100	7.742	12.150	84.16
OS-16	Post-Development 100 YR	100	0.521	12.050	7.16
OS-17	Post-Development 100 YR	100	1.829	12.050	27.65
OS-18	Post-Development 100 YR	100	1.506	12.050	22.60
OS-1A	Post-Development 100 YR	100	0.510	12.050	7.65
OS-1B	Post-Development 100 YR	100	0.648	12.050	9.44
OS-2	Post-Development 100 YR	100	0.274	12.050	3.98
OS-3	Post-Development 100 YR	100	1.181	12.050	17.85
OS-4	Post-Development 100 YR	100	3.807	12.050	53.61
OS-5	Post-Development 100 YR	100	3.426	12.150	36.99
OS-6	Post-Development 100 YR	100	1.065	12.050	15.51
OS-7	Post-Development 100 YR	100	0.579	12.050	8.98
OS-8	Post-Development 100 YR	100	1.872	12.100	24.73
OS-9	Post-Development 100 YR	100	0.871	12.150	9.05

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
DP-1	Post-Development 100 YR	100	0.764	12.050	11.13
DP-10	Post-Development 100 YR	100	11.054	12.050	143.30
DP-11	Post-Development 100 YR	100	2.975	12.100	35.70
DP-12	Post-Development 100 YR	100	3.754	12.100	44.11
DP-13	Post-Development 100 YR	100	16.350	12.250	139.15
DP-16	Post-Development 100 YR	100	30.220	12.100	361.52
DP-17	Post-Development 100 YR	100	30.083	12.300	255.78
DP-18	Post-Development 100 YR	100	9.471	12.100	115.49
DP-19	Post-Development 100 YR	100	13.817	12.100	125.58
DP-2	Post-Development 100 YR	100	2.416	12.050	34.63
DP-20	Post-Development 100 YR	100	8.142	12.100	88.44
DP-21	Post-Development 100 YR	100	5.136	12.100	61.96
DP-22	Post-Development 100 YR	100	7.540	12.100	92.42
DP-23	Post-Development 100 YR	100	8.133	12.150	84.40
DP-24	Post-Development 100 YR	100	3.290	12.050	44.82
DP-25	Post-Development 100 YR	100	5.764	12.300	32.88
DP-26	Post-Development 100 YR	100	9.647	12.150	101.89
DP-27	Post-Development 100 YR	100	6.028	12.050	81.42
DP-28	Post-Development 100 YR	100	9.972	12.150	110.17
DP-29	Post-Development 100 YR	100	14.023	12.150	154.97
DP-3	Post-Development 100 YR	100	4.441	12.100	38.98
DP-30	Post-Development 100 YR	100	0.769	12.100	9.73
DP-31	Post-Development 100 YR	100	1.197	12.100	15.22
DP-32	Post-Development 100 YR	100	3.406	12.100	39.76

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
DP-33	Post-Development 100 YR	100	5.231	12.100	69.08
DP-34	Post-Development 100 YR	100	26.136	12.400	167.92
DP-4	Post-Development 100 YR	100	0.536	12.000	8.36
DP-5	Post-Development 100 YR	100	0.874	12.050	12.59
DP-6	Post-Development 100 YR	100	0.992	12.050	14.76
DP-7	Post-Development 100 YR	100	2.479	12.050	37.97
DP-8	Post-Development 100 YR	100	43.909	12.350	284.14
DP-9	Post-Development 100 YR	100	1.472	12.050	22.81
J-75	Post-Development 100 YR	100	22.558	12.250	180.87
O-100	Post-Development 100 YR	100	1.960	12.050	28.14
O-101	Post-Development 100 YR	100	2.175	12.050	29.20
O-102	Post-Development 100 YR	100	0.532	12.050	7.76
O-108	Post-Development 100 YR	100	2.182	12.050	29.95
O-110	Post-Development 100 YR	100	0.405	12.050	5.70
O-122	Post-Development 100 YR	100	0.405	12.050	6.29
O-125	Post-Development 100 YR	100	1.411	12.100	18.67
O-126	Post-Development 100 YR	100	17.508	12.150	178.97
O-127	Post-Development 100 YR	100	1.298	12.000	20.57
O-129	Post-Development 100 YR	100	3.426	12.150	36.99
O-137	Post-Development 100 YR	100	1.178	12.050	17.75
O-138	Post-Development 100 YR	100	3.210	12.100	41.28
O-73	Post-Development 100 YR	100	1.143	12.050	17.59
O-74	Post-Development 100 YR	100	1.263	12.050	17.27
O-75	Post-Development 100 YR	100	0.223	12.050	3.35

Subsection: Master Network Summary

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
O-80	Post-Development 100 YR	100	0.858	12.050	11.80
O-81	Post-Development 100 YR	100	0.637	12.050	9.41
O-82	Post-Development 100 YR	100	1.016	12.050	15.33
O-86	Post-Development 100 YR	100	1.444	12.050	19.35
O-96	Post-Development 100 YR	100	0.742	12.050	11.50
O-98	Post-Development 100 YR	100	2.127	12.100	27.04

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
Exist. HFR Pond 16 (IN)	Post-Development 100 YR	100	37.106	12.150	400.62	(N/A)	(N/A)
Exist. HFR Pond 16 (OUT)	Post-Development 100 YR	100	36.950	12.350	258.25	7,462.42	6.804
FH North Pond 1 (IN)	Post-Development 100 YR	100	2.793	12.050	38.16	(N/A)	(N/A)
FH North Pond 1 (OUT)	Post-Development 100 YR	100	2.517	12.250	20.41	7,394.10	0.769
FH North Pond 12 (IN)	Post-Development 100 YR	100	6.505	12.100	86.17	(N/A)	(N/A)
FH North Pond 12 (OUT)	Post-Development 100 YR	100	5.764	12.300	32.88	7,548.93	1.898
FH North Pond 4 (IN)	Post-Development 100 YR	100	17.420	12.100	217.23	(N/A)	(N/A)
FH North Pond 4 (OUT)	Post-Development 100 YR	100	16.350	12.250	139.15	7,428.27	3.872
FH North Pond 8 (IN)	Post-Development 100 YR	100	32.794	12.150	383.43	(N/A)	(N/A)

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
FH North Pond 8 (OUT)	Post-Development 100 YR	100	30.083	12.300	255.78	7,378.06	7.039
Golf Course Pond 6 (IN)	Post-Development 100 YR	100	17.962	12.100	215.40	(N/A)	(N/A)
Golf Course Pond 6 (OUT)	Post-Development 100 YR	100	17.947	12.150	212.28	7,436.83	3.002
Golf Course Pond 7 (IN)	Post-Development 100 YR	100	21.391	12.100	253.17	(N/A)	(N/A)
Golf Course Pond 7 (OUT)	Post-Development 100 YR	100	21.391	12.150	250.27	7,424.94	1.819

Time-Depth Curve: TYPE II 24 HOUR	
Label	TYPE II 24 HOUR
Start Time	0.000 hours
Increment	0.250 hours
End Time	24.000 hours
Return Event	100 years

CUMULATIVE RAINFALL (in)
Output Time Increment = 0.250 hours
Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.1
1.250	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.2	0.2	0.2
3.750	0.2	0.2	0.2	0.3	0.3
5.000	0.3	0.3	0.3	0.3	0.4
6.250	0.4	0.4	0.4	0.5	0.5
7.500	0.5	0.5	0.6	0.6	0.6
8.750	0.6	0.7	0.7	0.7	0.8
10.000	0.8	0.9	0.9	1.0	1.1
11.250	1.2	1.3	1.8	3.0	3.3
12.500	3.4	3.5	3.6	3.6	3.7
13.750	3.7	3.8	3.8	3.9	3.9
15.000	3.9	4.0	4.0	4.0	4.1
16.250	4.1	4.1	4.1	4.2	4.2
17.500	4.2	4.2	4.2	4.3	4.3
18.750	4.3	4.3	4.3	4.4	4.4
20.000	4.4	4.4	4.4	4.4	4.4
21.250	4.5	4.5	4.5	4.5	4.5
22.500	4.5	4.5	4.5	4.6	4.6
23.750	4.6	4.6	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 1

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,390.00	0.0000	0.004	0.000	0.000	0.000
7,392.00	0.0000	0.242	0.277	0.185	0.185
7,394.00	0.0000	0.311	0.827	0.552	0.736
7,396.00	0.0000	0.387	1.045	0.697	1.433

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 12

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,544.50	0.0000	0.002	0.000	0.000	0.000
7,546.00	0.0000	0.462	0.494	0.247	0.247
7,548.00	0.0000	0.600	1.588	1.059	1.306
7,550.00	0.0000	0.749	2.019	1.346	2.652
7,552.00	0.0000	0.905	2.477	1.652	4.304

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 4

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft^2)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,421.50	0.0000	0.004	0.000	0.000	0.000
7,422.00	0.0000	0.004	0.012	0.002	0.002
7,424.00	0.0000	0.248	0.283	0.189	0.191
7,426.00	0.0000	0.981	1.722	1.148	1.339
7,428.00	0.0000	1.226	3.304	2.202	3.542
7,430.00	0.0000	1.432	3.983	2.655	6.197
7,432.00	0.0000	1.651	4.621	3.080	9.277

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 8

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,369.00	0.0000	0.009	0.000	0.000	0.000
7,370.00	0.0000	0.009	0.027	0.009	0.009
7,372.00	0.0000	0.415	0.485	0.323	0.332
7,374.00	0.0000	0.918	1.950	1.300	1.633
7,376.00	0.0000	1.411	3.467	2.311	3.944
7,378.00	0.0000	1.594	4.505	3.003	6.947
7,380.00	0.0000	1.788	5.070	3.380	10.327
7,382.00	0.0000	2.032	5.726	3.817	14.145

Subsection: Composite Rating Curve
Label: FH North Pond 1

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,390.00	0.00	(N/A)	0.00
7,390.50	0.21	(N/A)	0.00
7,391.00	0.42	(N/A)	0.00
7,391.50	0.63	(N/A)	0.00
7,392.00	0.84	(N/A)	0.00
7,392.50	1.05	(N/A)	0.00
7,392.75	1.15	(N/A)	0.00
7,393.00	2.70	(N/A)	0.00
7,393.50	9.09	(N/A)	0.00
7,394.00	18.16	(N/A)	0.00
7,394.50	29.05	(N/A)	0.00
7,395.00	41.22	(N/A)	0.00
7,395.50	47.14	(N/A)	0.00
7,396.00	48.76	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 12

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,544.50	0.00	(N/A)	0.00
7,545.00	0.09	(N/A)	0.00
7,545.50	0.19	(N/A)	0.00
7,546.00	0.28	(N/A)	0.00
7,546.50	0.37	(N/A)	0.00
7,546.75	0.42	(N/A)	0.00
7,547.00	1.93	(N/A)	0.00
7,547.50	8.19	(N/A)	0.00
7,548.00	17.12	(N/A)	0.00
7,548.50	27.99	(N/A)	0.00
7,549.00	33.62	(N/A)	0.00
7,549.50	35.86	(N/A)	0.00
7,550.00	37.97	(N/A)	0.00
7,550.50	39.96	(N/A)	0.00
7,551.00	41.87	(N/A)	0.00
7,551.50	43.69	(N/A)	0.00
7,552.00	45.43	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 4

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,421.50	0.00	(N/A)	0.00
7,422.00	0.09	(N/A)	0.00
7,422.50	0.19	(N/A)	0.00
7,423.00	0.29	(N/A)	0.00
7,423.50	0.38	(N/A)	0.00
7,424.00	0.48	(N/A)	0.00
7,424.50	0.57	(N/A)	0.00
7,425.00	0.67	(N/A)	0.00
7,425.50	0.77	(N/A)	0.00
7,426.00	11.42	(N/A)	0.00
7,426.50	30.84	(N/A)	0.00
7,427.00	55.96	(N/A)	0.00
7,427.50	85.67	(N/A)	0.00
7,428.00	119.35	(N/A)	0.00
7,428.50	156.50	(N/A)	0.00
7,429.00	196.62	(N/A)	0.00
7,429.50	205.78	(N/A)	0.00
7,430.00	211.74	(N/A)	0.00
7,430.50	217.53	(N/A)	0.00
7,431.00	223.18	(N/A)	0.00
7,431.50	228.71	(N/A)	0.00
7,432.00	234.08	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 4

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Contributing Structures
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 8

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)	Contributing Structures
7,369.00	0.00	(N/A)	0.00	(no Q: Riser - 1,Orifice - 1,Culvert - 1)
7,369.25	0.00	(N/A)	0.00	(no Q: Riser - 1,Orifice - 1,Culvert - 1)
7,369.50	0.00	(N/A)	0.00	(no Q: Riser - 1,Orifice - 1,Culvert - 1)
7,370.00	0.12	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,370.50	0.23	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,371.00	0.35	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,371.50	0.47	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,372.00	0.58	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,372.50	0.69	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,373.00	0.81	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,373.50	0.92	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,374.00	1.04	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,374.50	1.15	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,374.75	1.21	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,375.00	7.17	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,375.50	32.24	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,376.00	68.01	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,376.50	112.00	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,377.00	162.75	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,377.50	219.40	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,378.00	254.44	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,378.50	266.12	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,379.00	277.33	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,379.50	288.06	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,380.00	298.45	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,380.50	308.47	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,381.00	318.15	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,381.50	327.59	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)
7,382.00	336.74	(N/A)	0.00	Orifice - 1,Culvert - 1 (no Q: Riser - 1)

Subsection: Composite Rating Curve
Label: FH North Pond 8

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Contributing Structures
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 1

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,390.00 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,390.00	0.00	0.000	0.004	0.00	0.00	0.00
7,390.50	0.21	0.007	0.029	0.00	0.21	3.74
7,391.00	0.42	0.033	0.077	0.00	0.42	16.33
7,391.50	0.63	0.088	0.148	0.00	0.63	43.31
7,392.00	0.84	0.185	0.242	0.00	0.84	90.25
7,392.50	1.05	0.310	0.258	0.00	1.05	151.00
7,392.75	1.15	0.375	0.267	0.00	1.15	182.89
7,393.00	2.70	0.443	0.275	0.00	2.70	217.24
7,393.50	9.09	0.585	0.293	0.00	9.09	292.39
7,394.00	18.16	0.736	0.311	0.00	18.16	374.53
7,394.50	29.05	0.896	0.329	0.00	29.05	462.88
7,395.00	41.22	1.066	0.348	0.00	41.22	556.98
7,395.50	47.14	1.244	0.367	0.00	47.14	649.42
7,396.00	48.76	1.433	0.387	0.00	48.76	742.29

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 12

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,544.50 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,544.50	0.00	0.000	0.002	0.00	0.00	0.00
7,545.00	0.09	0.013	0.066	0.00	0.09	6.48
7,545.50	0.19	0.081	0.219	0.00	0.19	39.23
7,546.00	0.28	0.247	0.462	0.00	0.28	119.92
7,546.50	0.37	0.486	0.495	0.00	0.37	235.77
7,546.75	0.42	0.612	0.512	0.00	0.42	296.70
7,547.00	1.93	0.742	0.529	0.00	1.93	361.15
7,547.50	8.19	1.015	0.564	0.00	8.19	499.59
7,548.00	17.12	1.306	0.600	0.00	17.12	649.32
7,548.50	27.99	1.615	0.636	0.00	27.99	809.69
7,549.00	33.62	1.942	0.672	0.00	33.62	973.58
7,549.50	35.86	2.288	0.710	0.00	35.86	1,143.10
7,550.00	37.97	2.652	0.749	0.00	37.97	1,321.75
7,550.50	39.96	3.036	0.787	0.00	39.96	1,509.54
7,551.00	41.87	3.439	0.825	0.00	41.87	1,706.45
7,551.50	43.69	3.862	0.865	0.00	43.69	1,912.72
7,552.00	45.43	4.304	0.905	0.00	45.43	2,128.56

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 4

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,421.50 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,421.50	0.00	0.000	0.004	0.00	0.00	0.00
7,422.00	0.09	0.002	0.004	0.00	0.09	1.06
7,422.50	0.19	0.009	0.030	0.00	0.19	4.74
7,423.00	0.29	0.035	0.079	0.00	0.29	17.47
7,423.50	0.38	0.092	0.152	0.00	0.38	44.96
7,424.00	0.48	0.191	0.248	0.00	0.48	92.92
7,424.50	0.57	0.348	0.386	0.00	0.57	169.09
7,425.00	0.67	0.582	0.554	0.00	0.67	282.28
7,425.50	0.77	0.907	0.752	0.00	0.77	439.80
7,426.00	11.42	1.339	0.981	0.00	11.42	659.57
7,426.50	30.84	1.844	1.040	0.00	30.84	923.46
7,427.00	55.96	2.379	1.100	0.00	55.96	1,207.47
7,427.50	85.67	2.945	1.162	0.00	85.67	1,510.87
7,428.00	119.35	3.542	1.226	0.00	119.35	1,833.49
7,428.50	156.50	4.167	1.276	0.00	156.50	2,173.36
7,429.00	196.62	4.818	1.327	0.00	196.62	2,528.43
7,429.50	205.78	5.494	1.379	0.00	205.78	2,864.99
7,430.00	211.74	6.197	1.432	0.00	211.74	3,211.06
7,430.50	217.53	6.926	1.485	0.00	217.53	3,569.83
7,431.00	223.18	7.682	1.540	0.00	223.18	3,941.46
7,431.50	228.71	8.466	1.595	0.00	228.71	4,326.23
7,432.00	234.08	9.277	1.651	0.00	234.08	4,724.32

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 8

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,369.00 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,369.00	0.00	0.000	0.009	0.00	0.00	0.00
7,369.25	0.00	0.002	0.009	0.00	0.00	1.09
7,369.50	0.00	0.004	0.009	0.00	0.00	2.18
7,370.00	0.12	0.009	0.009	0.00	0.12	4.47
7,370.50	0.23	0.023	0.054	0.00	0.23	11.44
7,371.00	0.35	0.069	0.137	0.00	0.35	33.85
7,371.50	0.47	0.166	0.257	0.00	0.47	80.81
7,372.00	0.58	0.332	0.415	0.00	0.58	161.47
7,372.50	0.69	0.566	0.522	0.00	0.69	274.74
7,373.00	0.81	0.857	0.642	0.00	0.81	415.47
7,373.50	0.92	1.210	0.774	0.00	0.92	586.62
7,374.00	1.04	1.633	0.918	0.00	1.04	791.20
7,374.50	1.15	2.120	1.031	0.00	1.15	1,027.05
7,374.75	1.21	2.385	1.091	0.00	1.21	1,155.47
7,375.00	7.17	2.665	1.151	0.00	7.17	1,297.03
7,375.50	32.24	3.272	1.278	0.00	32.24	1,615.90
7,376.00	68.01	3.944	1.411	0.00	68.01	1,976.89
7,376.50	112.00	4.661	1.456	0.00	112.00	2,367.74
7,377.00	162.75	5.400	1.501	0.00	162.75	2,776.25
7,377.50	219.40	6.162	1.547	0.00	219.40	3,201.72
7,378.00	254.44	6.947	1.594	0.00	254.44	3,616.84
7,378.50	266.12	7.756	1.641	0.00	266.12	4,020.00
7,379.00	277.33	8.589	1.690	0.00	277.33	4,434.25
7,379.50	288.06	9.446	1.738	0.00	288.06	4,859.76
7,380.00	298.45	10.327	1.788	0.00	298.45	5,296.84
7,380.50	308.47	11.236	1.848	0.00	308.47	5,746.74
7,381.00	318.15	12.175	1.908	0.00	318.15	6,210.82
7,381.50	327.59	13.144	1.970	0.00	327.59	6,689.43
7,382.00	336.74	14.145	2.032	0.00	336.74	7,182.75

Index

Project Summary	
Title	Flying Horse North Filing No.1
Engineer	MAW
Company	CCES
Date	11/20/2017

Notes	2 Year (Filing 1 Only)

Table of Contents

Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
BS-18	Post-Development 2 YR (Filing 1 Only)	2	0.494	12.100	3.42
BS-19	Post-Development 2 YR (Filing 1 Only)	2	0.161	12.050	2.08
BS-20	Post-Development 2 YR (Filing 1 Only)	2	0.532	14.350	1.14
BS-21	Post-Development 2 YR (Filing 1 Only)	2	0.840	12.300	3.20
BS-22	Post-Development 2 YR (Filing 1 Only)	2	0.207	12.100	1.05
BS-23	Post-Development 2 YR (Filing 1 Only)	2	0.626	12.200	4.08
BS-23A	Post-Development 2 YR (Filing 1 Only)	2	0.174	12.250	0.64

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
DP-16	Post-Development 2 YR (Filing 1 Only)	2	2.856	12.150	11.61
DP-17	Post-Development 2 YR (Filing 1 Only)	2	0.989	24.000	1.13

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
FH North Pond 8 (IN)	Post-Development 2 YR (Filing 1 Only)	2	3.026	12.200	12.12	(N/A)	(N/A)
FH North Pond 8 (OUT)	Post-Development 2 YR (Filing 1 Only)	2	0.989	24.000	1.13	7,374.42	2.034
Golf Course Pond 6 (IN)	Post-Development 2 YR (Filing 1 Only)	2	1.372	12.350	4.24	(N/A)	(N/A)
Golf Course Pond 6 (OUT)	Post-Development 2 YR (Filing 1 Only)	2	1.370	12.400	4.18	7,436.02	2.471

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
Golf Course Pond 7 (IN)	Post-Development 2 YR (Filing 1 Only)	2	1.738	12.350	5.39	(N/A)	(N/A)
Golf Course Pond 7 (OUT)	Post-Development 2 YR (Filing 1 Only)	2	1.738	12.350	5.35	7,424.03	1.453

Time-Depth Curve: TYPE II 24 HOUR	
Label	TYPE II 24 HOUR
Start Time	0.000 hours
Increment	0.250 hours
End Time	24.000 hours
Return Event	2 years

CUMULATIVE RAINFALL (in)
Output Time Increment = 0.250 hours
Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
1.250	0.0	0.0	0.0	0.0	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.750	0.1	0.1	0.1	0.1	0.1
5.000	0.1	0.1	0.2	0.2	0.2
6.250	0.2	0.2	0.2	0.2	0.2
7.500	0.2	0.2	0.3	0.3	0.3
8.750	0.3	0.3	0.3	0.3	0.4
10.000	0.4	0.4	0.4	0.5	0.5
11.250	0.5	0.6	0.8	1.4	1.5
12.500	1.5	1.6	1.6	1.7	1.7
13.750	1.7	1.7	1.8	1.8	1.8
15.000	1.8	1.8	1.8	1.9	1.9
16.250	1.9	1.9	1.9	1.9	1.9
17.500	1.9	1.9	1.9	1.9	2.0
18.750	2.0	2.0	2.0	2.0	2.0
20.000	2.0	2.0	2.0	2.0	2.0
21.250	2.0	2.0	2.0	2.1	2.1
22.500	2.1	2.1	2.1	2.1	2.1
23.750	2.1	2.1	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 8

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft^2)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,369.50	0.000	0.009	0.000	0.000	0.000
7,370.00	0.000	0.009	0.027	0.004	0.004
7,372.00	0.000	0.415	0.485	0.323	0.328
7,374.00	0.000	0.918	1.950	1.300	1.628
7,376.00	0.000	1.411	3.467	2.311	3.939
7,378.00	0.000	1.594	4.505	3.003	6.943
7,380.00	0.000	1.788	5.070	3.380	10.323
7,382.00	0.000	2.032	5.726	3.817	14.140

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,369.50	0.00	(N/A)	0.00
7,370.00	0.12	(N/A)	0.00
7,370.50	0.23	(N/A)	0.00
7,371.00	0.35	(N/A)	0.00
7,371.50	0.47	(N/A)	0.00
7,372.00	0.58	(N/A)	0.00
7,372.50	0.69	(N/A)	0.00
7,373.00	0.81	(N/A)	0.00
7,373.50	0.92	(N/A)	0.00
7,374.00	1.04	(N/A)	0.00
7,374.50	1.15	(N/A)	0.00
7,374.75	1.21	(N/A)	0.00
7,375.00	7.17	(N/A)	0.00
7,375.50	32.24	(N/A)	0.00
7,376.00	68.01	(N/A)	0.00
7,376.50	112.00	(N/A)	0.00
7,377.00	162.75	(N/A)	0.00
7,377.50	219.40	(N/A)	0.00
7,378.00	254.44	(N/A)	0.00
7,378.50	266.12	(N/A)	0.00
7,379.00	277.33	(N/A)	0.00
7,379.50	288.06	(N/A)	0.00
7,380.00	298.45	(N/A)	0.00
7,380.50	308.47	(N/A)	0.00
7,381.00	318.15	(N/A)	0.00
7,381.50	327.59	(N/A)	0.00
7,382.00	336.74	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1

Subsection: Composite Rating Curve
Label: FH North Pond 8

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Contributing Structures
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 8

Return Event: 2 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,369.50 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,369.50	0.00	0.000	0.009	0.00	0.00	0.00
7,370.00	0.12	0.004	0.009	0.00	0.12	2.30
7,370.50	0.23	0.019	0.054	0.00	0.23	9.26
7,371.00	0.35	0.065	0.137	0.00	0.35	31.67
7,371.50	0.47	0.162	0.257	0.00	0.47	78.63
7,372.00	0.58	0.328	0.415	0.00	0.58	159.29
7,372.50	0.69	0.562	0.522	0.00	0.69	272.56
7,373.00	0.81	0.852	0.642	0.00	0.81	413.29
7,373.50	0.92	1.206	0.774	0.00	0.92	584.45
7,374.00	1.04	1.628	0.918	0.00	1.04	789.02
7,374.50	1.15	2.115	1.031	0.00	1.15	1,024.87
7,374.75	1.21	2.380	1.091	0.00	1.21	1,153.29
7,375.00	7.17	2.661	1.151	0.00	7.17	1,294.86
7,375.50	32.24	3.268	1.278	0.00	32.24	1,613.72
7,376.00	68.01	3.939	1.411	0.00	68.01	1,974.71
7,376.50	112.00	4.656	1.456	0.00	112.00	2,365.56
7,377.00	162.75	5.395	1.501	0.00	162.75	2,774.07
7,377.50	219.40	6.157	1.547	0.00	219.40	3,199.55
7,378.00	254.44	6.943	1.594	0.00	254.44	3,614.66
7,378.50	266.12	7.751	1.641	0.00	266.12	4,017.82
7,379.00	277.33	8.584	1.690	0.00	277.33	4,432.07
7,379.50	288.06	9.441	1.738	0.00	288.06	4,857.58
7,380.00	298.45	10.323	1.788	0.00	298.45	5,294.67
7,380.50	308.47	11.232	1.848	0.00	308.47	5,744.56
7,381.00	318.15	12.170	1.908	0.00	318.15	6,208.65
7,381.50	327.59	13.140	1.970	0.00	327.59	6,687.26
7,382.00	336.74	14.140	2.032	0.00	336.74	7,180.58

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Project Summary	
Title	Flying Horse North Filing No.1
Engineer	MAW
Company	CCES
Date	11/20/2017

Notes	5 Year (Filing 1 Only)

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Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
BS-18	Post-Development 5 YR (Filing 1 Only)	5	1.112	12.100	12.24
BS-19	Post-Development 5 YR (Filing 1 Only)	5	0.312	12.050	4.62
BS-20	Post-Development 5 YR (Filing 1 Only)	5	1.513	12.150	9.35
BS-21	Post-Development 5 YR (Filing 1 Only)	5	2.002	12.250	13.46
BS-22	Post-Development 5 YR (Filing 1 Only)	5	0.502	12.100	5.15
BS-23	Post-Development 5 YR (Filing 1 Only)	5	1.353	12.150	12.91
BS-23A	Post-Development 5 YR (Filing 1 Only)	5	0.432	12.150	3.28

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)
DP-16	Post-Development 5 YR (Filing 1 Only)	5	6.787	12.150	47.28
DP-17	Post-Development 5 YR (Filing 1 Only)	5	4.742	13.600	10.89

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
FH North Pond 8 (IN)	Post-Development 5 YR (Filing 1 Only)	5	7.213	12.200	49.73	(N/A)	(N/A)
FH North Pond 8 (OUT)	Post-Development 5 YR (Filing 1 Only)	5	4.742	13.600	10.89	7,375.07	2.747
Golf Course Pond 6 (IN)	Post-Development 5 YR (Filing 1 Only)	5	3.515	12.200	22.52	(N/A)	(N/A)
Golf Course Pond 6 (OUT)	Post-Development 5 YR (Filing 1 Only)	5	3.510	12.250	21.92	7,436.11	2.529

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
Golf Course Pond 7 (IN)	Post-Development 5 YR (Filing 1 Only)	5	4.324	12.200	26.09	(N/A)	(N/A)
Golf Course Pond 7 (OUT)	Post-Development 5 YR (Filing 1 Only)	5	4.325	12.250	26.02	7,424.14	1.493

Time-Depth Curve: TYPE II 24 HOUR	
Label	TYPE II 24 HOUR
Start Time	0.000 hours
Increment	0.250 hours
End Time	24.000 hours
Return Event	5 years

CUMULATIVE RAINFALL (in)
Output Time Increment = 0.250 hours
Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.0
1.250	0.0	0.0	0.1	0.1	0.1
2.500	0.1	0.1	0.1	0.1	0.1
3.750	0.1	0.1	0.1	0.2	0.2
5.000	0.2	0.2	0.2	0.2	0.2
6.250	0.2	0.2	0.3	0.3	0.3
7.500	0.3	0.3	0.3	0.3	0.4
8.750	0.4	0.4	0.4	0.4	0.5
10.000	0.5	0.5	0.5	0.6	0.6
11.250	0.7	0.8	1.0	1.8	1.9
12.500	2.0	2.0	2.1	2.1	2.2
13.750	2.2	2.2	2.3	2.3	2.3
15.000	2.3	2.3	2.3	2.4	2.4
16.250	2.4	2.4	2.4	2.4	2.5
17.500	2.5	2.5	2.5	2.5	2.5
18.750	2.5	2.5	2.5	2.6	2.6
20.000	2.6	2.6	2.6	2.6	2.6
21.250	2.6	2.6	2.6	2.6	2.6
22.500	2.7	2.7	2.7	2.7	2.7
23.750	2.7	2.7	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 8

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft^2)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,369.50	0.000	0.009	0.000	0.000	0.000
7,370.00	0.000	0.009	0.027	0.004	0.004
7,372.00	0.000	0.415	0.485	0.323	0.328
7,374.00	0.000	0.918	1.950	1.300	1.628
7,376.00	0.000	1.411	3.467	2.311	3.939
7,378.00	0.000	1.594	4.505	3.003	6.943
7,380.00	0.000	1.788	5.070	3.380	10.323
7,382.00	0.000	2.032	5.726	3.817	14.140

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft^3/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,369.50	0.00	(N/A)	0.00
7,370.00	0.12	(N/A)	0.00
7,370.50	0.23	(N/A)	0.00
7,371.00	0.35	(N/A)	0.00
7,371.50	0.47	(N/A)	0.00
7,372.00	0.58	(N/A)	0.00
7,372.50	0.69	(N/A)	0.00
7,373.00	0.81	(N/A)	0.00
7,373.50	0.92	(N/A)	0.00
7,374.00	1.04	(N/A)	0.00
7,374.50	1.15	(N/A)	0.00
7,374.75	1.21	(N/A)	0.00
7,375.00	7.17	(N/A)	0.00
7,375.50	32.24	(N/A)	0.00
7,376.00	68.01	(N/A)	0.00
7,376.50	112.00	(N/A)	0.00
7,377.00	162.75	(N/A)	0.00
7,377.50	219.40	(N/A)	0.00
7,378.00	254.44	(N/A)	0.00
7,378.50	266.12	(N/A)	0.00
7,379.00	277.33	(N/A)	0.00
7,379.50	288.06	(N/A)	0.00
7,380.00	298.45	(N/A)	0.00
7,380.50	308.47	(N/A)	0.00
7,381.00	318.15	(N/A)	0.00
7,381.50	327.59	(N/A)	0.00
7,382.00	336.74	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1

Subsection: Composite Rating Curve
Label: FH North Pond 8

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Contributing Structures
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 8

Return Event: 5 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,369.50 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,369.50	0.00	0.000	0.009	0.00	0.00	0.00
7,370.00	0.12	0.004	0.009	0.00	0.12	2.30
7,370.50	0.23	0.019	0.054	0.00	0.23	9.26
7,371.00	0.35	0.065	0.137	0.00	0.35	31.67
7,371.50	0.47	0.162	0.257	0.00	0.47	78.63
7,372.00	0.58	0.328	0.415	0.00	0.58	159.29
7,372.50	0.69	0.562	0.522	0.00	0.69	272.56
7,373.00	0.81	0.852	0.642	0.00	0.81	413.29
7,373.50	0.92	1.206	0.774	0.00	0.92	584.45
7,374.00	1.04	1.628	0.918	0.00	1.04	789.02
7,374.50	1.15	2.115	1.031	0.00	1.15	1,024.87
7,374.75	1.21	2.380	1.091	0.00	1.21	1,153.29
7,375.00	7.17	2.661	1.151	0.00	7.17	1,294.86
7,375.50	32.24	3.268	1.278	0.00	32.24	1,613.72
7,376.00	68.01	3.939	1.411	0.00	68.01	1,974.71
7,376.50	112.00	4.656	1.456	0.00	112.00	2,365.56
7,377.00	162.75	5.395	1.501	0.00	162.75	2,774.07
7,377.50	219.40	6.157	1.547	0.00	219.40	3,199.55
7,378.00	254.44	6.943	1.594	0.00	254.44	3,614.66
7,378.50	266.12	7.751	1.641	0.00	266.12	4,017.82
7,379.00	277.33	8.584	1.690	0.00	277.33	4,432.07
7,379.50	288.06	9.441	1.738	0.00	288.06	4,857.58
7,380.00	298.45	10.323	1.788	0.00	298.45	5,294.67
7,380.50	308.47	11.232	1.848	0.00	308.47	5,744.56
7,381.00	318.15	12.170	1.908	0.00	318.15	6,208.65
7,381.50	327.59	13.140	1.970	0.00	327.59	6,687.26
7,382.00	336.74	14.140	2.032	0.00	336.74	7,180.58

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Project Summary	
Title	Flying Horse North Filing No.1
Engineer	MAW
Company	CCES
Date	11/20/2017

Notes	100 Year (Filing 1 Only)

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Subsection: Master Network Summary

Catchments Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft³/s)
BS-18	Post-Development 100 YR (Filing 1 Only)	100	4.033	12.050	55.75
BS-19	Post-Development 100 YR (Filing 1 Only)	100	0.955	12.050	14.95
BS-20	Post-Development 100 YR (Filing 1 Only)	100	6.797	12.100	76.94
BS-21	Post-Development 100 YR (Filing 1 Only)	100	7.687	12.200	75.52
BS-22	Post-Development 100 YR (Filing 1 Only)	100	1.962	12.050	27.95
BS-23	Post-Development 100 YR (Filing 1 Only)	100	4.695	12.100	56.90
BS-23A	Post-Development 100 YR (Filing 1 Only)	100	1.722	12.100	19.56

Node Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft³/s)
DP-16	Post-Development 100 YR (Filing 1 Only)	100	26.105	12.100	278.35
DP-17	Post-Development 100 YR (Filing 1 Only)	100	25.124	12.350	197.59

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft³/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
FH North Pond 8 (IN)	Post-Development 100 YR (Filing 1 Only)	100	27.812	12.150	297.19	(N/A)	(N/A)
FH North Pond 8 (OUT)	Post-Development 100 YR (Filing 1 Only)	100	25.124	12.350	197.59	7,377.31	5.861

Subsection: Master Network Summary

Pond Summary

Label	Scenario	Return Event (years)	Hydrograph Volume (ac-ft)	Time to Peak (hours)	Peak Flow (ft^3/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ac-ft)
Golf Course Pond 6 (IN)	Post-Development 100 YR (Filing 1 Only)	100	14.484	12.150	149.77	(N/A)	(N/A)
Golf Course Pond 6 (OUT)	Post-Development 100 YR (Filing 1 Only)	100	14.470	12.150	147.51	7,436.65	2.876
Golf Course Pond 7 (IN)	Post-Development 100 YR (Filing 1 Only)	100	17.387	12.150	176.39	(N/A)	(N/A)
Golf Course Pond 7 (OUT)	Post-Development 100 YR (Filing 1 Only)	100	17.388	12.150	177.48	7,424.73	1.731

Time-Depth Curve: TYPE II 24 HOUR	
Label	TYPE II 24 HOUR
Start Time	0.000 hours
Increment	0.250 hours
End Time	24.000 hours
Return Event	100 years

CUMULATIVE RAINFALL (in)
Output Time Increment = 0.250 hours
Time on left represents time for first value in each row.

Time (hours)	Depth (in)	Depth (in)	Depth (in)	Depth (in)	Depth (in)
0.000	0.0	0.0	0.0	0.0	0.1
1.250	0.1	0.1	0.1	0.1	0.1
2.500	0.1	0.1	0.2	0.2	0.2
3.750	0.2	0.2	0.2	0.3	0.3
5.000	0.3	0.3	0.3	0.3	0.4
6.250	0.4	0.4	0.4	0.5	0.5
7.500	0.5	0.5	0.6	0.6	0.6
8.750	0.6	0.7	0.7	0.7	0.8
10.000	0.8	0.9	0.9	1.0	1.1
11.250	1.2	1.3	1.8	3.0	3.3
12.500	3.4	3.5	3.6	3.6	3.7
13.750	3.7	3.8	3.8	3.9	3.9
15.000	3.9	4.0	4.0	4.0	4.1
16.250	4.1	4.1	4.1	4.2	4.2
17.500	4.2	4.2	4.2	4.3	4.3
18.750	4.3	4.3	4.3	4.4	4.4
20.000	4.4	4.4	4.4	4.4	4.4
21.250	4.5	4.5	4.5	4.5	4.5
22.500	4.5	4.5	4.5	4.6	4.6
23.750	4.6	4.6	(N/A)	(N/A)	(N/A)

Subsection: Elevation-Area Volume Curve
Label: FH North Pond 8

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Elevation (ft)	Planimeter (ft²)	Area (acres)	A1+A2+sqr (A1*A2) (acres)	Volume (ac-ft)	Volume (Total) (ac-ft)
7,369.50	0.000	0.009	0.000	0.000	0.000
7,370.00	0.000	0.009	0.027	0.004	0.004
7,372.00	0.000	0.415	0.485	0.323	0.328
7,374.00	0.000	0.918	1.950	1.300	1.628
7,376.00	0.000	1.411	3.467	2.311	3.939
7,378.00	0.000	1.594	4.505	3.003	6.943
7,380.00	0.000	1.788	5.070	3.380	10.323
7,382.00	0.000	2.032	5.726	3.817	14.140

Subsection: Composite Rating Curve | Return Event: 100 years
Label: FH North Pond 8 | Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Water Surface Elevation (ft)	Flow (ft³/s)	Tailwater Elevation (ft)	Convergence Error (ft)
7,369.50	0.00	(N/A)	0.00
7,370.00	0.12	(N/A)	0.00
7,370.50	0.23	(N/A)	0.00
7,371.00	0.35	(N/A)	0.00
7,371.50	0.47	(N/A)	0.00
7,372.00	0.58	(N/A)	0.00
7,372.50	0.69	(N/A)	0.00
7,373.00	0.81	(N/A)	0.00
7,373.50	0.92	(N/A)	0.00
7,374.00	1.04	(N/A)	0.00
7,374.50	1.15	(N/A)	0.00
7,374.75	1.21	(N/A)	0.00
7,375.00	7.17	(N/A)	0.00
7,375.50	32.24	(N/A)	0.00
7,376.00	68.01	(N/A)	0.00
7,376.50	112.00	(N/A)	0.00
7,377.00	162.75	(N/A)	0.00
7,377.50	219.40	(N/A)	0.00
7,378.00	254.44	(N/A)	0.00
7,378.50	266.12	(N/A)	0.00
7,379.00	277.33	(N/A)	0.00
7,379.50	288.06	(N/A)	0.00
7,380.00	298.45	(N/A)	0.00
7,380.50	308.47	(N/A)	0.00
7,381.00	318.15	(N/A)	0.00
7,381.50	327.59	(N/A)	0.00
7,382.00	336.74	(N/A)	0.00

Contributing Structures
(no Q: Riser - 1,Orifice - 1,Culvert - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Orifice - 1,Culvert - 1 (no Q: Riser - 1)
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1

Subsection: Composite Rating Curve
Label: FH North Pond 8

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Composite Outflow Summary

Contributing Structures
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Orifice - 1,Culvert - 1
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)
Riser - 1,Culvert - 1 (no Q: Orifice - 1)

Subsection: Elevation-Volume-Flow Table (Pond)
Label: FH North Pond 8

Return Event: 100 years
Storm Event: TYPE II 24 HOUR

Infiltration	
Infiltration Method (Computed)	No Infiltration

Initial Conditions	
Elevation (Water Surface, Initial)	7,369.50 ft
Volume (Initial)	0.000 ac-ft
Flow (Initial Outlet)	0.00 ft³/s
Flow (Initial Infiltration)	0.00 ft³/s
Flow (Initial, Total)	0.00 ft³/s
Time Increment	0.050 hours

Elevation (ft)	Outflow (ft³/s)	Storage (ac-ft)	Area (acres)	Infiltration (ft³/s)	Flow (Total) (ft³/s)	2S/t + O (ft³/s)
7,369.50	0.00	0.000	0.009	0.00	0.00	0.00
7,370.00	0.12	0.004	0.009	0.00	0.12	2.30
7,370.50	0.23	0.019	0.054	0.00	0.23	9.26
7,371.00	0.35	0.065	0.137	0.00	0.35	31.67
7,371.50	0.47	0.162	0.257	0.00	0.47	78.63
7,372.00	0.58	0.328	0.415	0.00	0.58	159.29
7,372.50	0.69	0.562	0.522	0.00	0.69	272.56
7,373.00	0.81	0.852	0.642	0.00	0.81	413.29
7,373.50	0.92	1.206	0.774	0.00	0.92	584.45
7,374.00	1.04	1.628	0.918	0.00	1.04	789.02
7,374.50	1.15	2.115	1.031	0.00	1.15	1,024.87
7,374.75	1.21	2.380	1.091	0.00	1.21	1,153.29
7,375.00	7.17	2.661	1.151	0.00	7.17	1,294.86
7,375.50	32.24	3.268	1.278	0.00	32.24	1,613.72
7,376.00	68.01	3.939	1.411	0.00	68.01	1,974.71
7,376.50	112.00	4.656	1.456	0.00	112.00	2,365.56
7,377.00	162.75	5.395	1.501	0.00	162.75	2,774.07
7,377.50	219.40	6.157	1.547	0.00	219.40	3,199.55
7,378.00	254.44	6.943	1.594	0.00	254.44	3,614.66
7,378.50	266.12	7.751	1.641	0.00	266.12	4,017.82
7,379.00	277.33	8.584	1.690	0.00	277.33	4,432.07
7,379.50	288.06	9.441	1.738	0.00	288.06	4,857.58
7,380.00	298.45	10.323	1.788	0.00	298.45	5,294.67
7,380.50	308.47	11.232	1.848	0.00	308.47	5,744.56
7,381.00	318.15	12.170	1.908	0.00	318.15	6,208.65
7,381.50	327.59	13.140	1.970	0.00	327.59	6,687.26
7,382.00	336.74	14.140	2.032	0.00	336.74	7,180.58

Index

DRAINAGE MAPS

CLASSIC SM
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DESIGN POINTS SURFACE ROUTING SUMMARY - EXISTING CONDITIONS

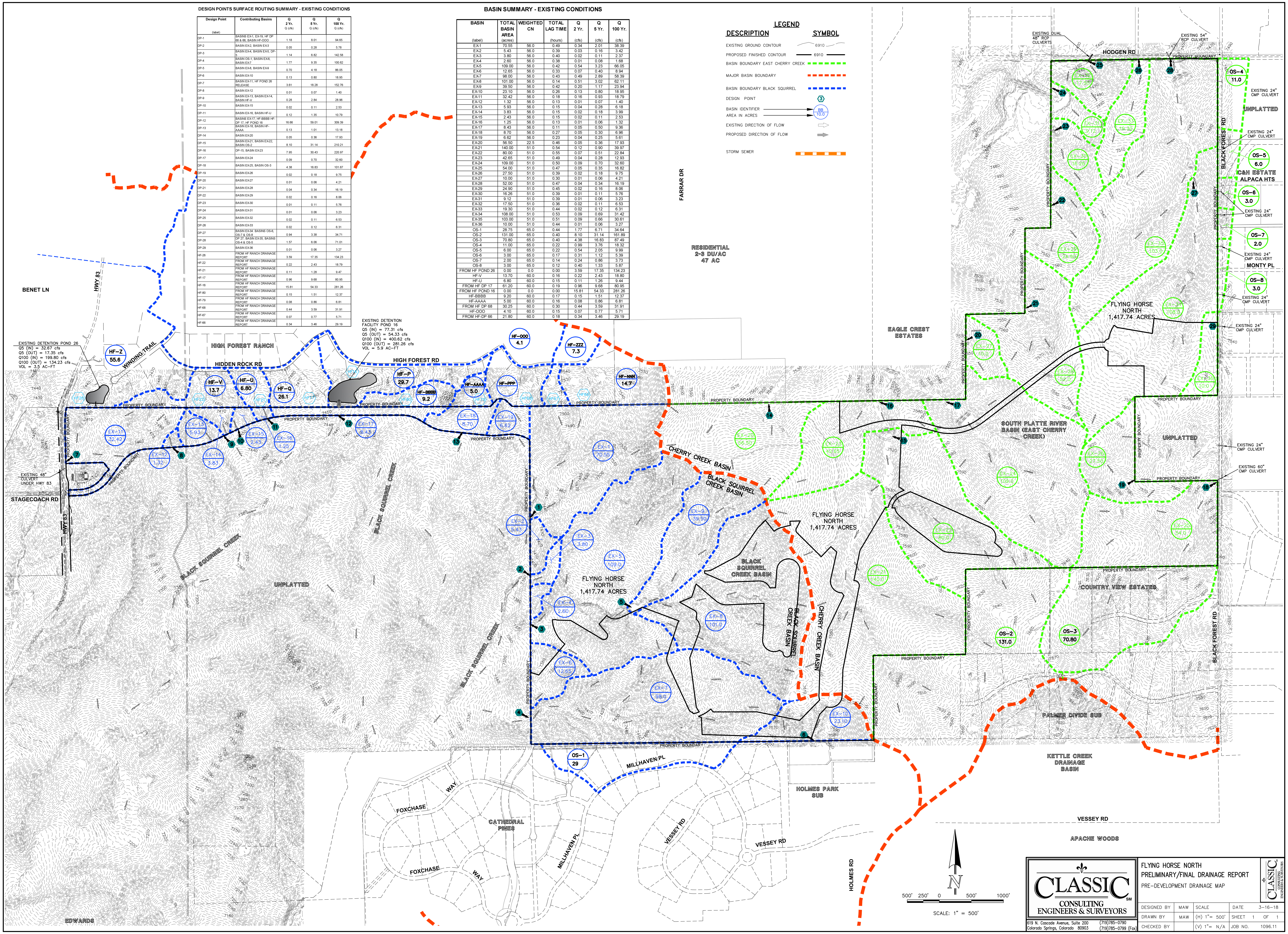
Design Point (label)	Contributing Basins	Q 2 Yr. Q (cfs)	Q 5 Yr. Q (cfs)	Q 100 Yr. Q (cfs)
DP-1	BASINS EX-1, EX-19, HF DP 68 & 66, BASIN HF-OOO	1.18	8.01	94.65
DP-2	BASIN EX-2, BASIN EX-3	0.05	0.28	5.78
DP-3	BASIN EX-4, BASIN EX-5, DP-5	1.14	6.82	142.58
DP-4	BASIN OS-1, BASIN EX-6, BASIN EX-7	1.77	9.35	100.62
DP-5	BASIN EX-8, BASIN EX-9	0.70	4.18	86.05
DP-6	BASIN EX-10	0.13	0.80	18.95
DP-7	BASIN EX-11, HF POND 26 RELEASE	3.61	18.28	152.76
DP-8	BASIN EX-12	0.01	0.07	1.40
DP-9	BASIN EX-13, BASIN EX-14, BASIN HF-V	0.28	2.84	28.96
DP-10	BASIN EX-15	0.02	0.11	2.53
DP-11	BASIN EX-16, BASIN HF-U	0.12	1.35	10.79
DP-12	BASINS EX-17, HF-BBBB HF-DP 17, HF POND 16	16.66	59.01	309.39
DP-13	BASIN EX-18, BASIN HF-AAAA	0.13	1.01	13.18
DP-14	BASIN EX-20	0.05	0.36	17.93
DP-15	BASIN EX-21, BASIN EX-22, BASIN OS-2	8.10	31.14	210.21
DP-16	DP-15, BASIN EX-23	7.95	30.43	220.97
DP-17	BASIN EX-24	0.09	0.70	32.60
DP-18	BASIN EX-25, BASIN OS-3	4.38	16.83	101.67
DP-19	BASIN EX-26	0.02	0.18	9.75
DP-20	BASIN EX-27	0.01	0.06	4.21
DP-21	BASIN EX-28	0.04	0.34	16.19
DP-22	BASIN EX-29	0.02	0.16	8.06
DP-23	BASIN EX-30	0.01	0.11	5.76
DP-24	BASIN EX-31	0.01	0.06	3.23
DP-25	BASIN EX-32	0.02	0.11	6.53
DP-26	BASIN EX-33	0.02	0.12	6.31
DP-27	BASIN EX-34. BASINS OS-6, OS-7 & OS-8	0.94	3.38	34.71
DP-28	DP 27, BASIN EX-35, BASINS OS-4 & OS-5	1.57	6.06	71.01
DP-29	BASIN EX-36	0.01	0.06	3.27
HF-26	FROM HF RANCH DRAINAGE REPORT	3.59	17.35	134.23
HF-22	FROM HF RANCH DRAINAGE REPORT	0.22	2.43	18.79
HF-21	FROM HF RANCH DRAINAGE REPORT	0.11	1.28	9.47
HF-17	FROM HF RANCH DRAINAGE REPORT	0.96	9.68	80.95
HF-16	FROM HF RANCH DRAINAGE REPORT	15.81	54.33	281.26
HF-80	FROM HF RANCH DRAINAGE REPORT	0.15	1.51	12.37
HF-79	FROM HF RANCH DRAINAGE REPORT	0.08	0.86	6.81
HF-68	FROM HF RANCH DRAINAGE REPORT	0.44	3.59	31.91
HF-67	FROM HF RANCH DRAINAGE REPORT	0.07	0.77	5.71
HF-66	FROM HF RANCH DRAINAGE REPORT	0.34	3.46	29.19

BASIN SUMMARY - EXISTING CONDITIONS

BASIN (label)	TOTAL BASIN AREA (acres)	WEIGHTED CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
EX-1	70.55	56.0	0.49	0.34	2.01	38.39
EX-2	5.43	56.0	0.39	0.03	0.16	3.42
EX-3	3.80	56.0	0.40	0.02	0.11	2.37
EX-4	2.60	56.0	0.38	0.01	0.08	1.68
EX-5	109.00	56.0	0.42	0.54	3.23	66.05
EX-6	12.65	56.0	0.33	0.07	0.40	8.94
EX-7	98.00	56.0	0.43	0.49	2.89	58.39
EX-8	101.00	56.0	0.14	0.51	3.02	62.11
EX-9	39.50	56.0	0.42	0.20	1.17	23.94
EX-10	23.10	56.0	0.26	0.13	0.80	18.95
EX-11	32.42	56.0	0.18	0.16	0.93	18.79
EX-12	1.32	56.0	0.13	0.01	0.07	1.40
EX-13	5.93	56.0	0.15	0.04	0.28	6.18
EX-14	3.83	56.0	0.15	0.02	0.18	3.99
EX-15	2.43	56.0	0.15	0.02	0.11	2.53
EX-16	1.25	56.0	0.13	0.01	0.06	1.32
EX-17	8.43	56.0	0.11	0.05	0.50	9.36
EX-18	8.70	56.0	0.27	0.05	0.30	6.96
EX-19	6.62	56.0	0.23	0.04	0.25	5.61
EX-20	56.50	22.5	0.46	0.05	0.36	17.93
EX-21	140.00	51.0	0.54	0.12	0.90	39.97
EX-22	80.00	51.0	0.55	0.07	0.51	22.84
EX-23	42.65	51.0	0.49	0.04	0.28	12.93
EX-24	109.00	51.0	0.50	0.09	0.70	32.60
EX-25	54.00	51.0	0.47	0.05	0.35	16.82
EX-26	27.50	51.0	0.39	0.02	0.18	9.75
EX-27	10.00	51.0	0.30	0.01	0.06	4.21
EX-28	52.00	51.0	0.47	0.04	0.34	16.19
EX-29	24.90	51.0	0.45	0.02	0.16	8.06
EX-30	16.26	51.0	0.39	0.01	0.11	5.76
EX-31	9.12	51.0	0.39	0.01	0.06	3.23
EX-32	17.50	51.0	0.36	0.02	0.11	6.53
EX-33	19.30	51.0	0.44	0.02	0.12	6.31
EX-34	108.00	51.0	0.53	0.09	0.69	31.42
EX-35	103.00	51.0	0.51	0.09	0.66	30.61
EX-36	10.00	51.0	0.44	0.01	0.06	3.27
OS-1	28.75	65.0	0.44	1.77	6.71	34.64
OS-2	131.00	65.0	0.40	8.10	31.14	161.89
OS-3	70.80	65.0	0.40	4.38	16.83	87.49
OS-4	11.00	65.0	0.22	0.99	3.76	18.32
OS-5	6.00	65.0	0.22	0.54	2.05	9.99
OS-6	3.00	65.0	0.17	0.31	1.12	5.39
OS-7	2.00	65.0	0.14	0.24	0.86	3.73
OS-8	3.00	65.0	0.12	0.40	1.33	5.87
FROM HF POND 26	0.00	0.0	0.00	3.59	17.35	134.23
HF-V	13.70	60.0	0.16	0.22	2.43	18.80
HF-U	6.80	60.0	0.15	0.11	1.26	9.44
FROM HF DP 17	61.20	60.0	0.19	0.96	9.68	80.95
FROM HF POND 16	0.00	0.0	0.00	15.81	54.33	281.26
HF-BBBB	9.20	60.0	0.17	0.15	1.51	12.37
HF-AAAA	5.00	60.0	0.16	0.08	0.86	6.81
FROM HF DP 68	30.25	60.0	0.30	0.44	3.59	31.91
HF-OOO	4.10	60.0	0.15	0.07	0.77	5.71
FROM HF-DP 66	21.80	60.0	0.18	0.34	3.46	29.19

LEGEND

DESCRIPTION	SYMBOL
EXISTING GROUND CONTOUR	6910
PROPOSED FINISHED CONTOUR	6910
BASIN BOUNDARY EAST CHERRY CREEK	
MAJOR BASIN BOUNDARY	
BASIN BOUNDARY BLACK SQUIRREL	
DESIGN POINT	3
BASIN IDENTIFIER / AREA IN ACRES	BB / 10.0
EXISTING DIRECTION OF FLOW	
PROPOSED DIRECTION OF FLOW	
STORM SEWER	



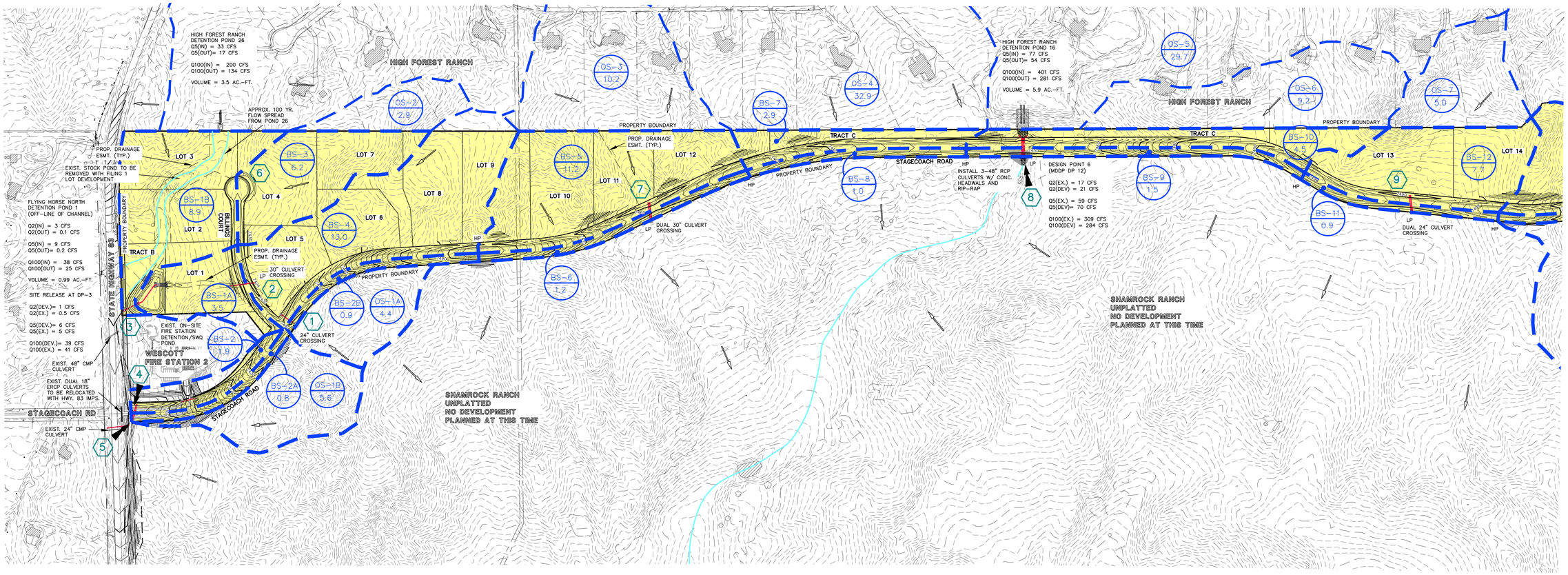
SCALE: 1" = 500'

FLYING HORSE NORTH
PRELIMINARY/FINAL DRAINAGE REPORT
PRE-DEVELOPMENT DRAINAGE MAP

CLASSIC CONSULTING ENGINEERS & SURVEYORS

619 N. Cascade Avenue, Suite 200
Colorado Springs, Colorado 80903
(719)785-0790
(719)785-0799 (Fax)

DESIGNED BY	MAW	SCALE	DATE 3-16-18
DRAWN BY	MAW	(H) 1"= 500'	SHEET 1 OF 1
CHECKED BY		(V) 1"= N/A	JOB NO. 1096.11



BASIN SUMMARY - DEVELOPED CONDITIONS

BASIN (label)	AREA (acres)	COMPOSITE CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
OS-1A	4.40	61.0	0.20	0.4	1.6	7.7
OS-1B	5.60	61.0	0.21	0.5	1.9	9.4
EX-DP-3 (Pre-Dev.)	36.00	60.0	0.25	0.5	4.8	41.3
OS-2	2.90	61.0	0.20	0.1	0.6	4.0
OS-3	10.20	65.0	0.19	1.0	3.8	17.9
OS-4	32.90	65.0	0.23	2.8	11.2	53.6
OS-5	29.70	65.0	0.39	1.9	7.1	37.0
OS-6	9.20	65.0	0.21	0.9	3.2	15.5
OS-7	5.00	65.0	0.18	0.5	2.0	9.0
BS-1A	3.50	65.0	0.17	0.4	1.4	6.3
BS-1B	8.90	65.0	0.20	0.4	2.4	13.8
BS-2	1.90	89.0	0.35	2.9	4.2	8.4
BS-2A	0.80	89.0	0.13	1.2	1.8	3.5
BS-2B	0.90	89.0	0.12	1.4	2.0	4.0
BS-3	6.20	65.0	0.20	0.6	2.3	10.8
BS-4	13.00	67.0	0.23	1.9	5.5	23.6
BS-5	11.20	65.0	0.18	1.1	4.4	20.1
BS-6	1.20	89.0	0.09	1.9	2.8	5.4
BS-7	2.90	65.0	0.13	4.4	6.4	12.8
BS-8	1.00	89.0	0.12	1.6	2.2	4.5
BS-9	1.50	89.0	0.13	2.3	3.3	6.6
BS-10	4.50	65.0	0.24	6.0	8.7	17.5
BS-11	0.90	89.0	0.08	1.5	2.1	4.1
BS-12	7.70	65.0	0.19	0.8	3.0	13.8

DESIGN POINTS SURFACE ROUTING SUMMARY - DEVELOPED CONDITIONS

Design Point (label)	Contributing Basins	Q 2 Yr. Q (cfs)	Q 5 Yr. Q (cfs)	Q 100 Yr. Q (cfs)
DP-1 DEV	OS-1A, BS-2B	1.6	3.4	11
DP-2 DEV	DP-1, BS-4	3.2	8.8	35
TOTAL INFLOW TO POND 1 (UD Detention hydrograph)	**DP-1, DP-2, BS-1A**	**4**	**7**	**38**
DP-3 DEV (Pond Pack routing)	**OS-2, BS-3, BS-1B, Release from FHN Pond 1**	**1**	**6**	**39**
DP-4 DEV	BS-2	2.9	4.2	8
DP-5 DEV	OS-1B, BS-2A	1.5	3.5	13
DP-6 DEV	OS-2, BS-3	0.6	2.8	15
DP-7 DEV	OS-3, BS-5	2.1	8.2	38
DP-8 DEV	OS-4, OS-5, OS-6, BS-7, BS-10, Release from Exist. HFR Pond 16	20.9	70.4	284
DP-9 DEV	OS-7, BS-12	1.3	5.0	23

LEGEND

DESCRIPTION	SYMBOL
EXISTING GROUND CONTOUR	6910
PROPOSED FINISHED CONTOUR	6910
BASIN BOUNDARY BLACK SQUIRREL	
DESIGN POINT	3
BASIN IDENTIFIER / AREA IN ACRES	BB 10.0
EXISTING DIRECTION OF FLOW	
PROPOSED DIRECTION OF FLOW	
STORM SEWER	
FILING NO. 1 PLAT AREA	

N

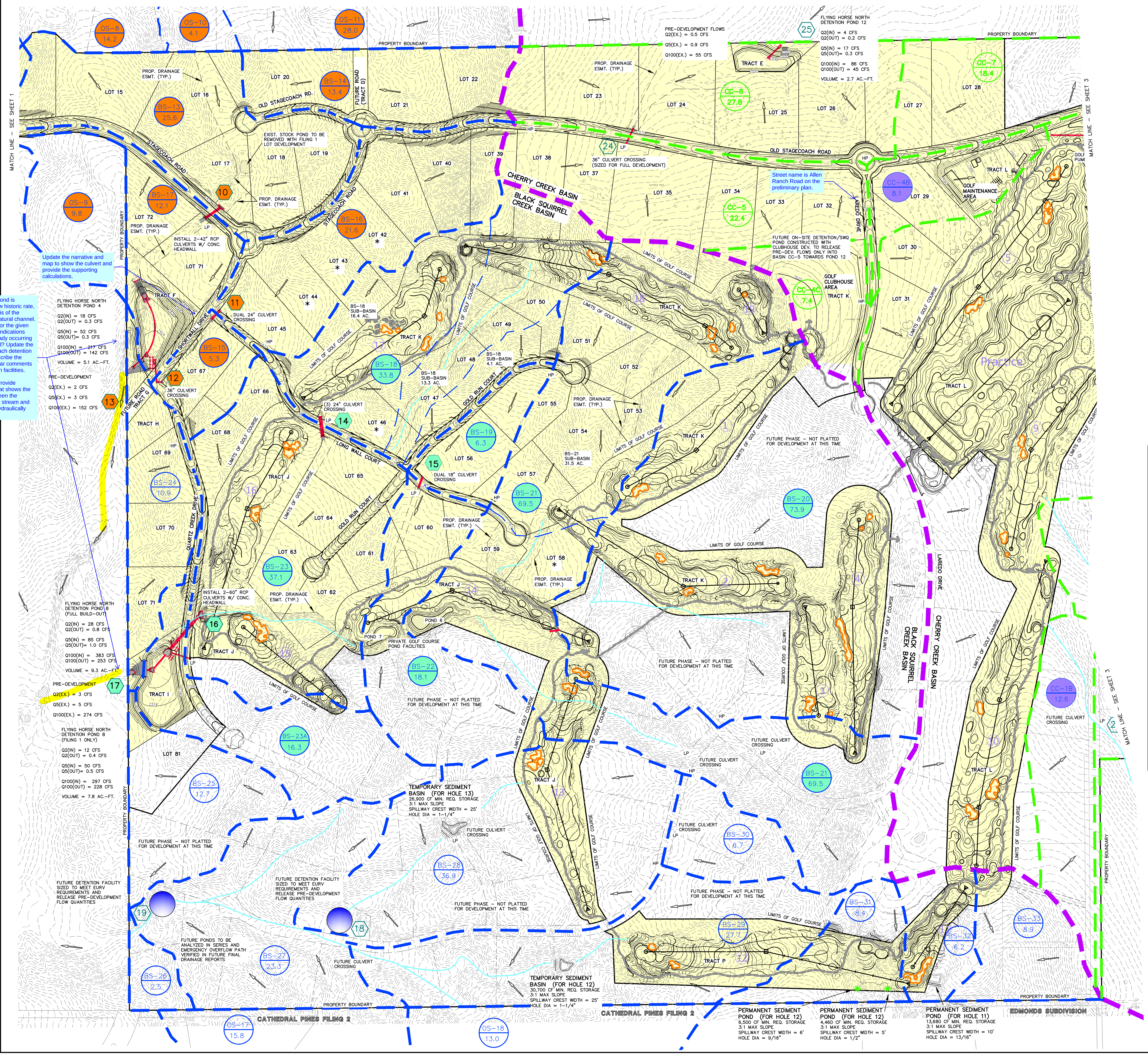
200 100 0 200 400

SCALE: 1" = 200'

CLASSIC CONSULTING ENGINEERS & SURVEYORS SM
619 N. Cascade Avenue, Suite 200 (719)785-0790
Colorado Springs, Colorado 80903 (719)785-0799 (Fax)

FLYING HORSE NORTH
PRELIMINARY/FINAL DRAINAGE REPORT
FILING NO. 1 DRAINAGE MAP

DESIGNED BY	MAW	SCALE		DATE	10-20-17
DRAWN BY	MAW	(H) 1"= 200'		SHEET	1 OF 4
CHECKED BY		(V) 1"= N/A		JOB NO.	1096.11



BASIN SUMMARY - DEVELOPED CONDITIONS

BASIN (label)	AREA (acres)	COMPOSITE CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
OS-8	14.20	65.0	0.27	2.1	6.2	24.7
OS-9	9.80	60.0	0.37	0.1	1.0	9.1
OS-10	4.10	65.0	0.17	0.7	2.1	8.2
OS-11	28.00	65.0	0.35	2.4	8.2	38.7
OS-12	68.10	62.7	0.37	2.2	11.9	75.8
OS-13	36.90	63.0	0.33	1.4	7.4	45.0
OS-14	26.40	62.0	0.31	0.7	4.6	31.0
OS-15	70.80	63.9	0.38	3.3	14.8	84.2
OS-16	4.50	65.0	0.24	0.4	1.5	7.2
OS-17	15.80	65.0	0.19	1.6	5.9	27.7
OS-18	13.00	65.0	0.20	1.3	4.7	22.6
BS-13	25.60	65.0	0.23	3.7	10.2	40.7
BS-14	13.40	65.0	0.23	2.6	6.8	26.5
BS-15	5.30	65.0	0.18	1.6	3.7	12.2
BS-16	21.60	65.0	0.34	4.6	11.8	44.1
BS-17	12.10	65.0	0.21	3.1	7.7	26.7
BS-18	33.80	63.6	0.41	3.5	12.4	56.0
BS-19	6.30	65.0	0.18	2.1	4.6	15.0
BS-20	73.90	63.4	0.31	7.4	24.6	112.4
BS-21	69.50	64.3	0.35	7.8	23.9	103.0
BS-22	18.10	64.4	0.22	3.7	9.6	36.5
BS-23	37.10	63.3	0.33	4.5	13.6	58.2
BS-23A	16.30	64.4	0.29	5.5	12.0	38.3
BS-24	10.90	63.0	0.17	0.6	3.3	17.6
EX-24 (Pre-Dev.)	13.20	60.0	0.17	0.2	2.2	17.8
BS-25	12.70	63.0	0.23	0.4	2.7	17.3
BS-26	2.50	60.0	0.18	0.0	0.4	3.4
BS-27	23.30	65.0	0.22	2.1	8.0	38.8
BS-28	36.90	64.4	0.32	2.2	9.3	49.4
BS-29	27.70	64.0	0.33	1.4	6.5	35.9
BS-30	6.70	65.0	0.20	0.7	2.4	11.7
BS-31	8.40	62.5	0.23	0.3	1.9	11.8
BS-32	6.20	62.6	0.20	0.3	1.6	9.4
BS-33	8.90	64.7	0.19	0.8	3.2	15.3
CC-1A	9.80	65.0	0.23	0.8	3.3	16.0
CC-1B	12.60	64.8	0.25	1.0	4.0	19.4
CC-2A	11.00	65.0	0.22	1.0	3.8	18.3
CC-2B	20.80	65.0	0.22	1.9	7.1	34.6
CC-2C	6.40	65.0	0.18	0.7	2.5	11.5
CC-3	52.50	63.1	0.43	1.8	8.8	54.5
CC-4A	108.70	62.6	0.44	15.4	39.0	156.0
CC-4B	8.10	76.1	0.26	4.0	7.3	20.6
CC-4C (Pre-Dev.)	7.40	61.0	0.13	0.2	1.8	11.2
CC-5	22.40	65.0	0.26	1.8	7.1	34.3
CC-6	27.80	65.0	0.25	2.3	9.1	43.2
CC-7	18.40	65.0	0.29	1.4	5.4	27.0

DESIGN POINTS SURFACE ROUTING SUMMARY - DEVELOPED CONDITIONS

Design Point (label)	Contributing Basins	Q 2 Yr. Q (cfs)	Q 5 Yr. Q (cfs)	Q 100 Yr. Q (cfs)
DP-10 DEV	OS-8, OS-10, OS-11, BS-13, BS-14	10.7	32.0	143
DP-11 DEV	BS-16	4.6	11.8	36
DP-12 DEV	DP-11, BS-15	4.0	11.0	44
TOTAL INFLOW TO POND 4 (UD Detention hydrograph)	**DP-10, DP-12, BS-17, OS-9**	**10**	**16**	**217**
DP-13 DEV	**Release from FHN Pond 4**	**0.3**	**0.3**	**142**
DP-14 DEV	BS-18	3.5	12.4	56
DP-15 DEV	BS-19	2.1	4.6	15
DP-16 DEV	DP-14, DP-15, BS-20, BS-21, BS-22, BS-23	25.0	78.0	362
TOTAL INFLOW TO FHN POND 8 (Full Build-out) (UD Detention hydrograph)	**DP-10, DP-12, BS-17, OS-9**	**24**	**37**	**390**
DP-17 DEV (Full Build-out)	**Release from FHN Pond 8**	**0.8**	**1.0**	**253**
TOTAL INFLOW TO FHN POND 8 (Filing 1 Only) (UD Detention hydrograph)	**DP-10, DP-12, BS-17, OS-9**	**9**	**14**	**301**
DP-17 DEV (Filing 1 Only)	**Release from FHN Pond 8**	**0.4**	**0.5**	**219**
DP-18 DEV	BS-28, BS-29, BS-30, OS-18	5.0	21.6	115
DP-19 DEV	BS-27, OS-17, Release from DP-18	3.8	16.8	126
DP-20 DEV	CC-1A, OS-12	3.2	14.3	88
DP-21 DEV	CC-2A, OS-13	2.1	10.5	62
DP-22 DEV	CC-2B, Release from DP-21	3.7	16.6	92
DP-23 DEV	CC-3, OS-14	2.5	13.0	84
DP-24 DEV	CC-4C (Pre-Dev.), CC-5	1.9	8.4	45
TOTAL INFLOW TO POND 12 (UD Detention hydrograph)	**CC-4C, CC-5, CC-6**	**4**	**17**	**86**
DP-25 DEV	**Release from FHN Pond 12**	**0.2**	**0.3**	**45**

LEGEND

DESCRIPTION	SYMBOL
EXISTING GROUND CONTOUR	6910
PROPOSED FINISHED CONTOUR	6910
BASIN BOUNDARY EAST CHERRY CREEK	
MAJOR BASIN BOUNDARY	
BASIN BOUNDARY BLACK SQUIRREL	
DESIGN POINT	3
LOTS WITH CULVERT DESIGN REQUIRED	*
BASIN IDENTIFIER / AREA IN ACRES	BB 10.0
EXISTING DIRECTION OF FLOW	
PROPOSED DIRECTION OF FLOW	
STORM SEWER	
FILING NO. 1 PLAT AREA	

200 100 0 200 400

SCALE: 1" = 200'

CLASSIC CONSULTING ENGINEERS & SURVEYORS

619 N. Cascade Avenue, Suite 200
Colorado Springs, Colorado 80903
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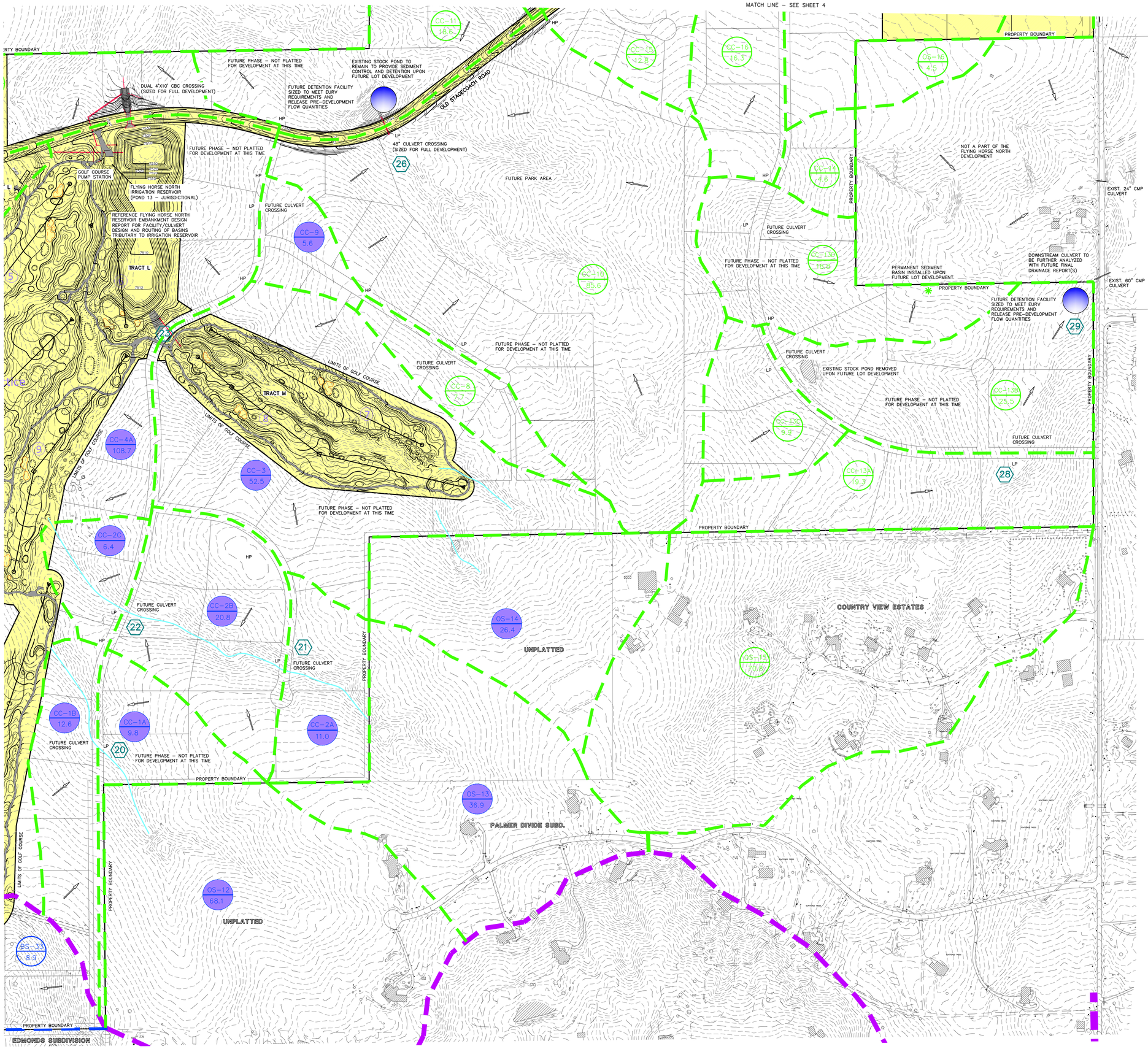
(719)785-0799 (Fax)

FLYING HORSE NORTH
PRELIMINARY/FINAL DRAINAGE REPORT
FILING NO. 1 DRAINAGE MAP

DESIGNED BY	MAW	SCALE	DATE 10-25-17
DRAWN BY	MAW	(H) 1"= 200'	SHEET 2 OF 4
CHECKED BY		(V) 1"= N/A	JOB NO. 1096.11



BASIN SUMMARY - DEVELOPED CONDITIONS

BASIN (label)	AREA (acres)	COMPOSITE CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
OS-8	14.20	65.0	0.27	2.1	6.2	24.7
OS-9	9.80	60.0	0.37	0.1	1.0	9.1
OS-10	4.10	65.0	0.17	0.7	2.1	8.2
OS-11	28.00	65.0	0.35	2.4	8.2	38.7
OS-12	68.10	62.7	0.37	2.2	11.9	75.8
OS-13	36.90	63.0	0.33	1.4	7.4	45.0
OS-14	26.40	62.0	0.31	0.7	4.6	31.0
OS-15	70.80	63.9	0.38	3.3	14.8	84.2
OS-16	4.50	65.0	0.24	0.4	1.5	7.2
OS-17	15.80	65.0	0.19	1.6	5.9	27.7
OS-18	13.00	65.0	0.20	1.3	4.7	22.6
CC-1A	9.80	65.0	0.23	0.8	3.3	16.0
CC-1B	12.60	64.8	0.25	1.0	4.0	19.4
CC-2A	11.00	65.0	0.22	1.0	3.8	18.3
CC-2B	20.80	65.0	0.22	1.9	7.1	34.6
CC-2C	6.40	65.0	0.18	0.7	2.5	11.5
CC-3	52.50	63.1	0.43	1.8	8.8	54.5
CC-4A	108.70	62.6	0.44	15.4	39.0	156.0
CC-4B	8.10	76.1	0.26	4.0	7.3	20.6
CC-4C (Pre-Dev.)	7.40	61.0	0.13	0.2	1.8	11.2
CC-5	22.40	65.0	0.26	1.8	7.1	34.3
CC-6	27.80	65.0	0.25	2.3	9.1	43.2
CC-7	18.40	65.0	0.29	1.4	5.4	27.0
CC-8	7.70	65.0	0.25	0.6	2.5	12.0
CC-9	5.60	65.0	0.19	0.6	2.1	9.8
CC-10	85.60	62.6	0.39	2.6	14.1	91.9
CC-11	18.60	63.1	0.21	0.9	5.0	28.1
CC-12	12.20	65.0	0.26	1.0	3.9	18.7
CC-13A	19.30	65.0	0.31	1.4	5.4	27.3
CC-13B	25.50	65.0	0.31	1.8	7.2	36.1
CC-13C	9.90	65.0	0.22	0.9	3.4	16.5
CC-13D	18.80	65.0	0.25	1.5	6.2	29.2
CC-14	4.60	65.0	0.21	0.4	1.6	7.8
CC-15	12.80	65.0	0.24	1.1	4.3	20.4
CC-16	16.30	65.0	0.30	1.2	4.6	23.6
CC-17	25.00	65.0	0.35	1.7	6.5	32.8
CC-18	6.20	66.5	0.30	0.7	2.2	9.7
CC-19	3.70	65.0	0.25	0.3	1.2	5.8
CC-20	39.30	65.0	0.25	3.2	12.9	61.0
CC-21	6.20	61.0	0.20	0.1	1.2	8.5
CC-22	13.80	65.0	0.25	1.1	4.5	21.4
CC-23	5.70	64.7	0.33	0.4	1.5	7.7
CC-24	39.60	65.0	0.25	3.3	13.0	61.5
CC-25	3.50	65.0	0.23	0.3	1.2	5.7
CC-26	16.70	65.0	0.29	1.4	5.3	25.6
CC-27	18.90	64.4	0.31	1.2	4.9	25.8
CC-28	154.80	64.4	0.63	6.5	24.7	136.3

DESIGN POINTS SURFACE ROUTING SUMMARY - DEVELOPED CONDITIONS

Design Point (label)	Contributing Basins	Q 2 Yr. Q (cfs)	Q 5 Yr. Q (cfs)	Q 100 Yr. Q (cfs)
DP-20 DEV	CC-1A, OS-12	3.2	14.3	88
DP-21 DEV	CC-2A, OS-13	2.1	10.5	62
DP-22 DEV	CC-2B, Release from DP-21	3.7	16.6	92
DP-23 DEV	CC-3, OS-14	2.5	13.0	84
DP-24 DEV	CC-4C (Pre-Dev.), CC-5	1.9	8.4	45
TOTAL INFLOW TO POND 12 (UD Detention hydrograph)	**CC-4C, CC-5, CC-6**	**6**	**9**	**85**
DP-25 DEV	**Release from FHN Pond 12**	**0.2**	**0.3**	**45**
DP-26 DEV	CC-8, CC-10	3.0	15.9	102
DP-27 DEV	CC-15, CC-20	4.3	17.2	81
DP-28 DEV	CC-13A, OS-15	4.6	19.8	110
DP-29 DEV	CC-13B, CC-13C, Release from DP-28	5.8	26.6	155
DP-30 DEV	CC-18	0.7	2.2	10
DP-31 DEV	CC-19, Release from DP-30	0.9	3.2	15
DP-32 DEV	CC-17, OS-16	2.0	7.8	40
DP-33 DEV	CC-23, CC-24	3.6	14.4	69
DP-34 DEV	CC-26, CC-27, CC-28 and Release from CC-16 & DP-32	6.0	23.5	168

LEGEND

DESCRIPTION	SYMBOL
EXISTING GROUND CONTOUR	6910
PROPOSED FINISHED CONTOUR	6910
BASIN BOUNDARY EAST CHERRY CREEK	
MAJOR BASIN BOUNDARY	
DESIGN POINT	3
BASIN IDENTIFIER / AREA IN ACRES	BB 10.0
EXISTING DIRECTION OF FLOW	
PROPOSED DIRECTION OF FLOW	
STORM SEWER	
FILING NO. 1 PLAT AREA	

200 100 0 200 400

SCALE: 1" = 200'

CLASSIC CONSULTING ENGINEERS & SURVEYORS
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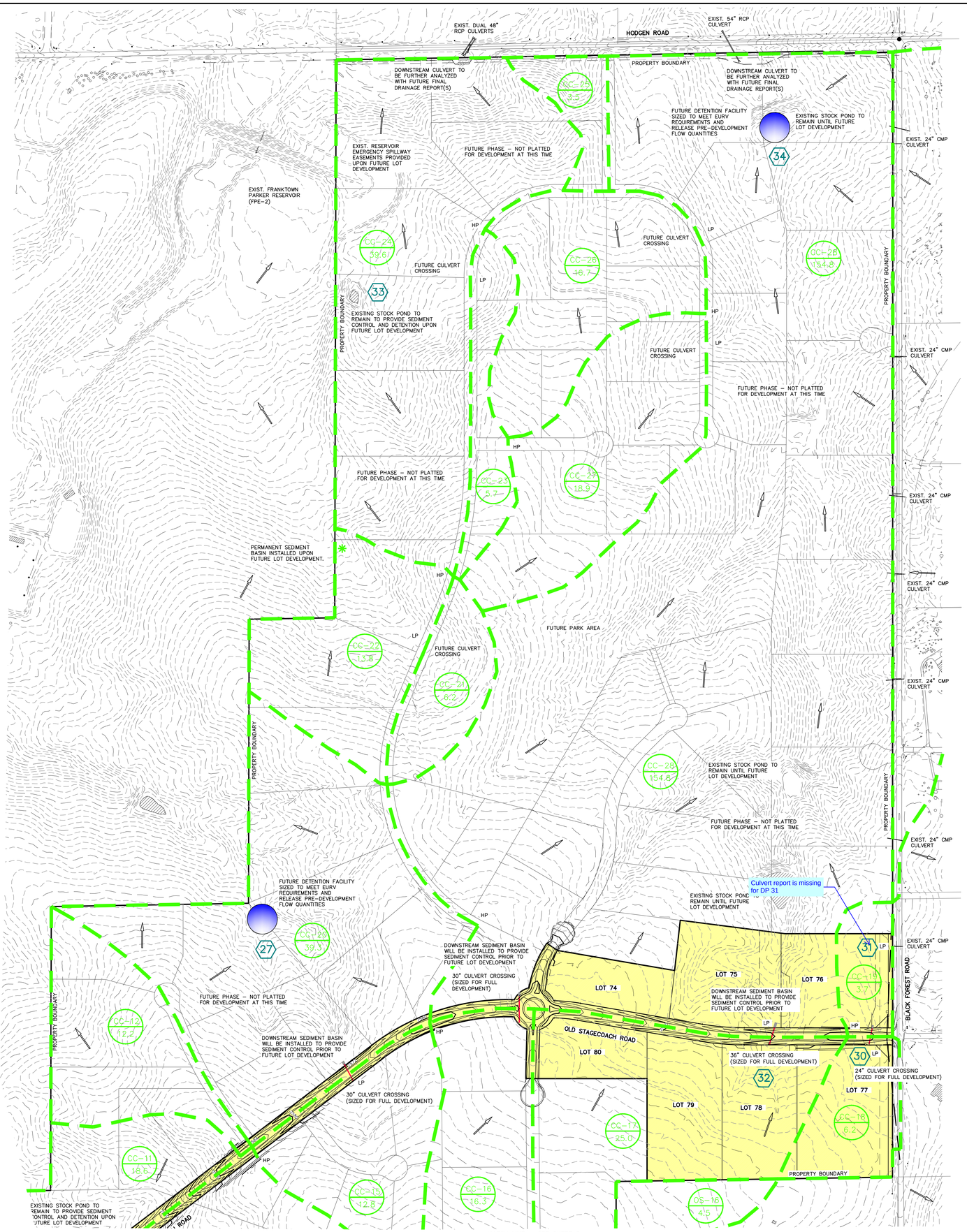
FLYING HORSE NORTH
PRELIMINARY/FINAL DRAINAGE REPORT
FILING NO. 1 AND
PRELIMINARY PLAN DRAINAGE MAP

DESIGNED BY	MAW	SCALE		DATE	10-25-17
DRAWN BY	MAW	(H) 1"= 200'		SHEET	3 OF 4
CHECKED BY		(V) 1"= N/A		JOB NO.	1096.11



BASIN SUMMARY - DEVELOPED CONDITIONS

BASIN (label)	AREA (acres)	COMPOSITE CN	TOTAL LAG TIME (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 100 Yr. (cfs)
OS-16	4.50	65.0	0.24	0.4	1.5	7.2
OS-17	15.80	65.0	0.19	1.6	5.9	27.7
OS-18	13.00	65.0	0.20	1.3	4.7	22.6
CC-11	18.60	63.1	0.21	0.9	5.0	28.1
CC-12	12.20	65.0	0.26	1.0	3.9	18.7
CC-13A	19.30	65.0	0.31	1.4	5.4	27.3
CC-13B	25.50	65.0	0.31	1.8	7.2	36.1
CC-13C	9.90	65.0	0.22	0.9	3.4	16.5
CC-13D	18.80	65.0	0.25	1.5	6.2	29.2
CC-14	4.60	65.0	0.21	0.4	1.6	7.8
CC-15	12.80	65.0	0.24	1.1	4.3	20.4
CC-16	16.30	65.0	0.30	1.2	4.6	23.6
CC-17	25.00	65.0	0.35	1.7	6.5	32.8
CC-18	6.20	66.5	0.30	0.7	2.2	9.7
CC-19	3.70	65.0	0.25	0.3	1.2	5.8
CC-20	39.30	65.0	0.25	3.2	12.9	61.0
CC-21	6.20	61.0	0.20	0.1	1.2	8.5
CC-22	13.80	65.0	0.25	1.1	4.5	21.4
CC-23	5.70	64.7	0.33	0.4	1.5	7.7
CC-24	39.60	65.0	0.25	3.3	13.0	61.5
CC-25	3.50	65.0	0.23	0.3	1.2	5.7
CC-26	16.70	65.0	0.26	1.4	5.3	25.6
CC-27	18.90	64.4	0.31	1.2	4.9	25.8
CC-28	154.80	64.4	0.63	6.5	24.7	136.3

DESIGN POINTS SURFACE ROUTING SUMMARY - DEVELOPED CONDITIONS

Design Point (label)	Contributing Basins	Q 2 Yr. Q (cfs)	Q 5 Yr. Q (cfs)	Q 100 Yr. Q (cfs)
DP-27 DEV	CC-15, CC-20	4.3	17.2	81
DP-28 DEV	CC-13A, OS-15	4.6	19.8	110
DP-29 DEV	CC-13B, CC-13C, Release from DP-28	5.8	26.6	155
DP-30 DEV	CC-18	0.7	2.2	10
DP-31 DEV	CC-19, Release from DP-30	0.9	3.2	15
DP-32 DEV	CC-17, OS-16	2.0	7.8	40
DP-33 DEV	CC-23, CC-24	3.6	14.4	69
DP-34 DEV	CC-26, CC-27, CC-28 and Release from CC-16 & DP-32	6.0	23.5	168

LEGEND

DESCRIPTION	SYMBOL
EXISTING GROUND CONTOUR	6910
PROPOSED FINISHED CONTOUR	6910
BASIN BOUNDARY EAST CHERRY CREEK	
MAJOR BASIN BOUNDARY	
BASIN BOUNDARY BLACK SQUIRREL	
DESIGN POINT	3
BASIN IDENTIFIER / AREA IN ACRES	BB 10.0
EXISTING DIRECTION OF FLOW	
PROPOSED DIRECTION OF FLOW	
STORM SEWER	
FILING NO. 1 PLAT AREA	

200 100 0 200 400

SCALE: 1" = 200'

CLASSIC CONSULTING ENGINEERS & SURVEYORS
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Colorado Springs, Colorado 80903 (719)785-0799 (Fax)

FLYING HORSE NORTH
PRELIMINARY/FINAL DRAINAGE REPORT
FILING NO. 1 AND
PRELIMINARY PLAN DRAINAGE MAP

DESIGNED BY	MAW	SCALE		DATE	10-17-17
DRAWN BY	MAW	(H) 1"= 200'		SHEET	4 OF 4
CHECKED BY		(V) 1"= N/A		JOB NO.	1096.11