

**HAY CREEK VALLEY**  
**FINAL DRAINAGE REPORT AMENDMENT**

**Prepared for:**

**VIEW HOMES, INC.**  
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(719) 382-9433

**Prepared by:**



**Matrix**

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February 2025

Project No. 22.886.076

Original PCD File SF2324  
Amendment PCD File SF

**Engineer's Statement:**

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

\_\_\_\_\_  
Jesse Sullivan  
Registered Professional Engineer  
State of Colorado  
No. 55600

\_\_\_\_\_  
Date



**Owner/Developer's Statement:**

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

**View Homes, Inc.**

Business Name

By: Timothy Buschar  
Timothy Buschar

2/5/25  
Date

Title: Director of Entitlement

Address: 555 Middle Creek Parkway Suite 500  
Colorado Springs, CO 80921

**El Paso County:**

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

\_\_\_\_\_  
Joshua Palmer, P.E.  
County Engineer / ECM Administrator  
Conditions:

\_\_\_\_\_  
Date

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**ATTACHMENTS**

1. Vicinity Map
2. HEC-HMS Results
3. Rational Calcs
4. Outfall Protection Calcs
5. Outfall Swale Calcs
6. Pond Calculations
7. StormCAD Results
8. Proposed Conditions Drainage Map

## **Introduction**

The Hay Creek Valley site is comprised of approximately 214.6 acres of unplatted and mostly undeveloped land. The site is located on Snow Mountain Heights approximately 700 feet south of its intersection with Hay Creek Road. The site is currently comprised of six (6) parcels which are to be subdivided into 20 lots and four (4) tracts. The existing access road will be replaced with a private road located within a proposed 70-foot-wide tract which will terminate with a cul-de-sac in the southwestern section of the site.

## ***Purpose and Scope of Study***

The purpose of this Final Drainage Report Amendment (FDR) is to provide updated information and calculations for the site and update the original **Hay Creek Valley Final Drainage Report**, Prepared by Matrix Design Group, Dated January 2024. Changes to the site include:

- The storm sewer layout has been revised to convey the stormwater around lots 8 and 9 reducing the impact the proposed drainage easement will have on the future development of those lots.
- The StormCAD model has been updated to show the changes to the proposed storm sewer.
- The HEC-HMS model has been updated based on the revised layout to accurately depict the new storm sewer layout.
- The length of the emergency spillway for Pond 1 has been decreased from 150-feet to 15-feet.
- A datum shift has been implemented to the site resulting in all surfaces and infrastructure to drop 3.60 feet. There are no changes to the site layout both vertically and horizontally other than the uniform elevation shift. The proposed pond elevations have been updated with the datum shift. No changes to the pond layout or dimensions have occurred. The StormCAD model has been updated to show the datum changes to the proposed storm sewer.

## **Storm Sewer Layout**

The proposed storm sewer has been designed to safely convey stormwater flows through the site while maintaining compliance with the DCM. Since the approval of the FDR The design of the storm sewer on the western side of the site has been modified. Inlets 8 and 9 (proposed private), and pipe 15 (proposed private) will remain unchanged from the original design. Pipe 16 (proposed private), which is directly downstream of inlet 8, will now tie-in to MH-51 (proposed private) to the east. The stormwater is conveyed downstream via proposed private 36-inch RCP which will combine with the flows from Design Point 13 at MH-52 eventually outfalling via proposed private 36-in flared end section. All other storm sewer will remain unchanged from the design in the approved FDR. Outfall protection calculations and channel calculations are included in the Attachments.



### **StormCAD Model**

The StormCAD Model has been updated to reflect the changes made to the storm sewer system and the datum shift changes. An updated Storm Plan showing the modifications along with StormCAD modeling of HGLs are included in the attachments. Please note that all elevations are modified due to the datum shift by this amendment and, therefore, no revision clouding is provided on the StormCAD layouts or profiles.

### **HEC-HMS Model**

The HEC-HMS model has been updated to accurately represent the stormwater flows conveyed in the proposed private 36-inch RCP that is changing with this amendment. Updated HEC-HMS results showing the modifications are included in the attachments.

### **Pond Elevation Shift**

The MHFD detention basin stage worksheet has been updated to reflect the datum changes made. An updated Stage worksheet is included in the attachments.

### **Emergency Spillway**

The Pond 1 emergency spillway has been reduced in length from 150-feet to 15-feet. The rip rap sizing for the lining of the emergency spillway has been changed from type VL to type L. Calculations including the MHFD Detention spreadsheet, and rip rap sizing are included in the Attachments.

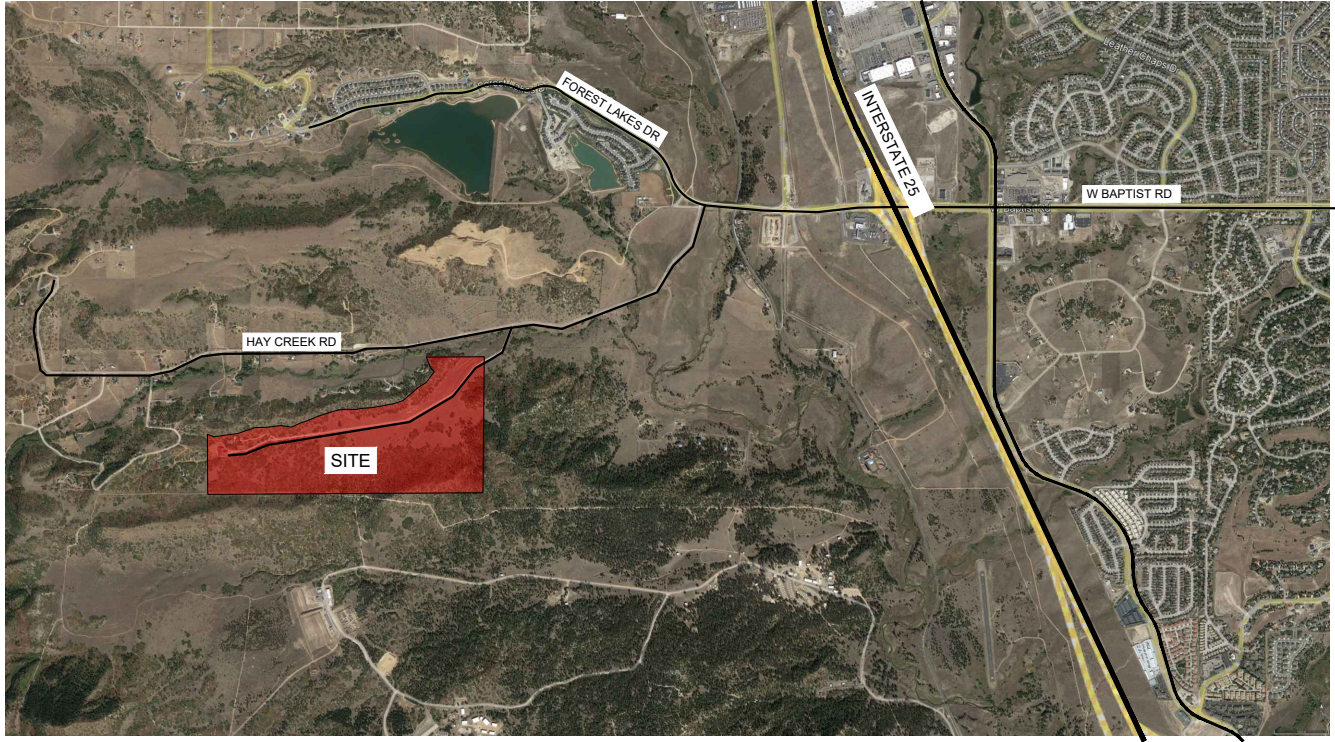
### **Conclusion**

The changes reflected in this amendment adhere to the original *Hay Creek Valley Final Drainage Report*, January 2024, prepared by Matrix Design Group, Inc. These changes do not impact the general drainage patterns within the study area and do not increase developed flows discharged to the Beaver Creek Drainage Basin. These proposed improvements should not adversely affect downstream or surrounding developments and are in conformance with the pertinent studies for the area.

## **References**

1. ***El Paso County and City of Colorado Springs Drainage Criteria Manual, Volume 1 & 2***, El Paso County, May 2014
2. ***El Paso County Engineering Criteria Manual***, El Paso County, Rev. December 2016
3. ***Urban Storm Drainage Criteria Manual, Vol. 1-3*** by Urban Drainage and Flood Control District (UDFCD), January 2016
4. ***Hay Creek Valley Final Drainage Report***, by Matrix Design Group, Inc., January 2024,

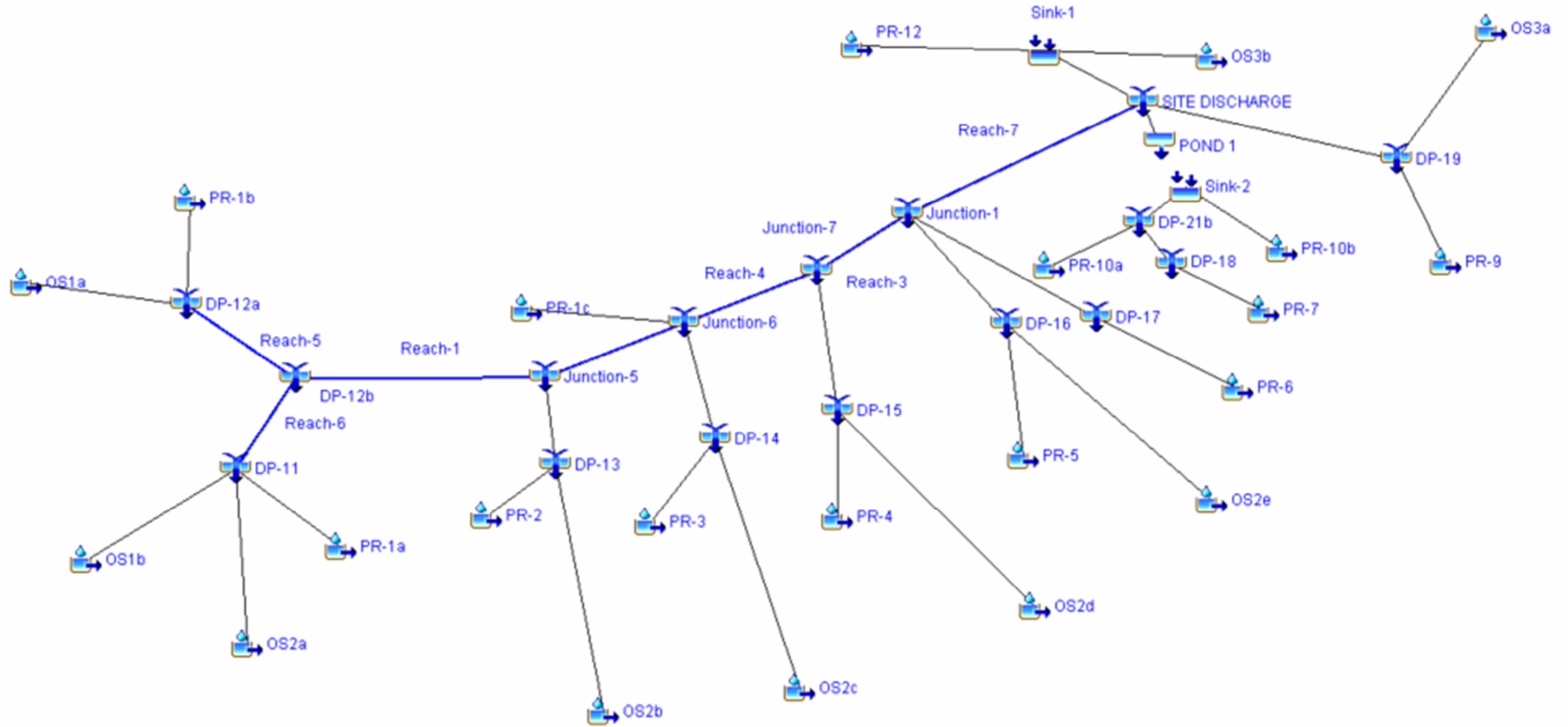
**Attachments**



VICINITY MAP  
HAY CREEK VALLEY  
(NTS)

Model Name: **Hay\_Creek\_Forest\_Manor**

*These models reflect full development of the areas included in the Hay Creek development with full spectrum detention provided to maintain historic flows. Areas tributary to Pond 1 have been modeled to drain to "Sink 2" while the pond itself has been modeled using the outflow hydrographs from the MHFD-Detention Spreadsheet.*

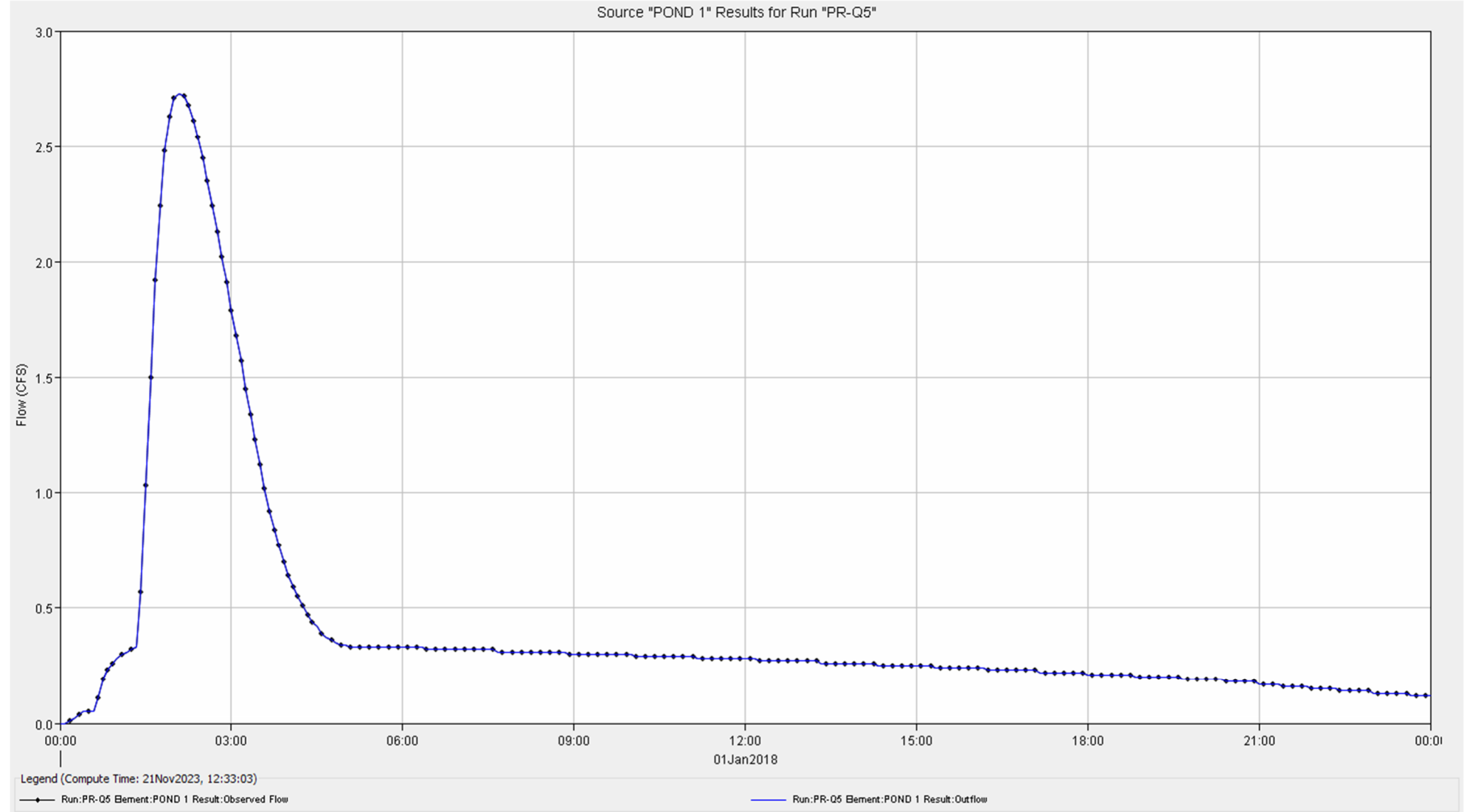


Project: Hay Creek Simulation Run: PR-Q5

Start of Run: 01Jan2018, 00:00 Basin Model: PR - HAY CREEK\_FM  
 End of Run: 02Jan2018, 00:00 Meteorologic Model: 5 YEAR EVENT  
 Compute Time: 29Oct2024, 09:17:02 Control Specifications: 1 DAY

Volume Units:  IN  AC-FT

Hydrologic Element	Drainage A... (MI2)	Peak Disch... (CFS)	Time of Peak	Volume (IN)
DP-11	0.1205	3.7	01Jan2018, 12:50	0.14
DP-12a	0.0244	1.3	01Jan2018, 12:40	0.21
DP-12b	0.1449	5.0	01Jan2018, 12:50	0.16
DP-13	0.0195	1.1	01Jan2018, 12:35	0.20
DP-14	0.0383	2.0	01Jan2018, 12:35	0.21
DP-15	0.0265	1.4	01Jan2018, 12:40	0.21
DP-16	0.0313	1.6	01Jan2018, 12:40	0.21
DP-17	0.0217	1.4	01Jan2018, 12:30	0.22
DP-18	0.0434	2.4	01Jan2018, 12:40	0.21
DP-19	0.0286	0.7	01Jan2018, 12:40	0.10
DP-21b	0.0508	5.3	01Jan2018, 12:50	0.47
Junction-1	0.3654	11.8	01Jan2018, 13:15	0.18
Junction-5	0.1644	5.5	01Jan2018, 13:05	0.16
Junction-6	0.2859	9.8	01Jan2018, 13:05	0.18
Junction-7	0.3124	10.5	01Jan2018, 13:10	0.18
OS1a	0.0146	0.8	01Jan2018, 12:40	0.21
OS1b	0.0925	2.6	01Jan2018, 12:50	0.13
OS2a	0.0078	0.4	01Jan2018, 12:25	0.14
OS2b	0.0074	0.4	01Jan2018, 12:30	0.18
OS2c	0.0095	0.8	01Jan2018, 12:25	0.20
OS2d	0.0043	0.3	01Jan2018, 12:35	0.20
OS2e	0.0049	0.3	01Jan2018, 12:30	0.20
OS3a	0.0076	0.1	01Jan2018, 12:40	0.06
OS3b	0.0051	0.1	01Jan2018, 12:30	0.06
POND 1	0.0840	4.0	01Jan2018, 01:50	0.23
PR-10a	0.0074	3.3	01Jan2018, 13:00	2.00
PR-10b	0.0006	0.8	01Jan2018, 12:15	2.66
PR-12	0.0256	0.6	01Jan2018, 12:55	0.11
PR-1a	0.0202	0.9	01Jan2018, 12:50	0.21
PR-1b	0.0098	0.5	01Jan2018, 12:40	0.21
PR-1c	0.0832	3.4	01Jan2018, 13:00	0.21
PR-2	0.0121	0.7	01Jan2018, 12:35	0.21
PR-3	0.0288	1.5	01Jan2018, 12:40	0.21
PR-4	0.0222	1.2	01Jan2018, 12:40	0.21
PR-5	0.0264	1.4	01Jan2018, 12:40	0.21
PR-6	0.0217	1.4	01Jan2018, 12:30	0.22
PR-7	0.0434	2.4	01Jan2018, 12:40	0.21
PR-9	0.0210	0.6	01Jan2018, 12:40	0.11
Reach-1	0.1449	5.0	01Jan2018, 13:05	0.15
Reach-2	0.1644	5.5	01Jan2018, 13:10	0.16
Reach-3	0.3124	10.5	01Jan2018, 13:15	0.18
Reach-4	0.2859	9.8	01Jan2018, 13:15	0.18
Reach-5	0.0244	1.3	01Jan2018, 12:45	0.21
Reach-6	0.1205	3.7	01Jan2018, 12:50	0.14
Reach-7	0.3654	11.7	01Jan2018, 13:15	0.18
Sink-1	0.5087	12.8	01Jan2018, 13:15	0.18
Sink-2	0.0514	5.4	01Jan2018, 12:45	0.50
SITE DIS...	0.4780	12.3	01Jan2018, 13:15	0.19



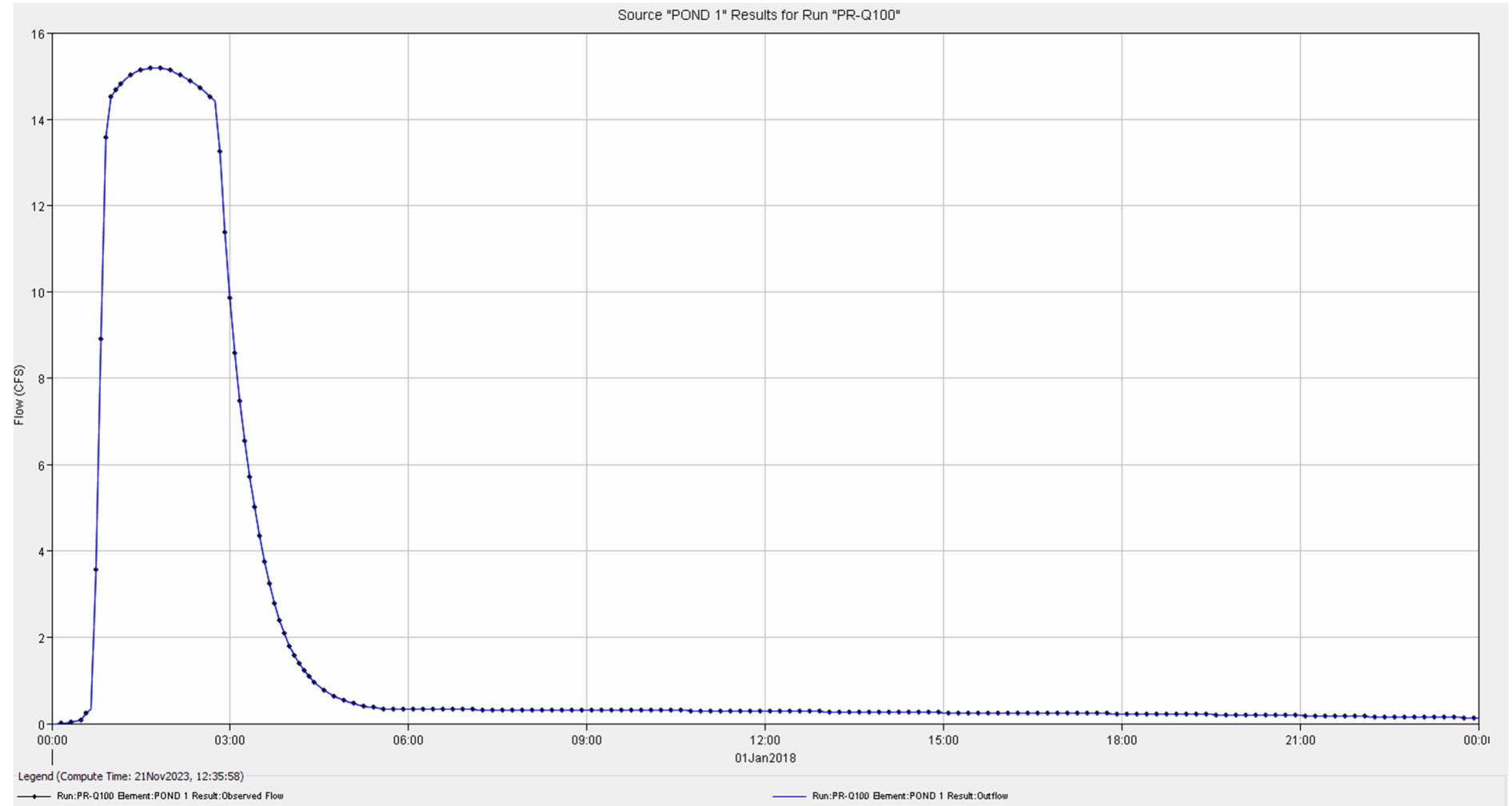


Project: Hay Creek Simulation Run: PR-Q100

Start of Run: 01Jan2018, 00:00 Basin Model: PR - HAY CREEK\_FM  
 End of Run: 02Jan2018, 00:00 Meteorologic Model: 100 YEAR EVENT  
 Compute Time: 29Oct2024, 09:13:03 Control Specifications: 1 DAY

Volume Units:  IN  AC-FT

Hydrologic Element	Drainage A... (MI2)	Peak Disch... (CFS)	Time of Peak	Volume (IN)
DP-11	0.1205	19.5	01Jan2018, 12:55	0.74
DP-12a	0.0244	6.1	01Jan2018, 12:45	0.92
DP-12b	0.1449	25.2	01Jan2018, 12:55	0.77
DP-13	0.0195	5.1	01Jan2018, 12:40	0.89
DP-14	0.0383	8.7	01Jan2018, 12:40	0.89
DP-15	0.0265	6.5	01Jan2018, 12:45	0.91
DP-16	0.0313	7.4	01Jan2018, 12:45	0.91
DP-17	0.0217	6.3	01Jan2018, 12:35	0.92
DP-18	0.0434	10.9	01Jan2018, 12:45	0.92
DP-19	0.0286	4.5	01Jan2018, 12:45	0.63
DP-21b	0.0508	18.1	01Jan2018, 12:50	1.45
Junction-1	0.3654	63.5	01Jan2018, 13:05	0.84
Junction-5	0.1644	28.2	01Jan2018, 13:05	0.77
Junction-6	0.2859	50.3	01Jan2018, 13:05	0.83
Junction-7	0.3124	54.8	01Jan2018, 13:05	0.83
OS1a	0.0146	3.6	01Jan2018, 12:45	0.92
OS1b	0.0925	14.2	01Jan2018, 13:00	0.70
OS2a	0.0078	1.8	01Jan2018, 12:30	0.71
OS2b	0.0074	2.0	01Jan2018, 12:35	0.85
OS2c	0.0095	2.7	01Jan2018, 12:25	0.80
OS2d	0.0043	1.1	01Jan2018, 12:40	0.86
OS2e	0.0049	1.3	01Jan2018, 12:35	0.84
OS3a	0.0076	0.9	01Jan2018, 12:50	0.53
OS3b	0.0051	0.8	01Jan2018, 12:35	0.53
POND 1	0.0840	15.9	01Jan2018, 01:45	0.95
PR-10a	0.0074	7.8	01Jan2018, 13:00	4.55
PR-10b	0.0006	1.7	01Jan2018, 12:15	5.49
PR-12	0.0256	3.5	01Jan2018, 13:00	0.65
PR-1a	0.0202	4.3	01Jan2018, 12:55	0.92
PR-1b	0.0098	2.5	01Jan2018, 12:45	0.92
PR-1c	0.0832	15.7	01Jan2018, 13:05	0.91
PR-2	0.0121	3.2	01Jan2018, 12:40	0.92
PR-3	0.0288	7.0	01Jan2018, 12:45	0.92
PR-4	0.0222	5.5	01Jan2018, 12:45	0.92
PR-5	0.0264	6.3	01Jan2018, 12:45	0.92
PR-6	0.0217	6.3	01Jan2018, 12:35	0.92
PR-7	0.0434	10.9	01Jan2018, 12:45	0.92
PR-9	0.0210	3.6	01Jan2018, 12:45	0.66
Reach-1	0.1449	25.0	01Jan2018, 13:05	0.76
Reach-2	0.1644	28.2	01Jan2018, 13:10	0.77
Reach-3	0.3124	54.8	01Jan2018, 13:10	0.83
Reach-4	0.2859	50.3	01Jan2018, 13:10	0.82
Reach-5	0.0244	6.1	01Jan2018, 12:45	0.92
Reach-6	0.1205	19.5	01Jan2018, 13:00	0.74
Reach-7	0.3654	63.3	01Jan2018, 13:10	0.84
Sink-1	0.5087	70.7	01Jan2018, 13:05	0.83
Sink-2	0.0514	18.4	01Jan2018, 12:50	1.50
SITE DIS...	0.4780	66.8	01Jan2018, 13:10	0.85



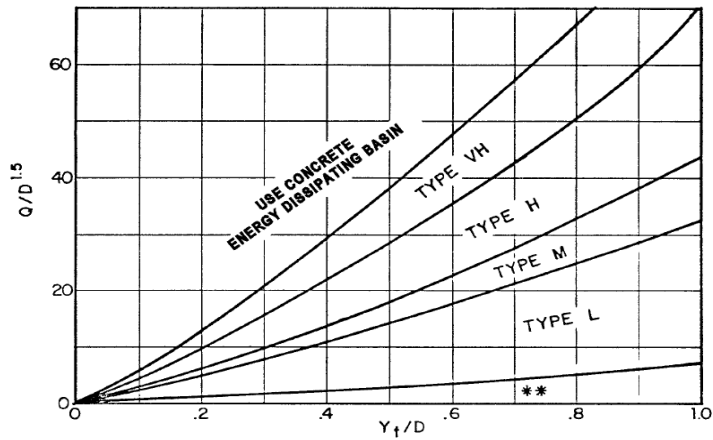




OUTFALL PROTECTION CALCULATIONS

DP13b

Pipe Size (D)	36	Inches
Q	28.2	cfs
L	9	Feet
W	9	Feet
D	0	Feet
d <sub>50</sub>	0.36	Feet
	4.28	Inches
Depth of Flow	1.25	Feet
Q/D <sup>1.5</sup>	5.43	
Y <sub>t</sub> /D	0.417	
Rip Rap	Type L for 3 x Pipe Dia Downstream	
Length of Rock	9	Feet
Width of Rock	9.0	Feet



Use D<sub>a</sub> instead of D whenever flow is supercritical in the barrel.  
\*\* Use Type L for a distance of 3D downstream.

Figure 9-38. Riprap erosion protection at circular conduit outlet (valid for Q/D2.5 ≤ 6.0)

CLASSIFICATION AND GRADATION OF ORDINARY RIP RAP			
Rip Rap Designation by Weight	% Smaller Than Given Size (inches)	Intermediate Rock Dimension	d <sub>50</sub> * (inches)
Type VL	70 - 100	12	6**
	50 - 70	9	
	35 - 50	6	
Type L	70 - 100	15	9**
	50 - 70	12	
	35 - 50	9	
Type M	70 - 100	21	12
	50 - 70	18	
	35 - 50	12	
Type H	70 - 100	30	18
	50 - 70	24	
	35 - 50	18	
Type VH	70 - 100	42	24
	50 - 70	33	
	35 - 50	24	
	2 - 10	9	

\* d<sub>50</sub> = Mean particle size  
\*\* Bury types VL and L with native top soil and revegetate to protect from vandalism.

3.2.2 Low Tailwater Basin

The design of low tailwater riprap basins is necessary when the receiving channel may have little or no flow or tailwater at time when the pipe or culvert is in operation. Figure 9-37 provides a plan and profile view of a typical low tailwater riprap basin.

By providing a low tailwater basin at the end of a storm drain conduit or culvert, the kinetic energy of the discharge dissipates under controlled conditions without causing scour at the channel bottom.

Low tailwater is defined as being equal to or less than 1/3 of the height of the storm drain, that is:

$$y_t \leq \frac{D}{3} \text{ or } y_t \leq \frac{H}{3}$$

Where:

y<sub>t</sub> = tailwater depth at design flow (feet)

D = diameter of circular pipe (feet)

H = height of rectangular pipe (feet)

Rock Size

The procedure for determining the required riprap size downstream of a conduit outlet is in Section 3.2.3.

After selecting the riprap size, the minimum thickness of the riprap layer, T, in feet, in the basin is defined as:

$$T = 2D_{50} \text{ Equation 9-15}$$

3.2.3 Rock Sizing for Riprap Apron and Low Tailwater Basin

Scour resulting from highly turbulent, rapidly decelerating flow is a common problem at conduit outlets. The following section summarizes the method for sizing riprap protection for both riprap aprons (Section 3.2.1) and low tailwater basins (Section 3.2.2).

Use Figure 9-38 to determine the required rock size for circular conduits and Figure 9-39 for rectangular conduits. Figure 9-38 is valid for Q/D<sup>1.5</sup> of 6.0 or less and Figure 9-39 is valid for Q/WH<sup>1.5</sup> of 8.0 or less. The parameters in these two figures are:

1. Q/D<sup>1.5</sup> or Q/WH<sup>1.5</sup> in which Q is the design discharge in cfs, D<sub>c</sub> is the diameter of a circular conduit in feet, and W and H are the width and height of a rectangular conduit in feet.
2. Y<sub>t</sub>/D<sub>c</sub> or Y<sub>t</sub>/H in which Y<sub>t</sub> is the tailwater depth in feet, D<sub>c</sub> is the diameter of a circular conduit in feet, and H is the height of a rectangular conduit in feet. In cases where Y<sub>t</sub> is unknown or a hydraulic jump is suspected downstream of the outlet, use Y<sub>t</sub>/D<sub>c</sub> = Y<sub>t</sub>/H = 0.40 when using Figures 9-38 and 9-39.
3. The riprap size requirements in Figures 9-38 and 9-39 are based on the non-dimensional parametric Equations 9-16 and 9-17 (Steven, Simons, and Watts 1971 and Smith 1975).

Circular culvert:

$$d_{50} = \frac{0.023Q}{Y_t^{1.2} D_c^{0.3}} \text{ Equation 9-16}$$

Rectangular culvert:

$$d_{50} = \frac{0.014H^{0.5}Q}{Y_t W} \text{ Equation 9-17}$$

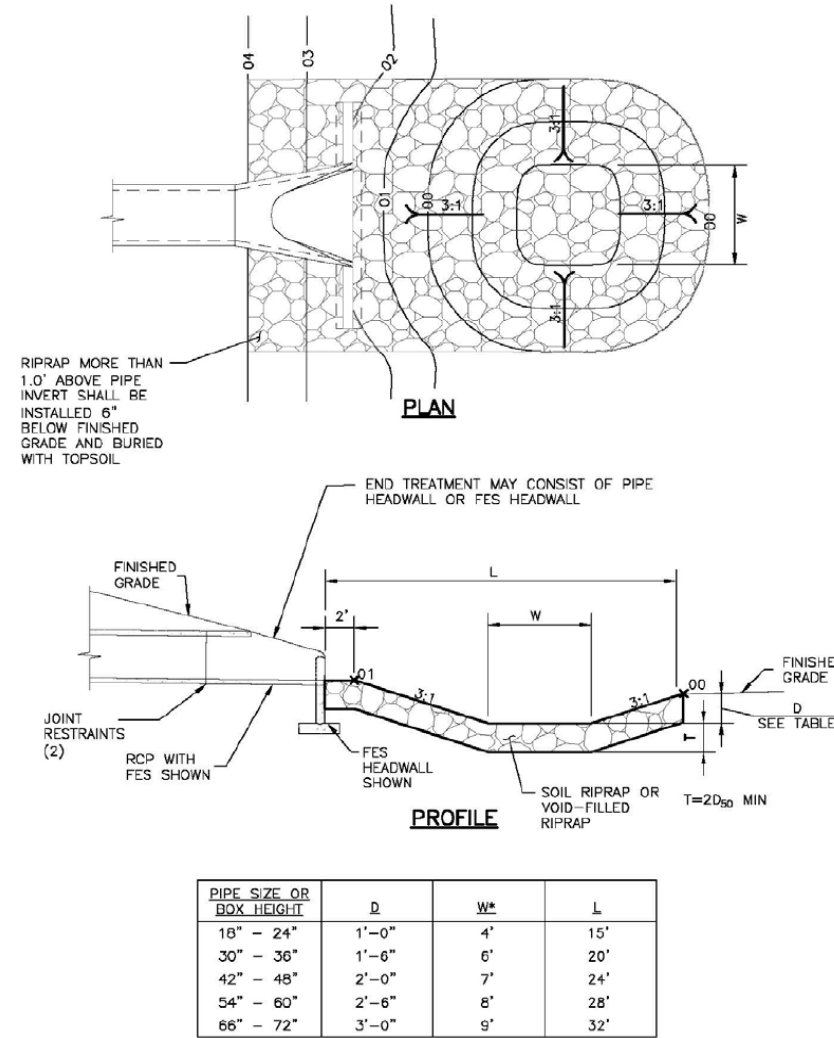


Figure 9-37. Low tailwater riprap basin

# Channel Report

## DP-13b Outfall Swale

### Trapezoidal

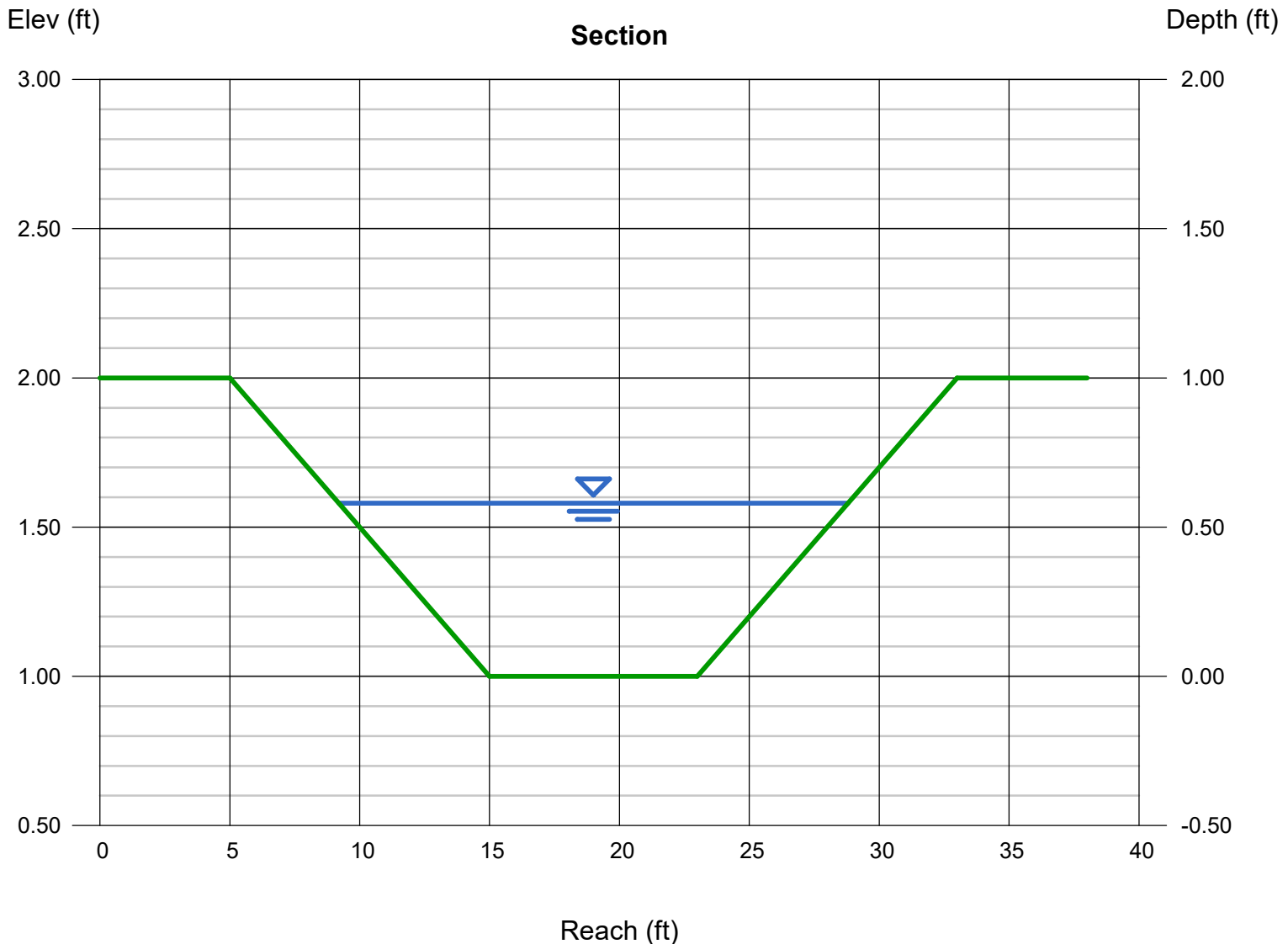
Bottom Width (ft) = 8.00  
Side Slopes (z:1) = 10.00, 10.00  
Total Depth (ft) = 1.00  
Invert Elev (ft) = 1.00  
Slope (%) = 2.00  
N-Value = 0.032

### Highlighted

Depth (ft) = 0.58  
Q (cfs) = 28.20  
Area (sqft) = 8.00  
Velocity (ft/s) = 3.52  
Wetted Perim (ft) = 19.66  
Crit Depth, Yc (ft) = 0.58  
Top Width (ft) = 19.60  
EGL (ft) = 0.77

### Calculations

Compute by: Known Q  
Known Q (cfs) = 28.20

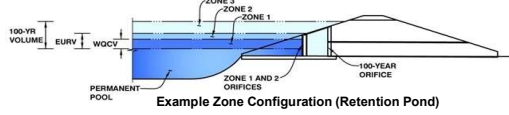


# DETENTION BASIN STAGE-STORAGE TABLE BUILDER

*MHFD-Detention, Version 4.06 (July 2022)*

**Project:** Hay Creek Valley

**Basin ID:** Beaver Creek



**Example Zone Configuration (Retention Pond)**

**Watershed Information**

Selected BMP Type = **EDB**

Watershed Area = **35.31** acres

Watershed Length = **3,000** ft

Watershed Length to Centroid = **1,500** ft

Watershed Slope = **0.048** ft/ft

Watershed Imperviousness = **15.34%** percent

Percentage Hydrologic Soil Group A = **0.0%** percent

Percentage Hydrologic Soil Group B = **100.0%** percent

Percentage Hydrologic Soil Groups C/D = **0.0%** percent

Target WQCV Drain Time = **40.0** hours

Location for 1-hr Rainfall Depths = User Input

*After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.*

Water Quality Capture Volume (WQCV) = **0.279** acre-feet

Excess Urban Runoff Volume (EURV) = **0.527** acre-feet

2-yr Runoff Volume (P1 = 1.19 in.) = **0.629** acre-feet

5-yr Runoff Volume (P1 = 1.5 in.) = **1.236** acre-feet

10-yr Runoff Volume (P1 = 1.75 in.) = **1.825** acre-feet

25-yr Runoff Volume (P1 = 2 in.) = **2.823** acre-feet

50-yr Runoff Volume (P1 = 2.25 in.) = **3.522** acre-feet

100-yr Runoff Volume (P1 = 2.52 in.) = **4.509** acre-feet

500-yr Runoff Volume (P1 = 3.55 in.) = **7.573** acre-feet

Approximate 2-yr Detention Volume = **0.352** acre-feet

Approximate 5-yr Detention Volume = **0.532** acre-feet

Approximate 10-yr Detention Volume = **0.935** acre-feet

Approximate 25-yr Detention Volume = **1.213** acre-feet

Approximate 50-yr Detention Volume = **1.280** acre-feet

Approximate 100-yr Detention Volume = **1.607** acre-feet

**Optional User Overrides**

acre-feet
acre-feet
inches
inches
inches
inches
inches
inches
inches
inches
inches
inches

Depth Increment =  ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft <sup>2</sup> )	Optional Override Area (ft <sup>2</sup> )	Area (acre)	Volume (ft <sup>3</sup> )	Volume (ac-ft)
Top of Micropool	--	0.00	--	--	--	60	0.001		
6861.9	--	0.90	--	--	--	60	0.001	54	0.001
6862.4	--	1.40	--	--	--	237	0.005	128	0.003
6862.9	--	1.90	--	--	--	1,096	0.025	461	0.011
6863.4	--	2.40	--	--	--	4,884	0.112	1,956	0.045
6863.9	--	2.90	--	--	--	12,158	0.279	6,217	0.143
6864.4	--	3.40	--	--	--	22,987	0.528	15,003	0.344
6864.9	--	3.90	--	--	--	33,167	0.761	29,042	0.667
6865.4	--	4.40	--	--	--	43,762	1.005	48,274	1.108
6865.9	--	4.90	--	--	--	49,829	1.144	71,672	1.645
6866.4	--	5.40	--	--	--	56,410	1.295	98,231	2.255
6866.9	--	5.90	--	--	--	59,443	1.365	127,195	2.920
6867.4	--	6.40	--	--	--	62,026	1.424	157,562	3.617
6867.9	--	6.90	--	--	--	64,682	1.485	189,239	4.344
6868.4	--	7.40	--	--	--	67,380	1.547	222,254	5.102
6868.9	--	7.90	--	--	--	70,161	1.611	256,640	5.892
6869.4	--	8.40	--	--	--	72,971	1.675	292,423	6.713

**Define Zones and Basin Geometry**

Zone 1 Volume (WQCV) = **0.279** acre-feet

Zone 2 Volume (EURV - Zone 1) = **0.247** acre-feet

Zone 3 Volume (100-year - Zones 1 & 2) = **1.081** acre-feet

Total Detention Basin Volume = **1.607** acre-feet

Initial Surcharge Volume (ISV) = **user** ft<sup>3</sup>

Initial Surcharge Depth (ISD) = **user** ft

Total Available Detention Depth (H<sub>total</sub>) = **user** ft

Depth of Trickle Channel (H<sub>TC</sub>) = **user** ft

Slope of Trickle Channel (S<sub>TC</sub>) = **user** ft/ft

Slopes of Main Basin Sides (S<sub>main</sub>) = **user** H:V

Basin Length-to-Width Ratio (R<sub>L/W</sub>) = **user**

Initial Surcharge Area (A<sub>ISV</sub>) = **user** ft<sup>2</sup>

Surcharge Volume Length (L<sub>ISV</sub>) = **user** ft

Surcharge Volume Width (W<sub>ISV</sub>) = **user** ft

Depth of Basin Floor (H<sub>FLOOR</sub>) = **user** ft

Length of Basin Floor (L<sub>FLOOR</sub>) = **user** ft

Width of Basin Floor (W<sub>FLOOR</sub>) = **user** ft

Area of Basin Floor (A<sub>FLOOR</sub>) = **user** ft<sup>2</sup>

Volume of Basin Floor (V<sub>FLOOR</sub>) = **user** ft<sup>3</sup>

Depth of Main Basin (H<sub>MAIN</sub>) = **user** ft

Length of Main Basin (L<sub>MAIN</sub>) = **user** ft

Width of Main Basin (W<sub>MAIN</sub>) = **user** ft

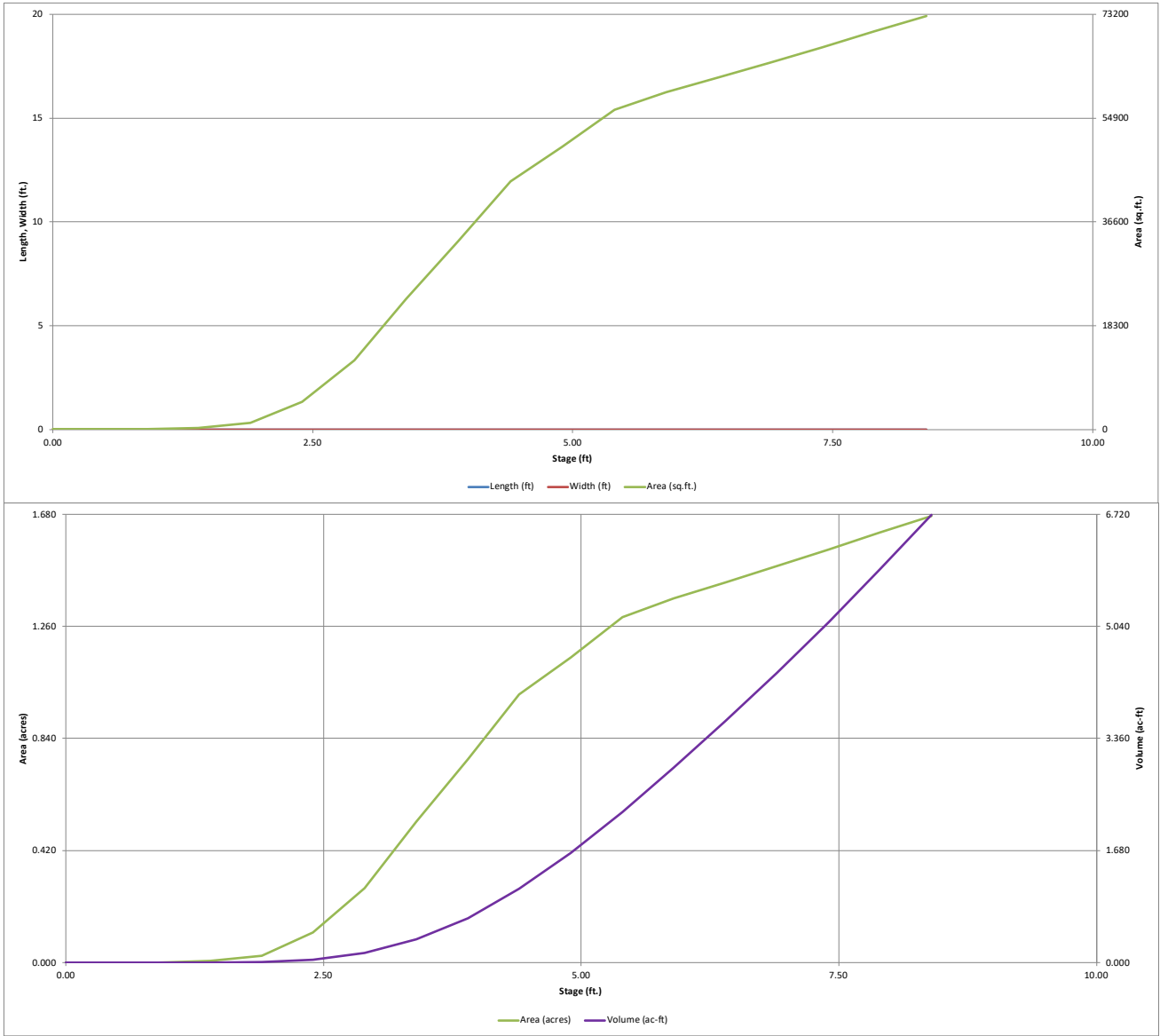
Area of Main Basin (A<sub>MAIN</sub>) = **user** ft<sup>2</sup>

Volume of Main Basin (V<sub>MAIN</sub>) = **user** ft<sup>3</sup>

Calculated Total Basin Volume (V<sub>total</sub>) = **user** acre-feet

# DETENTION BASIN STAGE-STORAGE TABLE BUILDER

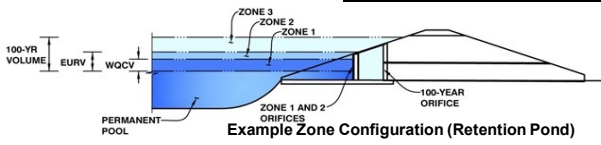
*MHFD-Detention, Version 4.06 (July 2022)*



# DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

**Project:** Hay Creek Valley  
**Basin ID:** Beaver Creek



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.27	0.279	Orifice Plate
Zone 2 (EURV)	3.71	0.247	Circular Orifice
Zone 3 (100-year)	4.87	1.081	Weir&Pipe (Restrict)
<b>Total (all zones)</b>		<b>1.607</b>	

**User Input:** Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =  ft (distance below the filtration media surface)  
Underdrain Orifice Diameter =  inches

**Calculated Parameters for Underdrain**  
Underdrain Orifice Area =  ft<sup>2</sup>  
Underdrain Orifice Centroid =  feet

**User Input:** Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =  ft (relative to basin bottom at Stage = 0 ft)  
Depth at top of Zone using Orifice Plate =  ft (relative to basin bottom at Stage = 0 ft)  
Orifice Plate: Orifice Vertical Spacing =  inches  
Orifice Plate: Orifice Area per Row =  sq. inches (diameter = 1 inch)

**Calculated Parameters for Plate**  
WQ Orifice Area per Row =  ft<sup>2</sup>  
Elliptical Half-Width =  feet  
Elliptical Slot Centroid =  feet  
Elliptical Slot Area =  ft<sup>2</sup>

**User Input:** Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.50						
Orifice Area (sq. inches)	0.83	0.83						

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

**User Input:** Vertical Orifice (Circular or Rectangular)

	Zone 2 Circular	Not Selected	
Invert of Vertical Orifice =	3.00	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	3.15	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	3.50	N/A	inches

**Calculated Parameters for Vertical Orifice**

	Zone 2 Circular	Not Selected	
Vertical Orifice Area =	0.07	N/A	ft <sup>2</sup>
Vertical Orifice Centroid =	0.15	N/A	feet

**User Input:** Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, H <sub>o</sub> =	3.86	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	6.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	4.00	N/A	feet
Overflow Grate Type =	Type C Grate	N/A	
Debris Clogging % =	50%	N/A	%

**Calculated Parameters for Overflow Weir**

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H <sub>g</sub> =	3.86	N/A	feet
Overflow Weir Slope Length =	4.00	N/A	feet
Grate Open Area / 100-yr Orifice Area =	11.25	N/A	
Overflow Grate Open Area w/o Debris =	16.70	N/A	ft <sup>2</sup>
Overflow Grate Open Area w/ Debris =	8.35	N/A	ft <sup>2</sup>

**User Input:** Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.25	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	18.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	14.10	N/A	inches

**Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate**

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	1.49	N/A	ft <sup>2</sup>
Outlet Orifice Centroid =	0.64	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	2.17	N/A	radians

**User Input:** Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	6.25	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	15.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

**Calculated Parameters for Spillway**

Spillway Design Flow Depth =	0.83	feet
Stage at Top of Freeboard =	8.08	feet
Basin Area at Top of Freeboard =	1.63	acres
Basin Volume at Top of Freeboard =	6.18	acre-ft

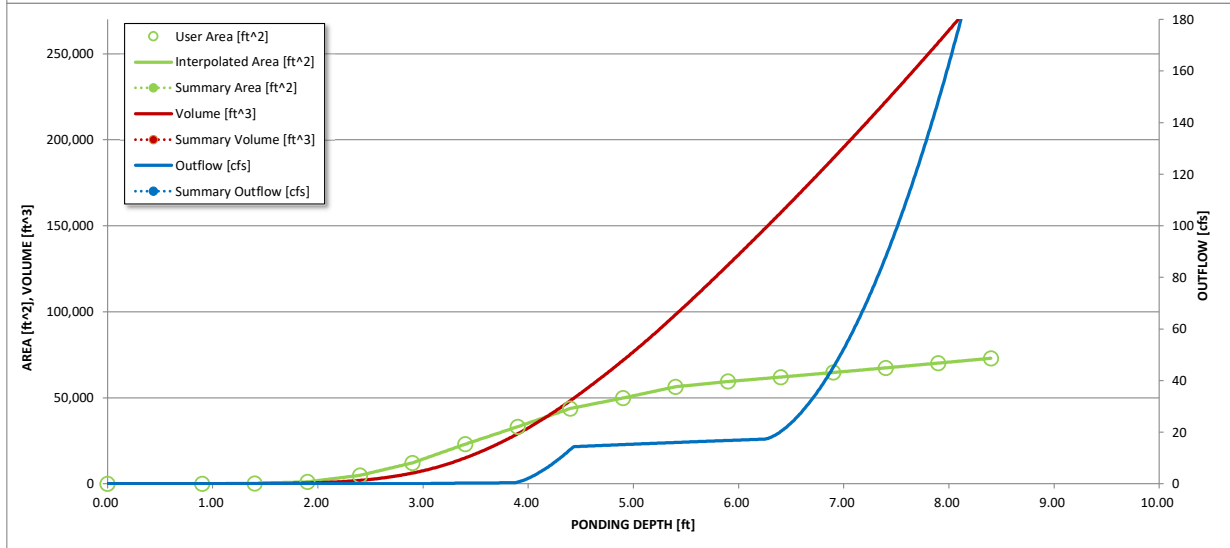
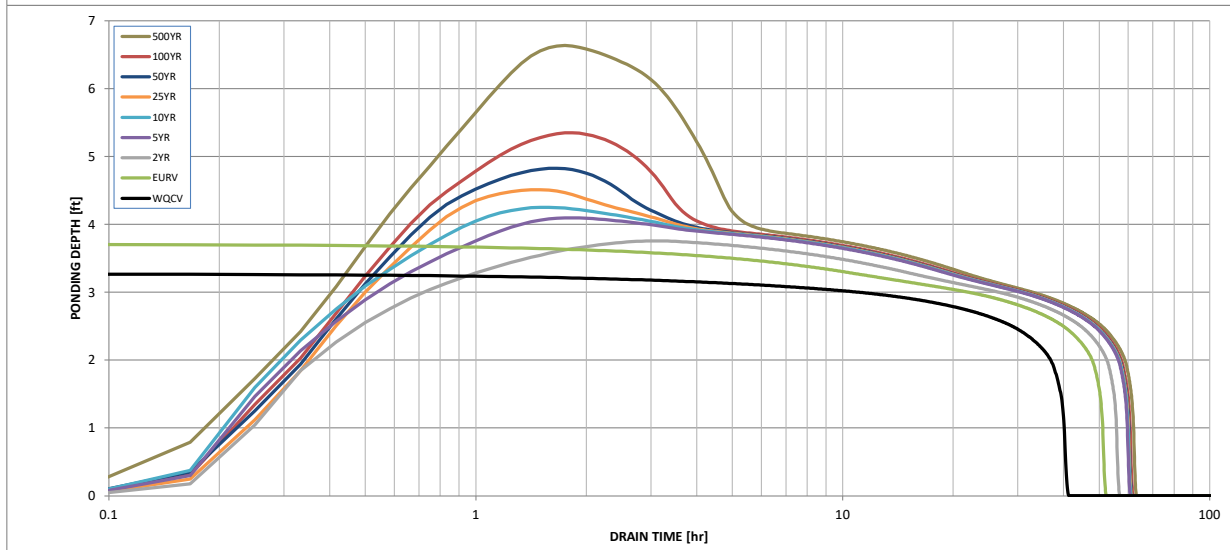
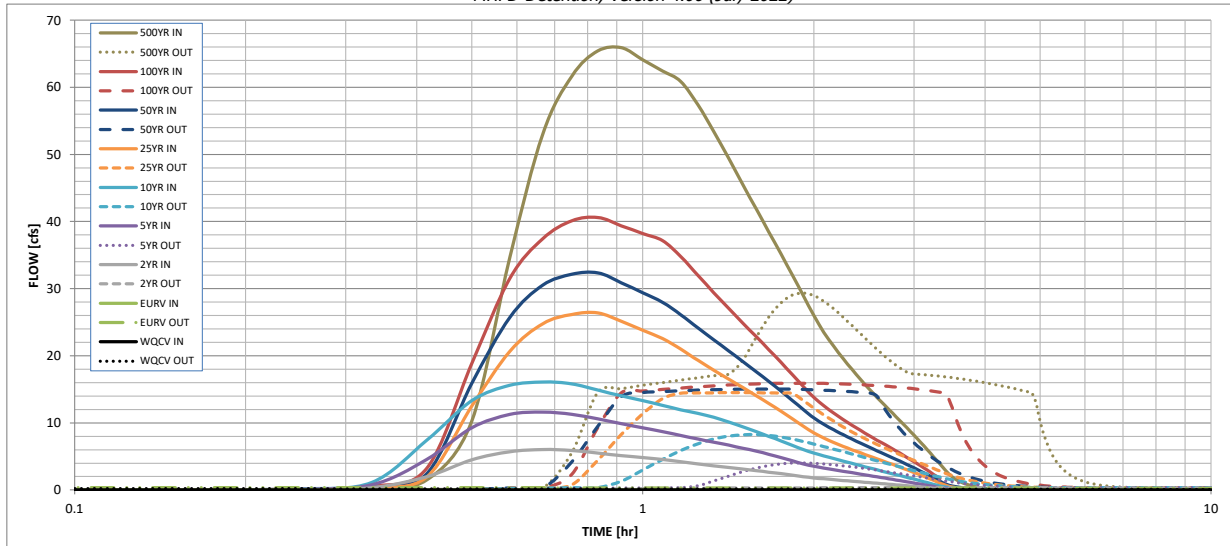
## Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.55
One-Hour Rainfall Depth (in) =	0.279	0.527	0.629	1.236	1.825	2.823	3.522	4.509	7.573
CUHP Runoff Volume (acre-ft) =	N/A	N/A	0.629	1.236	1.825	2.823	3.522	4.509	7.573
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	2.9	8.1	12.4	22.7	28.6	36.6	61.3
OPTIONAL Predevelopment Peak Q (cfs) =	N/A	N/A							
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.08	0.23	0.35	0.64	0.81	1.04	1.74
Peak Inflow Q (cfs) =	N/A	N/A	6.0	11.6	16.1	26.4	32.4	40.6	65.9
Peak Outflow Q (cfs) =	0.2	0.3	0.3	4.0	8.3	14.5	15.1	15.9	29.3
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.5	0.7	0.6	0.5	0.4	0.5
Structure Controlling Flow =	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Outlet Plate 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	0.2	0.5	0.8	0.9	0.9	1.0
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	48	52	52	49	45	41	37	26
Time to Drain 99% of Inflow Volume (hours) =	40	50	54	57	56	54	53	52	48
Maximum Ponding Depth (ft) =	3.27	3.71	3.75	4.09	4.25	4.51	4.83	5.35	6.63
Area at Maximum Ponding Depth (acres) =	0.46	0.67	0.69	0.85	0.93	1.03	1.12	1.28	1.45
Maximum Volume Stored (acre-ft) =	0.280	0.530	0.558	0.820	0.954	1.210	1.555	2.178	3.948

# DETENTION BASIN OUTLET STRUCTURE DESIGN

*MHFD-Detention, Version 4.06 (July 2022)*



S-A-V-D Chart AXIS Override	X-axis	Left Y-axis	Right Y-axis
minimum bound			
maximum bound			

# DETENTION BASIN OUTLET STRUCTURE DESIGN

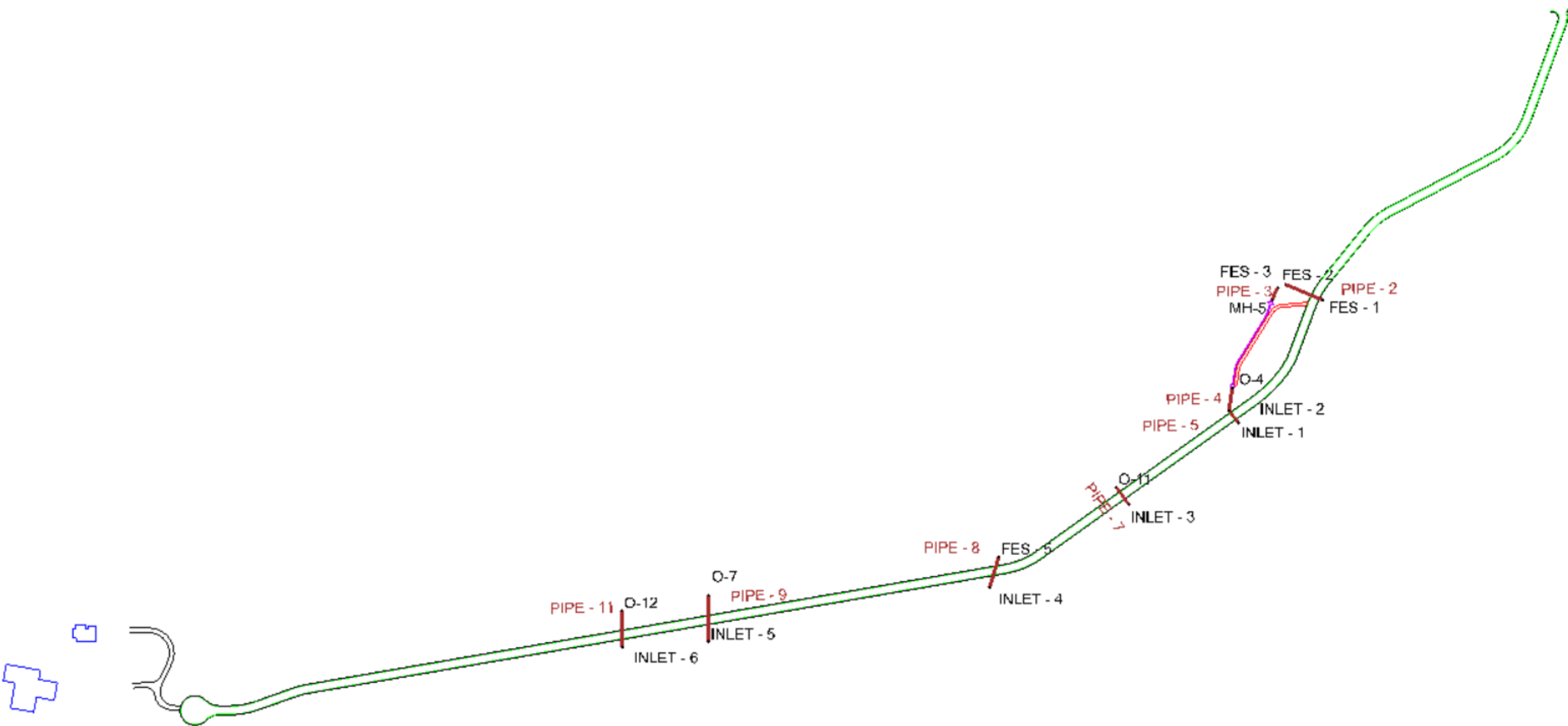
Outflow Hydrograph Workbook Filename: ..\Outflow Hydrographs.xlsx

**Inflow Hydrographs**

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

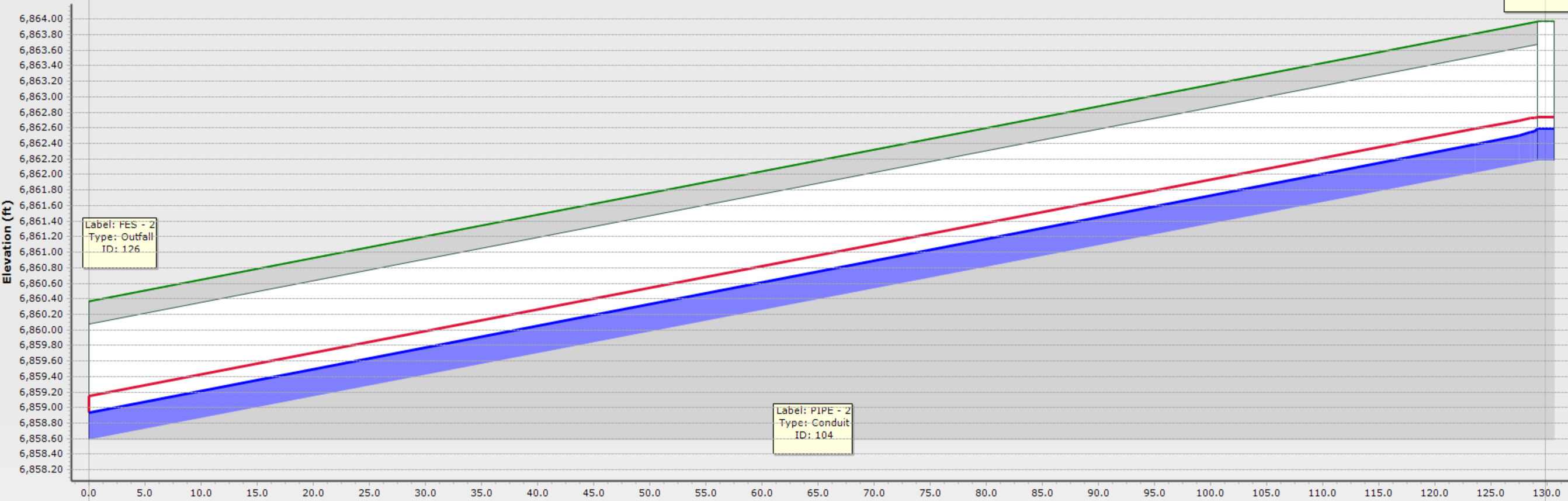
Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]	
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	
	0:15:00	0.00	0.00	0.08	0.13	0.17	0.11	0.14	0.14	0.14	0.26
	0:20:00	0.00	0.00	0.33	0.77	1.13	0.34	0.41	0.56	0.56	1.48
	0:25:00	0.00	0.00	1.99	4.56	7.40	1.93	2.42	3.20	3.20	10.28
	0:30:00	0.00	0.00	4.49	9.28	13.31	12.49	15.88	18.87	18.87	34.66
	0:35:00	0.00	0.00	5.71	11.25	15.59	20.67	25.74	31.52	31.52	53.05
	0:40:00	0.00	0.00	6.02	11.61	16.09	24.78	30.51	37.45	37.45	61.68
	0:45:00	0.00	0.00	5.87	11.27	15.81	26.16	32.13	40.11	40.11	65.41
	0:50:00	0.00	0.00	5.50	10.59	14.88	26.38	32.36	40.61	40.61	65.89
	0:55:00	0.00	0.00	5.12	9.87	14.01	25.12	30.88	39.36	39.36	64.10
	1:00:00	0.00	0.00	4.82	9.27	13.31	23.79	29.38	38.20	38.20	62.42
	1:05:00	0.00	0.00	4.54	8.68	12.62	22.54	27.96	37.15	37.15	60.85
	1:10:00	0.00	0.00	4.21	8.12	11.95	20.98	26.12	34.77	34.77	57.37
	1:15:00	0.00	0.00	3.88	7.58	11.43	19.30	24.14	31.97	31.97	53.41
	1:20:00	0.00	0.00	3.60	7.11	10.83	17.84	22.36	29.40	29.40	49.38
	1:25:00	0.00	0.00	3.35	6.67	10.13	16.53	20.72	27.05	27.05	45.51
	1:30:00	0.00	0.00	3.12	6.24	9.41	15.26	19.15	24.86	24.86	41.86
	1:35:00	0.00	0.00	2.89	5.81	8.70	14.05	17.63	22.85	22.85	38.46
	1:40:00	0.00	0.00	2.66	5.33	8.00	12.88	16.17	20.91	20.91	35.19
	1:45:00	0.00	0.00	2.44	4.84	7.31	11.73	14.74	19.02	19.02	32.02
	1:50:00	0.00	0.00	2.21	4.34	6.64	10.60	13.33	17.18	17.18	28.94
	1:55:00	0.00	0.00	1.99	3.88	6.00	9.50	11.97	15.40	15.40	26.02
	2:00:00	0.00	0.00	1.80	3.53	5.50	8.49	10.73	13.79	13.79	23.48
	2:05:00	0.00	0.00	1.66	3.27	5.08	7.72	9.77	12.53	12.53	21.41
	2:10:00	0.00	0.00	1.54	3.02	4.69	7.08	8.96	11.47	11.47	19.60
	2:15:00	0.00	0.00	1.42	2.79	4.32	6.52	8.24	10.52	10.52	17.97
	2:20:00	0.00	0.00	1.31	2.57	3.97	6.00	7.58	9.66	9.66	16.47
	2:25:00	0.00	0.00	1.21	2.36	3.63	5.53	6.98	8.87	8.87	15.09
	2:30:00	0.00	0.00	1.10	2.16	3.31	5.07	6.40	8.12	8.12	13.79
	2:35:00	0.00	0.00	1.00	1.96	3.00	4.64	5.85	7.43	7.43	12.58
	2:40:00	0.00	0.00	0.91	1.77	2.70	4.22	5.32	6.77	6.77	11.44
	2:45:00	0.00	0.00	0.82	1.58	2.42	3.81	4.80	6.13	6.13	10.33
	2:50:00	0.00	0.00	0.72	1.40	2.15	3.41	4.29	5.49	5.49	9.24
	2:55:00	0.00	0.00	0.63	1.22	1.88	3.01	3.79	4.85	4.85	8.16
	3:00:00	0.00	0.00	0.54	1.04	1.62	2.61	3.29	4.22	4.22	7.08
	3:05:00	0.00	0.00	0.45	0.87	1.36	2.21	2.79	3.58	3.58	6.01
	3:10:00	0.00	0.00	0.36	0.69	1.10	1.82	2.30	2.95	2.95	4.94
	3:15:00	0.00	0.00	0.28	0.52	0.84	1.43	1.80	2.33	2.33	3.88
	3:20:00	0.00	0.00	0.19	0.36	0.60	1.04	1.32	1.71	1.71	2.84
	3:25:00	0.00	0.00	0.13	0.25	0.44	0.67	0.86	1.13	1.13	1.96
	3:30:00	0.00	0.00	0.09	0.19	0.35	0.45	0.60	0.77	0.77	1.40
	3:35:00	0.00	0.00	0.07	0.15	0.29	0.31	0.43	0.54	0.54	1.03
	3:40:00	0.00	0.00	0.06	0.12	0.23	0.23	0.31	0.38	0.38	0.75
	3:45:00	0.00	0.00	0.05	0.10	0.19	0.16	0.23	0.26	0.26	0.53
	3:50:00	0.00	0.00	0.04	0.08	0.15	0.12	0.17	0.18	0.18	0.37
	3:55:00	0.00	0.00	0.03	0.06	0.12	0.09	0.13	0.11	0.11	0.25
	4:00:00	0.00	0.00	0.03	0.05	0.09	0.07	0.09	0.07	0.07	0.17
	4:05:00	0.00	0.00	0.02	0.04	0.07	0.05	0.07	0.06	0.06	0.13
	4:10:00	0.00	0.00	0.02	0.03	0.05	0.04	0.05	0.05	0.05	0.10
	4:15:00	0.00	0.00	0.01	0.02	0.04	0.03	0.04	0.04	0.04	0.08
	4:20:00	0.00	0.00	0.01	0.01	0.03	0.02	0.03	0.03	0.03	0.06
	4:25:00	0.00	0.00	0.01	0.01	0.02	0.02	0.02	0.02	0.02	0.05
	4:30:00	0.00	0.00	0.01	0.01	0.01	0.01	0.02	0.02	0.02	0.03
	4:35:00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.02
	4:40:00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.01
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	

# StormCAD LAYOUT



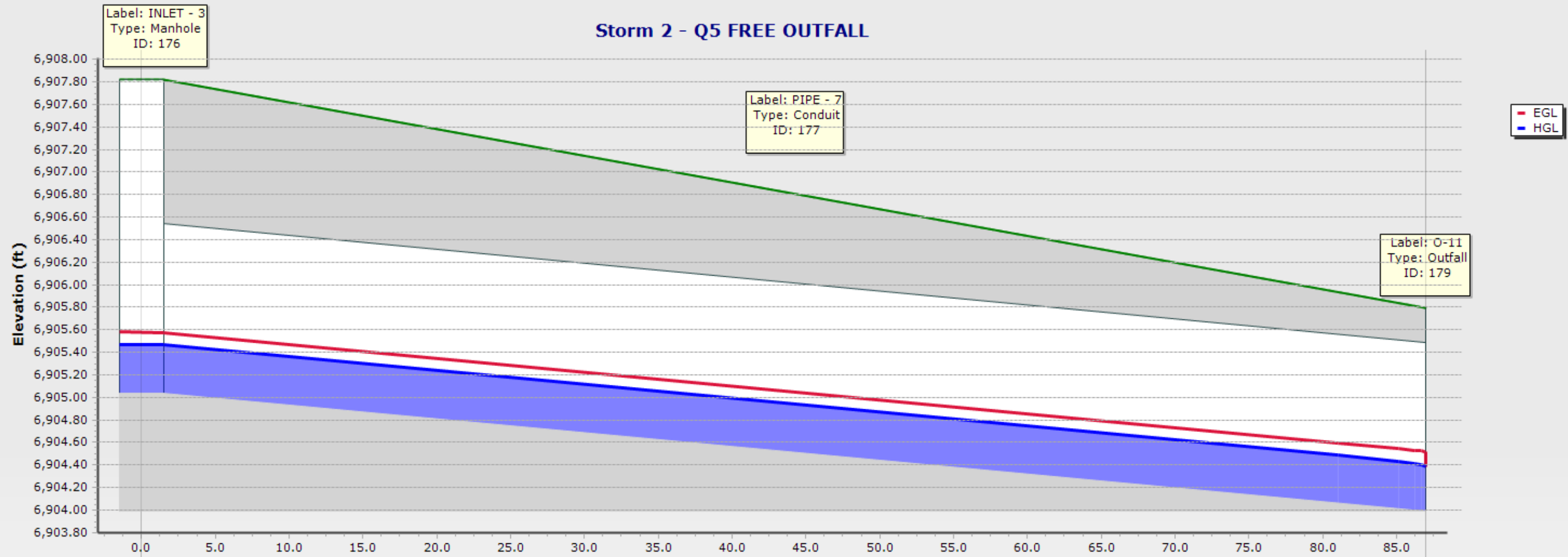


Storm 1 - Q5 FREE OUTFALL



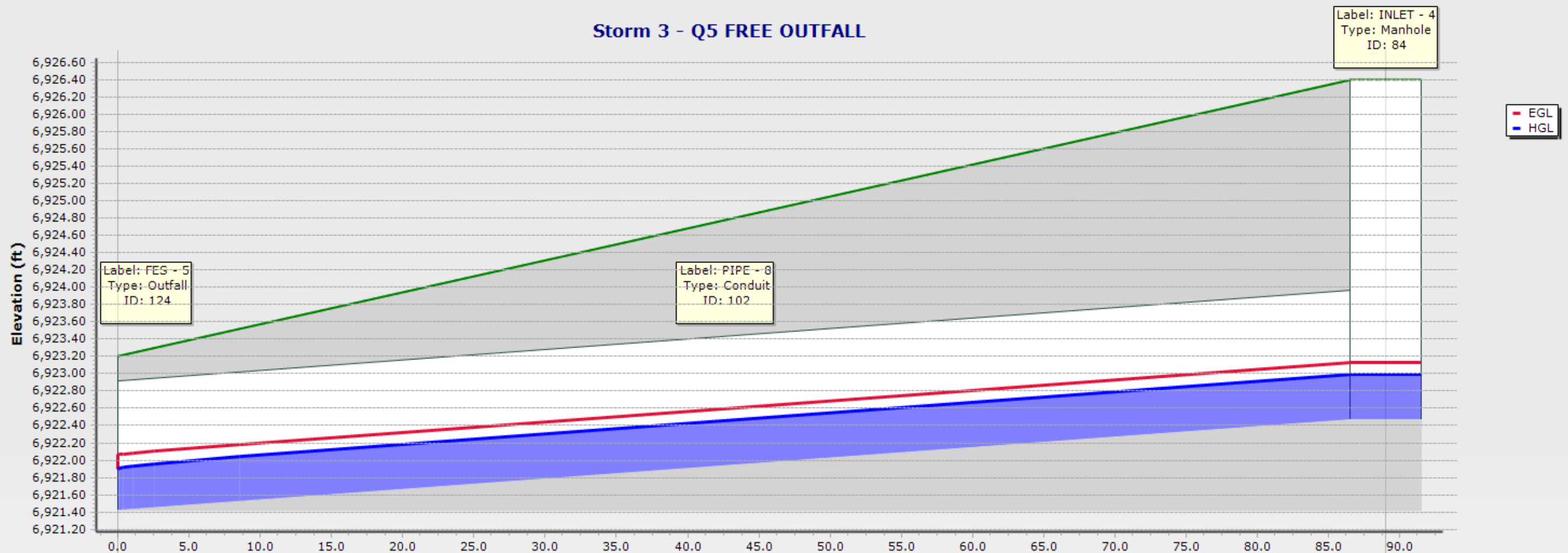
ID\Label	104 \ PIPE - 2	120 \ FES - 1
Link Length (ft)	129.9	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.20	
Slope (ft/ft)	0.028	
ID\Label	126 \ FES - 2	120 \ FES - 1
Ground (ft)	6860.37	6863.97
Invert (ft)	6858.58	6862.18
Station (ft)	0.0	129.9

### Storm 2 - Q5 FREE OUTFALL



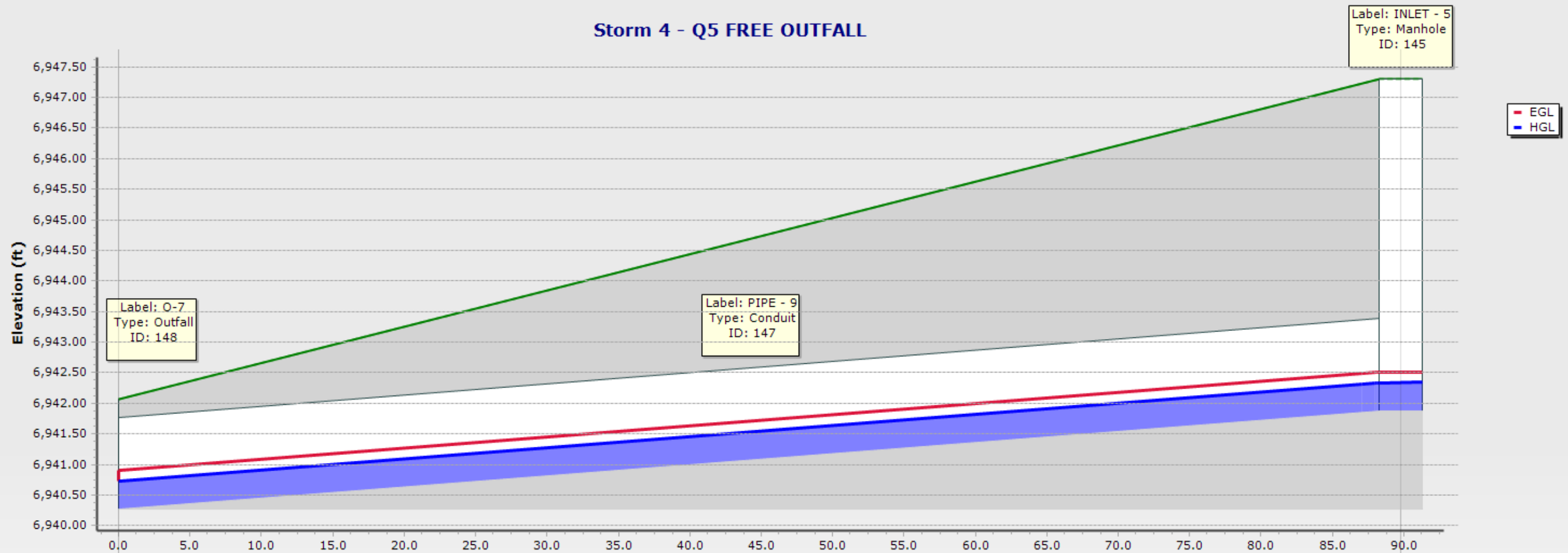
ID\Label	177 \ PIPE - 7	
Link Length (ft)	87.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.10	
Slope (ft/ft)	0.012	
ID\Label	176 \ INLET - 3	179 \ O-11
Ground (ft)	6907.82	6905.79
Invert (ft)	6905.04	6903.99
Station (ft)	0.0	87.0

### Storm 3 - Q5 FREE OUTFALL



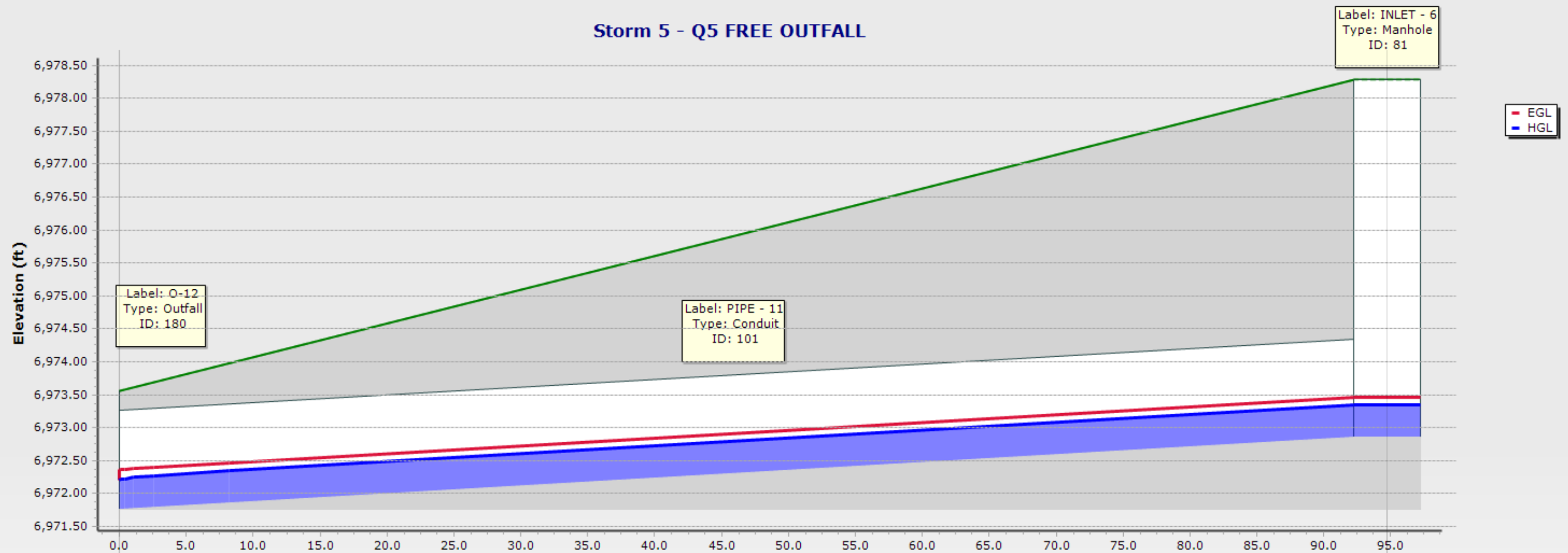
ID\Label	102 \ PIPE - 8	84 \ INLET - 4
Link Length (ft)	89.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.60	
Slope (ft/ft)	0.012	
ID\Label	124 \ FES - 5	84 \ INLET - 4
Ground (ft)	6923.21	6926.40
Invert (ft)	6921.42	6922.47
Station (ft)	0.0	89.0

### Storm 4 - Q5 FREE OUTFALL



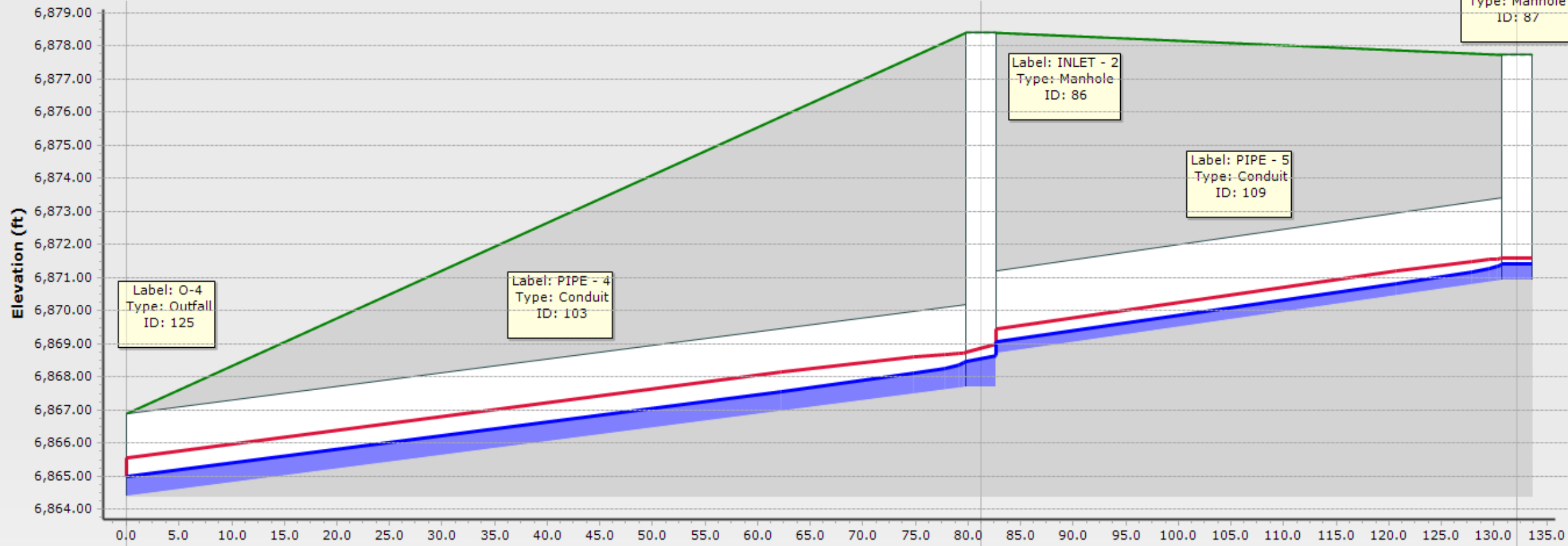
ID\Label	147 \ PIPE - 9	
Link Length (ft)	89.8	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.50	
Slope (ft/ft)	0.018	
ID\Label	148 \ O-7	145 \ INLET - 5
Ground (ft)	6942.06	6947.30
Invert (ft)	6940.27	6941.88
Station (ft)	0.0	89.8

### Storm 5 - Q5 FREE OUTFALL



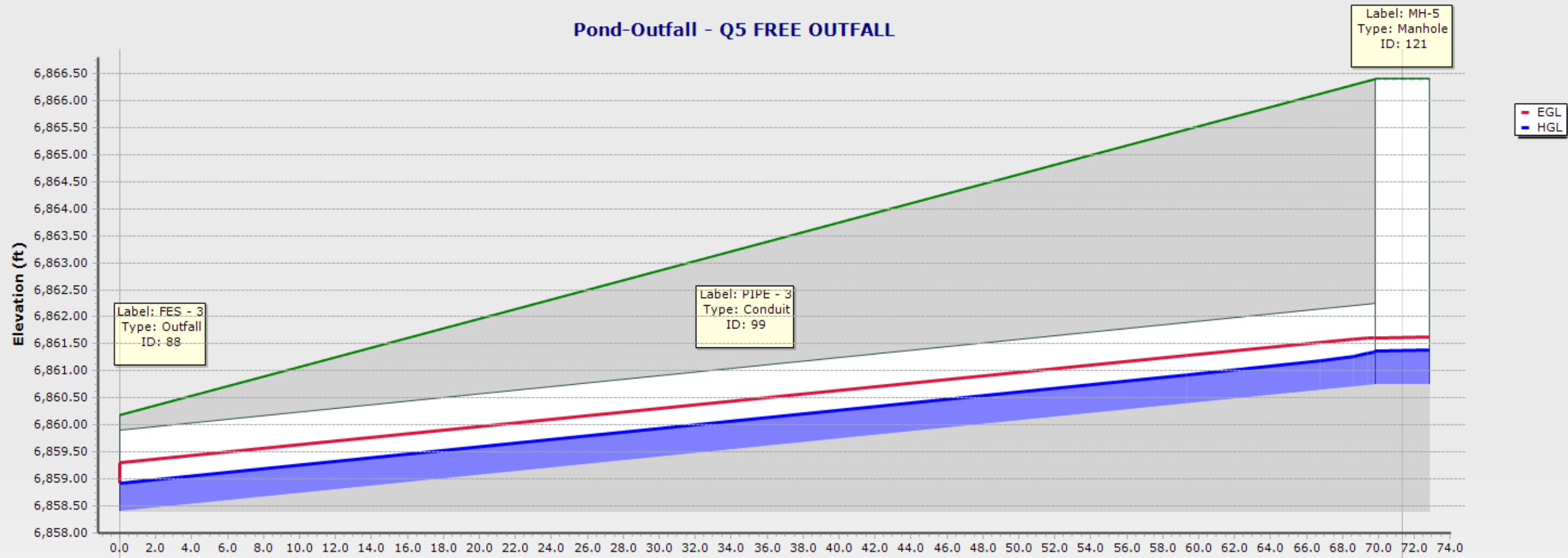
ID\Label	101 \ PIPE - 11	
Link Length (ft)	94.7	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.40	
Slope (ft/ft)	0.012	
ID\Label	80 \ O-12	81 \ INLET - 6
Ground (ft)	6973.55	6978.28
Invert (ft)	6971.76	6972.85
Station (ft)	0.0	94.7

### Pond Inlet - Q5 FREE OUTFALL



ID\Label	103 \ PIPE - 4	109 \ PIPE - 5	
Link Length (ft)	81.2	50.9	
Rise (in)\Material	30.0 \ CMP	30.0 \ CMP	
Flow (cfs)	5.30	2.30	
Slope (ft/ft)	0.041	0.043	
ID\Label	125 \ O-4	86 \ INLET - 2	87 \ INLET - 1
Ground (ft)	6866.90	6878.40	6877.73
Invert (ft)	6864.40	6867.70	6870.91
Station (ft)	0.0	81.2	132.2

### Pond-Outfall - Q5 FREE OUTFALL



ID\Label	99 \ PIPE - 3	
Link Length (ft)	71.3	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	2.70	
Slope (ft/ft)	0.033	
ID\Label	88 \ FES - 3	121 \ MH-5
Ground (ft)	6860.19	6866.40
Invert (ft)	6858.40	6860.75
Station (ft)	0.0	71.3



	Label ▲	Velocity (ft/s)	Start Node	Invert (Start) (ft)	Hydraulic Grade Line (In) (ft)	Invert (Stop) (ft)	Hydraulic Grade Line (Out) (ft)	Stop Node	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Flow (cfs)	Capacity (Full Flow) (cfs)	Depth (Normal) / Rise (%)	Diameter (in)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)
104: PIPE - 2	PIPE - 2	3.67	FES - 1	6,862.18	6,862.59	6,858.58	6,858.94	FES - 2	129.9	0.028	0.024	1.20	9.47	24.0	18.0	6,863.97	6,860.37
99: PIPE - 3	PIPE - 3	4.92	MH-5	6,860.75	6,861.37	6,858.40	6,858.92	FES - 3	71.3	0.033	0.024	2.70	10.33	34.9	18.0	6,866.40	6,860.19
103: PIPE - 4	PIPE - 4	6.13	INLET - 2	6,867.70	6,868.46	6,864.40	6,864.98	O-4	81.2	0.041	0.024	5.30	44.78	23.2	30.0	6,878.40	6,866.90
109: PIPE - 5	PIPE - 5	4.90	INLET - 1	6,870.91	6,871.41	6,868.70	6,869.08	INLET - 2	50.9	0.043	0.024	2.30	46.27	15.2	30.0	6,877.73	6,878.40
177: PIPE - 7	PIPE - 7	2.67	INLET - 3	6,905.04	6,905.47	6,903.99	6,904.38	O-11	87.0	0.012	0.024	1.10	6.25	28.4	18.0	6,907.82	6,905.79
102: PIPE - 8	PIPE - 8	2.94	INLET - 4	6,922.47	6,922.99	6,921.42	6,921.90	FES - 5	89.0	0.012	0.024	1.60	6.18	34.7	18.0	6,926.40	6,923.21
147: PIPE - 9	PIPE - 9	3.35	INLET - 5	6,941.88	6,942.34	6,940.27	6,940.72	O-7	89.8	0.018	0.024	1.50	7.62	30.1	18.0	6,947.30	6,942.06
101: PIPE - 11	PIPE - 11	2.80	INLET - 6	6,972.85	6,973.34	6,971.76	6,972.20	O-12	94.7	0.012	0.024	1.40	6.10	32.6	18.0	6,978.28	6,973.55

Figure 1- Q5 – Free Outfall CONDUIT SUMMARY

	Label ▲	Flow (Known) (cfs)	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Headloss Method	Headloss Coefficient (Standard)	Flow (Total Out) (cfs)
120: FES - 1	FES - 1	1.20	6,863.97	6,863.97	0.41	6,862.60	6,862.59	Standard	0.050	1.20
87: INLET - 1	INLET - 1	2.30	6,877.73	6,877.73	0.50	6,871.41	6,871.41	Standard	0.050	2.30
86: INLET - 2	INLET - 2	5.30	6,878.40	6,878.40	0.76	6,868.64	6,868.46	Standard	0.640	5.30
176: INLET - 3	INLET - 3	1.10	6,907.82	6,907.82	0.43	6,905.47	6,905.47	Standard	0.050	1.10
84: INLET - 4	INLET - 4	1.60	6,926.40	6,926.40	0.52	6,923.00	6,922.99	Standard	0.050	1.60
145: INLET - 5	INLET - 5	1.50	6,947.30	6,947.30	0.46	6,942.35	6,942.34	Standard	0.050	1.50
81: INLET - 6	INLET - 6	1.40	6,978.28	6,978.28	0.49	6,973.34	6,973.34	Standard	0.050	1.40
121: MH-5	MH-5	2.70	6,866.40	6,866.40	0.62	6,861.39	6,861.37	Standard	0.050	2.70

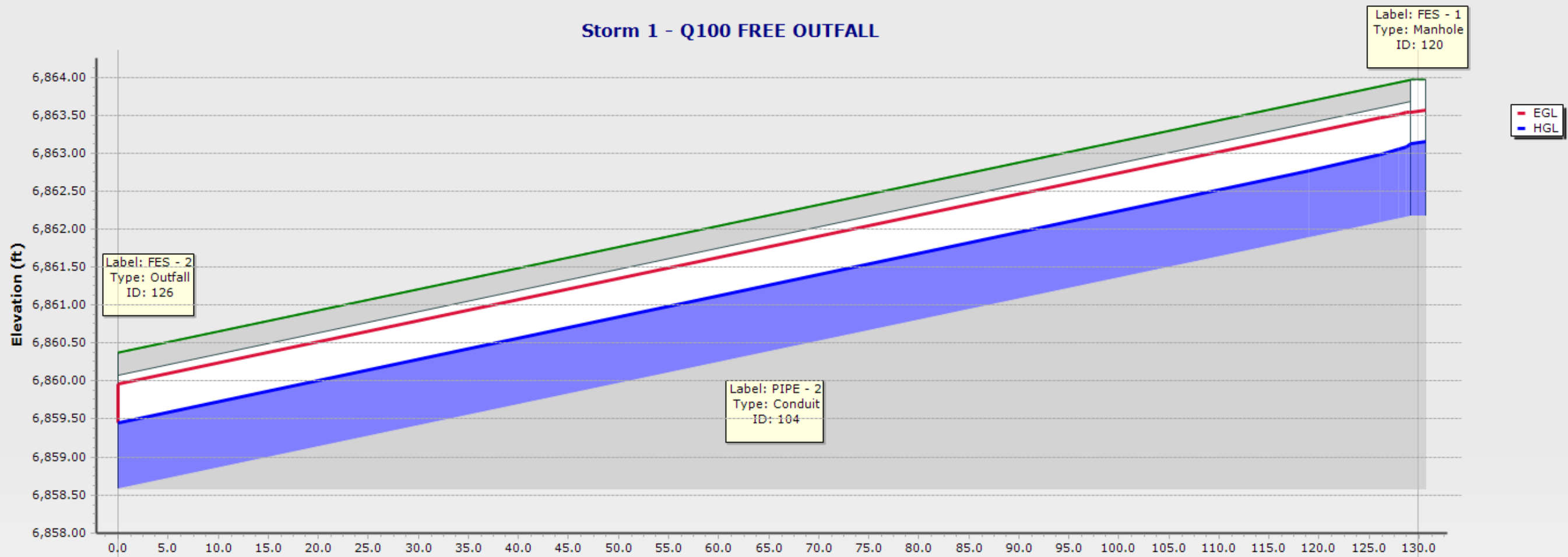
Figure 2- Q5 – Free Outfall NODE SUMMARY

	Label ▲	Notes	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Boundary Condition Type	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
126: FES - 2	FES - 2		6,860.37	6,858.58	Free Outfall	6,858.94	1.20
88: FES - 3	FES - 3	18" FES	6,860.19	6,858.40	Free Outfall	6,858.92	2.70
124: FES - 5	FES - 5		6,923.21	6,921.42	Free Outfall	6,921.90	1.60
125: O-4	O-4		6,866.90	6,864.40	Free Outfall	6,864.98	5.30
148: O-7	O-7		6,942.06	6,940.27	Free Outfall	6,940.72	1.50
179: O-11	O-11	CDOT TYPE C INLET	6,905.79	6,903.99	Free Outfall	6,904.38	1.10
180: O-12	O-12		6,973.55	6,971.76	Free Outfall	6,972.20	1.40

Figure 3- Q5 – Free Outfall OUTFALL SUMMARY

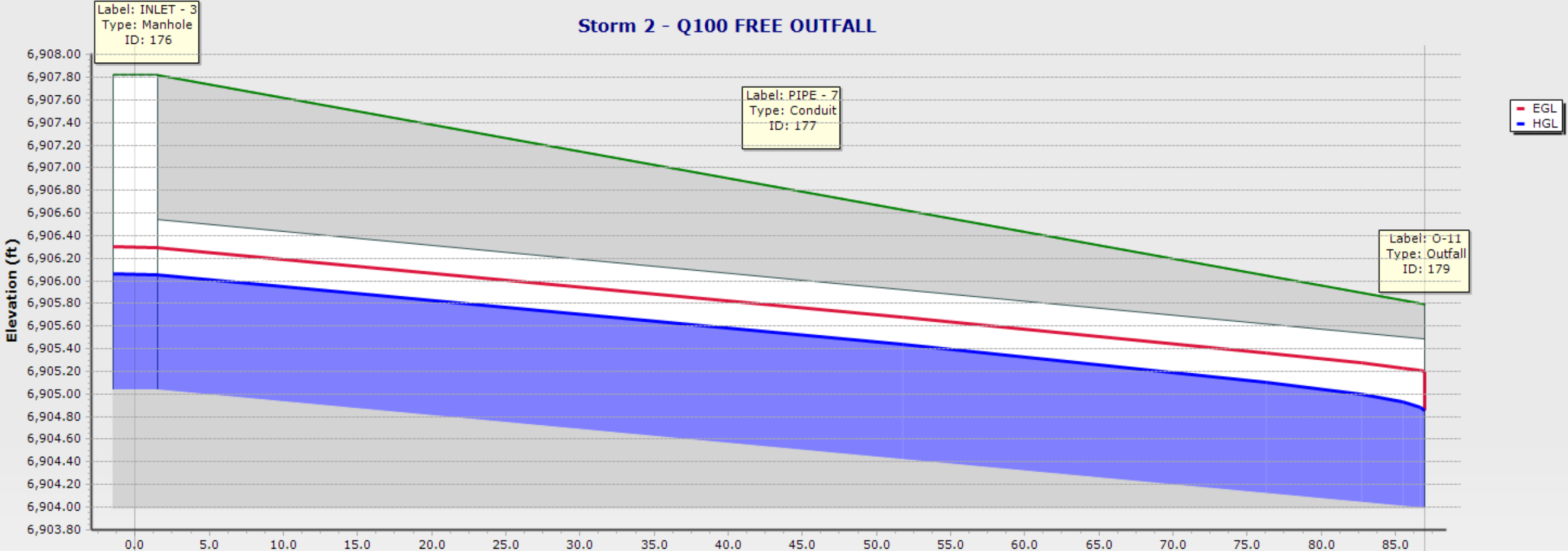


### Storm 1 - Q100 FREE OUTFALL



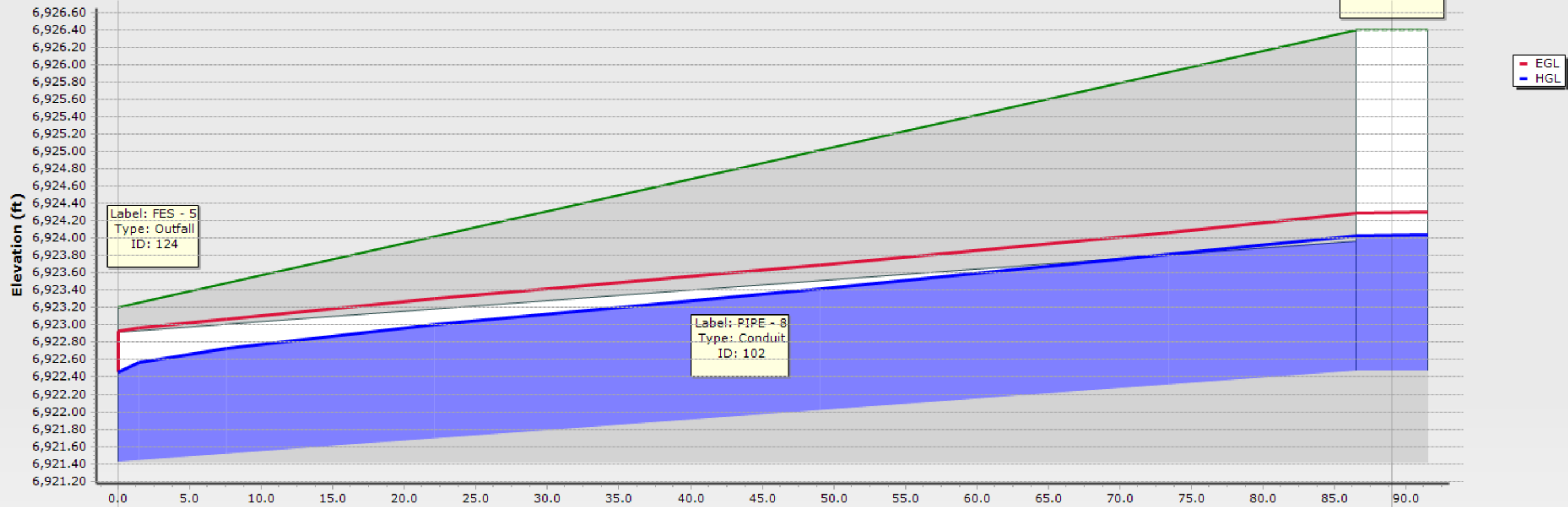
ID\Label	104 \ PIPE - 2	
Link Length (ft)	129.9	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	6.10	
Slope (ft/ft)	0.028	
ID\Label	126 \ FES - 2	120 \ FES - 1
Ground (ft)	6860.37	6863.97
Invert (ft)	6858.58	6862.18
Station (ft)	0.0	129.9

Storm 2 - Q100 FREE OUTFALL



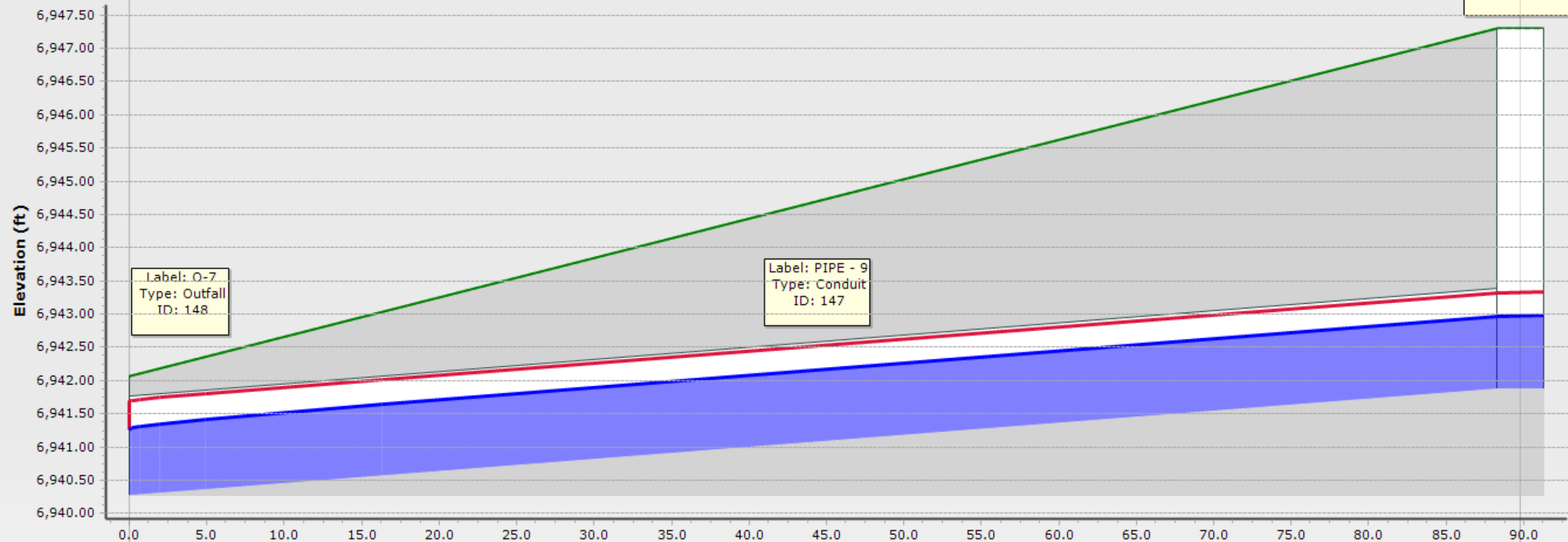
ID\Label	177 \ PIPE - 7	
Link Length (ft)	87.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	5.00	
Slope (ft/ft)	0.012	
ID\Label	176 \ INLET - 3	179 \ O-11
Ground (ft)	6907.82	6905.79
Invert (ft)	6905.04	6903.99
Station (ft)	0.0	87.0

### Storm 3 - Q100 FREE OUTFALL



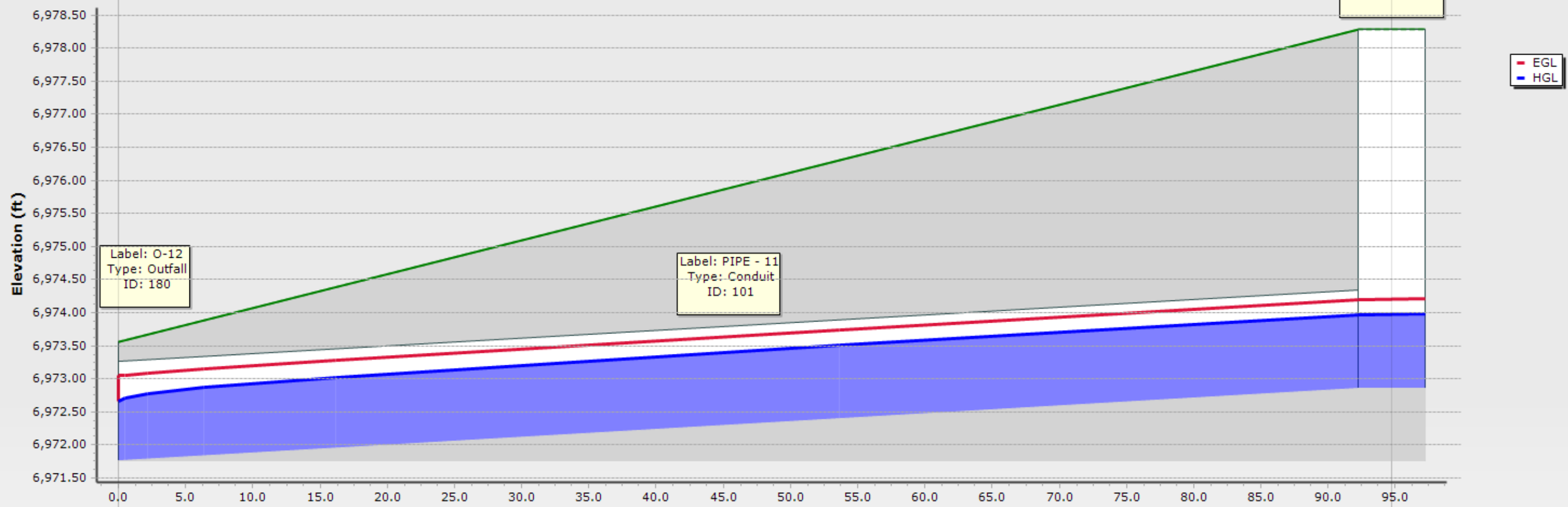
ID\Label	102 \ PIPE - 8	
Link Length (ft)	89.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	7.20	
Slope (ft/ft)	0.012	
ID\Label	124 \ FES - 5	84 \ INLET - 4
Ground (ft)	6923.21	6926.40
Invert (ft)	6921.42	6922.47
Station (ft)	0.0	89.0

### Storm 4 - Q100 FREE OUTFALL



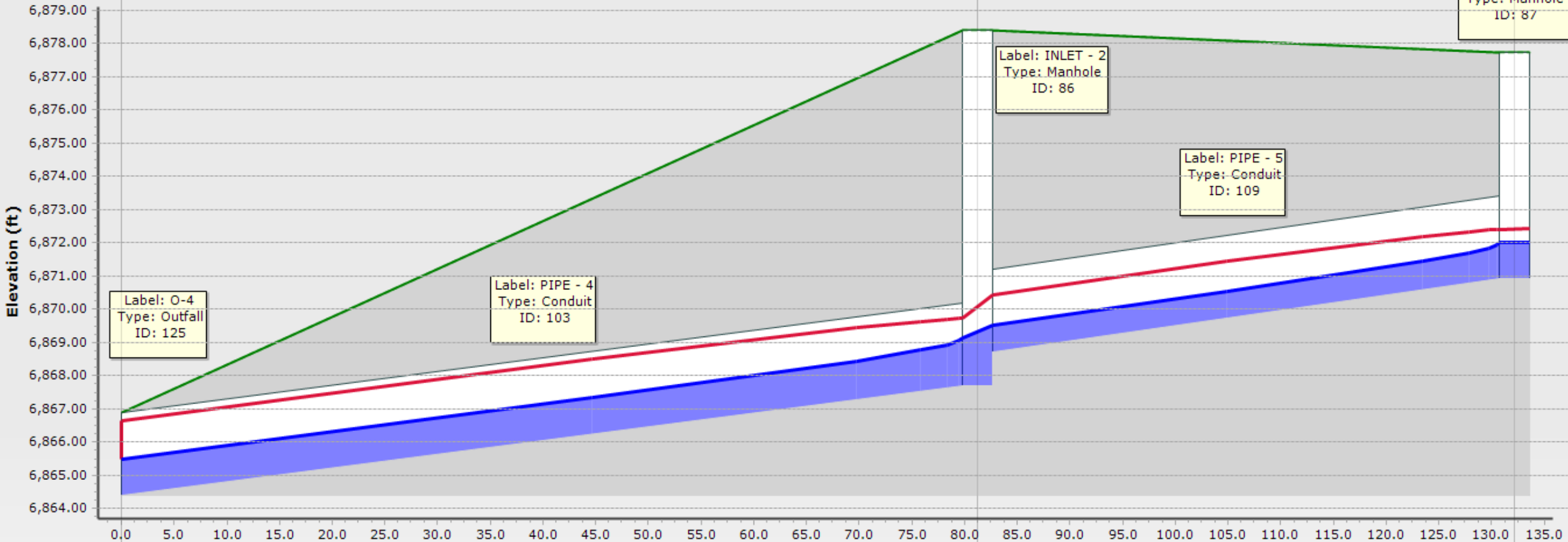
ID\Label	147 \ PIPE - 9	
Link Length (ft)	89.8	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	6.60	
Slope (ft/ft)	0.018	
ID\Label	148 \ O-7	145 \ INLET - 5
Ground (ft)	6942.06	6947.30
Invert (ft)	6940.27	6941.88
Station (ft)	0.0	89.8

### Storm 5 - Q100 FREE OUTFALL



ID\Label	101 \ PIPE - 11	
Link Length (ft)	94.7	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	5.50	
Slope (ft/ft)	0.012	
ID\Label	180 \ O-12	81 \ INLET - 6
Ground (ft)	6973.55	6978.28
Invert (ft)	6971.76	6972.85
Station (ft)	0.0	94.7

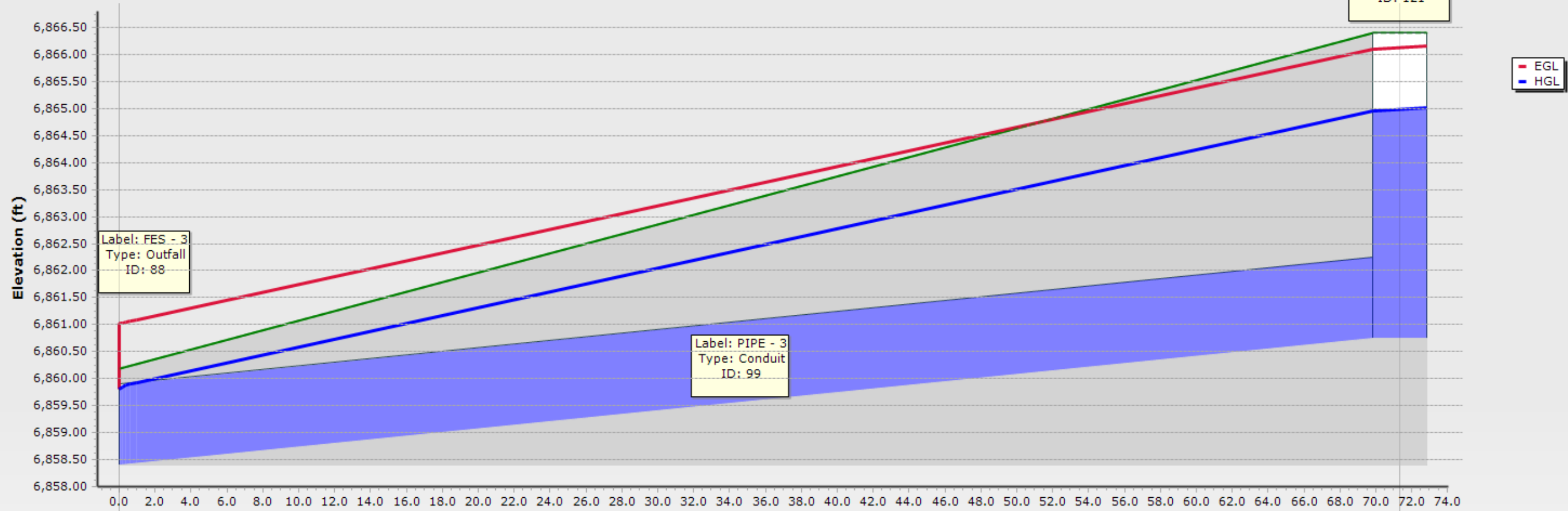
### Pond Inlet - Q100 FREE OUTFALL



— EGL  
— HGL

ID\Label	103 \ PIPE - 4	109 \ PIPE - 5	86 \ INLET - 2	87 \ INLET - 1
Link Length (ft)	81.2	50.9		
Rise (in)\Material	30.0 \ CMP	30.0 \ CMP		
Flow (cfs)	17.90	10.60		
Slope (ft/ft)	0.041	0.043		
ID\Label	125 \ O-4			
Ground (ft)	6866.90		6878.40	6877.73
Invert (ft)	6864.40		6867.70	6870.91
Station (ft)	0.0		81.2	132.2

### Pond-Outfall - Q100 FREE OUTFALL



ID\Label	99 \ PIPE - 3	
Link Length (ft)	71.3	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	15.20	
Slope (ft/ft)	0.033	
ID\Label	88 \ FES - 3	121 \ MH-5
Ground (ft)	6860.19	6866.40
Invert (ft)	6858.40	6860.75
Station (ft)	0.0	71.3



	Label ▲	Velocity (ft/s)	Start Node	Invert (Start) (ft)	Hydraulic Grade Line (In) (ft)	Invert (Stop) (ft)	Hydraulic Grade Line (Out) (ft)	Stop Node	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Flow (cfs)	Capacity (Full Flow) (cfs)	Depth (Normal) / Rise (%)	Diameter (in)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)
104: PIPE - 2	PIPE - 2	5.69	FES - 1	6,862.18	6,863.13	6,858.58	6,859.46	FES - 2	129.9	0.028	0.024	6.10	9.47	58.4	18.0	6,863.97	6,860.37
99: PIPE - 3	PIPE - 3	8.60	MH-5	6,860.75	6,864.95	6,858.40	6,859.81	FES - 3	71.3	0.033	0.024	15.20	10.33	(N/A)	18.0	6,866.40	6,860.19
103: PIPE - 4	PIPE - 4	8.61	INLET - 2	6,867.70	6,869.13	6,864.40	6,865.50	O-4	81.2	0.041	0.024	17.90	44.78	44.0	30.0	6,878.40	6,866.90
109: PIPE - 5	PIPE - 5	7.65	INLET - 1	6,870.91	6,872.00	6,868.70	6,869.51	INLET - 2	50.9	0.043	0.024	10.60	46.27	32.6	30.0	6,877.73	6,878.40
177: PIPE - 7	PIPE - 7	3.93	INLET - 3	6,905.04	6,906.05	6,903.99	6,904.85	O-11	87.0	0.012	0.024	5.00	6.25	67.6	18.0	6,907.82	6,905.79
102: PIPE - 8	PIPE - 8	4.07	INLET - 4	6,922.47	6,924.03	6,921.42	6,922.46	FES - 5	89.0	0.012	0.024	7.20	6.18	(N/A)	18.0	6,926.40	6,923.21
147: PIPE - 9	PIPE - 9	4.85	INLET - 5	6,941.88	6,942.96	6,940.27	6,941.26	O-7	89.8	0.018	0.024	6.60	7.62	71.9	18.0	6,947.30	6,942.06
101: PIPE - 11	PIPE - 11	3.91	INLET - 6	6,972.85	6,973.96	6,971.76	6,972.66	O-12	94.7	0.012	0.024	5.50	6.10	74.2	18.0	6,978.28	6,973.55

Figure 4- Q100 – Free Outfall CONDUIT SUMMARY

	Label ▲	Flow (Known) (cfs)	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Headloss Method	Headloss Coefficient (Standard)	Flow (Total Out) (cfs)
120: FES - 1	FES - 1	6.10	6,863.97	6,863.97	0.95	6,863.15	6,863.13	Standard	0.050	6.10
87: INLET - 1	INLET - 1	10.60	6,877.73	6,877.73	1.09	6,872.02	6,872.00	Standard	0.050	10.60
86: INLET - 2	INLET - 2	17.90	6,878.40	6,878.40	1.43	6,869.51	6,869.13	Standard	0.640	17.90
176: INLET - 3	INLET - 3	5.00	6,907.82	6,907.82	1.01	6,906.07	6,906.05	Standard	0.050	5.00
84: INLET - 4	INLET - 4	7.20	6,926.40	6,926.40	1.56	6,924.04	6,924.03	Standard	0.050	7.20
145: INLET - 5	INLET - 5	6.60	6,947.30	6,947.30	1.08	6,942.98	6,942.96	Standard	0.050	6.60
81: INLET - 6	INLET - 6	5.50	6,978.28	6,978.28	1.11	6,973.98	6,973.96	Standard	0.050	5.50
121: MH-5	MH-5	15.20	6,866.40	6,866.40	4.20	6,865.01	6,864.95	Standard	0.050	15.20

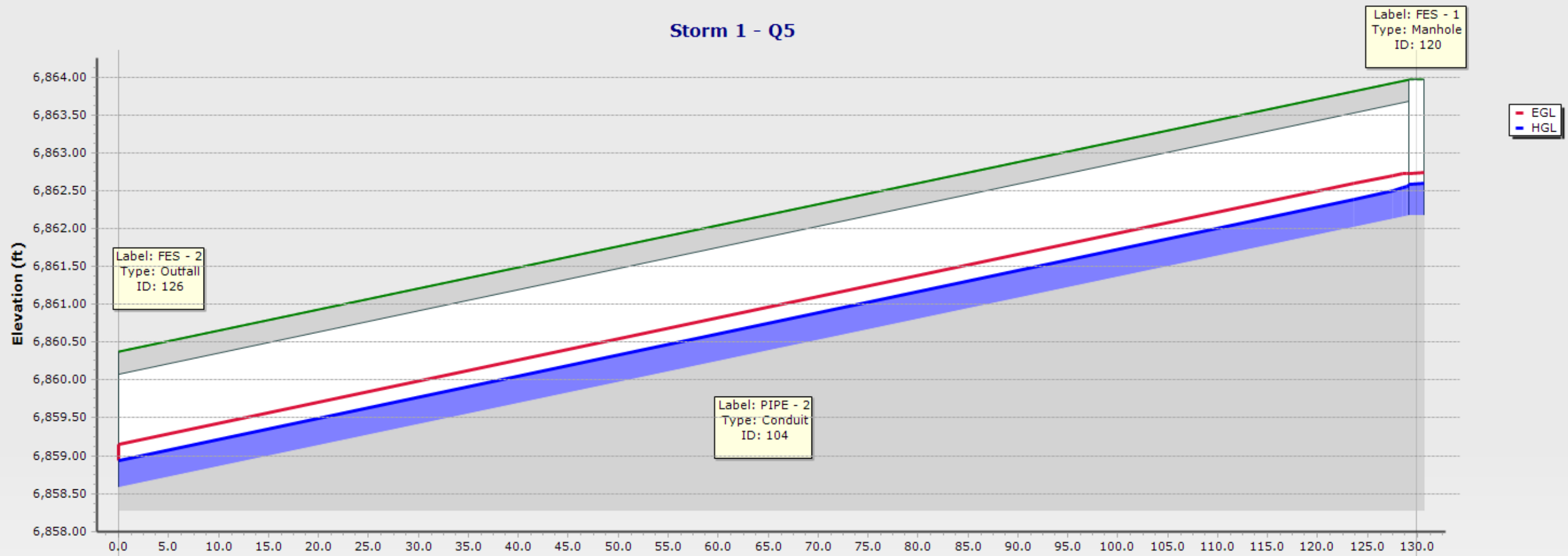
Figure 5- Q100 – Free Outfall NODE SUMMARY

	Label ▲	Notes	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Boundary Condition Type	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
126: FES - 2	FES - 2		6,860.37	6,858.58	Free Outfall	6,859.46	6.10
88: FES - 3	FES - 3	18" FES	6,860.19	6,858.40	Free Outfall	6,859.81	15.20
124: FES - 5	FES - 5		6,923.21	6,921.42	Free Outfall	6,922.46	7.20
125: O-4	O-4		6,866.90	6,864.40	Free Outfall	6,865.50	17.90
148: O-7	O-7		6,942.06	6,940.27	Free Outfall	6,941.26	6.60
179: O-11	O-11	CDOT TYPE C INLET	6,905.79	6,903.99	Free Outfall	6,904.85	5.00
180: O-12	O-12		6,973.55	6,971.76	Free Outfall	6,972.66	5.50

Figure 6- Q100 Free Outfall OUTFALL SUMMARY

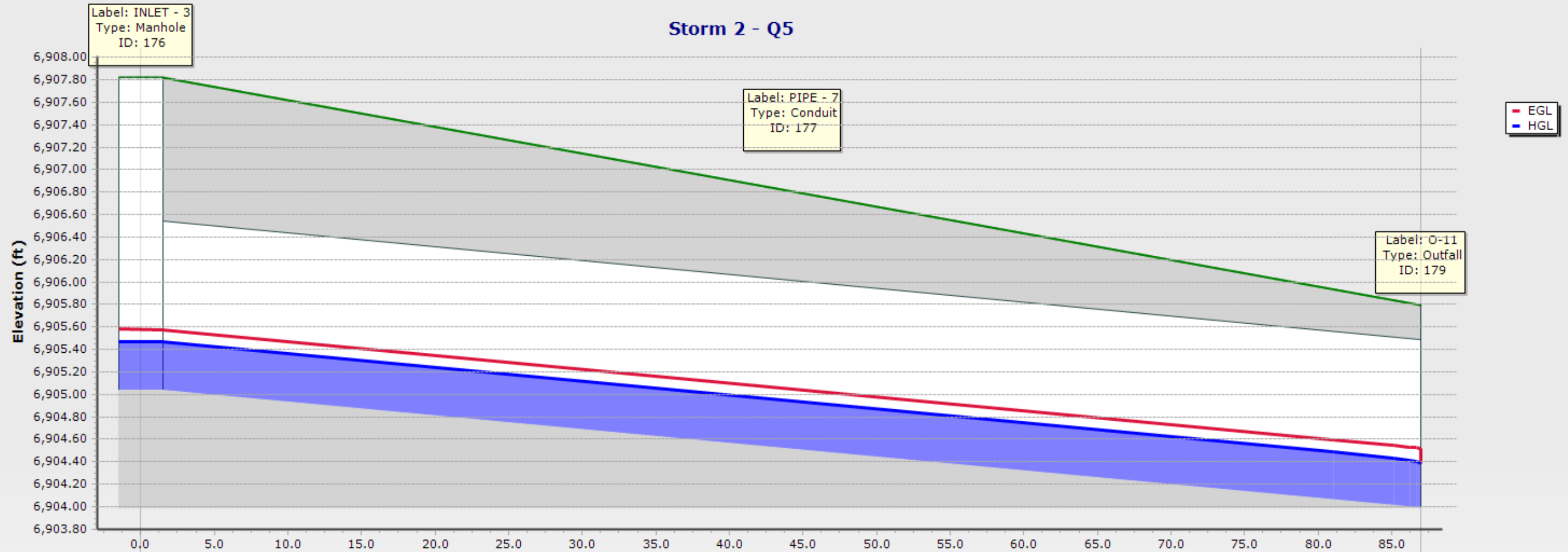


### Storm 1 - Q5



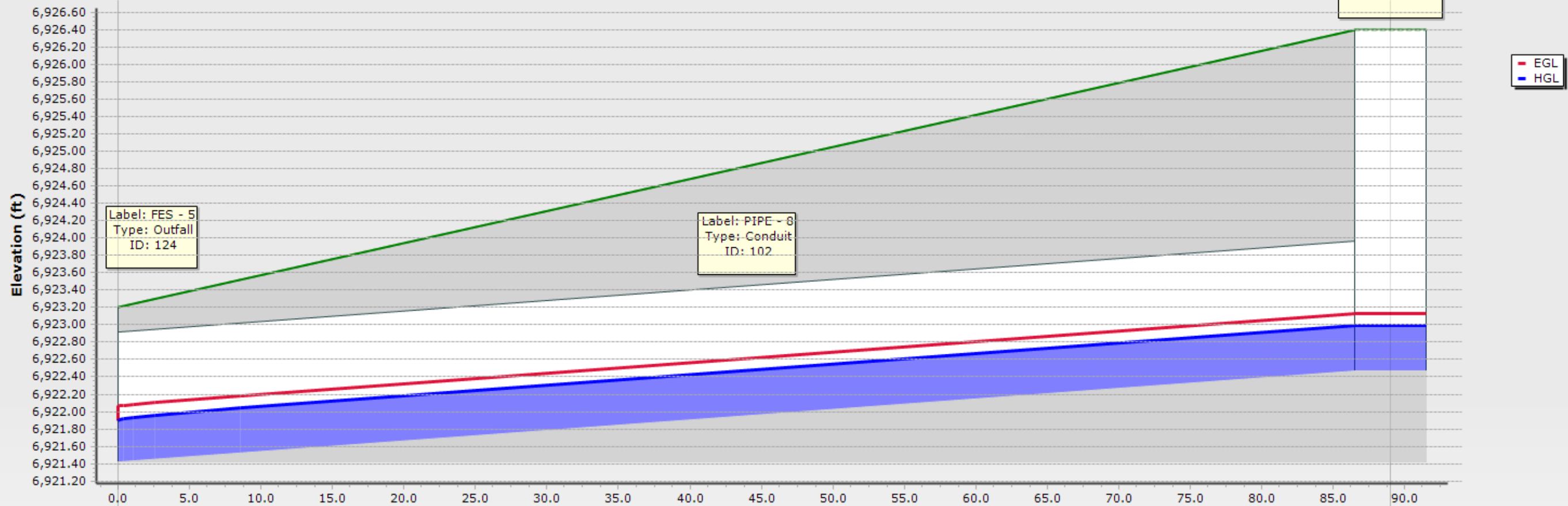
ID\Label	104 \ PIPE - 2	
Link Length (ft)	129.9	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.20	
Slope (ft/ft)	0.028	
ID\Label	126 \ FES - 2	120 \ FES - 1
Ground (ft)	6860.37	6863.97
Invert (ft)	6858.58	6862.18
Station (ft)	0.0	129.9

### Storm 2 - Q5



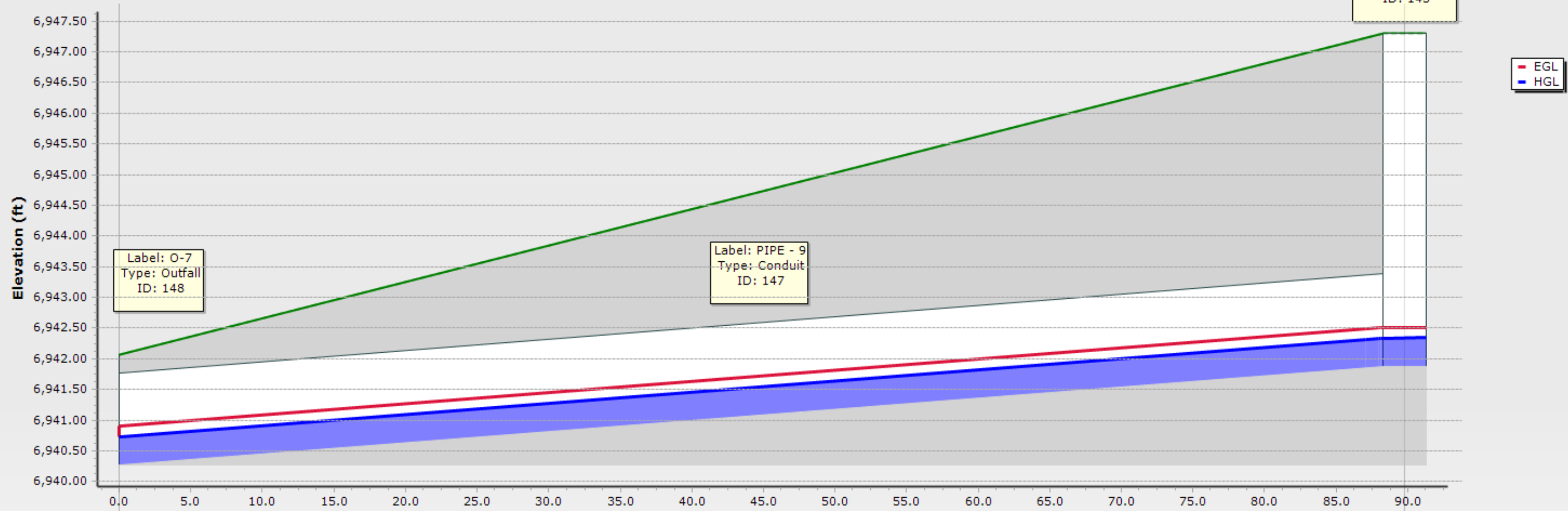
ID\Label	177 \ PIPE - 7	
Link Length (ft)	87.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.10	
Slope (ft/ft)	0.012	
ID\Label	176 \ INLET - 3	179 \ O-11
Ground (ft)	6907.82	6905.79
Invert (ft)	6905.04	6903.99
Station (ft)	0.0	87.0

### Storm 3 - Q5



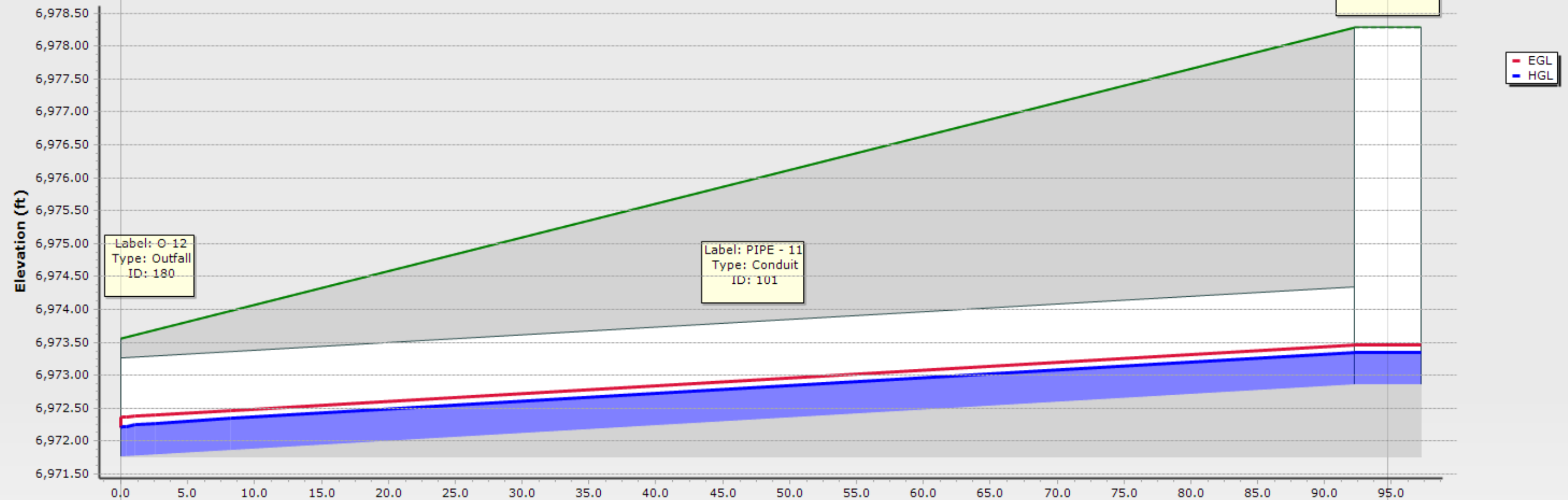
ID\Label	102 \ PIPE - 8	
Link Length (ft)	89.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.60	
Slope (ft/ft)	0.012	
ID\Label	124 \ FES - 5	84 \ INLET - 4
Ground (ft)	6923.21	6926.40
Invert (ft)	6921.42	6922.47
Station (ft)	0.0	89.0

### Storm 4 - Q5



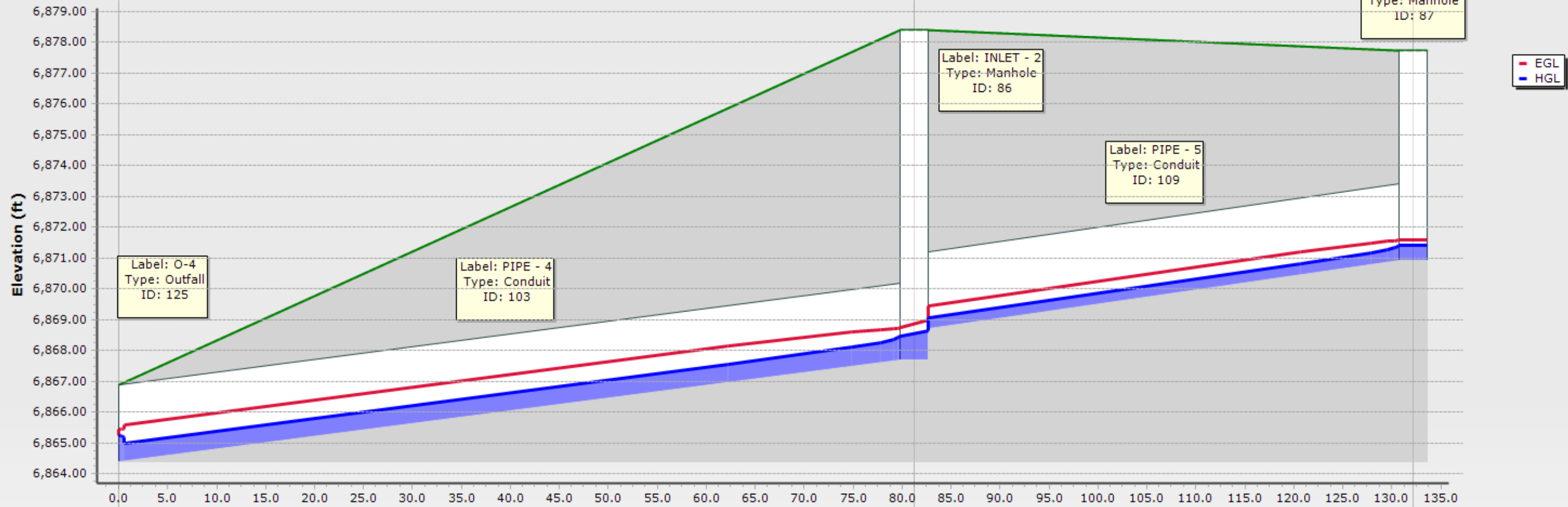
ID\Label	147 \ PIPE - 9	
Link Length (ft)	89.8	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.50	
Slope (ft/ft)	0.018	
ID\Label	148 \ O-7	145 \ INLET - 5
Ground (ft)	6942.06	6947.30
Invert (ft)	6940.27	6941.88
Station (ft)	0.0	89.8

### Storm 5 - Q5



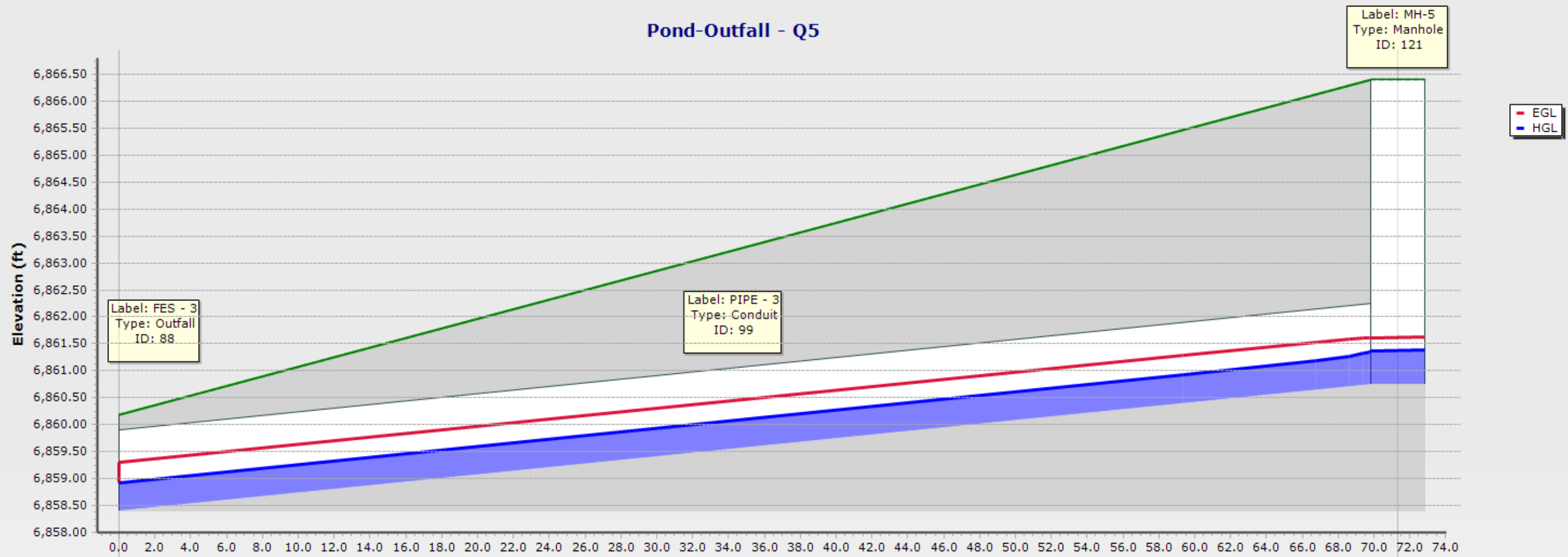
ID\Label	101 \ PIPE - 11	
Link Length (ft)	94.7	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.40	
Slope (ft/ft)	0.012	
ID\Label	80 \ O-12	81 \ INLET - 6
Ground (ft)	6973.55	6978.28
Invert (ft)	6971.76	6972.85
Station (ft)	0.0	94.7

### Pond Inlet - Q5



ID\Label	103 \ PIPE - 4	109 \ PIPE - 5	
Link Length (ft)	81.2	50.9	
Rise (in)\Material	30.0 \ CMP	30.0 \ CMP	
Flow (cfs)	5.30	2.30	
Slope (ft/ft)	0.041	0.043	
ID\Label	125 \ O-4	86 \ INLET - 2	87 \ INLET - 1
Ground (ft)	6866.90	6878.40	6877.73
Invert (ft)	6864.40	6867.70	6870.91
Station (ft)	0.0	81.2	132.2

### Pond-Outfall - Q5



ID\Label	99 \ PIPE - 3	
Link Length (ft)	71.3	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	2.70	
Slope (ft/ft)	0.033	
ID\Label	88 \ FES - 3	121 \ MH-5
Ground (ft)	6860.19	6866.40
Invert (ft)	6858.40	6860.75
Station (ft)	0.0	71.3



	Label ▲	Velocity (ft/s)	Start Node	Invert (Start) (ft)	Hydraulic Grade Line (In) (ft)	Invert (Stop) (ft)	Hydraulic Grade Line (Out) (ft)	Stop Node	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Flow (cfs)	Capacity (Full Flow) (cfs)	Depth (Normal) / Rise (%)	Diameter (in)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)
104: PIPE - 2	PIPE - 2	3.67	FES - 1	6,862.18	6,862.59	6,858.58	6,858.94	FES - 2	129.9	0.028	0.024	1.20	9.47	24.0	18.0	6,863.97	6,860.37
99: PIPE - 3	PIPE - 3	4.92	MH-5	6,860.75	6,861.37	6,858.40	6,858.92	FES - 3	71.3	0.033	0.024	2.70	10.33	34.9	18.0	6,866.40	6,860.19
103: PIPE - 4	PIPE - 4	6.13	INLET - 2	6,867.70	6,868.46	6,864.40	6,865.25	O-4	81.2	0.041	0.024	5.30	44.78	23.2	30.0	6,878.40	6,866.90
109: PIPE - 5	PIPE - 5	4.90	INLET - 1	6,870.91	6,871.41	6,868.70	6,869.08	INLET - 2	50.9	0.043	0.024	2.30	46.27	15.2	30.0	6,877.73	6,878.40
177: PIPE - 7	PIPE - 7	2.67	INLET - 3	6,905.04	6,905.47	6,903.99	6,904.38	O-11	87.0	0.012	0.024	1.10	6.25	28.4	18.0	6,907.82	6,905.79
102: PIPE - 8	PIPE - 8	2.94	INLET - 4	6,922.47	6,922.99	6,921.42	6,921.90	FES - 5	89.0	0.012	0.024	1.60	6.18	34.7	18.0	6,926.40	6,923.21
147: PIPE - 9	PIPE - 9	3.35	INLET - 5	6,941.88	6,942.34	6,940.27	6,940.72	O-7	89.8	0.018	0.024	1.50	7.62	30.1	18.0	6,947.30	6,942.06
101: PIPE - 11	PIPE - 11	2.80	INLET - 6	6,972.85	6,973.34	6,971.76	6,972.20	O-12	94.7	0.012	0.024	1.40	6.10	32.6	18.0	6,978.28	6,973.55

Figure 7- Q5 CONDUIT SUMMARY

	Label ▲	Flow (Known) (cfs)	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Headloss Method	Headloss Coefficient (Standard)	Flow (Total Out) (cfs)
120: FES - 1	FES - 1	1.20	6,863.97	6,863.97	0.41	6,862.60	6,862.59	Standard	0.050	1.20
87: INLET - 1	INLET - 1	2.30	6,877.73	6,877.73	0.50	6,871.41	6,871.41	Standard	0.050	2.30
86: INLET - 2	INLET - 2	5.30	6,878.40	6,878.40	0.76	6,868.64	6,868.46	Standard	0.640	5.30
176: INLET - 3	INLET - 3	1.10	6,907.82	6,907.82	0.43	6,905.47	6,905.47	Standard	0.050	1.10
84: INLET - 4	INLET - 4	1.60	6,926.40	6,926.40	0.52	6,923.00	6,922.99	Standard	0.050	1.60
145: INLET - 5	INLET - 5	1.50	6,947.30	6,947.30	0.46	6,942.35	6,942.34	Standard	0.050	1.50
81: INLET - 6	INLET - 6	1.40	6,978.28	6,978.28	0.49	6,973.34	6,973.34	Standard	0.050	1.40
121: MH-5	MH-5	2.70	6,866.40	6,866.40	0.62	6,861.39	6,861.37	Standard	0.050	2.70

Figure 8- Q5 NODE SUMMARY

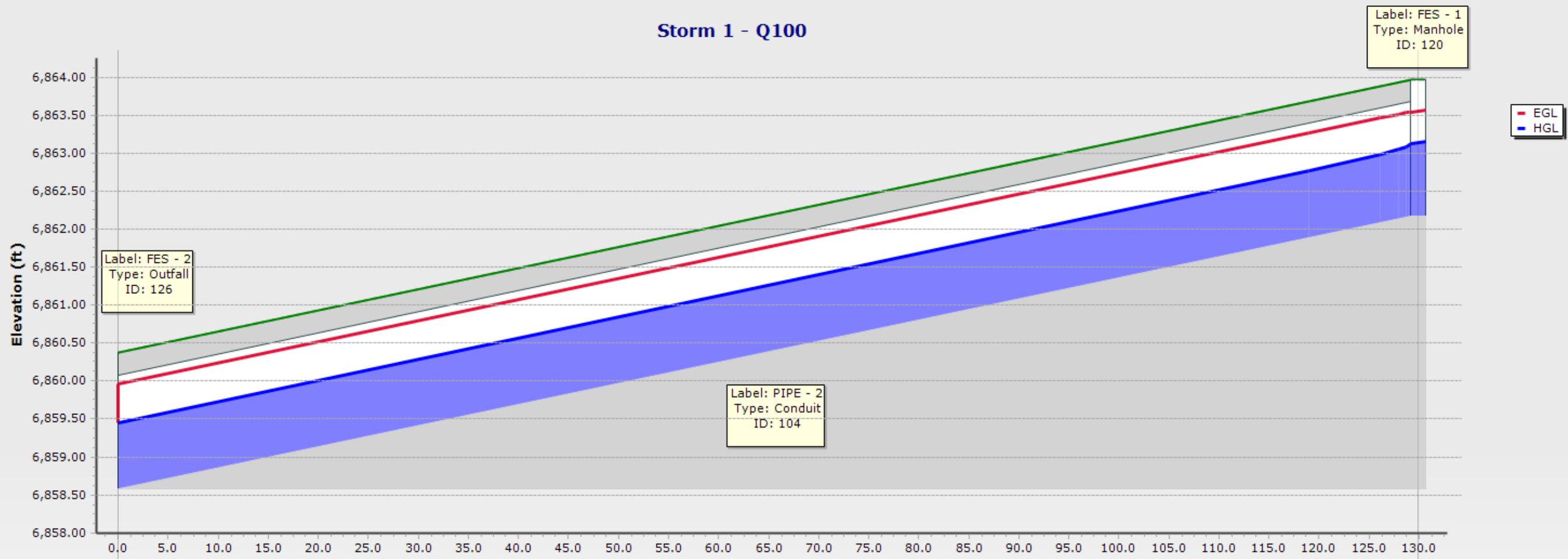
	Label ▲	Notes	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Boundary Condition Type	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
126: FES - 2	FES - 2		6,860.37	6,858.58	Free Outfall	6,858.94	1.20
88: FES - 3	FES - 3	18" FES	6,860.19	6,858.40	Free Outfall	6,858.92	2.70
124: FES - 5	FES - 5		6,923.21	6,921.42	Free Outfall	6,921.90	1.60
125: O-4	O-4		6,866.90	6,864.40	User Defined Tailwater	6,865.25	5.30
148: O-7	O-7		6,942.06	6,940.27	Free Outfall	6,940.72	1.50
179: O-11	O-11	CDOT TYPE C INLET	6,905.79	6,903.99	Free Outfall	6,904.38	1.10
180: O-12	O-12		6,973.55	6,971.76	Free Outfall	6,972.20	1.40

Figure 9- Q5 OUTFALL SUMMARY

THE TAILWATER CONDITION HAS BEEN DEFINED AS THE POND WSE DURING THE CORRESPONDING STORM EVENT FOR OUTFALLS THAT DISCHARGE INTO POND 1.

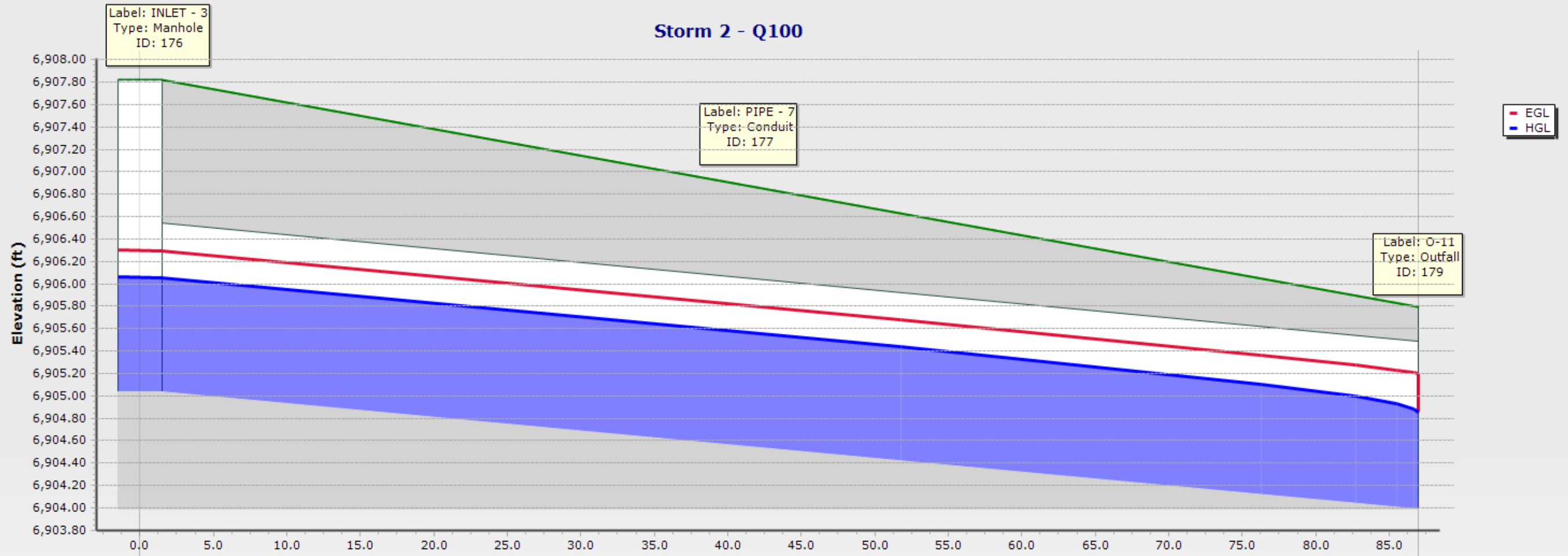


### Storm 1 - Q100



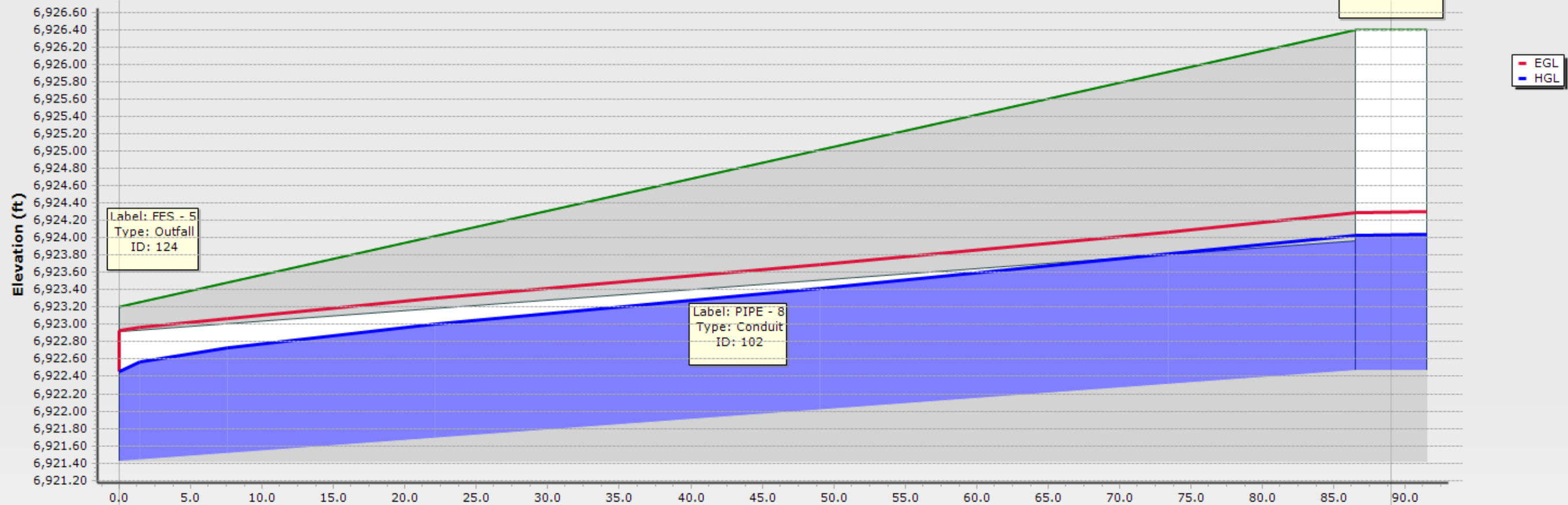
ID\Label	104 \ PIPE - 2	
Link Length (ft)	129.9	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	6.10	
Slope (ft/ft)	0.028	
ID\Label	126 \ FES - 2	120 \ FES - 1
Ground (ft)	6860.37	6863.97
Invert (ft)	6858.58	6862.18
Station (ft)	0.0	129.9

### Storm 2 - Q100



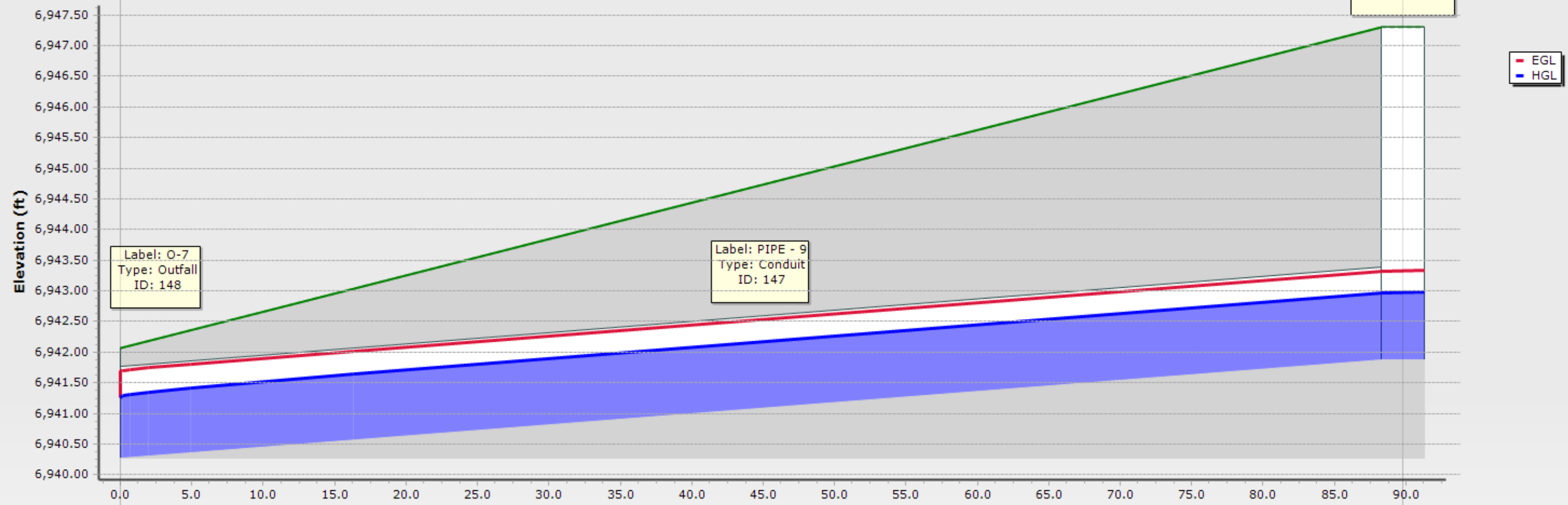
ID\Label	177 \ PIPE - 7	
Link Length (ft)	87.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	5.00	
Slope (ft/ft)	0.012	
ID\Label	176 \ INLET - 3	179 \ O-11
Ground (ft)	6907.82	6905.79
Invert (ft)	6905.04	6903.99
Station (ft)	0.0	87.0

### Storm 3 - Q100



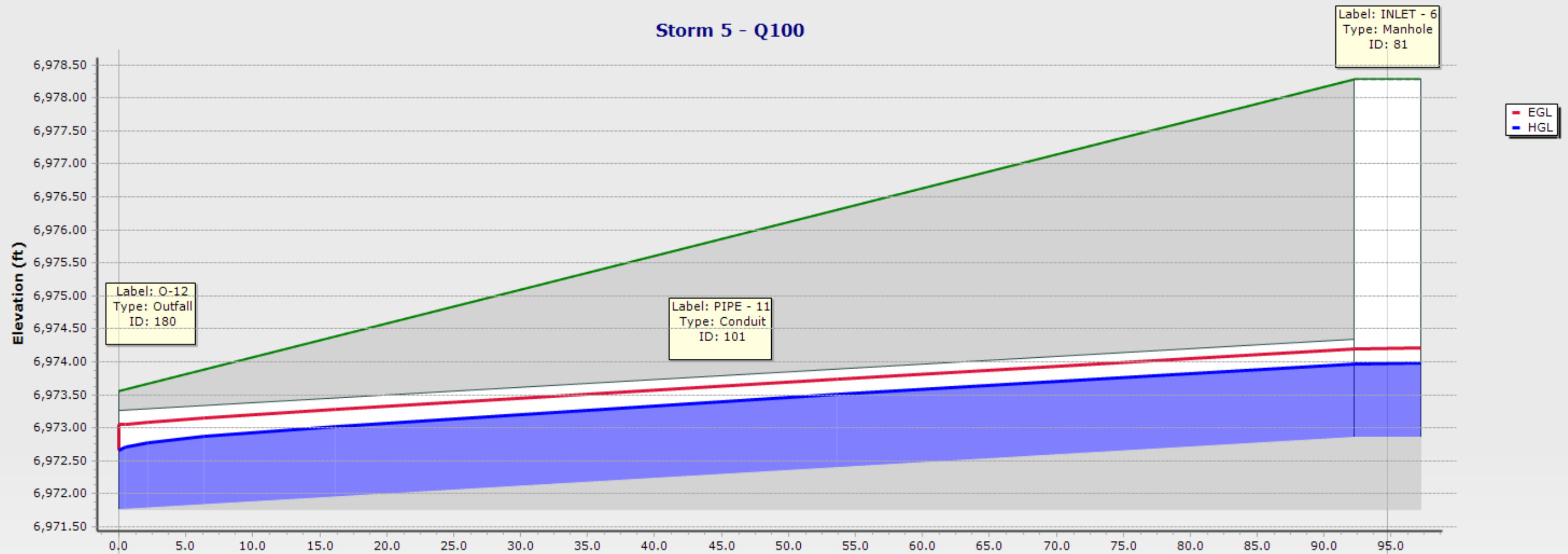
ID\Label	102 \ PIPE - 8	
Link Length (ft)	89.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	7.20	
Slope (ft/ft)	0.012	
ID\Label	124 \ FES - 5	84 \ INLET - 4
Ground (ft)	6923.21	6926.40
Invert (ft)	6921.42	6922.47
Station (ft)	0.0	89.0

### Storm 4 - Q100



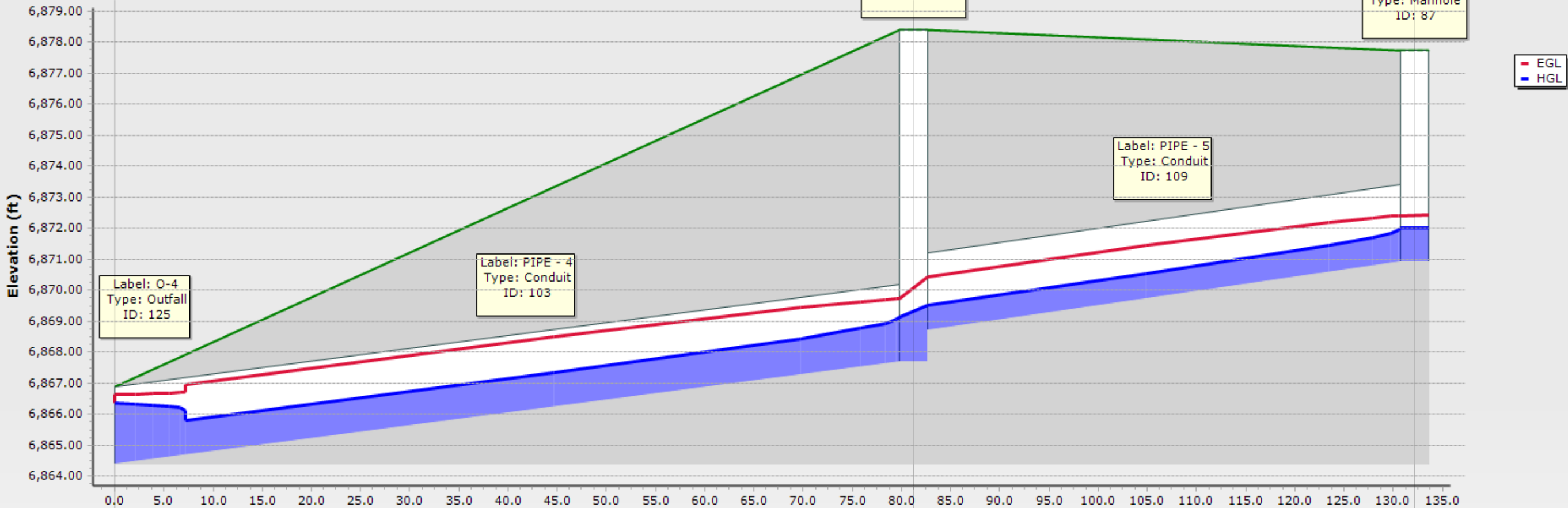
ID\Label	147 \ PIPE - 9	
Link Length (ft)	89.8	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	6.60	
Slope (ft/ft)	0.018	
ID\Label	148 \ O-7	145 \ INLET - 5
Ground (ft)	6942.06	6947.30
Invert (ft)	6940.27	6941.88
Station (ft)	0.0	89.8

### Storm 5 - Q100



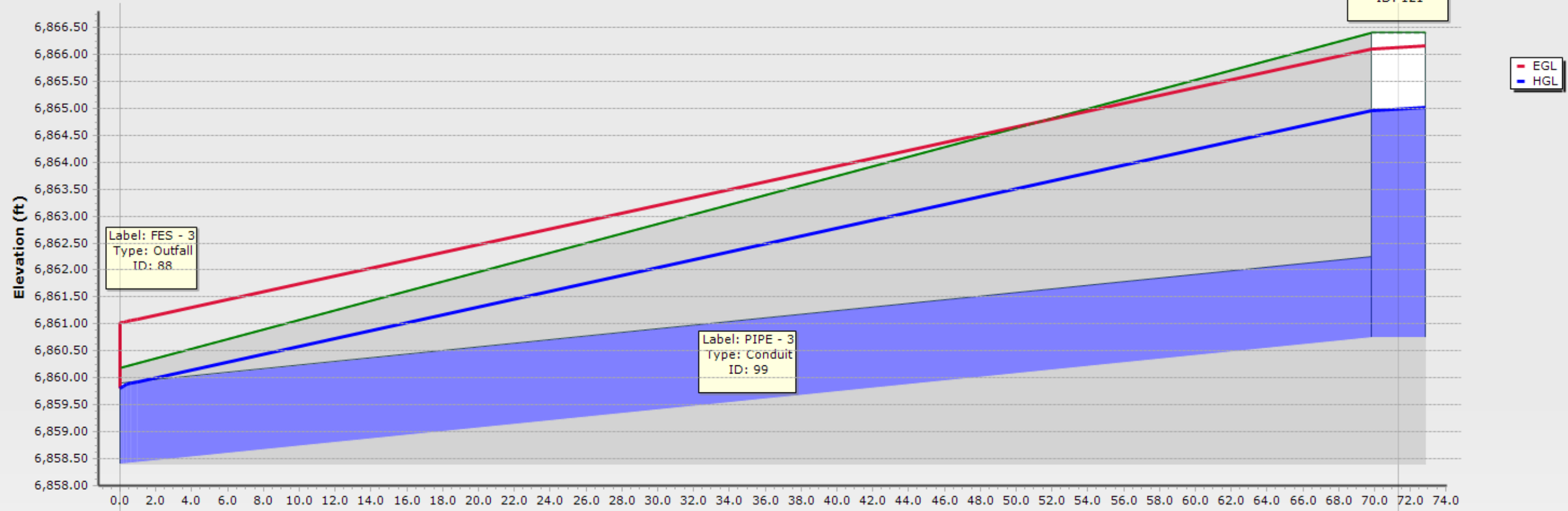
ID\Label	101 \ PIPE - 11	
Link Length (ft)	94.7	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	5.50	
Slope (ft/ft)	0.012	
ID\Label	180 \ O-12	81 \ INLET - 6
Ground (ft)	6973.55	6978.28
Invert (ft)	6971.76	6972.85
Station (ft)	0.0	94.7

### Pond Inlet - Q100



ID\Label	103 \ PIPE - 4	109 \ PIPE - 5	
Link Length (ft)	81.2	50.9	
Rise (in)\Material	30.0 \ CMP	30.0 \ CMP	
Flow (cfs)	17.90	10.60	
Slope (ft/ft)	0.041	0.043	
ID\Label	125 \ O-4	86 \ INLET - 2	87 \ INLET - 1
Ground (ft)	6866.90	6878.40	6877.73
Invert (ft)	6864.40	6867.70	6870.91
Station (ft)	0.0	81.2	132.2

### Pond-Outfall - Q100



ID\Label	99 \ PIPE - 3	
Link Length (ft)	71.3	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	15.20	
Slope (ft/ft)	0.033	
ID\Label	88 \ FES - 3	121 \ MH-5
Ground (ft)	6860.19	6866.40
Invert (ft)	6858.40	6860.75
Station (ft)	0.0	71.3



	Label ▲	Velocity (ft/s)	Start Node	Invert (Start) (ft)	Hydraulic Grade Line (In) (ft)	Invert (Stop) (ft)	Hydraulic Grade Line (Out) (ft)	Stop Node	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Flow (cfs)	Capacity (Full Flow) (cfs)	Depth (Normal) / Rise (%)	Diameter (in)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)
104: PIPE - 2	PIPE - 2	5.69	FES - 1	6,862.18	6,863.13	6,858.58	6,859.46	FES - 2	129.9	0.028	0.024	6.10	9.47	58.4	18.0	6,863.97	6,860.37
99: PIPE - 3	PIPE - 3	8.60	MH-5	6,860.75	6,864.95	6,858.40	6,859.81	FES - 3	71.3	0.033	0.024	15.20	10.33	(N/A)	18.0	6,866.40	6,860.19
103: PIPE - 4	PIPE - 4	8.61	INLET - 2	6,867.70	6,869.13	6,864.40	6,866.35	O-4	81.2	0.041	0.024	17.90	44.78	44.0	30.0	6,878.40	6,866.90
109: PIPE - 5	PIPE - 5	7.65	INLET - 1	6,870.91	6,872.00	6,868.70	6,869.51	INLET - 2	50.9	0.043	0.024	10.60	46.27	32.6	30.0	6,877.73	6,878.40
177: PIPE - 7	PIPE - 7	3.93	INLET - 3	6,905.04	6,906.05	6,903.99	6,904.85	O-11	87.0	0.012	0.024	5.00	6.25	67.6	18.0	6,907.82	6,905.79
102: PIPE - 8	PIPE - 8	4.07	INLET - 4	6,922.47	6,924.03	6,921.42	6,922.46	FES - 5	89.0	0.012	0.024	7.20	6.18	(N/A)	18.0	6,926.40	6,923.21
147: PIPE - 9	PIPE - 9	4.85	INLET - 5	6,941.88	6,942.96	6,940.27	6,941.26	O-7	89.8	0.018	0.024	6.60	7.62	71.9	18.0	6,947.30	6,942.06
101: PIPE - 11	PIPE - 11	3.91	INLET - 6	6,972.85	6,973.96	6,971.76	6,972.66	O-12	94.7	0.012	0.024	5.50	6.10	74.2	18.0	6,978.28	6,973.55

Figure 10- Q100 CONDUIT SUMMARY

	Label ▲	Flow (Known) (cfs)	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Headloss Method	Headloss Coefficient (Standard)	Flow (Total Out) (cfs)
120: FES - 1	FES - 1	6.10	6,863.97	6,863.97	0.95	6,863.15	6,863.13	Standard	0.050	6.10
87: INLET - 1	INLET - 1	10.60	6,877.73	6,877.73	1.09	6,872.02	6,872.00	Standard	0.050	10.60
86: INLET - 2	INLET - 2	17.90	6,878.40	6,878.40	1.43	6,869.51	6,869.13	Standard	0.640	17.90
176: INLET - 3	INLET - 3	5.00	6,907.82	6,907.82	1.01	6,906.07	6,906.05	Standard	0.050	5.00
84: INLET - 4	INLET - 4	7.20	6,926.40	6,926.40	1.56	6,924.04	6,924.03	Standard	0.050	7.20
145: INLET - 5	INLET - 5	6.60	6,947.30	6,947.30	1.08	6,942.98	6,942.96	Standard	0.050	6.60
81: INLET - 6	INLET - 6	5.50	6,978.28	6,978.28	1.11	6,973.98	6,973.96	Standard	0.050	5.50
121: MH-5	MH-5	15.20	6,866.40	6,866.40	4.20	6,865.01	6,864.95	Standard	0.050	15.20

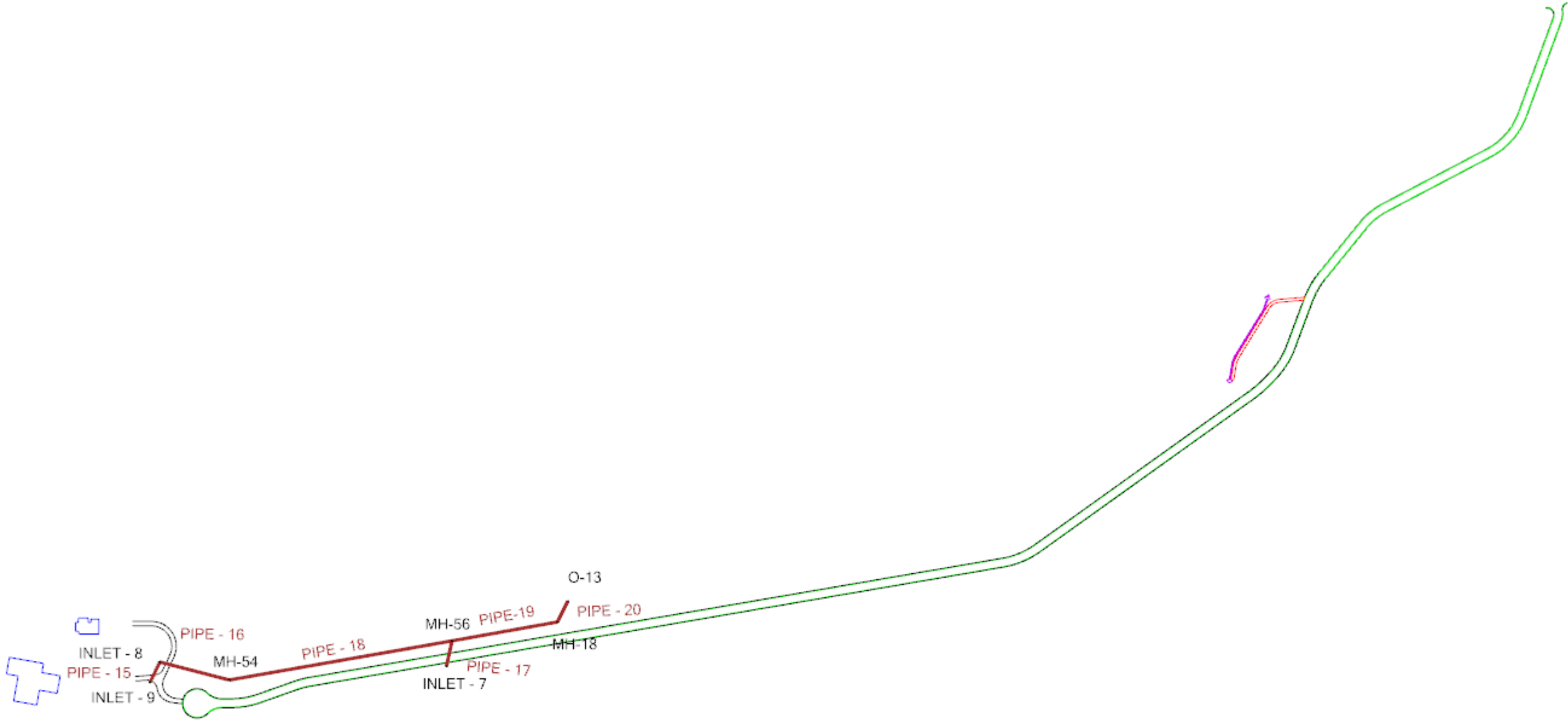
Figure 11- Q100 NODE SUMMARY

	Label ▲	Notes	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Boundary Condition Type	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
126: FES - 2	FES - 2		6,860.37	6,858.58	Free Outfall	6,859.46	6.10
88: FES - 3	FES - 3	18" FES	6,860.19	6,858.40	Free Outfall	6,859.81	15.20
124: FES - 5	FES - 5		6,923.21	6,921.42	Free Outfall	6,922.46	7.20
125: O-4	O-4		6,866.90	6,864.40	User Defined Tailwater	6,866.35	17.90
148: O-7	O-7		6,942.06	6,940.27	Free Outfall	6,941.26	6.60
179: O-11	O-11	CDOT TYPE C INLET	6,905.79	6,903.99	Free Outfall	6,904.85	5.00
180: O-12	O-12		6,973.55	6,971.76	Free Outfall	6,972.66	5.50

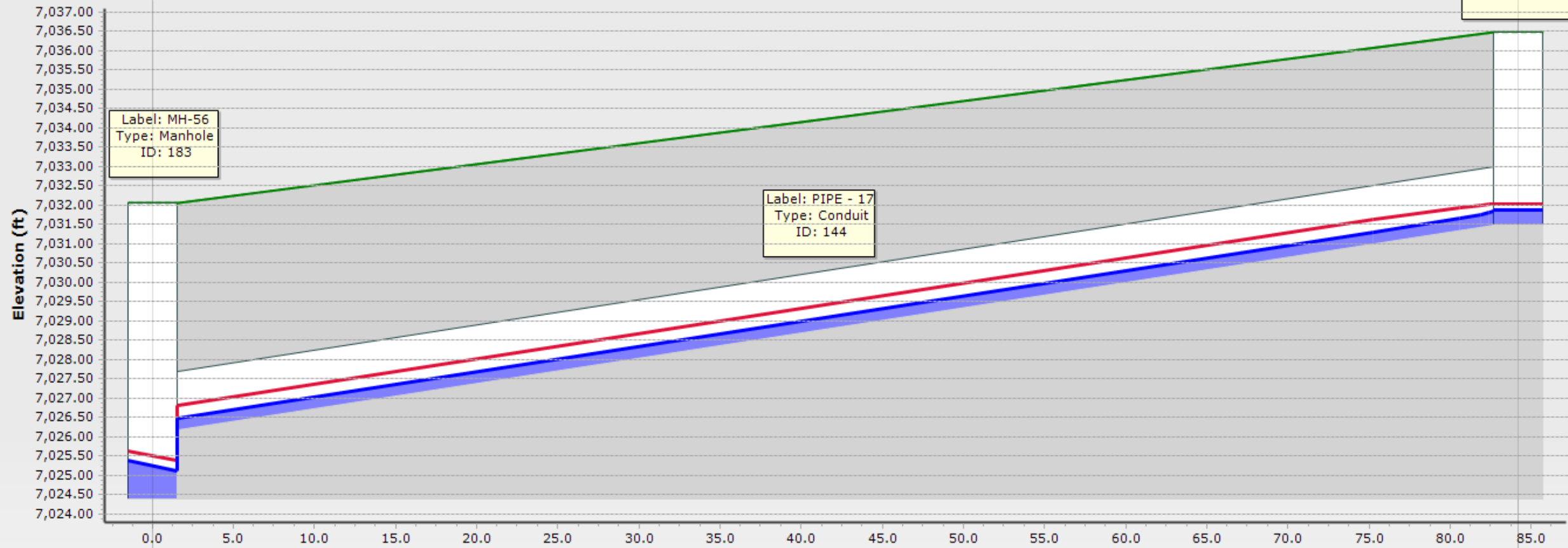
Figure 12- Q100 OUTFALL SUMMARY

THE TAILWATER CONDITION HAS BEEN DEFINED AS THE POND WSE DURING THE CORRESPONDING STORM EVENT FOR OUTFALLS THAT DISCHARGE INTO POND 1.

# StormCAD LAYOUT

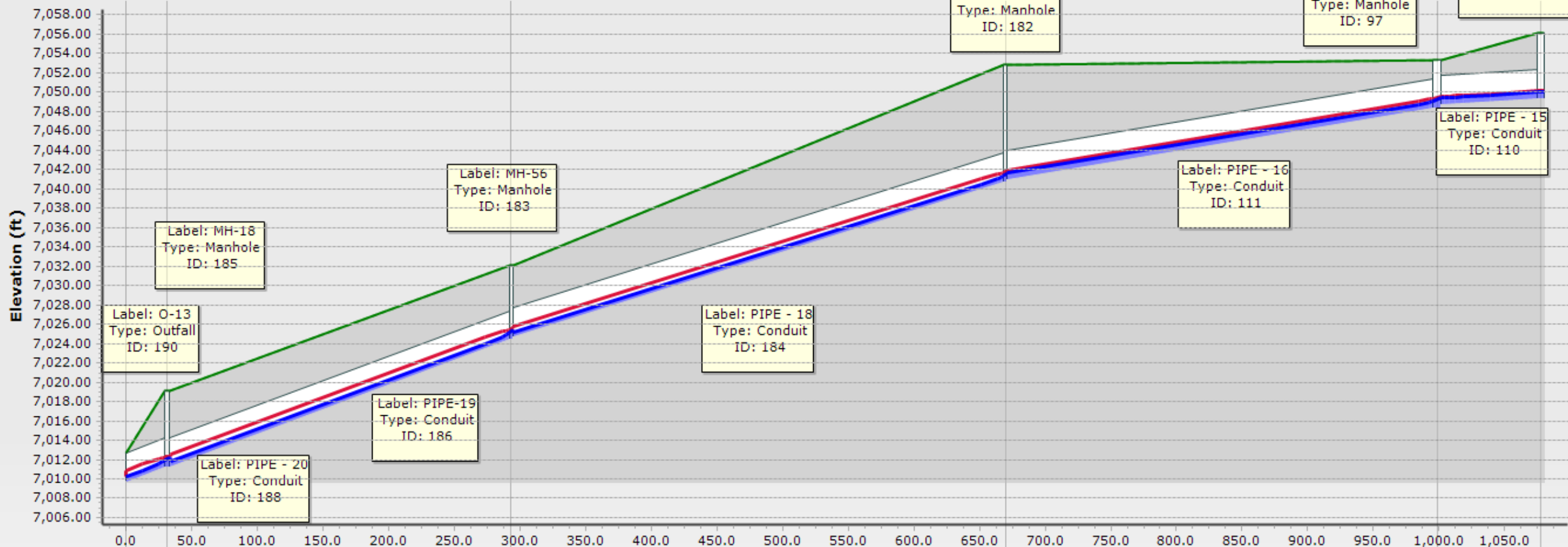


### STORM 6 - Q5 FREE OUTFALL



ID\Label		144 \ PIPE - 17	
Link Length (ft)		84.2	
Rise (in)\Material		18.0 \ CMP	
Flow (cfs)		1.10	
Slope (ft/ft)		0.063	
ID\Label	183 \ MH-56		117 \ INLET - 7
Ground (ft)	7032.06		7036.48
Invert (ft)	7024.39		7031.49
Station (ft)	0.0		84.2

### STORM 7 - Q5 FREE OUTFALL



ID\Label	188 \ PIPE - 20	186 \ PIPE-19	184 \ PIPE - 18	111 \ PIPE - 16	110 \ PIPE - 15
Link Length (ft)	31.4	261.9	376.5	328.7	78.8
Rise (in)\Material	36.0 \ CMP	36.0 \ CMP	36.0 \ CMP	36.0 \ CMP	36.0 \ CMP
Flow (cfs)	5.50	5.50	5.00	5.00	3.70
Slope (ft/ft)	0.050	0.050	0.042	0.023	0.007
ID\Label	90 \ <del>185</del> MH-18	183 \ MH-56	182 \ MH-54	97 \ INLET - 8	96 \ INLET - 9
Ground (ft)	7012.77 7019.07	7032.06	7052.87	7053.31	7056.10
Invert (ft)	7009.77 7011.26	7024.39	7040.69	7048.39	7049.28
Station (ft)	0.0 31.4	293.2	669.8	998.5	1077.2



	Label ▲	Velocity (ft/s)	Start Node	Invert (Start) (ft)	Hydraulic Grade Line (In) (ft)	Invert (Stop) (ft)	Hydraulic Grade Line (Out) (ft)	Stop Node	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Flow (cfs)	Capacity (Full Flow) (cfs)	Depth (Normal) / Rise (%)	Diameter (in)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)
110: PIPE - 15	PIPE - 15	2.97	INLET - 9	7,049.28	7,049.98	7,048.69	7,049.42	INLET - 8	78.8	0.007	0.024	3.70	31.27	23.2	36.0	7,056.10	7,053.31
111: PIPE - 16	PIPE - 16	4.79	INLET - 8	7,048.39	7,049.09	7,040.99	7,041.61	MH-54	328.7	0.023	0.024	5.00	54.20	20.5	36.0	7,053.31	7,052.87
144: PIPE - 17	PIPE - 17	4.78	INLET - 7	7,031.49	7,031.88	7,026.19	7,026.47	MH-56	84.2	0.063	0.024	1.10	14.28	18.8	18.0	7,036.48	7,032.06
184: PIPE - 18	PIPE - 18	5.99	MH-54	7,040.69	7,041.39	7,024.69	7,025.39	MH-56	376.5	0.042	0.024	5.00	74.47	17.5	36.0	7,052.87	7,032.06
186: PIPE - 19	PIPE - 19	6.54	MH-56	7,024.39	7,025.13	7,011.26	7,012.16	MH-18	261.9	0.050	0.024	5.50	80.89	17.7	36.0	7,032.06	7,019.07
188: PIPE - 20	PIPE - 20	6.53	MH-18	7,011.26	7,012.00	7,009.69	7,010.22	O-13	31.4	0.050	0.024	5.50	80.82	17.7	36.0	7,019.07	7,012.71

Figure 1- Q5 – Free Outfall CONDUIT SUMMARY

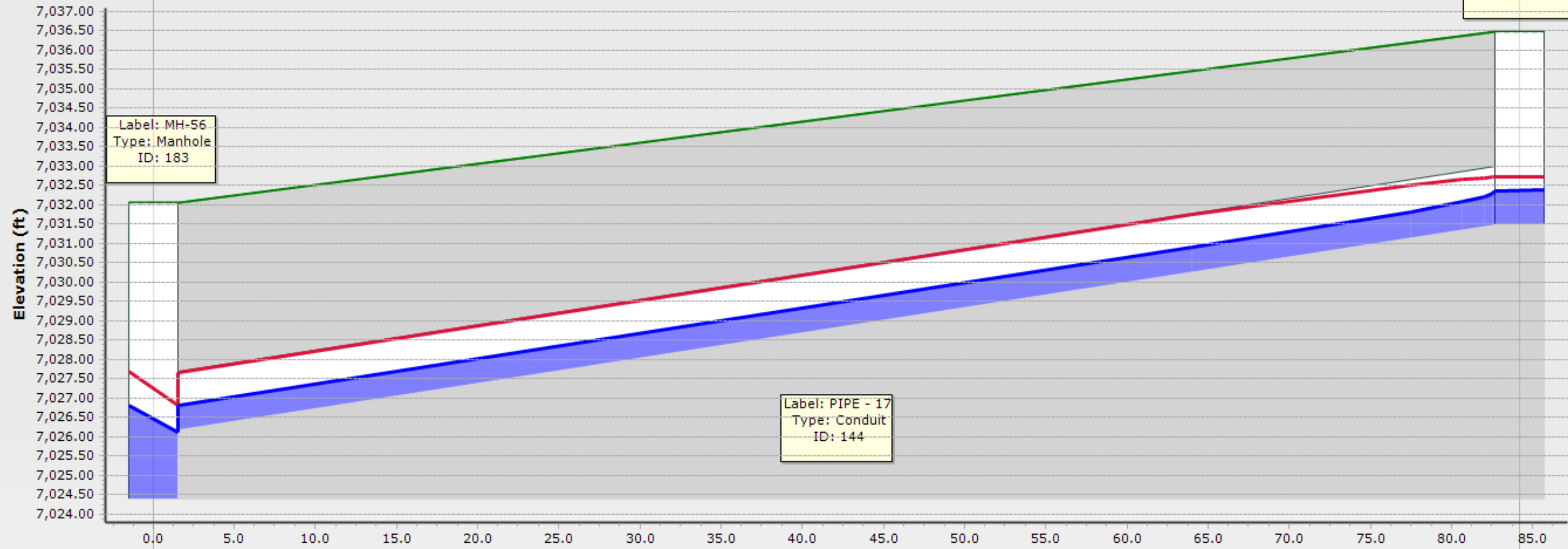
	Label ▲	Flow (Known) (cfs)	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Headloss Method	Headloss Coefficient (Standard)	Flow (Total Out) (cfs)
117: INLET - 7	INLET - 7	1.10	7,036.48	7,036.48	0.39	7,031.89	7,031.88	Standard	0.050	1.10
97: INLET - 8	INLET - 8	5.00	7,053.31	7,053.31	0.70	7,049.42	7,049.09	Standard	1.320	5.00
96: INLET - 9	INLET - 9	3.70	7,056.10	7,056.10	0.70	7,049.98	7,049.98	Standard	0.050	3.70
185: MH-18	MH-18	5.50	7,019.07	7,019.07	0.74	7,012.16	7,012.00	Standard	0.640	5.50
182: MH-54	MH-54	5.00	7,052.87	7,052.87	0.70	7,041.55	7,041.39	Standard	0.640	5.00
183: MH-56	MH-56	5.50	7,032.06	7,032.06	0.74	7,025.39	7,025.13	Standard	1.020	5.50

Figure 2- Q5 – Free Outfall NODE SUMMARY

	Label ▲	Notes	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Boundary Condition Type	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
190: O-13	O-13		7,012.71	7,009.71	Free Outfall	7,010.22	5.50

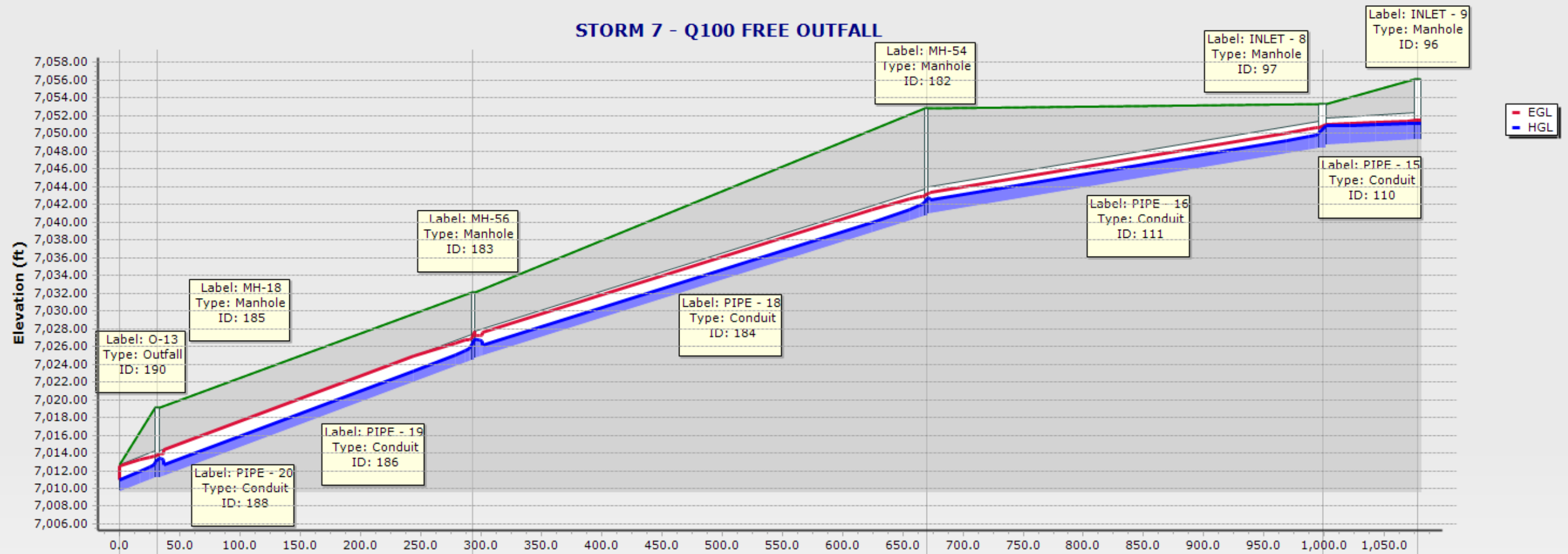
Figure 3- Q5 – Free Outfall OUTFALL SUMMARY

### STORM 6 - Q100 FREE OUTFALL



ID\Label	144 \ PIPE - 17	
Link Length (ft)	84.2	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	5.10	
Slope (ft/ft)	0.063	
ID\Label	183 \ MH-56	117 \ INLET - 7
Ground (ft)	7032.06	7036.48
Invert (ft)	7024.39	7031.49
Station (ft)	0.0	84.2

### STORM 7 - Q100 FREE OUTFALL



ID\Label	188 \ PIPE - 20	186 \ PIPE - 19	184 \ PIPE - 18	111 \ PIPE - 16	110 \ PIPE - 15
Link Length (ft)	31.4	261.9	376.5	328.7	78.8
Rise (in)\Material	36.0 \ CMP	36.0 \ CMP	36.0 \ CMP	36.0 \ CMP	36.0 \ CMP
Flow (cfs)	28.20	28.20	25.20	25.20	19.50
Slope (ft/ft)	0.050	0.050	0.042	0.023	0.007
ID\Label	90 \ <del>Q-13</del> MH-18	183 \ MH-56	182 \ MH-54	97 \ INLET - 8	96 \ INLET - 9
Ground (ft)	7012.77 7019.07	7032.06	7052.87	7053.31	7056.10
Invert (ft)	7009.77 7011.26	7024.39	7040.69	7048.39	7049.28
Station (ft)	0.0 31.4	293.2	669.8	998.5	1077.2



	Label ▲	Velocity (ft/s)	Start Node	Invert (Start) (ft)	Hydraulic Grade Line (In) (ft)	Invert (Stop) (ft)	Hydraulic Grade Line (Out) (ft)	Stop Node	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Flow (cfs)	Capacity (Full Flow) (cfs)	Depth (Normal) / Rise (%)	Diameter (in)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)
110: PIPE - 15	PIPE - 15	4.66	INLET - 9	7,049.28	7,051.16	7,048.69	7,050.87	INLET - 8	78.8	0.007	0.024	19.50	31.27	57.2	36.0	7,056.10	7,053.31
111: PIPE - 16	PIPE - 16	7.53	INLET - 8	7,048.39	7,050.01	7,040.99	7,042.73	MH-54	328.7	0.023	0.024	25.20	54.20	47.9	36.0	7,053.31	7,052.87
144: PIPE - 17	PIPE - 17	7.40	INLET - 7	7,031.49	7,032.36	7,026.19	7,026.81	MH-56	84.2	0.063	0.024	5.10	14.28	41.3	18.0	7,036.48	7,032.06
184: PIPE - 18	PIPE - 18	9.52	MH-54	7,040.69	7,042.31	7,024.69	7,026.83	MH-56	376.5	0.042	0.024	25.20	74.47	40.1	36.0	7,052.87	7,032.06
186: PIPE - 19	PIPE - 19	10.42	MH-56	7,024.39	7,026.11	7,011.26	7,013.43	MH-18	261.9	0.050	0.024	28.20	80.89	40.7	36.0	7,032.06	7,019.07
188: PIPE - 20	PIPE - 20	10.41	MH-18	7,011.26	7,012.98	7,009.69	7,010.94	O-13	31.4	0.050	0.024	28.20	80.82	40.8	36.0	7,019.07	7,012.71

Figure 4- Q100 – Free Outfall CONDUIT SUMMARY

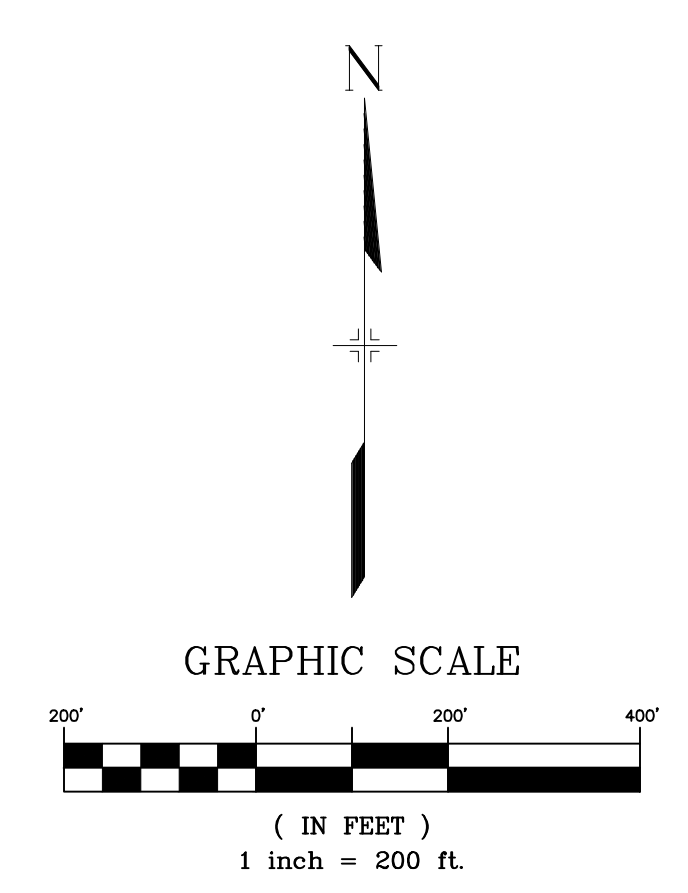
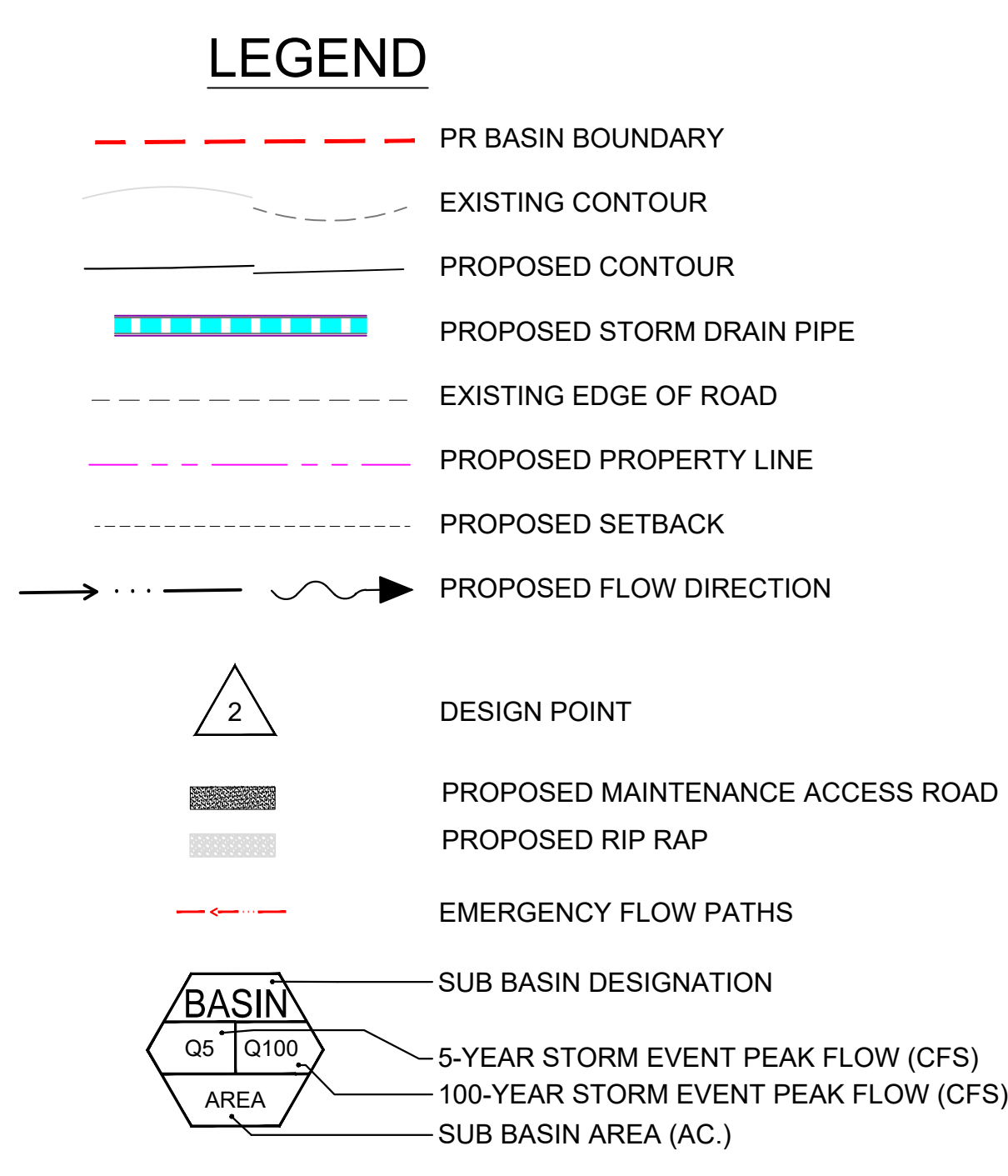
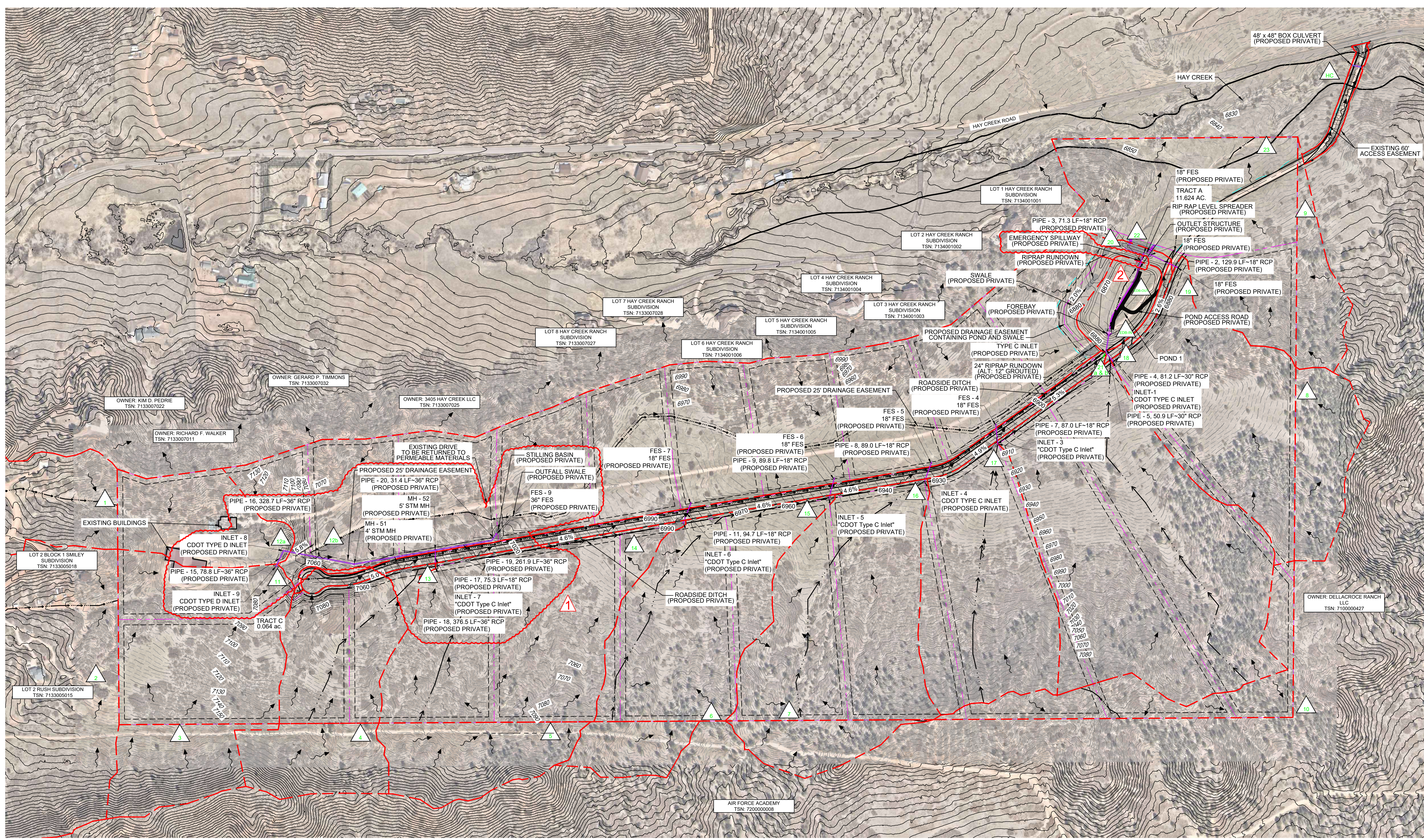
	Label ▲	Flow (Known) (cfs)	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Headloss Method	Headloss Coefficient (Standard)	Flow (Total Out) (cfs)
117: INLET - 7	INLET - 7	5.10	7,036.48	7,036.48	0.87	7,032.38	7,032.36	Standard	0.050	5.10
97: INLET - 8	INLET - 8	25.20	7,053.31	7,053.31	1.62	7,050.87	7,050.01	Standard	1.320	25.20
96: INLET - 9	INLET - 9	19.50	7,056.10	7,056.10	1.88	7,051.18	7,051.16	Standard	0.050	19.50
185: MH-18	MH-18	28.20	7,019.07	7,019.07	1.72	7,013.43	7,012.98	Standard	0.640	28.20
182: MH-54	MH-54	25.20	7,052.87	7,052.87	1.62	7,042.73	7,042.31	Standard	0.640	25.20
183: MH-56	MH-56	28.20	7,032.06	7,032.06	1.72	7,026.83	7,026.11	Standard	1.020	28.20

Figure 5- Q100 – Free Outfall NODE SUMMARY

	Label ▲	Notes	Elevation (Ground) (ft)	Elevation (Invert) (ft)	Boundary Condition Type	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
190: O-13	O-13		7,012.71	7,009.71	Free Outfall	7,010.94	28.20

Figure 6- Q100 Free Outfall OUTFALL SUMMARY





Hay Creek  
 Proposed Conditions  
 Sub-basin Summary

Basin	Area		Q5	Q100
	acres	cfs	cfs	cfs
OS1a	9.4	0.8	3.6	
OS1b	59.2	2.6	14.2	
OS2a	5.0	0.4	1.8	
OS2b	4.7	0.4	2.0	
OS2c	6.1	0.8	2.7	
OS2d	2.8	0.3	1.1	
OS2e	3.2	0.3	1.3	
OS3a	4.9	0.1	0.9	
OS3b	3.3	0.1	0.8	
OS4	0.7	1.9	3.9	
PR-1a	12.9	0.9	4.3	
PR-1b	6.3	0.5	2.5	
PR-1c	53.2	3.4	15.7	
PR-2	7.8	0.7	3.2	
PR-3	18.4	1.5	7.0	
PR-4	14.2	1.2	5.5	
PR-5	16.9	1.4	6.3	
PR-6	13.9	1.4	6.3	
PR-7	27.8	2.4	10.9	
PR-8	5.9	3.1	17.0	
PR-9	13.4	0.6	3.6	
PR-10a	4.7	3.3	7.8	
PR-10b	0.4	0.8	1.7	
PR-11	2.4	0.6	4.0	
PR-12	16.4	0.6	3.5	

Proposed Design Point Summary

Design Point	Sub-Basins	Total	Q(5)	Q(100)
		Area (ac.)	(cfs)	(cfs)
1	OS1a	9.35	0.80	3.60
2	OS1b	59.21	2.60	14.20
3	OS2a	5.01	0.40	1.80
4	OS2b	4.74	0.40	2.00
5	OS2c	6.11	0.80	2.70
6	OS2d	2.77	0.30	1.10
8	OS3a	4.88	0.10	0.90
9	OS3b	3.29	0.10	0.80
10	PR-8	5.86	3.11	17.01
11	OS1b, OS2a, PR-1a	77.15	3.70	19.50
12a	OS1a, PR-1b	15.63	1.30	6.10
12b	OS1a, OS2a, OS1b, PR-1a, PR-1b	92.78	5.00	25.20
13a	OS2b, PR-2	12.51	1.10	5.10
14	OS2c, PR-3	24.55	2.00	8.70
15	OS2d, PR-4	16.96	1.40	6.50
16	OS2e, PR-5	20.09	1.60	7.40
17	PR-6	13.87	1.40	6.30
20	PR-1c, DP-12b, DP-13, DP-14, DP-15, DP-16, DP-17	233.97	11.80	63.50
21a	PR-10a	4.73	3.30	7.80
21b	PR-7, PR-10a	32.49	5.30	18.10
EDB-IN	PR-7, PR-10a, PR-10b, PR-11	35.31	5.40	18.40
EDB-OUT	PR-7, PR-10a, PR-10b, PR-11	35.31	4.00	15.90
22	EDB-OUT, DP-18, DP-19	283.15	12.30	66.80
23	DP-9, DP-21, PR-12	302.82	12.80	70.70
HC	From PPRBD Regional Floodplain Administrator			127.00