

HAY CREEK VALLEY
FINAL DRAINAGE REPORT

Prepared for:

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January 2024

Project No. 22.886.076

PCD File SF2324

Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Jesse Sullivan
Registered Professional Engineer
State of Colorado
No. 55600

Date



Owner/Developer's Statement:

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

View Homes, Inc.

Business Name

By: _____

Timothy Buschar

Date

Title: _____

Director of Entitlement

Address: 555 Middle Creek Parkway Suite 500
Colorado Springs, CO 80921

El Paso County:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

07/02/2024

Joshua Palmer, P.E.
County Engineer / ECM Administrator
Conditions:

Date

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I. INTRODUCTION

The Hay Creek Valley site is comprised of approximately 214.6 acres of unplatted and mostly undeveloped land. The site is located on Smow Mountain Heights approximately 700 feet south of its intersection with Hay Creek Road. The site is currently comprised of six (6) parcels which are to be subdivided into 20 lots and four (4) tracts. The existing access road will be replaced with a private road located within a proposed 70-foot-wide tract which will terminate with a cul-de-sac in the southwestern section of the site.

a. PURPOSE AND SCOPE OF STUDY

The purpose of this Final Drainage Report (FDR) is to evaluate the specific drainage infrastructure requirements which will provide compliance with the approved Hay Creek Valley MDDP / Preliminary Drainage Report, by Matrix, dated August 2023 (MDDP), and the El Paso County Drainage Criteria Manual (DCM) to provide storm water conveyance for associated developments. This study will identify off-site, and on-site drainage patterns associated with respective land uses, provide hydrologic and hydraulic analysis of tributary basins and conveyance structures to a detention pond, and identify effective, safe routing to the downstream outfall. The improvements associated with this report maintain compliance with the DCM by providing full spectrum detention where necessary, which is to be constructed concurrently with the improvements associated with this FDR.

b. DBPS RELATED INVESTIGATIONS

The proposed development is located within the Beaver Creek Drainage Basin. No Drainage Basin Planning Study (DBPS) has been completed for this basin.

c. GENERAL PROJECT DESCRIPTION

The Hay Creek Valley Subdivision is located to the southwest of the intersection of Hay Creek Road and Smow Mountain Heights. The site is located as follows:

1. General Location: Southwest $\frac{1}{4}$ of Section 34 and the Southeast $\frac{1}{4}$ of Section 33, Township 11 South, Range 67 West of the 6th P.M. in the County of El Paso, State of Colorado.
2. Drainageway: The Hay Creek Subdivision is located on the southern edge of the Beaver Creek Drainage Basin. Most of the site drains north and into Hay Creek located approximately 200 feet north of the site. Hay Creek is a tributary to Beaver Creek which ultimately drains into Monument Creek. A small portion of the southeast corner of the site drains south into the Air Force Academy Major Drainage Basin.
3. Surrounding Developments: The site is bound Lots 1 through 8 Hay Creek Ranch Subdivision, and 4 unplatted parcels to the north, and by the Air Force Academy the south. The site is bound by Lot 2 Rush Subdivision and Lot 2 Block 1 Smiley Subdivision to the west, and an unplatted parcel to the east.
4. Lots to be Platted: The site is to be subdivided into 20 lots zoned RR-5 and 4 tracts.
5. Area of Disturbance: The Hay Creek Valley development is expected to disturb a total area of approximately 17.22 acres.
6. Streamside Zone: This project is not located within a streamside zone.
7. Vegetation: The Hay Creek Valley site contains a single-family residence, a barn and Smow Mountain Heights, a private road that provided access to the site from hay Creek

Road. The vegetation of the site consists of sparse, natural vegetative land cover in the form of grasses and shrubs with sparse trees throughout.

Refer to Appendix D for the Vicinity Map.

d. SOILS CONDITIONS

Soils can be classified in four different hydrologic groups, A, B, C, or D to help predict stormwater runoff rates. Hydrologic group “A” is characterized by deep, well-drained coarse-grained soils with a rapid infiltration rate when thoroughly wet and having a low runoff potential. Group “D” typically has a clay layer at or near to the surface, or a very shallow depth to impervious bedrock and has a very slow infiltration rate and a high runoff potential. See Soils Map, Appendix C. The following soil types are present in the Hay Creek Valley site:

Table 1.1 – NRCS Soil Survey for El Paso County – Hay Creek Valley

Soil ID Number	Soil	Hydrologic Classification	Drainage Class	Percent of Site
38	Jarre-Tecolote Complex, 8 to 65 percent slopes	B	Well Drained	50.8%
71	Pring coarse sandy loam, 3 to 8 percent slopes	B	Well Drained	14.5%
93	Tomah-Crowfoot complex, 8 to 15 percent slopes	B	Well Drained	34.7%

DATA SOURCES

Topographical information for the district was found using a combination of *United States Geological Survey* (USGS) mapping as well as field surveying. The *Web Soil Survey*, created by the *Natural Resources Conservation Service*, was utilized to investigate the existing general soil types within the district. Offsite contours are taken from the *2018 El Paso County LIDAR* survey and/or USGS Quad Sheets.

e. APPLICABLE CRITERIA AND STANDARDS

This report has been prepared in accordance to the criteria set forth in the City of Colorado Springs and El Paso County DCM, El Paso County Engineering Criteria Manual (ECM) and El Paso County Resolutions 15-042 and 19-245. In addition to the DCM, the **Urban Storm Drainage Criteria Manuals, Volumes 1 through 3**, dated 2016 have been used to supplement the County’s Criteria Manual.

II. Hydrologic Methodology

a. MAJOR BASINS AND SUBBASINS

The majority of the Hay Creek Valley site is located within the Beaver Creek Drainage Basin with a small portion of the site tributary to the Air Force Academy Major Drainage Basin. Runoff presently flows overland until reaching an existing natural drainage swale located within the site. This drainage swale directs flows internally until discharging from near the northeastern corner of the site. Drainage

from the developed road will be directed to Pond 1, where the runoff will be treated for water quality and detained to maintain the historic major event discharge rate from the site.

b. METHODOLOGY

i. UD Methods

The hydrology for this project uses both the **SCS Hydrograph Procedure** and the **Rational Method** as recommended by the Drainage Criteria Manual (DCM) for the minor and major storms. The Rational Method is used for drainage basins less than 100-acres in size. The Rational Method uses the following equation:

$$Q=C*i*A$$

Where:

- Q = Maximum runoff rate in cubic feet per second (cfs)
- C = Runoff coefficient
- i = Average rainfall intensity (inches per hour)
- A = Area of drainage sub-basin (acres)

Rational Method coefficients from 6-6 of the Drainage Criteria Manual for developed land were utilized in the Rational Method calculations. This method will be used primarily for sizing of storm sewer infrastructure. See Appendix B for more information.

Time of Concentration

The time of concentration consists of the initial time of overland flow and the travel time in a channel to the inlet or point of interest. A minimum time of concentrations of 5 minutes is utilized for urban areas. The Rational Calculation spreadsheet included in Appendix A shows an initial overland flow length, a channel or street flow length for each sub-basin, and also demonstrates the time of concentration calculations for initial (overland) and channel (or street) conditions. A maximum “True Initial” Flow Length of 300 feet will be used for pre-developed sub-basins and a maximum length of 100 feet will be used for Developed sub-basins for time of concentration calculations in compliance with the DCM.

Rainfall Intensity

The hypothetical rainfall depths for the 1-hour storm duration were derived using Table 6-2 of the Colorado Springs DCM (shown below). See Appendix B.

Table 2.1 – Project Area 1-Hour Rainfall Depth

Storm Recurrence Interval	Rainfall Depth (inches)
5-year	1.50
100-year	2.52

The rainfall intensity equation for the Rational Method was taken from Drainage Criteria Manual Volume 1 Figure 6-5.

C-Factors

C-factors for the Rational Method are based on anticipated land use and are taken from Table 6-6. Proposed single family residential is considered as the Single Family – 5 acres category. Areas which will be future open spaces or detention facilities are modeled under the Parks and Cemeteries category. Undeveloped or predevelopment areas are model under Undeveloped Areas-Historic Flow Analysis—Greenbelts, Agriculture category.

ii. HEC-HMS Methods

The DCM requires the use of the SCS Hydrograph Procedure to compute storm water runoff quantities when the drainage area is greater than 100 acres. This report uses HEC-HMS for the routing and analysis of the site conditions.

SCS Lag

The SCS lag calculations are completed as part of this report. Proposed development areas within the Hay Creek area are calculated based on the methodology indicated in the DCM Equations:

Lag Time:

$$t_{lag} = 0.6 \cdot t_c \quad (\text{Eq. 6-13})$$

The Time of Concentration is the sum of overland flow time and the t_i values for the various consecutive flow segments:

$$t_c = t_i + t_{t1} + t_{t2} + t_{t3} \dots t_{tm} \quad (\text{Eq. 6-14})$$

Where:

t_c = time of concentration (hr)

t_i = overland (initial) flow time (hr)

t_{tm} = travel time for each flow segment (hr)

m = number of flow segments

Overland Flow:

$$T_i = 0.007(n \cdot L)^{0.8} / (P_2)^{0.5} S^{0.4} \quad (\text{Eq. 6-15})$$

Where:

T_i = overland flow time (hr)

n = Manning's roughness coefficient

L = flow length (ft)

P_2 = 2-year, 24-hour rainfall (in)

S = slope of hydraulic grade line (ft/ft)

Concentrated Flow:

$$T_t = L / (3600 \cdot V) \tag{Eq. 6-16}$$

Where:

T_t = travel time (hr)

L = flow length (ft)

V = velocity (ft/s)

3,600 = conversion factor from seconds to hours

Runoff Analysis

The site has been analyzed using HEC-HMS and the NRCS SCS method. The model indicates approximately 10.5 cfs decrease for Q100 event post development of the Hay Creek site. **(EX-Q5, EX-Q100, PR-Q5, and PR-Q100)** These models look at the 5, and 100-year events for onsite detention within the Hay Creek Site and demonstrate a slight reduction for the 100-year event. Print outs from each model can be found in Appendix A.

Hydrographs for the proposed Pond 1 are taken from MHFD-Detention, where appropriate, and input to the model to represent the detention required within the development to maintain the historic flow discharge downstream of the site and provide water quality for the development in accordance with the DCM.

NRCS SCS curve numbers (CN) are taken from Table 6-10 of the DCM and weighted for each basin based on the soil and development types. As recommended in the DCM storms of 2-hour duration are used for the 5-year event. A 24-hour storm is used for the 100-year event. Lag times are calculated for the basins using the formulas from the DCM shown above.

iii. HGL Profile Methods

Preliminary sizing of storm sewer has been completed using the Manning’s channel flow calculation.

To confirm DCM compliant capacity and velocity values the site has been modeled in StormCAD using the Standard head loss method and head loss values taken from Table 9-4 of the DCM. HGL profiles modeled in StormCAD are included in Appendix C.

Table 9-4. STORMCAD Standard Method Coefficients

Bend Loss		
Bend Angle	K Coefficient	
0°	0.05	
22.5°	0.10	
45°	0.40	
60°	0.64	
90°	1.32	
LATERAL LOSS		
One Lateral K Coefficient		
Bend Angle	Non-surcharged	Surcharged
45°	0.27	0.47
60°	0.52	0.90
90°	1.02	1.77
Two Laterals K Coefficient		
45°	0.96	
60°	1.16	
90°	1.52	

III. Project Characteristics

a. BASIN LOCATION AND FLOWS

The Hay Creek Valley site is found on the southern border of the Beaver Creek Drainage Basin. In addition to the 214.6-acre site, there are off-site basins east, west, and south of the site that contribute a total tributary area of 98.5 acres. The Hay Creek Valley Road & Storm improvements are anticipated to disturb approximately 17.28 acres.

b. MAJOR DRAINAGEWAYS

Beaver Creek

The majority of the Hay Creek Valley site is located within the Beaver Creek Drainage Basin. Runoff generated within this basin presently flows overland with slopes ranging from 5 to 50% until reaching an existing natural drainage swale located within the site. This drainage swale directs the sites flows internally until discharging from the site near the northeastern corner. Drainage from the developed road will be directed to Pond 1, where the runoff will be treated for water quality and detained to maintain the historic major event discharge rate from the site.

Air Force Academy

The area along the southeastern border of the site drains southeast into the Air Force Academy Major Drainage Basin. Runoff generated within this basin presently flows overland with slopes ranging from 15 to 45% until exiting the site to the southeast into the adjacent property.

c. LAND USES

Presently, the site is unplatted and consists mostly of undeveloped land. The 214.6-acre area is entirely zoned RR-5. The site will consist of residential lots containing 5-acres or more and three tracts, one containing the proposed Pond 1, one containing the proposed roadway, and the other containing the Preble's mouse habitat which is undevelopable.

IV. BASIN HYDROLOGY

- a. The **Pre-development conditions** for the Hay Creek Valley site have been analyzed and are presented by design points and are described as follows:

Predevelopment conditions have been analyzed using both the SCS Hydrograph Procedure and the routed Rational Method. The existing conditions will discuss the entry of runoff from off-site basins as it relates to the respective design point. Runoff generated, either on-site or off-site, drains overland towards the northeastern corner of the site where it is captured by the existing natural swale that runs northeast, exiting the site and releasing flows to be collected in Hay Creek. Generally, all undeveloped basins are considered to be vegetated with sparse grasses. A delineation of the basin boundaries can be found in Appendix D in drawings DR-01 and DR-02. Runoff calculations can be found in Appendix A. The existing runoff design points are described below:

Design Point 1 (Rational ($Q_5 = 3.5$ cfs, $Q_{100} = 18.9$ cfs) HEC-HMS ($Q_5 = 0.8$ cfs, $Q_{100} = 4.1$ cfs)) (sub-basin: EX-OS1a; Area: 9.4 Ac.) (Slopes: 5 to 15%) This point represents the discharge from offsite sub-basin EX-OS1a into the site. Stormwater runoff will sheet flow to the east and into sub-basin EX-1.

Design Point 2 (Rational ($Q_5 = 12.3$ cfs, $Q_{100} = 74.6$ cfs) HEC-HMS ($Q_5 = 2.6$ cfs, $Q_{100} = 15.7$ cfs)) (sub-basin: EX-OS1b; Area: 59.2 Ac.) (Slopes: 5 to 10%) This point represents the discharge from offsite sub-basin EX-OS1b into the site. Stormwater runoff will sheet flow to the east and into sub-basin EX-2.

Design Point 3 (Rational ($Q_5 = 7.8$ cfs, $Q_{100} = 42.0$ cfs) HEC-HMS ($Q_5 = 1.5$ cfs, $Q_{100} = 8.0$ cfs)) (sub-basin: EX-OS2a; Area: 15.9 Ac.) (Slopes: 20 to 50%) This point represents the discharge from

offsite sub-basin EX-OS2a into the site. Stormwater runoff will sheet flow to the north and into sub-basin EX-2.

Design Point 4 (Rational ($Q_5 = 1.3$ cfs, $Q_{100} = 6.7$ cfs) HEC-HMS ($Q_5 = 0.3$ cfs, $Q_{100} = 1.4$ cfs)) (sub-basin: EX-OS2b; Area: 2.8 Ac.) (Slopes: 10 to 40%) This point represents the discharge from offsite sub-basin EX-OS2b into the site. Stormwater runoff will sheet flow to the north and into sub-basin EX-3.

Design Point 5 (Rational ($Q_5 = 1.6$ cfs, $Q_{100} = 8.2$ cfs) HEC-HMS ($Q_5 = 0.3$ cfs, $Q_{100} = 1.6$ cfs)) (sub-basin: EX-OS2c; Area: 3.2 Ac.) (Slopes: 10 to 50%) This point represents the discharge from offsite sub-basin EX-OS2c into the site. Stormwater runoff will sheet flow to the north and into sub-basin EX-3.

Design Point 6 (Rational ($Q_5 = 2.7$ cfs, $Q_{100} = 17.7$ cfs) HEC-HMS ($Q_5 = 0.2$ cfs, $Q_{100} = 2.0$ cfs)) (sub-basin: EX-OS3; Area: 8.1 Ac.) (Slopes: 10 to 45%) This point represents the discharge from offsite sub-basin EX-OS3 into the site. Stormwater runoff will sheet flow to the west and into sub-basin EX-5.

Design Point 7 (Rational ($Q_5 = 2.3$ cfs, $Q_{100} = 15.6$ cfs) HEC-HMS ($Q_5 = 2.3$ cfs, $Q_{100} = 15.6$ cfs)) (sub-basin: EX-4; Area: 5.9 Ac.) (Slopes: 10 to 50%) This point represents the discharge from sub-basin EX-4 into the adjacent property. Stormwater runoff will sheet flow to the south and into the adjacent property then continue south along historic paths.

Design Point 8 (Rational ($Q_5 = 26.1$ cfs, $Q_{100} = 153.1$ cfs) HEC-HMS ($Q_5 = 5.4$ cfs, $Q_{100} = 30.9$ cfs)) (sub-basins: EX-OS1b, EX-OS2a, EX-2; Area: 123.3 Ac.) (Slopes: 5 to 30%) This point represents the combined discharge from sub-basins EX-OS1b, EX-OS2a, and EX-2 into sub-basin EX-1. Stormwater runoff will sheet flow to the north to combine with the flows from sub-basin EX-1 before continuing along historic paths.

Design Point 9 (Rational ($Q_5 = 17.6$ cfs, $Q_{100} = 106.5$ cfs) HEC-HMS ($Q_5 = 2.9$ cfs, $Q_{100} = 17.4$ cfs)) (sub-basins: EX-OS2b, EX-OS2c, EX-3; Area: 67.6 Ac.) (Slopes: 5 to 60%) This point represents the combined discharge from sub-basins EX-OS2b, EX-OS2c, and EX-3 into sub-basin EX-1. Stormwater runoff will sheet flow to the north to combine with the flows from sub-basin EX-1 before continuing along historic paths.

Design Point 10 (Rational ($Q_5 = 13.5$ cfs, $Q_{100} = 85.3$ cfs) HEC-HMS ($Q_5 = 1.8$ cfs, $Q_{100} = 11.9$ cfs)) (sub-basins: EX-OS3, EX-5; Area: 51.0 Ac.) (Slopes: 5 to 50%) This point represents the combined discharge from sub-basins EX-OS3, and EX-5 into sub-basin EX-1. Stormwater runoff will sheet flow to the north to combine with the flows from sub-basin EX-1 before continuing along historic paths.

Design Point 11 (Rational ($Q_5 = 35.0$ cfs, $Q_{100} = 210.9$ cfs) HEC-HMS ($Q_5 = 11.2$ cfs, $Q_{100} = 69.9$ cfs)) (sub-basins: EX-OS1a, EX-OS1b, EX-OS2a, EX-OS2b, EX-OS2c, EX-OS3, EX-1, EX-2, EX-3, EX-5; Area: 307.3 Ac.) (Slopes: 5 to 50%) This point represents the total discharge from the site. Stormwater runoff is collected in a natural swale and directed to the northeast. The channelized flow exits the site near the northeast corner of the site and continues north before draining into Hay Creek approximately 300 feet north of the site.

Design Point HC ($Q_{100} = 127$ cfs) This point represents the stormwater flows in Hay Creek at the existing private 24"x36" culvert to the north of the site. The existing private 24"x36" culvert conveys the flows in Hay Creek under the existing access road. The proposed flows in Hay Creek come from the HEC-2 analysis provided by the Regional Floodplain Administrator. The HEC-2 results are included in **Appendix C**.

b. The ***fully developed conditions*** for the site are as follows:

Post development conditions have been analyzed using both the SCS Hydrograph Procedure and the rational routed flow. The proposed conditions will discuss the entry of runoff from off-site basins as it relates to the respective design point. Runoff generated, either on-site or off-site, drains overland towards the northeastern corner of the site where it is captured by the existing natural swale that runs northeast, exiting the site and releasing flows to be collected in Hay Creek. Generally, the developed lots are considered to be residential lots containing 5 acres or more, having an imperviousness of 7.0%. Sub-basins PR-8a and PR-8b, which contain the proposed roadway and ditch, have an imperviousness of 66.4%. Sub basins PR-9, and PR-10, containing the proposed Pond 1 and open space are considered to have an imperviousness of 2.0%. A delineation of the basin boundaries can be found in Appendix D in drawing DR-03. Runoff calculations can be found in Appendix A. The proposed runoff design points are described below:

Design Point 1 (Rational ($Q_5 = 3.5$ cfs, $Q_{100} = 18.9$ cfs) HEC-HMS ($Q_5 = 0.8$ cfs, $Q_{100} = 3.6$ cfs)) (sub-basin: OS1a; Area: 9.4 Ac.) (Slopes: 5 to 15%) This point represents the discharge from offsite sub-basin OS1a into the site. Stormwater runoff will sheet flow to the east and into sub-basin PR-1b.

Design Point 2 (Rational ($Q_5 = 12.3$ cfs, $Q_{100} = 74.6$ cfs) HEC-HMS ($Q_5 = 2.6$ cfs, $Q_{100} = 14.2$ cfs)) (sub-basin: OS1b; Area: 59.2 Ac.) (Slopes: 5 to 10%) This point represents the discharge from offsite sub-basin OS1b into the site. Stormwater runoff will sheet flow to the east and into sub-basin PR-1a.

Design Point 3 (Rational ($Q_5 = 2.2$ cfs, $Q_{100} = 12.4$ cfs) HEC-HMS ($Q_5 = 0.4$ cfs, $Q_{100} = 1.8$ cfs)) (sub-basin: OS2a; Area: 5.0 Ac.) (Slopes: 20 to 50%) This point represents the discharge from offsite sub-basin OS2a into the site. Stormwater runoff will sheet flow to the north and into sub-basin PR-1a.

Design Point 4 (Rational ($Q_5 = 2.2$ cfs, $Q_{100} = 12.0$ cfs) HEC-HMS ($Q_5 = 0.4$ cfs, $Q_{100} = 2.0$ cfs)) (sub-basin: OS2b; Area: 4.7 Ac.) (Slopes: 20 to 50%) This point represents the discharge from offsite sub-basin OS2b into the site. Stormwater runoff will sheet flow to the north and into sub-basin PR-2.

Design Point 5 (Rational ($Q_5 = 3.3$ cfs, $Q_{100} = 17.0$ cfs) HEC-HMS ($Q_5 = 0.8$ cfs, $Q_{100} = 2.7$ cfs)) (sub-basin: OS2c; Area: 6.1 Ac.) (Slopes: 20 to 50%) This point represents the discharge from offsite sub-basin OS2c into the site. Stormwater runoff will sheet flow to the north and into sub-basin PR-3.

Design Point 6 (Rational ($Q_5 = 1.3$ cfs, $Q_{100} = 6.7$ cfs) HEC-HMS ($Q_5 = 0.3$ cfs, $Q_{100} = 1.1$ cfs)) (sub-basin: OS2d; Area: 2.8 Ac.) (Slopes: 10 to 50%) This point represents the combined discharge from offsite sub-basin OS2d into the site. Stormwater runoff will sheet flow to the north and into sub-basin PR-4.

Design Point 7 (Rational ($Q_5 = 1.6$ cfs, $Q_{100} = 8.0$ cfs) HEC-HMS ($Q_5 = 0.3$ cfs, $Q_{100} = 1.3$ cfs)) (sub-basin: OS2e; Area: 3.2 Ac.) (Slopes: 5 to 40%) This point represents the discharge from sub-basin OS2e into the site. Stormwater runoff will sheet flow to the north and into sub-basin PR-5.

Design Point 8 (Rational ($Q_5 = 1.5$ cfs, $Q_{100} = 10.1$ cfs) HEC-HMS ($Q_5 = 0.1$ cfs, $Q_{100} = 0.8$ cfs)) (sub-basins: OS3a; Area: 4.9 Ac.) (Slopes: 10 to 45%) This point represents the discharge from sub-basin OS3a into the site. Stormwater runoff will sheet flow to the west and into sub-basin PR-8.

Design Point 9 (Rational ($Q_5 = 1.1$ cfs, $Q_{100} = 7.6$ cfs) HEC-HMS ($Q_5 = 0.1$ cfs, $Q_{100} = 0.8$ cfs)) (sub-basins: OS3b; Area: 3.3 Ac.) (Slopes: 10 to 45%) This point represents the discharge from sub-basin OS3b into the site. Stormwater runoff will sheet flow to the west and into sub-basin PR-8.

Design Point 10 (Rational ($Q_5 = 3.1$ cfs, $Q_{100} = 17.0$ cfs)) (sub-basins: PR-8; Area: 5.9 Ac.) (Slopes: 10 to 50%) This point represents the discharge from sub-basin PR-8 into the adjacent property. Stormwater runoff will sheet flow to the south and into the adjacent property then continue south along historic paths. Note: HEC-HMS values not calculated for this basin due to size and its not being tributary to the site infrastructure.

Design Point 11 (Rational ($Q_5 = 15.0$ cfs, $Q_{100} = 88.8$ cfs) HEC-HMS ($Q_5 = 3.7$ cfs, $Q_{100} = 19.5$ cfs)) (sub-basins: OS1b, OS2a, PR-1a; Area: 77.2 Ac.) (Slopes: 5 to 30%) This point represents the flows from sub-basins OS1b, OS2a and PR-1a that are directed to the proposed private type C inlet at DP-11. The flows collected at DP-11 are conveyed northeast via proposed private 30-inch CMP storm drain towards Design Point 12b (DP-12b).

Design Point 12a (Rational ($Q_5 = 4.9$ cfs, $Q_{100} = 26.8$ cfs) HEC-HMS ($Q_5 = 1.3$ cfs, $Q_{100} = 6.1$ cfs)) (sub-basins: OS1a, PR-1b; Area: 15.6 Ac.) (Slopes: 5 to 30%) This point represents the flows from sub-basins OS1a, and PR-1b that are directed to the proposed private type D inlet at DP-12a. The flows collected at DP-12a are combined with the flows from DP-11 at DP-12b before being conveyed to the east via proposed private 30-inch CMP storm drain which outfalls via a proposed private flared end section that directs the flows to the east along historic paths.

Design Point 12b (Rational ($Q_5 = 18.4$ cfs, $Q_{100} = 107.5$ cfs) HEC-HMS ($Q_5 = 5.0$ cfs, $Q_{100} = 25.2$ cfs)) (sub-basins: OS1a, OS2a, OS1b, PR-1a, PR-1b; Area: 92.8 Ac.) (Slopes: 5 to 30%) This point represents the combination of the flows collected at DP-11 and DP-12a. The combined flows are conveyed to the east via proposed private 30-inch CMP storm drain which outfalls via a proposed private flared end section that discharges to a riprap splash pad before continuing along historic paths at a velocity of 4.95 ft/s which is compliant with DCM values for stable channel flow. See appendix A for supporting calculations.

Design Point 13 (Rational ($Q_5 = 4.6$ cfs, $Q_{100} = 24.8$ cfs) HEC-HMS ($Q_5 = 1.1$ cfs, $Q_{100} = 5.3$ cfs)) (sub-basins: OS2b, PR-2; Area: 12.5 Ac.) (Slopes: 5 to 30%) This point represents the flows from sub-basins OS2b and PR-2 that have been collected in the roadside ditch that runs along the south

side of the proposed roadway. The roadside ditch located upstream of Design Point 12 will be lined with vegetation. These flows travel northeast in the ditch before being collected in the proposed private Type C inlet at Design Point 13 (DP-13). These flows are conveyed under the proposed roadway to the north, discharging via a proposed private 18-inch FES that discharges to a riprap splash pad before continuing along historic paths at a velocity of 3.77 ft/s which is compliant with DCM values for stable channel flow. See appendix A for supporting calculations.

Design Point 14 (Rational ($Q_5 = 7.9$ cfs, $Q_{100} = 42.7$ cfs) HEC-HMS ($Q_5 = 1.4$ cfs, $Q_{100} = 5.5$ cfs)) (sub-basins: OS2c, PR-3; Area: 24.6 Ac.) (Slopes: 5 to 30%) This point represents the flows from sub-basins OS2c and PR-3 that have been collected in the roadside ditch that runs along the south side of the proposed roadway. The roadside ditch along this stretch will be lined with vegetation. These flows travel northeast in the ditch before being collected in the proposed private Type C inlet at Design Point 14 (DP-14). These flows are conveyed under the proposed roadway to the north, discharging via a proposed private 18-inch FES that discharges to a riprap splash pad before continuing along historic paths at a velocity of 3.91 ft/s which is compliant with DCM values for stable channel flow. See appendix A for supporting calculations.

Design Point 15 (Rational ($Q_5 = 5.5$ cfs, $Q_{100} = 29.8$ cfs) HEC-HMS ($Q_5 = 1.5$ cfs, $Q_{100} = 6.6$ cfs)) (sub-basins: OS2d, PR-4; Area: 17.0 Ac.) (Slopes: 5 to 60%) This point represents the flows from sub-basins OS2d, and PR-4 that have been collected in the roadside ditch that runs along the south side of the proposed roadway. The roadside ditch along this stretch will be lined with vegetation. These flows travel northeast in the ditch before being collected in the proposed private type C inlet at Design Point 15 (DP-15). These flows are conveyed under the proposed roadway to the north, discharging via a proposed private 18-inch FES that discharges to a riprap splash pad before continuing along historic paths at a velocity of 3.90 ft/s which is compliant with DCM values for stable channel flow. See appendix A for supporting calculations.

Design Point 16 (Rational ($Q_5 = 6.4$ cfs, $Q_{100} = 34.4$ cfs) HEC-HMS ($Q_5 = 1.6$ cfs, $Q_{100} = 7.2$ cfs)) (sub-basins: OS2e, PR-5; Area: 20.1 Ac.) (Slopes: 5 to 60%) This point represents the flows from sub-basins OS2e, and PR-5 that have been collected in the roadside ditch that runs along the south side of the proposed roadway. The roadside ditch along this stretch will be lined with vegetation. These flows travel northeast in the ditch before being collected in the proposed private type C inlet at Design Point 16 (DP-16). These flows are conveyed under the proposed roadway to the north, discharging via a proposed private 18-inch FES that discharges to a riprap splash pad before continuing along historic paths at a velocity of 4.01 ft/s which is compliant with DCM values for stable channel flow. See appendix A for supporting calculations.

Design Point 17 (Rational ($Q_5 = 4.9$ cfs, $Q_{100} = 26.9$ cfs) HEC-HMS ($Q_5 = 1.1$ cfs, $Q_{100} = 5.0$ cfs)) (sub-basin: PR-6; Area: 13.9 Ac.) (Slopes: 5 to 60%) This point represents the flows from sub-basin PR-6 that have been collected in the roadside ditch that runs along the south side of the proposed roadway. The roadside ditch along this stretch will be lined with vegetation. These flows travel northeast in the ditch before being collected in the proposed private Type C Inlet at Design Point 17 (DP-17). These flows are conveyed under the proposed roadway to the north, discharging via a proposed private 18-inch FES that discharges to a riprap splash pad before continuing along historic paths at a velocity of 2.95 ft/s which is compliant with DCM values for stable channel flow. See appendix A for supporting calculations.

Design Point 18 (Rational ($Q_5 = 9.0$ cfs, $Q_{100} = 49.0$ cfs) HEC-HMS ($Q_5 = 2.3$ cfs, $Q_{100} = 10.6$ cfs)) (sub-basin: PR-7; Area: 27.8 Ac.) (Slopes: 5 to 60%) This point represents the flows from sub-basin PR-7 that have been collected in the roadside ditch that runs along the south side of the proposed roadway. The roadside ditch along this stretch will be lined with vegetation. These flows travel northeast in the ditch before being collected in the proposed private Type C Inlet at Design Point 18 (DP-18). These flows are conveyed under the proposed roadway to the north via proposed private 18" RCP, to be combined with the flows collected at Design Point 20a (DP-20a).

Design Point 19 (Rational ($Q_5 = 5.5$ cfs, $Q_{100} = 31.7$ cfs) HEC-HMS ($Q_5 = 1.2$ cfs, $Q_{100} = 6.1$ cfs)) (sub-basin: OS3a, PR-9; Area: 18.3 Ac.) (Slopes: 5 to 10%) This point represents the flows from sub-basins OS3a, and PR-9 that have been collected in the roadside ditch that runs along the south side of the proposed roadway downstream of DP-19. The roadside ditch along this stretch will be lined with vegetation. These flows travel northeast in the ditch before being collected in the proposed private 18" FES at Design Point 19 (DP-19). These flows are conveyed under the proposed roadway to the west via proposed private 18" RCP.

Design Point 20 (Rational ($Q_5 = 29.8$ cfs, $Q_{100} = 166.4$ cfs) HEC-HMS ($Q_5 = 9.5$ cfs, $Q_{100} = 43.0$ cfs)) (sub-basins: OS1a, OS1b, OS2a, OS2b, OS2c, OS2d, OS2e, PR-1a, PR-1b, PR-1c, PR-2, PR-3, PR-4, PR-5, PR-6; Area: 234.0 Ac.) (Slopes: 5 to 50%) This point represents the outfall from the proposed private swale located along the northwestern border of the proposed private pond (Pond 1). The combined flows from sub-basins OS1a, OS1b, OS2a, OS2b, OS2c, OS2d, OS2e, PR-1a, PR-1b, PR-1c, PR-2, PR-3, PR-4, PR-5, and PR-6, are collected in the proposed swale and diverted around Pond 1 toward the proposed stilling basin at Design Point 22. The proposed swale will be lined with Type L Rip Rap.

Design Point 21a (Rational ($Q_5 = 5.2$ cfs, $Q_{100} = 10.8$ cfs) HEC-HMS ($Q_5 = 3.3$ cfs, $Q_{100} = 7.8$ cfs)) (sub-basins: PR-10a; Area: 4.7 Ac.) (Slopes: 2.8 to 6%) This point represents the Proposed Private Type-C inlet located on the north side of the proposed roadway southwest of the proposed pond (Pond 1). All flows from the proposed roadway will drain to the north and into the northern roadside ditch. All runoff from the proposed roadway will drain to the Type C inlet at Design Point 21a. The flows collected in the inlet will be conveyed downstream towards the proposed private forebay at Design Point EDB-IN via proposed private 18-inch RCP pipe.

Design Point 21b (Rational ($Q_5 = 10.4$ cfs, $Q_{100} = 40.4$ cfs) HEC-HMS ($Q_5 = 5.3$ cfs, $Q_{100} = 17.9$ cfs)) (sub-basins: PR-7, PR-10a; Area: 32.5 Ac.) (Slopes: 2.8 to 6%) This point represents the combination of flows from design points 18 and 21a. The combined flows will be conveyed downstream towards the proposed private forebay at Design Point EDB-IN via proposed private 18-inch RCP pipe.

Design Point EDB-IN (Rational ($Q_5 = 11.4$ cfs, $Q_{100} = 44.5$ cfs) HEC-HMS ($Q_5 = 4.9$ cfs, $Q_{100} = 16.5$ cfs)) (sub-basins: PR-7, PR-10a, PR-10b, PR-11; Area: 35.3 Ac.) (Slopes: 2.8 to 50%) This point represents the total discharge into the Proposed Private Extended Detention Basin (EDB). Flows will be treated for water quality and released at such a rate that the overall discharge from the site does not increase under proposed conditions.

Design Point EDB-OUT ($Q_5 = 4.0$ cfs, $Q_{100} = 15.9$ cfs) (sub-basins: PR-7, PR-10a, PR-10b, PR-11; Area: 35.3 Ac.) (Slopes: 2.8 to 50%) This point represents the discharge from the EDB. The

discharge from Pond 1 will be routed downstream via proposed private 18-inch RCP pipe that will convey the flows to the proposed private stilling basin located at Design Point 22.

Design Point 22 (Rational ($Q_5 = 50.2$ cfs, $Q_{100} = 260.0$ cfs) HEC-HMS ($Q_5 = 10.9$ cfs, $Q_{100} = 49.1$ cfs) (design points: DP-EDB-OUT, DP-19, DP-20; Area: 283.1 Ac.) (Slopes: 2.8 to 50%) This point represents the proposed private stilling basin located north of proposed Pond 1. Flows from Design Points 19, 20, and EDB-OUT all discharge to the stilling basin which will release the flows at a velocity of 1.57 ft/sec.

Design Point 23 (Rational ($Q_5 = 56.1$ cfs, $Q_{100} = 295.8$ cfs) HEC-HMS ($Q_5 = 11.5$ cfs, $Q_{100} = 53.0$ cfs)) (sub-basins: DP-22, OS3b, PR-12; Area: 302.8 Ac.) (Slopes: 2.8 to 60%) This point represents the total discharge from the site. Stormwater runoff from the site will continue north in the existing channel before draining into Hay Creek, a tributary of Beaver Creek.

Design Point HC ($Q_{100} = 127$ cfs) This point represents the stormwater flows in Hay Creek at the proposed private 48" x 48" Box culvert to the north of the site. The proposed private 48" x 48" culvert conveys the flows in Hay Creek under the proposed access road. Culvert calculations for the existing and proposed culvert at DP-HC can be found in Appendix A. The proposed flows in Hay Creek come from the HEC-2 analysis provided by the Regional Floodplain Administrator. The HEC-2 results are included in **appendix C**.

Notes:

- **MHFD-Detention Analysis for the proposed detention pond (Pond 1) which will be constructed as part of the Improvements associated with Hay Creek Valley can be found in Appendix A of this report.**
- **Though the storm pipes are shown as RCP they have been modeled in StormCAD as CMP with a Manning's n of 0.024 which is a more conservative design. Based on the analysis, CMP pipes can be used in place of RCP.**
- **Tables summarizing inlet sizes and capacities, storm pipe sizes and capacities and swale capacities for the proposed improvements can be found in Appendix A and/or in the following section.**
- **All ponds and associated infrastructure are to be owned and maintained by the HOA.**
- **The ratio of the total site discharge in proposed conditions vs existing conditions is 0.8, representing no significant increase in flows in the proposed condition.**
- **The hydraulic model for Beaver Creek indicated approximately 127 cfs from the entire Hay Creek tributary basin which contains the development. We therefore believe the above hydrological analysis with the Rational Method to be quite conservative.**

V. Hydraulic Analysis

a. Proposed Inlets

This project will use Type-C inlets in sump conditions. Sump inlet capacities have been determined utilizing the nomographs available from the El Paso County Drainage Criteria Manual Volume 1 (DCM). Riprap protection has been provided around all inlets to minimize erosion. To ensure a

suitable outfall from each storm drain, outlet protection sized according to the criteria set forth by the DCM has been provided at the outfall of each storm drain. The stormwater velocities at each discharge point have been calculated to ensure the outfalls are suitable. See design point descriptions for further details. The table below lists inlets by design point and corresponding capacity. Figure 1 shows the capacities for Type C inlets in sump conditions.

INLET SUMMARY										
HAY CREEK VALLEY										
DESIGN POINT or SUB-BASIN	SUB-BASINS/ DESCRIPTION	TOTAL AREA (AC)	INLET			Q(5) TOTAL INFLOW	Q5 INLET CAPACITY	Q(100) BYPASS FLOWS (cfs)	Q(100) TOTAL INFLOW (cfs)	MAX INLET CAPACITY
			SIZE (Ft.)	TYPE	CONDITION					
11	OS1b, OS2a, PR-1a	77.15	3x6	D	SUMP	3.7	39.0	0.0	19.5	39.0
12a	OS1a, PR-1b	15.63	3x3	D	SUMP	1.3	39.0	0.0	6.1	39.0
13	OS2b, PR-2	12.51	3x3	C	SUMP	1.1	16.0	0.0	5.3	16.0
14	OS2c, PR-3	24.32	3x3	C	SUMP	1.4	16.0	0.0	5.5	16.0
15	OS2d, PR-4	17.18	3x3	C	SUMP	1.5	16.0	0.0	6.6	16.0
16	OS2e, PR-5	20.09	3x3	C	SUMP	1.6	16.0	0.0	7.2	16.0
17	PR-6	20.01	3x3	C	SUMP	1.1	16.0	0.0	5.0	16.0
18	PR-7	21.64	3x3	C	SUMP	2.3	16.0	0.0	10.6	16.0
21a	PR-8a	4.73	3x3	C	SUMP	3.3	16.0	0.0	7.8	16.0

Note: Inlet sizes indicated are minimums. Larger sizes may be used in the construction plans for conservative design.

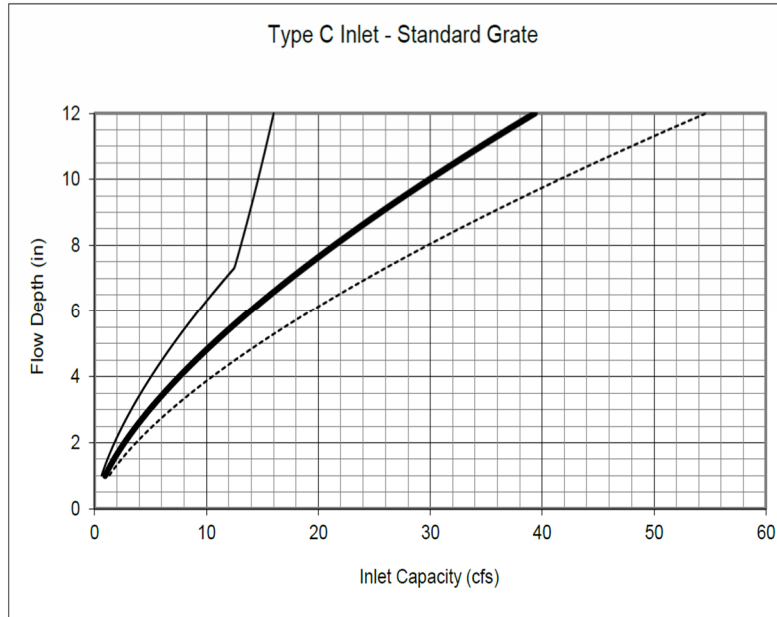


Figure 1

<i>Inlet Overflow Routing</i>	
<i>Inlet</i>	<i>Overflow Routing Under Sump Inlet Blockage Conditions</i>
11	Blockage of this inlet will cause runoff to surcharge the sump and direct runoff across the proposed driveway and into the type C inlet at DP-12a.
12a	Blockage of this inlet will cause runoff to surcharge the sump and direct runoff across the proposed driveway to the east before continuing along historic paths.
13	Blockage of this inlet will cause runoff to surcharge the sump and direct runoff to the east and continue in the roadside swale before being collected in the proposed Type C inlet at DP-14.
14	Blockage of this inlet will cause runoff to surcharge the sump and direct runoff to the east and continue in the roadside swale before being collected in the proposed Type C inlet at DP-15.
15	Blockage of this inlet will cause runoff to surcharge the sump and direct runoff to the east and continue in the roadside swale before being collected in the proposed Type C inlet at DP-16.
16	Blockage of this inlet will cause runoff to surcharge the sump and direct runoff to the east and continue in the roadside swale before being collected in the proposed Type C inlet at DP-17.
17	Blockage of this inlet will cause runoff to surcharge the sump and direct runoff to the east and continue in the roadside swale before being collected in the proposed Type C inlet at DP-18.
18	Blockage of this inlet will cause runoff to surcharge the sump and direct runoff across the proposed roadway and into the type C inlet at DP-21a.
21a	Blockage of this inlet will cause runoff to surcharge the sump and direct runoff into the proposed Extended Detention Basin.

b. Swales

The initial swale analysis was performed using Hydraflow Express to determine flow depths and velocities. Per the El Paso County DCM Volume 1, Chapter 6, section 6.5.2. Channel Velocity, **“Concrete, riprap, or soil cement linings as approved by the City/County shall be used where channel bottom velocities exceed 6.0 ft/sec. Grass lined channels shall not be used where**

velocity exceeds permissible velocities in Table 10-4 or the Froude number is greater than 0.9 for the 100-year storm.” Table 10-4 is included in Appendix B for reference. Concentrated stormwater flows that drain along the existing drainage path through sub-basin PR-1c will be collected in a proposed swale west of proposed Pond 1 which will direct the flows around the pond. A 25-foot drainage easement will extend from proposed lot 1 to lot 9 along the existing drainage path to ensure that future developments do not impede the flow of stormwater through the site. The swale calculations have been applied to the most critical swale scenarios for the Site. The table below summarizes the various swales included as part of these improvements.

Swale Capacities						
HAY CREEK VALLEY						
Design Point	Armoring Type	Anticipated Slope %	CHANNEL CAPACITY MAJOR STORM (cfs)	Q(100) TOTAL FLOW (cfs)	Q(100) VELOCITY (FT/S)	Q100 Flow Depth (ft)
13	Vegetation	5.0%	5.3	5.3	3.87	0.32
14	Vegetation	4.6%	5.5	5.5	3.86	0.33
15	Vegetation	4.6%	6.6	6.6	4.13	0.36
16	Vegetation	4.6%	7.2	7.2	4.19	0.38
17	Vegetation	4.9%	5.0	5.0	3.80	0.31
18	Vegetation	4.9%	10.6	10.6	4.76	0.46
19	Vegetation	2.6%	6.1	6.1	3.21	0.41
20	Vegetation	2.0%	43.0	43.0	4.20	0.69
21a	Vegetation	4.8%	7.8	7.8	4.39	0.39

Note:

1. Basins have been reconfigured as compared to those discussed in the approved MDDP to design for stable velocities under vegetated conditions.
2. Flows at design point 20 exceed 15 cfs and are therefore provided a drainage easement.

c. Driveway Culverts

Upon the development of the proposed lots, it will be necessary to place culverts along the roadside ditches to convey flows through driveways. Initial calculations for driveway culvert sizing at each lot is summarized in the table below:

Driveway Culvert Sizes HAY CREEK VALLEY			
Lot	Q(100) TOTAL FLOW IN DITCH (cfs)	Anticipated Slope %	Minimum Culvert Inside Diameter (in)
1-10	7.8	4.8%	18
11-12	5.3	5.0%	18
13	9.2	4.8%	18
14-15	6.7	4.8%	18
16	7.2	4.8%	18
17-18	7.9	4.8%	18
19	9.7	4.8%	18
20	6.1	2.7%	18

d. Storm Pipes

Preliminary sizing of storm sewer has been completed using the Manning’s channel flow calculation. To confirm DCM compliant capacity and velocity values the site has been modeled in StormCAD using the Standard head loss method and head loss values taken from Table 9-4 of the DCM. HGL profiles modeled in StormCAD are included in Appendix C. Outfall protection has been provided at discharge points in accordance with DCM standards. Outfall protection calculations are included in Appendix A. All outfalls have been designed to provide flow velocities consistent with a stable and suitable outfall.

e. Detention

Due to the development of the site and the resulting increase in imperviousness, detention will be required to limit the 100-year discharge to historic rates. The proposed private Extended Detention Basin (Pond 1) has been designed to over detain stormwater flows to reduce the total site discharge to predevelopment levels. The pond will provide detention and water quality treatment for stormwater runoff generated within the Hay Creek Valley site. The proposed private Forebay at the southwestern corner of Pond 1 has been sized based on the untreated WQCV calculated in the UD-BMP worksheet. The forebay calculations and UD-BMP worksheet can be found in Appendix A. The proposed private trickle channel has been sized to accommodate double the release from the proposed private forebay at a minimum. Trickle channel calculations are included in Appendix A. Pond 1 will outfall to a stilling basin to the north. Flows from the pond will combine with flows from Design Points 19 and 20 in the stilling basin which will release the flows with a velocity of 1.57 ft/sec during the 100-year storm event which is considered by the DCM to be stable for the major storm event in the context of open channel flows. The stilling basin will provide a suitable outfall for the concentrated flows into the existing natural swale. The proposed stilling basin has a depth of 24-inches and, when at max capacity, will infiltrate the full volume within 40 hours. Infiltration rates have been determined using the f_0 value for type B soils in Table 6-7 of the MHFD Drainage Criteria Manual Volume 1 (below). Design information including calculations are included in Appendix A. The table below summarizes the detention provided for this development.

Proposed Pond Summary HAY CREEK VALLEY								
Pond	Tributary Area	% Impervious	Pre-Development Peak		Pond Outflow		Pre vs. Post Ratio	
			Q5	Q100	Q5	Q100	Q5	Q100
Pond 1	29.19	17.08	6.2	28.2	2.7	15.2	0.4	0.5

Table 6-7. Recommended Horton's equation parameters

NRCS Hydrologic Soil Group	Infiltration (inches per hour)		Decay Coefficient— <i>a</i>
	Initial— <i>f_i</i>	Final— <i>f_o</i>	
A	5.0	1.0	0.0007
B	4.5	0.6	0.0018
C	3.0	0.5	0.0018
D	3.0	0.5	0.0018

Emergency Overflow

Pond 1: If the emergency overflow weir receives flows, these flows will continue downstream along the existing natural swale and drain into Hay Creek.

VI. Storm Water Quality

Per the DCM Volume 2, Section 4.1, El Paso County recommends the MHFD Four Step Process for receiving water protection that focuses on reducing runoff by disconnecting impervious area, eliminating “unnecessary” impervious area and encouraging infiltration into soils that are suitable, treat and slowly release the WQCV, stabilize stream channels, and implement source controls. The four-step process has been completed below.

Step 1: **Employ Runoff Reduction Practices.**

- The low-density nature of this development and the fact that none of the streets will have curb and gutter, means that most, if not all, runoff from impervious surfaces will sheet flow across pervious areas to grass buffers. The grass buffers, located alongside the proposed roadway will provide runoff reduction for the impervious areas that drain to them.

Step 2: **Stabilize Drainageways.**

- The site is in the Beaver Creek Drainage Fee Basin. Drainage fees, to be paid by the relevant Hay Creek Valley developers at the time of platting, will help fund proposed channel improvements. Information on planned future improvements to the Beaver Creek channel was unavailable for this report.

Step 3: Provide Water Quality Capture Volume (WQCV).

- As required by the DCM, runoff from the proposed streets which is feasible to detain, is directed into a proposed detention pond (Pond 1) via grass lined swale. The pond has been designed to meet the DCM standards for the release rates of Full Spectrum Detention Ponds for Water Quality Capture Volumes, and all of the other storm events listed in the MHFD- Detention spreadsheet. Exclusions are listed below:
 - The lots containing large lot residential sites are excluded from WQ treatment per section I.7.1.b.5 of the ECM.
 - Disturbed areas that are not practicable to detain are excluded from WQ treatment per section I.7.1.C.1.a.

Runoff reduction calculations have been provided for those portions of the proposed roadway that are not being detained to show compliance with the DCM requirements for treatment of the WQCV. Runoff Reduction calculations can be found in Appendix A.

Step 4: Consider Need for Industrial and Commercial BMPs.

- There are no commercial or industrial components of this development, therefore no BMPs of this nature are required.

VII. Erosion Control Plan

A grading and erosion control plan (GEC) for the proposed improvements will be submitted for review as separate submittals by the various developments. These will incorporate straw wattles, straw bale check dams, silt fence, vehicle tracking control, inlet & outlet control, sedimentation basins and other best management practices (CMs) identified in the DCM Volume 2.

VIII. Floodplains

Per the *Flood Insurance Rate Map (FIRM) 08041CO267 G*, effective date December 7, 2018, published by the Federal Emergency Management Agency (FEMA), Hay Creek, a Tributary to Beaver Creek runs along the northern bound of the Hay Creek Valley area and has designated 100-year floodplain. The developed portion of the site is generally not touched by the 100 year floodplain, however the road improvements associated with this site will cross the FEMA floodplain at the location where the site access easement crosses Hay Creek. Draft model backed BFEs for this area have been developed as part of Phase 1 for the ongoing El Paso County, CO, Risk MAP Project. The data have been reviewed and approved through FEMA's QA/QC process (May 11, 2022) and are currently in the MIP (Case No. 19-08-0037s). This data is considered "FEMA APPROVED BFEs" This will need to be shown on the prelim plan and plat. Refer to the map in Appendix C.

IX. Fee Development

a. UNDEVELOPED PLATTABLE LAND

The Hay Creek Valley site is located within the Beaver Creek Drainage Fee Basin and within previously unplatted land. The 2024 Drainage Basin Fees for the Beaver Creek Drainage Fee Basin are: \$14,846/impervious acre for the Drainage Fee and \$0.00/impervious acre for the Bridge Fee. Per the *El Paso County Engineering Criteria Manual*, Appendix L, Section 3.10.1a Fee Reductions for Low Density Lots, with the site being developed into 5-acre lots, drainage fees may be reduced by 25%.

<p style="text-align: center;">Hay Creek Final Drainage Report 2024 Drainage and Bridge Fees</p>							
	Platted Area (Acres)	Imperviousness (%)	Platted Area (Imp. ac.)	Fee/ Imp. Acre	Fee Due	Drainage Fee Reduction	Fee Due at Platting
Drainage Fee	214.63	8.12	17.428	\$14,846.00	\$258,735.43	\$64,683.86	\$194,051.58
Bridge Fee	214.63	8.12	17.428	\$0.00	\$0.00	\$0.00	\$0.00
TOTAL							<u>\$194,051.58</u>

Cost Estimate

Table 12.1

Engineer's Estimate of Probable Construction Costs				
BEAVER CREEK				
HAY CREEK VALLEY				
Private Non-Reimbursable				
Item	Unit	Quantity	Unit Cost	Extension
18" RCP/HP	LF	632	\$76.00	\$48,032.00
30" RCP/HP	LF	133	\$114.00	\$15,162.00
36" RCP/HP	LF	185	\$140.00	\$25,900.00
18" FES	EA	7	\$456.00	\$3,192.00
30" FES	EA	1	\$684.00	\$684.00
36" FES	EA	1	\$840.00	\$840.00
Type C Inlet	EA	6	\$6,000.00	\$36,000.00
Type D Inlet	EA	2	\$7,000.00	\$14,000.00
RIPRAP	CY	900	\$135.00	\$121,500.00
Sub Total				\$265,310.00
10% Contingency				\$26,531.00
TOTAL:				\$291,841.00

Engineer's Estimate of Probable Construction Costs				
BEAVER CREEK				
HAY CREEK VALLEY				
Permanent BMP (EDB): Private Non-reimbursable				
Item	Unit	Quantity	Unit Cost	Extension
DETENTION POND GRADING	EA	1	\$35,000.00	\$35,000.00
3' TRICKLE CHANNEL	LF	310	\$250.00	\$77,500.00
FOREBAY	EA	1	\$40,000.00	\$40,000.00
OUTLET STRUCTURE	EA	1	\$40,000.00	\$40,000.00
EMERGENCY SPILLWAY	EA	1	\$5,000.00	\$5,000.00
STILLING BASIN	EA	1	\$30,000.00	\$30,000.00
Sub Total				\$227,500.00
10% Contingency				\$22,750.00
TOTAL:				\$250,250.00
Overall Total				\$542,091.00

Since the engineer has no control over the cost of labor, materials, equipment, or services furnished by others, or over the contractor's method of determining prices, or over the competitive bidding or market conditions, the opinion of probable construction costs provided herein are made on the basis of the engineer's experience and qualifications and represents the best judgment as an experienced and qualified professional familiar with the construction industry. The engineer cannot, and does not guarantee that proposals, bid or actual construction costs will not vary from the opinion of probable costs.

X. Summary

This report demonstrates that the proposed infrastructure associated with Hay Creek Valley is in conformance with the El Paso County Drainage Criteria Manual, Volumes 1 and 2, October 2018 and all previously approved studies related to the project site. Stormwater flows will generally remain the same in post-development conditions (Rational ($Q_5 = 53.7$ cfs, $Q_{100} = 282.8$ cfs) HEC-HMS ($Q_5 = 12.9$ cfs, $Q_{100} = 59.4$ cfs)) as in pre-development conditions (Rational ($Q_5 = 35.0$ cfs, $Q_{100} = 210.9$ cfs) HEC-HMS ($Q_5 = 11.2$ cfs, $Q_{100} = 69.9$ cfs)). These proposed improvements should not adversely affect downstream or surrounding developments and are in conformance with the pertinent studies for the area.

XI. References

1. ***El Paso County and City of Colorado Springs Drainage Criteria Manual, Volume 1 & 2***, El Paso County, May 2014
2. ***El Paso County Engineering Criteria Manual***, El Paso County, Rev. December 2016
3. ***Web Soil Survey of El Paso County Area, Colorado. Unites States Department of Agriculture Soil Conservation Service.***
4. ***Flood Insurance Rate Maps for El Paso County, Colorado and Incorporated Areas, Panel 279 of 1275, Federal Emergency Management Agency***, Effective Date December 7, 2018.
5. ***Urban Storm Drainage Criteria Manual, Vol. 1-3*** by Urban Drainage and Flood Control District (UDFCD), January 2016

Appendices

APPENDIX A

HYDROLOGIC AND HYDRAULIC CALCULATIONS

Project Name: Hay Creek
 Project Location: El Paso County, Colorado
 Designer: WCG
 Notes: EXISTING CONDITIONS

Channel Flow Type Key
 Heavy Meadow 2
 Tillage/Field 3
 Short Pasture and Lawns 4
 Nearly Bare Ground 5
 Grassed Waterway 6
 Paved Areas 7

Avg. Channel Velocity 4 ft/s (If specific channel vel is used, this will be ignored)
 Avg. Slope for Initial Flow 0.04 ft/ft (If Elevations are used, this will be ignored)

Sub-basin	Comments	Area		Soil Group	Rational 'C' Values										Flow Lengths						Average (decimal) Slope	Initial Tc (min)	Average (%) Slope	Channel Flow Type (See Key above)	Velocity (ft/s)	Channel Tc (min)	Total Tc (min)	Rainfall Intensity & Rational Flow Rate						Sub-basin		
		sf	acres		7%			100%			2%				Initial	True Initial Length ft	Channel ft	True Channel Length ft	i5 in/hr	Q5 cfs								HEC-HMS Q5 cfs	i100 in/hr	Q100 cfs	HEC-HMS Q100 cfs					
					C5	C100	Area (SF)	C5	C100	Area (SF)	C5	C100	Area	C5																		C100	Percent Impervious			
EX-OS1a		407292	9.35	0.0146	B	0.12	0.39	407292	0.90	0.96		0.09	0.36		0.12	0.39	7.00%	300	300	672	672	0.10	14.23	9.9	4	2.20	5.09	19.31	3.07	3.5	0.8	5.15	18.9	4.1	EX-OS1a	
EX-OS1b		2579029	59.21	0.0925	B	0.12	0.39	1173596	0.90	0.96		0.09	0.36	1405433	0.10	0.37	4.28%	300	300	2754	2754	0.07	16.48	6.7	4	1.81	25.33	41.80	1.99	12.3	2.6	3.34	74.6	15.7	EX-OS1b	
EX-OS2a		692771	15.90	0.0248	B	0.12	0.39		0.90	0.96	25423	0.09	0.36	667348	0.12	0.38	5.60%	300	300	84	84	0.31	9.70	31.3	4	3.50	0.40	10.10	4.09	7.8	1.5	6.87	42.0	8.0	EX-OS2a	
EX-OS2b		120503	2.77	0.0043	B	0.12	0.39		0.90	0.96	6033	0.09	0.36	114470	0.13	0.39	6.91%	300	300	113	113	0.15	12.31	14.8	4	2.69	0.70	13.00	3.69	1.3	0.3	6.20	6.7	1.4	EX-OS2b	
EX-OS2c		137929	3.17	0.0049	B	0.12	0.39		0.90	0.96	6548	0.09	0.36	131381	0.13	0.39	6.65%	268	268	0	0	0.17	11.09	17.2	4	2.90	0.00	11.09	3.94	1.6	0.3	6.62	8.2	1.6	EX-OS2c	
EX-OS3		354850	8.15	0.0127	B	0.12	0.39		0.90	0.96	475	0.09	0.36	354375	0.09	0.36	2.13%	300	300	265	265	0.16	12.54	15.8	4	2.78	1.59	14.12	3.56	2.7	0.2	5.98	17.7	2.0	EX-OS3	
EX-OS4		30365	0.70	0.0011	B	0.12	0.39		0.90	0.96	20445	0.09	0.36	9920	0.64	0.76	67.98%	175	175	0	0	0.02	8.78	2.0	7	2.83	0.00	8.78	4.30	1.9		7.23	3.9		EX-OS4	
EX-1		2441168	56.04	0.0876	B	0.12	0.39		0.90	0.96	30061	0.09	0.36	2411107	0.10	0.37	3.21%	300	300	4763	4763	0.05	18.23	5.0	4	1.57	50.72	68.94	1.44	8.2	1.6	2.43	50.4	9.9	EX-1	
EX-2		2100638	48.22	0.0754	B	0.12	0.39		0.90	0.96	46438	0.09	0.36	2054200	0.11	0.37	4.17%	300	300	2795	2795	0.06	16.66	6.4	4	1.77	26.31	42.96	1.96	10.3	2.0	3.29	59.7	11.2	EX-2	
EX-3		2684942	61.64	0.0963	B	0.12	0.39		0.90	0.96	31890	0.09	0.36	2653052	0.10	0.37	3.16%	300	300	2002	2002	0.11	13.86	11.4	4	2.36	14.12	27.97	2.52	15.6	2.3	4.23	96.6	14.7	EX-3	
EX-4		256265	5.88	0.0092	B	0.12	0.39		0.90	0.96	0	0.09	0.36	256265	0.09	0.36	2.00%	206	206	0	0	0.29	8.53	28.6	4	3.50	0.00	8.53	4.35	2.3	2.3	7.30	15.6	15.6	EX-4	
EX-5		1865454	42.82	0.0669	B	0.12	0.39		0.90	0.96	18117	0.09	0.36	1847337	0.10	0.37	2.95%	300	300	1427	1427	0.11	14.18	10.7	4	2.29	10.39	24.56	2.71	11.4	1.5	4.55	71.8	10.0	EX-5	
DESIGN POINTS	Sub-basins																																			DESIGN POINTS
1	EX-OS1a	407292	9.35	0.0146	B	0.12	0.39	407292	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.0%	300	300	672	672	0.10	14.23	9.9	4	2.20	5.09	19.31	3.07	3.5	0.8	5.15	18.9	4.1	1	
2	EX-OS1b	2579029	59.21	0.0925	B	0.12	0.39	1173596	0.90	0.96	0	0.09	0.36	1405433	0.10	0.37	4.3%	300	300	2754	2754	0.07	16.48	6.7	4	1.81	25.33	41.80	1.99	12.3	2.6	3.34	74.6	15.7	2	
3	EX-OS2a	692771	15.90	0.0248	B	0.12	0.39		0.90	0.96	25423	0.09	0.36	667348	0.12	0.38	5.6%	300	300	84	84	0.31	9.70	31.3	4	3.50	0.40	10.10	4.09	7.8	1.5	6.87	42.0	8.0	3	
4	EX-OS2b	120503	2.77	0.0043	B	0.12	0.39		0.90	0.96	6033	0.09	0.36	114470	0.13	0.39	6.9%	300	300	113	113	0.15	12.31	14.8	4	2.69	0.70	13.00	3.69	1.3	0.3	6.20	6.7	1.4	4	
5	EX-OS2c	137929	3.17	0.0049	B	0.12	0.39		0.90	0.96	6548	0.09	0.36	131381	0.13	0.39	6.7%	268	268	0	0	0.17	11.09	17.2	4	2.90	0.00	11.09	3.94	1.6	0.3	6.62	8.2	1.6	5	
6	EX-OS3	354850	8.15	0.0127	B	0.12	0.39		0.90	0.96	475	0.09	0.36	354375	0.09	0.36	2.1%	300	300	265	265	0.16	12.54	15.8	4	2.78	1.59	14.12	3.56	2.7	0.2	5.98	17.7	2.0	6	
7	EX-4	256265	5.88	0.0092	B	0.12	0.39		0.90	0.96	0	0.09	0.36	256265	0.09	0.36	2.0%	206	206	0	0	0.29	8.53	28.6	4	3.50	0.00	8.53	4.35	2.3	2.3	7.30	15.6	15.6	7	
8	EX-OS1b, EX-OS2a, EX-2	5372438	123.33	0.1927	B	0.12	0.39	1173596	0.90	0.96	71861	0.09	0.36	4126981	0.11	0.37	4.4%	300	300	2795	2795	0.06	16.67	6.4	4	1.77	26.31	42.97	1.96	26.1	5.4	3.29	153.1	30.9	8	
9	EX-OS2b, EX-OS2c, EX-3	2943374	67.57	0.1056	B	0.12	0.39		0.90	0.96	44471	0.09	0.36	2898903	0.10	0.37	3.5%	300	300	2002	2002	0.11	13.82	11.4	4	2.36	14.12	27.93	2.52	17.6	2.9	4.24	106.5	17.4	9	
10	EX-OS3, EX-5	2220304	50.97	0.0796	B	0.12	0.39		0.90	0.96	18592	0.09	0.36	2201712	0.10	0.37	2.8%	300	300	1427	1427	0.11	14.19	10.7	4	2.29	10.39	24.58	2.71	13.5	1.8	4.55	85.3	11.9	10	
11	DP-8, DP-9, DP-10, EX-OS1a, EX-1	13384576	307.27	0.4801	B	0.12	0.39	1580888	0.90	0.96	164985	0.09	0.36	11638703	0.10	0.37	3.8%	300	300	8062	8062	0.05	18.17	5.0	4	1.57	85.84	104.01	1.09	35.0	11.2	1.84	210.9	69.9	11	
HC	From PPRBD Regional Floodplane Administrator																																	127.0	127.0	HC

Rational Method - Proposed Conditions

Project Name: Hay Creek
 Project Location: El Paso County, Colorado
 Designer: WCG
 Notes: Proposed Condition

Average Channel Velocity: 4.00 ft/s
 Average Slope for Initial Flow: 0.04 ft/ft (If specific channel vel is used, this will be ignored) (If Elevations are used, this will be ignored)

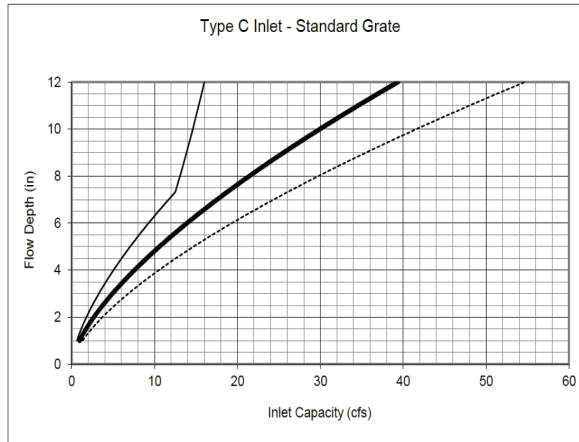
Heavy Meadow 2
Tillage/Field 3
Short Pasture and Lawns 4
Nearly Bare Ground 5
Grassed Waterway 6
Paved Areas 7

Sub-basin	Comments	Area		Soil Group	Rational 'C' Values													Flow Lengths						Rainfall Intensity & Rational Flow Rate						Sub-basin						
		sf	acres		5-Acre Lots (7% Impervious)		Pavement (100% Impervious)			Undeveloped/Pervious Areas (2% Impervious)			Composite		Percent Impervious	Initial		Channel		Average (decimal)		Average (%) Slope	Channel Flow Type (See Key above)		Velocity (ft/s)	Channel Tc (min)	Tc (min)	i5	Q5		HEC-HMS Q5	i100	Q100	HEC-HMS Q100		
					C5	C100	C5	C100	Area (SF)	C5	C100	Area	C5	C100		ft	Length ft	ft	Length ft	Slope	Tc (min)		Slope	Ground Type											Slope	Ground Type
					Initial	True Initial	Channel	True Channel	Initial	Initial	Average (%)	Ground Type	Slope	Ground Type		Total	i5	Q5	HEC-HMS Q5	i100	Q100		HEC-HMS Q100													
OS1a		407292	9.35	0.0146	B	0.12	0.39	407292	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	300	300	672	672	0.10	14.23	9.9	4	2.20	5.09	19.31	3.07	3.5	0.8	5.15	18.9	3.6	OS1a	
OS1b		2579029	59.21	0.0925	B	0.12	0.39	1173596	0.90	0.96	0	0.09	0.36	1405433	0.10	0.37	4.28%	300	300	2754	2754	0.07	16.48	6.7	4	1.81	25.33	41.80	1.99	12.3	2.6	3.34	74.6	14.2	OS1b	
OS2a		218316	5.01	0.0078	B	0.12	0.39	0	0.90	0.96	6435	0.09	0.36	211881	0.11	0.38	4.89%	300	300	203	203	0.25	10.54	24.8	4	3.49	0.97	11.51	3.88	2.2	0.4	6.52	12.4	1.8	OS2a	
OS2b		206454	4.74	0.0074	B	0.12	0.39	0	0.90	0.96	7795	0.09	0.36	198659	0.12	0.38	5.70%	300	300	33	33	0.20	11.18	20.4	4	3.16	0.17	11.34	3.90	2.2	0.4	6.56	12.0	2.0	OS2b	
OS2c		266071	6.11	0.0095	B	0.12	0.39	0	0.90	0.96	11200	0.09	0.36	254871	0.12	0.39	6.13%	280	280	0	0	0.35	8.99	35.0	4	3.50	0.00	8.98	4.27	3.3	0.8	7.17	17.0	2.7	OS2c	
OS2d		120503	2.77	0.0043	B	0.12	0.39	0	0.90	0.96	6033	0.09	0.36	114470	0.13	0.39	6.91%	300	300	44	44	0.13	12.72	13.4	4	2.56	0.29	13.01	3.69	1.3	0.3	6.19	6.7	1.1	OS2d	
OS2e		137929	3.17	0.0049	B	0.12	0.39	0	0.90	0.96	6548	0.09	0.36	131381	0.13	0.39	6.65%	285	285	0	0	0.15	11.87	15.4	4	2.75	0.00	11.86	3.83	1.6	0.3	6.44	8.0	1.3	OS2e	
OS3a		212463	4.88	0.0076	B	0.12	0.39	0	0.90	0.96	0	0.09	0.36	212463	0.09	0.36	2.00%	300	300	27	27	0.09	15.39	8.6	4	2.05	0.22	15.61	3.40	1.5	0.1	5.71	10.1	0.9	OS3a	
OS3b		143157	3.29	0.0051	B	0.12	0.39	0	0.90	0.96	0	0.09	0.36	143157	0.09	0.36	2.00%	300	300	195	195	0.22	11.24	22.0	4	3.28	0.99	12.22	3.79	1.1	0.1	6.36	7.6	0.8	OS3b	
OS4		31990	0.73	0.0011	B	0.12	0.39	20445	0.90	0.96	0	0.09	0.36	11545	0.61	0.74	64.63%	175	175	0	0	0.02	9.30	2.0	7	2.83	0.00	9.30	4.21	1.9	0.9	7.08	3.9	0.8	OS4	
PR-1a		563521	12.94	0.0202	B	0.12	0.39	563521	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	300	300	479	479	0.05	17.75	5.1	4	1.58	5.05	22.80	2.82	4.4	0.9	4.73	24.1	4.3	PR-1a	
PR-1b		273497	6.28	0.0098	B	0.12	0.39	273497	0.90	0.96	0	0.09	0.36	644	0.12	0.39	7.00%	300	300	644	644	0.10	14.33	9.7	4	2.18	4.92	19.24	3.07	2.3	0.5	5.16	12.7	2.5	PR-1b	
PR-1c		2318386	53.22	0.0832	B	0.12	0.39	2318386	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	300	300	3863	3863	0.05	17.64	5.2	4	1.60	40.33	57.96	1.62	10.4	3.4	2.72	56.9	15.7	PR-1c	
PR-2		338377	7.77	0.0121	B	0.12	0.39	338377	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	300	300	576	576	0.12	13.54	11.5	4	2.37	4.04	17.58	3.21	3.0	0.7	5.40	16.5	3.3	PR-2	
PR-3		803224	18.44	0.0288	B	0.12	0.39	803224	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	300	300	1250	1250	0.08	15.28	8.1	4	1.99	10.46	25.73	2.64	5.9	0.7	4.43	32.1	3.3	PR-3	
PR-4		618059	14.19	0.0222	B	0.12	0.39	618059	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	300	300	1290	1290	0.09	14.74	8.9	4	2.09	10.30	25.03	2.68	4.6	1.2	4.50	25.1	5.6	PR-4	
PR-5		737120	16.92	0.0264	B	0.12	0.39	737120	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	300	300	1020	1020	0.08	15.47	7.7	4	1.94	8.75	24.22	2.73	5.6	1.3	4.58	30.5	6.1	PR-5	
PR-6		604142	13.87	0.0217	B	0.12	0.39	604142	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	300	300	1180	1180	0.10	11.86	17.1	4	2.89	6.79	18.65	3.12	5.2	1.1	5.24	28.6	5.0	PR-6	
PR-7		1209380	27.76	0.0434	B	0.12	0.39	1209380	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	300	300	1400	1400	0.10	14.38	9.6	4	2.17	10.76	25.13	2.67	9.0	2.3	4.49	49.0	10.6	PR-7	
PR-8		255265	5.86	0.0092	B	0.12	0.39	255265	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	206	206	0	0	0.29	8.28	28.6	4	3.50	0.00	8.27	4.39	3.1	0.1	7.38	17.0	0.8	PR-8	
PR-9		585132	13.43	0.0210	B	0.12	0.39	585132	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	300	300	1400	1400	0.10	14.18	10.0	4	2.21	10.54	24.72	2.70	4.4	1.1	4.53	23.9	5.1	PR-9	
PR-10a		206019	4.73	0.0074	B	0.12	0.39	0	0.90	0.96	129912	0.09	0.36	76107	0.60	0.74	63.80%	300	300	3558	3558	0.05	9.23	4.8	4	1.53	38.67	47.89	1.83	5.2	3.3	3.07	10.8	7.8	PR-10a	
PR-10b		17696	0.41	0.0006	B	0.12	0.39	0	0.90	0.96	17173	0.09	0.36	523	0.88	0.94	97.10%	50	50	0	0	0.03	1.98	3.0	4	1.21	0.00	5.00	5.10	1.8	0.8	8.58	3.3	1.7	PR-10b	
PR-11		105045	2.41	0.0038	B	0.12	0.39	0	0.90	0.96	0	0.09	0.36	130492	0.11	0.45	2.48%	260	260	0	0	0.01	36.14	0.5	4	0.49	0.00	36.13	2.17	0.6	0.6	3.65	4.0	0.8	PR-11	
PR-12		713346	16.38	0.0256	B	0.12	0.39	0	0.90	0.96	0	0.09	0.36	701369	0.10	0.37	3.65%	300	300	395	395	0.04	19.25	4.2	4	1.43	4.59	23.84	2.75	4.7	0.6	4.62	28.2	3.4	PR-12	
DESIGN POINTS	Sub-Basins																																			DESIGN POINTS
1	OS1a	407292	9.35	0.0146	B	0.12	0.39	407292	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	300	300	672	672	0.10	14.23	9.9	4	2.20	5.09	19.31	3.07	3.5	0.8	5.15	18.9	3.6	1	
2	OS1b	2579029	59.21	0.0925	B	0.12	0.39	1173596	0.90	0.96	0	0.09	0.36	1405433	0.10	0.37	4.28%	300	300	2754	2754	0.07	16.48	6.7	4	1.81	25.33	41.80	1.99	12.3	2.6	3.34	74.6	14.2	2	
3	OS2a	218316	5.01	0.0078	B	0.12	0.39	0	0.90	0.96	6435	0.09	0.36	211881	0.11	0.38	4.89%	300	300	203	203	0.25	10.54	24.8	4	3.49	0.97	11.51	3.88	2.2	0.4	6.52	12.4	1.8	3	
4	OS2b	206454	4.74	0.0074	B	0.12	0.39	0	0.90	0.96	7795	0.09	0.36	198659	0.12	0.38	5.70%	300	300	33	33	0.20	11.18	20.4	4	3.16	0.17	11.34	3.90	2.2	0.4	6.56	12.0	2.0	4	
5	OS2c	266071	6.11	0.0095	B	0.12	0.39	0	0.90	0.96	11200	0.09	0.36	254871	0.12	0.39	6.13%	280	280	0	0	0.35	8.99	35.0	4	3.50	0.00	8.98	4.27	3.3	0.8	7.17	17.0	2.7	5	
6	OS2d	120503	2.77	0.0043	B	0.12	0.39	0	0.90	0.96	6033	0.09	0.36	114470	0.13	0.39	6.91%	300	300	44	44	0.13	12.72	13.4	4	2.56	0.29	13.01	3.69	1.3	0.3	6.19	6.7	1.1	6	
7	OS2e	137929	3.17	0.0049	B	0.12	0.39	0	0.90	0.96	6548	0.09	0.36	131381	0.13	0.39	6.65%	285	285	0	0	0.15	11.87	15.4	4	2.75	0.00	11.86	3.83	1.6	0.3	6.44	8.0	1.3	7	
8	OS3a	212463	4.88	0.0076	B	0.12	0.39	0	0.90	0.96	0	0.09	0.36	212463	0.09	0.36	2.00%	300	300	27	27	0.09	15.39	8.6	4	2.05	0.22	15.61	3.40	1.5	0.1	5.71	10.1	0.9	8	
9	OS3b	143157	3.29	0.0051	B	0.12	0.39	0	0.90	0.96	0	0.09	0.36	143157	0.09	0.36	2.00%	300	300	195	195	0.22	11.24	22.0	4	3.28	0.99	12.22	3.79	1.1	0.1	6.36	7.6	0.8	9	
10	PR-8	255265	5.86	0.0092	B	0.12	0.39	255265	0.90	0.96	0	0.09	0.36	0	0.12	0.39	7.00%	206	206	0	0	0.29	8.28	28.6	4	3.50	0.00	8.27	4.39	3.1	0.1	7.38	17.0	0.8	10	
11	OS1b, OS2a, PR-1a	3360866	77.15	0.1206	B	0.12	0.39	1737117	0.90	0.96	6435	0.09	0.36	1617314	0.11	0.38	4.77%	300	300	3533	3533															

INLET SUMMARY

Hay Creek

DESIGN POINT or SUB-BASIN	SUB-BASINS	TOTAL AREA (AC)	INLET			Q(5) BYPASS FLOWS (cfs)	Q(5) TOTAL INFLOW	Q5 INLET CAPACITY	Q(100) BYPASS FLOWS (cfs)	Q(100) TOTAL INFLOW (cfs)	MAX INLET CAPACITY	NOTES:
			SIZE (Ft.)	TYPE	CONDITION							
11	OS1b, OS2a, PR-1a	77.15	3x6	D	SUMP	0.0	3.70	39.0	0.0	19.50	39.0	
12a	OS1a, PR-1b	15.63	3x3	D	SUMP	0.0	1.30	39.0	0.0	6.10	39.0	
13	OS2b, PR-2	12.51	3x3	C	SUMP	0.0	1.10	16.0	0.0	5.30	16.0	
14	OS2c, PR-3	24.55	3x3	C	SUMP	0.0	1.40	16.0	0.0	5.50	16.0	
15	OS2d, PR-4	16.96	3x3	C	SUMP	0.0	1.50	16.0	0.0	6.60	16.0	
16	OS2e, PR-5	20.09	3x3	C	SUMP	0.0	1.60	16.0	0.0	7.20	16.0	
17	PR-6	13.87	3x3	C	SUMP	0.0	1.10	16.0	0.0	5.00	16.0	
18	PR-7	27.76	3x3	C	SUMP	0.0	2.30	16.0	0.0	10.60	16.0	
21a	PR-10a	4.73	3x3	C	SUMP	0.0	3.30	16.0	0.0	7.80	16.0	



Culvert Report

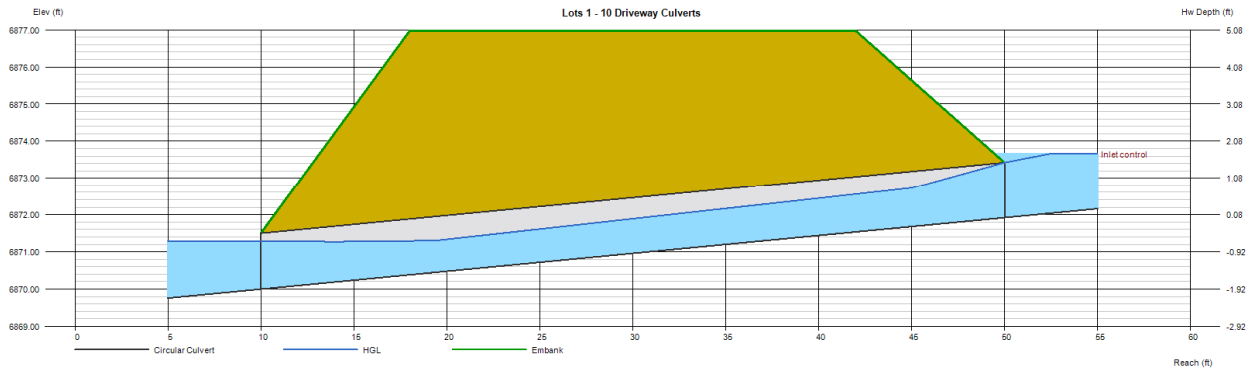
Lots 1 - 10 Driveway Culverts

Invert Elev Dn (ft)	=	6870.00
Pipe Length (ft)	=	40.00
Slope (%)	=	4.80
Invert Elev Up (ft)	=	6871.92
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	1
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment	
Top Elevation (ft)	= 6876.97
Top Width (ft)	= 24.00
Crest Width (ft)	= 30.00

Calculations	
Qmin (cfs)	= 0.00
Qmax (cfs)	= 7.80
Tailwater Elev (ft)	= (dc+D)/2

Highlighted	
Qtotal (cfs)	= 7.80
Qpipe (cfs)	= 7.80
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 4.82
Veloc Up (ft/s)	= 5.72
HGL Dn (ft)	= 6871.29
HGL Up (ft)	= 6873.00
Hw Elev (ft)	= 6873.67
Hw/D (ft)	= 1.16
Flow Regime	= Inlet Control



Culvert Report

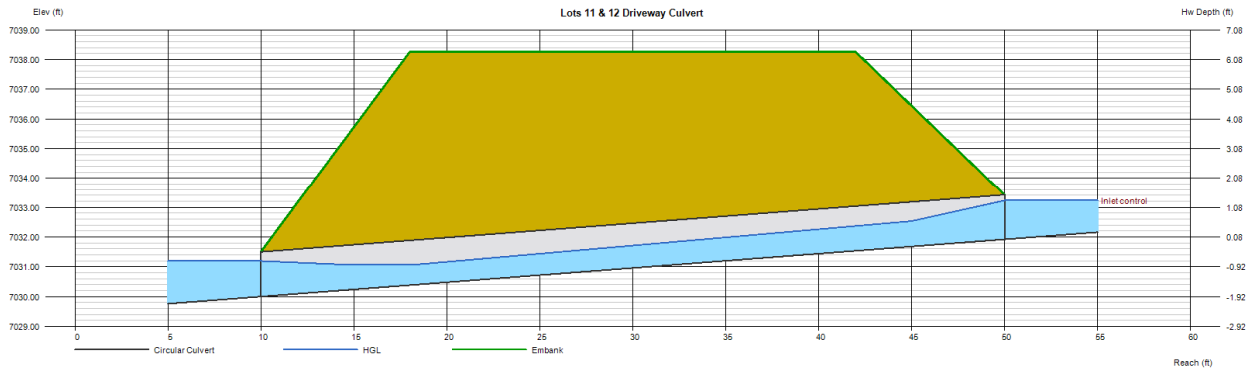
Lots 11 & 12 Driveway Culvert

Invert Elev Dn (ft)	=	7030.00
Pipe Length (ft)	=	40.00
Slope (%)	=	4.80
Invert Elev Up (ft)	=	7031.92
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	1
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment	
Top Elevation (ft)	= 7038.25
Top Width (ft)	= 24.00
Crest Width (ft)	= 30.00

Calculations	
Qmin (cfs)	= 0.00
Qmax (cfs)	= 5.30
Tailwater Elev (ft)	= (dc+D)/2

Highlighted	
Qtotal (cfs)	= 5.30
Qpipe (cfs)	= 5.30
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 3.52
Veloc Up (ft/s)	= 4.88
HGL Dn (ft)	= 7031.19
HGL Up (ft)	= 7032.81
Hw Elev (ft)	= 7033.23
Hw/D (ft)	= 0.87
Flow Regime	= Inlet Control



Culvert Report

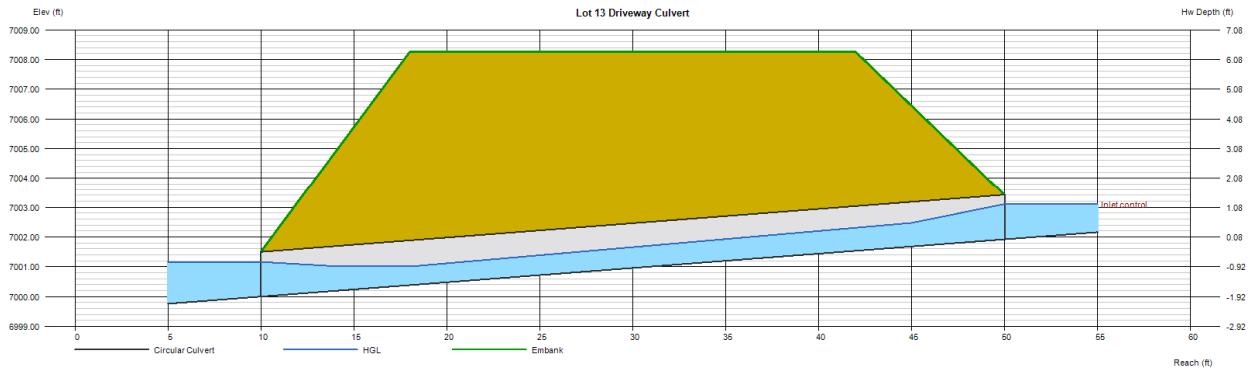
Lot 13 Driveway Culvert

Invert Elev Dn (ft)	=	7000.00
Pipe Length (ft)	=	40.00
Slope (%)	=	4.80
Invert Elev Up (ft)	=	7001.92
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	1
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment	
Top Elevation (ft)	= 7008.25
Top Width (ft)	= 24.00
Crest Width (ft)	= 30.00

Calculations	
Qmin (cfs)	= 0.00
Qmax (cfs)	= 9.20
Tailwater Elev (ft)	= (dc+D)/2

Highlighted	
Qtotal (cfs)	= 4.60
Qpipe (cfs)	= 4.60
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 3.13
Veloc Up (ft/s)	= 4.63
HGL Dn (ft)	= 7001.16
HGL Up (ft)	= 7002.74
Hw Elev (ft)	= 7003.11
Hw/D (ft)	= 0.79
Flow Regime	= Inlet Control



Culvert Report

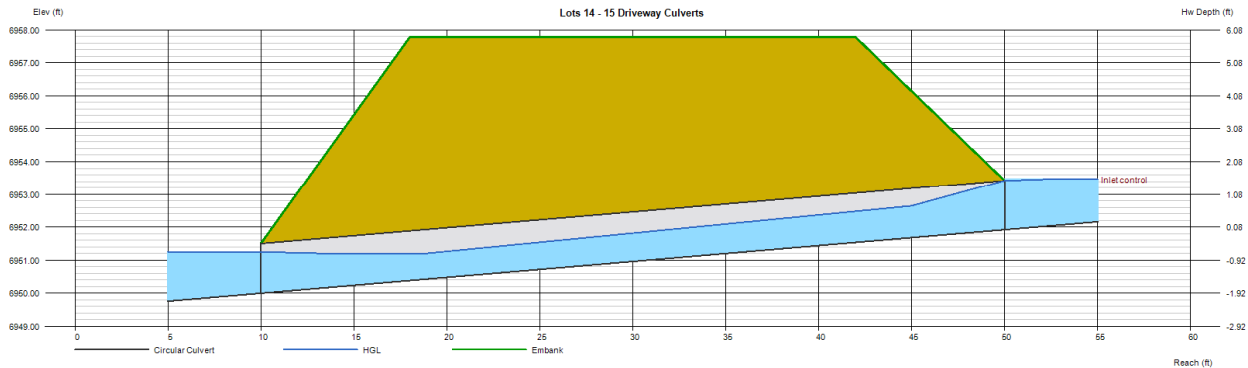
Lots 14 - 15 Driveway Culverts

Invert Elev Dn (ft)	= 6950.00
Pipe Length (ft)	= 40.00
Slope (%)	= 4.80
Invert Elev Up (ft)	= 6951.92
Rise (in)	= 18.0
Shape	= Circular
Span (in)	= 18.0
No. Barrels	= 1
n-Value	= 0.013
Culvert Type	= Circular Concrete
Culvert Entrance	= Square edge w/headwall (C)
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5

Embankment	
Top Elevation (ft)	= 6957.77
Top Width (ft)	= 24.00
Crest Width (ft)	= 30.00

Calculations	
Qmin (cfs)	= 0.00
Qmax (cfs)	= 6.70
Tailwater Elev (ft)	= (dc+D)/2

Highlighted	
Qtotal (cfs)	= 6.70
Qpipe (cfs)	= 6.70
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 4.26
Veloc Up (ft/s)	= 5.35
HGL Dn (ft)	= 6951.25
HGL Up (ft)	= 6952.92
Hw Elev (ft)	= 6953.47
Hw/D (ft)	= 1.03
Flow Regime	= Inlet Control



Culvert Report

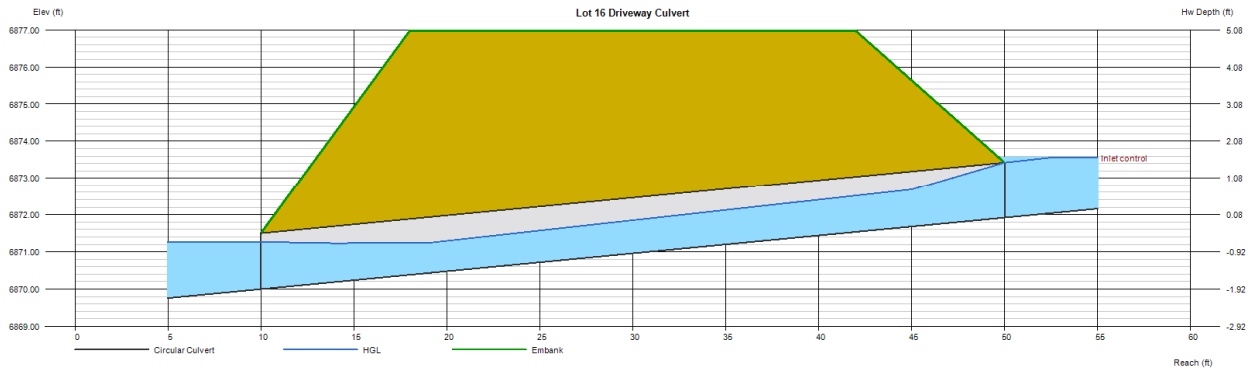
Lot 16 Driveway Culvert

Invert Elev Dn (ft)	=	6870.00
Pipe Length (ft)	=	40.00
Slope (%)	=	4.80
Invert Elev Up (ft)	=	6871.92
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	1
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment	
Top Elevation (ft)	= 6876.97
Top Width (ft)	= 24.00
Crest Width (ft)	= 30.00

Calculations	
Qmin (cfs)	= 0.00
Qmax (cfs)	= 7.20
Tailwater Elev (ft)	= (dc+D)/2

Highlighted	
Qtotal (cfs)	= 7.20
Qpipe (cfs)	= 7.20
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 4.52
Veloc Up (ft/s)	= 5.52
HGL Dn (ft)	= 6871.27
HGL Up (ft)	= 6872.96
Hw Elev (ft)	= 6873.56
Hw/D (ft)	= 1.09
Flow Regime	= Inlet Control



Culvert Report

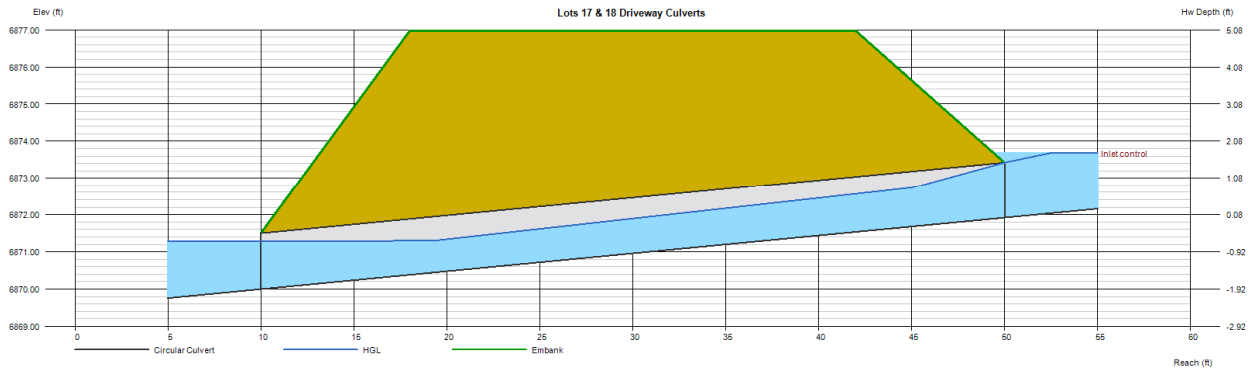
Lots 17 & 18 Driveway Culverts

Invert Elev Dn (ft)	=	6870.00
Pipe Length (ft)	=	40.00
Slope (%)	=	4.80
Invert Elev Up (ft)	=	6871.92
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	1
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment	
Top Elevation (ft)	= 6876.97
Top Width (ft)	= 24.00
Crest Width (ft)	= 30.00

Calculations	
Qmin (cfs)	= 0.00
Qmax (cfs)	= 7.90
Tailwater Elev (ft)	= (dc+D)/2

Highlighted	
Qtotal (cfs)	= 7.90
Qpipe (cfs)	= 7.90
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 4.87
Veloc Up (ft/s)	= 5.75
HGL Dn (ft)	= 6871.29
HGL Up (ft)	= 6873.01
Hw Elev (ft)	= 6873.69
Hw/D (ft)	= 1.18
Flow Regime	= Inlet Control



Culvert Report

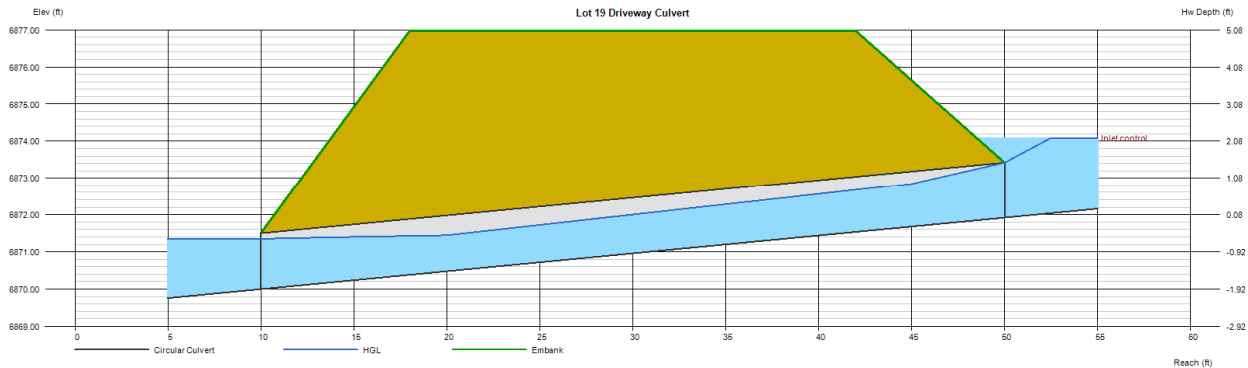
Lot 19 Driveway Culvert

Invert Elev Dn (ft)	= 6870.00
Pipe Length (ft)	= 40.00
Slope (%)	= 4.80
Invert Elev Up (ft)	= 6871.92
Rise (in)	= 18.0
Shape	= Circular
Span (in)	= 18.0
No. Barrels	= 1
n-Value	= 0.013
Culvert Type	= Circular Concrete
Culvert Entrance	= Square edge w/headwall (C)
Coeff. K,M,c,Y,k	= 0.0098, 2, 0.0398, 0.67, 0.5

Embankment	
Top Elevation (ft)	= 6876.97
Top Width (ft)	= 24.00
Crest Width (ft)	= 30.00

Calculations	
Qmin (cfs)	= 0.00
Qmax (cfs)	= 9.70
Tailwater Elev (ft)	= (dc+D)/2

Highlighted	
Qtotal (cfs)	= 9.70
Qpipe (cfs)	= 9.70
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 5.79
Veloc Up (ft/s)	= 6.40
HGL Dn (ft)	= 6871.35
HGL Up (ft)	= 6873.12
Hw Elev (ft)	= 6874.09
Hw/D (ft)	= 1.45
Flow Regime	= Inlet Control



Culvert Report

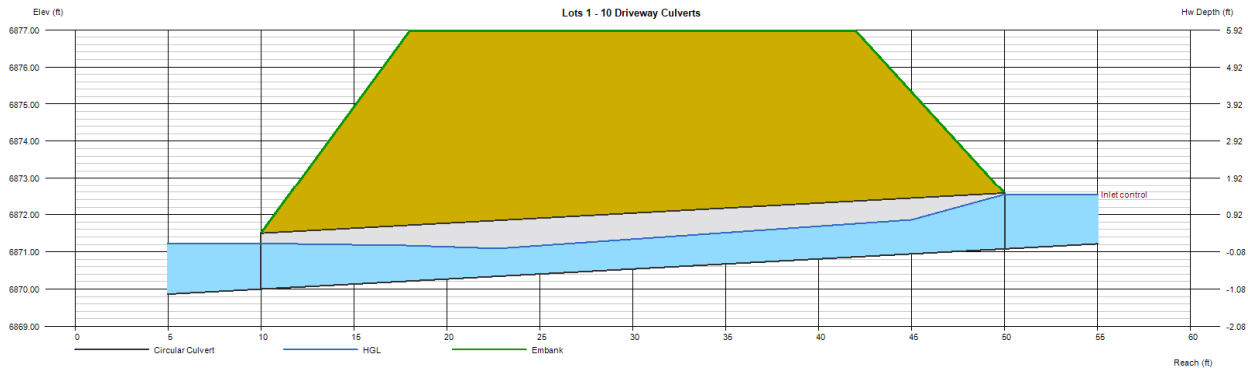
Lot 20 Driveway Culvert

Invert Elev Dn (ft)	=	6870.00
Pipe Length (ft)	=	40.00
Slope (%)	=	2.70
Invert Elev Up (ft)	=	6871.08
Rise (in)	=	18.0
Shape	=	Circular
Span (in)	=	18.0
No. Barrels	=	1
n-Value	=	0.013
Culvert Type	=	Circular Concrete
Culvert Entrance	=	Square edge w/headwall (C)
Coeff. K,M,c,Y,k	=	0.0098, 2, 0.0398, 0.67, 0.5

Embankment	
Top Elevation (ft)	= 6876.97
Top Width (ft)	= 24.00
Crest Width (ft)	= 30.00

Calculations	
Qmin (cfs)	= 0.00
Qmax (cfs)	= 6.10
Tailwater Elev (ft)	= (dc+D)/2

Highlighted	
Qtotal (cfs)	= 6.10
Qpipe (cfs)	= 6.10
Qovertop (cfs)	= 0.00
Veloc Dn (ft/s)	= 3.94
Veloc Up (ft/s)	= 5.15
HGL Dn (ft)	= 6871.23
HGL Up (ft)	= 6872.03
Hw Elev (ft)	= 6872.54
Hw/D (ft)	= 0.97
Flow Regime	= Inlet Control



Culvert Report

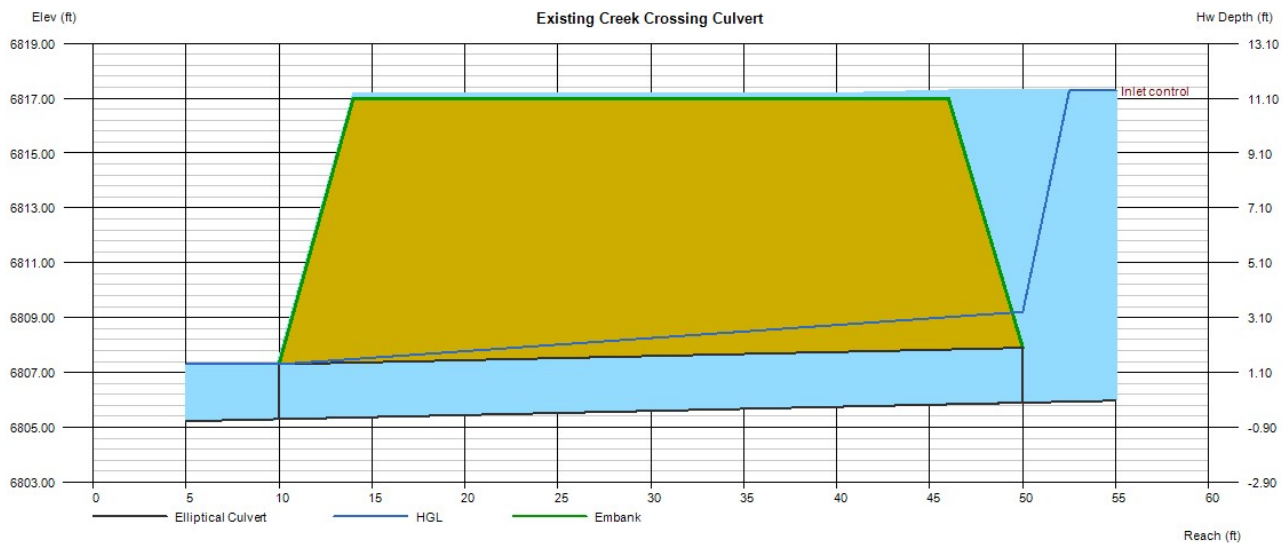
Existing Creek Crossing Culvert

Invert Elev Dn (ft)	= 6805.30
Pipe Length (ft)	= 40.00
Slope (%)	= 1.50
Invert Elev Up (ft)	= 6805.90
Rise (in)	= 24.0
Shape	= Elliptical
Span (in)	= 36.0
No. Barrels	= 1
n-Value	= 0.013
Culvert Type	= Horizontal Ellipse Concrete
Culvert Entrance	= Groove end projecting (H)
Coeff. K,M,c,Y,k	= 0.0045, 2, 0.0317, 0.69, 0.2

Embankment	
Top Elevation (ft)	= 6817.00
Top Width (ft)	= 32.00
Crest Width (ft)	= 100.00

Calculations	
Qmin (cfs)	= 127.00
Qmax (cfs)	= 127.00
Tailwater Elev (ft)	= (dc+D)/2

Highlighted	
Qtotal (cfs)	= 127.00
Qpipe (cfs)	= 83.74
Qovertop (cfs)	= 43.26
Veloc Dn (ft/s)	= 17.77
Veloc Up (ft/s)	= 17.77
HGL Dn (ft)	= 6807.30
HGL Up (ft)	= 6809.21
Hw Elev (ft)	= 6817.28
Hw/D (ft)	= 5.69
Flow Regime	= Inlet Control



Culvert Report

PROPOSED CREEK CROSSING CULVERT

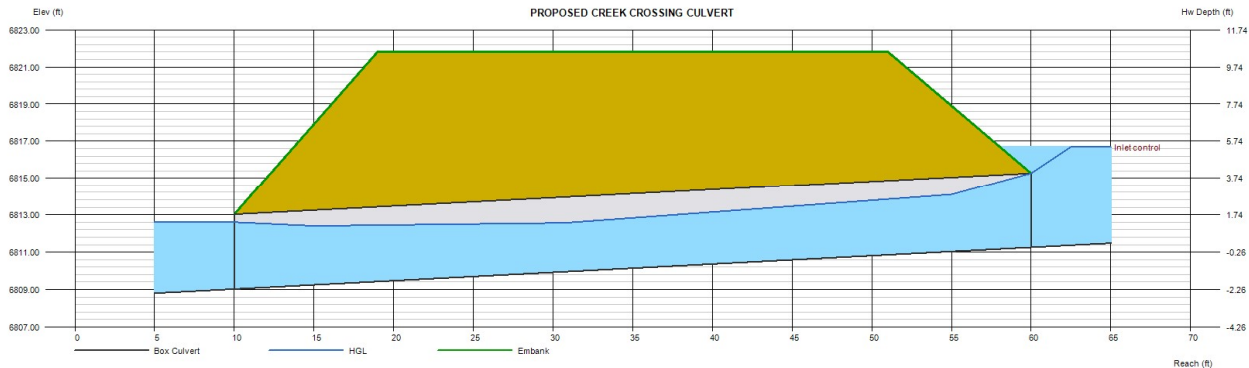
Invert Elev Dn (ft) = 6809.03
 Pipe Length (ft) = 50.00
 Slope (%) = 4.46
 Invert Elev Up (ft) = 6811.26
 Rise (in) = 48.0
 Shape = Box
 Span (in) = 48.0
 No. Barrels = 1
 n-Value = 0.013
 Culvert Type = 90D Headwall,
 Chamfered or Beveled Inlet Edges

Culvert Entrance = 90D headwall w/3/4-in chamfers
 Coeff. K,M,c,Y,k = 0.515, 0.667, 0.0375, 0.79, 0.2

Embankment
 Top Elevation (ft) = 6821.82
 Top Width (ft) = 32.00
 Crest Width (ft) = 100.00

Calculations
 Qmin (cfs) = 127.00
 Qmax (cfs) = 127.00
 Tailwater Elev (ft) = (dc+D)/2

Highlighted
 Qtotal (cfs) = 127.00
 Qpipe (cfs) = 127.00
 Qovertop (cfs) = 0.00
 Veloc Dn (ft/s) = 8.88
 Veloc Up (ft/s) = 10.08
 HGL Dn (ft) = 6812.60
 HGL Up (ft) = 6814.41
 Hw Elev (ft) = 6816.69
 Hw/D (ft) = 1.36
 Flow Regime = Inlet Control

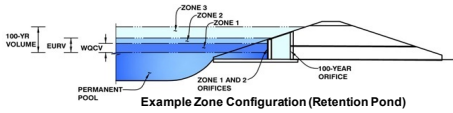


DETENTION BASIN STAGE-STORAGE TABLE BUILDER

MHFD-Detention, Version 4.06 (July 2022)

Project: Hay Creek Valley

Basin ID: Beaver Creek



Watershed Information

Table with watershed parameters: Selected BMP Type (EDB), Watershed Area (35.31 acres), Watershed Length (3,000 ft), etc.

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Table with water quality capture volumes (WQCV) and detention volumes for various return periods (2-yr to 100-yr).

Optional User Overrides

Table with optional user overrides for runoff volumes and detention volumes in inches and acre-feet.

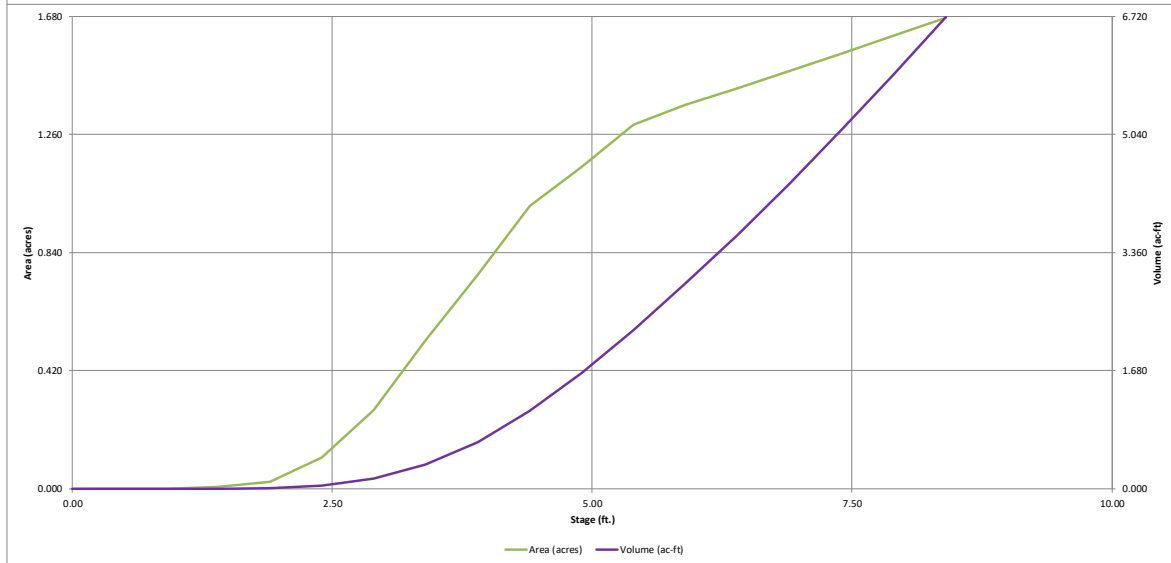
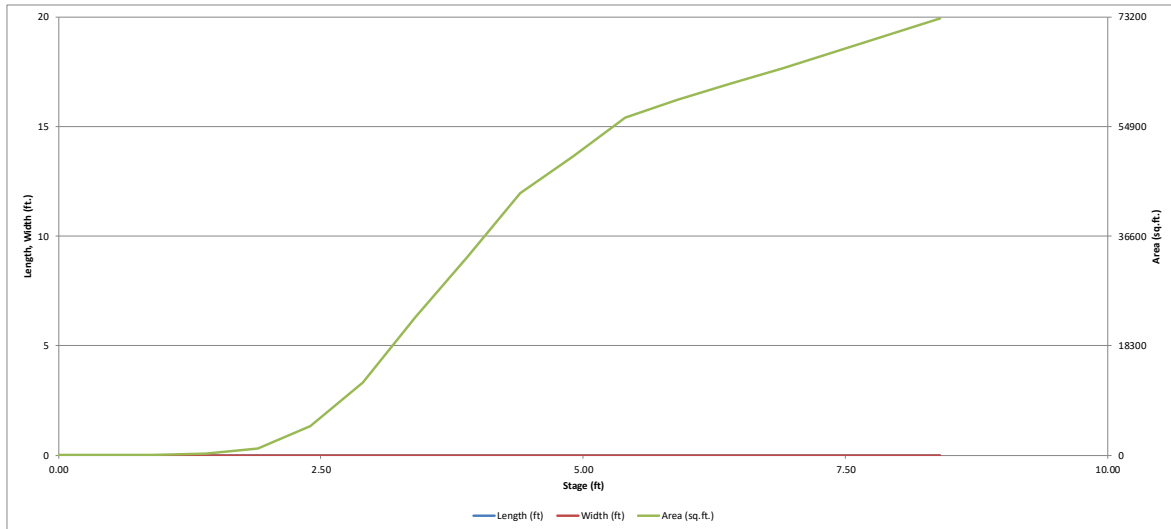
Define Zones and Basin Geometry

Table with basin geometry parameters: Zone 1 Volume (WQCV), Zone 2 Volume (EURV - Zone 1), Zone 3 Volume (100-year - Zones 1 & 2), Total Detention Basin Volume, Initial Surcharge Volume (ISV), Initial Surcharge Depth (ISD), Total Available Detention Depth (Htotal), Depth of Trickle Channel (Htc), Slope of Trickle Channel (Stc), Slopes of Main Basin Sides (Smain), Basin Length-to-Width Ratio (RLW), Initial Surcharge Area (ASV), Surcharge Volume Length (LSV), Surcharge Volume Width (WSV), Depth of Basin Floor (Hflood), Length of Basin Floor (Lflood), Width of Basin Floor (Wflood), Area of Basin Floor (Aflood), Volume of Basin Floor (Vflood), Depth of Main Basin (HMAN), Length of Main Basin (LMAN), Width of Main Basin (WMAN), Area of Main Basin (AMAN), Volume of Main Basin (VMAN), Calculated Total Basin Volume (Vtotal).

Main stage-storage table with columns: Stage - Storage Description, Stage (ft), Optional Override Stage (ft), Length (ft), Width (ft), Area (ft^2), Optional Override Area (ft^2), Area (acre), Volume (ft^3), Volume (ac-ft). Rows include Top of Micropool, 6865.5, 6866, 6867, 6868, 6869, 6870, 6871, 6872, 6873.

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

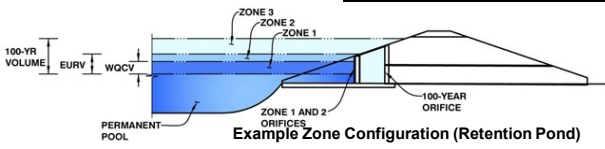
MHFD-Detention, Version 4.06 (July 2022)



DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-*Detention*, Version 4.06 (July 2022)

Project: Hay Creek Valley
Basin ID: Beaver Creek



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	3.27	0.279	Orifice Plate
Zone 2 (EURV)	3.71	0.247	Circular Orifice
Zone 3 (100-year)	4.87	1.081	Weir&Pipe (Restrict)
Total (all zones)		1.607	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain
Underdrain Orifice Area = ft²
Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice = ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate = ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing = inches
Orifice Plate: Orifice Area per Row = sq. inches (diameter = 1 inch)

Calculated Parameters for Plate
WQ Orifice Area per Row = ft²
Elliptical Half-Width = feet
Elliptical Slot Centroid = feet
Elliptical Slot Area = ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.50						
Orifice Area (sq. inches)	0.83	0.83						

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Zone 2 Circular	Not Selected	
Invert of Vertical Orifice =	3.00	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	3.15	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	3.50	N/A	inches

Calculated Parameters for Vertical Orifice
Vertical Orifice Area = ft²
Vertical Orifice Centroid = ft

User Input: Overflow Weir (Dropbox with Flat or Sloped Grate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, H _o =	3.86	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	6.00	N/A	feet
Overflow Weir Grate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	4.00	N/A	feet
Overflow Grate Type =	Type C Grate	N/A	
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir
Height of Grate Upper Edge, H_u = feet
Overflow Weir Slope Length = feet
Grate Open Area / 100-yr Orifice Area =
Overflow Grate Open Area w/o Debris = ft²
Overflow Grate Open Area w/ Debris = ft²

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.25	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	18.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	14.10		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate
Outlet Orifice Area = ft²
Outlet Orifice Centroid = feet
Half-Central Angle of Restrictor Plate on Pipe = radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	6.25	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	50.00	feet
Spillway End Slopes =	10.00	H:V
Freeboard above Max Water Surface =	1.81	feet

Calculated Parameters for Spillway
Spillway Design Flow Depth = feet
Stage at Top of Freeboard = feet
Basin Area at Top of Freeboard = acres
Basin Volume at Top of Freeboard = acre-ft

Routed Hydrograph Results

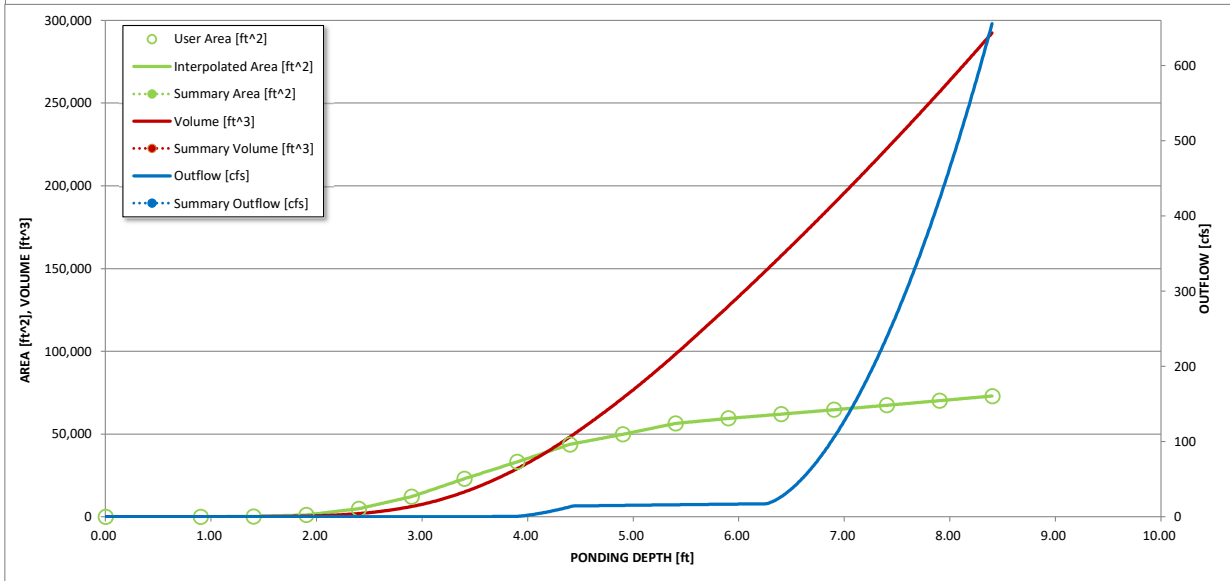
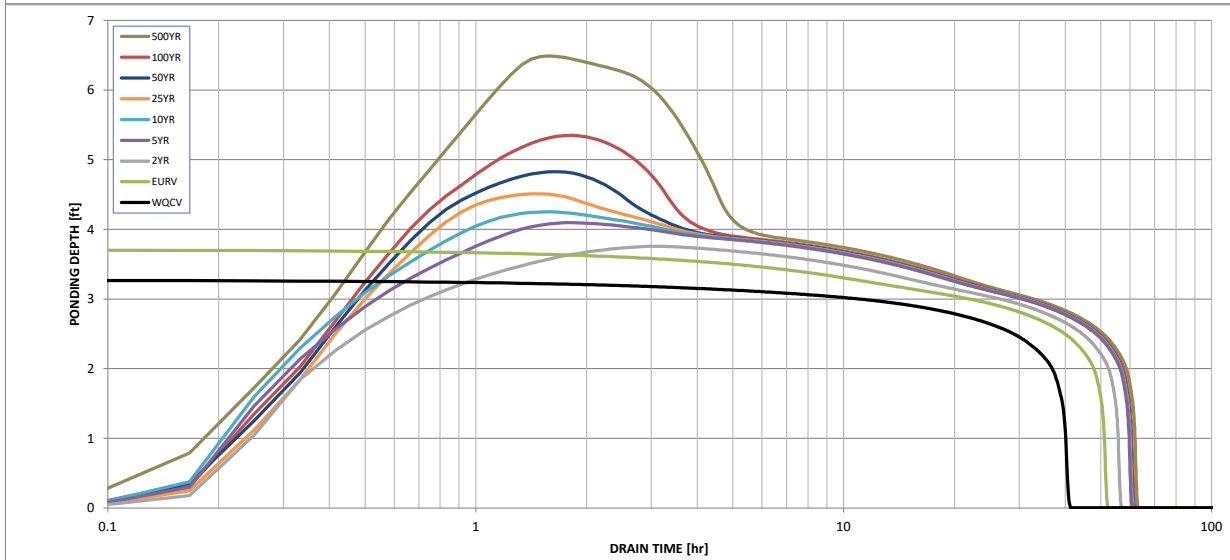
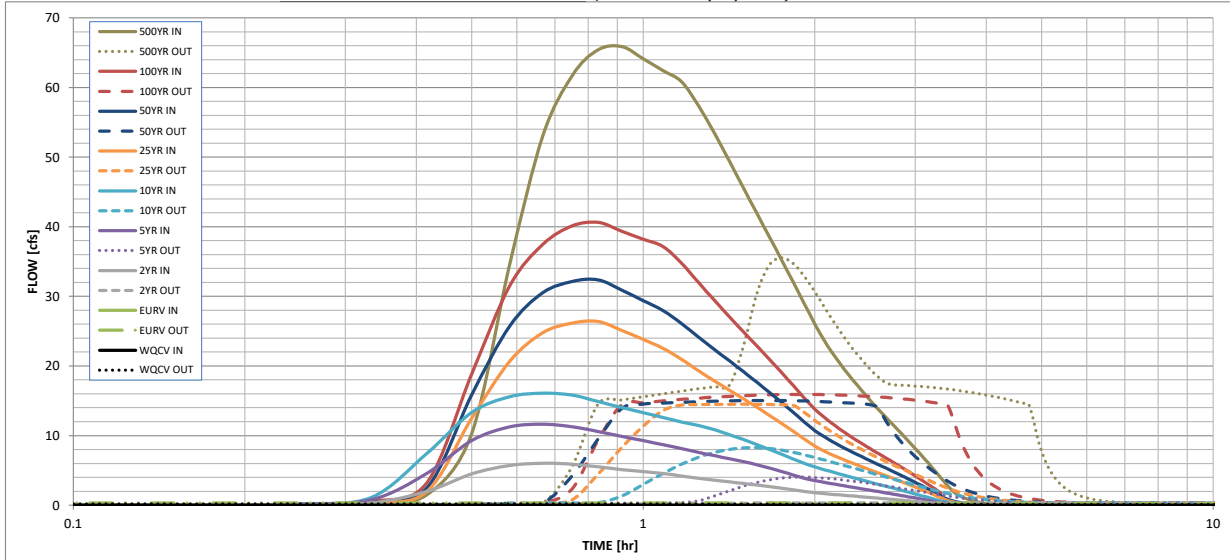
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF).

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
Design Storm Return Period =									
One-Hour Rainfall Depth (in) =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52	3.55
CUHP Runoff Volume (acre-ft) =	0.279	0.527	0.629	1.236	1.825	2.823	3.522	4.509	7.573
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.629	1.236	1.825	2.823	3.522	4.509	7.573
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	2.9	8.1	12.4	22.7	28.6	36.6	61.3
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A							
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.08	0.23	0.35	0.64	0.81	1.04	1.74
Peak Inflow Q (cfs) =	N/A	N/A	6.0	11.6	16.1	26.4	32.4	40.6	65.9
Peak Outflow Q (cfs) =	0.2	0.3	0.3	4.0	8.3	14.5	15.1	15.9	35.5
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.5	0.7	0.6	0.5	0.4	0.6
Structure Controlling Flow =	Vertical Orifice 1	Vertical Orifice 1	Vertical Orifice 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1	Outlet Plate 1	Outlet Plate 1	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	0.2	0.5	0.8	0.9	0.9	1.0
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	48	52	52	49	45	41	37	26
Time to Drain 99% of Inflow Volume (hours) =	40	50	54	57	56	54	53	52	48
Maximum Ponding Depth (ft) =	3.27	3.71	3.75	4.09	4.25	4.51	4.83	5.35	6.49
Area at Maximum Ponding Depth (acres) =	0.46	0.67	0.69	0.85	0.93	1.03	1.12	1.28	1.43
Maximum Volume Stored (acre-ft) =	0.280	0.530	0.558	0.820	0.954	1.210	1.555	2.178	3.731

WE ARE OVER DETAINING TO REDUCE THE TOTAL DISCHARGE FROM THE SITE TO BELOW PRE DEVELOPMENT VALUES. BECAUSE WE ARE OVER DETAINING, THE PLATE ON THE OUTLET FROM THE POND IS SET TO AN ELEVATION THAT ALSO AFFECTS THESE MORE FREQUENT STORM EVENTS.

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)



S-A-V-D Chart AXIS Override	X-axis	Left Y-AXIS	Right Y-AXIS
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.05
	0:15:00	0.00	0.00	0.08	0.13	0.17	0.11	0.14	0.14	0.26
	0:20:00	0.00	0.00	0.33	0.77	1.13	0.34	0.41	0.56	1.48
	0:25:00	0.00	0.00	1.99	4.56	7.40	1.93	2.42	3.20	10.28
	0:30:00	0.00	0.00	4.49	9.28	13.31	12.49	15.88	18.87	34.66
	0:35:00	0.00	0.00	5.71	11.25	15.59	20.67	25.74	31.52	53.05
	0:40:00	0.00	0.00	6.02	11.61	16.09	24.78	30.51	37.45	61.68
	0:45:00	0.00	0.00	5.87	11.27	15.81	26.16	32.13	40.11	65.41
	0:50:00	0.00	0.00	5.50	10.59	14.88	26.38	32.36	40.61	65.89
	0:55:00	0.00	0.00	5.12	9.87	14.01	25.12	30.88	39.36	64.10
	1:00:00	0.00	0.00	4.82	9.27	13.31	23.79	29.38	38.20	62.42
	1:05:00	0.00	0.00	4.54	8.68	12.62	22.54	27.96	37.15	60.85
	1:10:00	0.00	0.00	4.21	8.12	11.95	20.98	26.12	34.77	57.37
	1:15:00	0.00	0.00	3.88	7.58	11.43	19.30	24.14	31.97	53.41
	1:20:00	0.00	0.00	3.60	7.11	10.83	17.84	22.36	29.40	49.38
	1:25:00	0.00	0.00	3.35	6.67	10.13	16.53	20.72	27.05	45.51
	1:30:00	0.00	0.00	3.12	6.24	9.41	15.26	19.15	24.86	41.86
	1:35:00	0.00	0.00	2.89	5.81	8.70	14.05	17.63	22.85	38.46
	1:40:00	0.00	0.00	2.66	5.33	8.00	12.88	16.17	20.91	35.19
	1:45:00	0.00	0.00	2.44	4.84	7.31	11.73	14.74	19.02	32.02
	1:50:00	0.00	0.00	2.21	4.34	6.64	10.60	13.33	17.18	28.94
	1:55:00	0.00	0.00	1.99	3.88	6.00	9.50	11.97	15.40	26.02
	2:00:00	0.00	0.00	1.80	3.53	5.50	8.49	10.73	13.79	23.48
	2:05:00	0.00	0.00	1.66	3.27	5.08	7.72	9.77	12.53	21.41
	2:10:00	0.00	0.00	1.54	3.02	4.69	7.08	8.96	11.47	19.60
	2:15:00	0.00	0.00	1.42	2.79	4.32	6.52	8.24	10.52	17.97
	2:20:00	0.00	0.00	1.31	2.57	3.97	6.00	7.58	9.66	16.47
	2:25:00	0.00	0.00	1.21	2.36	3.63	5.53	6.98	8.87	15.09
	2:30:00	0.00	0.00	1.10	2.16	3.31	5.07	6.40	8.12	13.79
	2:35:00	0.00	0.00	1.00	1.96	3.00	4.64	5.85	7.43	12.58
	2:40:00	0.00	0.00	0.91	1.77	2.70	4.22	5.32	6.77	11.44
	2:45:00	0.00	0.00	0.82	1.58	2.42	3.81	4.80	6.13	10.33
	2:50:00	0.00	0.00	0.72	1.40	2.15	3.41	4.29	5.49	9.24
	2:55:00	0.00	0.00	0.63	1.22	1.88	3.01	3.79	4.85	8.16
	3:00:00	0.00	0.00	0.54	1.04	1.62	2.61	3.29	4.22	7.08
	3:05:00	0.00	0.00	0.45	0.87	1.36	2.21	2.79	3.58	6.01
	3:10:00	0.00	0.00	0.36	0.69	1.10	1.82	2.30	2.95	4.94
	3:15:00	0.00	0.00	0.28	0.52	0.84	1.43	1.80	2.33	3.88
	3:20:00	0.00	0.00	0.19	0.36	0.60	1.04	1.32	1.71	2.84
	3:25:00	0.00	0.00	0.13	0.25	0.44	0.67	0.86	1.13	1.96
	3:30:00	0.00	0.00	0.09	0.19	0.35	0.45	0.60	0.77	1.40
	3:35:00	0.00	0.00	0.07	0.15	0.29	0.31	0.43	0.54	1.03
	3:40:00	0.00	0.00	0.06	0.12	0.23	0.23	0.31	0.38	0.75
	3:45:00	0.00	0.00	0.05	0.10	0.19	0.16	0.23	0.26	0.53
	3:50:00	0.00	0.00	0.04	0.08	0.15	0.12	0.17	0.18	0.37
	3:55:00	0.00	0.00	0.03	0.06	0.12	0.09	0.13	0.11	0.25
	4:00:00	0.00	0.00	0.03	0.05	0.09	0.07	0.09	0.07	0.17
	4:05:00	0.00	0.00	0.02	0.04	0.07	0.05	0.07	0.06	0.13
	4:10:00	0.00	0.00	0.02	0.03	0.05	0.04	0.05	0.05	0.10
	4:15:00	0.00	0.00	0.01	0.02	0.04	0.03	0.04	0.04	0.08
	4:20:00	0.00	0.00	0.01	0.01	0.03	0.02	0.03	0.03	0.06
	4:25:00	0.00	0.00	0.01	0.01	0.02	0.02	0.02	0.02	0.05
	4:30:00	0.00	0.00	0.01	0.01	0.01	0.01	0.02	0.02	0.03
	4:35:00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.02
	4:40:00	0.00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	
6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	

Figure 13-12b. Emergency Spillway Profile at Embankment

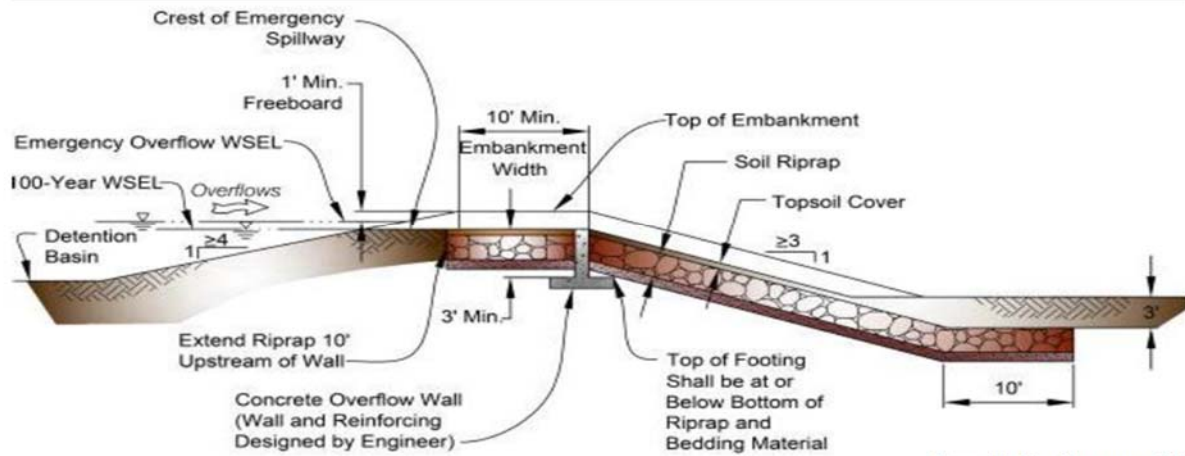


Figure 13-12c. Emergency Spillway Protection

Q=22.4 CFS
 LENGTH=50 Feet
 UNIT FLOW RATE: 0.45 CFS/FT

=> TYPE VL RIP RAP

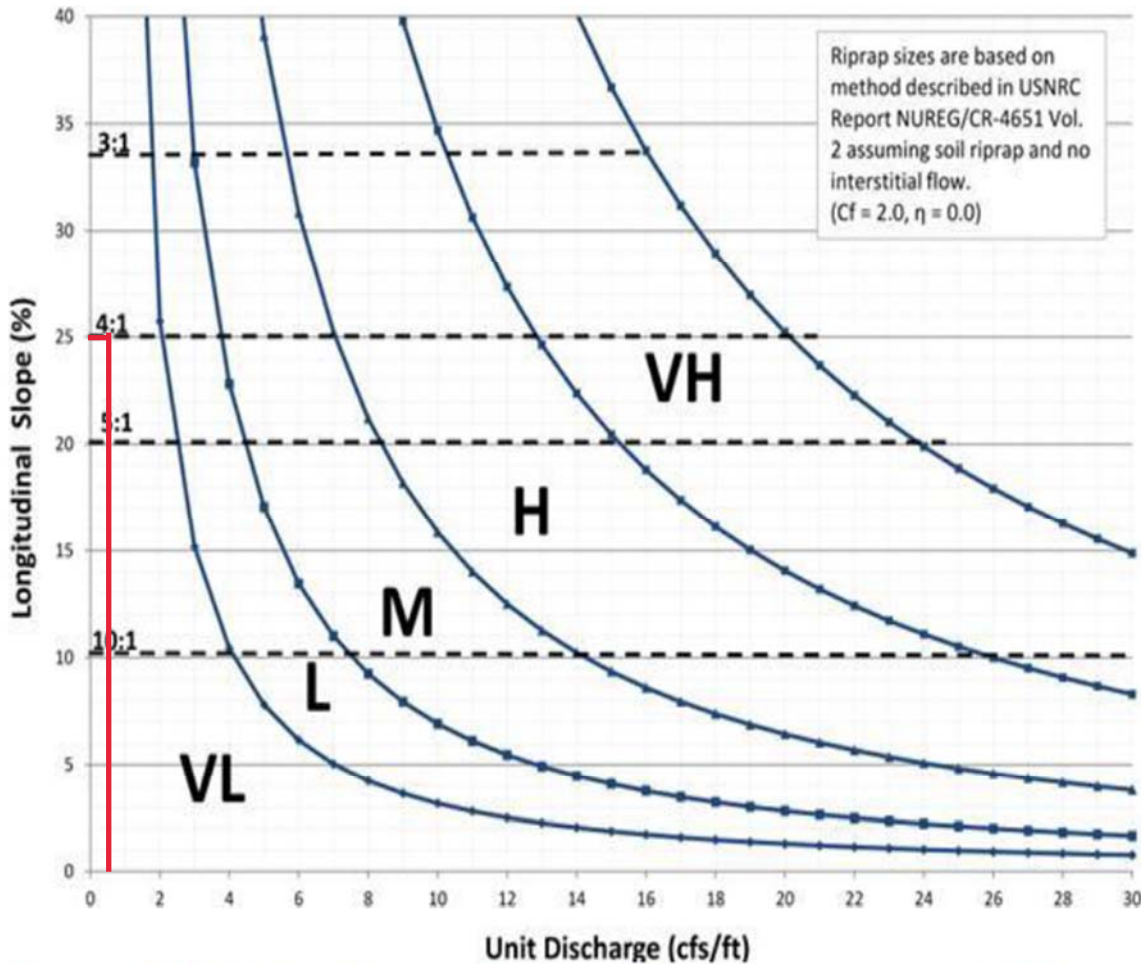
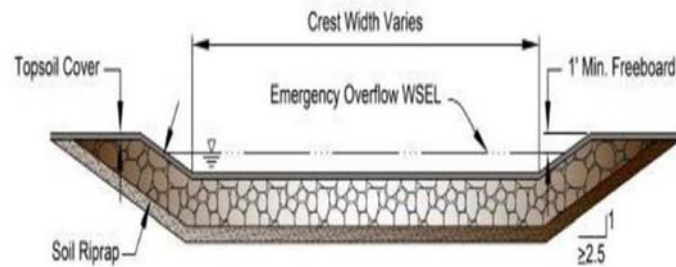


Figure 13-12d. Riprap Types for Emergency Spillway Protection

Weir Report

Stilling Basin Outfall

Trapezoidal Weir

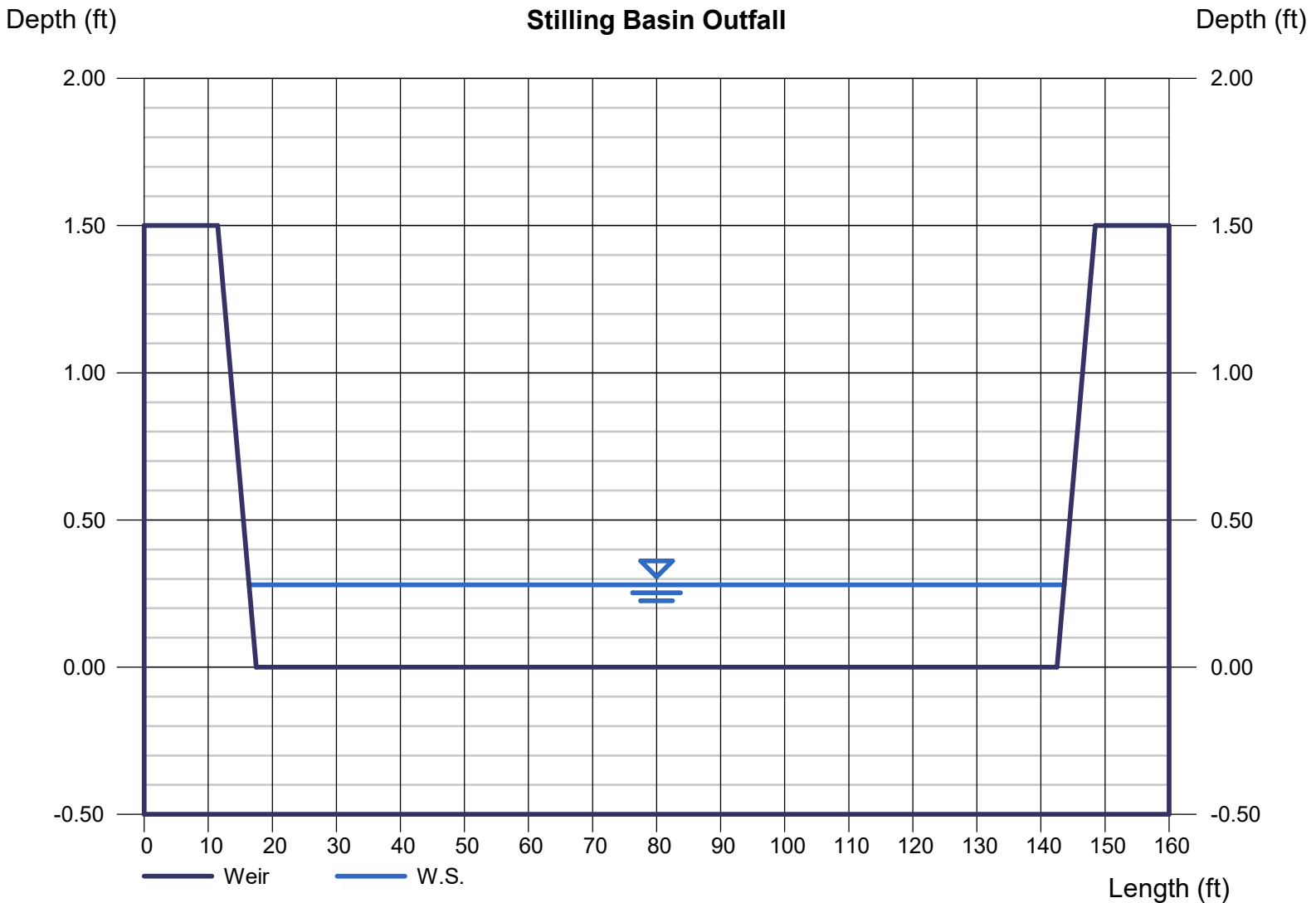
Crest = Sharp
Bottom Length (ft) = 125.00
Total Depth (ft) = 1.50
Side Slope (z:1) = 4.00

Highlighted

Depth (ft) = 0.28
Q (cfs) = 55.50
Area (sqft) = 35.31
Velocity (ft/s) = 1.57
Top Width (ft) = 127.24

Calculations

Weir Coeff. Cw = 3.10
Compute by: Known Q
Known Q (cfs) = 55.50



FOREBAY CALCULATION SHEET

Design Point	Imperviousness Decimal	Total Water Quality Control Volume (Cu. Ft.)	Pond Name	Pond Drainage Area (Acres)	Pond Drainage Area Less Pond Footprint (Acres)	Forebay Location	Drainage area tributary to Forebay	Proportion of Total Drainage Area	Trib. WQCV Volume (Cu. Ft.)	Forebay Volume		Forebay Outlet Sizing		Forebay Slot Sizing (inches)	Forebay Depth (ft)
										3% of WQCV (Cu. Ft.)	Q100 to Forebay (cfs)	2% of Q100 (cfs)			
21b	0.1719	12153.24		35.31	32.90		32.49	0.99	12211.42	366	24.7	0.5		3.33	1.5

Table EDB-4. EDB component criteria

	On-Site EDBs for Watersheds up to 1 Impervious Acre ¹	EDBs with Watersheds between 1 and 2 Impervious Acres ²	EDBs with Watersheds up to 5 Impervious Acres	EDBs with Watersheds over 5 Impervious Acres	EDBs with Watersheds over 20 Impervious Acres
Forebay Release and Configuration		Release 2% of the undetained 100-year peak discharge by way of a wall/notch configuration	Release 2% of the undetained 100-year peak discharge by way of a wall/notch configuration	Release 2% of the undetained 100-year peak discharge by way of a wall/notch configuration	Release 2% of the undetained 100-year peak discharge by way of a wall/notch or berm/pipe ² configuration
Minimum Forebay Volume	EDBs should not be used for watersheds with less than 1 impervious acre.	1% of the WQCV	2% of the WQCV	3% of the WQCV	3% of the WQCV
Maximum Forebay Depth		12 inches	18 inches	18 inches	30 inches
Trickle Channel Capacity		≥ the maximum possible forebay outlet capacity	≥ the maximum possible forebay outlet capacity	≥ the maximum possible forebay outlet capacity	≥ the maximum possible forebay outlet capacity
Micropool		Area ≥ 10 ft ²	Area ≥ 10 ft ²	Area ≥ 10 ft ²	Area ≥ 10 ft ²
Initial Surcharge Volume		Depth ≥ 4 inches	Depth ≥ 4 inches	Depth ≥ 4 in. Volume ≥ 0.3% WQCV	Depth ≥ 4 in. Volume ≥ 0.3% WQCV

¹ EDBs are not recommended for sites with less than 2 impervious acres. Consider a sand filter or rain garden.

² Round up to the first standard pipe size (minimum 8 inches).

EDB Pond	WQCV 0.279	Acre-Ft	Pond Footprint 2.41	Acres
Percent of WQCV for Forebay Impervious Percentage	3%	Between 5 and 20 impervious acres		
ISV	Impervious Acres 36	5.4 CU. FT.	Acres	

FOREBAY SLOT: WEIR SIZING EQUATION:

The Francis Formula - Imperial Units

Flow through a rectangular weir can be expressed in imperial units with the Francis formula

$$q = 3.33 (b - 0.2 h) h^{3/2} \quad (1b)$$

where

q = flow rate (ft³/s)

h = head on the weir (ft)

b = width of the weir (ft)

MINIMUM SLOT WIDTH = 3"

Channel Report

Trickle Channel

Rectangular

Bottom Width (ft) = 3.00

Total Depth (ft) = 0.50

Invert Elev (ft) = 2.00

Slope (%) = 0.50

N-Value = 0.013

Calculations

Compute by: Known Q

Known Q (cfs) = 5.00

Highlighted

Depth (ft) = 0.43

Q (cfs) = 5.000

Area (sqft) = 1.29

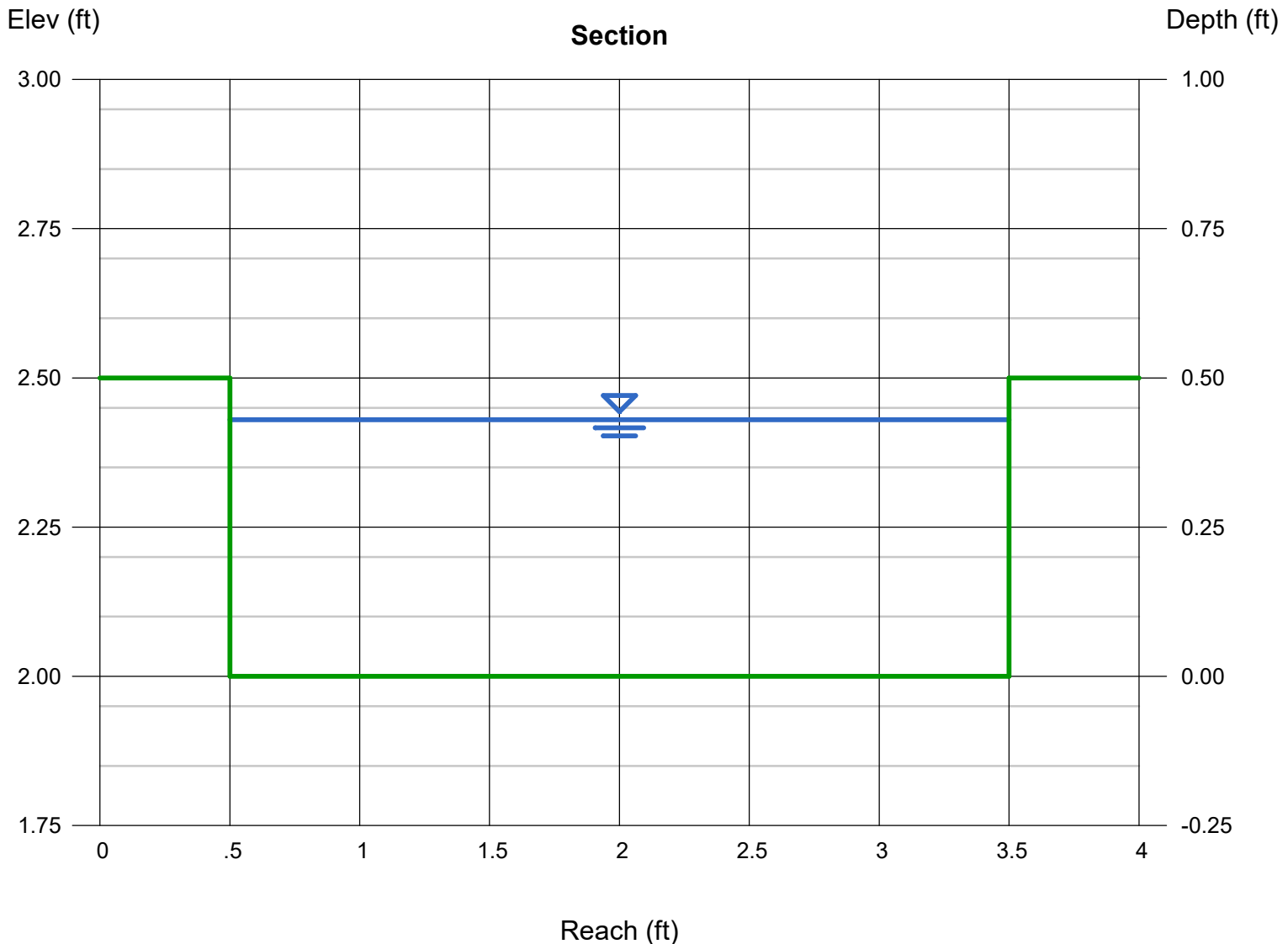
Velocity (ft/s) = 3.88

Wetted Perim (ft) = 3.86

Crit Depth, Y_c (ft) = 0.45

Top Width (ft) = 3.00

EGL (ft) = 0.66



Stormwater Detention and Infiltration Design Data Sheet

SDI-Design Data v2.00, Released January 2020

Stormwater Facility Name: **Pond 1**

Facility Location & Jurisdiction: **Hay Creek Valley, El Paso County**

User Input: Watershed Characteristics

Extended Detention Basin (EDB) ▼	EDB	
Watershed Area =	29.19	acres
Watershed Length =	3,000	ft
Watershed Length to Centroid =	1,500	ft
Watershed Slope =	0.048	ft/ft
Watershed Imperviousness =	17.1%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Target WQCV Drain Time =	40.0	hours
Location for 1-hr Rainfall Depths (use dropdown):		
User Input ▼		

After providing required inputs above including 1-hour rainfall depths, click 'Run CUHP' to generate runoff hydrographs using the embedded Colorado Urban Hydrograph Procedure.

Once CUHP has been run and the Stage-Area-Discharge information has been provided, click 'Process Data' to interpolate the Stage-Area-Volume-Discharge data and generate summary results in the table below. Once this is complete, click 'Print to PDF'.

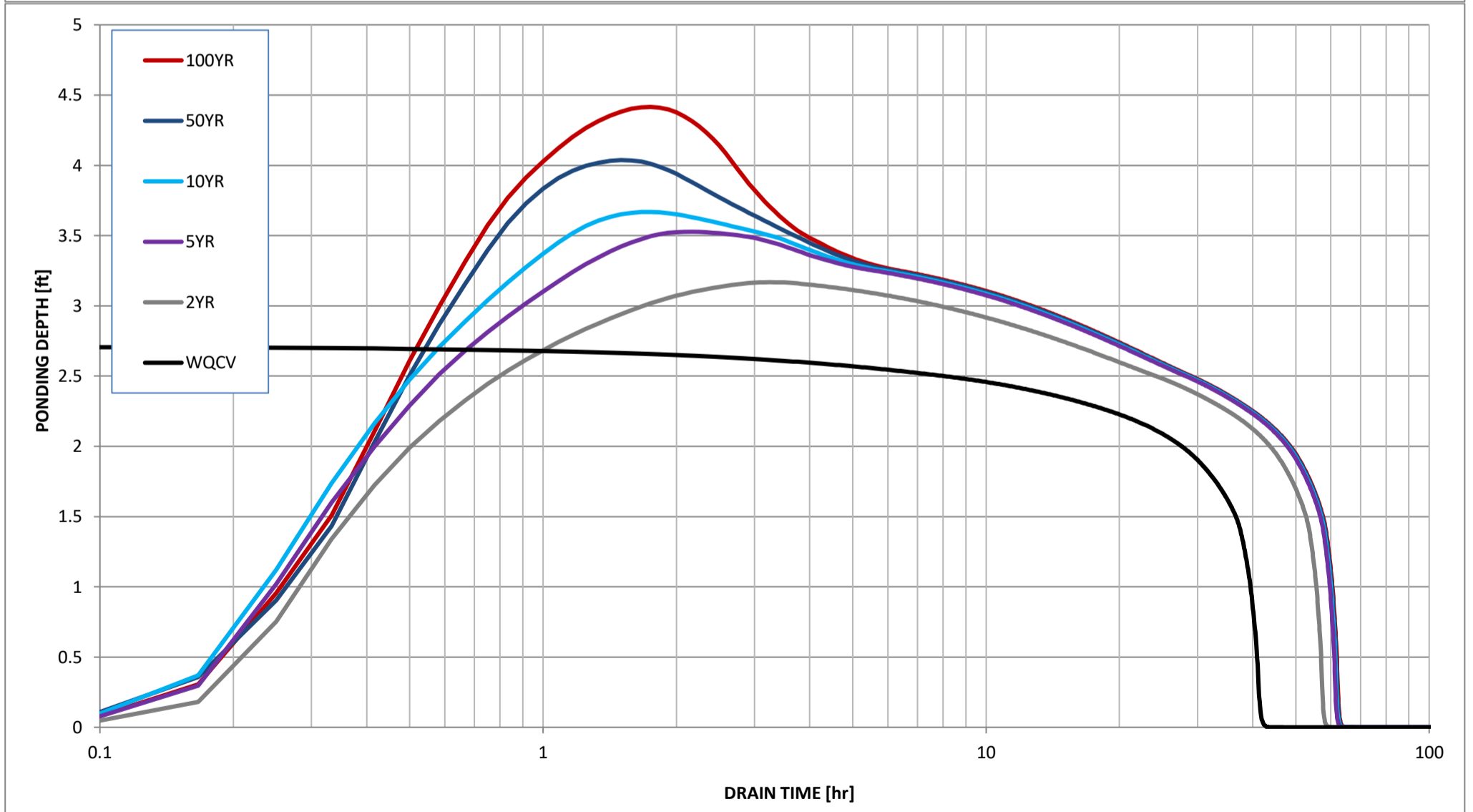
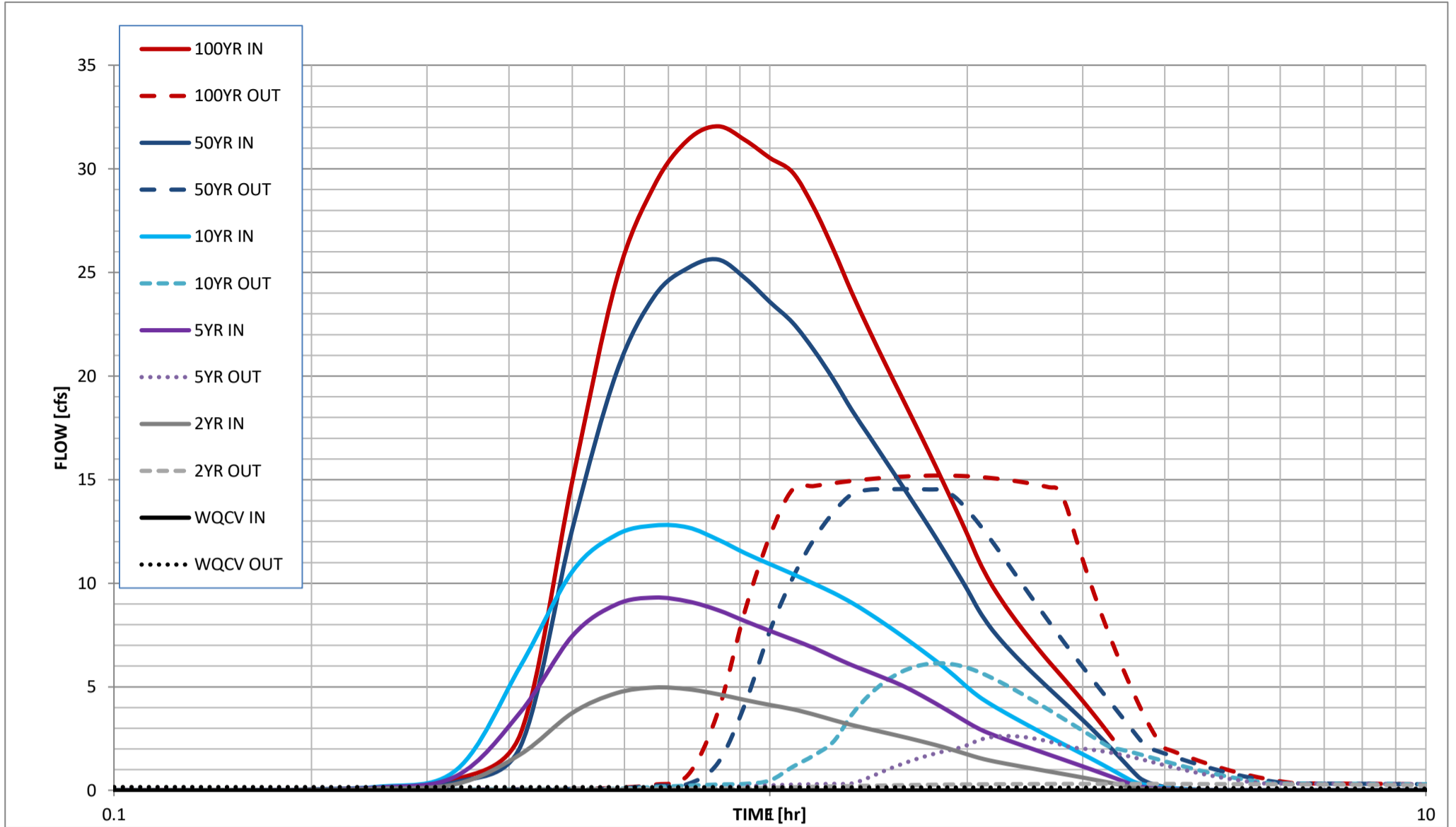
User Defined Stage [ft]	User Defined Area [ft^2]	User Defined Stage [ft]	User Defined Discharge [cfs]
0.00	60	0.00	0.00
0.40	60	0.25	0.01
0.90	237	0.50	0.02
1.40	1,096	0.75	0.02
1.90	4,884	1.00	0.03
2.40	12,158	1.25	0.05
2.90	22,987	1.50	0.05
3.40	33,167	1.75	0.06
3.90	43,762	2.00	0.07
4.40	49,829	2.25	0.07
4.90	56,410	2.50	0.08
5.40	59,443	2.75	0.18
5.90	62,026	3.00	0.28
6.40	64,682	3.25	0.34
6.90	67,380	3.50	2.08
7.40	70,161	3.75	8.29
7.90	72,971	4.00	14.49
		4.25	14.92
		4.50	15.35
		4.75	15.76
		5.00	16.16
		5.25	16.55
		5.50	16.93
		5.75	17.30
		6.00	37.17
		6.25	75.30
		6.50	127.50
		6.75	192.72
		7.00	270.62
		7.25	361.10
		7.50	464.21
		7.75	580.07
		7.90	655.78

After completing and printing this worksheet to a pdf, go to: <https://maperture.digitaldataservices.com/qvh/?viewer=cswdif>
 Create a new stormwater facility, and attach the PDF of this worksheet to that record.

Routed Hydrograph Results

	WQCV	2 Year	5 Year	10 Year	50 Year	100 Year	
Design Storm Return Period =							
One-Hour Rainfall Depth =	N/A	1.19	1.50	1.75	2.25	2.52	in
CUHP Runoff Volume =	0.251	0.564	1.075	1.568	2.970	3.784	acre-ft
Inflow Hydrograph Volume =	N/A	0.564	1.075	1.568	2.970	3.784	acre-ft
Time to Drain 97% of Inflow Volume =	38.2	51.8	52.6	50.0	42.5	38.3	hours
Time to Drain 99% of Inflow Volume =	39.8	54.8	57.5	56.5	53.7	52.3	hours
Maximum Ponding Depth =	2.72	3.17	3.53	3.67	4.04	4.42	ft
Maximum Poned Area =	0.43	0.65	0.82	0.89	1.04	1.15	acres
Maximum Volume Stored =	0.254	0.499	0.762	0.883	1.237	1.658	acre-ft

Stormwater Detention and Infiltration Design Data Sheet



Design Procedure Form: Runoff Reduction

UD-BMP (Version 3.07, March 2018)

Sheet 1 of 1

Designer: WCG
Company: Matrix Design Group
Date: January 11, 2024
Project: Hay Creek Valley
Location: El Paso County, CO

SITE INFORMATION (User Input in Blue Cells)

WQCV Rainfall Depth = 0.60 inches
 Depth of Average Runoff Producing Storm, d_0 = 0.43 inches (for Watersheds Outside of the Denver Region, Figure 3-1 in USDCM Vol. 3)

Area Type	SPA	UIA:RPA	UIA:RPA	UIA:RPA									
Area ID		PR-12	OS4	OS3b									
Downstream Design Point ID	Hay Crk	Hay Crk	Hay Crk	Hay Crk									
Downstream BMP Type	None	None	None	None									
DCIA (ft ²)	--	--	--	--									
UIA (ft ²)	--	24,328	20,445	1,330									
RPA (ft ²)	--	15,194	7,829	1,330									
SPA (ft ²)	672,115	--	--	--									
HSG A (%)	0%	0%	0%	0%									
HSG B (%)	100%	100%	100%	100%									
HSG C/D (%)	0%	0%	0%	0%									
Average Slope of RPA (ft/ft)	--	0.100	0.100	0.100									
UIA:RPA Interface Width (ft)	--	750.00	550.00	50.00									

CALCULATED RUNOFF RESULTS

Area ID		PR-12	OS4	OS3b									
UIA:RPA Area (ft ²)	--	39,522	28,274	2,660									
L / W Ratio	--	0.07	0.09	1.06									
UIA / Area	--	0.6156	0.7231	0.5000									
Runoff (in)	0.00	0.00	0.07	0.00									
Runoff (ft ³)	0	0	164	0									
Runoff Reduction (ft ³)	33606	1014	687	55									

CALCULATED WQCV RESULTS

Area ID		PR-12	OS4	OS3b									
WQCV (ft ³)	0	1014	852	55									
WQCV Reduction (ft ³)	0	1014	687	55									
WQCV Reduction (%)	0%	100%	81%	100%									
Untreated WQCV (ft ³)	0	0	164	0									

CALCULATED DESIGN POINT RESULTS (sums results from all columns with the same Downstream Design Point ID)

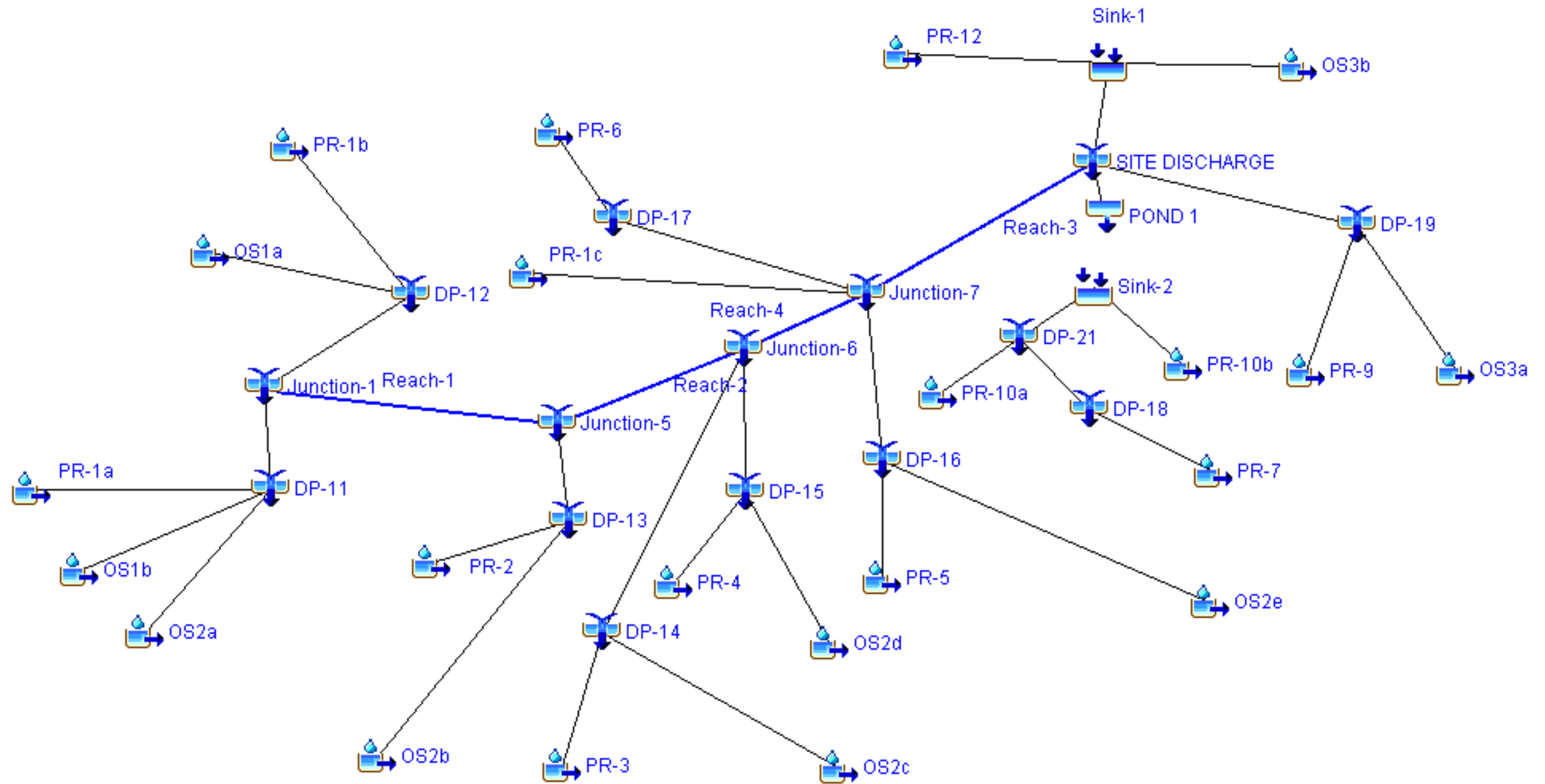
Downstream Design Point ID	Hay Crk												
DCIA (ft ²)	0												
UIA (ft ²)	46,103												
RPA (ft ²)	24,353												
SPA (ft ²)	672,115												
Total Area (ft ²)	742,571												
Total Impervious Area (ft ²)	46,103												
WQCV (ft ³)	1,921												
WQCV Reduction (ft ³)	1,756												
WQCV Reduction (%)	91%												
Untreated WQCV (ft ³)	164												

CALCULATED SITE RESULTS (sums results from all columns in worksheet)

Total Area (ft ²)	742,571
Total Impervious Area (ft ²)	46,103
WQCV (ft ³)	1,921
WQCV Reduction (ft ³)	1,756
WQCV Reduction (%)	91%
Untreated WQCV (ft ³)	164

Model Name: **Hay_Creek_Forest_Manor**

These models reflect full development of the areas included in the Hay Creek development with full spectrum detention provided to maintain historic flows. Areas tributary to Pond 1 have been modeled to drain to "Sink 2" while the pond itself has been modeled using the outflow hydrographs from the MHFD-Detention Spreadsheet.

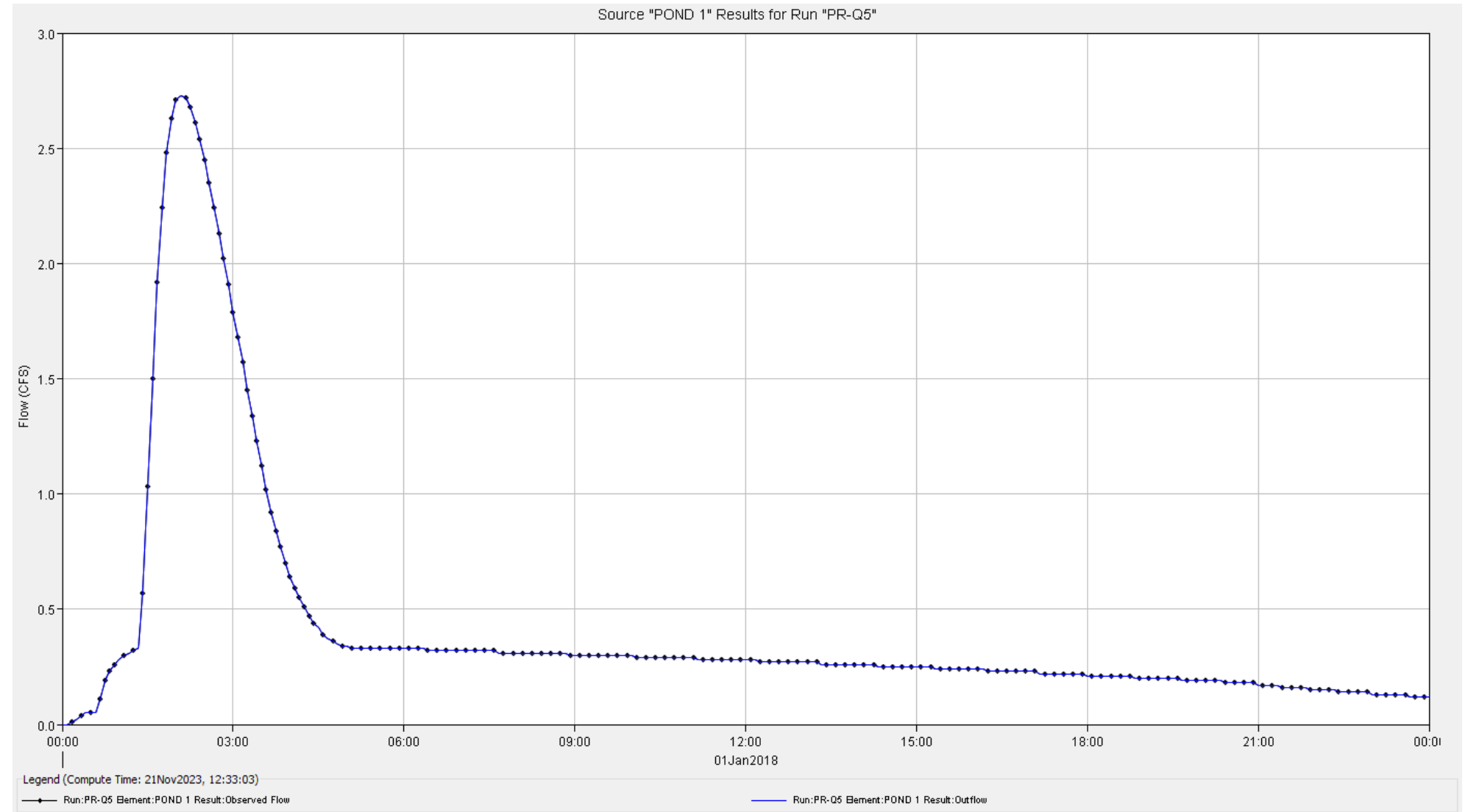


Project: HAY_CREEK_FOREST_MANOR - UP Simulation Run: PR-Q5

Start of Run: 01Jan2018, 00:00 Basin Model: PR - HAY CREEK_FM
 End of Run: 02Jan2018, 00:00 Meteorologic Model: 5 YEAR EVENT
 Compute Time: 21Nov2023, 12:38:39 Control Specifications: 1 DAY

Volume Units: IN AC-FT

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (IN)
DP-11	0.1205	3.7	01Jan2018, 12:50	0.14
DP-12	0.0244	1.3	01Jan2018, 12:40	0.21
DP-13	0.0195	1.1	01Jan2018, 12:30	0.20
DP-14	0.0380	2.2	01Jan2018, 12:30	0.21
DP-15	0.0268	1.5	01Jan2018, 12:40	0.21
DP-16	0.0313	1.6	01Jan2018, 12:40	0.21
DP-17	0.0313	1.7	01Jan2018, 12:40	0.21
DP-18	0.0338	2.1	01Jan2018, 12:30	0.21
DP-19	0.0286	1.2	01Jan2018, 12:40	0.17
DP-21	0.0412	4.7	01Jan2018, 12:45	0.54
Junction-1	0.1449	5.0	01Jan2018, 12:45	0.16
Junction-5	0.0195	1.1	01Jan2018, 12:30	0.20
Junction-6	0.0843	4.6	01Jan2018, 12:40	0.21
Junction-7	0.2301	10.9	01Jan2018, 12:50	0.21
OS1a	0.0146	0.8	01Jan2018, 12:40	0.21
OS1b	0.0925	2.6	01Jan2018, 12:50	0.13
OS2a	0.0078	0.4	01Jan2018, 12:25	0.14
OS2b	0.0074	0.4	01Jan2018, 12:30	0.18
OS2c	0.0095	0.8	01Jan2018, 12:25	0.20
OS2d	0.0043	0.3	01Jan2018, 12:35	0.20
OS2e	0.0049	0.3	01Jan2018, 12:30	0.20
OS3a	0.0076	0.1	01Jan2018, 12:40	0.06
OS3b	0.0051	0.1	01Jan2018, 12:30	0.06
POND 1	0.0840	2.7	01Jan2018, 02:05	0.18
PR-10a	0.0074	3.3	01Jan2018, 13:00	2.00
PR-10b	0.0006	0.8	01Jan2018, 12:15	2.66
PR-12	0.0256	0.6	01Jan2018, 12:55	0.11
PR-1a	0.0202	0.9	01Jan2018, 12:50	0.21
PR-1b	0.0098	0.5	01Jan2018, 12:40	0.21
PR-1c	0.0832	3.4	01Jan2018, 13:00	0.21
PR-2	0.0121	0.7	01Jan2018, 12:35	0.21
PR-3	0.0285	1.6	01Jan2018, 12:40	0.21
PR-4	0.0225	1.2	01Jan2018, 12:40	0.21
PR-5	0.0264	1.3	01Jan2018, 12:45	0.21
PR-6	0.0313	1.7	01Jan2018, 12:40	0.21
PR-7	0.0338	2.1	01Jan2018, 12:30	0.21
PR-9	0.0210	1.1	01Jan2018, 12:40	0.21
Reach-1	0.0000	0.0	01Jan2018, 00:00	
Reach-2	0.0195	1.1	01Jan2018, 12:45	0.20
Reach-3	0.2301	10.9	01Jan2018, 12:50	0.21
Reach-4	0.0843	4.6	01Jan2018, 12:50	0.21
Sink-1	0.3734	12.9	01Jan2018, 12:50	0.19
Sink-2	0.0418	4.9	01Jan2018, 12:40	0.57
SITE DISCHARGE	0.3427	12.3	01Jan2018, 12:50	0.20

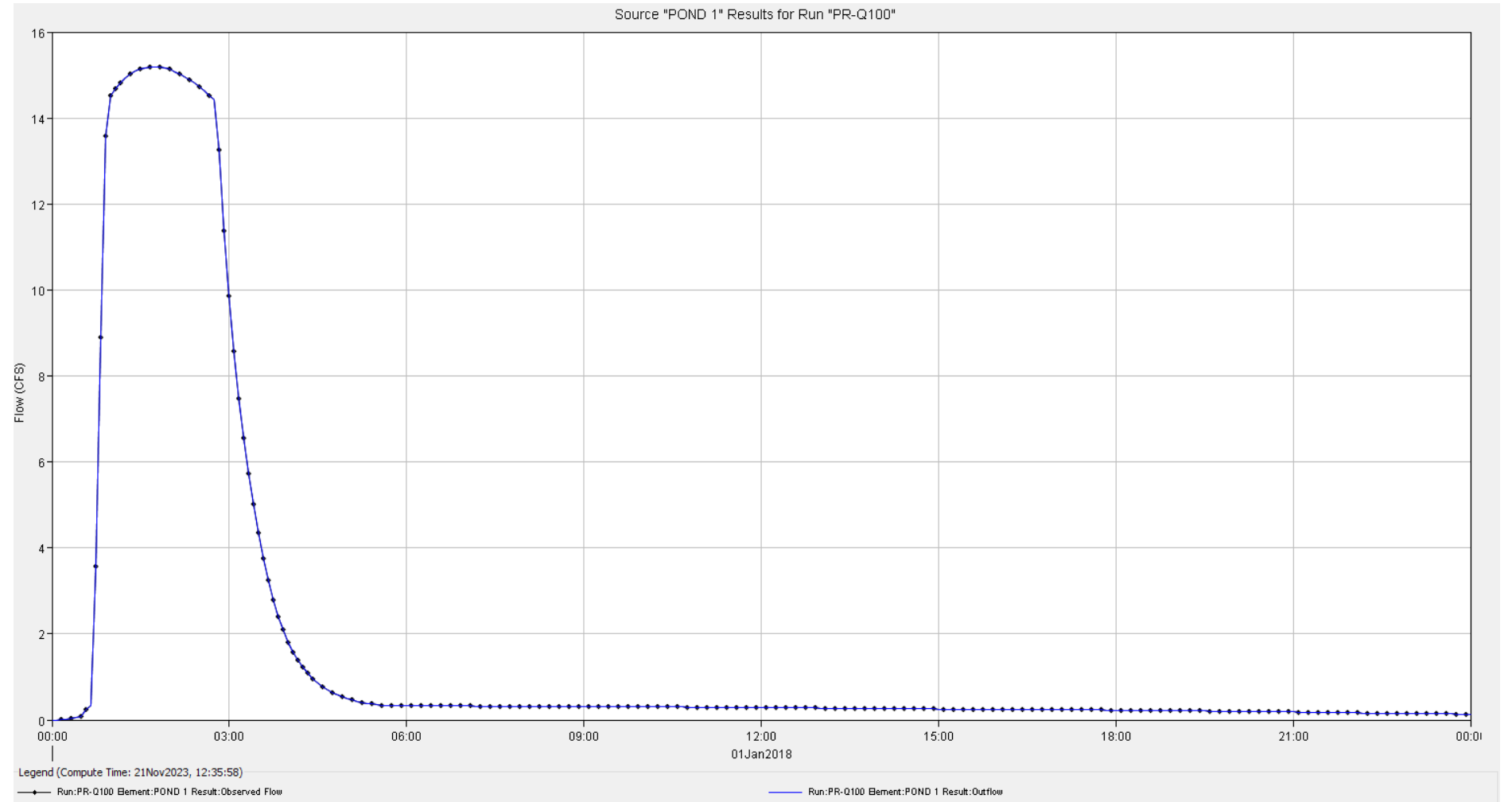


Project: HAY_CREEK_FOREST_MANOR - UP Simulation Run: PR-Q100

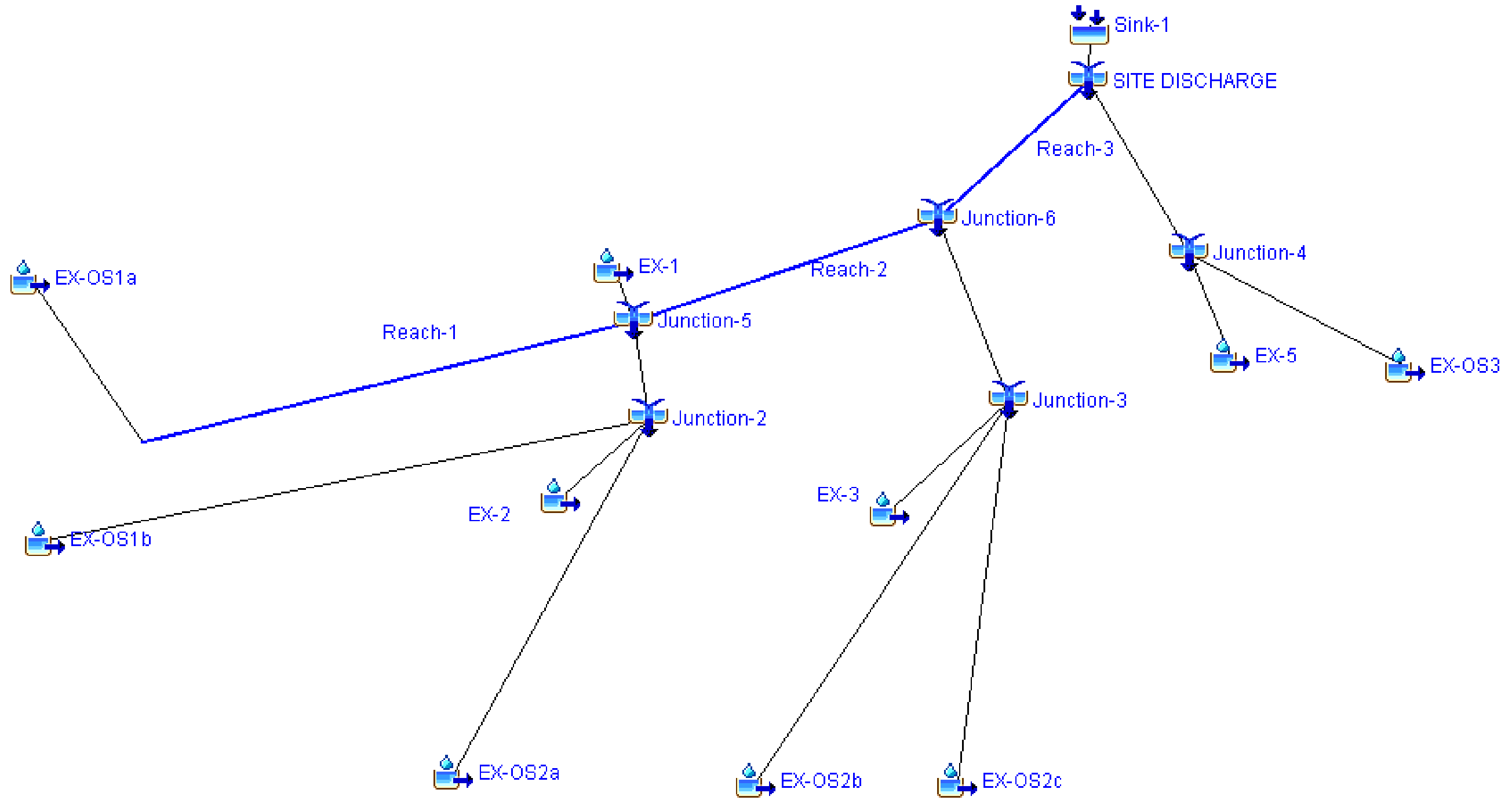
Start of Run: 01Jan2018, 00:00 Basin Model: PR - HAY CREEK_FM
 End of Run: 02Jan2018, 00:00 Meteorologic Model: 100 YEAR EVENT
 Compute Time: 21Nov2023, 12:35:58 Control Specifications: 1 DAY

Volume Units: IN AC-FT

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (IN)
DP-11	0.1205	19.5	01Jan2018, 12:55	0.74
DP-12	0.0244	6.1	01Jan2018, 12:45	0.92
DP-13	0.0195	5.3	01Jan2018, 12:35	0.89
DP-14	0.0380	9.2	01Jan2018, 12:40	0.89
DP-15	0.0268	6.7	01Jan2018, 12:45	0.91
DP-16	0.0313	7.2	01Jan2018, 12:45	0.91
DP-17	0.0313	7.9	01Jan2018, 12:45	0.92
DP-18	0.0338	9.7	01Jan2018, 12:35	0.92
DP-19	0.0286	6.1	01Jan2018, 12:45	0.82
DP-21	0.0412	16.0	01Jan2018, 12:45	1.58
Junction-1	0.1449	25.2	01Jan2018, 12:55	0.77
Junction-5	0.0195	5.3	01Jan2018, 12:35	0.89
Junction-6	0.0843	21.0	01Jan2018, 12:40	0.90
Junction-7	0.2301	49.7	01Jan2018, 12:50	0.90
OS1a	0.0146	3.6	01Jan2018, 12:45	0.92
OS1b	0.0925	14.2	01Jan2018, 13:00	0.70
OS2a	0.0078	1.8	01Jan2018, 12:30	0.71
OS2b	0.0074	2.0	01Jan2018, 12:35	0.85
OS2c	0.0095	2.7	01Jan2018, 12:25	0.80
OS2d	0.0043	1.1	01Jan2018, 12:40	0.86
OS2e	0.0049	1.3	01Jan2018, 12:35	0.84
OS3a	0.0076	0.9	01Jan2018, 12:50	0.53
OS3b	0.0051	0.8	01Jan2018, 12:35	0.53
POND 1	0.0840	15.2	01Jan2018, 01:40	0.79
PR-10a	0.0074	7.8	01Jan2018, 13:00	4.55
PR-10b	0.0006	1.7	01Jan2018, 12:15	5.49
PR-12	0.0256	3.4	01Jan2018, 13:05	0.65
PR-1a	0.0202	4.3	01Jan2018, 12:55	0.92
PR-1b	0.0098	2.5	01Jan2018, 12:45	0.92
PR-1c	0.0832	15.7	01Jan2018, 13:05	0.91
PR-2	0.0121	3.3	01Jan2018, 12:40	0.92
PR-3	0.0285	7.3	01Jan2018, 12:45	0.92
PR-4	0.0225	5.6	01Jan2018, 12:45	0.92
PR-5	0.0264	6.1	01Jan2018, 12:50	0.92
PR-6	0.0313	7.9	01Jan2018, 12:45	0.92
PR-7	0.0338	9.7	01Jan2018, 12:35	0.92
PR-9	0.0210	5.1	01Jan2018, 12:45	0.92
Reach-1	0.0000	0.0	01Jan2018, 00:00	
Reach-2	0.0195	5.2	01Jan2018, 12:45	0.88
Reach-3	0.2301	49.4	01Jan2018, 12:55	0.90
Reach-4	0.0843	20.9	01Jan2018, 12:50	0.89
Sink-1	0.3734	59.4	01Jan2018, 12:55	0.85
Sink-2	0.0418	16.5	01Jan2018, 12:40	1.63
SITE DISCHARGE	0.3427	55.5	01Jan2018, 12:55	0.87



Model Name: **Hay_Creek_Forest_Manor**
These models reflect the existing conditions on the Hay Creek development.



Project: HAY_CREEK_FOREST_MANOR - UP Simulation Run: EX-Q5

Start of Run: 01Jan2018, 00:00 Basin Model: EX - HAY CREEK_FM
 End of Run: 02Jan2018, 00:00 Meteorologic Model: 5 YEAR EVENT
 Compute Time: 28Mar2023, 08:56:35 Control Specifications: 1 DAY

Volume Units: IN AC-FT

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (IN)
EX-1	0.0876	1.6	01Jan2018, 13:00	0.10
EX-2	0.0754	2.0	01Jan2018, 12:50	0.13
EX-3	0.0963	2.3	01Jan2018, 12:40	0.10
EX-5	0.0669	1.5	01Jan2018, 12:40	0.09
EX-OS1a	0.0146	0.8	01Jan2018, 12:40	0.23
EX-OS1b	0.0925	2.6	01Jan2018, 12:50	0.14
EX-OS2a	0.0248	1.5	01Jan2018, 12:25	0.18
EX-OS2b	0.0043	0.3	01Jan2018, 12:35	0.23
EX-OS2c	0.0049	0.3	01Jan2018, 12:30	0.22
EX-OS3	0.0127	0.2	01Jan2018, 12:30	0.07
Junction-2	0.1927	5.4	01Jan2018, 12:45	0.14
Junction-3	0.1055	2.9	01Jan2018, 12:35	0.11
Junction-4	0.0796	1.8	01Jan2018, 12:40	0.09
Junction-5	0.2949	7.5	01Jan2018, 12:50	0.13
Junction-6	0.4004	9.8	01Jan2018, 12:50	0.12
Reach-1	0.0146	0.8	01Jan2018, 13:00	0.23
Reach-2	0.2949	7.5	01Jan2018, 13:00	0.13
Reach-3	0.4004	9.8	01Jan2018, 12:55	0.12
Sink-1	0.4800	11.2	01Jan2018, 12:50	0.12
SITE DISCH...	0.4800	11.2	01Jan2018, 12:50	0.12

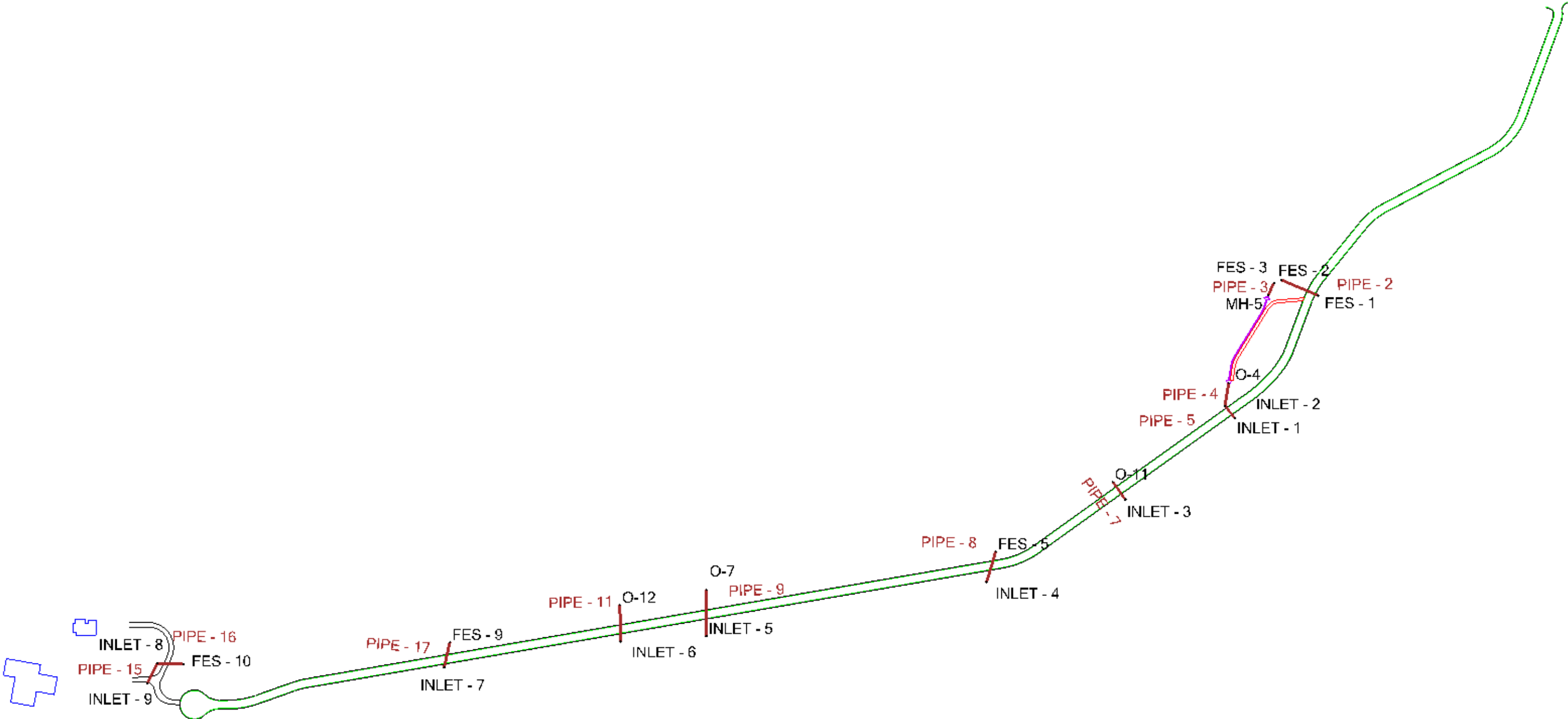
Project: HAY_CREEK_FOREST_MANOR - UP Simulation Run: EX-Q100

Start of Run: 01Jan2018, 00:00 Basin Model: EX - HAY CREEK_FM
 End of Run: 02Jan2018, 00:00 Meteorologic Model: 100 YEAR EVENT
 Compute Time: 28Mar2023, 08:57:29 Control Specifications: 1 DAY

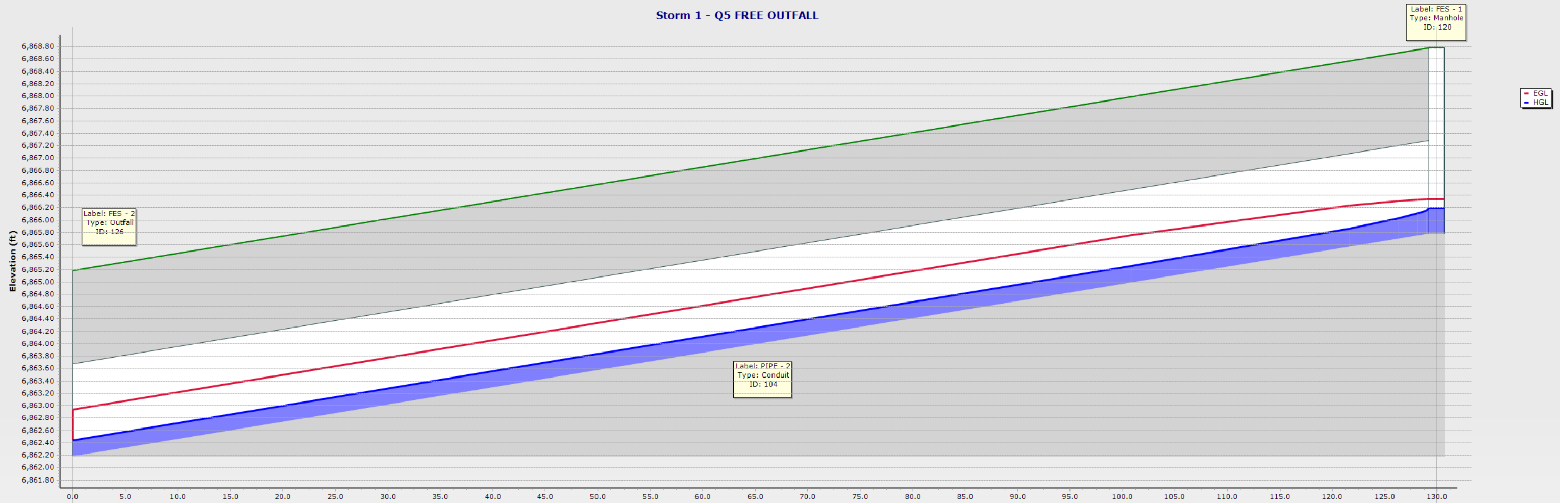
Volume Units: IN AC-FT

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (IN)
EX-1	0.0876	9.9	01Jan2018, 13:15	0.62
EX-2	0.0754	11.2	01Jan2018, 13:00	0.69
EX-3	0.0963	14.7	01Jan2018, 12:45	0.62
EX-5	0.0669	10.0	01Jan2018, 12:45	0.61
EX-OS1a	0.0146	4.1	01Jan2018, 12:45	1.01
EX-OS1b	0.0925	15.7	01Jan2018, 13:00	0.76
EX-OS2a	0.0248	8.0	01Jan2018, 12:30	0.89
EX-OS2b	0.0043	1.4	01Jan2018, 12:35	1.01
EX-OS2c	0.0049	1.6	01Jan2018, 12:30	0.99
EX-OS3	0.0127	2.0	01Jan2018, 12:40	0.57
Junction-2	0.1927	30.9	01Jan2018, 12:55	0.75
Junction-3	0.1055	17.4	01Jan2018, 12:45	0.65
Junction-4	0.0796	11.9	01Jan2018, 12:45	0.60
Junction-5	0.2949	43.9	01Jan2018, 13:00	0.72
Junction-6	0.4004	59.2	01Jan2018, 12:55	0.70
Reach-1	0.0146	4.1	01Jan2018, 12:55	1.00
Reach-2	0.2949	43.8	01Jan2018, 13:00	0.72
Reach-3	0.4004	59.1	01Jan2018, 13:00	0.70
Sink-1	0.4800	69.9	01Jan2018, 12:55	0.68
SITE DISCH...	0.4800	69.9	01Jan2018, 12:55	0.68

StormCAD LAYOUT

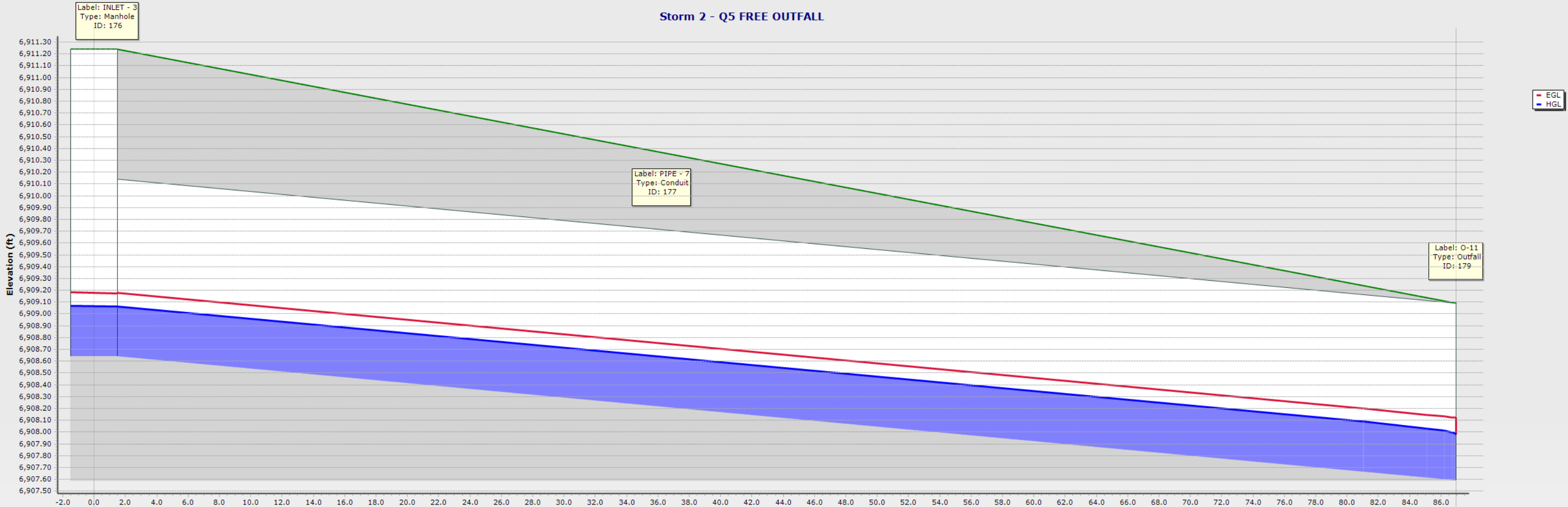


Storm 1 - Q5 FREE OUTFALL



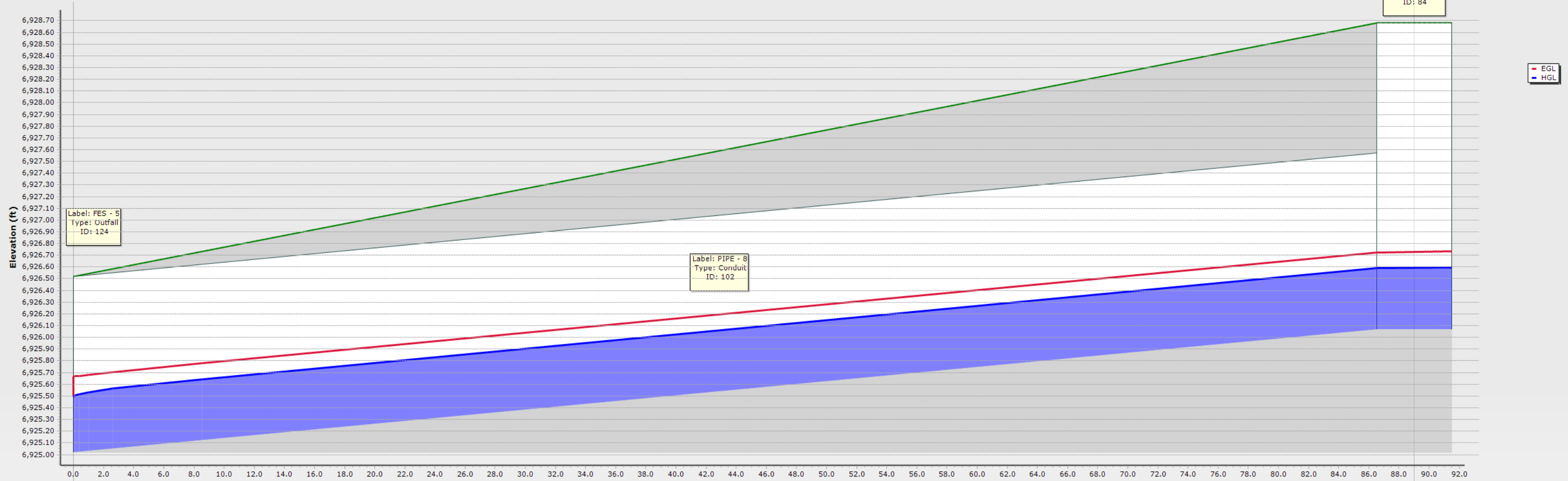
ID\Label	104 \ PIPE - 2	120 \ FES - 1
Link Length (ft)	129.9	
Rise (in)\Material	18.0 \ Concrete	
Flow (cfs)	1.20	
Slope (ft/ft)	0.028	
ID\Label	126 \ FES - 2	120 \ FES - 1
Ground (ft)	6865.18	6868.78
Invert (ft)	6862.18	6865.78
Station (ft)	0.0	129.9

Storm 2 - Q5 FREE OUTFALL



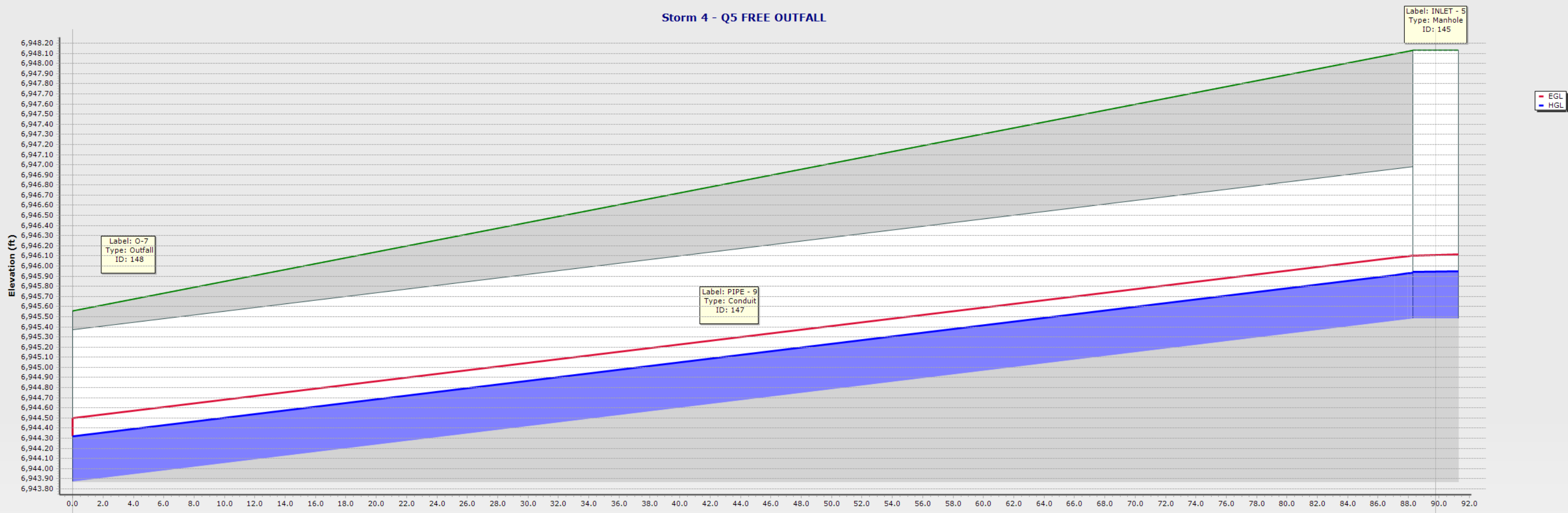
ID\Label			177 \ PIPE - 7
Link Length (ft)			87.0
Rise (in)\Material			18.0 \ CMP
Flow (cfs)			1.10
Slope (ft/ft)			0.012
ID\Label	176 \ INLET - 3		179 \ O-11
Ground (ft)	6911.24		6909.09
Invert (ft)	6908.64		6907.59
Station (ft)	0.0		87.0

Storm 3 - Q5 FREE OUTFALL



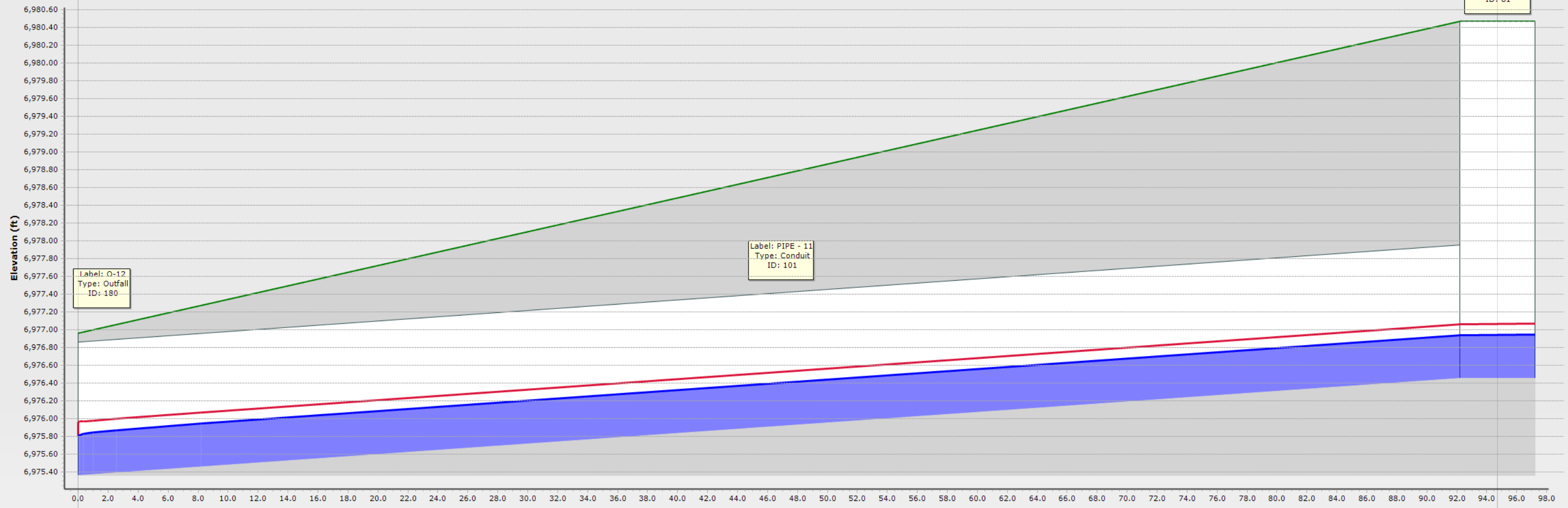
ID\Label	102 \ PIPE - 8	84 \ INLET - 4
Link Length (ft)	89.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.60	
Slope (ft/ft)	0.012	
ID\Label	4 \ FES - 5	84 \ INLET - 4
Ground (ft)	6926.52	6928.68
Invert (ft)	6925.02	6926.07
Station (ft)	0.0	89.0

Storm 4 - Q5 FREE OUTFALL



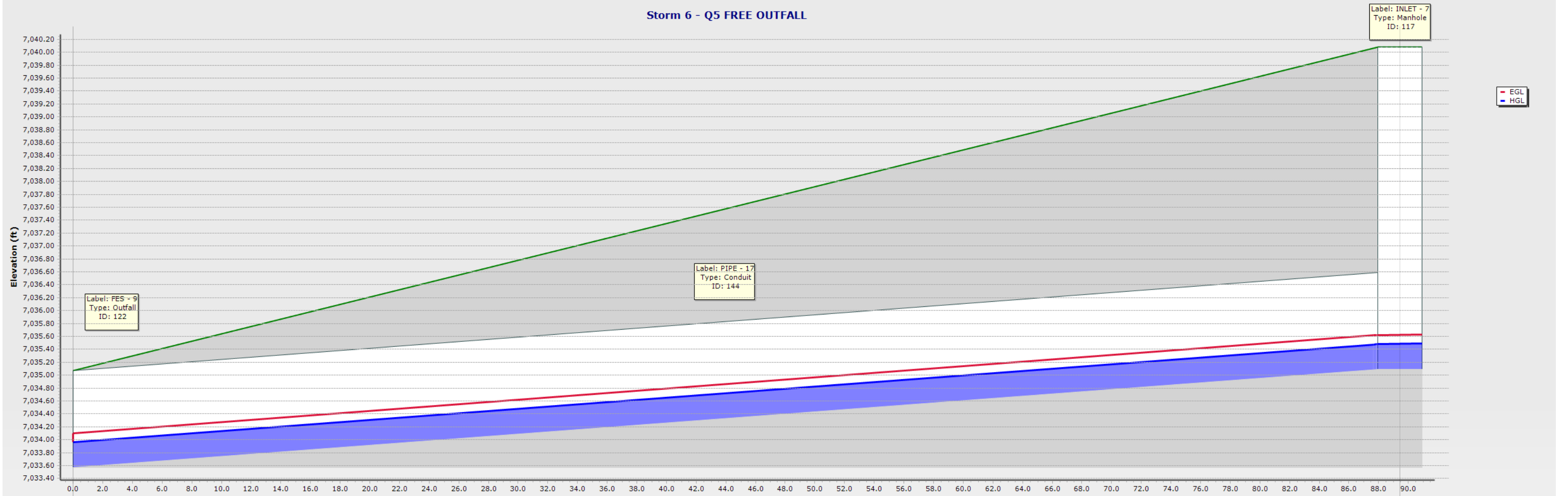
ID\Label	147 \ PIPE - 9	
Link Length (ft)	89.8	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.50	
Slope (ft/ft)	0.018	
ID\Label	148 \ O-7	145 \ INLET - 5
Ground (ft)	6945.56	6948.13
Invert (ft)	6943.87	6945.48
Station (ft)	0.0	89.8

Storm 5 - Q5 FREE OUTFALL



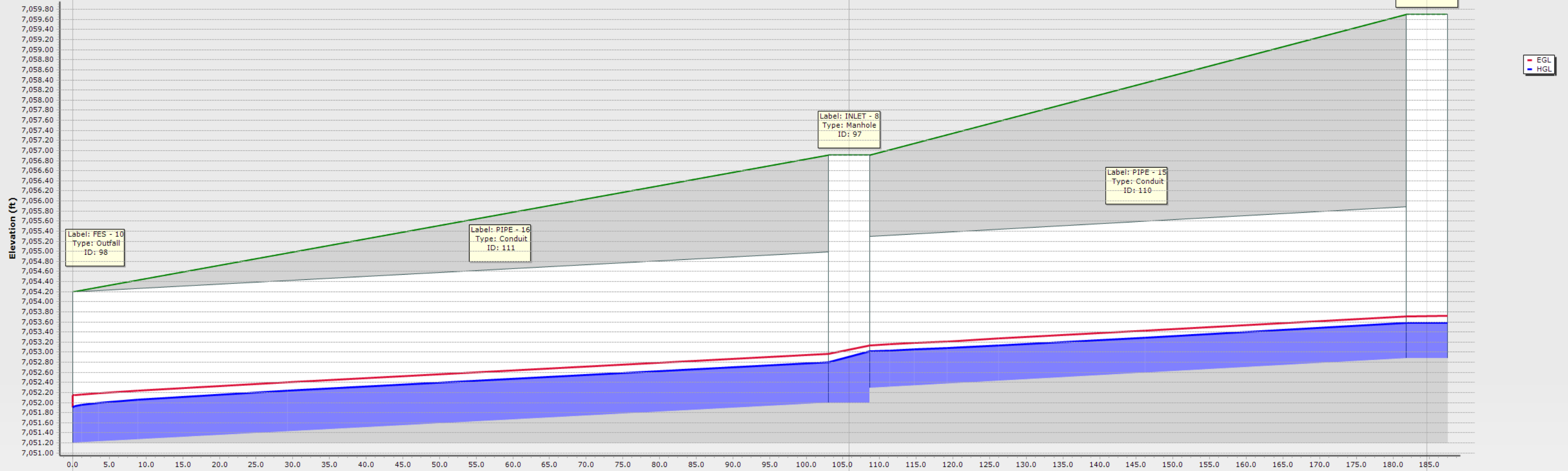
ID\Label	101 \ PIPE - 11	81 \ INLET - 6
Link Length (ft)	94.7	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.40	
Slope (ft/ft)	0.012	
ID\Label	180 \ O-12	
Ground (ft)	6976.96	6980.47
Invert (ft)	6975.36	6976.45
Station (ft)	0.0	94.7

Storm 6 - Q5 FREE OUTFALL



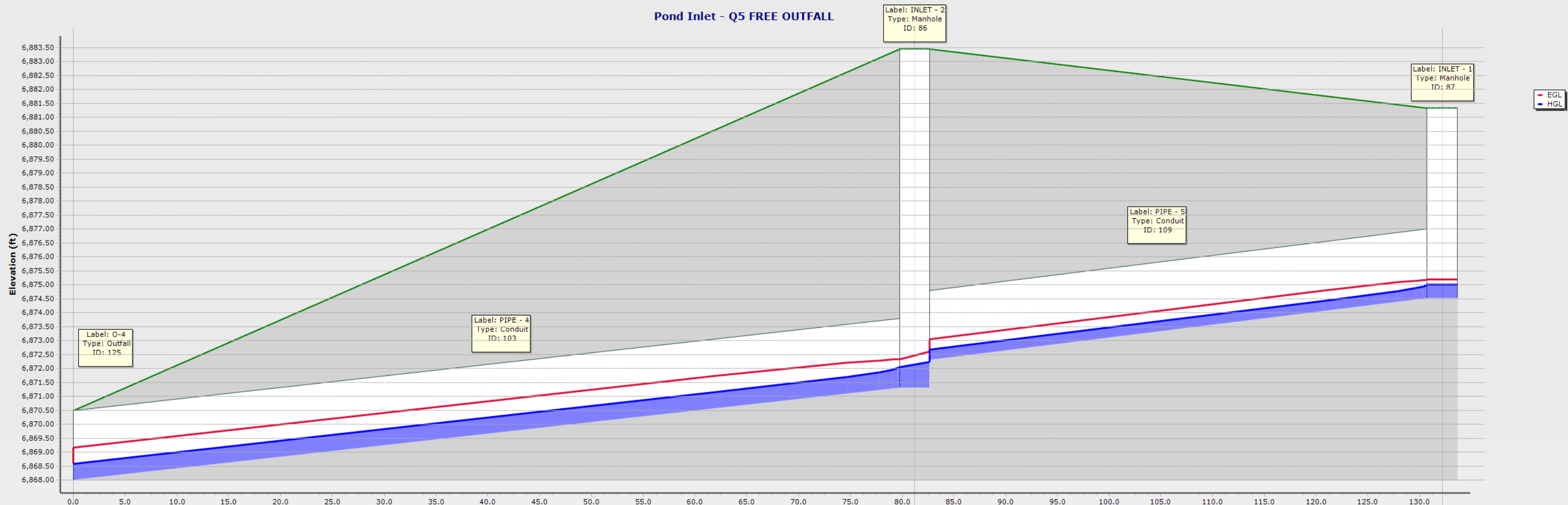
ID\Label	144 \ PIPE - 17	117 \ INLET - 7
Link Length (ft)	89.4	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.10	
Slope (ft/ft)	0.017	
ID\Label	122 \ FES - 9	117 \ INLET - 7
Ground (ft)	7035.07	7040.08
Invert (ft)	7033.57	7035.09
Station (ft)	0.0	89.4

Storm 7 - Q5 FREE OUTFALL



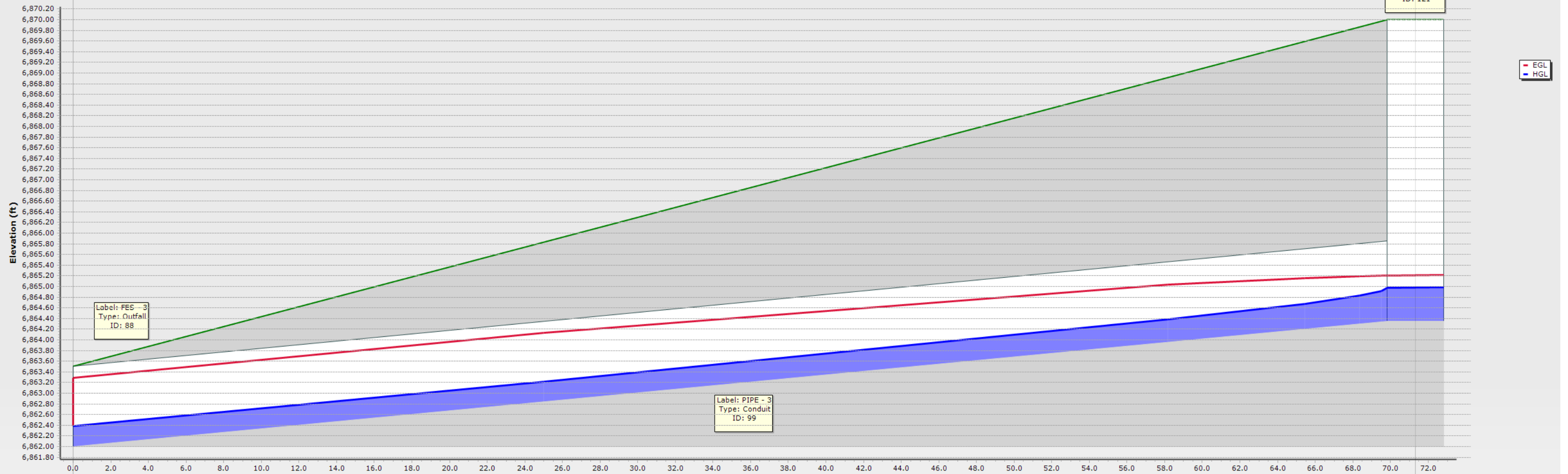
ID\Label	111 \ PIPE - 16	110 \ PIPE - 15	
Link Length (ft)	105.8	78.8	
Rise (in)\Material	36.0 \ CMP	36.0 \ CMP	
Flow (cfs)	5.00	3.70	
Slope (ft/ft)	0.007	0.007	
ID\Label	98 \ FES - 10	97 \ INLET - 8	96 \ INLET - 9
Ground (ft)	7054.20	7056.91	7059.70
Invert (ft)	7051.20	7051.99	7052.88
Station (ft)	0.0	105.8	184.6

Pond Inlet - Q5 FREE OUTFALL



ID\Label	103 \ PIPE - 4	109 \ PIPE - 5	
Link Length (ft)	81.2	50.9	
Rise (in)\Material	30.0 \ CMP	30.0 \ CMP	
Flow (cfs)	5.30	2.30	
Slope (ft/ft)	0.041	0.043	
ID\Label	125 \ O-4	86 \ INLET - 2	87 \ INLET - 1
Ground (ft)	6870.50	6883.45	6881.33
Invert (ft)	6868.00	6871.30	6874.51
Station (ft)	0.0	81.2	132.2

Pond-Outfall - Q5 FREE OUTFALL



ID\Label	99 \ PIPE - 3	121 \ MH-5
Link Length (ft)	71.3	
Rise (in)\Material	18.0 \ Concrete	
Flow (cfs)	2.70	
Slope (ft/ft)	0.033	
ID\Label	88 \ FES - 3	
Ground (ft)	6863.50	6870.00
Invert (ft)	6862.00	6864.35
Station (ft)	0.0	71.3

	Label	Velocity (ft/s)	Start Node	Invert (Start) (ft)	Hydraulic Grade Line (In) (ft)	Invert (Stop) (ft)	Hydraulic Grade Line (Out) (ft)	Stop Node	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Flow (cfs)	Capacity (Design) (cfs)	Depth (Normal) / Rise (%)	Diameter (in)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)
99: PIPE - 3	PIPE - 3	4.92	MH-5	6,864.35	6,864.97	6,862.00	6,862.52	FES - 3	71.3	0.033	0.024	2.70	10.33	34.9	18.0	6,870.00	6,863.50
101: PIPE - 11	PIPE - 11	2.80	INLET - 6	6,976.45	6,976.94	6,975.36	6,975.80	O-12	94.7	0.012	0.024	1.40	6.10	32.6	18.0	6,980.47	6,976.96
102: PIPE - 8	PIPE - 8	2.94	INLET - 4	6,926.07	6,926.59	6,925.02	6,925.50	FES - 5	89.0	0.012	0.024	1.60	6.18	34.7	18.0	6,928.68	6,926.52
103: PIPE - 4	PIPE - 4	6.13	INLET - 2	6,871.30	6,872.06	6,868.00	6,868.58	O-4	81.2	0.041	0.024	5.30	44.78	23.2	30.0	6,883.45	6,870.50
104: PIPE - 2	PIPE - 2	3.67	FES - 1	6,865.78	6,866.19	6,862.18	6,862.54	FES - 2	129.9	0.028	0.024	1.20	9.47	24.0	18.0	6,868.78	6,865.18
109: PIPE - 5	PIPE - 5	4.90	INLET - 1	6,874.51	6,875.01	6,872.30	6,872.68	INLET - 2	50.9	0.043	0.024	2.30	46.27	15.2	30.0	6,881.33	6,883.45
110: PIPE - 15	PIPE - 15	2.97	INLET - 9	7,052.88	7,053.58	7,052.29	7,053.02	INLET - 8	78.8	0.007	0.024	3.70	31.27	23.2	36.0	7,059.70	7,056.91
111: PIPE - 16	PIPE - 16	3.24	INLET - 8	7,051.99	7,052.80	7,051.20	7,051.90	FES - 10	105.8	0.007	0.024	5.00	31.21	27.1	36.0	7,056.91	7,054.20
144: PIPE - 17	PIPE - 17	3.01	INLET - 7	7,035.09	7,035.48	7,033.57	7,033.96	FES - 9	89.4	0.017	0.024	1.10	7.42	26.0	18.0	7,040.08	7,035.07
147: PIPE - 9	PIPE - 9	3.35	INLET - 5	6,945.48	6,945.94	6,943.87	6,944.32	O-7	89.8	0.018	0.024	1.50	7.62	30.1	18.0	6,948.13	6,945.56
177: PIPE - 7	PIPE - 7	2.67	INLET - 3	6,908.64	6,909.07	6,907.59	6,907.98	O-11	87.0	0.012	0.024	1.10	6.25	28.4	18.0	6,911.24	6,909.09

Figure 1-Q5 – Free Outfall CONDUIT SUMMARY

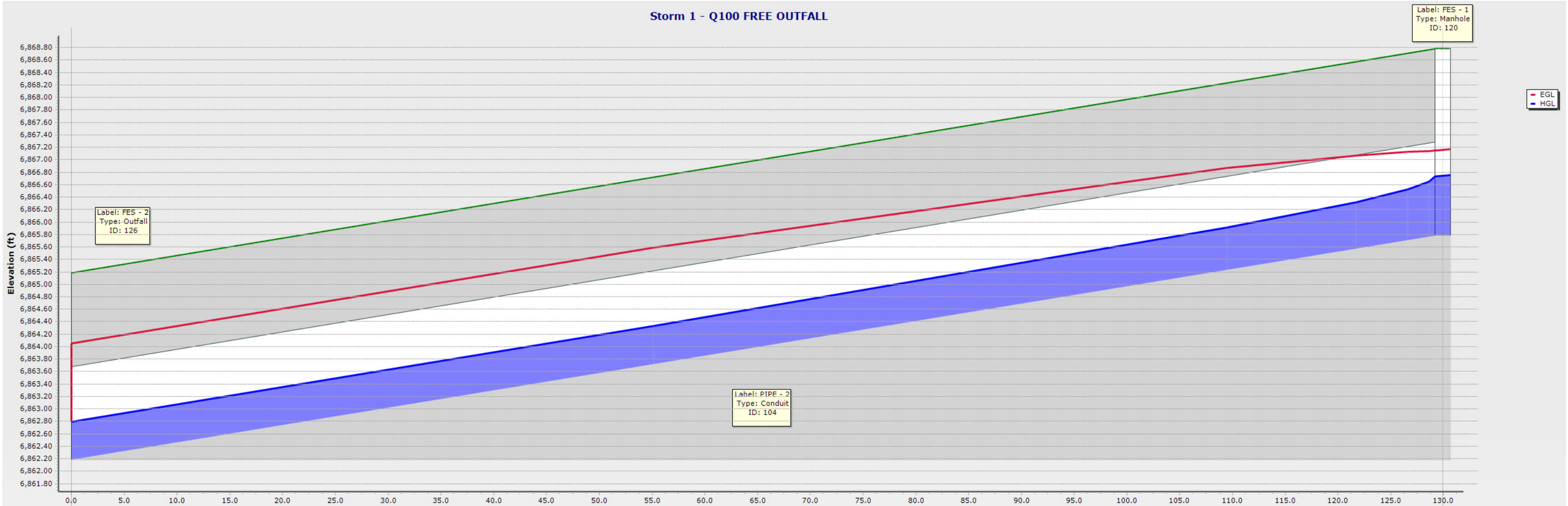
	ID	Label	Flow (Known) (cfs)	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Headloss Method	Headloss Coefficient (Standard)	Flow (Total Out) (cfs)
81: INLET - 6	81	INLET - 6	1.40	6,980.47	6,980.47	0.49	6,976.94	6,976.94	Standard	0.050	1.40
84: INLET - 4	84	INLET - 4	1.60	6,928.68	6,928.68	0.52	6,926.60	6,926.59	Standard	0.050	1.60
86: INLET - 2	86	INLET - 2	5.30	6,883.45	6,883.45	0.76	6,872.24	6,872.06	Standard	0.640	5.30
87: INLET - 1	87	INLET - 1	2.30	6,881.33	6,881.33	0.50	6,875.01	6,875.01	Standard	0.050	2.30
96: INLET - 9	96	INLET - 9	3.70	7,059.70	7,059.70	0.70	7,053.58	7,053.58	Standard	0.050	3.70
97: INLET - 8	97	INLET - 8	5.00	7,056.91	7,056.91	0.81	7,053.02	7,052.80	Standard	1.320	5.00
117: INLET - 7	117	INLET - 7	1.10	7,040.08	7,040.08	0.39	7,035.49	7,035.48	Standard	0.050	1.10
120: FES - 1	120	FES - 1	1.20	6,868.78	6,868.78	0.41	6,866.20	6,866.19	Standard	0.050	1.20
121: MH-5	121	MH-5	2.70	6,870.00	6,870.00	0.62	6,864.98	6,864.97	Standard	0.050	2.70
145: INLET - 5	145	INLET - 5	1.50	6,948.13	6,948.13	0.46	6,945.95	6,945.94	Standard	0.050	1.50
176: INLET - 3	176	INLET - 3	1.10	6,911.24	6,911.24	0.43	6,909.07	6,909.07	Standard	0.050	1.10

Figure 2-Q5 – Free Outfall NODE SUMMARY

	ID	Label	Elevation (Ground) (ft)	Set Rim to Ground Elevation?	Elevation (Invert) (ft)	Boundary Condition Type	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
88: FES - 3	88	FES - 3	6,863.50	<input checked="" type="checkbox"/>	6,862.00	Free Outfall	6,862.52	2.70
98: FES - 10	98	FES - 10	7,054.20	<input checked="" type="checkbox"/>	7,051.20	Free Outfall	7,051.90	5.00
122: FES - 9	122	FES - 9	7,035.07	<input checked="" type="checkbox"/>	7,033.57	Free Outfall	7,033.96	1.10
124: FES - 5	124	FES - 5	6,926.52	<input checked="" type="checkbox"/>	6,925.02	Free Outfall	6,925.50	1.60
125: O-4	125	O-4	6,870.50	<input checked="" type="checkbox"/>	6,868.00	Free Outfall	6,868.58	5.30
126: FES - 2	126	FES - 2	6,865.18	<input checked="" type="checkbox"/>	6,862.18	Free Outfall	6,862.54	1.20
148: O-7	148	O-7	6,945.56	<input checked="" type="checkbox"/>	6,944.06	Free Outfall	6,944.51	1.50
179: O-11	179	O-11	6,909.09	<input checked="" type="checkbox"/>	6,907.59	Free Outfall	6,907.98	1.10
180: O-12	180	O-12	6,976.96	<input checked="" type="checkbox"/>	6,975.46	Free Outfall	6,975.90	1.40

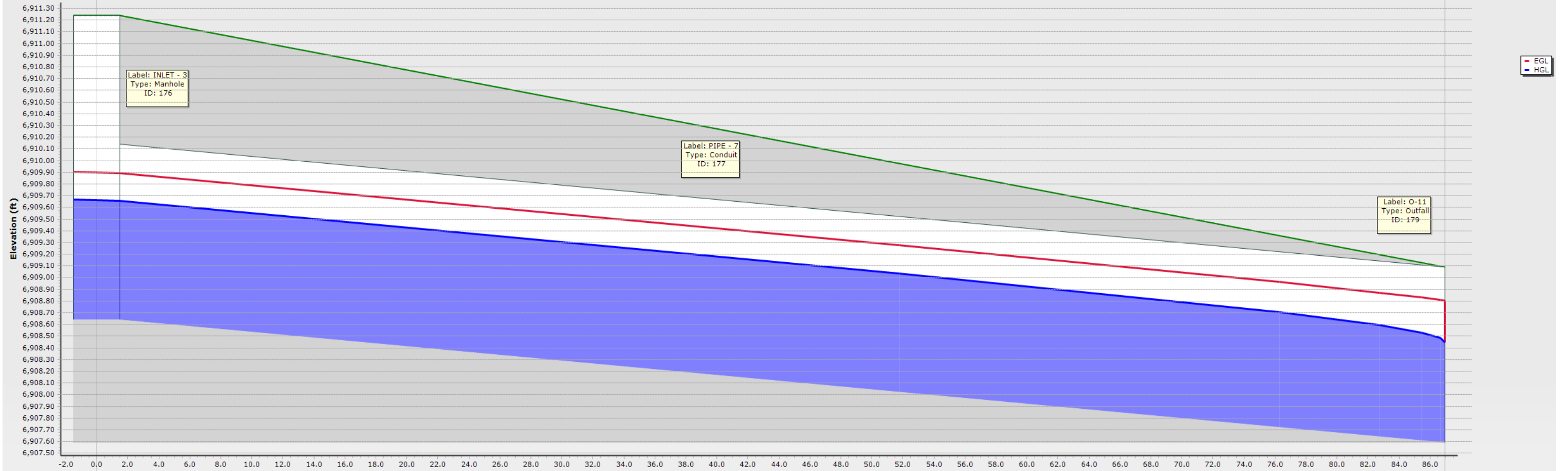
Figure 3-Q5 – Free Outfall OUTFALL SUMMARY

Storm 1 - Q100 FREE OUTFALL



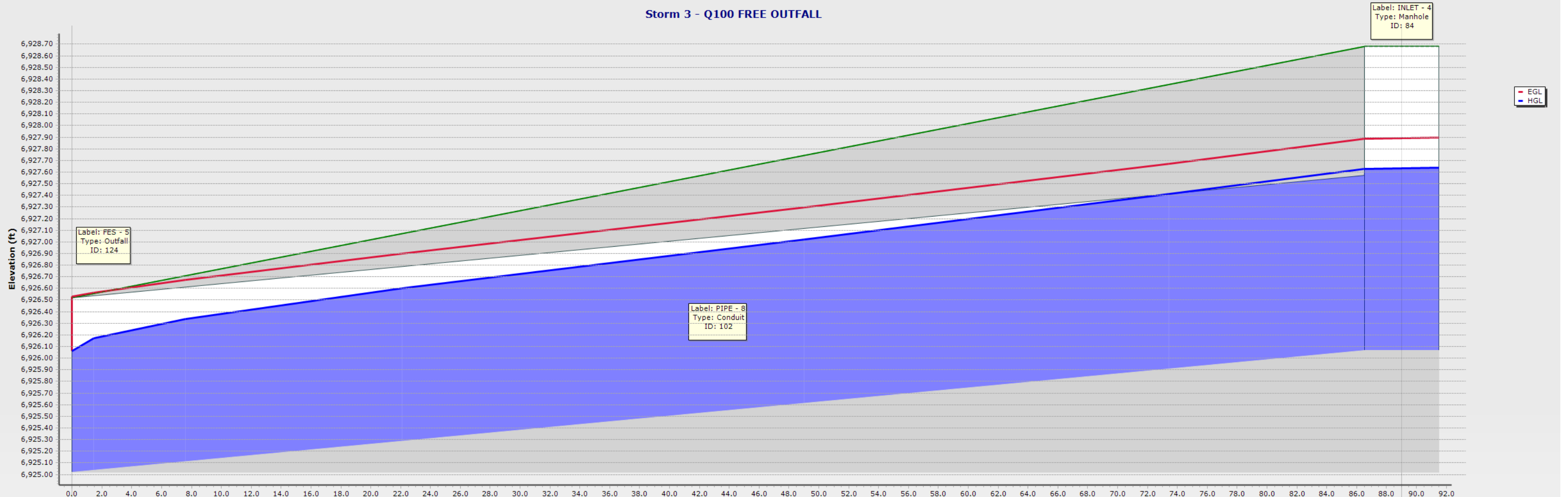
ID\Label	104 \ PIPE - 2	
Link Length (ft)	129.9	
Size (in)\Material	18.0 \ Concrete	
Flow (cfs)	6.10	
Slope (ft/ft)	0.028	
ID\Label	126 \ FES - 2	120 \ FES - 1
Ground (ft)	6865.18	6868.78
Invert (ft)	6862.18	6865.78
Station (ft)	0.0	129.9

Storm 2 - Q100 FREE OUTFALL



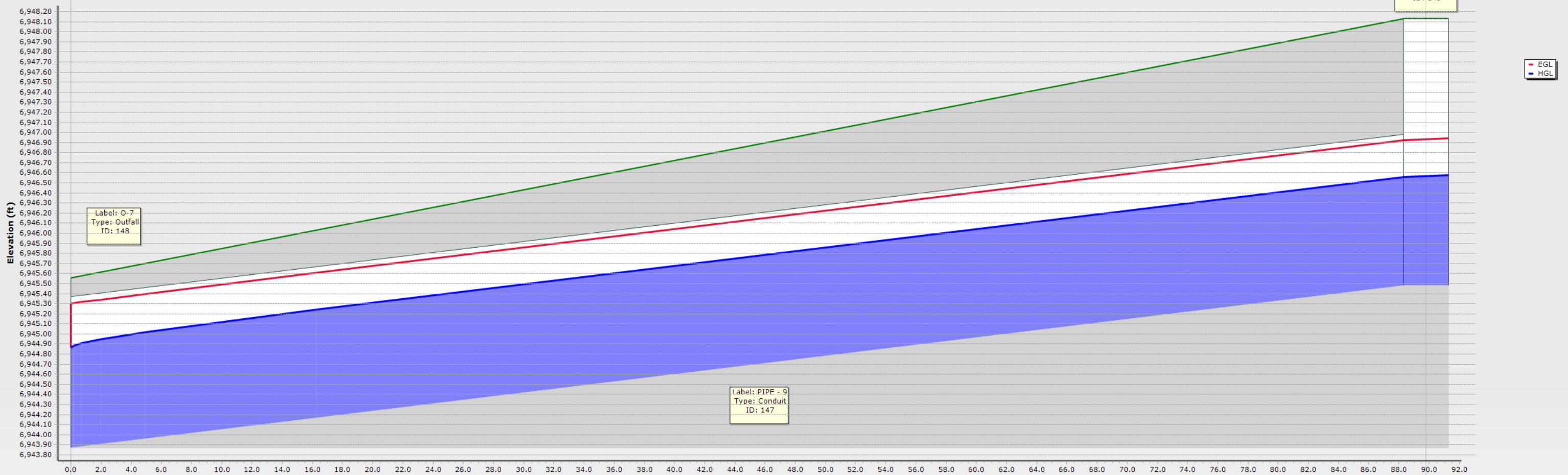
ID\Label			177 \ PIPE - 7
Link Length (ft)			87.0
Rise (in)\Material			18.0 \ CMP
Flow (cfs)			5.00
Slope (ft/ft)			0.012
ID\Label	176 \ INLET - 3		179 \ O-11
Ground (ft)	6911.24		6909.09
Invert (ft)	6908.64		6907.59
Station (ft)	0.0		87.0

Storm 3 - Q100 FREE OUTFALL



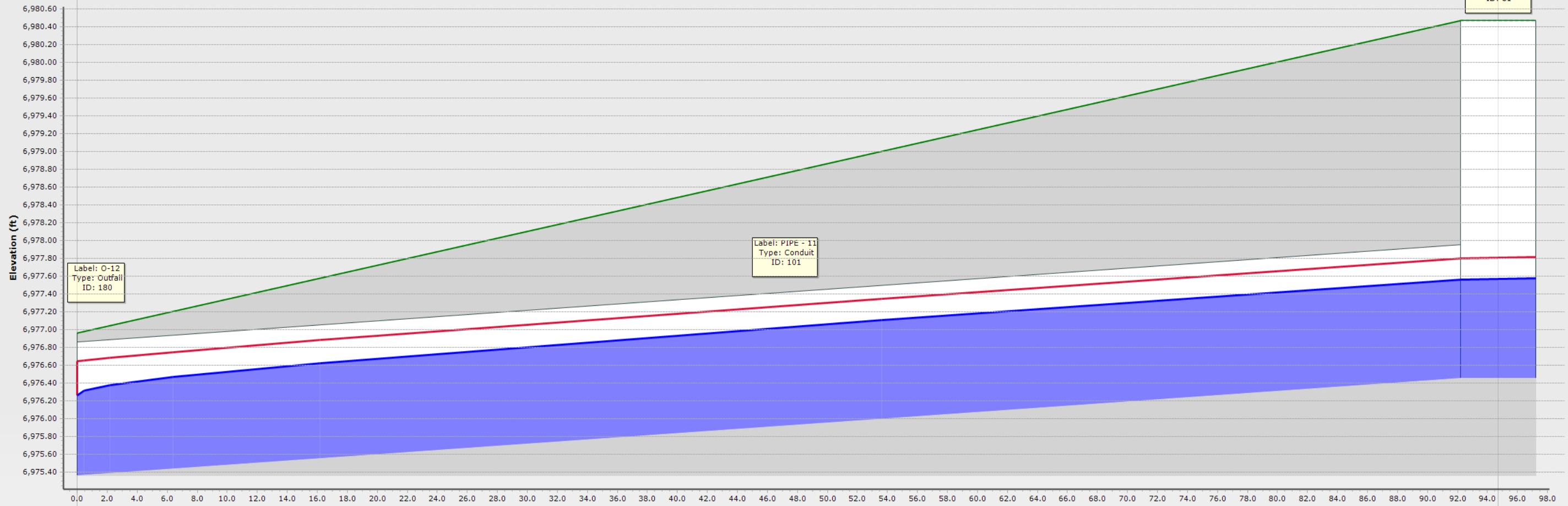
ID\Label	102 \ PIPE - 8	84 \ INLET - 4
Link Length (ft)	89.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	7.20	
Slope (ft/ft)	0.012	
ID\Label	124 \ FES - 5	84 \ INLET - 4
Ground (ft)	6926.52	6928.68
Invert (ft)	6925.02	6926.07
Station (ft)	0.0	89.0

Storm 4 - Q100 FREE OUTFALL



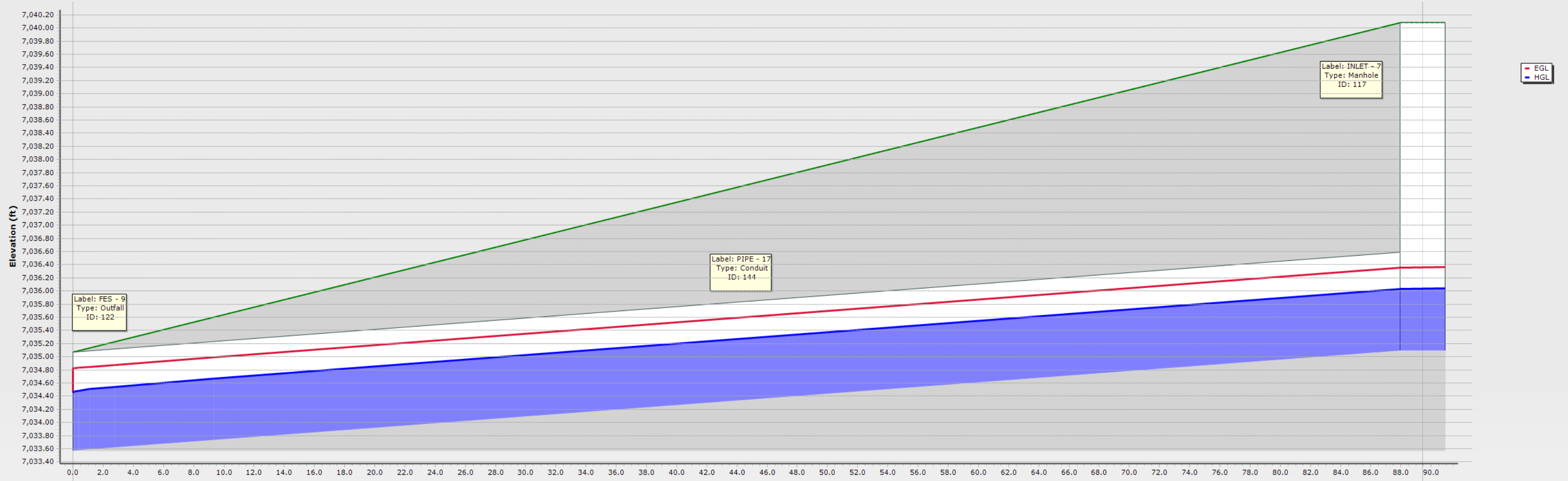
ID\Label	147 \ PIPE - 9	145 \ INLET - 5
Link Length (ft)	89.8	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	6.60	
Slope (ft/ft)	0.018	
ID\Label	148 \ O-7	145 \ INLET - 5
Ground (ft)	6945.56	6948.13
Invert (ft)	6943.87	6945.48
Station (ft)	0.0	89.8

Storm 5 - Q100 FREE OUTFALL



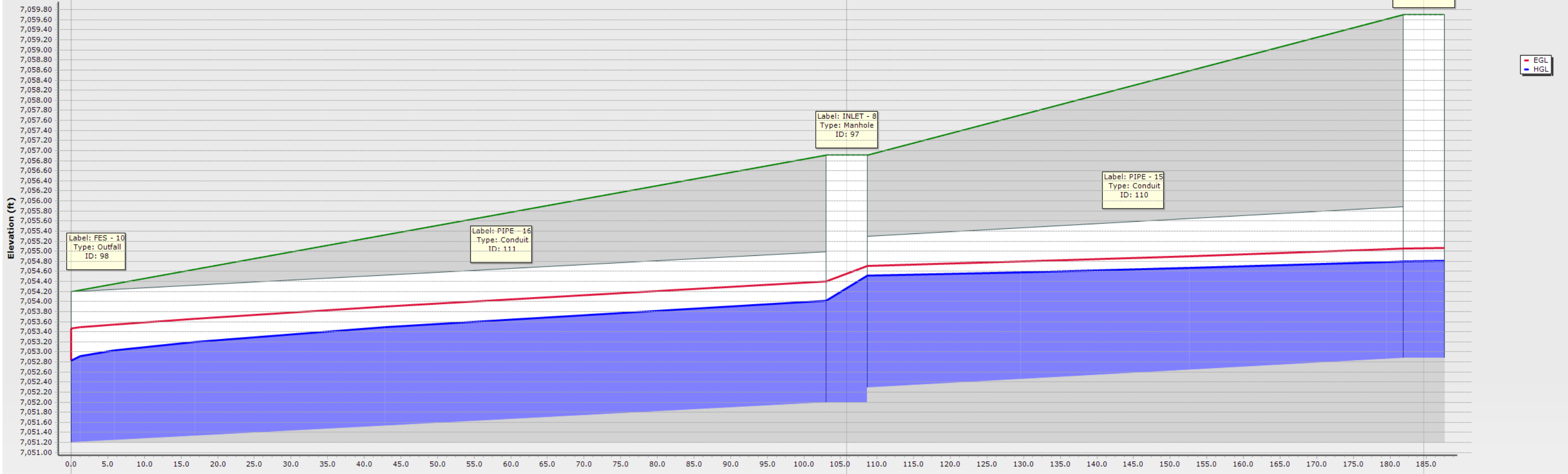
ID\Label	101 \ PIPE - 11	81 \ INLET - 6
Link Length (ft)	94.7	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	5.50	
Slope (ft/ft)	0.012	
ID\Label	80 \ O-12	81 \ INLET - 6
Ground (ft)	6976.96	6980.47
Invert (ft)	6975.36	6976.45
Station (ft)	0.0	94.7

Storm 6 - Q100 FREE OUTFALL



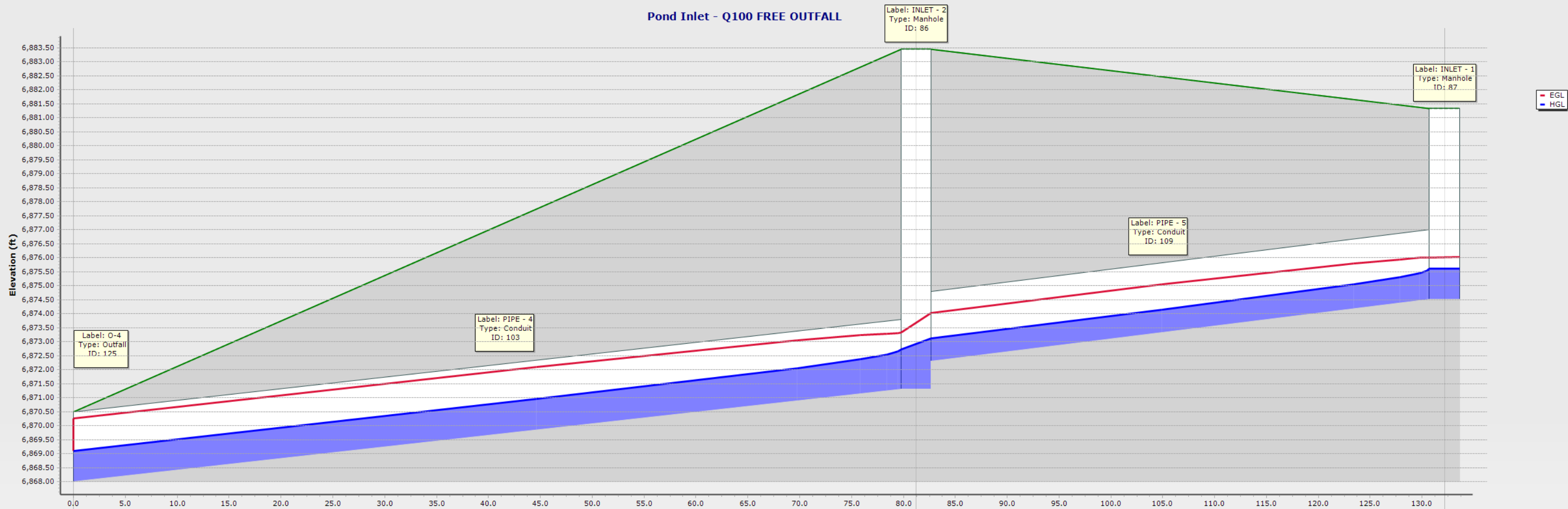
ID\Label	144 \ PIPE - 17	
Link Length (ft)	89.4	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	5.30	
Slope (ft/ft)	0.017	
ID\Label	122 \ FES - 9	117 \ INLET - 7
Ground (ft)	7035.07	7040.08
Invert (ft)	7033.57	7035.09
Station (ft)	0.0	89.4

Storm 7 - Q100 FREE OUTFALL



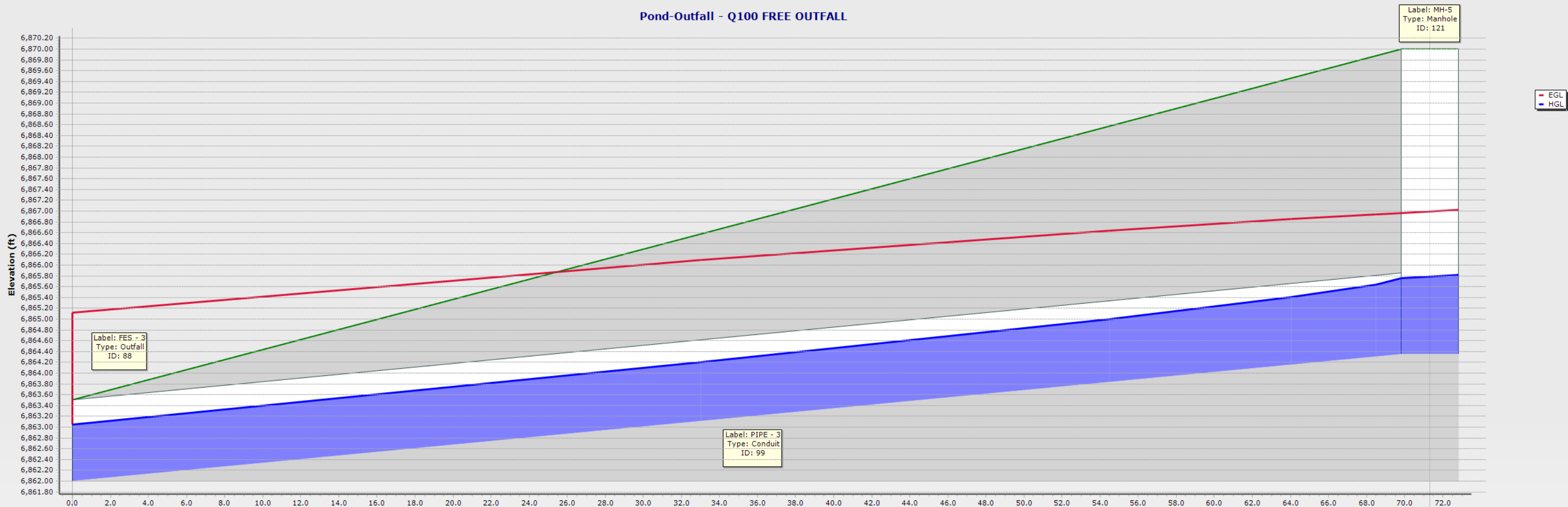
ID\Label	111 \ PIPE - 16	110 \ PIPE - 15	
Link Length (ft)	105.8	78.8	
Size (in)\Material	36.0 \ CMP	36.0 \ CMP	
Flow (cfs)	25.20	19.50	
Slope (ft/ft)	0.007	0.007	
ID\Label	FES - 10	97 \ INLET - 8	96 \ INLET - 9
Ground (ft)	7054.20	7056.91	7059.70
Invert (ft)	7051.20	7051.99	7052.88
Station (ft)	0.0	105.8	184.6

Pond Inlet - Q100 FREE OUTFALL



ID\Label	103 \ PIPE - 4	109 \ PIPE - 5	
Link Length (ft)	81.2	50.9	
Rise (in)\Material	30.0 \ CMP	30.0 \ CMP	
Flow (cfs)	17.90	10.60	
Slope (ft/ft)	0.041	0.043	
ID\Label	125 \ O-4	86 \ INLET - 2	87 \ INLET - 1
Ground (ft)	6870.50	6883.45	6881.33
Invert (ft)	6868.00	6871.30	6874.51
Station (ft)	0.0	81.2	132.2

Pond-Outfall - Q100 FREE OUTFALL



ID\Label	99 \ PIPE - 3
Link Length (ft)	71.3
Rise (in)\Material	18.0 \ Concrete
Flow (cfs)	15.20
Slope (ft/R)	0.033
ID\Label	121 \ MH-5
Ground (ft)	6863.50
Invert (ft)	6862.00
Station (ft)	0.0
	71.3

	Label	Velocity (ft/s)	Start Node	Invert (Start) (ft)	Hydraulic Grade Line (In) (ft)	Invert (Stop) (ft)	Hydraulic Grade Line (Out) (ft)	Stop Node	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Flow (cfs)	Capacity (Design) (cfs)	Depth (Normal) / Rise (%)	Diameter (in)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)
99: PIPE - 3	PIPE - 3	8.60	MH-5	6,864.35	6,868.55	6,862.00	6,863.41	FES - 3	71.3	0.033	0.024	15.20	10.33	(N/A)	18.0	6,870.00	6,863.50
101: PIPE - 11	PIPE - 11	3.91	INLET - 6	6,976.45	6,977.56	6,975.36	6,976.26	O-12	94.7	0.012	0.024	5.50	6.10	74.2	18.0	6,980.47	6,976.96
102: PIPE - 8	PIPE - 8	4.07	INLET - 4	6,926.07	6,927.63	6,925.02	6,926.06	FES - 5	89.0	0.012	0.024	7.20	6.18	(N/A)	18.0	6,928.68	6,926.52
103: PIPE - 4	PIPE - 4	8.61	INLET - 2	6,871.30	6,872.73	6,868.00	6,869.10	O-4	81.2	0.041	0.024	17.90	44.78	44.0	30.0	6,883.45	6,870.50
104: PIPE - 2	PIPE - 2	5.69	FES - 1	6,865.78	6,866.73	6,862.18	6,863.06	FES - 2	129.9	0.028	0.024	6.10	9.47	58.4	18.0	6,868.78	6,865.18
109: PIPE - 5	PIPE - 5	7.65	INLET - 1	6,874.51	6,875.60	6,872.30	6,873.11	INLET - 2	50.9	0.043	0.024	10.60	46.27	32.6	30.0	6,881.33	6,883.45
110: PIPE - 15	PIPE - 15	4.66	INLET - 9	7,052.88	7,054.79	7,052.29	7,054.52	INLET - 8	78.8	0.007	0.024	19.50	31.27	57.2	36.0	7,059.70	7,056.91
111: PIPE - 16	PIPE - 16	4.91	INLET - 8	7,051.99	7,054.01	7,051.20	7,052.82	FES - 10	105.8	0.007	0.024	25.20	31.21	68.1	36.0	7,056.91	7,054.20
144: PIPE - 17	PIPE - 17	4.56	INLET - 7	7,035.09	7,036.03	7,033.57	7,034.46	FES - 9	89.4	0.017	0.024	5.30	7.42	62.5	18.0	7,040.08	7,035.07
147: PIPE - 9	PIPE - 9	4.85	INLET - 5	6,945.48	6,946.56	6,943.87	6,944.86	O-7	89.8	0.018	0.024	6.60	7.62	71.9	18.0	6,948.13	6,945.56
177: PIPE - 7	PIPE - 7	3.93	INLET - 3	6,908.64	6,909.65	6,907.59	6,908.45	O-11	87.0	0.012	0.024	5.00	6.25	67.6	18.0	6,911.24	6,909.09

Figure 4- Q100 – Free Outfall CONDUIT SUMMARY

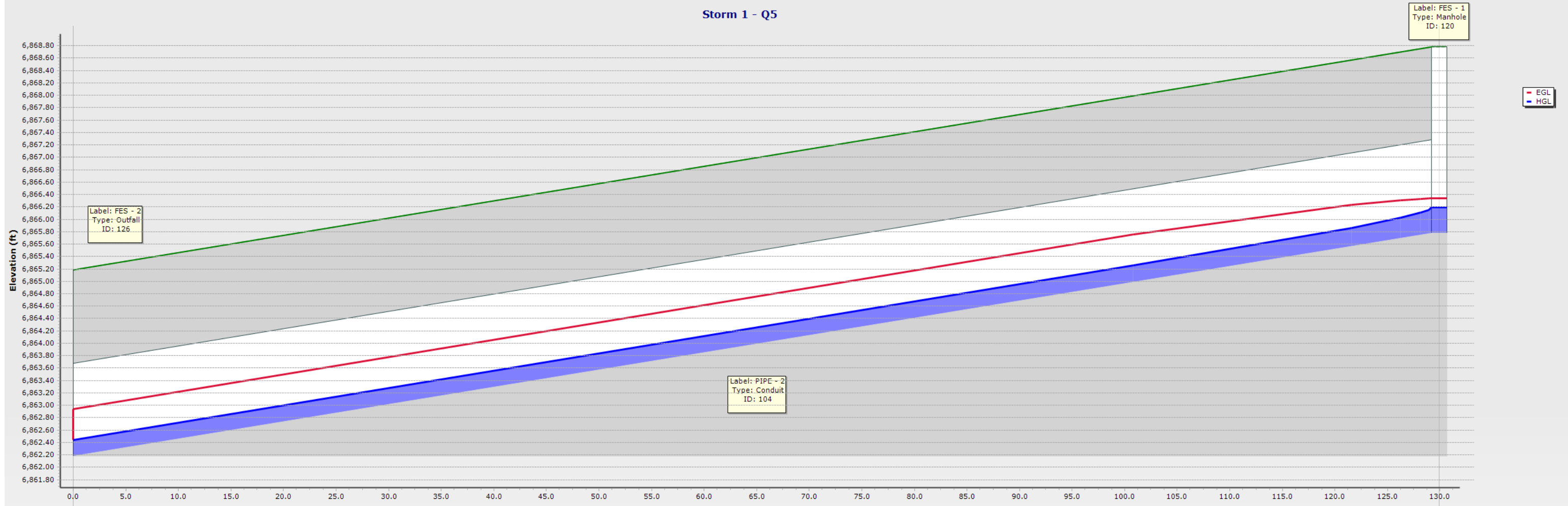
	ID	Label	Flow (Known) (cfs)	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Headloss Method	Headloss Coefficient (Standard)	Flow (Total Out) (cfs)
81: INLET - 6	81	INLET - 6	5.50	6,980.47	6,980.47	1.11	6,977.57	6,977.56	Standard	0.050	5.50
84: INLET - 4	84	INLET - 4	7.20	6,928.68	6,928.68	1.56	6,927.64	6,927.63	Standard	0.050	7.20
86: INLET - 2	86	INLET - 2	17.90	6,883.45	6,883.45	1.43	6,873.11	6,872.73	Standard	0.640	17.90
87: INLET - 1	87	INLET - 1	10.60	6,881.33	6,881.33	1.09	6,875.62	6,875.60	Standard	0.050	10.60
96: INLET - 9	96	INLET - 9	19.50	7,059.70	7,059.70	1.91	7,054.81	7,054.79	Standard	0.050	19.50
97: INLET - 8	97	INLET - 8	25.20	7,056.91	7,056.91	2.02	7,054.52	7,054.01	Standard	1.320	25.20
117: INLET - 7	117	INLET - 7	5.30	7,040.08	7,040.08	0.94	7,036.04	7,036.03	Standard	0.050	5.30
120: FES - 1	120	FES - 1	6.10	6,868.78	6,868.78	0.95	6,866.75	6,866.73	Standard	0.050	6.10
121: MH-5	121	MH-5	15.20	6,870.00	6,870.00	4.20	6,868.61	6,868.55	Standard	0.050	15.20
145: INLET - 5	145	INLET - 5	6.60	6,948.13	6,948.13	1.08	6,946.58	6,946.56	Standard	0.050	6.60
176: INLET - 3	176	INLET - 3	5.00	6,911.24	6,911.24	1.01	6,909.67	6,909.65	Standard	0.050	5.00

Figure 5- Q100 – Free Outfall NODE SUMMARY

	ID	Label	Elevation (Ground) (ft)	Set Rim to Ground Elevation?	Elevation (Invert) (ft)	Boundary Condition Type	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
88: FES - 3	88	FES - 3	6,863.50	<input checked="" type="checkbox"/>	6,862.00	Free Outfall	6,863.41	15.20
98: FES - 10	98	FES - 10	7,054.20	<input checked="" type="checkbox"/>	7,051.20	Free Outfall	7,052.82	25.20
122: FES - 9	122	FES - 9	7,035.07	<input checked="" type="checkbox"/>	7,033.57	Free Outfall	7,034.46	5.30
124: FES - 5	124	FES - 5	6,926.52	<input checked="" type="checkbox"/>	6,925.02	Free Outfall	6,926.06	7.20
125: O-4	125	O-4	6,870.50	<input checked="" type="checkbox"/>	6,868.00	Free Outfall	6,869.10	17.90
126: FES - 2	126	FES - 2	6,865.18	<input checked="" type="checkbox"/>	6,862.18	Free Outfall	6,863.06	6.10
148: O-7	148	O-7	6,945.56	<input checked="" type="checkbox"/>	6,944.06	Free Outfall	6,945.05	6.60
179: O-11	179	O-11	6,909.09	<input checked="" type="checkbox"/>	6,907.59	Free Outfall	6,908.45	5.00
180: O-12	180	O-12	6,976.96	<input checked="" type="checkbox"/>	6,975.46	Free Outfall	6,976.36	5.50

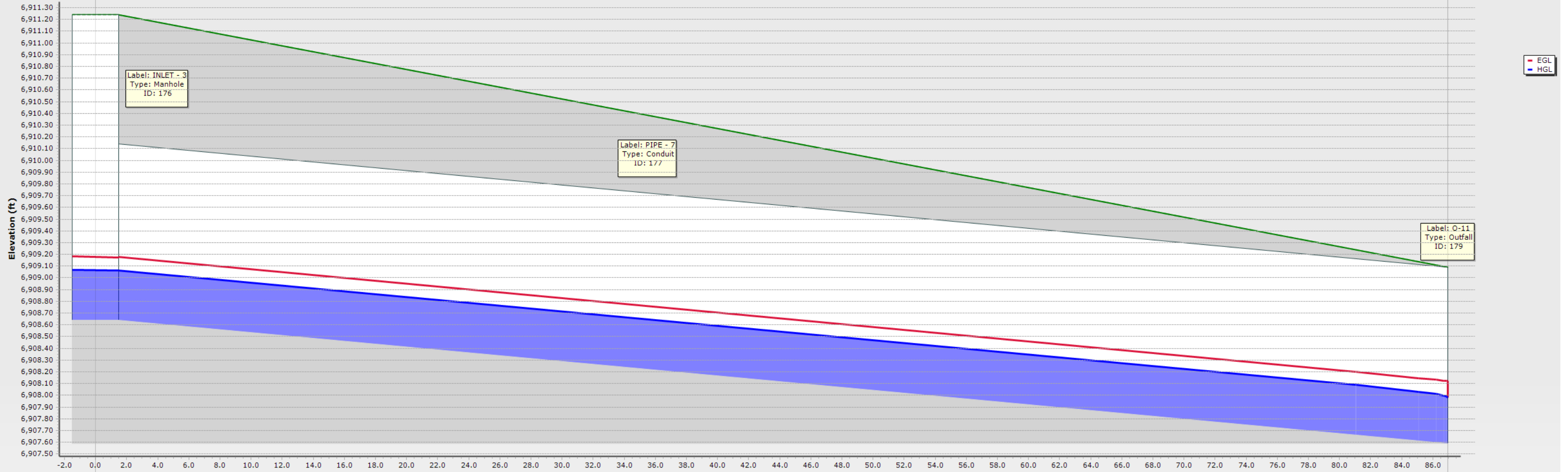
Figure 6- Q100 Free Outfall OUTFALL SUMMARY

Storm 1 - Q5



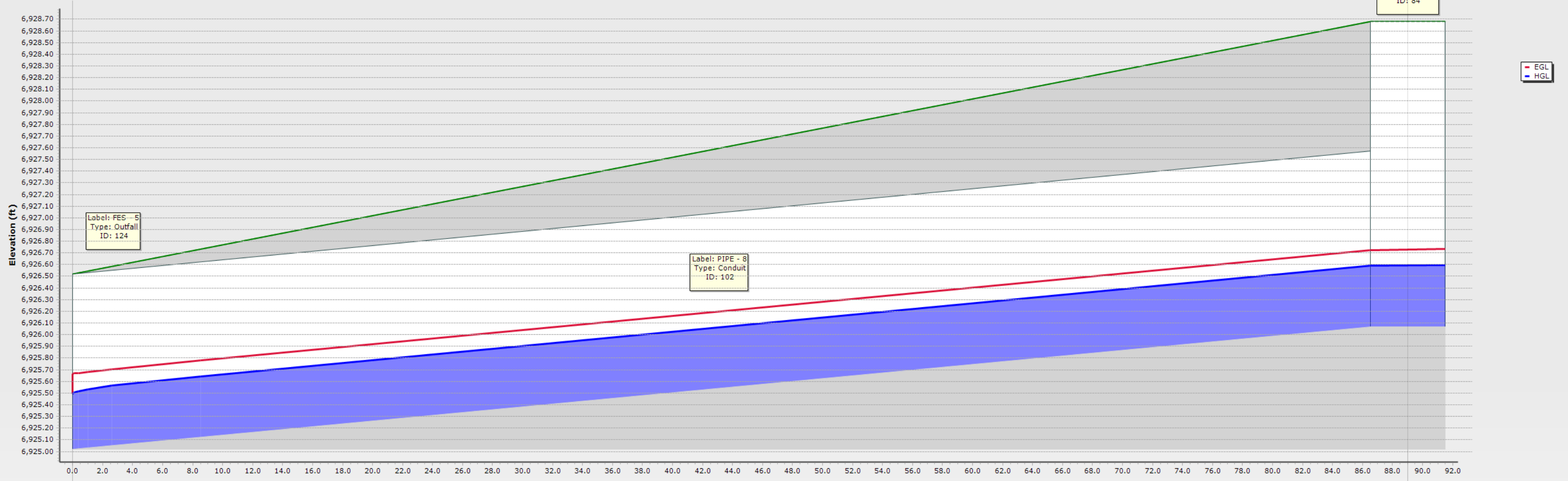
ID\Label		104 \ PIPE - 2	
Link Length (ft)		129.9	
Rise (in)\Material		18.0 \ Concrete	
Flow (cfs)		1.20	
Slope (ft/ft)		0.028	
ID\Label	126 \ FES - 2		120 \ FES - 1
Ground (ft)	6865.18		6868.78
Invert (ft)	6862.18		6865.78
Station (ft)	0.0		129.9

Storm 2 - Q5



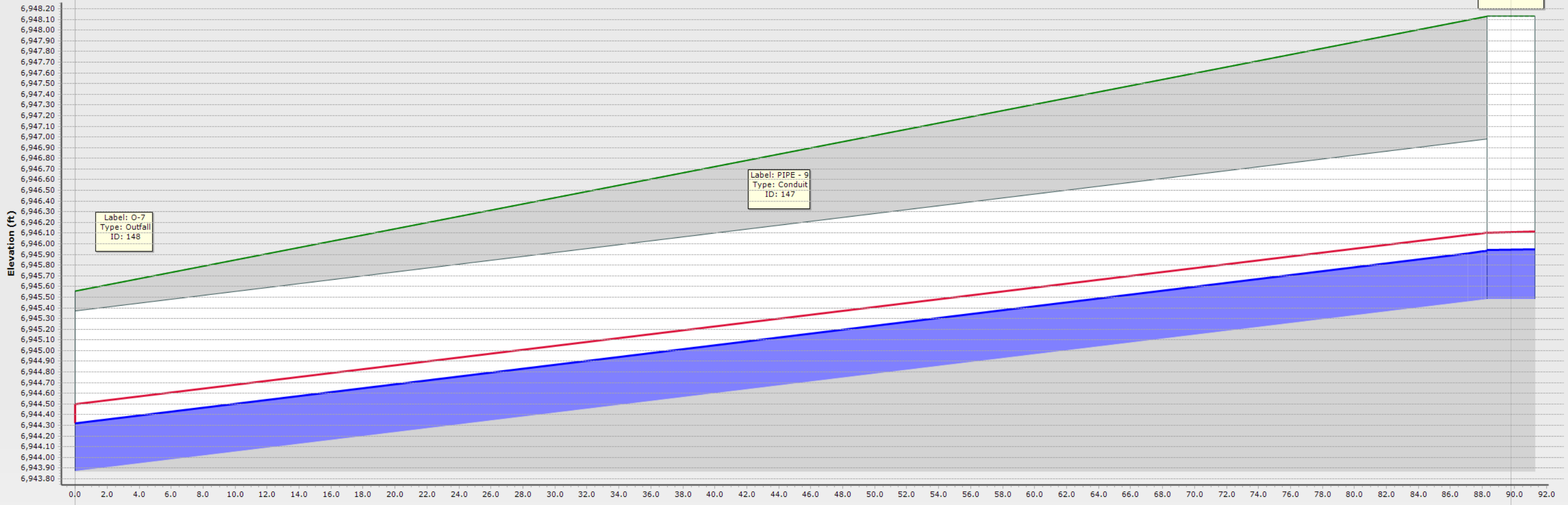
ID\Label	177 \ PIPE - 7	
Link Length (ft)	87.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.10	
Slope (ft/ft)	0.012	
ID\Label	176 \ INLET - 3	179 \ O-11
Ground (ft)	6911.24	6909.09
Invert (ft)	6908.64	6907.59
Station (ft)	0.0	87.0

Storm 3 - Q5



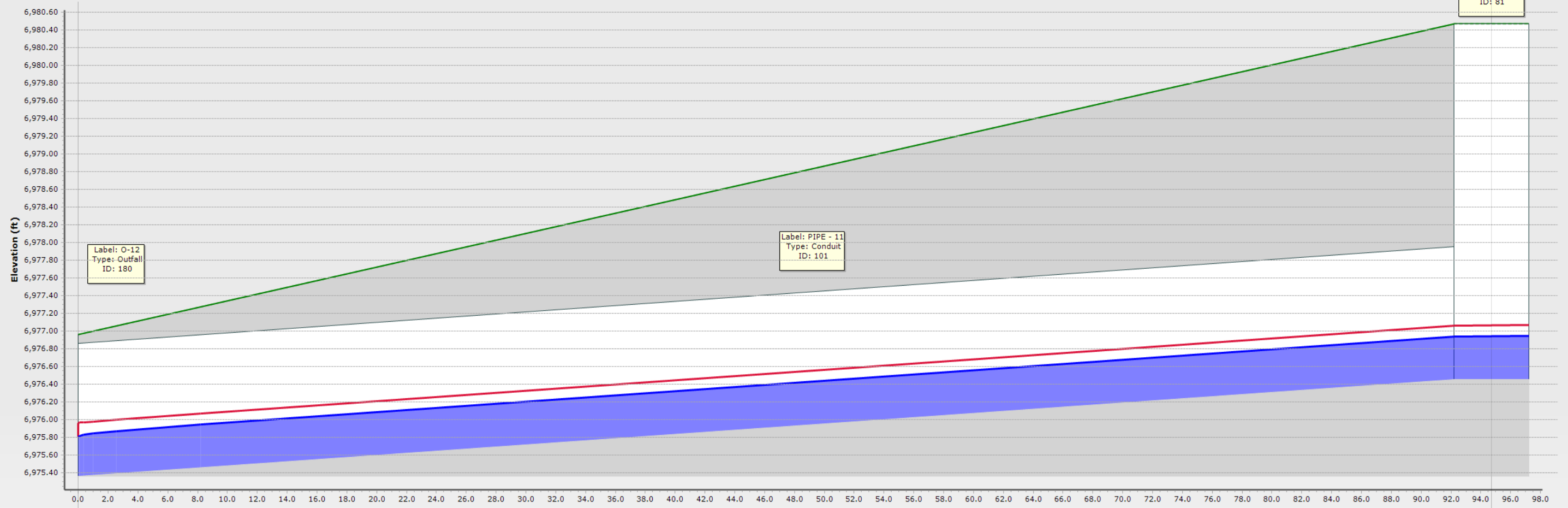
ID\Label		102 \ PIPE - 8	
Link Length (ft)		89.0	
Rise (in)\Material		18.0 \ CMP	
Flow (cfs)		1.60	
Slope (ft/ft)		0.012	
ID\Label	124 \ FES - 5		84 \ INLET - 4
Ground (ft)	6925.52		6928.68
Invert (ft)	6925.02		6926.07
Station (ft)	0.0		89.0

Storm 4 - Q5



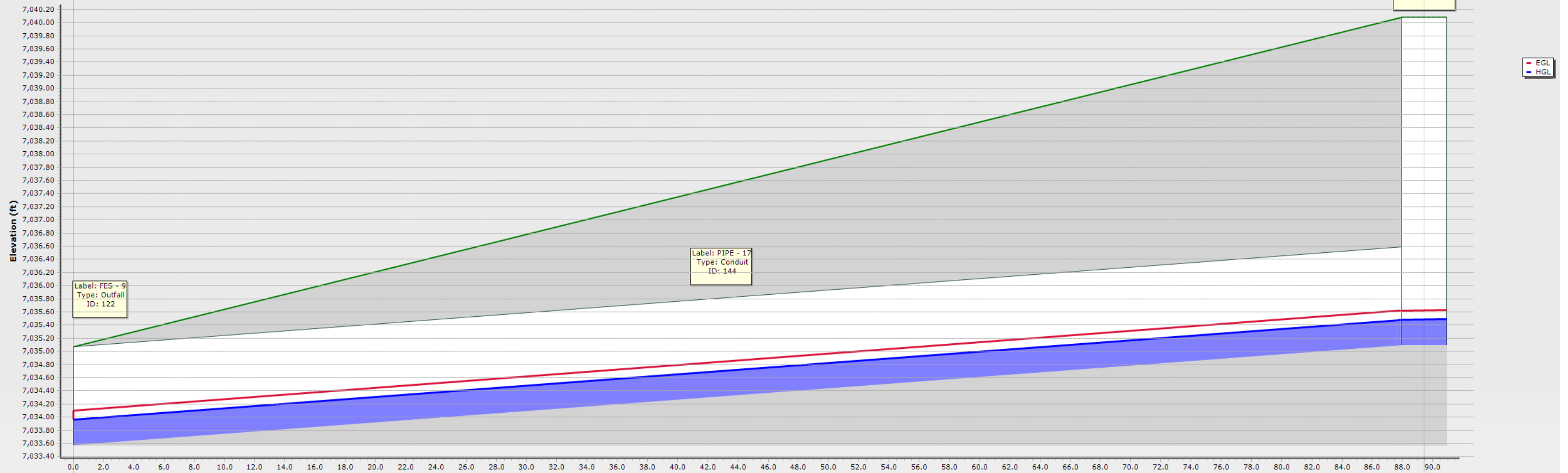
ID\Label	147 \ PIPE - 9	
Link Length (ft)	89.8	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.50	
Slope (ft/ft)	0.018	
ID\Label	148 \ O-7	145 \ INLET - 5
Ground (ft)	6945.56	6948.13
Invert (ft)	6943.87	6945.48
Station (ft)	0.0	89.8

Storm 5 - Q5



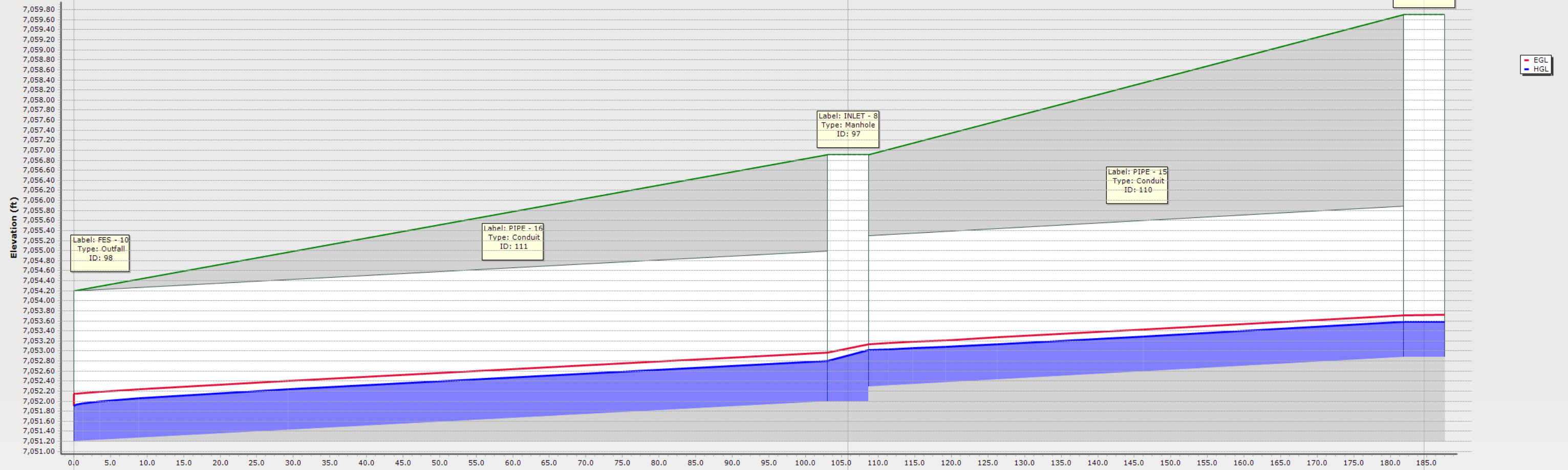
ID\Label	101 \ PIPE - 11	
Link Length (ft)	94.7	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.40	
Slope (ft/ft)	0.012	
ID\Label	80 \ O-12	81 \ INLET - 6
Ground (ft)	6975.96	6980.47
Invert (ft)	6975.36	6976.45
Station (ft)	0.0	94.7

Storm 6 - Q5



ID\Label	144 \ PIPE - 17	117 \ INLET - 7
Link Length (ft)	89.4	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	1.10	
Slope (ft/ft)	0.017	
ID\Label	12 \ FES - 9	117 \ INLET - 7
Ground (ft)	7035.07	7040.08
Invert (ft)	7033.57	7035.09
Station (ft)	0.0	89.4

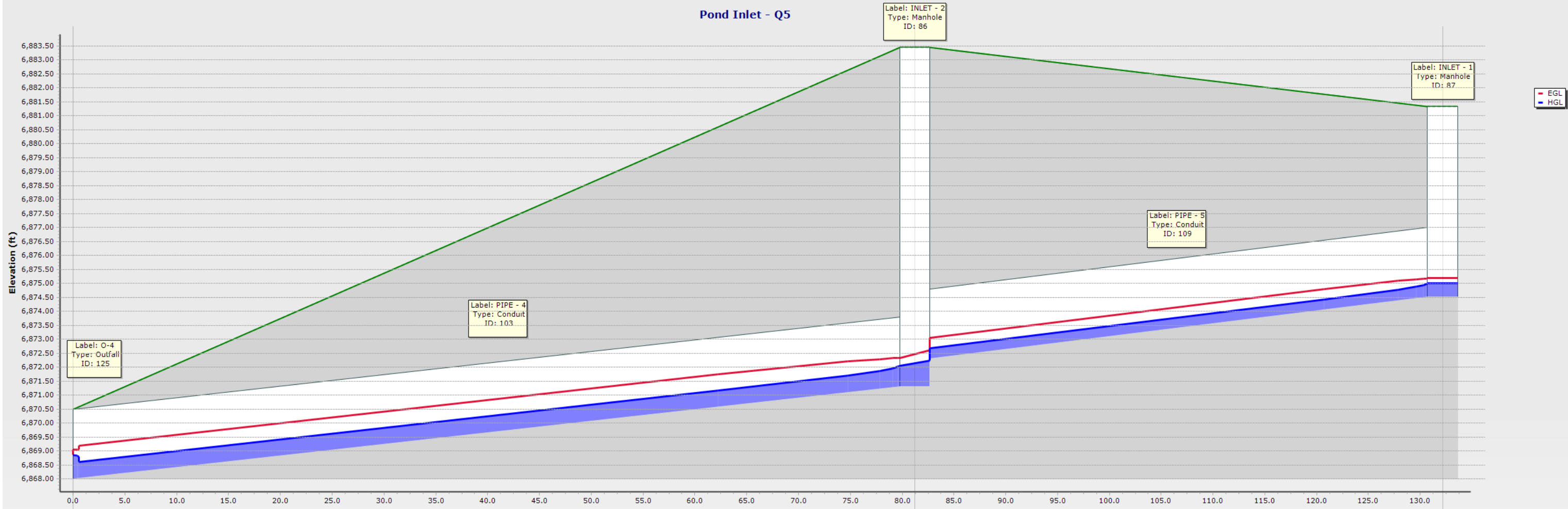
Storm 7 - Q5



ID\Label	111 \ PIPE - 16	110 \ PIPE - 15
Link Length (ft)	105.8	78.8
Rise (in)\Material	36.0 \ CMP	36.0 \ CMP
Flow (cfs)	5.00	3.70
Slope (ft/ft)	0.007	0.007

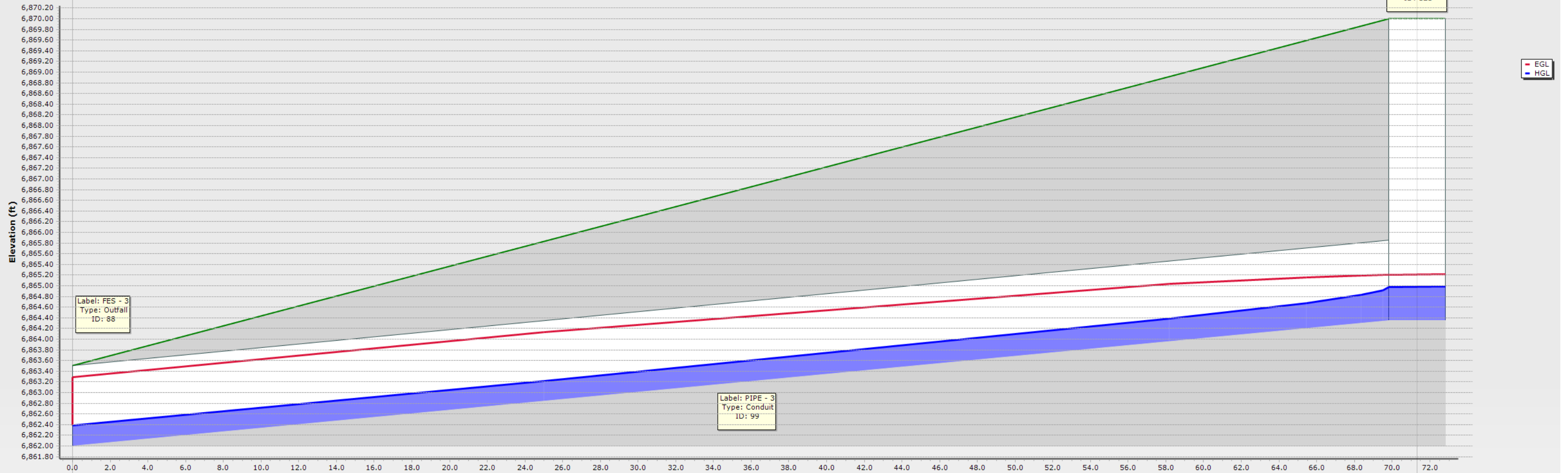
ID\Label	97 \ INLET - 8	96 \ INLET - 9
Ground (ft)	7056.91	7059.70
Invert (ft)	7051.99	7052.88
Station (ft)	105.8	184.6

Pond Inlet - Q5



ID\Label	103 \ PIPE - 4	109 \ PIPE - 5	
Link Length (ft)	81.2	50.9	
Rise (in)\Material	30.0 \ CMP	30.0 \ CMP	
Flow (cfs)	5.30	2.30	
Slope (ft/ft)	0.041	0.043	
ID\Label	125 \ O-4	86 \ INLET - 2	87 \ INLET - 1
Ground (ft)	6870.50	6883.45	6881.33
Invert (ft)	6868.00	6871.30	6874.51
Station (ft)	0.0	81.2	132.2

Pond-Outfall - Q5



ID\Label	99 \ PIPE - 3	121 \ MH-5
Link Length (ft)	71.3	
Rise (in)\Material	18.0 \ Concrete	
Flow (cfs)	2.70	
Slope (ft/ft)	0.033	
ID\Label	88 \ FES - 3	
Ground (ft)	6863.50	6870.00
Invert (ft)	6862.00	6864.35
Station (ft)	0.0	71.3

	Label	Velocity (ft/s)	Start Node	Invert (Start) (ft)	Hydraulic Grade Line (In) (ft)	Invert (Stop) (ft)	Hydraulic Grade Line (Out) (ft)	Stop Node	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Flow (cfs)	Capacity (Design) (cfs)	Depth (Normal) / Rise (%)	Diameter (in)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)
99: PIPE - 3	PIPE - 3	4.92	MH-5	6,864.35	6,864.97	6,862.00	6,862.52	FES - 3	71.3	0.033	0.024	2.70	10.33	34.9	18.0	6,870.00	6,863.50
101: PIPE - 11	PIPE - 11	2.80	INLET - 6	6,976.45	6,976.94	6,975.36	6,975.80	O-12	94.7	0.012	0.024	1.40	6.10	32.6	18.0	6,980.47	6,976.96
102: PIPE - 8	PIPE - 8	2.94	INLET - 4	6,926.07	6,926.59	6,925.02	6,925.50	FES - 5	89.0	0.012	0.024	1.60	6.18	34.7	18.0	6,928.68	6,926.52
103: PIPE - 4	PIPE - 4	6.13	INLET - 2	6,871.30	6,872.06	6,868.00	6,868.85	O-4	81.2	0.041	0.024	5.30	44.78	23.2	30.0	6,883.45	6,870.50
104: PIPE - 2	PIPE - 2	3.67	FES - 1	6,865.78	6,866.19	6,862.18	6,862.54	FES - 2	129.9	0.028	0.024	1.20	9.47	24.0	18.0	6,868.78	6,865.18
109: PIPE - 5	PIPE - 5	4.90	INLET - 1	6,874.51	6,875.01	6,872.30	6,872.68	INLET - 2	50.9	0.043	0.024	2.30	46.27	15.2	30.0	6,881.33	6,883.45
110: PIPE - 15	PIPE - 15	2.97	INLET - 9	7,052.88	7,053.58	7,052.29	7,053.02	INLET - 8	78.8	0.007	0.024	3.70	31.27	23.2	36.0	7,059.70	7,056.91
111: PIPE - 16	PIPE - 16	3.24	INLET - 8	7,051.99	7,052.80	7,051.20	7,051.90	FES - 10	105.8	0.007	0.024	5.00	31.21	27.1	36.0	7,056.91	7,054.20
144: PIPE - 17	PIPE - 17	3.01	INLET - 7	7,035.09	7,035.48	7,033.57	7,033.96	FES - 9	89.4	0.017	0.024	1.10	7.42	26.0	18.0	7,040.08	7,035.07
147: PIPE - 9	PIPE - 9	3.35	INLET - 5	6,945.48	6,945.94	6,943.87	6,944.32	O-7	89.8	0.018	0.024	1.50	7.62	30.1	18.0	6,948.13	6,945.56
177: PIPE - 7	PIPE - 7	2.67	INLET - 3	6,908.64	6,909.07	6,907.59	6,907.98	O-11	87.0	0.012	0.024	1.10	6.25	28.4	18.0	6,911.24	6,909.09

Figure 7- Q5 CONDUIT SUMMARY

	ID	Label	Flow (Known) (cfs)	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Headloss Method	Headloss Coefficient (Standard)	Flow (Total Out) (cfs)
81: INLET - 6	81	INLET - 6	1.40	6,980.47	6,980.47	0.49	6,976.94	6,976.94	Standard	0.050	1.40
84: INLET - 4	84	INLET - 4	1.60	6,928.68	6,928.68	0.52	6,926.60	6,926.59	Standard	0.050	1.60
86: INLET - 2	86	INLET - 2	5.30	6,883.45	6,883.45	0.76	6,872.24	6,872.06	Standard	0.640	5.30
87: INLET - 1	87	INLET - 1	2.30	6,881.33	6,881.33	0.50	6,875.01	6,875.01	Standard	0.050	2.30
96: INLET - 9	96	INLET - 9	3.70	7,059.70	7,059.70	0.70	7,053.58	7,053.58	Standard	0.050	3.70
97: INLET - 8	97	INLET - 8	5.00	7,056.91	7,056.91	0.81	7,053.02	7,052.80	Standard	1.320	5.00
117: INLET - 7	117	INLET - 7	1.10	7,040.08	7,040.08	0.39	7,035.49	7,035.48	Standard	0.050	1.10
120: FES - 1	120	FES - 1	1.20	6,868.78	6,868.78	0.41	6,866.20	6,866.19	Standard	0.050	1.20
121: MH-5	121	MH-5	2.70	6,870.00	6,870.00	0.62	6,864.98	6,864.97	Standard	0.050	2.70
145: INLET - 5	145	INLET - 5	1.50	6,948.13	6,948.13	0.46	6,945.95	6,945.94	Standard	0.050	1.50
176: INLET - 3	176	INLET - 3	1.10	6,911.24	6,911.24	0.43	6,909.07	6,909.07	Standard	0.050	1.10

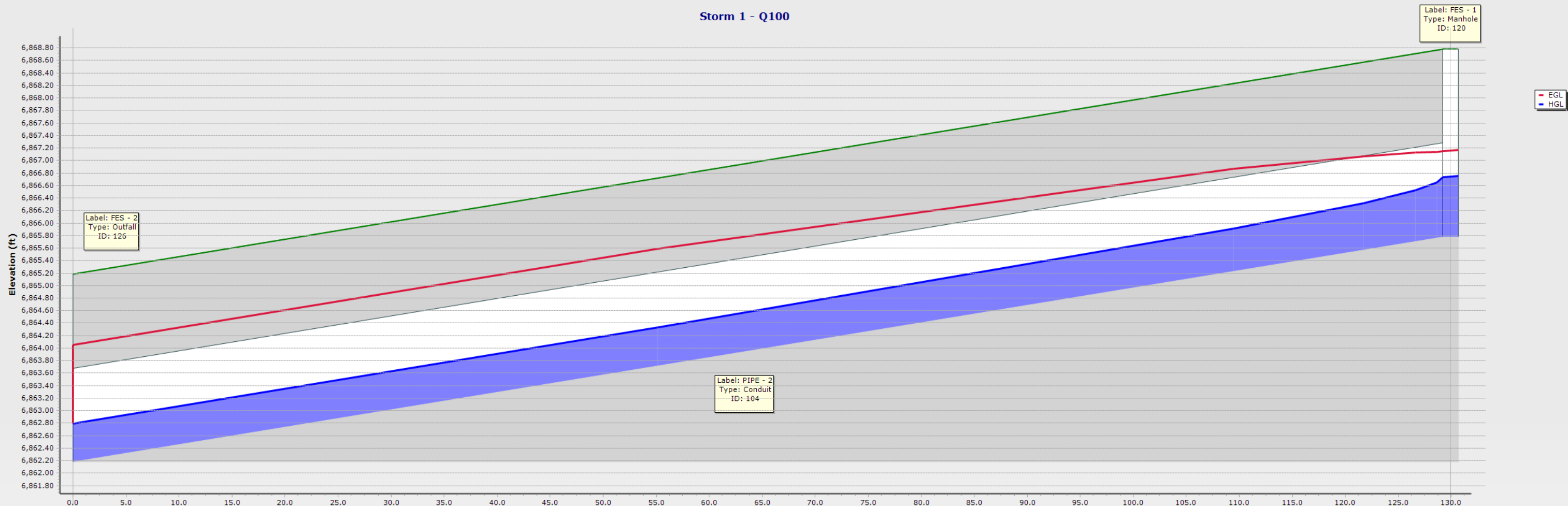
Figure 8- Q5 NODE SUMMARY

	ID	Label	Elevation (Ground) (ft)	Set Rim to Ground Elevation?	Elevation (Invert) (ft)	Boundary Condition Type	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
88: FES - 3	88	FES - 3	6,863.50	<input checked="" type="checkbox"/>	6,862.00	Free Outfall	6,862.52	2.70
98: FES - 10	98	FES - 10	7,054.20	<input checked="" type="checkbox"/>	7,051.20	Free Outfall	7,051.90	5.00
122: FES - 9	122	FES - 9	7,035.07	<input checked="" type="checkbox"/>	7,033.57	Free Outfall	7,033.96	1.10
124: FES - 5	124	FES - 5	6,926.52	<input checked="" type="checkbox"/>	6,925.02	Free Outfall	6,925.50	1.60
125: O-4	125	O-4	6,870.50	<input checked="" type="checkbox"/>	6,868.00	User Defined Tailwater	6,868.85	5.30
126: FES - 2	126	FES - 2	6,865.18	<input checked="" type="checkbox"/>	6,862.18	Free Outfall	6,862.54	1.20
148: O-7	148	O-7	6,945.56	<input checked="" type="checkbox"/>	6,944.06	Free Outfall	6,944.51	1.50
179: O-11	179	O-11	6,909.09	<input checked="" type="checkbox"/>	6,907.59	Free Outfall	6,907.98	1.10
180: O-12	180	O-12	6,976.96	<input checked="" type="checkbox"/>	6,975.46	Free Outfall	6,975.90	1.40

Figure 9- Q5 OUTFALL SUMMARY

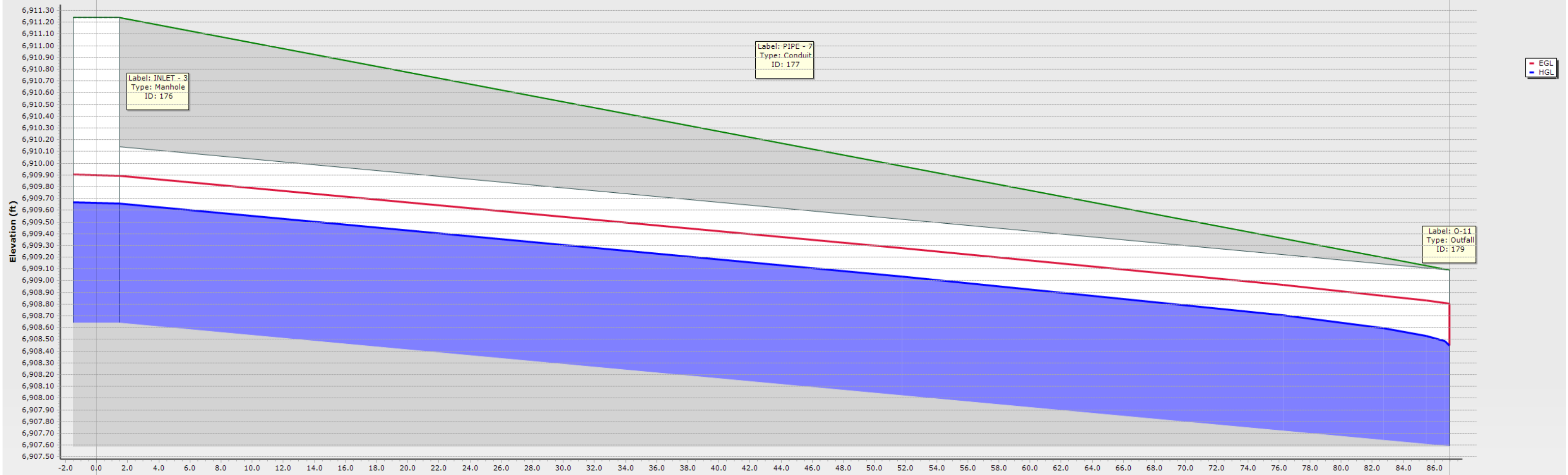
THE TAILWATER CONDITION HAS BEEN DEINED AS THE POND WSE DURING THE CORRESPONDING STORM EVENT FOR OUTFALLS THAT DISCHARGE INTO POND 1.

Storm 1 - Q100



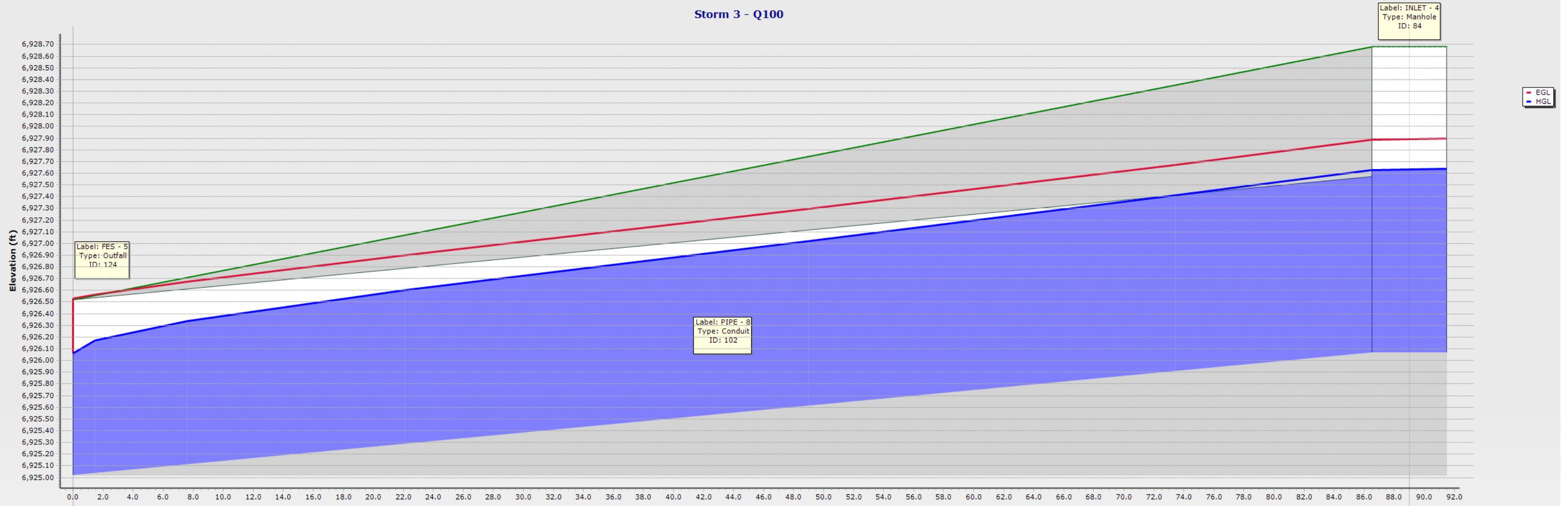
ID\Label	104 \ PIPE - 2	120 \ FES - 1
Link Length (ft)	129.9	
Rise (in)\Material	18.0 \ Concrete	
Flow (cfs)	6.10	
Slope (ft/ft)	0.028	
ID\Label	126 \ FES - 2	120 \ FES - 1
Ground (ft)	6865.18	6868.78
Invert (ft)	6862.18	6865.78
Station (ft)	0.0	129.9

Storm 2 - Q100



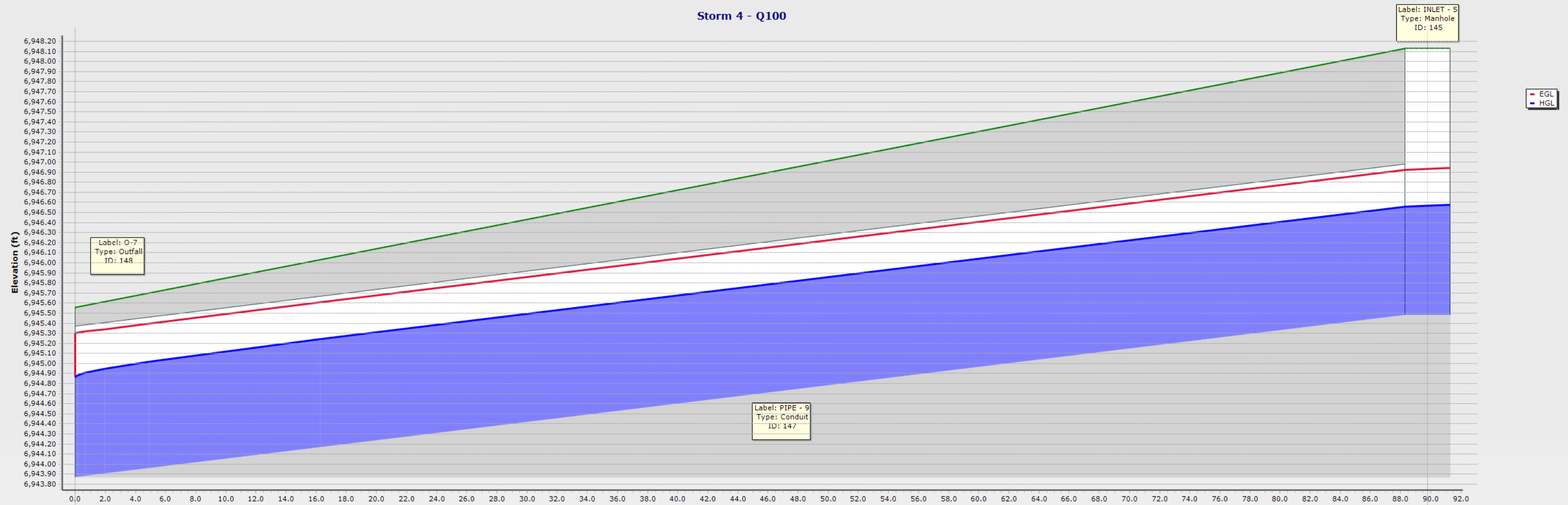
ID\Label	177 \ PIPE - 7	
Link Length (ft)	87.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	5.00	
Slope (ft/ft)	0.012	
ID\Label	176 \ INLET - 3	179 \ 0-11
Ground (ft)	6911.24	6909.09
Invert (ft)	6908.64	6907.59
Station (ft)	0.0	87.0

Storm 3 - Q100



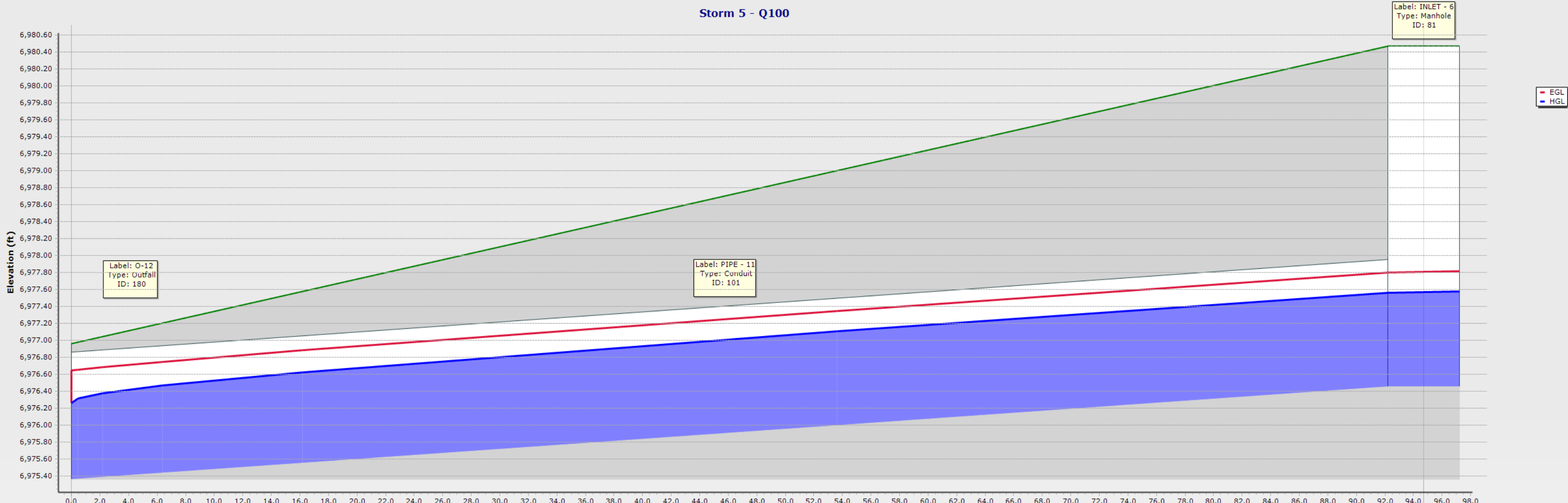
ID\Label	102 \ PIPE - 8	84 \ INLET - 4
Link Length (ft)	89.0	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	7.20	
Slope (ft/ft)	0.012	
ID\Label	124 \ FES - 5	84 \ INLET - 4
Ground (ft)	6926.52	6928.68
Invert (ft)	6925.02	6925.07
Station (ft)	0.0	89.0

Storm 4 - Q100



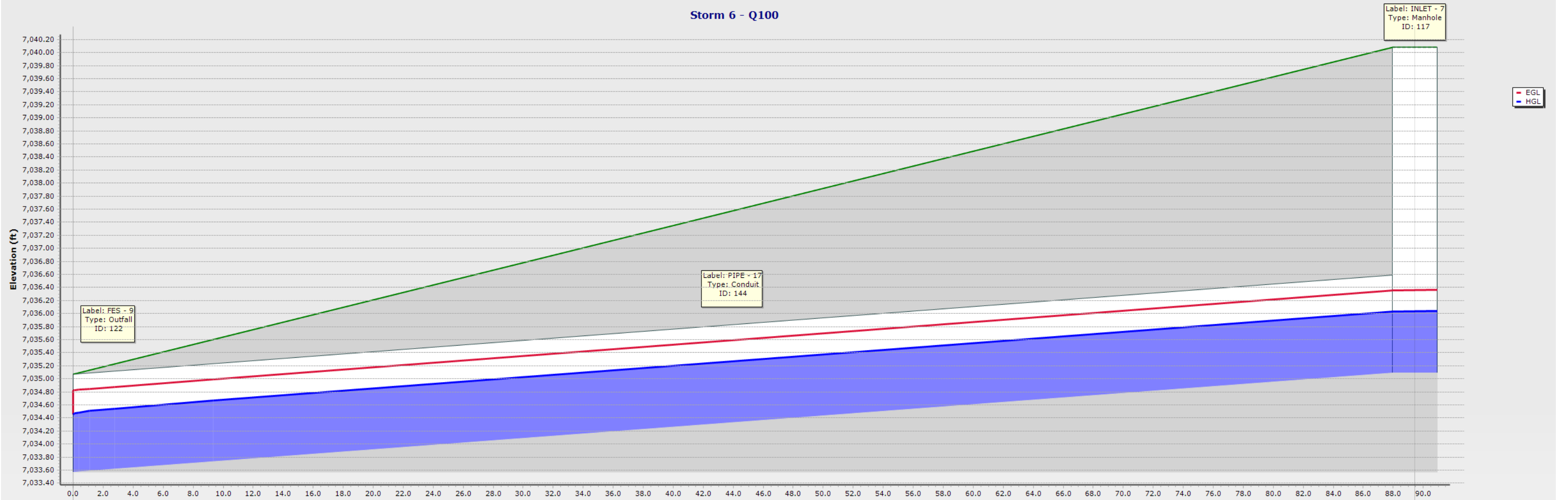
ID\Label	147 \ PIPE - 9	
Link Length (ft)	89.8	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	6.60	
Slope (ft/ft)	0.018	
ID\Label	148 \ O-7	145 \ INLET - 5
Ground (ft)	6945.56	6948.13
Invert (ft)	6943.87	6945.48
Station (ft)	0.0	89.8

Storm 5 - Q100



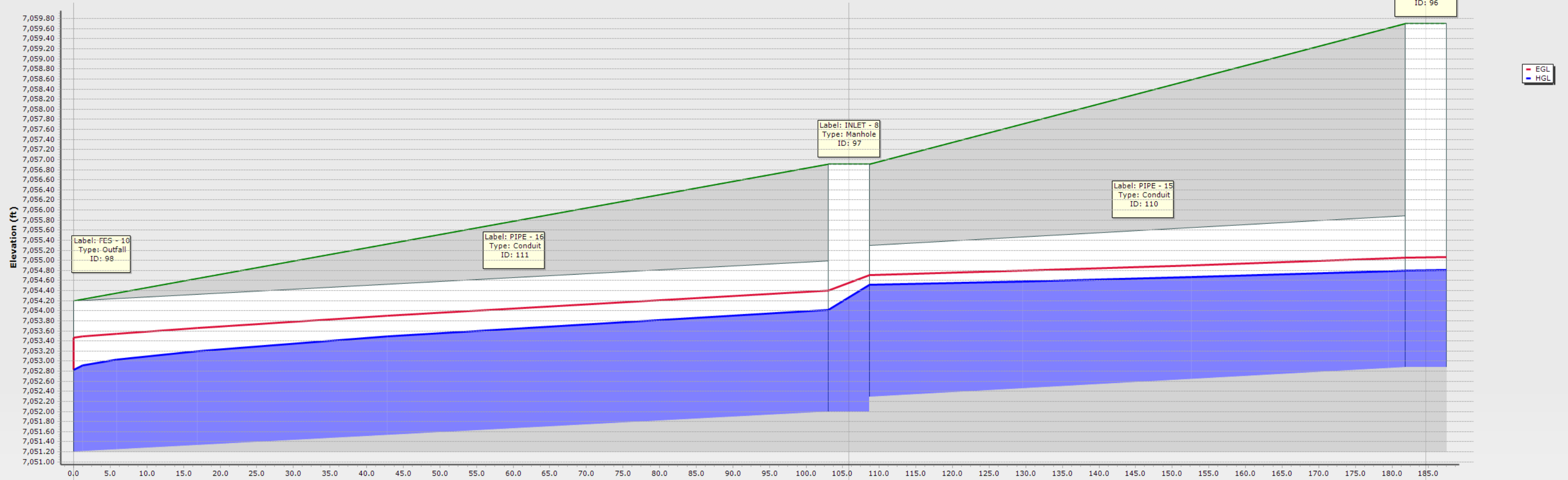
ID\Label	101 \ PIPE - 11	81 \ INLET - 6
Link Length (ft)	94.7	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	5.50	
Slope (ft/ft)	0.012	
ID\Label	180 \ O-12	81 \ INLET - 6
Ground (ft)	6976.96	6980.47
Invert (ft)	6975.36	6976.45
Station (ft)	0.0	94.7

Storm 6 - Q100



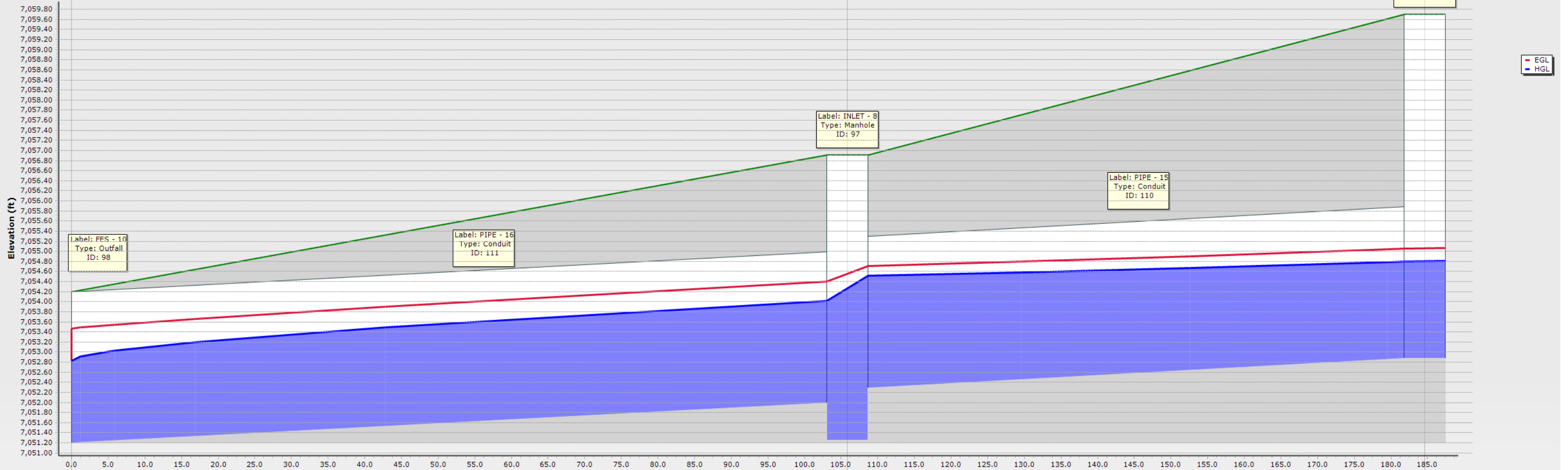
ID\Label	144 \ PIPE - 17	117 \ INLET - 7
Link Length (ft)	89.4	
Rise (in)\Material	18.0 \ CMP	
Flow (cfs)	5.30	
Slope (ft/ft)	0.017	
ID\Label	12 \ FES - 9	117 \ INLET - 7
Ground (ft)	7035.07	7040.08
Invert (ft)	7033.57	7035.09
Station (ft)	0.0	89.4

Storm 7 - Q100



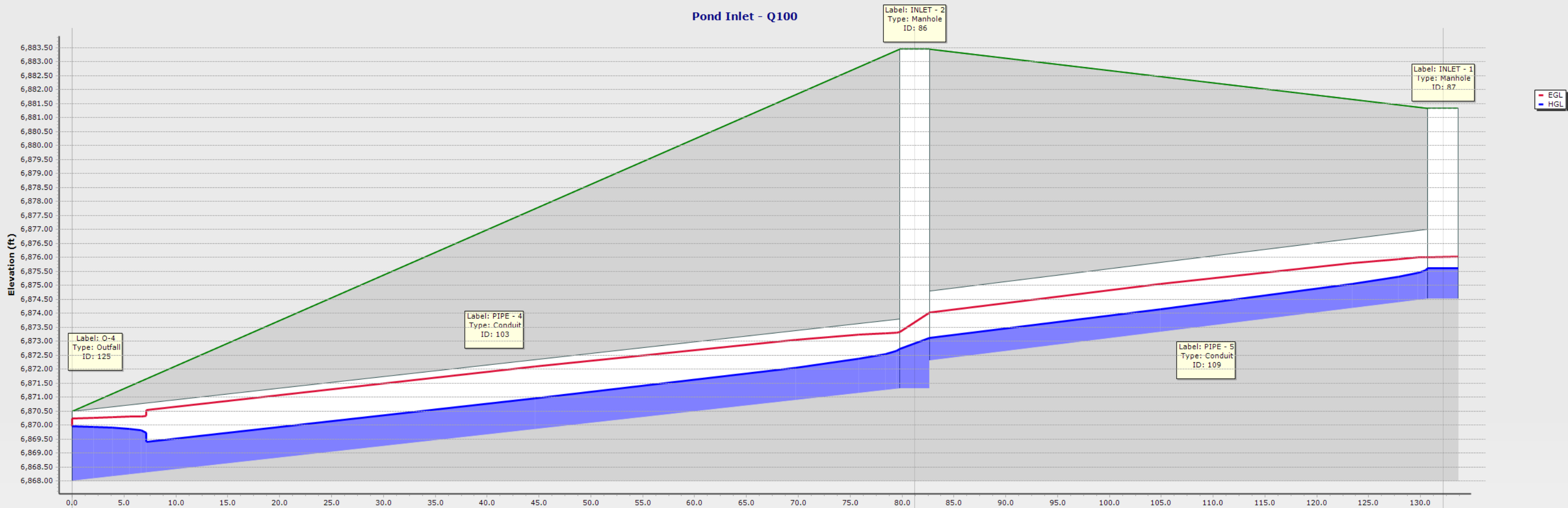
ID\Label	111 \ PIPE - 16	110 \ PIPE - 15	
Link Length (ft)	105.8	78.8	
Rise (in)\Material	36.0 \ CMP	36.0 \ CMP	
Flow (cfs)	25.20	19.50	
Slope (ft/ft)	0.007	0.007	
ID\Label	98 \ FES - 10	97 \ INLET - 8	96 \ INLET - 9
Ground (ft)	7054.20	7056.91	7059.70
Invert (ft)	7051.20	7051.99	7052.88
Station (ft)	0.0	105.8	184.6

Storm 7 - Q100



ID\Label	111 \ PIPE - 16	110 \ PIPE - 15	
Link Length (ft)	105.8	78.8	
Rise (in)\Material	36.0 \ CMP	36.0 \ CMP	
Flow (cfs)	25.20	19.50	
Slope (ft/ft)	0.007	0.007	
ID\Label	FES - 10	97 \ INLET - 8	96 \ INLET - 9
Ground (ft)	7054.20	7056.91	7059.70
Invert (ft)	7051.20	7051.25	7052.88
Station (ft)	0.0	105.8	184.6

Pond Inlet - Q100



ID/Label	Link Length (ft)	Rise (in)/Material	Flow (cfs)	Slope (ft/ft)
103 \ PIPE - 4	81.2	30.0 \ CMP	17.90	0.041
109 \ PIPE - 5	50.9	30.0 \ CMP	10.60	0.043

ID/Label	Ground (ft)	Invert (ft)	Station (ft)
125 \ O-4	6870.50	6868.00	0.0
86 \ INLET - 2	6883.45	6871.30	81.2
87 \ INLET - 1	6881.33	6874.51	132.2

Pond-Outfall - Q100



ID\Label	99 \ PIPE - 3	
Link Length (ft)	71.3	
Rise (in)\Material	18.0 \ Concrete	
Flow (cfs)	15.20	
Slope (ft/ft)	0.033	
ID\Label	88 \ FES - 3	121 \ MH-5
Ground (ft)	6863.50	6870.00
Invert (ft)	6862.00	6864.35
Station (ft)	0.0	71.3

	Label	Velocity (ft/s)	Start Node	Invert (Start) (ft)	Hydraulic Grade Line (In) (ft)	Invert (Stop) (ft)	Hydraulic Grade Line (Out) (ft)	Stop Node	Length (User Defined) (ft)	Slope (Calculated) (ft/ft)	Manning's n	Flow (cfs)	Capacity (Design) (cfs)	Depth (Normal) / Rise (%)	Diameter (in)	Elevation Ground (Start) (ft)	Elevation Ground (Stop) (ft)
99: PIPE - 3	PIPE - 3	8.60	MH-5	6,864.35	6,868.55	6,862.00	6,863.41	FES - 3	71.3	0.033	0.024	15.20	10.33	(N/A)	18.0	6,870.00	6,863.50
101: PIPE - 11	PIPE - 11	3.91	INLET - 6	6,976.45	6,977.56	6,975.36	6,976.26	O-12	94.7	0.012	0.024	5.50	6.10	74.2	18.0	6,980.47	6,976.96
102: PIPE - 8	PIPE - 8	4.07	INLET - 4	6,926.07	6,927.63	6,925.02	6,926.06	FES - 5	89.0	0.012	0.024	7.20	6.18	(N/A)	18.0	6,928.68	6,926.52
103: PIPE - 4	PIPE - 4	8.61	INLET - 2	6,871.30	6,872.73	6,868.00	6,869.95	O-4	81.2	0.041	0.024	17.90	44.78	44.0	30.0	6,883.45	6,870.50
104: PIPE - 2	PIPE - 2	5.69	FES - 1	6,865.78	6,866.73	6,862.18	6,863.06	FES - 2	129.9	0.028	0.024	6.10	9.47	58.4	18.0	6,868.78	6,865.18
109: PIPE - 5	PIPE - 5	7.65	INLET - 1	6,874.51	6,875.60	6,872.30	6,873.11	INLET - 2	50.9	0.043	0.024	10.60	46.27	32.6	30.0	6,881.33	6,883.45
110: PIPE - 15	PIPE - 15	4.66	INLET - 9	7,052.88	7,054.79	7,052.29	7,054.52	INLET - 8	78.8	0.007	0.024	19.50	31.27	57.2	36.0	7,059.70	7,056.91
111: PIPE - 16	PIPE - 16	4.91	INLET - 8	7,051.99	7,054.01	7,051.20	7,052.82	FES - 10	105.8	0.007	0.024	25.20	31.21	68.1	36.0	7,056.91	7,054.20
144: PIPE - 17	PIPE - 17	4.56	INLET - 7	7,035.09	7,036.03	7,033.57	7,034.46	FES - 9	89.4	0.017	0.024	5.30	7.42	62.5	18.0	7,040.08	7,035.07
147: PIPE - 9	PIPE - 9	4.85	INLET - 5	6,945.48	6,946.56	6,943.87	6,944.86	O-7	89.8	0.018	0.024	6.60	7.62	71.9	18.0	6,948.13	6,945.56
177: PIPE - 7	PIPE - 7	3.93	INLET - 3	6,908.64	6,909.65	6,907.59	6,908.45	O-11	87.0	0.012	0.024	5.00	6.25	67.6	18.0	6,911.24	6,909.09

Figure 10- Q100 CONDUIT SUMMARY

	ID	Label	Flow (Known) (cfs)	Elevation (Ground) (ft)	Elevation (Rim) (ft)	Depth (Out) (ft)	Hydraulic Grade Line (In) (ft)	Hydraulic Grade Line (Out) (ft)	Headloss Method	Headloss Coefficient (Standard)	Flow (Total Out) (cfs)
81: INLET - 6	81	INLET - 6	5.50	6,980.47	6,980.47	1.11	6,977.57	6,977.56	Standard	0.050	5.50
84: INLET - 4	84	INLET - 4	7.20	6,928.68	6,928.68	1.56	6,927.64	6,927.63	Standard	0.050	7.20
86: INLET - 2	86	INLET - 2	17.90	6,883.45	6,883.45	1.43	6,873.11	6,872.73	Standard	0.640	17.90
87: INLET - 1	87	INLET - 1	10.60	6,881.33	6,881.33	1.09	6,875.62	6,875.60	Standard	0.050	10.60
96: INLET - 9	96	INLET - 9	19.50	7,059.70	7,059.70	1.91	7,054.81	7,054.79	Standard	0.050	19.50
97: INLET - 8	97	INLET - 8	25.20	7,056.91	7,056.91	2.02	7,054.52	7,054.01	Standard	1.320	25.20
117: INLET - 7	117	INLET - 7	5.30	7,040.08	7,040.08	0.94	7,036.04	7,036.03	Standard	0.050	5.30
120: FES - 1	120	FES - 1	6.10	6,868.78	6,868.78	0.95	6,866.75	6,866.73	Standard	0.050	6.10
121: MH-5	121	MH-5	15.20	6,870.00	6,870.00	4.20	6,868.61	6,868.55	Standard	0.050	15.20
145: INLET - 5	145	INLET - 5	6.60	6,948.13	6,948.13	1.08	6,946.58	6,946.56	Standard	0.050	6.60
176: INLET - 3	176	INLET - 3	5.00	6,911.24	6,911.24	1.01	6,909.67	6,909.65	Standard	0.050	5.00

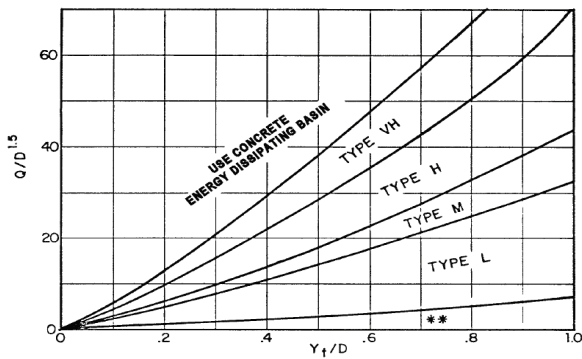
Figure 11- Q100 NODE SUMMARY

	ID	Label	Elevation (Ground) (ft)	Set Rim to Ground Elevation?	Elevation (Invert) (ft)	Boundary Condition Type	Hydraulic Grade (ft)	Flow (Total Out) (cfs)
88: FES - 3	88	FES - 3	6,863.50	<input checked="" type="checkbox"/>	6,862.00	Free Outfall	6,863.41	15.20
98: FES - 10	98	FES - 10	7,054.20	<input checked="" type="checkbox"/>	7,051.20	Free Outfall	7,052.82	25.20
122: FES - 9	122	FES - 9	7,035.07	<input checked="" type="checkbox"/>	7,033.57	Free Outfall	7,034.46	5.30
124: FES - 5	124	FES - 5	6,926.52	<input checked="" type="checkbox"/>	6,925.02	Free Outfall	6,926.06	7.20
125: O-4	125	O-4	6,870.50	<input checked="" type="checkbox"/>	6,868.00	User Defined Tailwater	6,869.95	17.90
126: FES - 2	126	FES - 2	6,865.18	<input checked="" type="checkbox"/>	6,862.18	Free Outfall	6,863.06	6.10
148: O-7	148	O-7	6,945.56	<input checked="" type="checkbox"/>	6,944.06	Free Outfall	6,945.05	6.60
179: O-11	179	O-11	6,909.09	<input checked="" type="checkbox"/>	6,907.59	Free Outfall	6,908.45	5.00
180: O-12	180	O-12	6,976.96	<input checked="" type="checkbox"/>	6,975.46	Free Outfall	6,976.36	5.50

Figure 12- Q100 OUTFALL SUMMARY

THE TAILWATER CONDITION HAS BEEN DEINED AS THE POND WSE DURING THE CORRESPONDING STORM EVENT FOR OUTFALLS THAT DISCHARGE INTO POND 1.

	DP12b		DP13		DP14		DP15		DP16		DP17		DP19	
Pipe Size (D)	36	Inches	18	Inches	18	Inches	18	Inches	18	Inches	18	Inches	18	Inches
Q	25.2	cfs	5.3	cfs	9.2	cfs	6.7	cfs	7.2	cfs	7.9	cfs	6.1	cfs
L	9	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet
W	9	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet
D	0	Feet	0	Feet	0	Feet	0	Feet	0	Feet	0	Feet	0	Feet
d ₅₀	0.12	Feet	0.07	Feet	0.12	Feet	0.09	Feet	0.10	Feet	0.11	Feet	0.08	Feet
	1.44	Inches	0.86	Inches	1.49	Inches	1.08	Inches	1.17	Inches	1.28	Inches	0.99	Inches
Depth of Flow	2.82	Feet	1.41	Feet	1.4	Feet	1.4	Feet	1.41	Feet	1.4	Feet	1.4	Feet
Q/D ^{1.5}	4.85		2.88		5.01		3.65		3.92		4.30		3.32	
Y _t /D	0.940		0.940		0.940		0.94		0.940		0.940		0.94	
Rip Rap	Type L for 3 x Pipe Dia Downstream		Type L for 3 x Pipe Dia Downstream		Type L for 3 x Pipe Dia Downstream		Type L for 3 x Pipe Dia Downstream		Type L for 3 x Pipe Dia Downstream		Type L for 3 x Pipe Dia Downstream		Type L for 3 x Pipe Dia Downstream	
Length of Rock	9	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet
Width of Rock	9.0	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet	4.5	Feet



Use D₀ instead of D whenever flow is supercritical in the barrel.
 ** Use Type L for a distance of 3D downstream.

Figure 9-38. Riprap erosion protection at circular conduit outlet (valid for Q/D^{2.5} ≤ 6.0)

CLASSIFICATION AND GRADATION OF ORDINARY RIP RAP			
Rip Rap Designation by Weight	% Smaller Than Given Size (inches)	Intermediate Rock Dimension	d ₅₀ * (inches)
Type VL	70 - 100	12	6**
	50 - 70	9	
	35 - 50	6	
Type L	70 - 100	15	9**
	50 - 70	12	
	35 - 50	9	
Type M	70 - 100	21	12
	50 - 70	18	
	35 - 50	12	
Type H	70 - 100	30	18
	50 - 70	24	
	35 - 50	18	
Type VH	70 - 100	42	24
	50 - 70	33	
	35 - 50	24	
	2 - 10	9	

* d₅₀ = Mean particle size
 ** Bury types VL and L with native top soil and revegetate to protect from vandalism.

3.2.3 Rock Sizing for Riprap Apron and Low Tailwater Basin

Scour resulting from highly turbulent, rapidly decelerating flow is a common problem at conduit outlets. The following section summarizes the method for sizing riprap protection for both riprap aprons (Section 3.2.1) and low tailwater basins (Section 3.2.2).

Use Figure 9-38 to determine the required rock size for circular conduits and Figure 9-39 for rectangular conduits. Figure 9-38 is valid for Q/D^{2.5} of 6.0 or less and Figure 9-39 is valid for Q/WH^{2.5} of 8.0 or less. The parameters in these two figures are:

1. Q/D^{1.5} or Q/WH^{0.5} in which Q is the design discharge in cfs, D_c is the diameter of a circular conduit in feet, and W and H are the width and height of a rectangular conduit in feet.
2. Y_t/D_c or Y_t/H in which Y_t is the tailwater depth in feet, D_c is the diameter of a circular conduit in feet, and H is the height of a rectangular conduit in feet. In cases where Y_t is unknown or a hydraulic jump is suspected downstream of the outlet, use Y_t/D_c = Y_t/H = 0.40 when using Figures 9-38 and 9-39.
3. The riprap size requirements in Figures 9-38 and 9-39 are based on the non-dimensional parametric Equations 9-16 and 9-17 (Steven, Simons, and Watts 1971 and Smith 1975).

Circular culvert:

$$d_{50} = \frac{0.023Q}{Y_t^{1.2} D_c^{0.3}} \quad \text{Equation 9-16}$$

Rectangular culvert:

$$d_{50} = \frac{0.014H^{0.5}Q}{Y_t W} \quad \text{Equation 9-17}$$

Channel Report

DP-12b Outfall

Trapezoidal

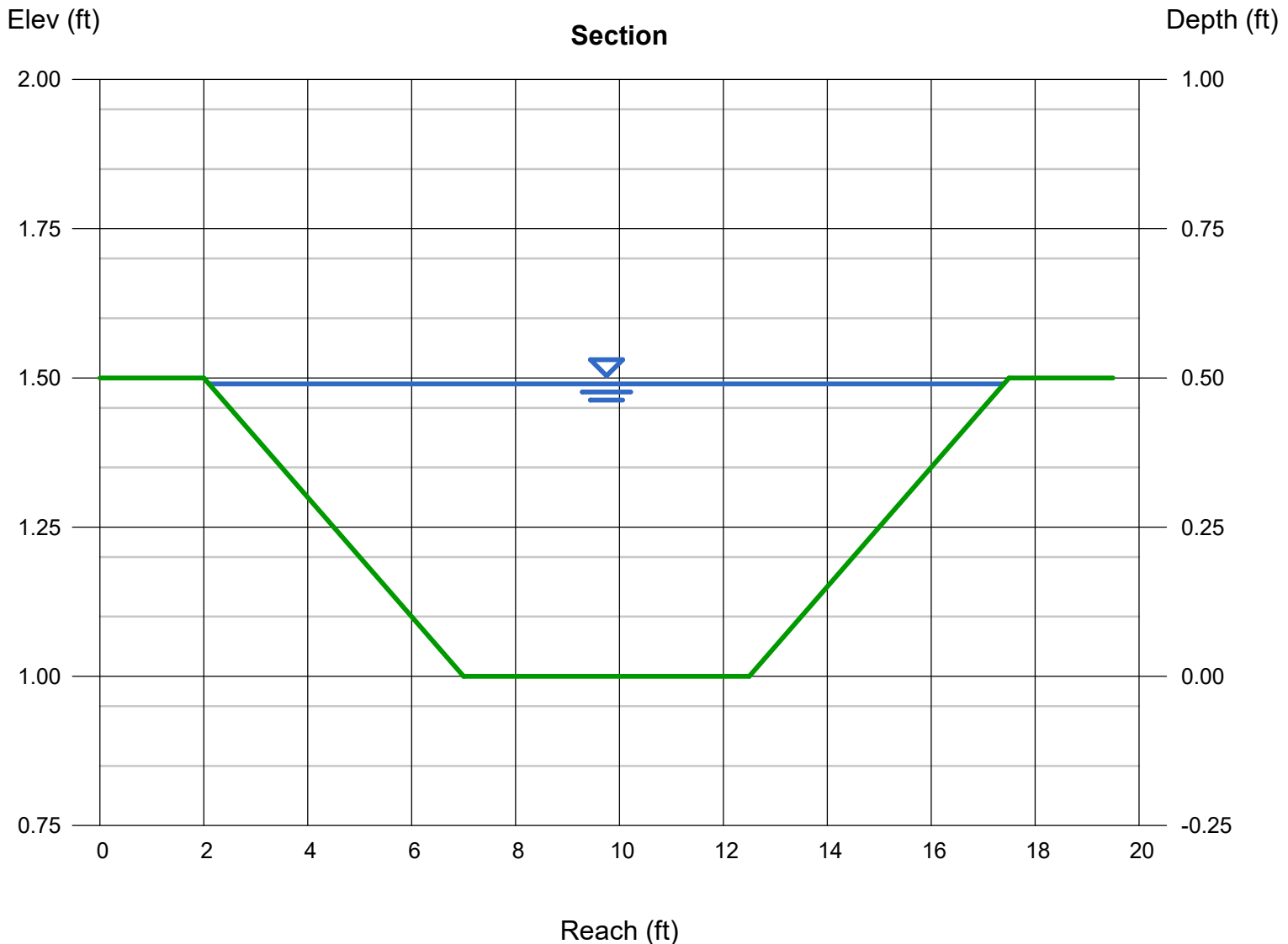
Bottom Width (ft) = 5.50
Side Slopes (z:1) = 10.00, 10.00
Total Depth (ft) = 0.50
Invert Elev (ft) = 1.00
Slope (%) = 8.20
N-Value = 0.040

Highlighted

Depth (ft) = 0.49
Q (cfs) = 25.20
Area (sqft) = 5.10
Velocity (ft/s) = 4.95
Wetted Perim (ft) = 15.35
Crit Depth, Yc (ft) = 0.50
Top Width (ft) = 15.30
EGL (ft) = 0.87

Calculations

Compute by: Known Q
Known Q (cfs) = 25.20



Channel Report

DP-13 Outfall

Trapezoidal

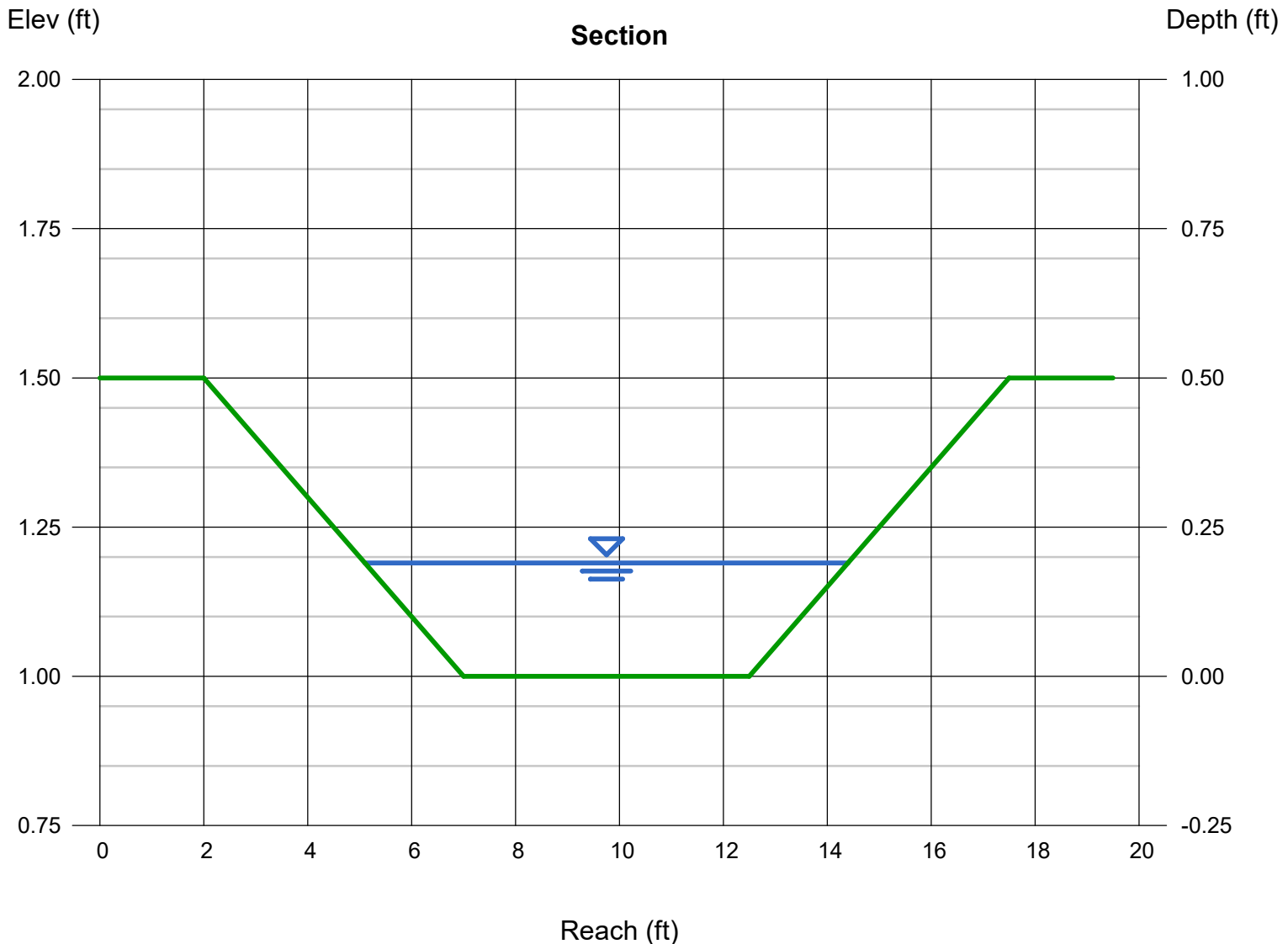
Bottom Width (ft) = 5.50
Side Slopes (z:1) = 10.00, 10.00
Total Depth (ft) = 0.50
Invert Elev (ft) = 1.00
Slope (%) = 9.50
N-Value = 0.032

Highlighted

Depth (ft) = 0.19
Q (cfs) = 5.300
Area (sqft) = 1.41
Velocity (ft/s) = 3.77
Wetted Perim (ft) = 9.32
Crit Depth, Yc (ft) = 0.27
Top Width (ft) = 9.30
EGL (ft) = 0.41

Calculations

Compute by: Known Q
Known Q (cfs) = 5.30



Channel Report

DP-14 Outfall

Trapezoidal

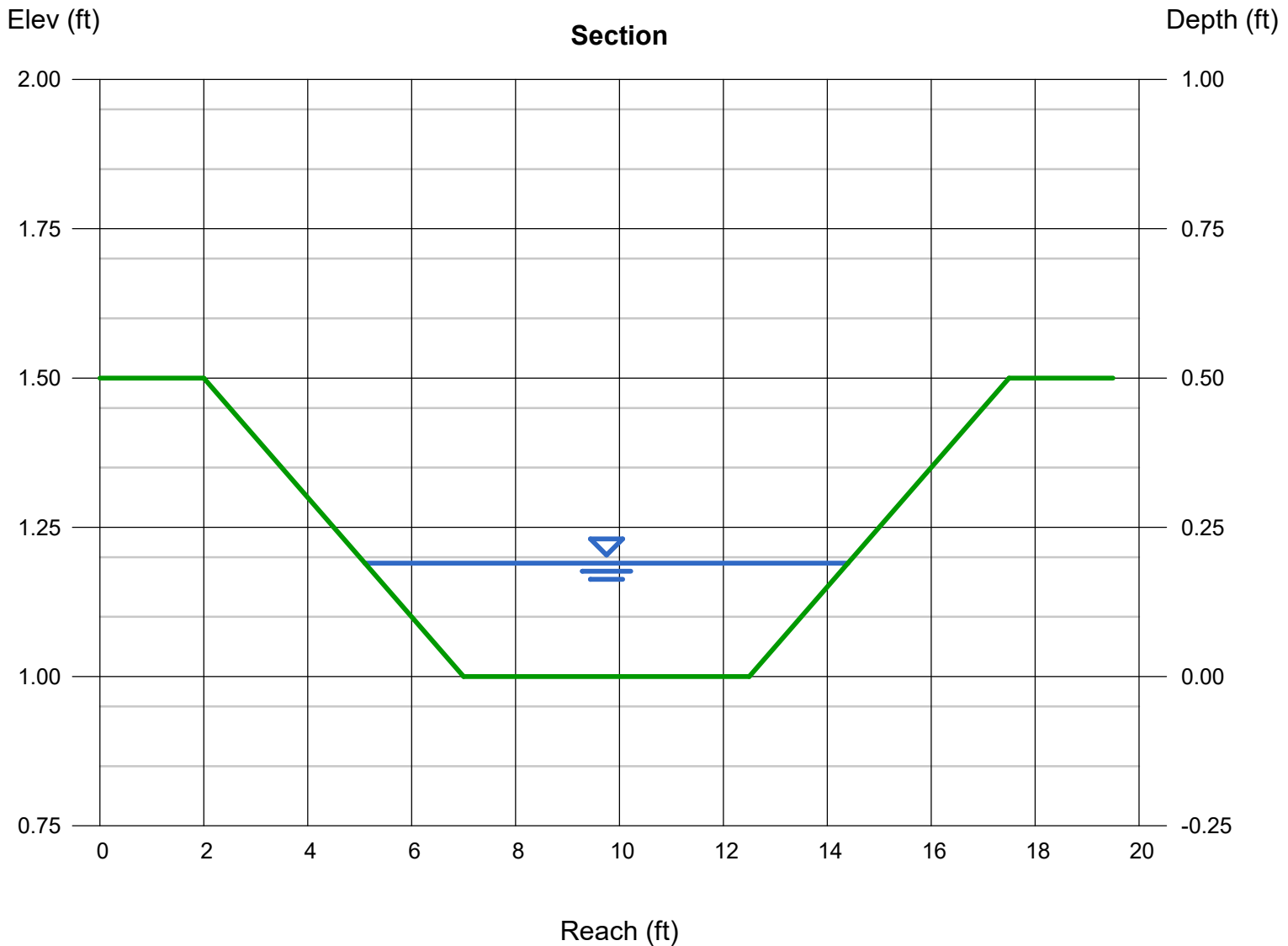
Bottom Width (ft) = 5.50
Side Slopes (z:1) = 10.00, 10.00
Total Depth (ft) = 0.50
Invert Elev (ft) = 1.00
Slope (%) = 9.50
N-Value = 0.032

Highlighted

Depth (ft) = 0.19
Q (cfs) = 5.500
Area (sqft) = 1.41
Velocity (ft/s) = 3.91
Wetted Perim (ft) = 9.32
Crit Depth, Yc (ft) = 0.27
Top Width (ft) = 9.30
EGL (ft) = 0.43

Calculations

Compute by: Known Q
Known Q (cfs) = 5.50



Channel Report

DP-15 Outfall

Trapezoidal

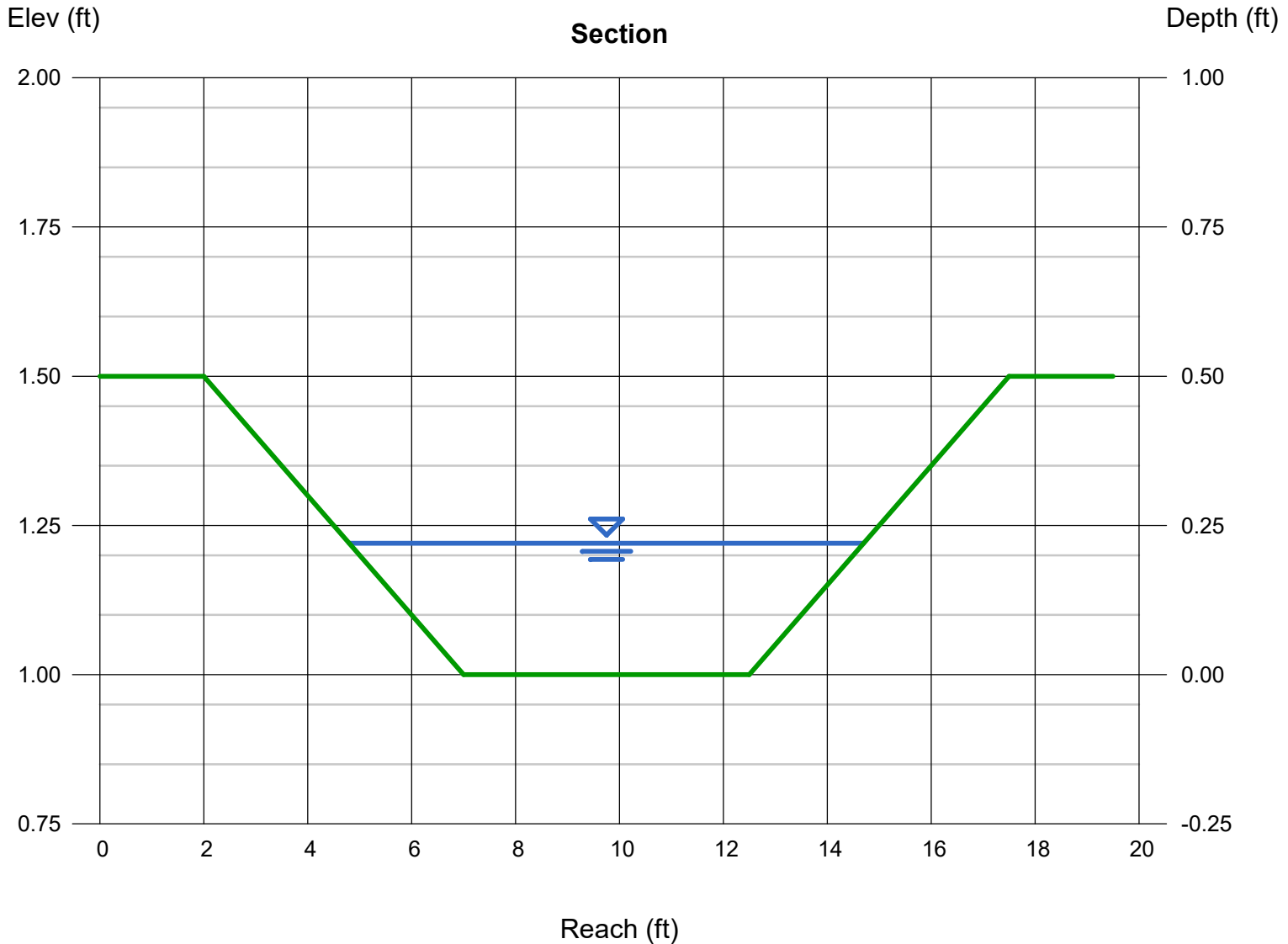
Bottom Width (ft) = 5.50
Side Slopes (z:1) = 10.00, 10.00
Total Depth (ft) = 0.50
Invert Elev (ft) = 1.00
Slope (%) = 8.50
N-Value = 0.032

Highlighted

Depth (ft) = 0.22
Q (cfs) = 6.600
Area (sqft) = 1.69
Velocity (ft/s) = 3.90
Wetted Perim (ft) = 9.92
Crit Depth, Yc (ft) = 0.30
Top Width (ft) = 9.90
EGL (ft) = 0.46

Calculations

Compute by: Known Q
Known Q (cfs) = 6.60



Channel Report

DP-16 Outfall

Trapezoidal

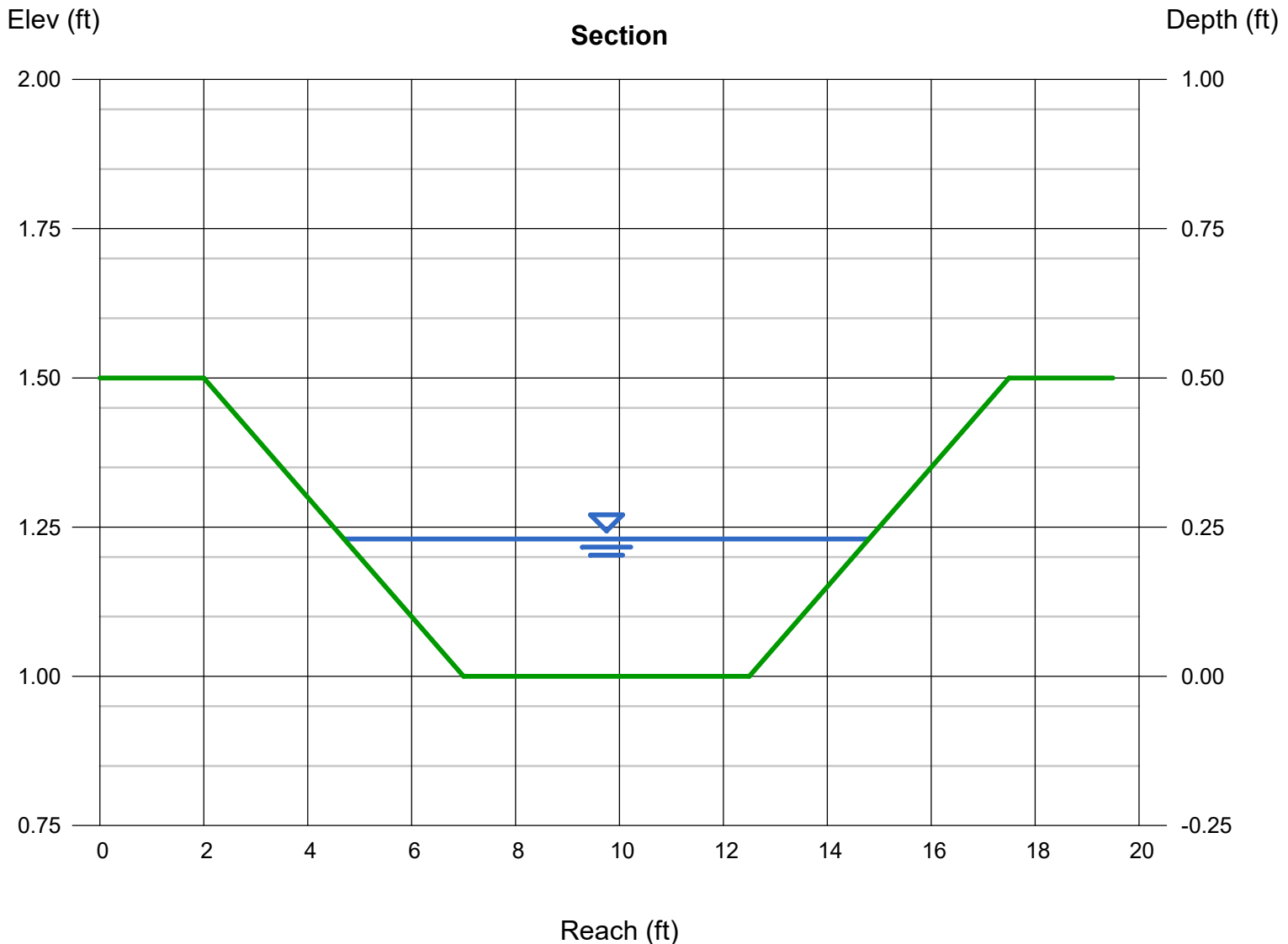
Bottom Width (ft) = 5.50
Side Slopes (z:1) = 10.00, 10.00
Total Depth (ft) = 0.50
Invert Elev (ft) = 1.00
Slope (%) = 8.70
N-Value = 0.032

Highlighted

Depth (ft) = 0.23
Q (cfs) = 7.200
Area (sqft) = 1.79
Velocity (ft/s) = 4.01
Wetted Perim (ft) = 10.12
Crit Depth, Yc (ft) = 0.31
Top Width (ft) = 10.10
EGL (ft) = 0.48

Calculations

Compute by: Known Q
Known Q (cfs) = 7.20



Channel Report

DP-17 Outfall

Trapezoidal

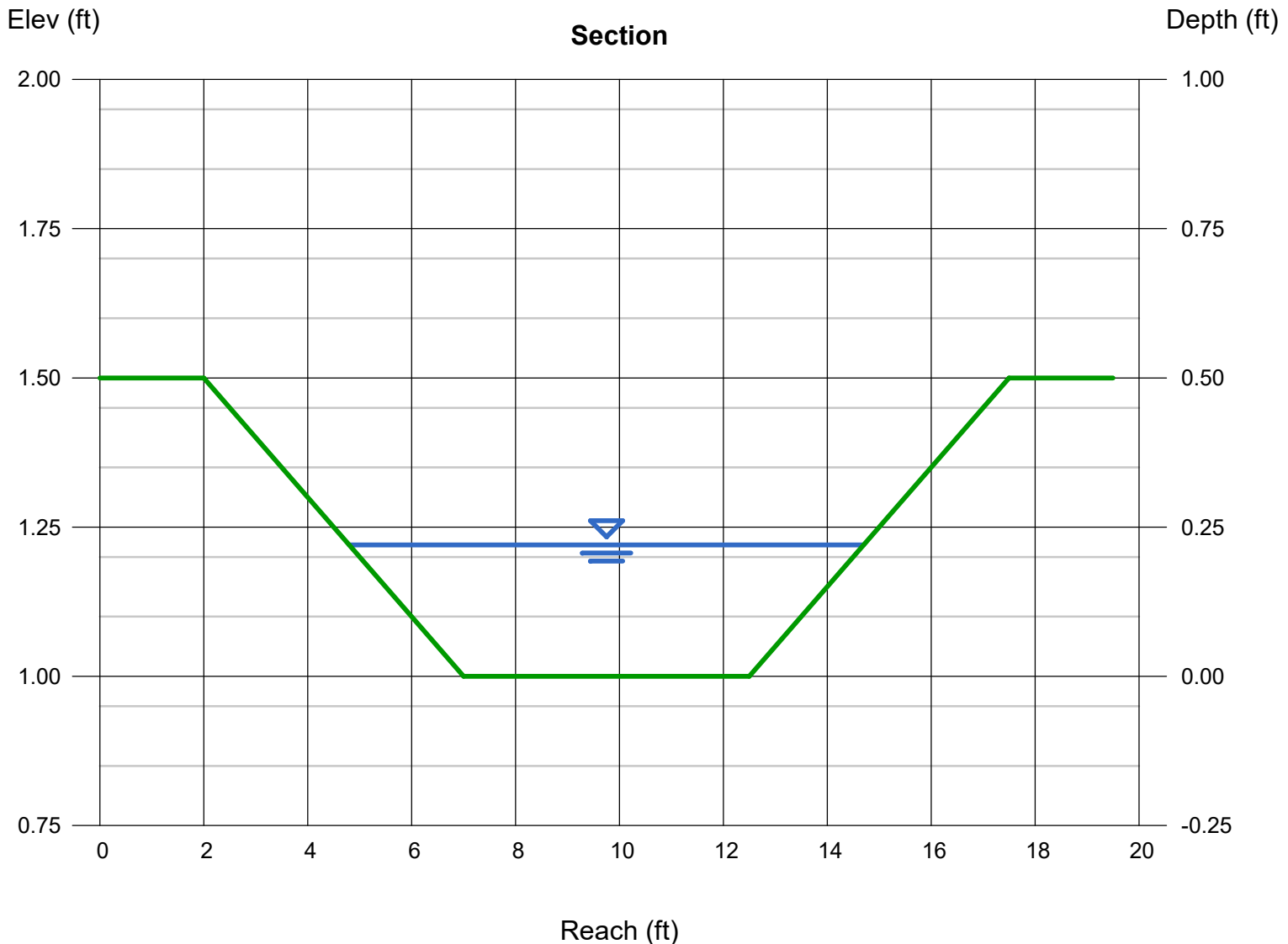
Bottom Width (ft) = 5.50
Side Slopes (z:1) = 10.00, 10.00
Total Depth (ft) = 0.50
Invert Elev (ft) = 1.00
Slope (%) = 4.80
N-Value = 0.032

Highlighted

Depth (ft) = 0.22
Q (cfs) = 5.000
Area (sqft) = 1.69
Velocity (ft/s) = 2.95
Wetted Perim (ft) = 9.92
Crit Depth, Yc (ft) = 0.26
Top Width (ft) = 9.90
EGL (ft) = 0.36

Calculations

Compute by: Known Q
Known Q (cfs) = 5.00



Hydraulic Analysis Report

Project Data

Project Title:

Designer:

Project Date: Wednesday, November 22, 2023

Project Units: U.S. Customary Units

Notes:

Channel Analysis: RIPRAP RUNDOWN @ 5.39%

Notes:

Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 10.0000 ft/ft

Side Slope 2 (Z2): 10.0000 ft/ft

Channel Width: 10.0000 ft

Longitudinal Slope: 0.0539 ft/ft

Manning's n: 0.0897

Flow: 49.7000 cfs

Result Parameters

Depth: 0.9226 ft

Area of Flow: 17.7388 ft²

Wetted Perimeter: 28.5446 ft

Hydraulic Radius: 0.6214 ft

Average Velocity: 2.8018 ft/s

Top Width: 28.4526 ft

Froude Number: 0.6253

Critical Depth: 0.7172 ft

Critical Velocity: 4.0352 ft/s

Critical Slope: 0.1476 ft/ft

Critical Top Width: 24.34 ft

Calculated Max Shear Stress: 3.1031 lb/ft²

Calculated Avg Shear Stress: 2.0901 lb/ft²

Channel Lining Analysis: Channel Lining Design Analysis @5.39%

Notes:

Lining Input Parameters

Channel Lining Type: Riprap, Cobble, or Gravel

D50: 0.75 ft

Riprap Specific Weight: 165 lb/ft³

Water Specific Weight: 62.4 lb/ft³

Riprap Shape is Angular

Safety Factor: 1

Calculated Safety Factor: 1.11886

Lining Results

Angle of Repose: 41.7 degrees

Relative Flow Depth: 0.831266

Manning's n method: Bathurst

Manning's n: 0.0896539

Channel Bottom Shear Results

V*: 1.26542

Reynold's Number: 77984.2

Shield's Parameter: 0.0714523

shear stress on channel bottom: 3.10314 lb/ft²

Permissible shear stress for channel bottom: 5.36984 lb/ft²

channel bottom is stable

Stable D50: 0.484926 ft

Channel Side Shear Results

K1: 1

K2: 1

Kb: 0

shear stress on side of channel: 3.10314 lb/ft²

Permissible shear stress for side of channel: 5.36984 lb/ft²

Stable Side D50: 0.484926 lb/ft²

side of channel is stable

Channel Lining Stability Results

the channel is stable

Channel Summary

Name of Selected Channel: RIPRAP RUNDOWN @ 5.39%

Channel Analysis: RIPRAP RUNDOWN @ 11.3%

Notes:

Input Parameters

Channel Type: Trapezoidal
Side Slope 1 (Z1): 10.0000 ft/ft
Side Slope 2 (Z2): 10.0000 ft/ft
Channel Width: 10.0000 ft
Longitudinal Slope: 0.1130 ft/ft
Manning's n: 0.0534
Flow: 49.7000 cfs

Result Parameters

Depth: 0.5891 ft
Area of Flow: 9.3606 ft²
Wetted Perimeter: 21.8400 ft
Hydraulic Radius: 0.4286 ft
Average Velocity: 5.3095 ft/s
Top Width: 21.7813 ft
Froude Number: 1.4273
Critical Depth: 0.7172 ft
Critical Velocity: 4.0356 ft/s
Critical Slope: 0.0524 ft/ft
Critical Top Width: 24.34 ft
Calculated Max Shear Stress: 4.1536 lb/ft²
Calculated Avg Shear Stress: 3.0221 lb/ft²

Channel Lining Analysis: Channel Lining Design Analysis @11.3%

Notes:

Lining Input Parameters

Channel Lining Type: Riprap, Cobble, or Gravel

D50: 0.75 ft

Riprap Specific Weight: 165 lb/ft³

Water Specific Weight: 62.4 lb/ft³

Riprap Shape is Angular

Safety Factor: 1

Calculated Safety Factor: 1.1571

Lining Results

Angle of Repose: 41.7 degrees

Relative Flow Depth: 0.573005

Manning's n method: Bathurst

Manning's n: 0.0534294

Channel Bottom Shear Results

V*: 1.46402

Reynold's Number: 90223.3

Shield's Parameter: 0.0793312

shear stress on channel bottom: 4.1536 lb/ft²

Permissible shear stress for channel bottom: 5.89025 lb/ft²

channel bottom is stable

Stable D50: 0.611961 ft

Channel Side Shear Results

K1: 1

K2: 1

Kb: 0

shear stress on side of channel: 4.1536 lb/ft²

Permissible shear stress for side of channel: 5.89025 lb/ft²

Stable Side D50: 0.611961 lb/ft²

side of channel is stable

Channel Lining Stability Results

the channel is stable

Channel Summary

Name of Selected Channel: RIPRAP RUNDOWN @ 11.3%

Channel Analysis: RIPRAP RUNDOWN @ 8.0%

Notes:

Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 10.0000 ft/ft

Side Slope 2 (Z2): 10.0000 ft/ft

Channel Width: 10.0000 ft

Longitudinal Slope: 0.0800 ft/ft

Manning's n: 0.0713

Flow: 49.7000 cfs

Result Parameters

Depth: 0.7458 ft

Area of Flow: 13.0211 ft²

Wetted Perimeter: 24.9912 ft

Hydraulic Radius: 0.5210 ft

Average Velocity: 3.8169 ft/s

Top Width: 24.9168 ft

Froude Number: 0.9305

Critical Depth: 0.7174 ft

Critical Velocity: 4.0337 ft/s

Critical Slope: 0.0931 ft/ft

Critical Top Width: 24.35 ft

Calculated Max Shear Stress: 3.7232 lb/ft²

Calculated Avg Shear Stress: 2.6010 lb/ft²

Channel Lining Analysis: Channel Lining Design Analysis @8.0%

Notes:

Lining Input Parameters

Channel Lining Type: Riprap, Cobble, or Gravel

D50: 0.75 ft

Riprap Specific Weight: 165 lb/ft³

Water Specific Weight: 62.4 lb/ft³

Riprap Shape is Angular

Safety Factor: 1

Calculated Safety Factor: 1.1421

Lining Results

Angle of Repose: 41.7 degrees

Relative Flow Depth: 0.69678

Manning's n method: Bathurst

Manning's n: 0.0712516

Channel Bottom Shear Results

V*: 1.3861

Reynold's Number: 85421.2

Shield's Parameter: 0.0762399

shear stress on channel bottom: 3.72322 lb/ft²

Permissible shear stress for channel bottom: 5.6927 lb/ft²

channel bottom is stable

Stable D50: 0.560228 ft

Channel Side Shear Results

K1: 1

K2: 1

Kb: 0

shear stress on side of channel: 3.72322 lb/ft²

Permissible shear stress for side of channel: 5.6927 lb/ft²

Stable Side D50: 0.560228 lb/ft²

side of channel is stable

Channel Lining Stability Results

the channel is stable

Channel Summary

Name of Selected Channel: RIPRAP RUNDOWN @ 8.0%

Hydraulic Analysis Report

Project Data

Project Title: Hay Creek

Designer:

Project Date: Thursday, January 12, 2023

Project Units: U.S. Customary Units

Notes:

Channel Analysis: Discharge into Bypass Swale

Notes:

Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 3.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Channel Width: 5.0000 ft

Longitudinal Slope: 0.1050 ft/ft

Manning's n: 0.0887

Flow: 43.0000 cfs

Result Parameters

Depth: 1.1218 ft

Area of Flow: 9.3839 ft²

Wetted Perimeter: 12.0946 ft

Hydraulic Radius: 0.7759 ft

Average Velocity: 4.5823 ft/s

Top Width: 11.7306 ft

Froude Number: 0.9029

Critical Depth: 1.0603 ft

Critical Velocity: 4.9575 ft/s

Critical Slope: 0.1305 ft/ft

Critical Top Width: 11.36 ft

Calculated Max Shear Stress: 7.3498 lb/ft²

Calculated Avg Shear Stress: 5.0835 lb/ft²

Channel Lining Analysis: CLDA 10.5%

Notes:

Lining Input Parameters

Channel Lining Type: Riprap, Cobble, or Gravel

D50: 2 ft

Riprap Specific Weight: 165 lb/ft³

Water Specific Weight: 62.4 lb/ft³

Riprap Shape is Angular

Safety Factor: 1

Calculated Safety Factor: 1.50016

Lining Results

Angle of Repose: 42.1 degrees

Relative Flow Depth: 0.399975

Manning's n method: Bathurst

Manning's n: 0.088685

Channel Bottom Shear Results

V*: 1.94748

Reynold's Number: 320046

Shield's Parameter: 0.15

shear stress on channel bottom: 7.34978 lb/ft²

Permissible shear stress for channel bottom: 22.9176 lb/ft²

channel bottom is stable

Stable D50: 0.962215 ft

Channel Side Shear Results

K1: 0.868

K2: 1

Kb: 2

shear stress on side of channel: 7.34978 lb/ft²

Permissible shear stress for side of channel: 22.9176 lb/ft²

Stable Side D50: 0.835203 lb/ft²

side of channel is stable

Channel Bend Shear Results

Curvature Radius: 18.5 ft

No further correction will occur once $R/T > 10$

shear stress on bottom of channel in bend: 14.6996 lb/ft²

bottom of bend of the channel is stable

Length of Protection beyond PT: 5.03177 ft

Additional Freeboard required because of Superelevation: 0.413825 ft

Channel Bend Side Shear Results

shear stress on side of channel in bend: 12.7592 lb/ft²

The side of the bend of the channel is stable

Channel Lining Stability Results

the channel is stable

Channel Summary

Name of Selected Channel: Discharge into Bypass Swale

***SWALE
CALCULATIONS***

Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Friday, Nov 17 2023

DP-13 Q100

Trapezoidal

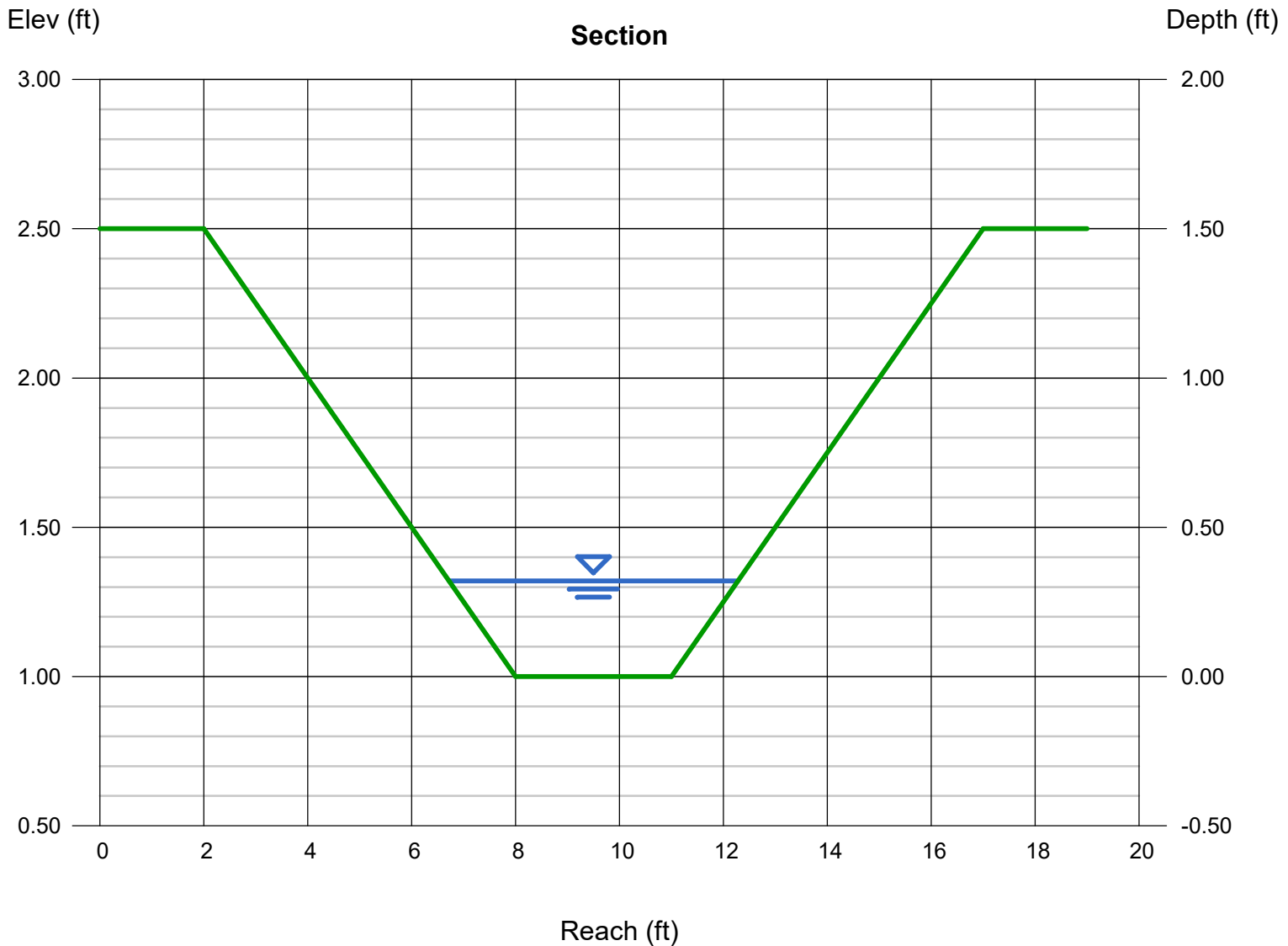
Bottom Width (ft) = 3.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 1.50
Invert Elev (ft) = 1.00
Slope (%) = 5.00
N-Value = 0.032

Highlighted

Depth (ft) = 0.32
Q (cfs) = 5.300
Area (sqft) = 1.37
Velocity (ft/s) = 3.87
Wetted Perim (ft) = 5.64
Crit Depth, Y_c (ft) = 0.39
Top Width (ft) = 5.56
EGL (ft) = 0.55

Calculations

Compute by: Known Q
Known Q (cfs) = 5.30



Channel Report

DP-14 Q100

Trapezoidal

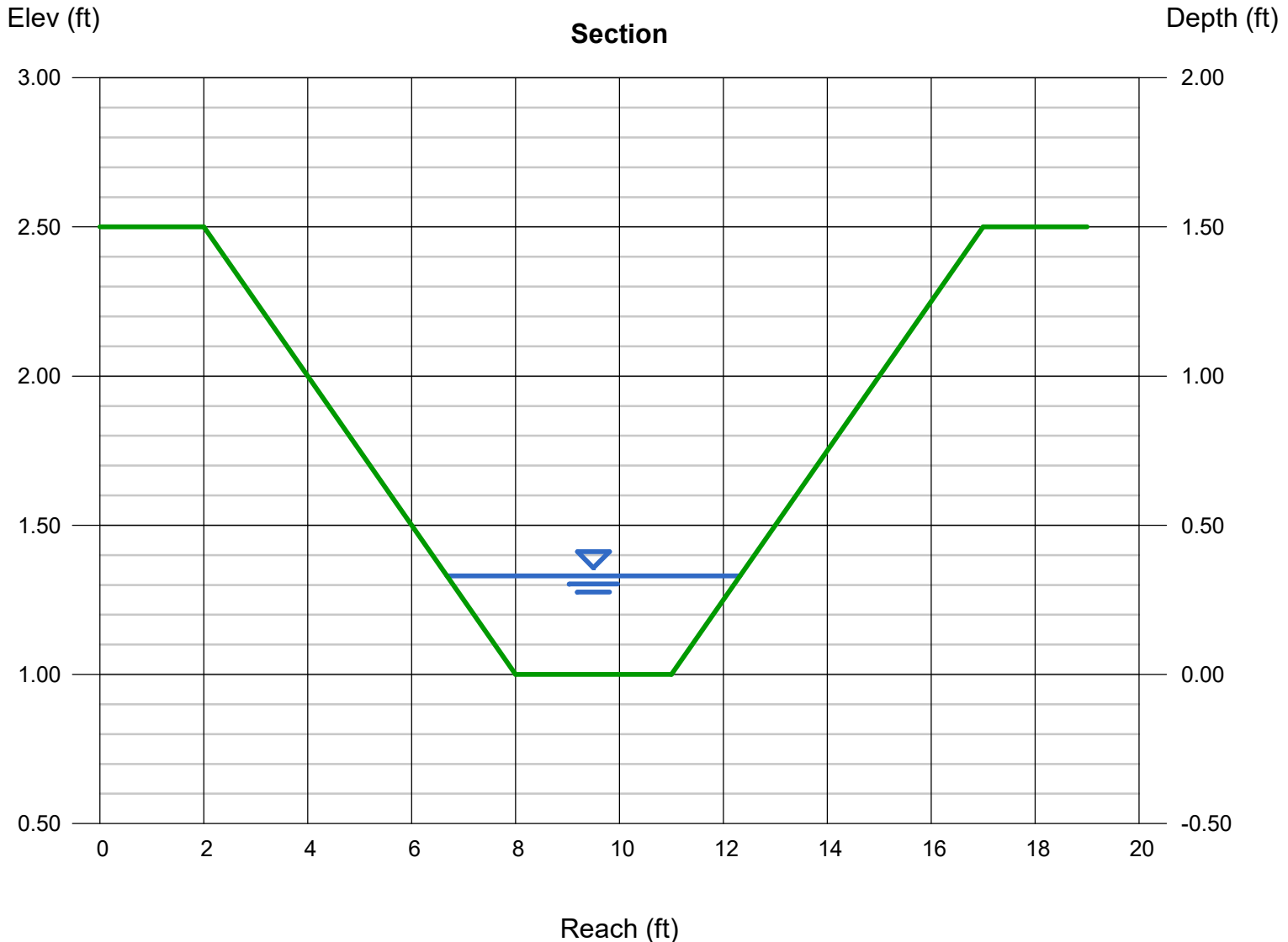
Bottom Width (ft) = 3.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 1.50
Invert Elev (ft) = 1.00
Slope (%) = 4.60
N-Value = 0.032

Highlighted

Depth (ft) = 0.33
Q (cfs) = 5.500
Area (sqft) = 1.43
Velocity (ft/s) = 3.86
Wetted Perim (ft) = 5.72
Crit Depth, Yc (ft) = 0.40
Top Width (ft) = 5.64
EGL (ft) = 0.56

Calculations

Compute by: Known Q
Known Q (cfs) = 5.50



Channel Report

DP-15 Q100

Trapezoidal

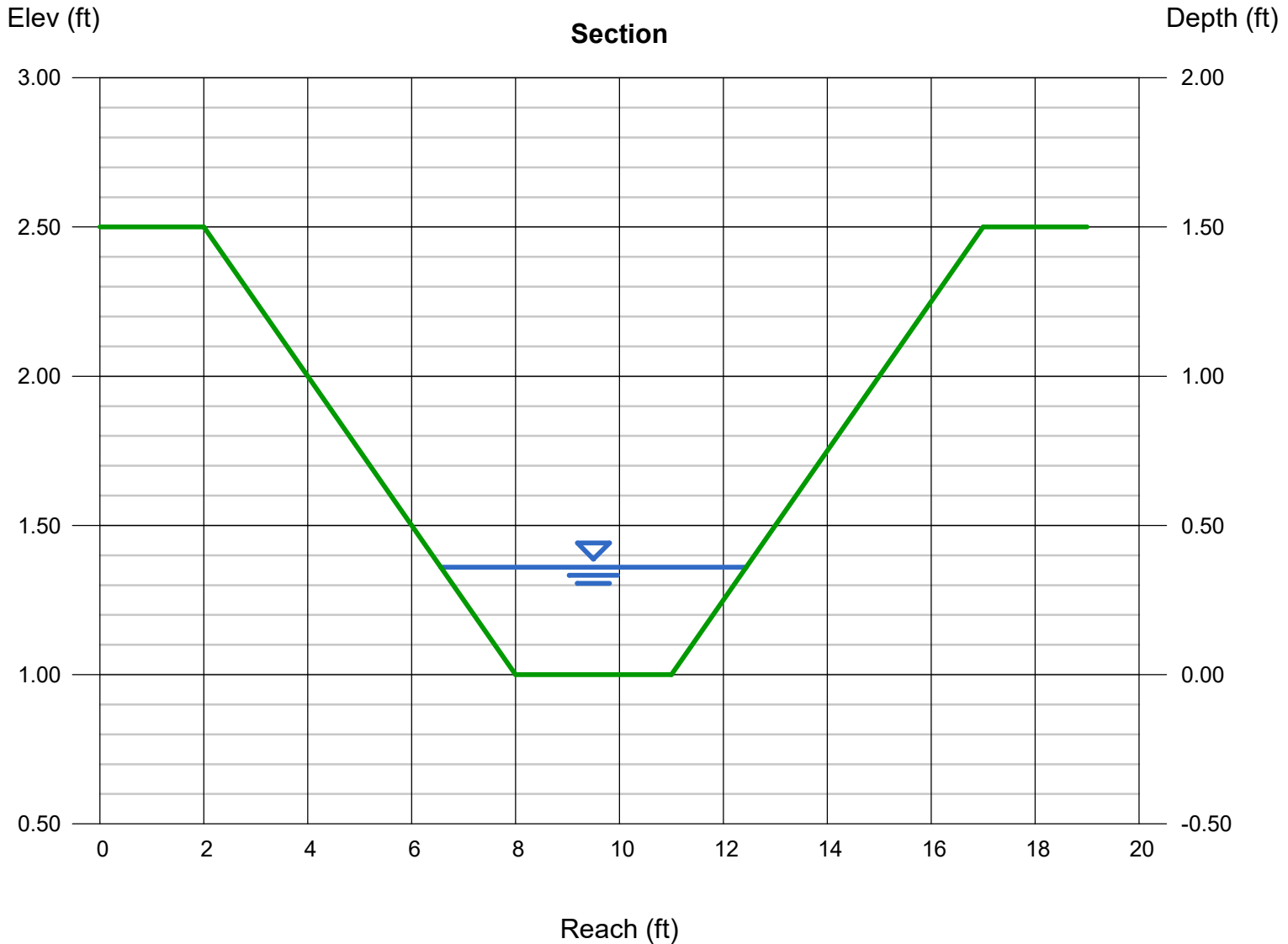
Bottom Width (ft) = 3.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 1.50
Invert Elev (ft) = 1.00
Slope (%) = 4.60
N-Value = 0.032

Highlighted

Depth (ft) = 0.36
Q (cfs) = 6.600
Area (sqft) = 1.60
Velocity (ft/s) = 4.13
Wetted Perim (ft) = 5.97
Crit Depth, Yc (ft) = 0.44
Top Width (ft) = 5.88
EGL (ft) = 0.63

Calculations

Compute by: Known Q
Known Q (cfs) = 6.60



Channel Report

DP-16 Q100

Trapezoidal

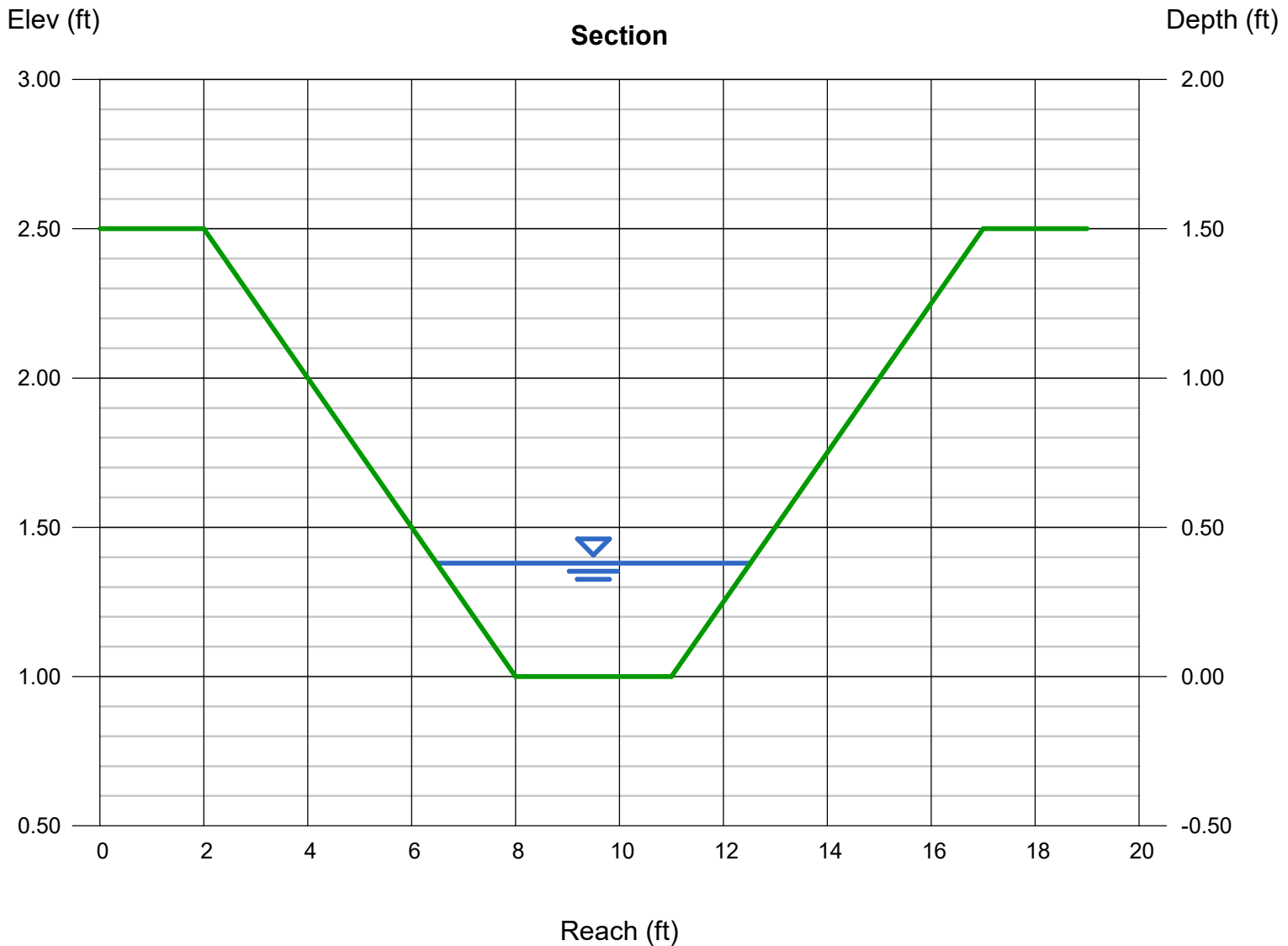
Bottom Width (ft) = 3.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 1.50
Invert Elev (ft) = 1.00
Slope (%) = 4.60
N-Value = 0.032

Highlighted

Depth (ft) = 0.38
Q (cfs) = 7.200
Area (sqft) = 1.72
Velocity (ft/s) = 4.19
Wetted Perim (ft) = 6.13
Crit Depth, Yc (ft) = 0.46
Top Width (ft) = 6.04
EGL (ft) = 0.65

Calculations

Compute by: Known Q
Known Q (cfs) = 7.20



Channel Report

DP-17 Q100

Trapezoidal

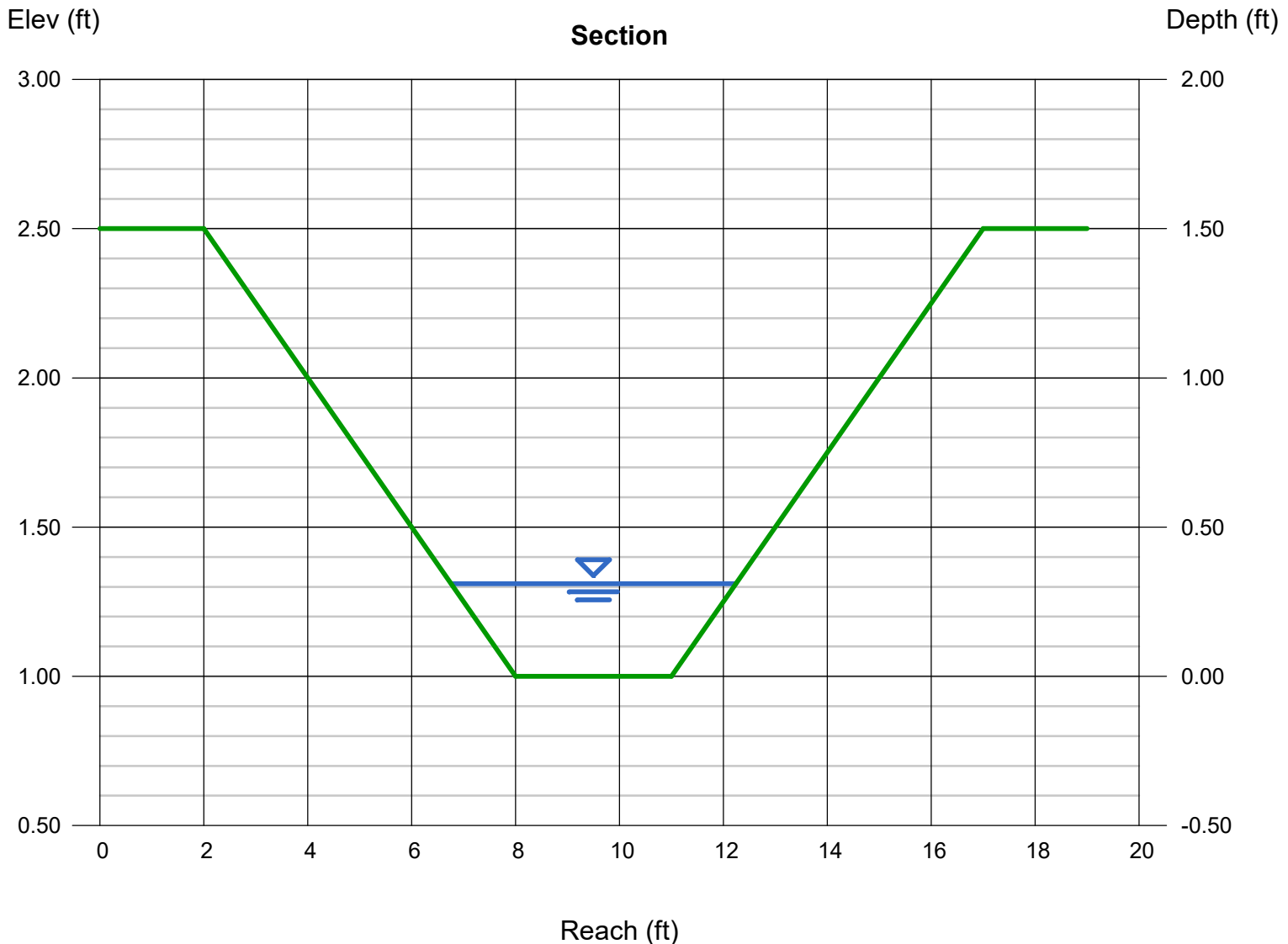
Bottom Width (ft) = 3.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 1.50
Invert Elev (ft) = 1.00
Slope (%) = 4.90
N-Value = 0.032

Highlighted

Depth (ft) = 0.31
Q (cfs) = 5.000
Area (sqft) = 1.31
Velocity (ft/s) = 3.80
Wetted Perim (ft) = 5.56
Crit Depth, Yc (ft) = 0.38
Top Width (ft) = 5.48
EGL (ft) = 0.53

Calculations

Compute by: Known Q
Known Q (cfs) = 5.00



Channel Report

DP-18 Q100

Trapezoidal

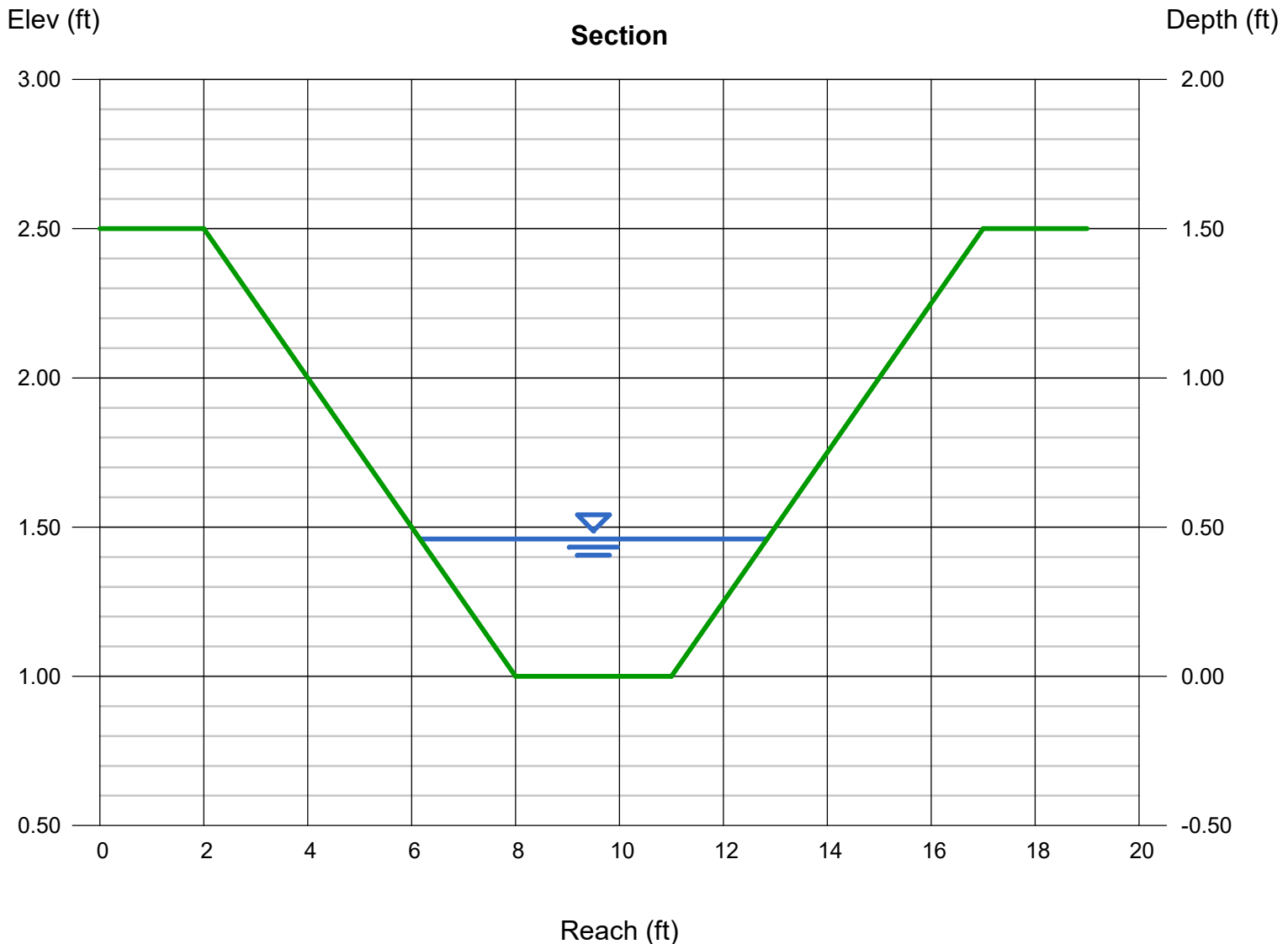
Bottom Width (ft) = 3.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 1.50
Invert Elev (ft) = 1.00
Slope (%) = 4.90
N-Value = 0.032

Highlighted

Depth (ft) = 0.46
Q (cfs) = 10.60
Area (sqft) = 2.23
Velocity (ft/s) = 4.76
Wetted Perim (ft) = 6.79
Crit Depth, Yc (ft) = 0.57
Top Width (ft) = 6.68
EGL (ft) = 0.81

Calculations

Compute by: Known Q
Known Q (cfs) = 10.60



Channel Report

DP-19 Q100

Trapezoidal

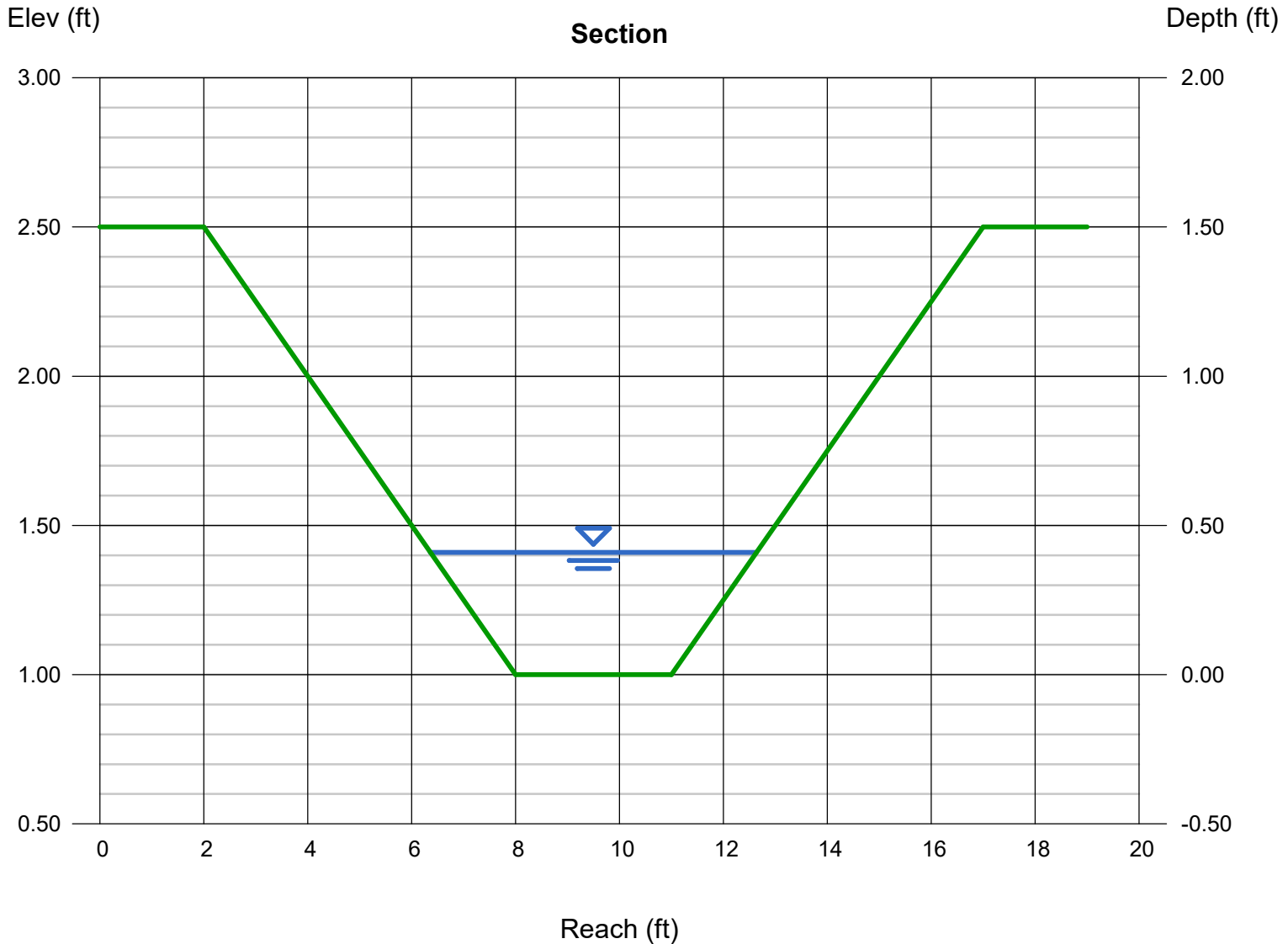
Bottom Width (ft) = 3.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 1.50
Invert Elev (ft) = 1.00
Slope (%) = 2.60
N-Value = 0.032

Highlighted

Depth (ft) = 0.41
Q (cfs) = 6.100
Area (sqft) = 1.90
Velocity (ft/s) = 3.21
Wetted Perim (ft) = 6.38
Crit Depth, Yc (ft) = 0.42
Top Width (ft) = 6.28
EGL (ft) = 0.57

Calculations

Compute by: Known Q
Known Q (cfs) = 6.10



Channel Report

DP-20 Q100

Trapezoidal

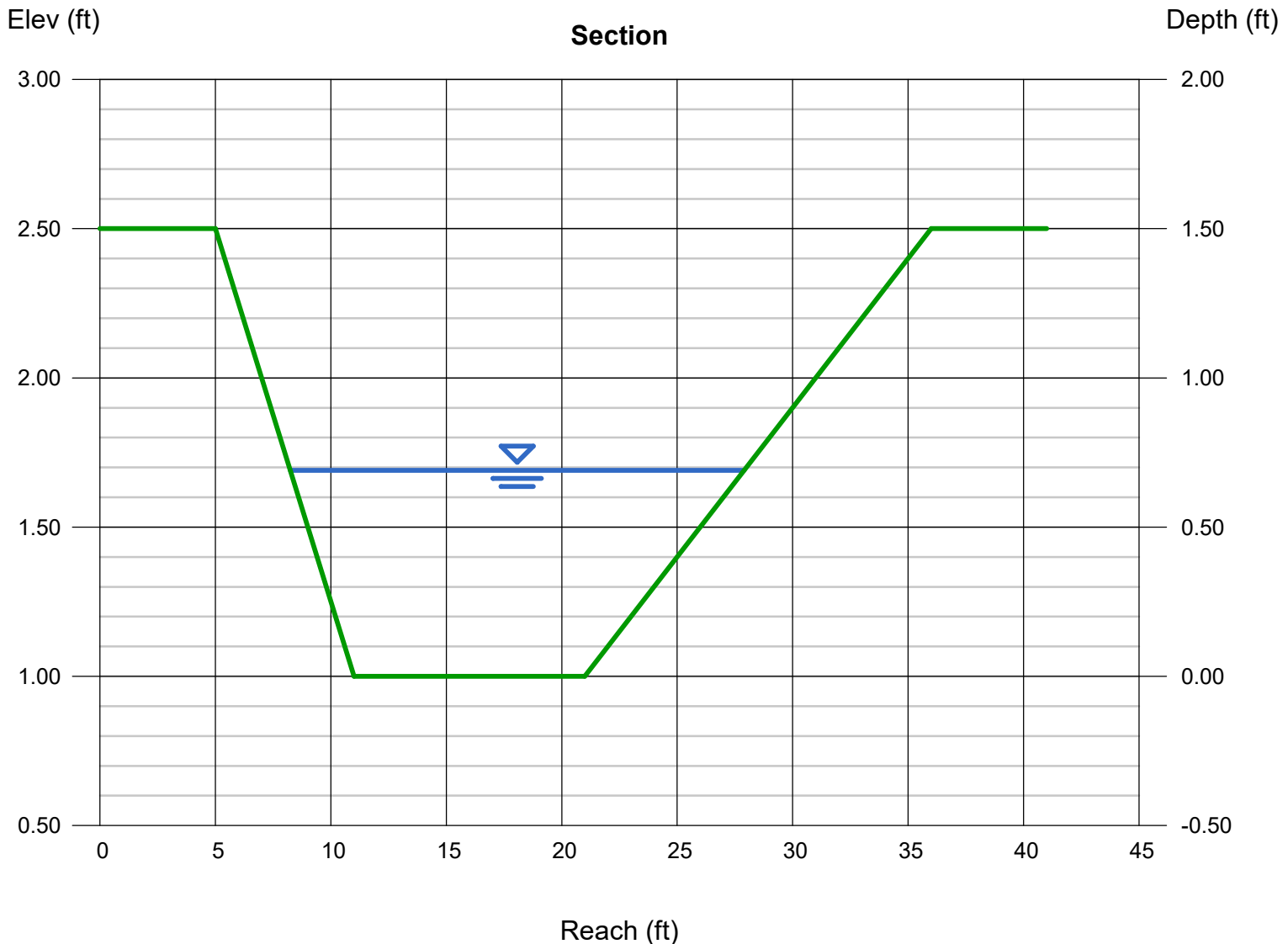
Bottom Width (ft) = 10.00
Side Slopes (z:1) = 4.00, 10.00
Total Depth (ft) = 1.50
Invert Elev (ft) = 1.00
Slope (%) = 2.00
N-Value = 0.032

Highlighted

Depth (ft) = 0.69
Q (cfs) = 43.00
Area (sqft) = 10.23
Velocity (ft/s) = 4.20
Wetted Perim (ft) = 19.78
Crit Depth, Yc (ft) = 0.71
Top Width (ft) = 19.66
EGL (ft) = 0.96

Calculations

Compute by: Known Q
Known Q (cfs) = 43.00



Channel Report

Hydraflow Express Extension for Autodesk® Civil 3D® by Autodesk, Inc.

Friday, Nov 17 2023

DP-21a Q100

Trapezoidal

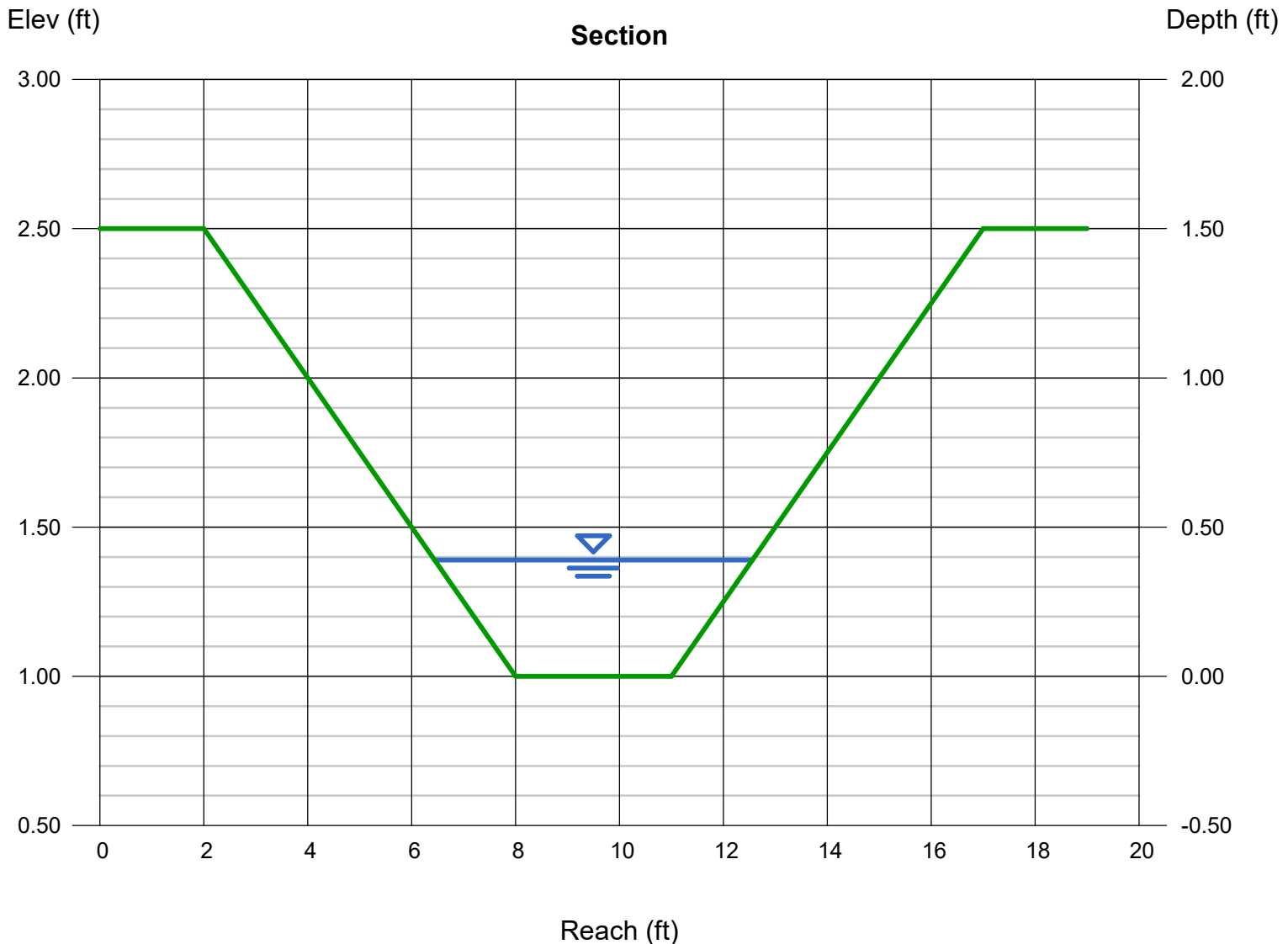
Bottom Width (ft) = 3.00
Side Slopes (z:1) = 4.00, 4.00
Total Depth (ft) = 1.50
Invert Elev (ft) = 1.00
Slope (%) = 4.80
N-Value = 0.032

Highlighted

Depth (ft) = 0.39
Q (cfs) = 7.800
Area (sqft) = 1.78
Velocity (ft/s) = 4.39
Wetted Perim (ft) = 6.22
Crit Depth, Y_c (ft) = 0.48
Top Width (ft) = 6.12
EGL (ft) = 0.69

Calculations

Compute by: Known Q
Known Q (cfs) = 7.80



APPENDIX B

STANDARD DESIGN CHARTS AND TABLES

Table 6-6. Runoff Coefficients for Rational Method
(Source: UDFCD 2001)

Land Use or Surface Characteristics	Percent Impervious	Runoff Coefficients											
		2-year		5-year		10-year		25-year		50-year		100-year	
		HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D	HSG A&B	HSG C&D
Business													
Commercial Areas	95	0.79	0.80	0.81	0.82	0.83	0.84	0.85	0.87	0.87	0.88	0.88	0.89
Neighborhood Areas	70	0.45	0.49	0.49	0.53	0.53	0.57	0.58	0.62	0.60	0.65	0.62	0.68
Residential													
1/8 Acre or less	65	0.41	0.45	0.45	0.49	0.49	0.54	0.54	0.59	0.57	0.62	0.59	0.65
1/4 Acre	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
1/3 Acre	30	0.18	0.22	0.25	0.30	0.32	0.38	0.39	0.47	0.43	0.52	0.47	0.57
1/2 Acre	25	0.15	0.20	0.22	0.28	0.30	0.36	0.37	0.46	0.41	0.51	0.46	0.56
1 Acre	20	0.12	0.17	0.20	0.26	0.27	0.34	0.35	0.44	0.40	0.50	0.44	0.55
Industrial													
Light Areas	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Heavy Areas	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Parks and Cemeteries	7	0.05	0.09	0.12	0.19	0.20	0.29	0.30	0.40	0.34	0.46	0.39	0.52
Playgrounds	13	0.07	0.13	0.16	0.23	0.24	0.31	0.32	0.42	0.37	0.48	0.41	0.54
Railroad Yard Areas	40	0.23	0.28	0.30	0.35	0.36	0.42	0.42	0.50	0.46	0.54	0.50	0.58
Undeveloped Areas													
Historic Flow Analysis-- Greenbelts, Agriculture	2	0.03	0.05	0.09	0.16	0.17	0.26	0.26	0.38	0.31	0.45	0.36	0.51
Pasture/Meadow	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Forest	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50
Exposed Rock	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Offsite Flow Analysis (when landuse is undefined)	45	0.26	0.31	0.32	0.37	0.38	0.44	0.44	0.51	0.48	0.55	0.51	0.59
Streets													
Paved	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Gravel	80	0.57	0.60	0.59	0.63	0.63	0.66	0.66	0.70	0.68	0.72	0.70	0.74
Drive and Walks	100	0.89	0.89	0.90	0.90	0.92	0.92	0.94	0.94	0.95	0.95	0.96	0.96
Roofs	90	0.71	0.73	0.73	0.75	0.75	0.77	0.78	0.80	0.80	0.82	0.81	0.83
Lawns	0	0.02	0.04	0.08	0.15	0.15	0.25	0.25	0.37	0.30	0.44	0.35	0.50

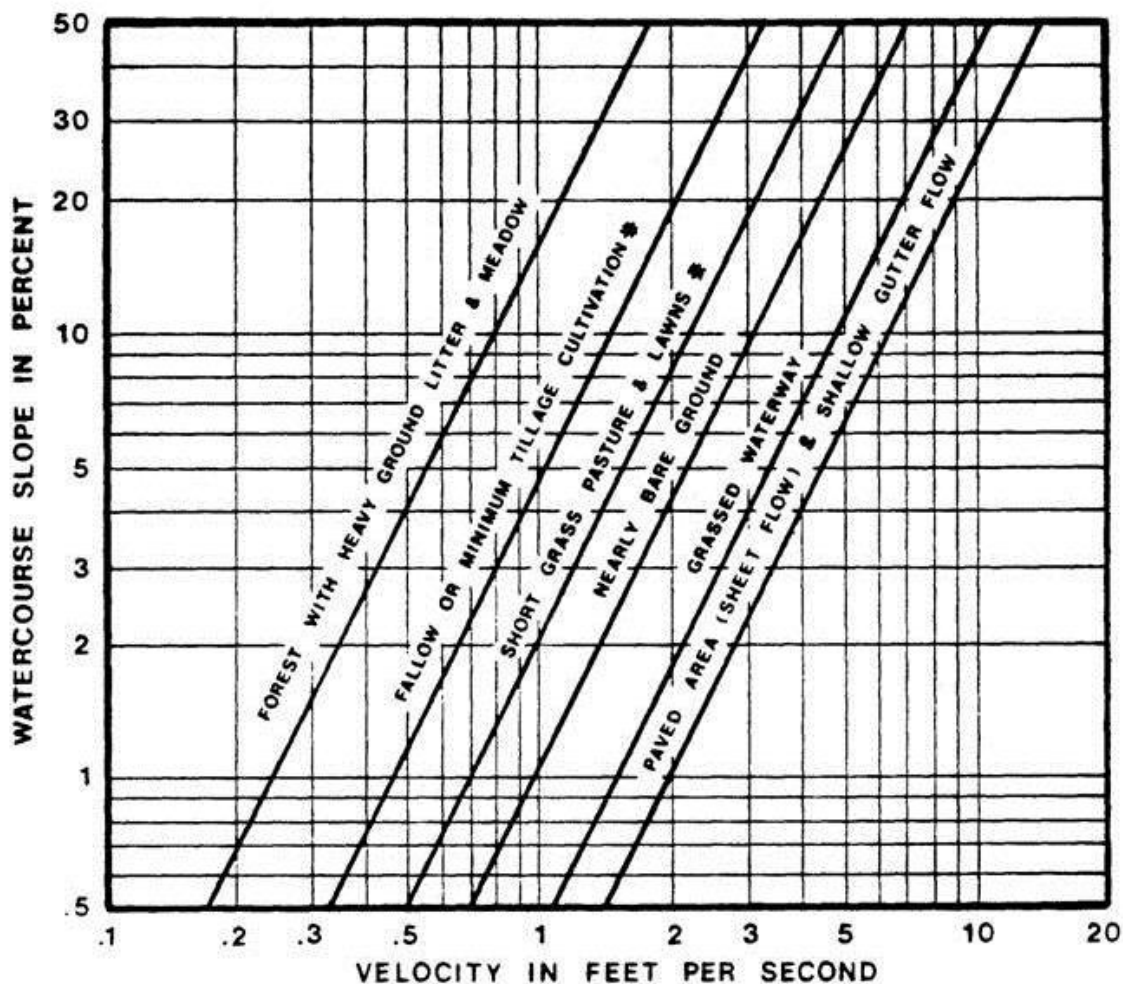
3.2 Time of Concentration

One of the basic assumptions underlying the Rational Method is that runoff is a function of the average rainfall rate during the time required for water to flow from the hydraulically most remote part of the drainage area under consideration to the design point. However, in practice, the time of concentration can be an empirical value that results in reasonable and acceptable peak flow calculations.

For urban areas, the time of concentration (t_c) consists of an initial time or overland flow time (t_i) plus the travel time (t_t) in the storm sewer, paved gutter, roadside drainage ditch, or drainage channel. For non-urban areas, the time of concentration consists of an overland flow time (t_i) plus the time of travel in a concentrated form, such as a swale or drainageway. The travel portion (t_t) of the time of concentration can be estimated from the hydraulic properties of the storm sewer, gutter, swale, ditch, or drainageway. Initial time, on the other hand, will vary with surface slope, depression storage, surface cover, antecedent rainfall, and infiltration capacity of the soil, as well as distance of surface flow. The time of concentration is represented by Equation 6-7 for both urban and non-urban areas.

Type of Development	Percent Impervious
Commercial	95%
Industrial	85%
Multi-Family	65%
Single Family - 0.1377 acre lots (6,000 SF)	53%
Single-Family - 0.20 acre lots	43%
Single-Family - 0.25 acre lots	40%
Single-Family - 0.33 acre lots	30%
Single-Family - 0.5 acre lots	25%
Single-Family - 1.0 acre lots	20%
Single-Family - 2.5 acre lots	11%
Single-Family - 5 acre lots	7%

Figure 6-25. Estimate of Average Concentrated Shallow Flow



El Paso County Drainage Basin Fees

Resolution No. 23-400

Basin Number	Receiving Waters	Year Studied	Drainage Basin Name	2024 Drainage Fee (per Impervious Acre)	2024 Bridge Fee (per Impervious Acre)
--------------	------------------	--------------	---------------------	--	--

Drainage Basins with DBPS's:

CHMS0200	Chico Creek	2013	Haegler Ranch	\$13,971	\$2,062
CHWS1200	Chico Creek	2001	Bennett Ranch	\$15,641	\$6,000
CHWS1400	Chico Creek	2013	Falcon	\$40,088	\$5,507
FOFO2000	Fountain Creek	2001	West Fork Jimmy Camp Creek	\$17,003	\$5,031
FOFO2600	Fountain Creek	1991*	Big Johnson / Crews Gulch	\$24,832	\$3,207
FOFO2800	Fountain Creek	1988*	Widefield	\$24,832	\$0
FOFO2900	Fountain Creek	1988*	Security	\$24,832	\$0
FOFO3000	Fountain Creek	1991*	Windmill Gulch	\$24,832	\$372
FOFO3100 / FOFO3200	Fountain Creek	1988*	Carson Street / Little Johnson	\$15,147	\$0
FOFO3400	Fountain Creek	1984*	Peterson Field	\$17,911	\$1,358
FOFO3600	Fountain Creek	1991*	Fisher's Canyon	\$24,832	\$0
FOFO4000	Fountain Creek	1996	Sand Creek	\$25,632	\$10,484
FOFO4200	Fountain Creek	1977	Spring Creek	\$12,879	\$0
FOFO4600	Fountain Creek	1984*	Southwest Area	\$24,832	\$0
FOFO4800	Fountain Creek	1991	Bear Creek	\$24,832	\$1,358
FOFO5800	Fountain Creek	1964	Camp Creek	\$2,752	\$0
FOMO1000	Monument Creek	1981	Douglas Creek	\$15,617	\$345
FOMO1200	Monument Creek	1977	Templeton Gap	\$16,032	\$372
FOMO2000	Monument Creek	1971	Pulpit Rock	\$8,234	\$0
FOMO2200	Monument Creek	1994	Cottonwood Creek / S. Pine	\$24,832	\$1,358
FOMO2400	Monument Creek	1966	Dry Creek	\$19,603	\$710
FOMO3600	Monument Creek	1989*	Black Squirrel Creek	\$11,275	\$710
FOMO3700	Monument Creek	1987*	Middle Tributary	\$20,722	\$0
FOMO3800	Monument Creek	1987*	Monument Branch	\$24,832	\$0
FOMO4000	Monument Creek	1996	Smith Creek	\$10,124	\$1,358
FOMO4200	Monument Creek	1989*	Black Forest	\$24,832	\$676
FOMO5200	Monument Creek	1993*	Dirty Woman Creek	\$24,832	\$1,358
FOMO5300	Fountain Creek	1993*	Crystal Creek	\$24,832	\$1,358

Miscellaneous Drainage Basins: ¹

CHBS0800	Chico Creek		Book Ranch	\$23,300	\$3,373
CHEC0400	Chico Creek		Upper East Chico	\$12,694	\$368
CHWS0200	Chico Creek		Telephone Exchange	\$13,947	\$327
CHWS0400	Chico Creek		Livestock Company	\$22,973	\$273
CHWS0600	Chico Creek		West Squirrel	\$11,975	\$4,970
CHWS0800	Chico Creek		Solberg Ranch	\$24,832	\$0
FOFO1200	Fountain Creek		Crooked Canyon	\$7,497	\$0
FOFO1400	Fountain Creek		Calhan Reservoir	\$6,259	\$365
FOFO1600	Fountain Creek		Sand Canyon	\$4,522	\$0
FOFO2000	Fountain Creek		Jimmy Camp Creek	\$24,832	\$1,161
FOFO2200	Fountain Creek		Fort Carson	\$19,603	\$710
FOFO2700	Fountain Creek		West Little Johnson	\$1,636	\$0
FOFO3800	Fountain Creek		Stratton	\$11,911	\$533
FOFO5000	Fountain Creek		Midland	\$19,603	\$710
FOFO6000	Fountain Creek		Palmer Trail	\$19,603	\$710
FOFO6800	Fountain Creek		Black Canyon	\$19,603	\$710
FOMO4600	Monument Creek		Beaver Creek	\$14,846	\$0
FOMO3000	Monument Creek		Kettle Creek	\$13,410	\$0
FOMO3400	Monument Creek		Elkhorn	\$2,253	\$0
FOMO5000	Monument Creek		Monument Rock	\$10,763	\$0
FOMO5400	Monument Creek		Palmer Lake	\$17,210	\$0
FOMO5600	Monument Creek		Raspberry Mountain	\$5,789	\$0
PLPL0200	Monument Creek		Bald Mountain	\$12,337	\$0

Interim Drainage Basins: ²

FOFO1800	Fountain Creek		Little Fountain Creek	\$3,175	\$0
FOMO4400	Monument Creek		Jackson Creek	\$9,829	\$0
FOMO4800	Monument Creek		Teachout Creek	\$6,825	\$1,026

1. The miscellaneous drainage fee previous to September 1999 resolution was the average of all drainage fees for basins with Basin Planning Studies performed within the last 14 years.

2. Interim Drainage Fees are based upon draft Drainage Basin Planning Studies or the Drainage Basin Identification and Fee Estimation Report. (Best available information suitable for setting a fee.)

Channel Slope	Lining	Permissible Mean Channel Velocity* (ft/sec)
0 - 5%	Sodded grass	7
	Bermudagrass	6
	Reed canarygrass	5
	Tall fescue	5
	Kentucky bluegrass	5
	Grass-legume mixture	4
	Red fescue	2.5
	Redtop	2.5
	Sericea lespedeza	2.5
	Annual lespedeza	2.5
	Small grains (temporary)	2.5
5 - 10%	Sodded grass	6

Channel Slope	Lining	Permissible Mean Channel Velocity* (ft/sec)
	Bermudagrass	5
	Reed canarygrass	4
	Tall fescue	4
	Kentucky bluegrass	4
	Grass-legume mixture	3
Greater than 10%	Sodded grass	5
	Bermudagrass	4
	Reed canarygrass	3
	Tall fescue	3
	Kentucky bluegrass	3

*For highly erodible soils, decrease permissible velocities by 25%.

*Grass lined channels are dependent upon assurances of continuous growth and maintenance of grass.

APPENDIX C

REPORT REFERENCES

```

1*****
  *****
* WATER SURFACE PROFILES *
  * U.S. ARMY CORPS OF ENGINEERS *
* VERSION OF SEPTEMBER 1988 *
  * THE HYDROLOGIC ENGINEERING CENTER *
* ERROR: 01,02 *
  * 609 SECOND STREET, SUITE D *
* UPDATED: 4 APRIL 1989 *
  * DAVIS, CALIFORNIA 95616-4687 *
* RUN DATE 11/ 3/ 3 TIME 12:40:22 *
  * (916) 756-1104, (916) 551-1748 *
*****
  *****

```

THIS DOCUMENT IS THE HEC-2 MODEL PRINT OUT FOR THE 2003 LOMR FOR BEAVER CREEK. IT WAS SOURCED FROM THE REGIONAL FLOODPLAIN ADMINISTRATOR AT PIKES PEAK REGIONAL BUILDING DEPARTMENT (PPRBD).

```

XXXXXXXXX XXXXX
XXXXX      X   X XXXXXXXX XXXXX
           X   X X       X   X           X
           X   X X       X
           XXXXXXXX XXXX   X           XXXXX
XXXXX      X   X X       X           X
           X   X X       X   X           X
           X   X XXXXXXXX XXXXX
XXXXXXXXX
END OF BANNER

```

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1
  11/ 3/ 3      12:40:22

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PAGE 1

```

THIS RUN EXECUTED 11/ 3/ 3 12:40:22
*****
  HEC2 RELEASE DATED SEP 88 UPDATED APR 1989

```

```

ERROR CORR - 01,02
MODIFICATION -

```

```

*****

```

```

T1 BEAVER CREEK LOMR PROJECT NO. 03012
T2 BEAVER CREEK FROM MONUMENT CREEK CONFLUENCE THROUGH SOUTH FORK

```

T3 FILE NAME BVREFF.DAT
 T3 100-YEAR FREQUENCY CROSS-SECTIONS L TO R FACING UPSTREAM

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q
WSEL	FQ							
	0	3	0	0	0	0	0	0

6733.2

J2	NPROF	IPLT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW
CHNIM	ITRACE							

THE QT ROW/CARD REPRESENTS THE FLOW VALUES USED IN THE HEC-2 MODELS. THIS QT CARD IS AT THE CONFLUENCE OF BEAVER AND MONUMENT CREEK WHICH IS LOCATED JUST DOWNSTREAM OF THE HAY AND BEAVER CREEK CONFLUENCE.

	1	0	-1					
QT	2	6317	20974					
NC	.07	.07	.05	.2	.4			

CONFLUENCE WITH MONUMENT CREEK

X1	60	15	1280	1360	0	0	0
GR	6752	1000	6750	1085	6740	1170	6735
	1235	6730	1250				
GR	6725	1280	6720	1295	6718	1305	6720
	1310	6725	1360				
GR	6730	1445	6735	1525	6740	1560	6745
	1670	6750	1810				

THIS QT CARD IS APPROXIMATELY AT THE HAY CREEK CONFLUENCE WITH BEAVER CREEK AND DEMONSTRATES A 127 CFS INCREASE OVER THE UPSTREAM QT CARD. (6915 CFS -6788 CFS=127 CFS). WE THEREFORE BELIEVE 127 CFS TO BE A REASONABLE FLOW TO USE FOR HAY CREEK.

QT	2	3507	6915				
NC	.07	.07	.05	.1	.3		
X1	1	14	1507	1545	1047	1047	1047
GR	6748	1000	6744	1163	6742	1193	6740
	1313	6740	1358				
GR	6738	1386	6736	1507	6734	1520	6732
	1526	6732	1541				
GR	6740	1548	6746	1563	6746	1748	6748
	1798						
NC	.07	.07	.05	.1	.3		
X1	2	11	1136	1195	759	759	759
GR	6758	1034	6756	1049	6750	1064	6748
	1076	6748	1136				
GR	6746	1147	6746	1184	6748	1195	6750
	1227	6756	1254				
GR	6758	1274					
QT	2	1847	5128				

NC .013 .013 .015 .1 .3

HAY CREEK ROAD 2-72", 1-78" AND 1-84" CMP'S
CROSS SECTION MODELED FOR ROAD PROFILE
FLOW OVER ROAD REDUCED FOR CULVERTS CAPACITY
CULVERTS CALCULATED CAPACITY = 1660 cfs

NOTE THAT, ONCE THE FLOW THROUGH
THE CULVERTS IS ACCOUNTED FOR, THE QT
CARD MATCHES THE UPSTREAM QT CARD
AT 6788 CFS. (5128 CFS + 1660 CFS = 6788 CFS)

X1 3 6 1130 1250 845 845 845
GR 6778 1000 6772 1130 6772 1250 6774
1390 6776 1500
GR 6778 1720

1
11/ 3/ 3 12:40:22

PAGE 2

THIS QT CARD IS AT THE HAY CREEK ROAD CULVERT
JUST UPSTREAM OF THE CONFLUENCE WITH HAY
CREEK.

NC .025 .025 .025 .1 .3
QT 2 3107 6788

DELACROCE RANCH ROAD CROSSING 1-84" AND 1-72" CMP
CROSS SECTION MODELED FOR ROAD PROFILE
FLOW OVER ROAD REDUCED FOR CULVERTS CAPACITY
CULVERTS CALCULATED CAPACITY = 400 cfs

X1 4 5 1070 1280 984 984 984
X4 1 6786 1000
GR 6790 900 6784 1070 6784 1280 6790
1420 6796 1520
NC .07 .07 .08
X1 5 14 1243 1314 722 722 722
GR 6806 1188 6804 1198 6800 1211 6798
1221 6786 1246
GR 6786 1272 6788 1314 6790 1344 6796
1391 6796 1427
GR 6798 1428 6800 1435 6804 1475 6806
1512

NC .013 .013 .013 .1 .3
QT 2 3216 6154

NC .013 .013 .015 .1 .3
X1 6.1 9 1209 1230 1160 1160 1160
GR 6840 1000 6828 1091 6828 1209 6806.6
1209 6806.6 1230
GR 6828 1230 6828 1316 6830 1457 6840
1619

SB	1.05 0	1.25 6807.3	2.6 6806.6	0	20	1.0	210
NC	.013	.013	.015	.3	.5		
LONG VALLEY DRIVE, EXISITNG 2-10' x 10' CBC FACE							
X1	6.2	9	1209	1230	48	48	48
X2	0	0	1	6817.3	6828		
BT	9 1209	1000 6828	6840 6828	6840	1091	6828	6828
BT	1209 6828	6828 6828	6817.3 1316	1230	6828	6817.3	1230
BT	6828 6840	6828	1457	6830	6830	1619	6840
GR	6840 1209	1000 6807.3	6828 1230	1091	6828	1209	6807.3
GR	6828 1619	1230	6828	1316	6830	1457	6840
NC	.07	.07	.08	.1	.3		
X1	7	9	1146	1193	856	856	856
GR	6862 1146	1033 6854	6858 1193	1042	6856	1071	6854
GR	6856 1354	1279	6858	1312	6860	1326	6862
NC	.015	.015	.02				
BRISTLECONE LAKE CONCRETE SPILLWAY							
X1	8	8	1080	1520	1026	1026	1026
GR	6910 1080	1000 6889	6908 1090	1020	6900	1050	6890
GR	6889	1500	6890	1520	6908	1580	
QT	2	4860	8624				
NC	.07	.07	.05				
FLOW INTO BRISTLECONE LAKE							
X1	9	13	1484	1817	2638	2638	2638
GR	6910 1173	1000 6894	6908 1260	1050	6906	1135	6900
GR	6890 1845	1484 6900	6890 1871	1817	6894	1824	6894
GR	6902	1905	6908	1982	6910	1991	

X1	10	15	1169	1529	590	590	590
GR	6916	1000	6910	1044	6908	1064	6904
	1112	6900	1230				
GR	6898	1270	6898	1334	6896	1349	6896
	1361	6898	1379				
GR	6900	1392	6902	1530	6910	1614	6912
	1645	6916	1662				
NC	.1	.1	.08				
X1	11	13	1149	1363	701	701	701
GR	6924	1000	6920	1041	6918	1069	6916
	1077	6910	1149				
GR	6908	1165	6904	1177	6904	1216	6906
	1226	6906	1346				
GR	6916	1381	6918	1392	6924	1456	
X1	12	14	1213	1240	643	643	643
GR	6936	1000	6928	1110	6926	1162	6922
	1176	6922	1213				
GR	6916	1226	6916	1228	6922	1240	6922
	1246	6918	1256				
GR	6916	1317	6916	1447	6930	1486	6936
	1566						
X1	13	10	1139	1345	689	689	689
GR	6948	1000	6944	1053	6940	1139	6938
	1152	6928	1195				
GR	6928	1221	6930	1230	6932	1274	6934
	1321	6948	1429				
X1	14	11	1184	1401	834	834	834
GR	6964	1000	6956	1071	6954	1119	6948
	1234	6946	1305				
GR	6944	1316	6944	1342	6946	1392	6956
	1414	6958	1435				
GR	6964	1527					
X1	15	17	1091	1278	764	764	764
GR	6978	1000	6970	1047	6966	1081	6962
	1091	6962	1133				
GR	6960	1145	6958	1161	6958	1177	6960
	1219	6962	1278				
GR	6962	1312	6960	1319	6960	1333	6962
	1336	6970	1371				

GR	6974	1430	6978	1448			
X1	16	15	1266	1400	718	718	718
GR	6994	1000	6992	1025	6986	1193	6984
	1205	6980	1266				
GR	6974	1321	6974	1331	6976	1364	6980
	1400	6982	1505				
GR	6986	1572	6988	1584	6990	1609	6992
	1615	6994	1618				

QT 2 4772 8270

CONFLUENCE OF NORTH AND SOUTH BEAVER CREEK

X1	17	16	1101	1378	547	547	547
GR	7008	1000	7002	1072	6998	1101	6994
	1117	6992	1121				
GR	6988	1139	6988	1169	6990	1196	6992
	1213	6996	1277				
GR	6996	1324	6998	1378	7000	1410	7002
	1427	7004	1489				
GR	7008	1552					

QT 2 3757 6754

BEAVER CREEK SOUTH FORK

X1	18	10	1075	1269	598	598	598
GR	7022	1000	7018	1075	7016	1088	7004
	1122	7002	1128				
GR	7002	1151	7008	1187	7012	1237	7020
	1269	7022	1285				

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X1	19	16	1115	1341	800	800	800
GR	7046	1000	7044	1021	7038	1072	7034
	1115	7032	1129				
GR	7030	1177	7028	1198	7026	1207	7026
	1228	7030	1242				
GR	7032	1259	7034	1341	7038	1412	7040
	1473	7042	1536				
GR	7046	1578					

X1	20	14	1046	1295	625	625	625
GR	7068	1000	7062	1031	7058	1046	7056
	1061	7052	1072				

20974.	1400.	15429.	4145.	193.	921.	588.	0.	0.
6725.00								
.00	7.27	16.75	7.05	.070	.050	.070	.000	6718.00
1239.93								
.012557	0.	0.	0.	0	4	0	.00	258.76
1498.70								
0								
CCHV=	.100	CEHV=	.300					
*SECNO	1.000							

3301 HV CHANGED MORE THAN HVINS

1.00	11.87	6743.87	.00	.00	6744.18	.30	7.10	.31
6736.00								
6915.	4409.	2319.	187.	1514.	359.	74.	44.	8.
6732.00								
.07	2.91	6.46	2.53	.070	.050	.070	.000	6732.00
1164.88								
.002082	1047.	1047.	1047.	3	0	0	.00	392.81
1557.69								
0								
CCHV=	.100	CEHV=	.300					
*SECNO	2.000							

3301 HV CHANGED MORE THAN HVINS

3685 20 TRIALS ATTEMPTED WSEL,CWSEL
 3693 PROBABLE MINIMUM SPECIFIC ENERGY
 3720 CRITICAL DEPTH ASSUMED

2.00	5.95	6751.95	6751.95	.00	6753.89	1.94	3.60	.49
6748.00								
6915.	1999.	4311.	605.	277.	329.	103.	67.	13.
6748.00								
.09	7.21	13.09	5.87	.070	.050	.070	.000	6746.00
1059.12								
.019770	759.	759.	759.	20	8	0	.00	176.67
1235.79								
0								
CCHV=	.100	CEHV=	.300					
*SECNO	3.000							

3301 HV CHANGED MORE THAN HVINS

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SECNO	DEPTH	CWSEL	CRIWS	WSELK	EG	HV	HL	OLOSS
BANK	ELEV							
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA
LEFT/RIGHT	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN
TIME								
SSTA								
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID
ENDST								

3685 20 TRIALS ATTEMPTED WSEL,CWSEL
 3693 PROBABLE MINIMUM SPECIFIC ENERGY
 3720 CRITICAL DEPTH ASSUMED

HAY CREEK ROAD 2-72", 1-78" AND 1-84" CMP'S
 CROSS SECTION MODELED FOR ROAD PROFILE
 FLOW OVER ROAD REDUCED FOR CULVERTS CAPACITY
 CULVERTS CALCULATED CAPACITY = 1660 cfs

3.00	2.75	6774.75	6774.75	.00	6775.69	.94	4.43	.10
6772.00								
5128.	506.	2806.	1817.	82.	330.	260.	80.	18.
6772.00								
.12	6.18	8.51	6.99	.013	.015	.013	.000	6772.00
1070.47								
.001918	845.	845.	845.	20	14	0	.00	360.65
1431.12								
0								

CCHV= .100 CEHV= .300

*SECNO 4.000

3685 20 TRIALS ATTEMPTED WSEL,CWSEL
 3693 PROBABLE MINIMUM SPECIFIC ENERGY
 3720 CRITICAL DEPTH ASSUMED

DELACROCE RANCH ROAD CROSSING 1-84" AND 1-72" CMP
 CROSS SECTION MODELED FOR ROAD PROFILE
 FLOW OVER ROAD REDUCED FOR CULVERTS CAPACITY
 CULVERTS CALCULATED CAPACITY = 400 cfs

4.00	2.82	6786.82	6786.82	.00	6787.96	1.14	3.30	.06
6784.00								
6788.	884.	5375.	530.	136.	592.	93.	97.	26.
6784.00								
.15	6.52	9.08	5.72	.025	.025	.025	.000	6784.00
979.55								
.005866	984.	984.	984.	20	8	0	.00	366.20
1345.75								
0								

*SECNO 5.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .54

5.00	7.93	6793.93	.00	.00	6795.16	1.23	7.17	.03
6798.00								
6788.	0.	5205.	1583.	0.	563.	208.	110.	30.
6788.00								
.18	.00	9.25	7.59	.000	.080	.070	.000	6786.00
1229.48								
.020365	722.	722.	722.	3	0	0	.00	145.32
1374.79								
0								

CCHV= .100 CEHV= .300

CCHV= .100 CEHV= .300

*SECNO 6.100

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SECNO	DEPTH	CWSEL	CRIWS	WSELK	EG	HV	HL	OLOSS
BANK ELEV	Q	QLOB	QCH	QROB	ALOB	ACH	AROB	TWA
LEFT/RIGHT	TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	ELMIN
SSTA	SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR
ENDST								TOPWID

3301 HV CHANGED MORE THAN HVINS

3685 20 TRIALS ATTEMPTED WSEL,CWSEL
 3693 PROBABLE MINIMUM SPECIFIC ENERGY
 3720 CRITICAL DEPTH ASSUMED

6.10	13.82	6820.42	6820.42	.00	6827.40	6.98	9.61	1.73
6828.00								
6154.	0.	6154.	0.	0.	290.	0.	125.	33.
6828.00								
.19	.00	21.20	.00	.000	.015	.000	.000	6806.60
1209.00								
.004232	1160.	1160.	1160.	20	14	0	.00	21.00
1230.00								
0								

SPECIAL BRIDGE

5227 DOWNSTREAM ELEV IS 6818.62 , NOT 6820.42 HYDRAULIC JUMP OCCURS
 DOWNSTREAM (IF LOW FLOW CONTROLS)

SB	XK	XKOR	COFQ	RDLEN	BWC	BWP	BAREA	SS
ELCHU		ELCHD						
1.05		1.25	2.60	.00	20.00	1.00	210.00	.00
6807.30		6806.60						

CCHV= .300 CEHV= .500
 *SECNO 6.200

3301 HV CHANGED MORE THAN HVINS

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = 1.45

PRESSURE AND WEIR FLOW, Weir Submergence Based on TRAPEZOIDAL Shape

EGPRS	EGLWC	H3	QWEIR	QPR	BAREA	TRAPEZOID	ELLC
ELTRD	WEIRLN					AREA	
6837.09	6829.67	.00	1566.	4591.	210.	190.	6817.30
6828.00	358.						

LONG VALLEY DRIVE, EXISITNG 2-10' x 10' CBC FACE

6.20	18.51	6825.81	.00	.00	6829.70	3.89	2.30	.00
6828.00								
6154.	0.	6154.	0.	0.	389.	0.	125.	33.
6828.00								
.19	.00	15.84	.00	.000	.015	.000	.000	6807.30
1209.00								
.002023	48.	48.	48.	6	0	5	.00	21.00
1230.00								
0								
CCHV=	.100	CEHV=	.300					

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SECNO	DEPTH	CWSEL	CRIWS	WSELK	EG	HV	HL	OLOSS	
BANK ELEV	Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA
LEFT/RIGHT	TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN
SSTA	SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID
ENDST									

*SECNO 7.000

3301 HV CHANGED MORE THAN HVINS

3685 20 TRIALS ATTEMPTED WSEL,CWSEL
 3693 PROBABLE MINIMUM SPECIFIC ENERGY
 3720 CRITICAL DEPTH ASSUMED

7.00	3.79	6857.79	6857.79	.00	6859.09	1.29	4.78	.26
6854.00								
6154.	2041.	1773.	2340.	233.	178.	267.	135.	35.
6854.00								
.22	8.77	9.94	8.78	.070	.080	.070	.000	6854.00
1045.01								
.048456	856.	856.	856.	20	21	0	.00	263.57
1308.58								
0								

*SECNO 8.000

3685 20 TRIALS ATTEMPTED WSEL,CWSEL
 3693 PROBABLE MINIMUM SPECIFIC ENERGY
 3720 CRITICAL DEPTH ASSUMED

BRISTLECONE LAKE CONCRETE SPILLWAY

8.00	1.86	6890.86	6890.86	.00	6891.77	.91	11.36	.04
6890.00								
6154.	4.	6145.	5.	1.	802.	1.	153.	44.
6890.00								
.26	3.76	7.66	3.78	.015	.020	.015	.000	6889.00
1077.43								
.004781	1026.	1026.	1026.	20	11	0	.00	445.42
1522.85								
0								

*SECNO 9.000

3301 HV CHANGED MORE THAN HVINS

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = 3.00

FLOW INTO BRISTLECONE LAKE

9.00	6.25	6896.25	.00	.00	6896.39	.14	4.54	.08
6890.00								
8624.	1738.	6774.	112.	988.	2081.	88.	273.	76.
6890.00								
.51	1.76	3.26	1.28	.070	.050	.070	.000	6890.00
1227.40								
.001042	2638.	2638.	2638.	6	0	0	.00	627.35
1854.74								
0								

*SECNO 10.000

3301 HV CHANGED MORE THAN HVINS

3685 20 TRIALS ATTEMPTED WSEL,CWSEL

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SECNO	DEPTH	CWSEL	CRIWS	WSELK	EG	HV	HL	OLOSS	
BANK ELEV	Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA
LEFT/RIGHT	TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN
SSTA	SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID
ENDST									

3693 PROBABLE MINIMUM SPECIFIC ENERGY

3720 CRITICAL DEPTH ASSUMED

10.00	6.42	6902.42	6902.42	.00	6903.79	1.37	1.64	.37
6904.00								
8624.	0.	7868.	756.	0.	806.	197.	301.	83.
6900.00								
.52	.00	9.76	3.84	.000	.050	.070	.000	6896.00
1158.67								
.020723	590.	590.	590.	20	8	0	.00	375.72
1534.39								
0								

*SECNO 11.000

3301 HV CHANGED MORE THAN HVINS

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = 1.51

11.00	8.51	6912.51	.00	.00	6913.11	.60	9.25	.08
6910.00								

6754.	4.	6742.	7.	3.	890.	6.	460.	118.
7034.00								
.75	1.31	7.58	1.31	.100	.080	.100	.000	7026.00
1106.50								
.026968	800.	800.	800.	4	0	0	.00	248.53
1355.03								
0								
1								

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SECNO	DEPTH	CWSEL	CRIWS	WSELK	EG	HV	HL	OLOSS
BANK ELEV								
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA
LEFT/RIGHT								
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN
SSTA								
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID
ENDST								

*SECNO 20.000

3301 HV CHANGED MORE THAN HVINS

20.00	6.36	7054.36	.00	.00	7055.81	1.45	19.96	.17
7058.00								
6754.	0.	6754.	0.	0.	698.	0.	472.	121.
7060.00								
.77	.00	9.67	.00	.000	.080	.000	.000	7048.00
1065.51								
.038415	625.	625.	625.	2	0	0	.00	160.28
1225.79								
0								

*SECNO 21.000

21.00	8.56	7080.56	.00	.00	7081.53	.97	25.68	.05
7080.00								
6754.	1.	6752.	1.	1.	854.	1.	487.	125.
7080.00								
.80	.96	7.90	.96	.100	.080	.100	.000	7072.00
1061.22								
.022960	879.	879.	879.	5	0	0	.00	188.29
1249.51								
0								

*SECNO 22.000

3301 HV CHANGED MORE THAN HVINS

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = .65

22.00	6.61	7098.61	7098.55	.00	7100.44	1.83	18.65	.26
7096.00								
6754.	55.	6570.	128.	14.	598.	31.	497.	127.
7096.00								

.82	4.05	10.98	4.11	.100	.080	.100	.000	7092.00
1021.54								
.054047	554.	554.	554.	6	19	0	.00	181.31
1202.86								
0								

*SECNO 23.000

3302 WARNING: CONVEYANCE CHANGE OUTSIDE OF ACCEPTABLE RANGE, KRATIO = 1.42

23.00	9.38	7117.38	.00	.00	7119.39	2.01	18.90	.05
7110.00								
6754.	1332.	5064.	358.	231.	398.	65.	505.	129.
7110.00								
.83	5.76	12.71	5.49	.100	.080	.100	.000	7108.00
1056.30								
.026617	514.	514.	514.	4	0	0	.00	126.40
1182.70								
0								
1								

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THIS RUN EXECUTED 11/ 3/ 3 12:40:22

HEC2 RELEASE DATED SEP 88 UPDATED APR 1989

ERROR CORR - 01,02

MODIFICATION -

NOTE- ASTERISK (*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

100-YEAR FREQUENCY CROSS

SUMMARY PRINTOUT TABLE 150

SECNO	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRIWS
EG	10*KS	VCH	AREA	.01K			
* 60.000	.00	.00	.00	6718.00	20974.00	6733.36	6733.36
6736.77	125.57	16.75	1701.40	1871.69			
1.000	1047.00	.00	.00	6732.00	6915.00	6743.87	.00
6744.18	20.82	6.46	1946.76	1515.58			
* 2.000	759.00	.00	.00	6746.00	6915.00	6751.95	6751.95
6753.89	197.70	13.09	709.71	491.80			

*	3.000	845.00	.00	.00	6772.00	5128.00	6774.75	6774.75
	6775.69	19.18	8.51	671.52	1171.06			
*	4.000	984.00	.00	.00	6784.00	6788.00	6786.82	6786.82
	6787.96	58.66	9.08	820.00	886.28			
*	5.000	722.00	.00	.00	6786.00	6788.00	6793.93	.00
	6795.16	203.65	9.25	771.30	475.67			
*	6.100	1160.00	.00	.00	6806.60	6154.00	6820.42	6820.42
	6827.40	42.32	21.20	290.26	946.04			
*	6.200	48.00	6828.00	6817.30	6807.30	6154.00	6825.81	.00
	6829.70	20.23	15.84	388.61	1368.18			
*	7.000	856.00	.00	.00	6854.00	6154.00	6857.79	6857.79
	6859.09	484.56	9.94	677.64	279.57			
*	8.000	1026.00	.00	.00	6889.00	6154.00	6890.86	6890.86
	6891.77	47.81	7.66	804.16	890.04			
*	9.000	2638.00	.00	.00	6890.00	8624.00	6896.25	.00
	6896.39	10.42	3.26	3157.00	2671.12			
*	10.000	590.00	.00	.00	6896.00	8624.00	6902.42	6902.42
	6903.79	207.23	9.76	1002.54	599.08			
*	11.000	701.00	.00	.00	6904.00	8624.00	6912.51	.00
	6913.11	91.30	6.30	1435.60	902.54			
*	12.000	643.00	.00	.00	6916.00	8624.00	6921.74	.00
	6922.57	270.02	6.08	1181.02	524.82			
	13.000	689.00	.00	.00	6928.00	8624.00	6937.37	.00
	6938.49	197.89	8.55	1036.84	613.05			
	14.000	834.00	.00	.00	6944.00	8624.00	6952.38	.00
	6953.10	155.30	6.85	1278.76	692.03			
	15.000	764.00	.00	.00	6958.00	8624.00	6965.44	.00
	6966.26	191.28	7.60	1231.38	623.56			

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SECNO	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRIWS
EG	10*KS	VCH	AREA	.01K			
16.000	718.00	.00	.00	6974.00	8624.00	6982.28	6981.98
6983.91	317.27	10.55	940.80	484.16			
17.000	547.00	.00	.00	6988.00	8270.00	6997.53	.00
6998.36	219.38	7.28	1136.59	558.36			

18.000	598.00	.00	.00	7002.00	6754.00	7011.92	.00
7013.30	288.96	9.43	716.36	397.32			
19.000	800.00	.00	.00	7026.00	6754.00	7034.79	.00
7035.68	269.68	7.58	898.56	411.28			
20.000	625.00	.00	.00	7048.00	6754.00	7054.36	.00
7055.81	384.15	9.67	698.47	344.60			
21.000	879.00	.00	.00	7072.00	6754.00	7080.56	.00
7081.53	229.60	7.90	856.49	445.74			
* 22.000	554.00	.00	.00	7092.00	6754.00	7098.61	7098.55
7100.44	540.47	10.98	643.15	290.52			
* 23.000	514.00	.00	.00	7108.00	6754.00	7117.38	.00
7119.39	266.17	12.71	694.80	413.98			

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100-YEAR FREQUENCY CROSS

SUMMARY PRINTOUT TABLE 150

	SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
*	60.000	20974.00	6733.36	.00	.00	.16	258.76	.00
	1.000	6915.00	6743.87	.00	10.52	.00	392.81	1047.00
*	2.000	6915.00	6751.95	.00	8.08	.00	176.67	759.00
*	3.000	5128.00	6774.75	.00	22.79	.00	360.65	845.00
*	4.000	6788.00	6786.82	.00	12.07	.00	366.20	984.00
*	5.000	6788.00	6793.93	.00	7.11	.00	145.32	722.00
*	6.100	6154.00	6820.42	.00	26.49	.00	21.00	1160.00
*	6.200	6154.00	6825.81	.00	5.38	.00	21.00	48.00
*	7.000	6154.00	6857.79	.00	31.99	.00	263.57	856.00
*	8.000	6154.00	6890.86	.00	33.06	.00	445.42	1026.00
*	9.000	8624.00	6896.25	.00	5.39	.00	627.35	2638.00
*	10.000	8624.00	6902.42	.00	6.17	.00	375.72	590.00
*	11.000	8624.00	6912.51	.00	10.10	.00	249.98	701.00

*	12.000	8624.00	6921.74	.00	9.23	.00	242.34	643.00
	13.000	8624.00	6937.37	.00	15.63	.00	192.27	689.00
	14.000	8624.00	6952.38	.00	15.01	.00	255.99	834.00
	15.000	8624.00	6965.44	.00	13.06	.00	268.67	764.00
	16.000	8624.00	6982.28	.00	16.84	.00	278.30	718.00
	17.000	8270.00	6997.53	.00	15.26	.00	262.59	547.00
	18.000	6754.00	7011.92	.00	14.39	.00	136.51	598.00
	19.000	6754.00	7034.79	.00	22.87	.00	248.53	800.00
	20.000	6754.00	7054.36	.00	19.57	.00	160.28	625.00
	21.000	6754.00	7080.56	.00	26.21	.00	188.29	879.00
*	22.000	6754.00	7098.61	.00	18.05	.00	181.31	554.00

1

11/ 3/ 3 12:40:22

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	SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
*	23.000	6754.00	7117.38	.00	18.77	.00	126.40	514.00

1

11/ 3/ 3 12:40:22

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SUMMARY OF ERRORS AND SPECIAL NOTES

CAUTION SECNO= 60.000 PROFILE= 1 CRITICAL DEPTH ASSUMED

CAUTION SECNO= 2.000 PROFILE= 1 CRITICAL DEPTH ASSUMED

CAUTION SECNO= 2.000 PROFILE= 1 PROBABLE MINIMUM SPECIFIC ENERGY

CAUTION SECNO= 2.000 PROFILE= 1 20 TRIALS ATTEMPTED TO BALANCE WSEL

CAUTION SECNO= 3.000 PROFILE= 1 CRITICAL DEPTH ASSUMED

CAUTION SECNO= 3.000 PROFILE= 1 PROBABLE MINIMUM SPECIFIC ENERGY

CAUTION SECNO= 3.000 PROFILE= 1 20 TRIALS ATTEMPTED TO BALANCE WSEL

CAUTION SECNO= 4.000 PROFILE= 1 CRITICAL DEPTH ASSUMED

CAUTION SECNO= 4.000 PROFILE= 1 PROBABLE MINIMUM SPECIFIC ENERGY

CAUTION SECNO= 4.000 PROFILE= 1 20 TRIALS ATTEMPTED TO BALANCE WSEL

WARNING SECNO=	5.000	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
CAUTION SECNO=	6.100	PROFILE=	1	CRITICAL DEPTH ASSUMED
CAUTION SECNO=	6.100	PROFILE=	1	PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO=	6.100	PROFILE=	1	20 TRIALS ATTEMPTED TO BALANCE WSEL
CAUTION SECNO=	6.200	PROFILE=	1	HYDRAULIC JUMP D.S.
WARNING SECNO=	6.200	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
CAUTION SECNO=	7.000	PROFILE=	1	CRITICAL DEPTH ASSUMED
CAUTION SECNO=	7.000	PROFILE=	1	PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO=	7.000	PROFILE=	1	20 TRIALS ATTEMPTED TO BALANCE WSEL
CAUTION SECNO=	8.000	PROFILE=	1	CRITICAL DEPTH ASSUMED
CAUTION SECNO=	8.000	PROFILE=	1	PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO=	8.000	PROFILE=	1	20 TRIALS ATTEMPTED TO BALANCE WSEL
WARNING SECNO=	9.000	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
CAUTION SECNO=	10.000	PROFILE=	1	CRITICAL DEPTH ASSUMED
CAUTION SECNO=	10.000	PROFILE=	1	PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION SECNO=	10.000	PROFILE=	1	20 TRIALS ATTEMPTED TO BALANCE WSEL
WARNING SECNO=	11.000	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	12.000	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	22.000	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE
WARNING SECNO=	23.000	PROFILE=	1	CONVEYANCE CHANGE OUTSIDE ACCEPTABLE RANGE

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding...

To obtain more detailed information in areas where Base Flood Elevations (BFEs) and/or Floodways have been determined, users are encouraged to consult the Flood Profiles and Floodway Data and/or Summary of Stillwater Elevations tables contained within the Flood Insurance Study (FIS) report...

Coastal Base Flood Elevations shown on this map apply only landward of 0.0' North American Vertical Datum of 1988 (NAVD83). Users of this FIRI should be aware that coastal flood elevations are also provided in the Summary of Stillwater Elevations tables in the Flood Insurance Study report for this jurisdiction.

Boundaries of the floodways were computed at cross sections and interpolated between cross sections. The floodways were based on hydraulic considerations with regard to requirements of the National Flood Insurance Program.

Coastal areas not in Special Flood Hazard Areas may be protected by flood control structures. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The horizontal datum was NAD83, GRS80 spheroid. Differences in datum, UTM zones, and/or datum used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences...

Flood elevations on this map are referenced to the North American Vertical Datum of 1988 (NAVD88). These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum.

NGS Information Services NOAA, NNGS-1 National Geodetic Survey SSMC-3 #5202 1315 East-West Highway Silver Spring, MD 20910-3282

To obtain current elevation, description, and/or location information for bench marks shown on this map, please contact the Information Services Branch of the National Geodetic Survey at (301) 713-3202 or visit its website at http://www.nga.noaa.gov.

Base Map information shown on this FIRI was provided in digital format by El Paso County, Colorado Springs Utilities, City of Fountain, Bureau of Land Management, National Oceanic and Atmospheric Administration, United States Geological Survey, and Anderson Consulting Engineers, Inc.

This map reflects more detailed and up-to-date stream channel configurations and floodplain delineations than those shown on the previous FIRI for this jurisdiction. The floodplains and floodways that were transferred from the previous FIRI may have been adjusted to conform to these new stream channel configurations.

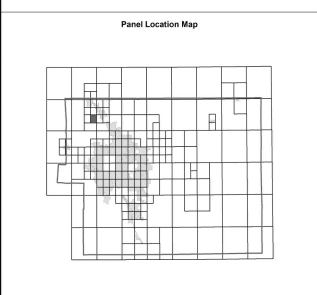
Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, map users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed Map Index for an overview map of the county showing the layout of map panels, community map repository addresses, and a Listing of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is located.

Contact FEMA Map Service Center (MSC) via the FEMA Map Information eXchange (FMIX) 1-877-336-2627 for information on available products associated with this FIRI.

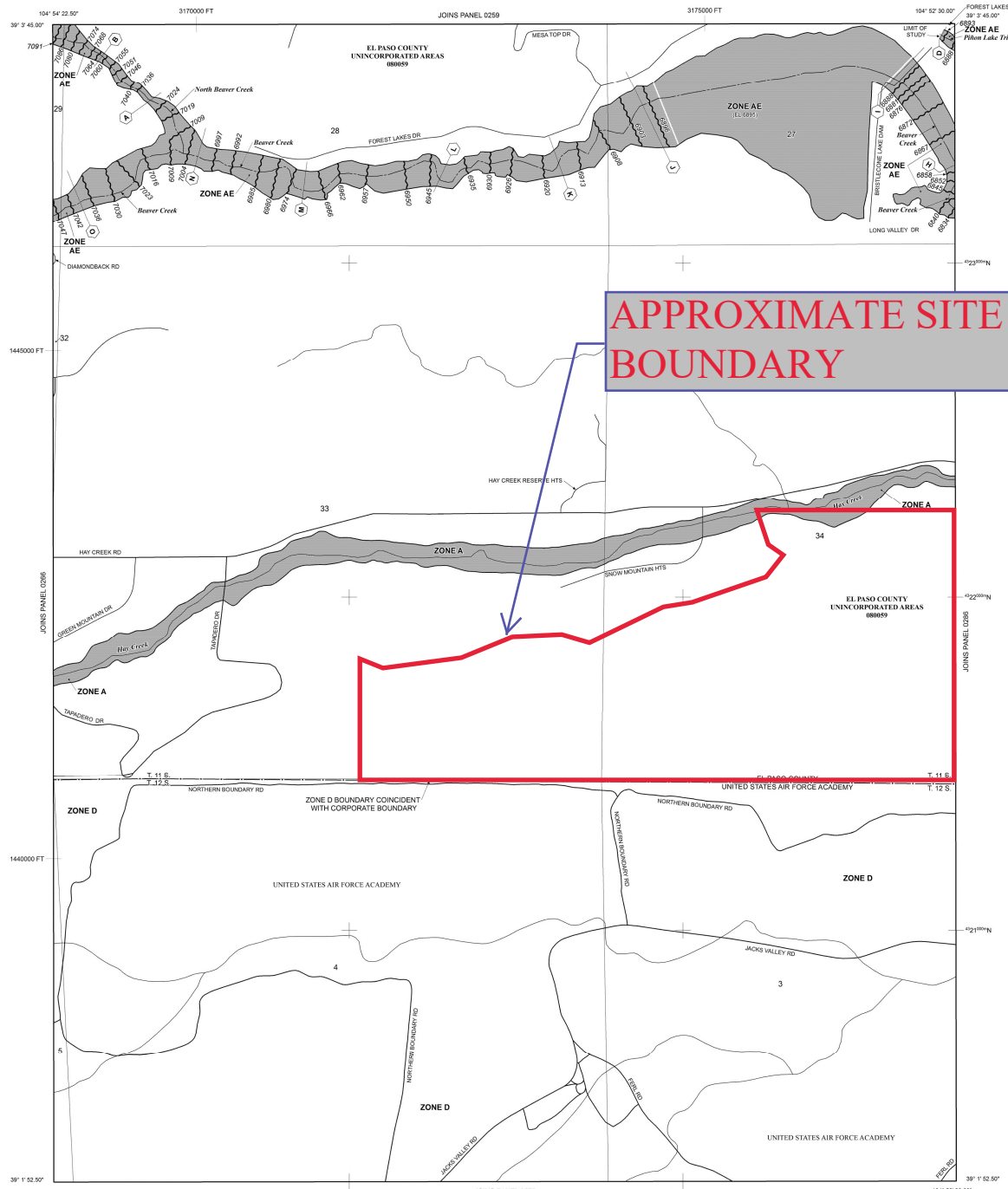
If you have questions about this map or questions concerning the National Flood Insurance Program in general, please call 1-877-FEMA MAP (1-877-336-2627) or visit the FEMA website at http://www.fema.gov.

Panel Location Map: REFER TO SECTION 3.3 OF THE EL PASO COUNTY FLOOD INSURANCE STUDY REPORT FOR STREAM BY STREAM VERTICAL DATUM CONVERSION INFORMATION



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).

Additional Flood Hazard information and resources are available from local communities and the Colorado Water Conservation Board.



APPROXIMATE SITE BOUNDARY

NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 11 SOUTH, RANGE 67 WEST, AND TOWNSHIP 12 SOUTH, RANGE 67 WEST.

LEGEND

- SPECIAL FLOOD HAZARD AREAS (SFHAs) SUBJECT TO INUNDATION BY THE 1% ANNUAL CHANCE FLOOD
ZONE A: No Base Flood Elevations determined.
ZONE AE: Base Flood Elevations determined.
ZONE AH: Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined.
ZONE AO: Flood depths of 1 to 3 feet (usually sheet flow on sloping terrain); average depths determined.
ZONE AR: Special Flood Hazard Area formerly protected from the 1% annual chance flood by a flood control system that has been abandoned.
ZONE AV: Area to be protected from 1% annual chance flood by a Federal flood protection system under construction; no Base Flood Elevations determined.
ZONE V: Coastal flood zone with velocity hazard (wave action); no Base Flood Elevations determined.
ZONE VE: Coastal flood zone with velocity hazard (wave action); Base Flood Elevations determined.
FLOODWAY AREAS IN ZONE AE
OTHER FLOOD AREAS
OTHER AREAS
COASTAL BARRIER RESOURCES SYSTEM (CBRS) AREAS
OTHERWISE PROTECTED AREAS (OPAs)
MAP REPOSITORIES

PANEL 0267G
FIRM FLOOD INSURANCE RATE MAP
EL PASO COUNTY, COLORADO AND INCORPORATED AREAS
PANEL 267 OF 1300
(SEE MAP INDEX FOR FIRM PANEL LAYOUT)
CONTAINS: COMMUNITY NUMBER: 88088 PANEL: 0267
MAP NUMBER: 08041C0267G
MAP REVISED: DECEMBER 7, 2018
Federal Emergency Management Agency



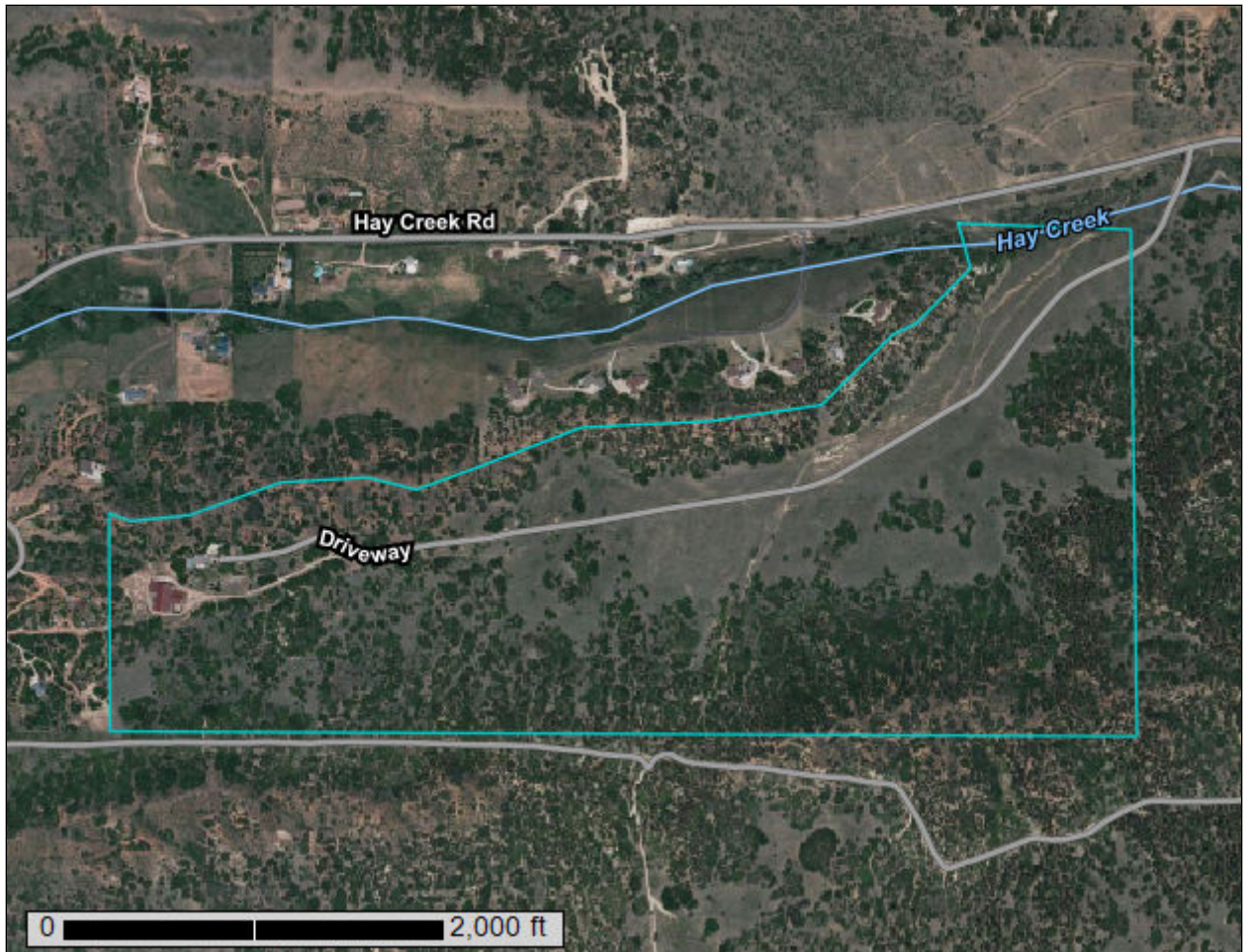
United States
Department of
Agriculture

NRCS

Natural
Resources
Conservation
Service

A product of the National
Cooperative Soil Survey,
a joint effort of the United
States Department of
Agriculture and other
Federal agencies, State
agencies including the
Agricultural Experiment
Stations, and local
participants

Custom Soil Resource Report for El Paso County Area, Colorado



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (<http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/>) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (<https://offices.sc.egov.usda.gov/locator/app?agency=nrcs>) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/?cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

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How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

Custom Soil Resource Report

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

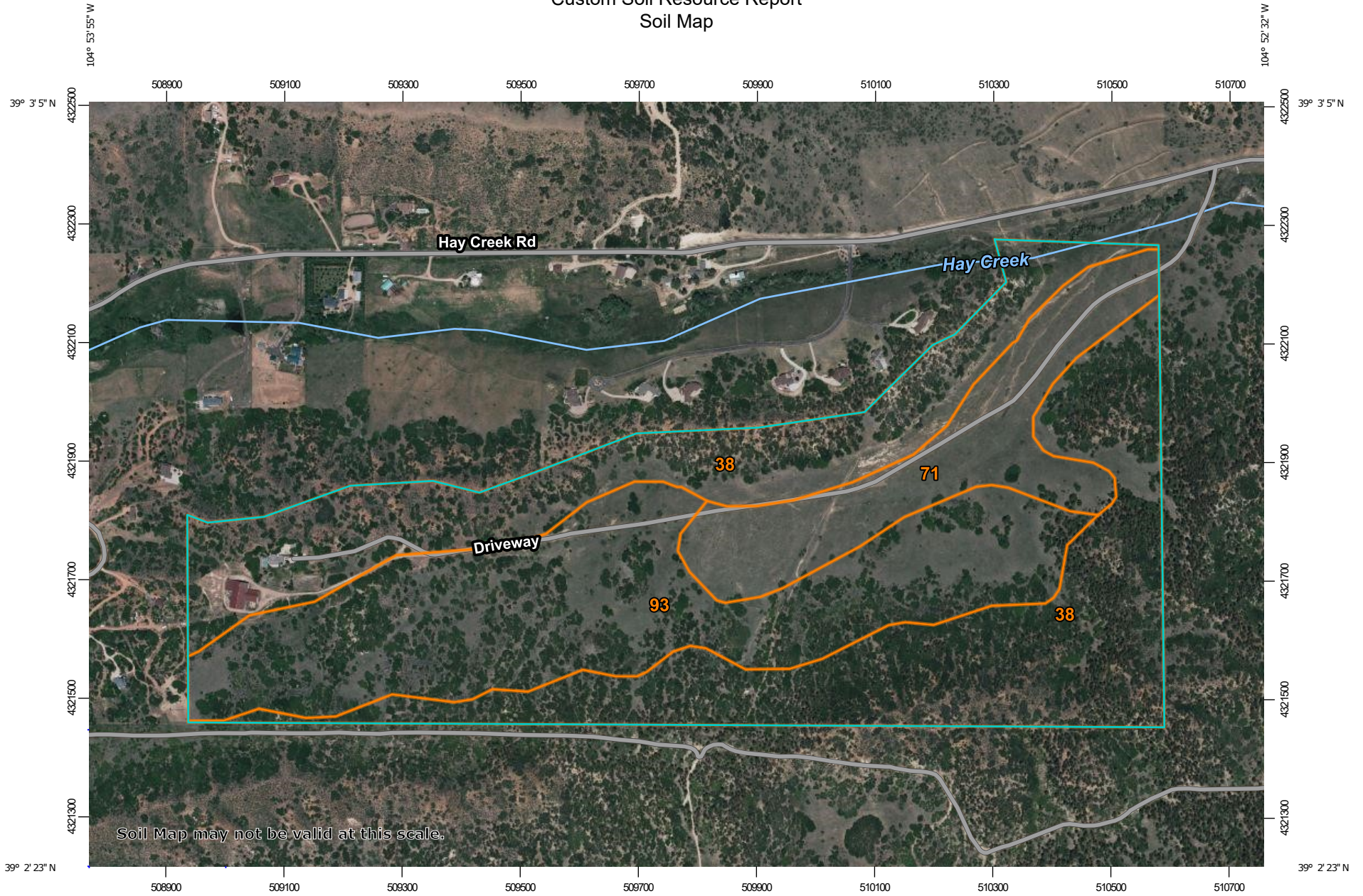
Custom Soil Resource Report

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

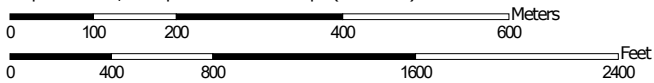
Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.

Custom Soil Resource Report Soil Map



Map Scale: 1:9,090 if printed on A landscape (11" x 8.5") sheet.



Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 13N WGS84

MAP LEGEND

Area of Interest (AOI)

 Area of Interest (AOI)

Soils

 Soil Map Unit Polygons

 Soil Map Unit Lines


 Soil Map Unit Points

Special Point Features

-  Blowout
-  Borrow Pit
-  Clay Spot
-  Closed Depression
-  Gravel Pit
-  Gravelly Spot
-  Landfill
-  Lava Flow
-  Marsh or swamp
-  Mine or Quarry
-  Miscellaneous Water
-  Perennial Water
-  Rock Outcrop
-  Saline Spot
-  Sandy Spot
-  Severely Eroded Spot
-  Sinkhole
-  Slide or Slip
-  Sodic Spot

-  Spoil Area
-  Stony Spot
-  Very Stony Spot
-  Wet Spot
-  Other
-  Special Line Features


Water Features

 Streams and Canals

Transportation

-  Rails
-  Interstate Highways
-  US Routes
-  Major Roads
-  Local Roads

Background

 Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
 Survey Area Data: Version 20, Sep 2, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 9, 2021—Jun 12, 2021

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Map Unit Legend

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
38	Jarre-Tecolote complex, 8 to 65 percent slopes	109.5	50.8%
71	Pring coarse sandy loam, 3 to 8 percent slopes	31.1	14.5%
93	Tomah-Crowfoot complex, 8 to 15 percent slopes	74.8	34.7%
Totals for Area of Interest		215.4	100.0%

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or

Custom Soil Resource Report

landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An *association* is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

El Paso County Area, Colorado

38—Jarre-Tecolote complex, 8 to 65 percent slopes

Map Unit Setting

National map unit symbol: 368c
Elevation: 6,700 to 7,500 feet
Frost-free period: 90 to 125 days
Farmland classification: Not prime farmland

Map Unit Composition

Jarre and similar soils: 40 percent
Tecolote and similar soils: 30 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Jarre

Setting

Landform: Alluvial fans
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium

Typical profile

A - 0 to 5 inches: gravelly sandy loam
Bt - 5 to 22 inches: gravelly sandy clay loam
2C - 22 to 60 inches: very gravelly sandy loam

Properties and qualities

Slope: 8 to 30 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 5.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: B
Ecological site: R048AY222CO - Loamy Park
Hydric soil rating: No

Description of Tecolote

Setting

Landform: Alluvial fans
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium

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Typical profile

A - 0 to 3 inches: very stony loam
E - 3 to 12 inches: very gravelly loamy sand
Bt - 12 to 45 inches: extremely gravelly sandy clay loam
C - 45 to 60 inches: extremely gravelly loamy sand

Properties and qualities

Slope: 8 to 65 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: High
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high
(0.20 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Very low (about 2.7 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 7e
Hydrologic Soil Group: B
Ecological site: R048AY255CO - Pine Grasslands
Hydric soil rating: No

Minor Components

Other soils

Percent of map unit:
Hydric soil rating: No

71—Pring coarse sandy loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 369k
Elevation: 6,800 to 7,600 feet
Farmland classification: Not prime farmland

Map Unit Composition

Pring and similar soils: 85 percent
Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Pring

Setting

Landform: Hills
Landform position (three-dimensional): Side slope
Down-slope shape: Linear
Across-slope shape: Linear

Custom Soil Resource Report

Parent material: Arkosic alluvium derived from sedimentary rock

Typical profile

A - 0 to 14 inches: coarse sandy loam

C - 14 to 60 inches: gravelly sandy loam

Properties and qualities

Slope: 3 to 8 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High (2.00 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None

Frequency of ponding: None

Available water supply, 0 to 60 inches: Low (about 6.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3e

Hydrologic Soil Group: B

Ecological site: R048AY222CO - Loamy Park

Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit:

Landform: Depressions

Hydric soil rating: Yes

Other soils

Percent of map unit:

Hydric soil rating: No

93—Tomah-Crowfoot complex, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 36bb

Elevation: 7,300 to 7,600 feet

Farmland classification: Not prime farmland

Map Unit Composition

Tomah and similar soils: 50 percent

Crowfoot and similar soils: 30 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Tomah

Setting

Landform: Hills, alluvial fans

Custom Soil Resource Report

Landform position (three-dimensional): Side slope, crest
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium derived from arkose and/or residuum weathered from arkose

Typical profile

A - 0 to 10 inches: loamy sand
E - 10 to 22 inches: coarse sand
Bt - 22 to 48 inches: stratified coarse sand to sandy clay loam
C - 48 to 60 inches: coarse sand

Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 4.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified
Land capability classification (nonirrigated): 6e
Hydrologic Soil Group: B
Ecological site: R049XY216CO - Sandy Divide
Hydric soil rating: No

Description of Crowfoot

Setting

Landform: Hills, alluvial fans
Landform position (three-dimensional): Side slope, crest
Down-slope shape: Linear
Across-slope shape: Linear
Parent material: Alluvium

Typical profile

A - 0 to 12 inches: loamy sand
E - 12 to 23 inches: sand
Bt - 23 to 36 inches: sandy clay loam
C - 36 to 60 inches: coarse sand

Properties and qualities

Slope: 8 to 15 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Well drained
Runoff class: Medium
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.60 to 2.00 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water supply, 0 to 60 inches: Low (about 4.7 inches)

Custom Soil Resource Report

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6e

Hydrologic Soil Group: B

Ecological site: R049XY216CO - Sandy Divide

Hydric soil rating: No

Minor Components

Other soils

Percent of map unit:

Hydric soil rating: No

Pleasant

Percent of map unit:

Landform: Depressions

Hydric soil rating: Yes

References

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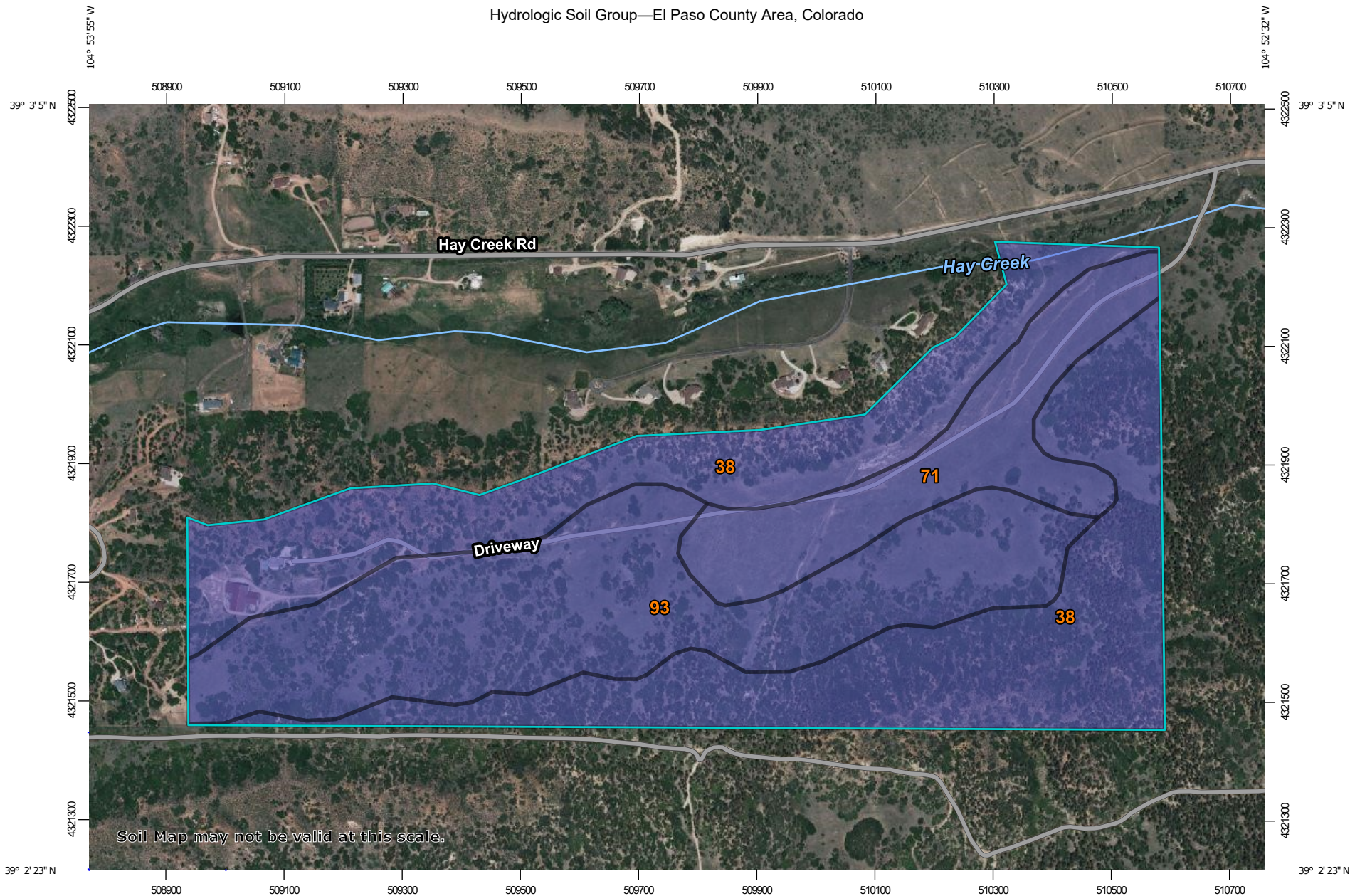
Custom Soil Resource Report

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

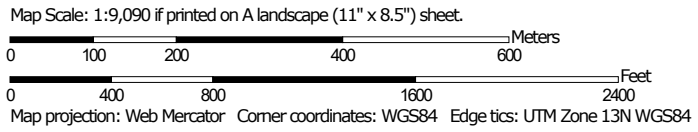
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





























Hydrologic Soil Group—El Paso County Area, Colorado



Soil Map may not be valid at this scale.



MAP LEGEND

- Area of Interest (AOI)**
 -  Area of Interest (AOI)
- Soils**
 - Soil Rating Polygons**
 -  A
 -  A/D
 -  B
 -  B/D
 -  C
 -  C/D
 -  D
 -  Not rated or not available
 - Soil Rating Lines**
 -  A
 -  A/D
 -  B
 -  B/D
 -  C
 -  C/D
 -  D
 -  Not rated or not available
 - Soil Rating Points**
 -  A
 -  A/D
 -  B
 -  B/D
- Water Features**
 -  Streams and Canals
- Transportation**
 -  Rails
 -  Interstate Highways
 -  US Routes
 -  Major Roads
 -  Local Roads
- Background**
 -  Aerial Photography
- Other**
 -  C
 -  C/D
 -  D
 -  Not rated or not available

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:24,000.

Warning: Soil Map may not be valid at this scale.
 Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
 Web Soil Survey URL:
 Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: El Paso County Area, Colorado
 Survey Area Data: Version 20, Sep 2, 2022

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 9, 2021—Jun 12, 2021

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
38	Jarre-Tecolote complex, 8 to 65 percent slopes	B	109.5	50.8%
71	Pring coarse sandy loam, 3 to 8 percent slopes	B	31.1	14.5%
93	Tomah-Crowfoot complex, 8 to 15 percent slopes	B	74.8	34.7%
Totals for Area of Interest			215.4	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

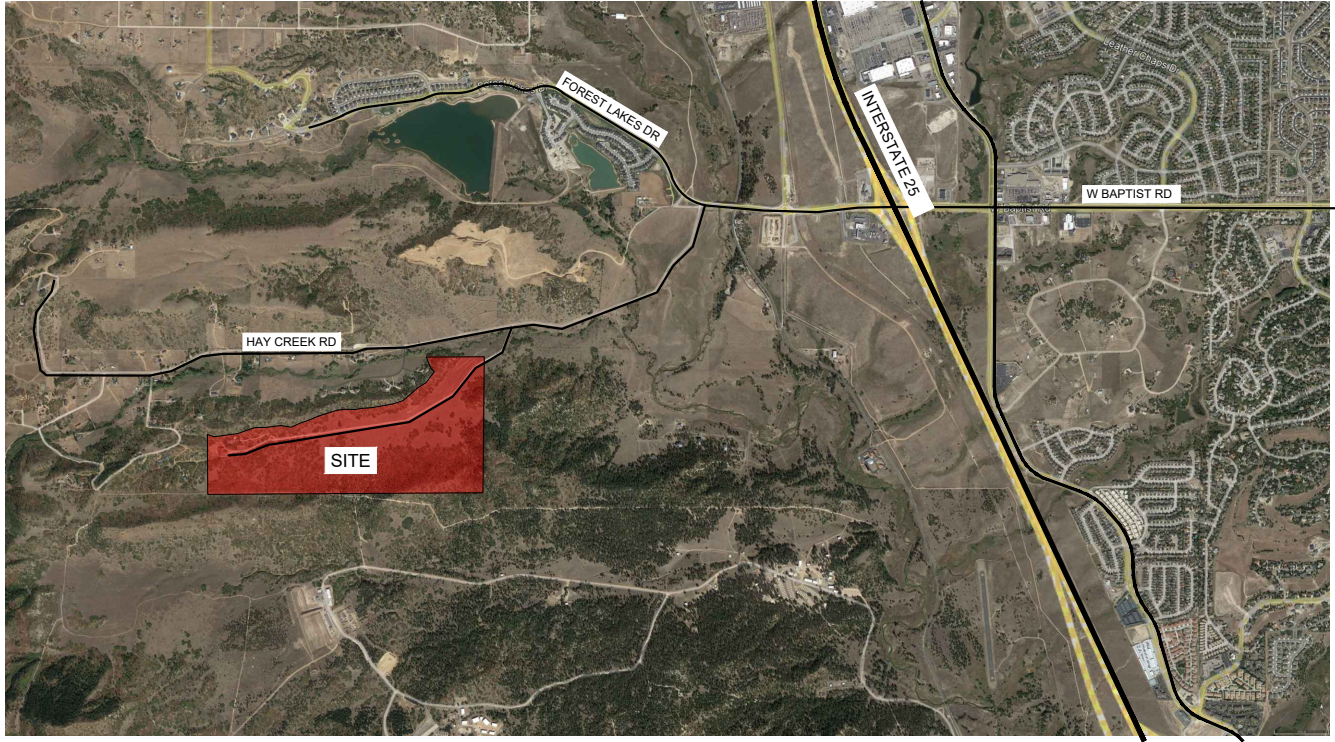
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Component Percent Cutoff: None Specified

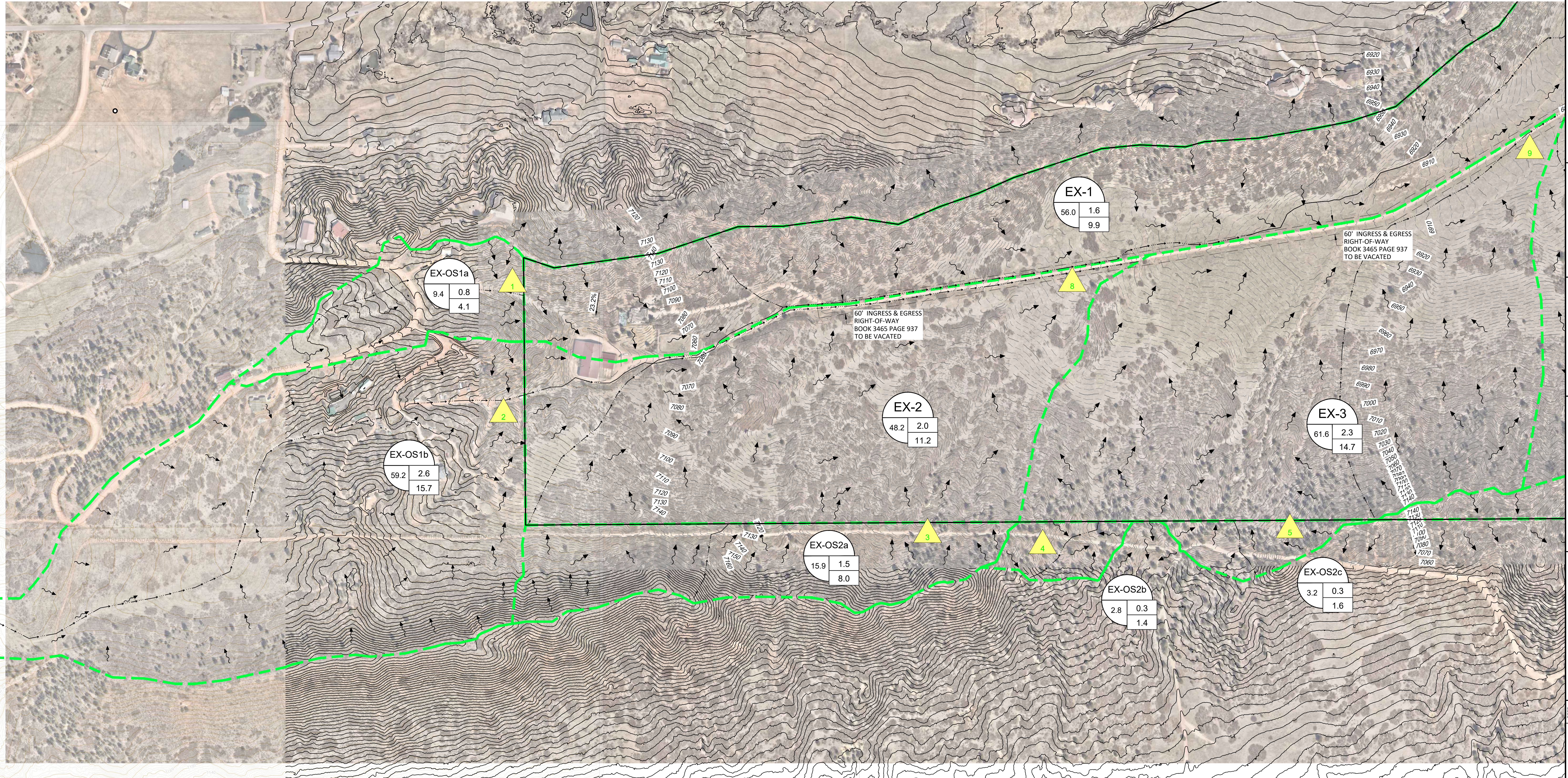
Tie-break Rule: Higher

APPENDIX D

MAPS

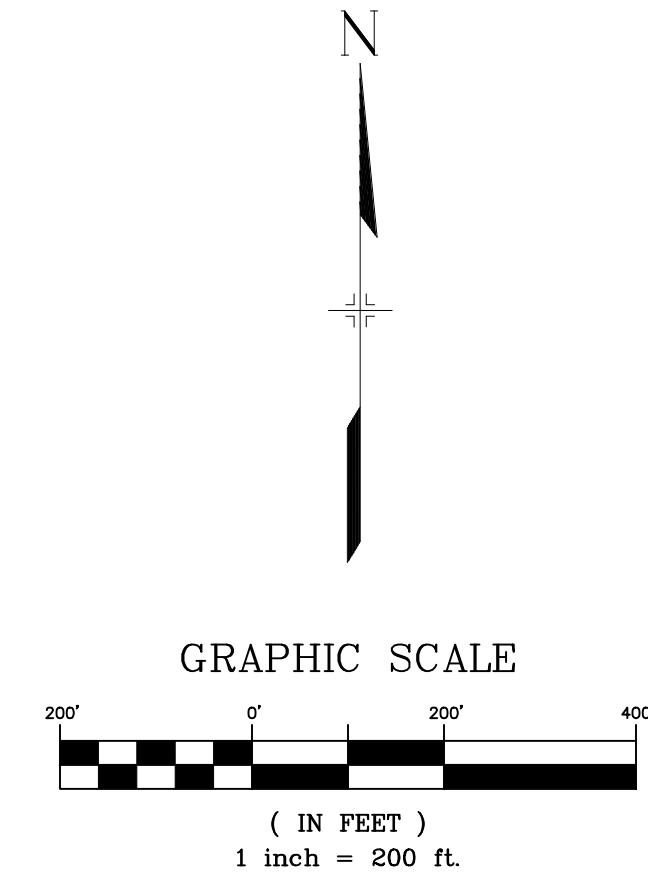


VICINITY MAP
HAY CREEK VALLEY
(NTS)



LEGEND

- BASIN BOUNDARY
- EXISTING PROPERTY LINE
- EXISTING CONTOUR
- FLOW DIRECTION
- DESIGN POINT
- SUB BASIN DESIGNATION
- 5-YEAR STORM EVENT PEAK FLOW (CFS)
- 100-YEAR STORM EVENT PEAK FLOW (CFS)
- SUB BASIN AREA (AC.)



Hay Creek Existing Conditions Sub-basin Summary			
Basin	Area		Q100
	acres	cfs	
EX-OS1a	9.4	0.8	4.1
EX-OS1b	59.2	2.6	15.7
EX-OS2a	15.9	1.5	8.0
EX-OS2b	2.8	0.3	1.4
EX-OS2c	3.2	0.3	1.6
EX-OS3	8.1	0.2	2.0
EX-OS4	0.7	1.9	3.9
EX-1	56.0	1.6	9.9
EX-2	48.2	2.0	11.2
EX-3	61.6	2.3	14.7
EX-4	5.9	2.3	15.6
EX-5	42.8	1.5	10.0

Existing Design Point Summary				
Design Point	Hay Creek		Q(5) (cfs)	Q(100) (cfs)
	Sub-Basins	Total Area (ac.)		
1	EX-OS1a	9.35	0.80	4.10
2	EX-OS1b	59.21	2.60	15.70
3	EX-OS2a	15.90	1.50	8.00
4	EX-OS2b	2.77	0.30	1.40
5	EX-OS2c	3.17	0.30	1.60
6	EX-OS3	8.15	0.20	2.00
7	EX-4	5.88	2.30	15.60
8	EX-OS1b, EX-OS2a, EX-2	123.33	5.40	30.90
9	EX-OS2b, EX-OS2c, EX-3	67.57	2.90	17.40
10	EX-OS3, EX-5	50.97	1.80	11.90
11	DP-8, DP-9, DP-10, EX-OS1a, EX-1	307.27	11.20	69.90
HC	From PPRBD Regional floodplain Administrator			127.00

EL PASO COUNTY
HAY CREEK
FINAL DRAINAGE REPORT

OFFSITE DRAINAGE BASINS

PRELIMINARY
DESIGN
FOR REVIEW AND APPROVAL BY
GOVERNING AGENCIES AND
IS SUBJECT TO CHANGE

FOR AND ON BEHALF OF
MATRIX DESIGN GROUP, INC.
PROJECT No. 22-886-076

DESIGNED BY: WCC
DRAWN BY: WCC
CHECKED BY: JTS

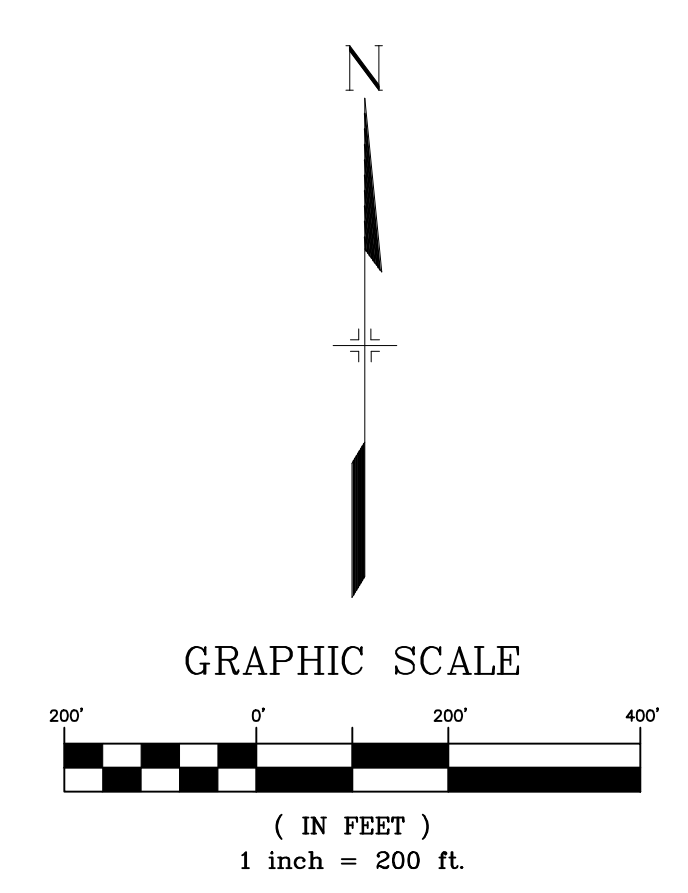
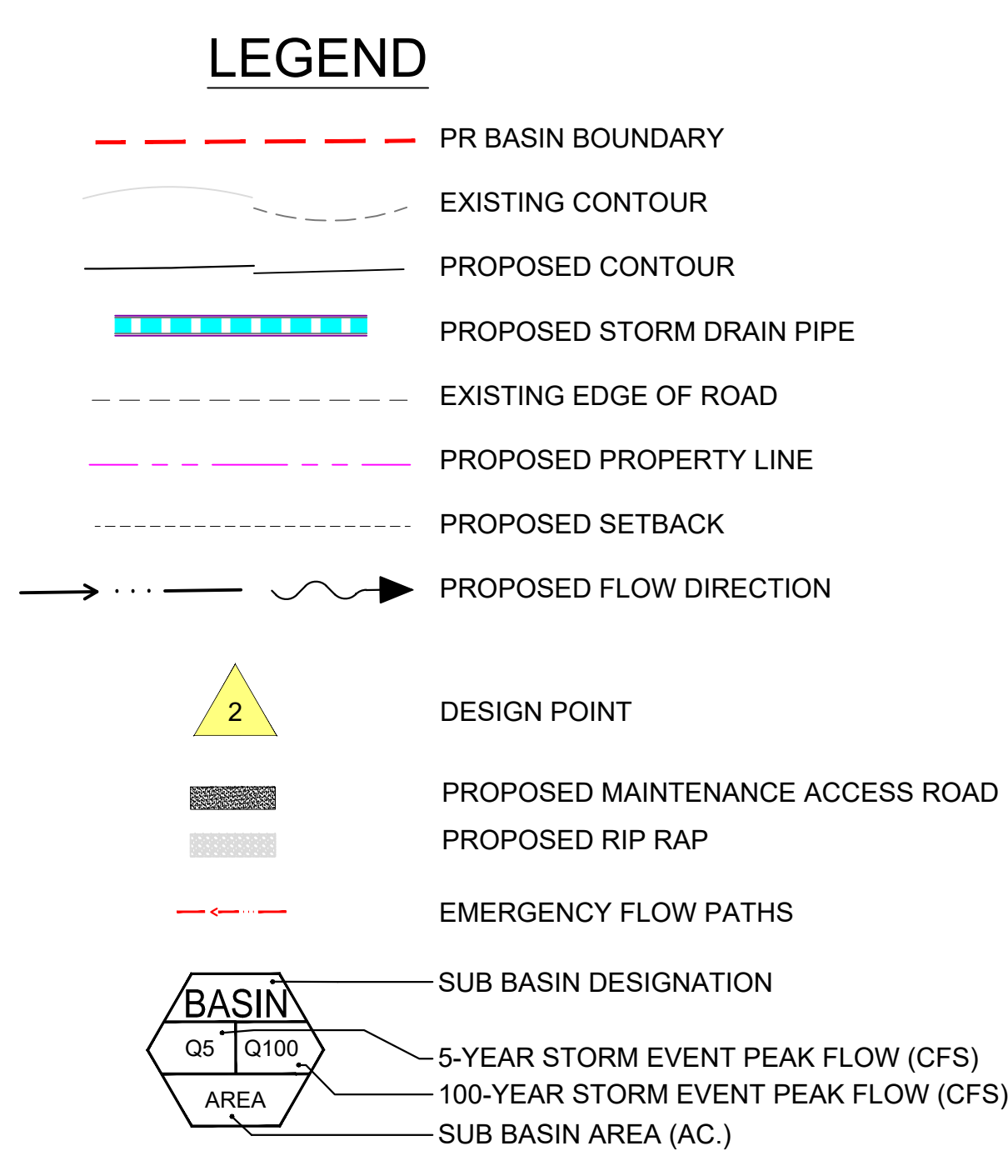
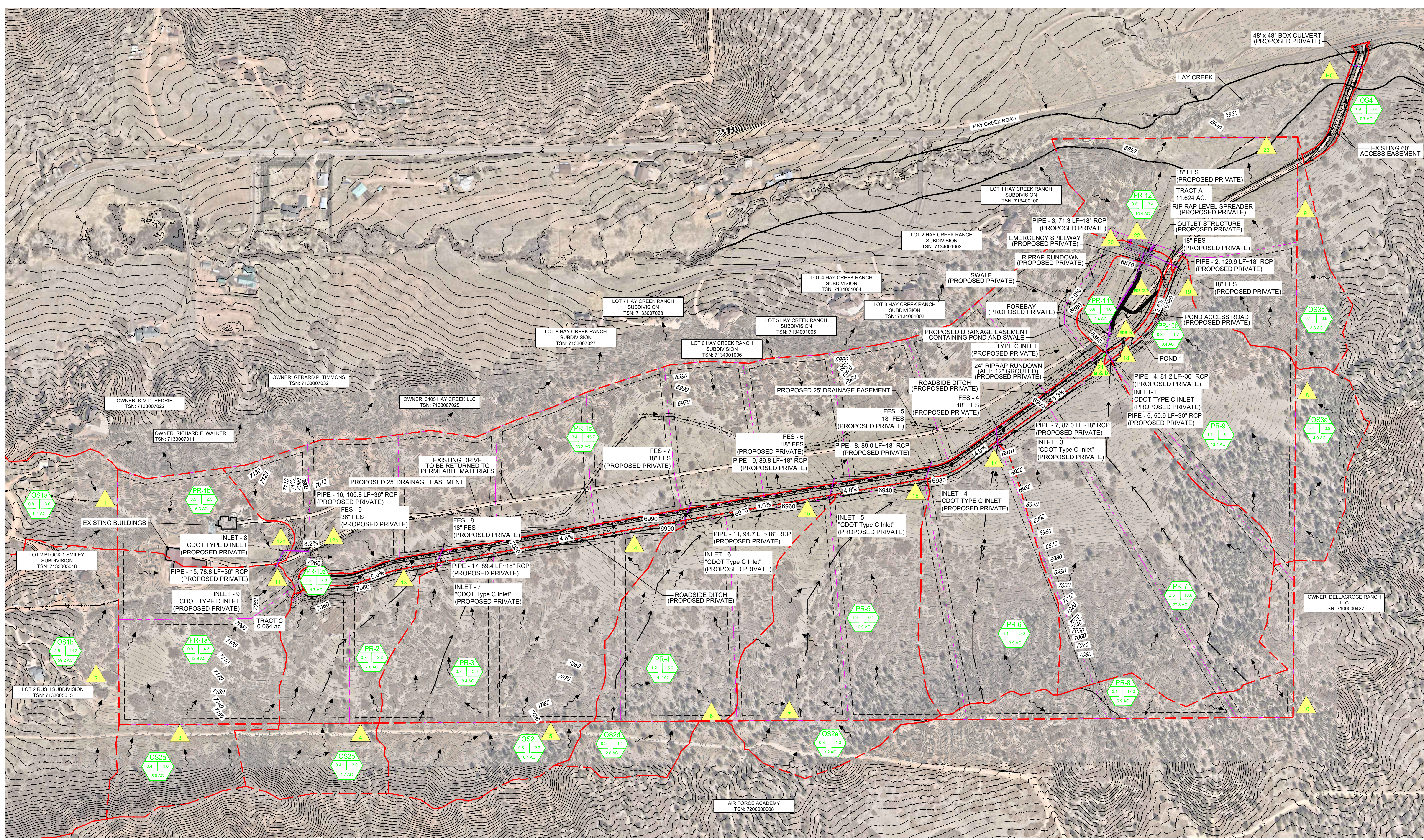
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DATE ISSUED: DECEMBER 2022
SHEET 1 OF 4
DRAWING: DR01

PREPARED BY: **Matrix**
Excellence by Design

SHEET KEY

NO.	DATE	DESCRIPTION	BY

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THIS DRAWING IS CURRENT AS OF PLOT DATE AND MAY BE SUBJECT TO CHANGE

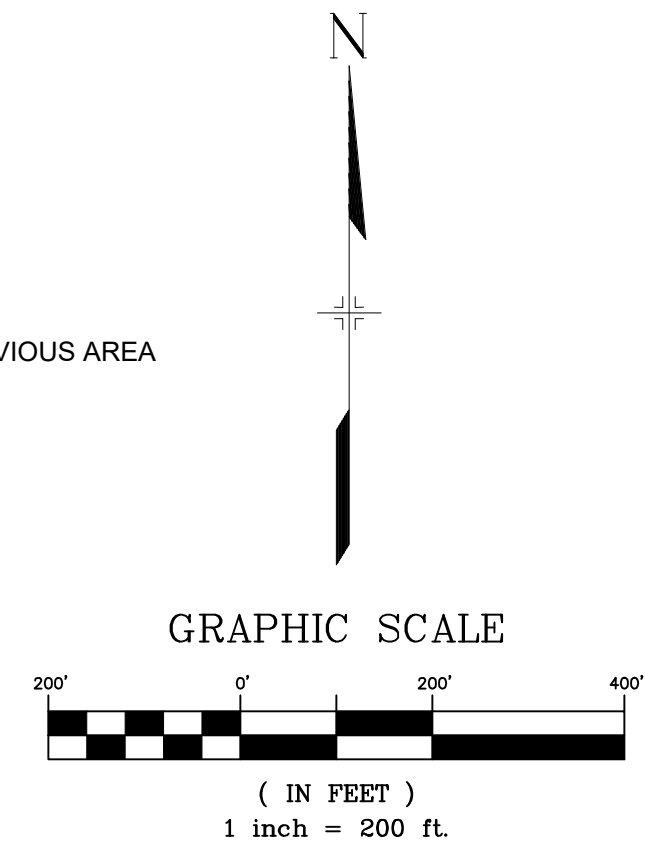
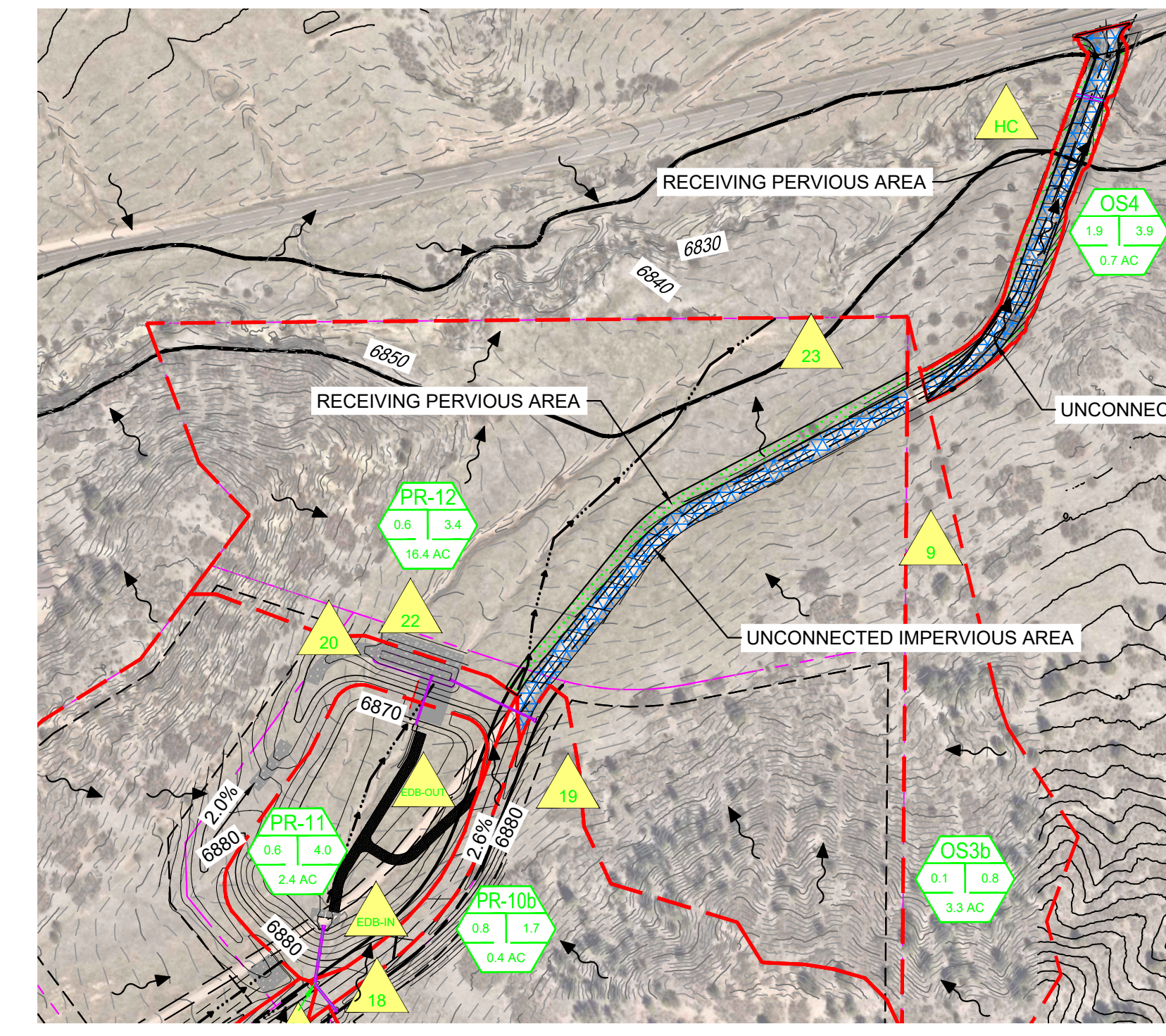
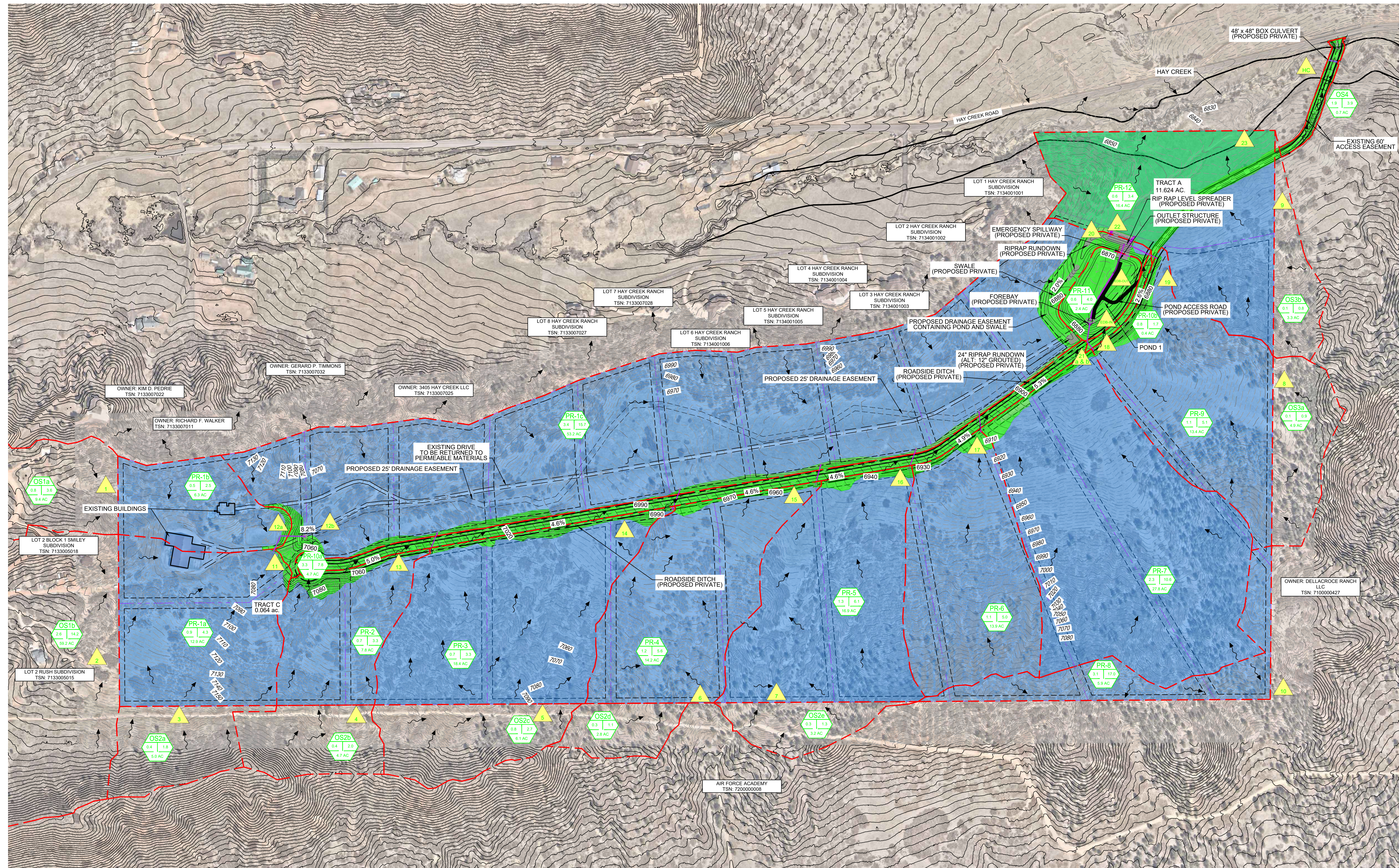


Hay Creek
 Proposed Conditions
 Sub-basin Summary

Basin	Area		Q5	Q100
	acres	cfs	cfs	cfs
OS1a	9.4	0.8	3.6	
OS1b	59.2	2.6	14.2	
OS2a	5.0	0.4	1.8	
OS2b	4.7	0.4	2.0	
OS2c	6.1	0.8	2.7	
OS2d	2.8	0.3	1.1	
OS2e	5.2	0.3	1.3	
OS3a	4.9	0.1	0.9	
OS3b	3.3	0.1	0.8	
OS4	0.7	1.9	3.9	
PR-1a	12.9	0.9	4.3	
PR-1b	6.3	0.5	2.5	
PR-1c	53.2	3.4	15.7	
PR-2	7.8	0.7	3.3	
PR-3	18.4	0.7	3.3	
PR-4	14.2	1.2	5.6	
PR-5	16.9	1.3	6.1	
PR-6	13.9	1.1	5.0	
PR-7	27.8	2.3	10.6	
PR-8	5.9	3.1	17.0	
PR-9	13.4	1.1	5.1	
PR-10a	4.7	3.3	7.8	
PR-10b	0.4	0.8	1.7	
PR-11	2.4	0.0	0.0	
PR-12	16.4	0.6	3.4	

Proposed Design Point Summary - Central Basin
 Hay Creek

Design Point	Sub-Basins	Total Area (ac.)	Q(5) (cfs)	Q(100) (cfs)
1	OS1a	9.35	0.80	3.60
2	OS1b	59.21	2.60	14.20
3	OS2a	5.01	0.40	1.80
4	OS2b	4.74	0.40	2.00
5	OS2c	6.11	0.80	2.70
6	OS2d	2.77	0.30	1.10
8	OS3a	4.88	0.10	0.90
9	OS3b	3.29	0.10	0.80
10	PR-8	5.86	3.11	17.01
11	OS1b, OS2a, PR-1a	77.15	3.70	19.50
12a	OS1a, PR-1b	15.63	1.90	6.10
12b	OS1a, OS2a, OS1b, PR-1a, PR-1b	92.78	5.00	25.20
13	OS2b, PR-2	12.51	1.10	5.30
14	OS2c, PR-3	24.55	1.40	5.50
15	OS2d, PR-4	16.96	1.50	6.60
16	OS2e, PR-5	20.09	1.60	7.20
17	PR-6	13.87	1.10	5.00
20	PR-1c, DP-12b, DP-13, DP-14, DP-15, DP-16, DP-17	233.97	9.50	43.00
21a	PR-10a	4.73	3.30	7.80
21b	PR-7, PR-10a	32.49	5.30	17.90
EDB-IN	PR-7, PR-10a, PR-10b, PR-11	35.31	4.90	16.50
EDB-OUT	PR-7, PR-10a, PR-10b, PR-11	35.31	4.00	15.90
22	EDB-OUT, DP-18, DP-19	283.15	10.90	49.10
23	DP-9, DP-21, PR-12	302.82	11.50	53.00
HC	From PPRBD Regional floodplain Administrator			127.00



LEGEND

- PR BASIN BOUNDARY
- EXISTING CONTOUR
- PROPOSED CONTOUR
- PROPOSED STORM DRAIN PIPE
- EXISTING EDGE OF ROAD
- PROPOSED SETBACK
- PROPOSED PROPERTY LINE
- PROPOSED FLOW DIRECTION
- ▲ DESIGN POINT
- PROPOSED MAINTENANCE ACCESS ROAD
- PROPOSED RIP RAP
- DISTURBED AREA
- UNDISTURBED AREA
- RECEIVING PERVIOUS AREA
- UNCONNECTED IMPERVIOUS AREA
- EMERGENCY FLOW PATHS
- BASIN SUB BASIN DESIGNATION
- OS 100 5-YEAR STORM EVENT PEAK FLOW (CFS)
- AREA 100-YEAR STORM EVENT PEAK FLOW (CFS)
- AREA SUB BASIN AREA (AC.)

Hay Creek Disturbed Area							
Basin ID	Total Area (ac)	Total Proposed Disturbed Area (ac)	Disturbed Area Tributary to Pond 1 (ac)	Disturbed Area Located within a large lot residential site (Excl. L7.1.C.1.a) (ac)	Impervious areas which are not practicable to detain (Excl. L7.1.C.1.a) (ac)	Impervious areas to receive water quality treatment via Runoff Reduction (ac)	Total Area Receiving Water Quality Treatment (ac)
OS3b	3.29	0.03	0.00	0.00	0.02	0.02	0.02
OS4	0.73	0.74	0.00	0.00	0.74	0.47	0.47
PR-1a	12.94	0.11	0.00	0.11	0.00	0.00	0.00
PR-1b	6.28	0.17	0.00	0.17	0.00	0.00	0.00
PR-1c	53.22	3.85	0.00	3.8	0.00	0.00	0.00
PR-2	7.77	0.80	0.00	0.82	0.00	0.00	0.00
PR-5	18.44	0.82	0.00	0.73	0.00	0.00	0.00
PR-4	14.19	0.40	0.00	0.54	0.00	0.00	0.00
PR-5	16.92	0.39	0.00	0.36	0.00	0.00	0.00
PR-6	13.87	0.42	0.00	0.53	0.00	0.00	0.00
PR-7	27.76	0.61	0.17	0.00	0.00	0.00	0.17
PR-8	5.86	0.00	0.00	0.00	0.00	0.00	0.00
PR-9	13.43	0.54	0.00	0.76	0.00	0.00	0.00
PR-10a	4.73	4.71	4.73	0.00	0.00	0.00	4.73
PR-10b	0.41	0.41	0.41	0.00	0.00	0.00	0.41
PR-11	2.41	2.41	2.41	0.00	0.00	0.00	2.41
PR-12	16.38	0.86	0.00	0.00	0.56	0.56	0.56
Total	215.34	17.22	7.72	7.82	1.30	1.03	8.75

NOTE: FUTURE RESIDENTIAL CONSTRUCTION FALLS UNDER THE LARGE LOT EXCLUSION FROM WQ TREATMENT, HOWEVER MUCH, IF NOT ALL, OF THE IMPROVEMENTS ASSOCIATED WITH THE RESIDENCES WILL DRAIN ACROSS VEGETATED AREAS AND UNDER THE RUNOFF REDUCTION METHODOLOGY WILL HAVE THE REQUIRED WQ TREATMENT VOLUME REDUCED TO ZERO.

EL PASO COUNTY

HAY CREEK
FINAL DRAINAGE REPORT

POST DEVELOPMENT DISTURBED AREAS

DESIGNED BY: WCC

DRAWN BY: WCC

CHECKED BY: JTS

SCALE: NAD83

DATE ISSUED: JANUARY 2024

SHEET: 4 OF 4

PROJECT NO: 22-886-076

FOR AND ON BEHALF OF
MATRIX DESIGN GROUP, INC.

PROJECT NO: 22-886-076

PRELIMINARY
THIS DRAWING HAS NOT
BEEN APPROVED BY
GOVERNING AGENCIES AND
IS SUBJECT TO CHANGE

SEAL

PREPARED BY: **Matrix**
Excellence by Design

REVISIONS

NO.	DATE	DESCRIPTION	BY

COMPUTER FILE MANAGEMENT

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