

**Sent to the State over two weeks ago and waiting for response.
See attached docs.**

From: Marc Whorton
Sent: Friday, October 30, 2020 10:34 AM
To: 'Hollister - DNR, Doug' <doug.hollister@state.co.us>; Wodiuk - DNR, Jessica <jessica.wodiuk@state.co.us>; John Hunyadi - DNR <john.hunyadi@state.co.us>
Subject: RE: Timberridge update

Hi all.

I apologize for the confusion. Please find attached the non-JD Pond forms and the SDI worksheets for the two ponds and one rain garden.

Please let me know if there is something further for me to provide for your approval.

Thanks and have a great weekend.

Marc A. Whorton, P.E. | Project Manager

719-785-2802 (Desk)

719-492-1220 (Cell)

Mwhorton@Classicconsulting.net

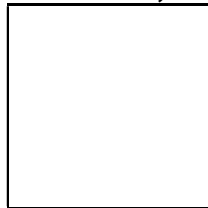
619 N. Cascade Ave. Suite 200
Colorado Springs, CO 80903



From: Hollister - DNR, Doug <doug.hollister@state.co.us>
Sent: Friday, October 30, 2020 6:33 AM
To: Wodiuk - DNR, Jessica <jessica.wodiuk@state.co.us>; John Hunyadi - DNR <john.hunyadi@state.co.us>; Marc Whorton <MWhorton@classicconsulting.net>
Subject: Re: Timberridge update

We have not received all the information needed to evaluate these structures from classic consulting.

Doug Hollister
District 10 Water Commissioner
North Regional Team Leader
Districts 10, 14, and 15



P 719.227.5291 | C 719.338.2012
4255 Sinton Rd., Colorado Springs, CO 80907
doug.hollister@state.co.us | dwr.colorado.gov



NON-JURISDICTIONAL WATER IMPOUNDMENT STRUCTURE¹

This notice is required per Section 37-87-125, C.R.S. (1998) and must be submitted to the Division Engineer's Office a minimum of 45 days prior to construction.

OWNER INFORMATION

Name: Timberidge Dev Group LLC Telephone/E-Mail: (719) 592-9333

Address: 2138 Flying Horse Club Dr. Colo. Spgs. CO 80921
 Street / P.O. Box/ Rural Route City State Zip Code

Responsible Person: Loren Moreland Telephone/E-Mail: (719) 592-9333

Address: 2138 Flying Horse Club Dr. Colo. Spgs. CO 80921
 Street / P.O. Box/ Rural Route City State Zip Code

Contractor: Kempton Const. Telephone/E-Mail: (719) 597-5319

STRUCTURE INFORMATION

Name of Dam: Timberidge Rain Garden Water Division: 2 Water District: 10

Location: (Provide Section, Township, Range, **and** GPS Point taken at crest of dam above streamline/outlet)

- Section: 27, Township: 12S, Range: 65W, 6th P.M.

- Northing 38 58' 29.97" meters, Easting 104 39' 46.87" meters (Datum should be UTM, NAD 83)

Dam Dimensions:

- Vertical Height²: 4.5 ft., Length: 90 ft., Crest Width: 6 ft., Slopes: U/S: 4 (H:1V), D/S 4 (H:1V)

Reservoir:

- Surface Area¹: 0.12 acres, Capacity¹: 0.16 acre-feet, Drainage Area*: 2.7 acres

*(If drainage area is unknown leave blank and a spillway size will be assigned):

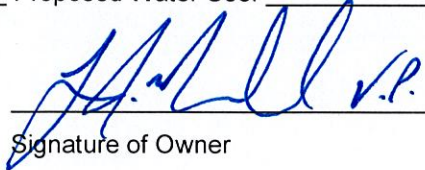
Emergency Spillway: (See Table 1, Spillway Sizing Guidelines)

- Bottom Width: 10 ft., Side Slopes: 3:1 H:1V, Freeboard³: 1.0 ft

Outlet Conduit Type: RCP, Size: 12 inches, Location: Into natural channel

Stream Name or Water Source⁴: Sand Creek Proposed Water Use: Detention/SWQ

Water Court Case or WDID : _____
 (Water District Identification Number)

 v.p. 8/22/2020
 Signature of Owner Date

Office Use Only

DIVISION ENGINEER'S REQUIREMENTS: _____

Dam I.D. _____ **Signature of Division Engineer** _____ **Date** _____

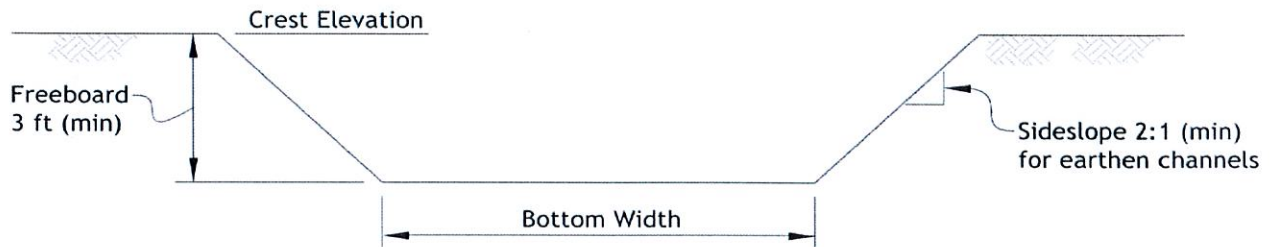
¹ A "Non-Jurisdictional Structure" is a dam creating a reservoir with a capacity of 100 acre-feet or less and a surface area of 20 acres or less and a vertical height (footnote 2) of 10 feet or less. Non-jurisdictional size dams are regulated and subject to the authority of the State Engineer consistent with sections 37-87-102 and 37-87-105 C.R.S.
² "Vertical Height" is measured from the elevation of the lowest point of the natural surface of the ground or the invert of the outlet conduit (whichever is lower) where that point occurs along the longitudinal centerline of the dam up to the crest of the emergency spillway of the dam.
³ "Freeboard" is the vertical distance from the bottom of spillway to the crest of the dam. Minimum Freeboard is 3 feet.
⁴ If construction in reservoir intercepts groundwater, a well permit is required. (Well permit applications can be found at www.water.state.co.us)



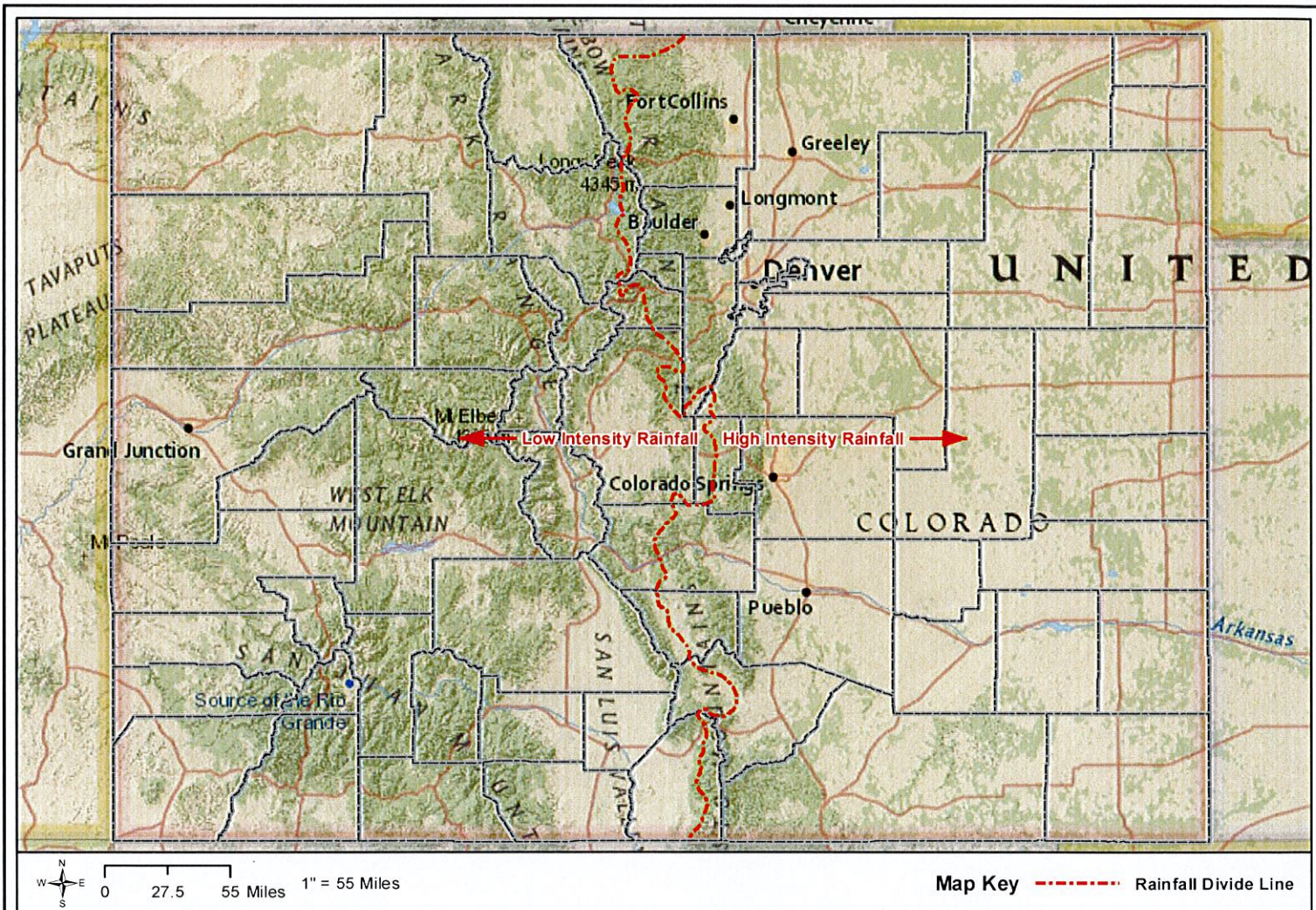
Table 1 DAM SAFETY BRANCH Spillway Sizing Guidelines for Non-Jurisdictional Dams

Drainage Area (Acres)	Minimum Recommended Bottom Width ¹ (Feet) Low Intensity Rainfall Zone	Minimum Recommended Bottom Width ¹ (Feet) High Intensity Rainfall Zone
175	8	8
225	8	10
275	8	12
325	8	15
375	10	17
425	11	19
475	12	21
525	13	24
575	15	26
625	16	28
675	17	30
725	19	33
775	20	35
825	21	37
875	22	39
925	24	42
975	25	44
1025	26	46
1075	28	48
1125	29	51
1175	30	53
1225	31	55
1275	33	57
1325	34	59
1375	35	62
1425	37	64
1475	38	66

¹Minimum recommended bottom width for drainage areas less than 175 acres is 8 feet



Spillway Section



COLORADO
 Division of Water Resources
 Department of Natural Resources

Rainfall Intensity Zones for Non-Jurisdictional Dam Spillway Sizing



DAM SAFETY BRANCH Specifications for Construction of Non-Jurisdictional Dams

- Site Selection:

- Foundation soils should be firm to provide adequate support for the embankment and should have low permeability to allow for water retention. Site selection should consider potential downstream property damage in the event of a dam failure. Construction of dams in boggy areas, areas with non-uniform fractured rock, or sands/gravels is not recommended and an engineer should be hired to evaluate the site conditions. Any part of the reservoir basin excavated below grade cannot expose groundwater.

- Embankment Design:

- Backfill material to be used for construction of the cutoff trench and embankment should be a suitable clay material and contain no material larger than 6 inches in diameter.
- The upstream slope should be constructed with a slope no steeper than 3:1, and the downstream slope should be no steeper than 2:1 (see cross section below). The dam crest should have a minimum width of 10 feet and the surface should be graded with positive drainage toward the reservoir basin.
- It is recommended that rock rip rap or other suitable material be placed on the upstream slope of the embankment to protect it from wave action. A suitable gravel or geosynthetic material should be placed under the rip rap to prevent fine material from washing out from behind the larger rock.
- The embankment should be fenced to restrict livestock from accessing the dam since they damage the protective vegetation and increase erosion.

- Embankment Construction

- The topsoil and all organic material should be removed from the foundation of the proposed dam site. Organic soil should only be reused for placement on the completed embankment to promote the re-growth of vegetation.
- A cutoff trench should be excavated under the full length of the centerline of the dam with sloping sides (1:1 min.), a minimum bottom width of 3 feet and a depth of 3 feet.
- The foundation of the dam should be scarified/ripped to a depth of 6-inches to provide proper contact between the native foundation and embankment. This surface should then be moisture treated before placement of fill.
- Fill material should be placed in layers not exceeding 12 inches in thickness prior to compaction. Suitable backfill material should have enough clay and moisture content to roll a small ball by hand. If this cannot be done, the soil is likely too dry or does not have adequate clay content.
- Each lift should be thoroughly compacted using a sheeps foot compactor. Care should be taken not to allow the top layers of the soil to dry out between placement of lifts.
- Fill should be placed in uniform lifts that cover the entire embankment length and width.

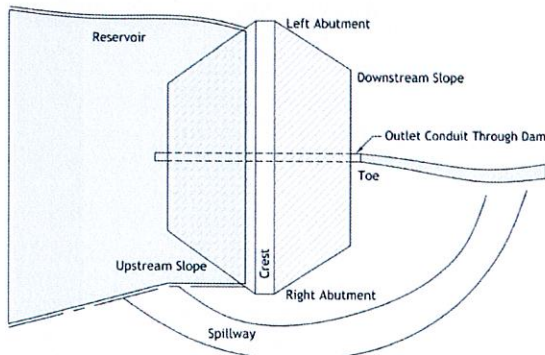
- Outlet

- Unless a waiver is granted in writing by the Division Engineer, all non-jurisdictional dams require an outlet conduit positioned at the natural low point of the reservoir basin. A minimum diameter of 12 inches is recommended and should be controlled at the upstream end by a valve and trash rack.

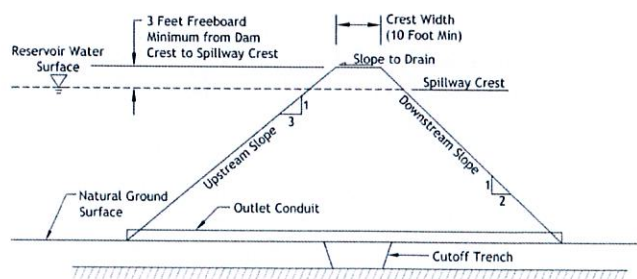
- Emergency Spillway

- The spillway should have sufficient width to provide capacity to route the runoff from the drainage basin above the dam during rainfall/runoff events.
- The emergency spillway should be located on natural ground far enough away to prevent erosion of the dam embankment. A spillway over the dam embankment is not acceptable.
- A minimum of 3 feet of freeboard is required from the bottom of the emergency spillway to the top of the dam.
- To determine the minimum spillway width, see the attached table for your area and drainage basin size.

- Example Plan View and Cross Section



Plan View



Cross Section Through Dam at Outlet



NON-JURISDICTIONAL WATER IMPOUNDMENT STRUCTURE¹

This notice is required per Section 37-87-125, C.R.S. (1998) and must be submitted to the Division Engineer's Office a minimum of 45 days prior to construction.

OWNER INFORMATION

Name: Timberridge Dev Group LLC Telephone/E-Mail: (719)592-9333

Address: 2138 Flying Horse Club Dr. Colo. Spgs. CO 80921
 Street / P.O. Box/ Rural Route City State Zip Code

Responsible Person: Loren Moreland Telephone/E-Mail: (719)592-9333

Address: 2138 Flying Horse Club Dr. Colo. Spgs. CO 80921
 Street / P.O. Box/ Rural Route City State Zip Code

Contractor: Kempton Const. Telephone/E-Mail: (719)597-5319

STRUCTURE INFORMATION

Name of Dam: Timberridge Pond 2 Water Division: 2 Water District: 10

Location: (Provide Section, Township, Range, **and** GPS Point taken at crest of dam above streamline/outlet)

- Section: 27, Township: 12S, Range: 65W, 6th P.M.

- Northing 38 58'23.87" meters, Easting 104 39'45.07" meters (Datum should be UTM, NAD 83)

Dam Dimensions:

- Vertical Height²: 10.0 ft., Length: 190 ft., Crest Width: 10 ft., Slopes: U/S: 4 (H:1V), D/S 4 (H:1V)

Reservoir:

- Surface Area¹: 1.1 acres, Capacity¹: 5.7 acre-feet, Drainage Area*: 100.4 acres

**(If drainage area is unknown leave blank and a spillway size will be assigned):*

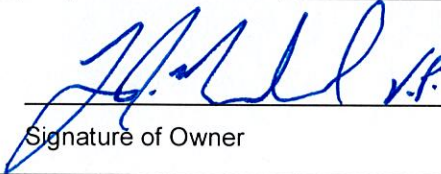
Emergency Spillway: (See Table 1, Spillway Sizing Guidelines)

- Bottom Width: 65 ft., Side Slopes: 3:1 H:1V, Freeboard³: 2.0 ft

Outlet Conduit Type: RCP, Size: 42 inches, Location: Into natural channel

Stream Name or Water Source⁴: Sand Creek Proposed Water Use: Detention/SWQ

Water Court Case or WDID : _____
 (Water District Identification Number)

 8/22/2020
 Signature of Owner Date

Office Use Only

DIVISION ENGINEER'S REQUIREMENTS: _____

Dam I.D. _____ Signature of Division Engineer _____ Date _____

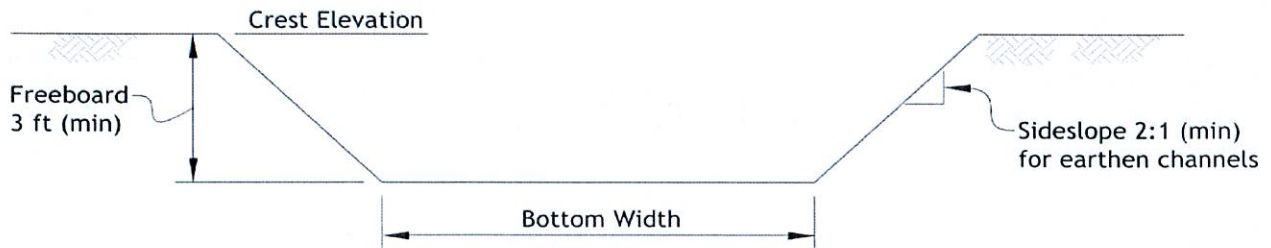
¹ A "Non-Jurisdictional Structure" is a dam creating a reservoir with a capacity of 100 acre-feet or less *and* a surface area of 20 acres or less *and* a vertical height (footnote 2) of 10 feet or less. Non-jurisdictional size dams are regulated and subject to the authority of the State Engineer consistent with sections 37-87-102 and 37-87-105 C.R.S.
² "Vertical Height" is measured from the elevation of the lowest point of the natural surface of the ground or the invert of the outlet conduit (whichever is lower) where that point occurs along the longitudinal centerline of the dam up to the crest of the emergency spillway of the dam.
³ "Freeboard" is the vertical distance from the bottom of spillway to the crest of the dam. Minimum Freeboard is 3 feet.
⁴ If construction in reservoir intercepts groundwater, a well permit is required. (Well permit applications can be found at www.water.state.co.us)



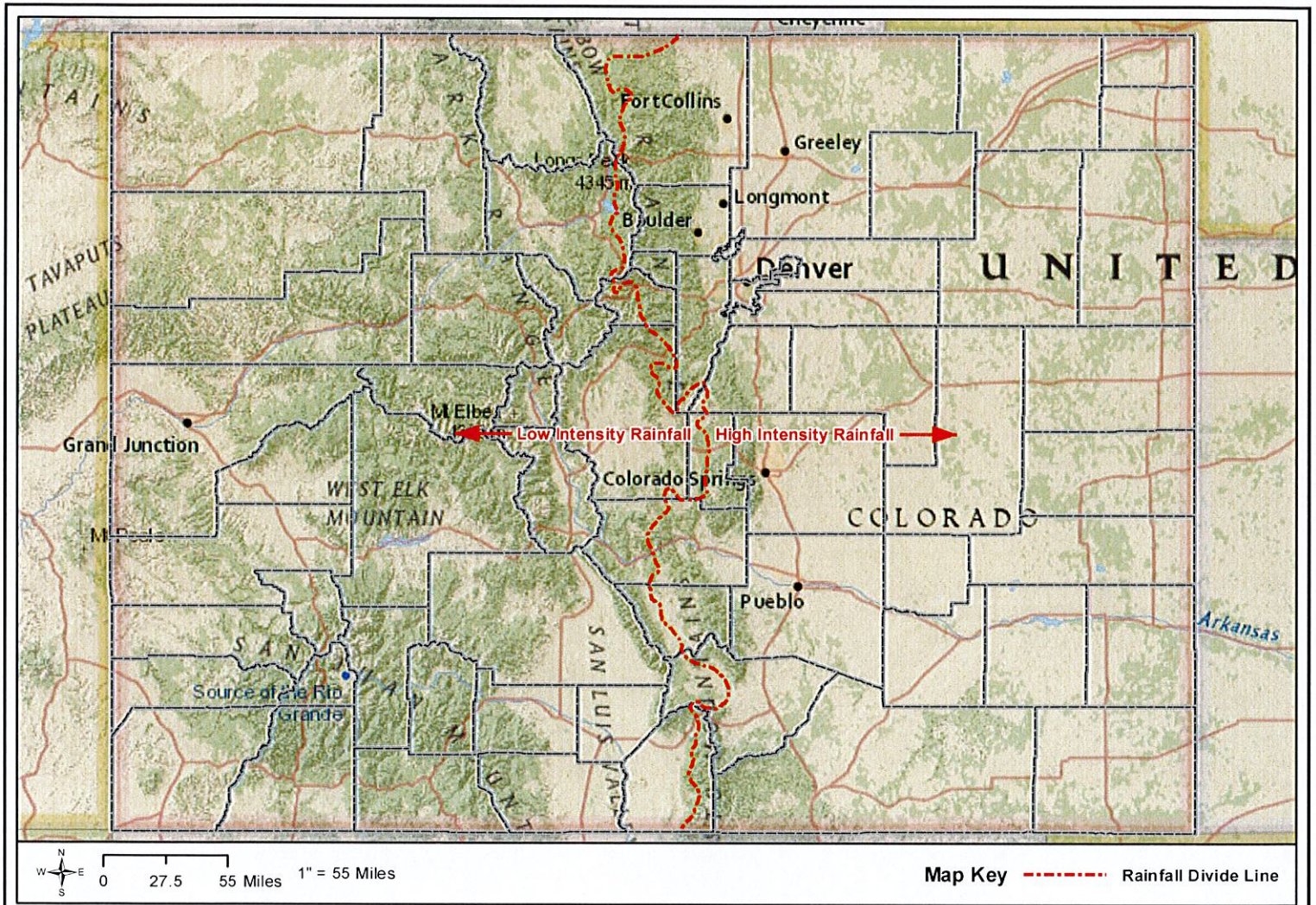
Table 1 DAM SAFETY BRANCH Spillway Sizing Guidelines for Non-Jurisdictional Dams

Drainage Area (Acres)	Minimum Recommended Bottom Width ¹ (Feet) Low Intensity Rainfall Zone	Minimum Recommended Bottom Width ¹ (Feet) High Intensity Rainfall Zone
175	8	8
225	8	10
275	8	12
325	8	15
375	10	17
425	11	19
475	12	21
525	13	24
575	15	26
625	16	28
675	17	30
725	19	33
775	20	35
825	21	37
875	22	39
925	24	42
975	25	44
1025	26	46
1075	28	48
1125	29	51
1175	30	53
1225	31	55
1275	33	57
1325	34	59
1375	35	62
1425	37	64
1475	38	66

¹Minimum recommended bottom width for drainage areas less than 175 acres is 8 feet



Spillway Section



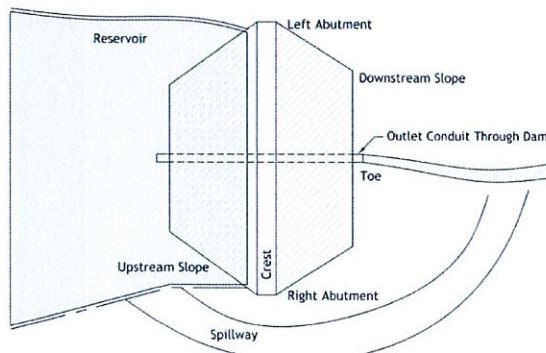
COLORADO
 Division of Water Resources
 Department of Natural Resources

Rainfall Intensity Zones for Non-Jurisdictional Dam Spillway Sizing

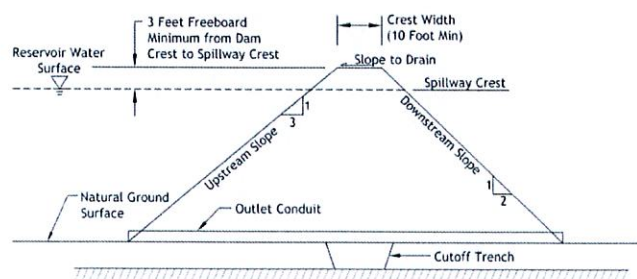


DAM SAFETY BRANCH Specifications for Construction of Non-Jurisdictional Dams

- Site Selection:
 - Foundation soils should be firm to provide adequate support for the embankment and should have low permeability to allow for water retention. Site selection should consider potential downstream property damage in the event of a dam failure. Construction of dams in boggy areas, areas with non-uniform fractured rock, or sands/gravels is not recommended and an engineer should be hired to evaluate the site conditions. Any part of the reservoir basin excavated below grade cannot expose groundwater.
- Embankment Design:
 - Backfill material to be used for construction of the cutoff trench and embankment should be a suitable clay material and contain no material larger than 6 inches in diameter.
 - The upstream slope should be constructed with a slope no steeper than 3:1, and the downstream slope should be no steeper than 2:1 (see cross section below). The dam crest should have a minimum width of 10 feet and the surface should be graded with positive drainage toward the reservoir basin.
 - It is recommended that rock rip rap or other suitable material be placed on the upstream slope of the embankment to protect it from wave action. A suitable gravel or geosynthetic material should be placed under the rip rap to prevent fine material from washing out from behind the larger rock.
 - The embankment should be fenced to restrict livestock from accessing the dam since they damage the protective vegetation and increase erosion.
- Embankment Construction
 - The topsoil and all organic material should be removed from the foundation of the proposed dam site. Organic soil should only be reused for placement on the completed embankment to promote the re-growth of vegetation.
 - A cutoff trench should be excavated under the full length of the centerline of the dam with sloping sides (1:1 min.), a minimum bottom width of 3 feet and a depth of 3 feet.
 - The foundation of the dam should be scarified/ripped to a depth of 6-inches to provide proper contact between the native foundation and embankment. This surface should then be moisture treated before placement of fill.
 - Fill material should be placed in layers not exceeding 12 inches in thickness prior to compaction. Suitable backfill material should have enough clay and moisture content to roll a small ball by hand. If this cannot be done, the soil is likely too dry or does not have adequate clay content.
 - Each lift should be thoroughly compacted using a sheeps foot compactor. Care should be taken not to allow the top layers of the soil to dry out between placement of lifts.
 - Fill should be placed in uniform lifts that cover the entire embankment length and width.
- Outlet
 - Unless a waiver is granted in writing by the Division Engineer, all non-jurisdictional dams require an outlet conduit positioned at the natural low point of the reservoir basin. A minimum diameter of 12 inches is recommended and should be controlled at the upstream end by a valve and trash rack.
- Emergency Spillway
 - The spillway should have sufficient width to provide capacity to route the runoff from the drainage basin above the dam during rainfall/runoff events.
 - The emergency spillway should be located on natural ground far enough away to prevent erosion of the dam embankment. A spillway over the dam embankment is not acceptable.
 - A minimum of 3 feet of freeboard is required from the bottom of the emergency spillway to the top of the dam.
 - To determine the minimum spillway width, see the attached table for your area and drainage basin size.
- Example Plan View and Cross Section



Plan View



Cross Section Through Dam at Outlet



NON-JURISDICTIONAL WATER IMPOUNDMENT STRUCTURE¹

This notice is required per Section 37-87-125, C.R.S. (1998) and must be submitted to the Division Engineer's Office a minimum of 45 days prior to construction.

OWNER INFORMATION

Name: Timberridge Dev Group LLC Telephone/E-Mail: (719)592-9333

Address: 2138 Flying Horse Club Dr. Colo. Spgs. CO 80921
 Street / P.O. Box/ Rural Route City State Zip Code

Responsible Person: Loren Moreland Telephone/E-Mail: (719)592-9333

Address: 2138 Flying Horse Club Dr. Colo. Spgs. CO 80921
 Street / P.O. Box/ Rural Route City State Zip Code

Contractor: Kempton Const. Telephone/E-Mail: (719)597-5319

STRUCTURE INFORMATION

Name of Dam: Timberridge Pond 1 Water Division: 2 Water District: 10

Location: (Provide Section, Township, Range, **and** GPS Point taken at crest of dam above streamline/outlet)

- Section: 28, Township: 12S, Range: 65W, 6th P.M.
- Northing 38 58'36.79" meters, Easting 104 39'51.48" meters (Datum should be UTM, NAD 83)

Dam Dimensions:

- Vertical Height²: 9.5 ft., Length: 200 ft., Crest Width: 10 ft., Slopes: U/S: 4 (H:1V), D/S 4 (H:1V)

Reservoir:

- Surface Area¹: 0.50 acres, Capacity¹: 1.3 acre-feet, Drainage Area*: 29.4 acres
 *(If drainage area is unknown leave blank and a spillway size will be assigned):

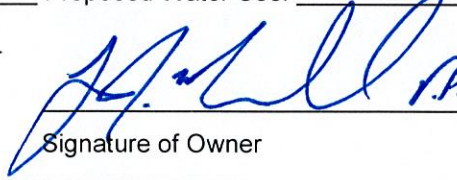
Emergency Spillway: (See Table 1, Spillway Sizing Guidelines)

- Bottom Width: 30 ft., Side Slopes: 3:1 H:1V, Freeboard³: 1.0 ft

Outlet Conduit Type: RCP, Size: 18 inches, Location: Into natural channel

Stream Name or Water Source⁴: Sand Creek Proposed Water Use: Detention/SWQ

Water Court Case or WDID : _____
 (Water District Identification Number)

 8/22/2020
 Signature of Owner Date

Office Use Only

DIVISION ENGINEER'S REQUIREMENTS: _____

Dam I.D. _____ Signature of Division Engineer Date

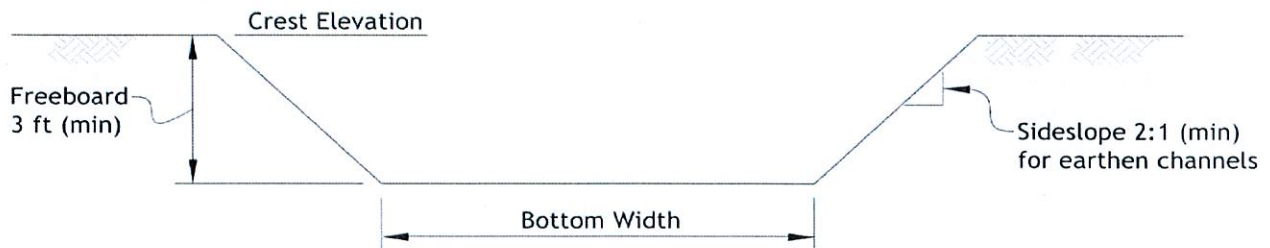
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² "Vertical Height" is measured from the elevation of the lowest point of the natural surface of the ground or the invert of the outlet conduit (whichever is lower) where that point occurs along the longitudinal centerline of the dam up to the crest of the emergency spillway of the dam.
³ "Freeboard" is the vertical distance from the bottom of spillway to the crest of the dam. Minimum Freeboard is 3 feet.
⁴ If construction in reservoir intercepts groundwater, a well permit is required. (Well permit applications can be found at www.water.state.co.us)



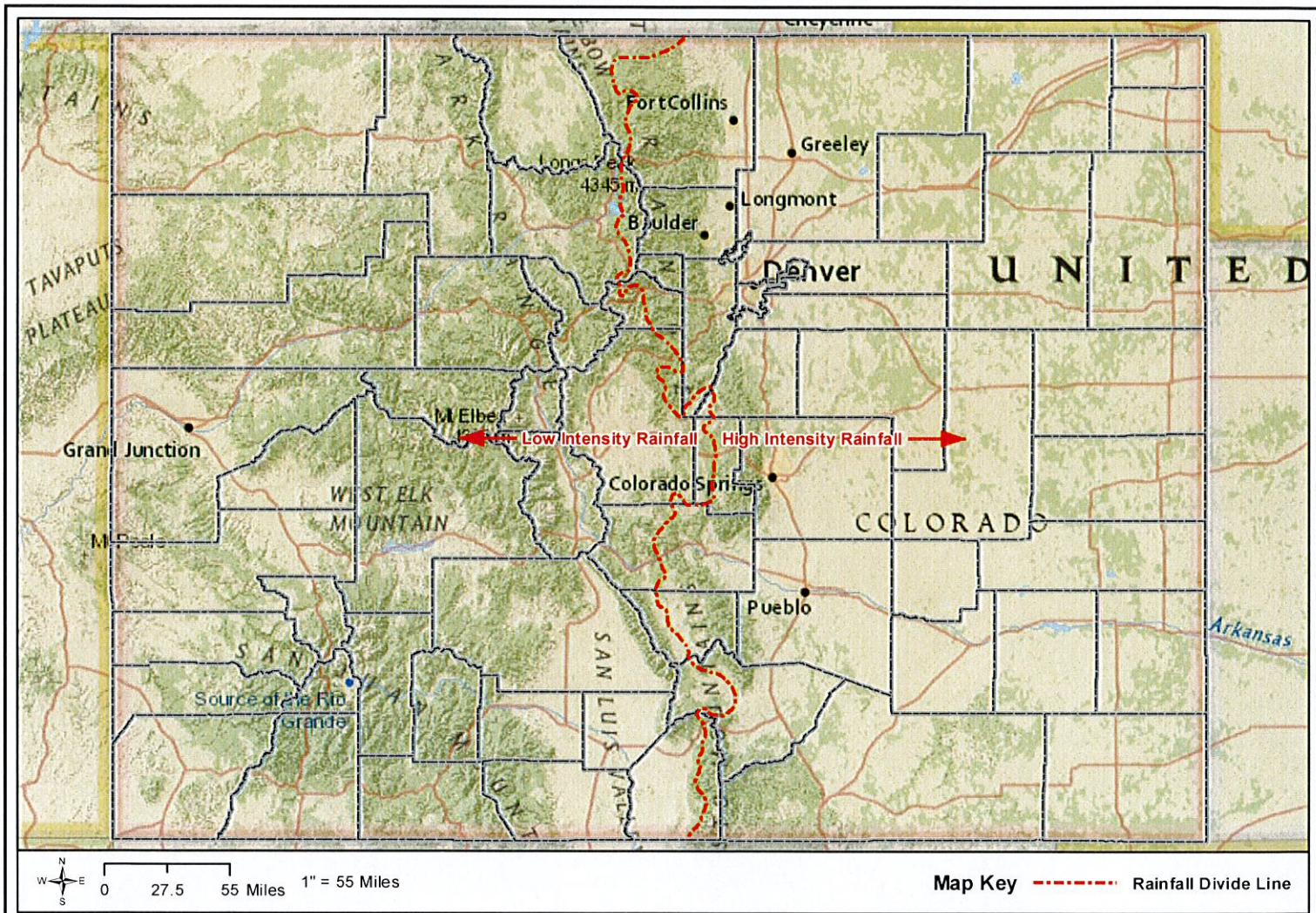
Table 1 DAM SAFETY BRANCH Spillway Sizing Guidelines for Non-Jurisdictional Dams

Drainage Area (Acres)	Minimum Recommended Bottom Width ¹ (Feet) Low Intensity Rainfall Zone	Minimum Recommended Bottom Width ¹ (Feet) High Intensity Rainfall Zone
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225	8	10
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375	10	17
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475	12	21
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1425	37	64
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¹Minimum recommended bottom width for drainage areas less than 175 acres is 8 feet



Spillway Section



COLORADO
 Division of Water Resources
 Department of Natural Resources

Rainfall Intensity Zones for Non-Jurisdictional Dam Spillway Sizing



DAM SAFETY BRANCH Specifications for Construction of Non-Jurisdictional Dams

- Site Selection:

- Foundation soils should be firm to provide adequate support for the embankment and should have low permeability to allow for water retention. Site selection should consider potential downstream property damage in the event of a dam failure. Construction of dams in boggy areas, areas with non-uniform fractured rock, or sands/gravels is not recommended and an engineer should be hired to evaluate the site conditions. Any part of the reservoir basin excavated below grade cannot expose groundwater.

- Embankment Design:

- Backfill material to be used for construction of the cutoff trench and embankment should be a suitable clay material and contain no material larger than 6 inches in diameter.
- The upstream slope should be constructed with a slope no steeper than 3:1, and the downstream slope should be no steeper than 2:1 (see cross section below). The dam crest should have a minimum width of 10 feet and the surface should be graded with positive drainage toward the reservoir basin.
- It is recommended that rock rip rap or other suitable material be placed on the upstream slope of the embankment to protect it from wave action. A suitable gravel or geosynthetic material should be placed under the rip rap to prevent fine material from washing out from behind the larger rock.
- The embankment should be fenced to restrict livestock from accessing the dam since they damage the protective vegetation and increase erosion.

- Embankment Construction

- The topsoil and all organic material should be removed from the foundation of the proposed dam site. Organic soil should only be reused for placement on the completed embankment to promote the re-growth of vegetation.
- A cutoff trench should be excavated under the full length of the centerline of the dam with sloping sides (1:1 min.), a minimum bottom width of 3 feet and a depth of 3 feet.
- The foundation of the dam should be scarified/ripped to a depth of 6-inches to provide proper contact between the native foundation and embankment. This surface should then be moisture treated before placement of fill.
- Fill material should be placed in layers not exceeding 12 inches in thickness prior to compaction. Suitable backfill material should have enough clay and moisture content to roll a small ball by hand. If this cannot be done, the soil is likely too dry or does not have adequate clay content.
- Each lift should be thoroughly compacted using a sheeps foot compactor. Care should be taken not to allow the top layers of the soil to dry out between placement of lifts.
- Fill should be placed in uniform lifts that cover the entire embankment length and width.

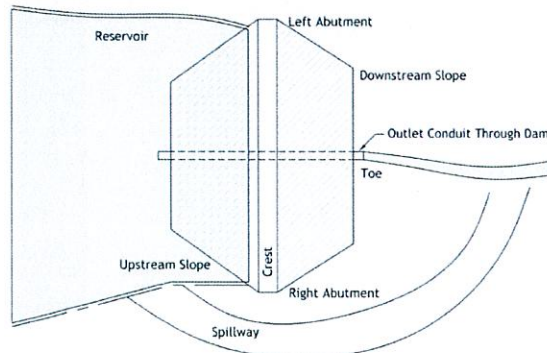
- Outlet

- Unless a waiver is granted in writing by the Division Engineer, all non-jurisdictional dams require an outlet conduit positioned at the natural low point of the reservoir basin. A minimum diameter of 12 inches is recommended and should be controlled at the upstream end by a valve and trash rack.

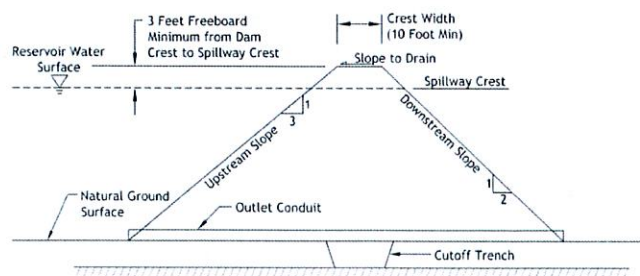
- Emergency Spillway

- The spillway should have sufficient width to provide capacity to route the runoff from the drainage basin above the dam during rainfall/runoff events.
- The emergency spillway should be located on natural ground far enough away to prevent erosion of the dam embankment. A spillway over the dam embankment is not acceptable.
- A minimum of 3 feet of freeboard is required from the bottom of the emergency spillway to the top of the dam.
- To determine the minimum spillway width, see the attached table for your area and drainage basin size.

- Example Plan View and Cross Section



Plan View



Cross Section Through Dam at Outlet

Stormwater Detention and Infiltration Design Data Sheet

Workbook Protected

Worksheet Protected

Stormwater Facility Name: Retreat at TimberRidge Filing No. 1 - Pond 1

Facility Location & Jurisdiction: Northwest of Poco Rd. and Sand Creek, El Paso County

User (Input) Watershed Characteristics

Watershed Slope =	0.020	ft/ft
Watershed Length-to-Width Ratio =	2.00	L:W
Watershed Area =	24.00	acres
Watershed Imperviousness =	11.0%	percent
Percentage Hydrologic Soil Group A =	0.0%	percent
Percentage Hydrologic Soil Group B =	100.0%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
Location for 1-hr Rainfall Depths (use dropdown):		
User Input	▼	

User Input: Detention Basin Characteristics

WQCV Design Drain Time = hours

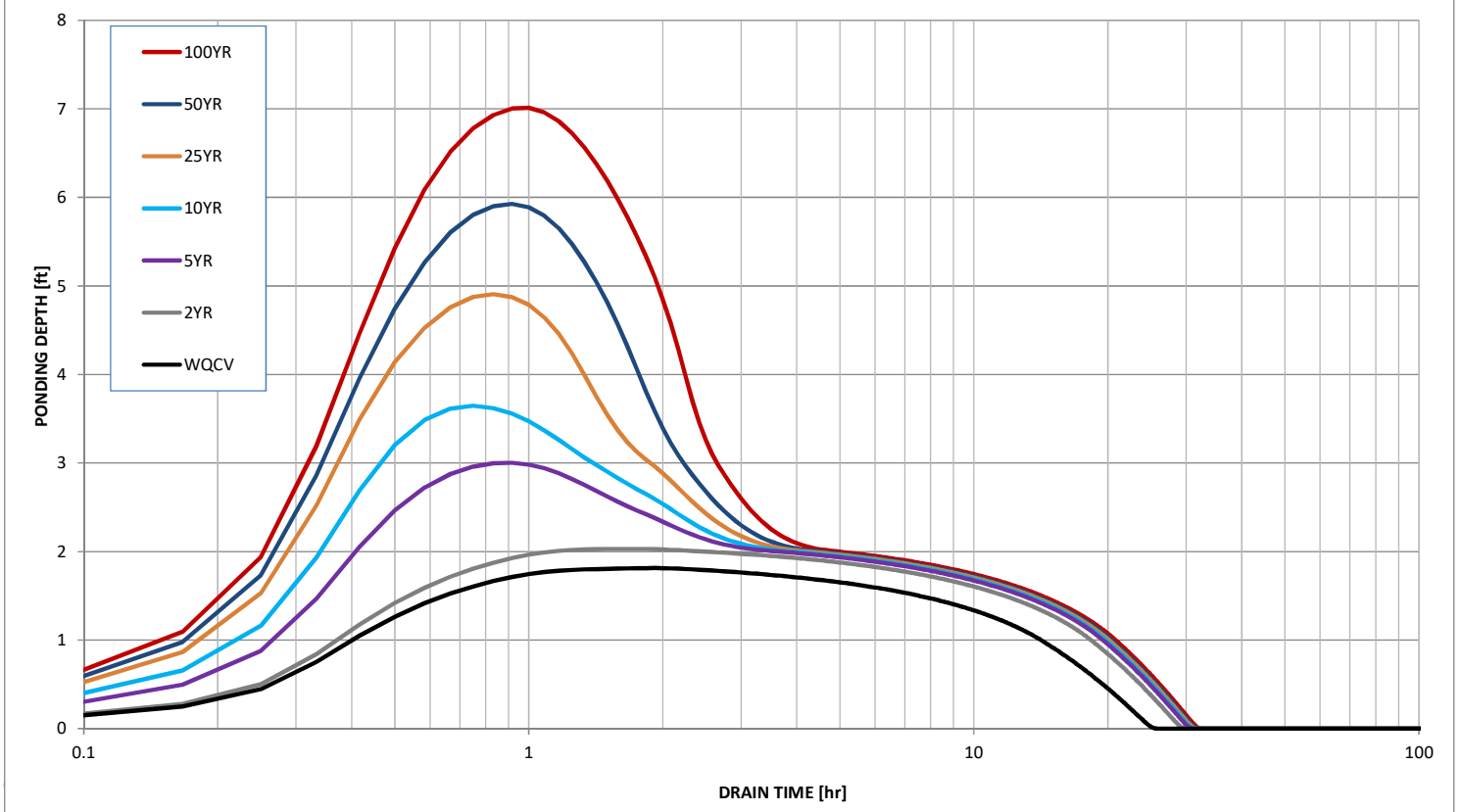
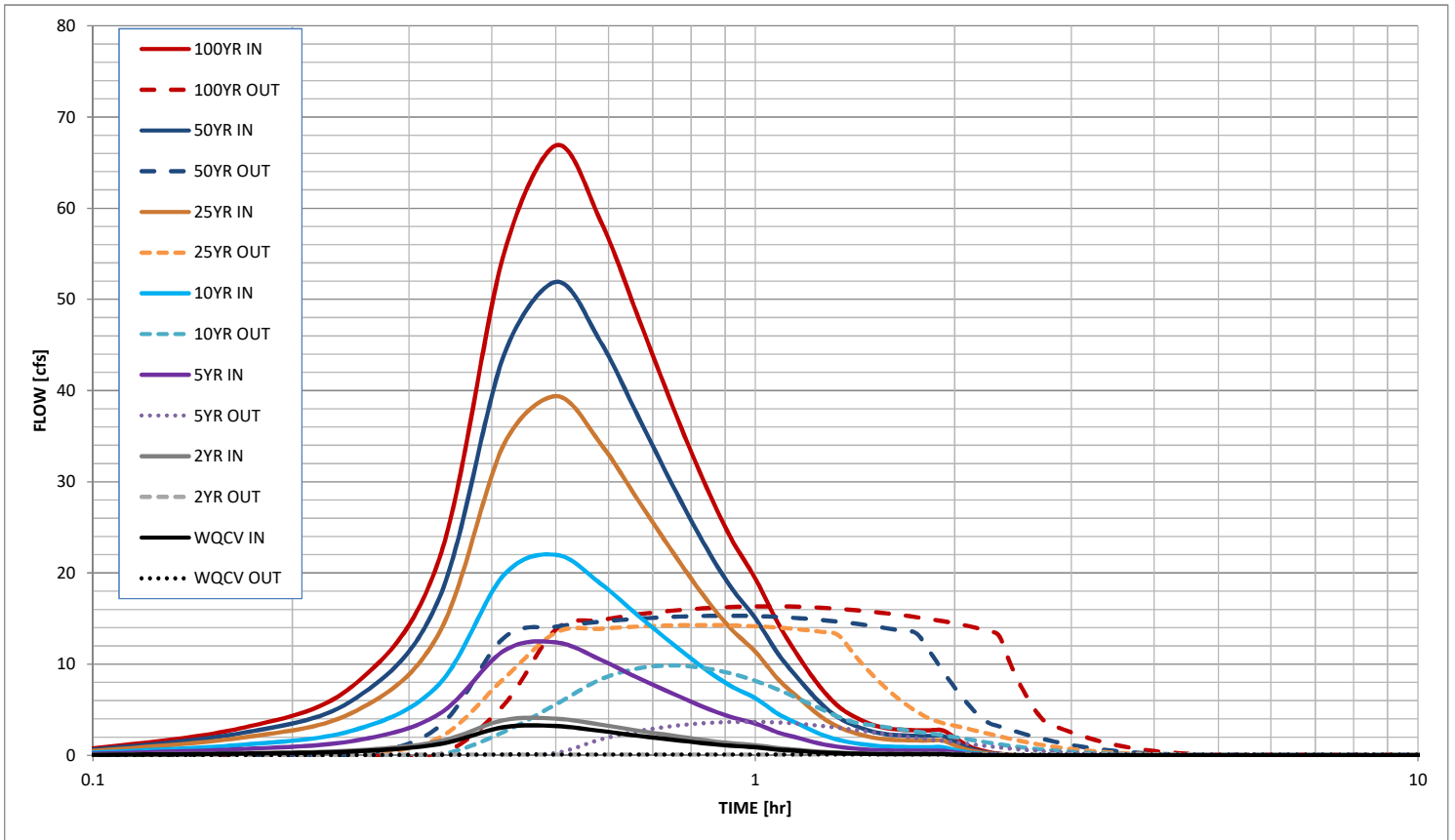
User Defined Stage [ft]	User Defined Area [ft^2]	User Defined Stage [ft]	User Defined Discharge [cfs]
0.00	50	0.00	0.00
1.00	3,443	1.00	0.09
2.00	6,871	2.00	0.09
3.00	9,740	3.00	3.68
4.00	12,575	4.00	13.32
5.00	15,295	5.00	14.39
6.00	18,016	6.00	15.39
7.00	22,223	7.00	16.33
8.00	26,430	8.00	17.21

After completing and printing this worksheet to a pdf, go to: <https://maperture.digitaldataservices.com/gvh/?viewer=cswdif>, create a new stormwater facility, and attach the pdf of this worksheet to that record.

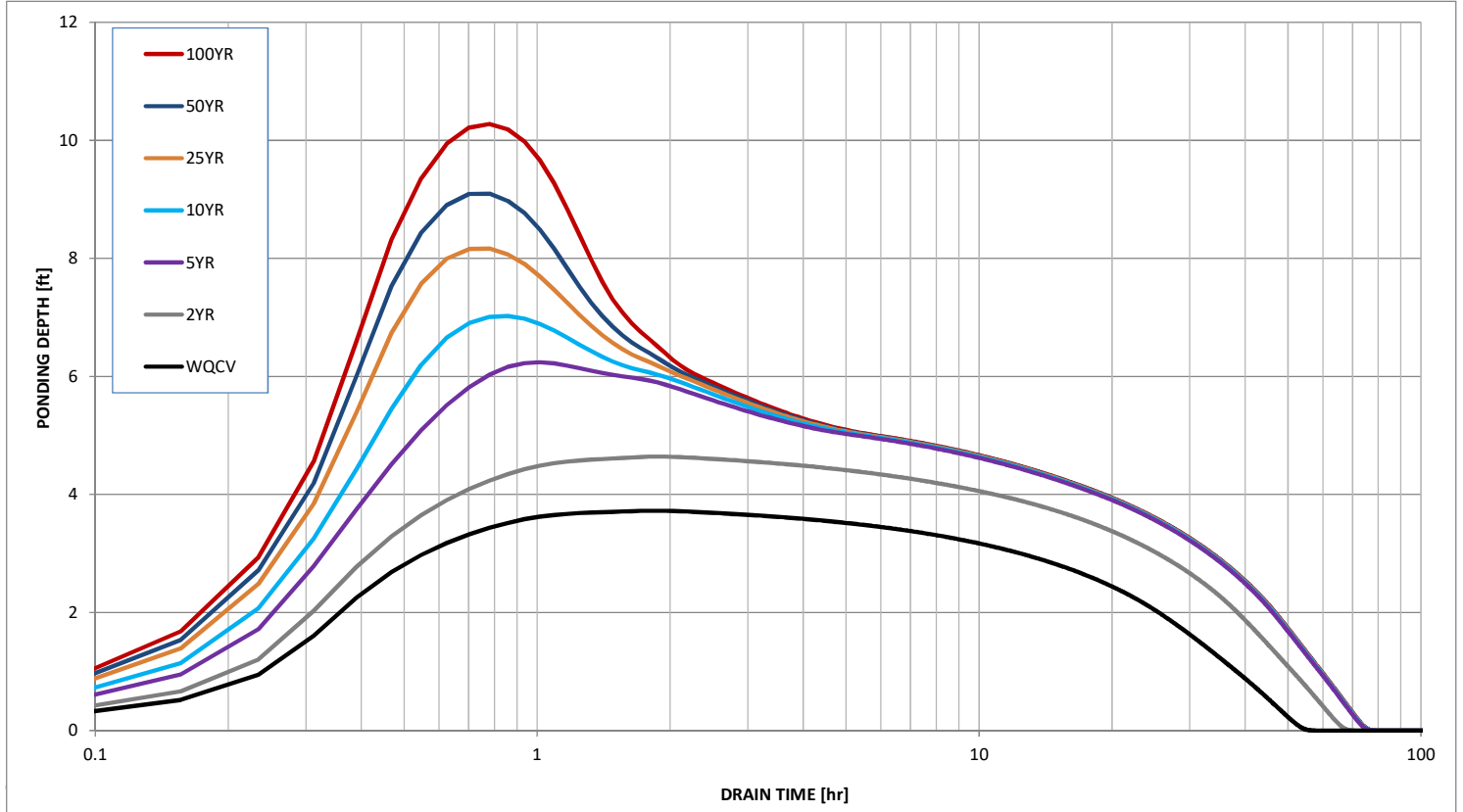
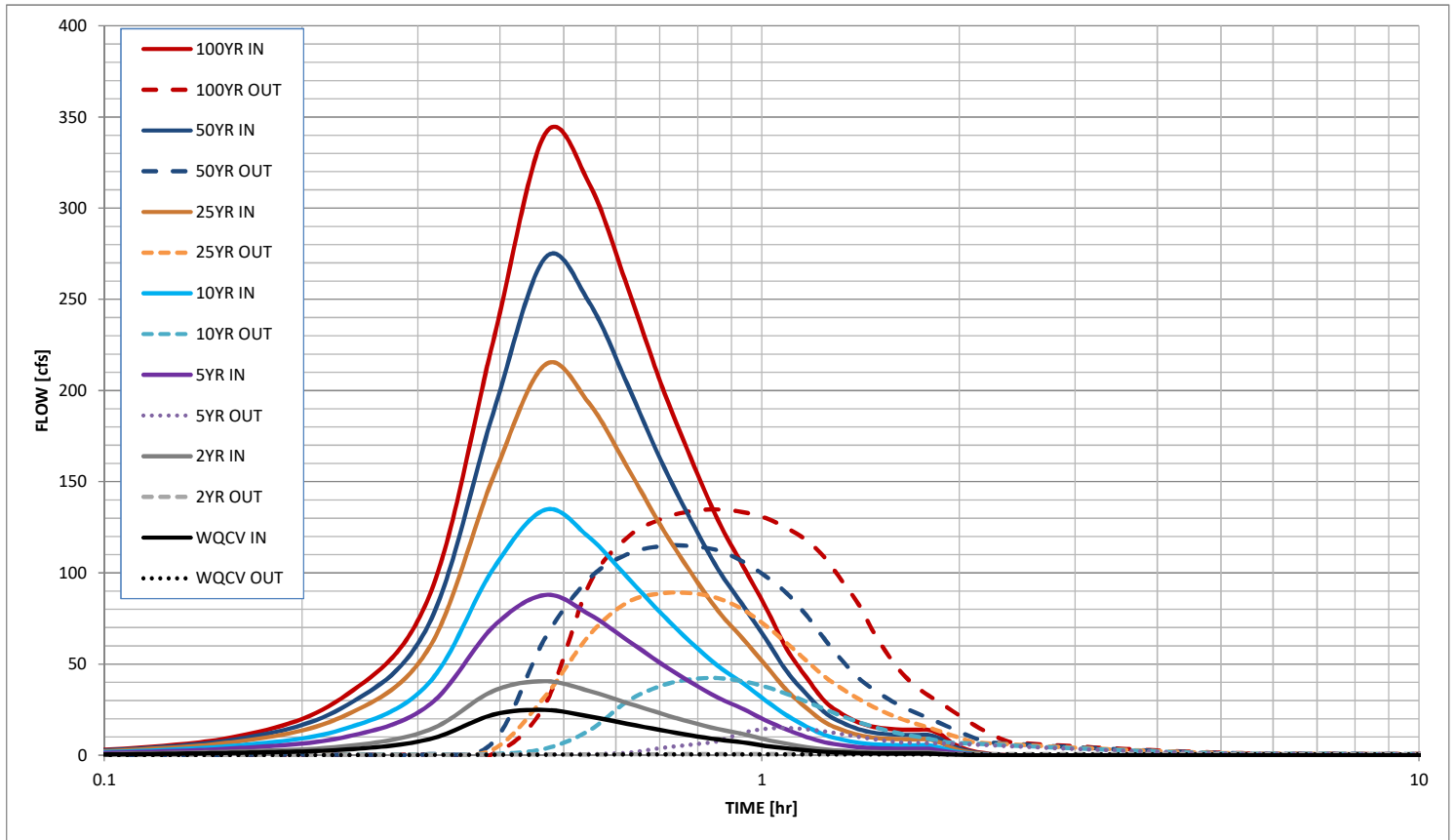
Routed Hydrograph Results

	WQCV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	
Design Storm Return Period =								
One-Hour Rainfall Depth =	0.53	1.19	1.50	1.75	2.00	2.25	2.52	in
Calculated Runoff Volume =	0.144	0.180	0.557	0.991	1.778	2.343	3.008	acre-ft
OPTIONAL Override Runoff Volume =								acre-ft
Inflow Hydrograph Volume =	0.143	0.180	0.556	0.990	1.778	2.343	3.007	acre-ft
Time to Drain 97% of Inflow Volume =	22	25	23	21	18	16	14	hours
Time to Drain 99% of Inflow Volume =	23	27	26	25	24	23	22	hours
Maximum Ponding Depth =	1.81	2.03	3.00	3.65	4.91	5.93	7.01	ft
Maximum Ponded Area =	0.142	0.159	0.223	0.265	0.345	0.408	0.511	acres
Maximum Volume Stored =	0.129	0.162	0.348	0.506	0.889	1.275	1.772	acre-ft

Stormwater Detention and Infiltration Design Data Sheet



Stormwater Detention and Infiltration Design Data Sheet



Stormwater Detention and Infiltration Design Data Sheet

Workbook Protected

Worksheet Protected

Stormwater Facility Name: Retreat at TimberRidge Filing No. 1 - Rain Garden 1

Facility Location & Jurisdiction: Approx. 1600' Southeast of int. of Vollmer Rd. and Poco Rd., El Paso County

User (Input) Watershed Characteristics

Watershed Slope =	<input type="text" value="0.050"/>	ft/ft
Watershed Length-to-Width Ratio =	<input type="text" value="2.00"/>	L:W
Watershed Area =	<input type="text" value="2.70"/>	acres
Watershed Imperviousness =	<input type="text" value="25.0%"/>	percent
Percentage Hydrologic Soil Group A =	<input type="text" value="0.0%"/>	percent
Percentage Hydrologic Soil Group B =	<input type="text" value="100.0%"/>	percent
Percentage Hydrologic Soil Groups C/D =	<input type="text" value="0.0%"/>	percent
Location for 1-hr Rainfall Depths (use dropdown):		
	<input type="text" value="User Input"/>	▼

User Input: Detention Basin Characteristics

WQCV Design Drain Time = hours

User Defined Stage [ft]	User Defined Area [ft^2]	User Defined Stage [ft]	User Defined Discharge [cfs]
0.00	3,125	0.00	0.00
1.00	4,130	1.00	0.08
2.00	5,332	2.00	3.69
3.00	6,590	3.00	4.15

After completing and printing this worksheet to a pdf, go to: <https://maperture.digitaldataservices.com/gvh/?viewer=cswdif>, create a new stormwater facility, and attach the pdf of this worksheet to that record.

Routed Hydrograph Results

	WQCV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
Design Storm Return Period =	WQCV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
One-Hour Rainfall Depth =	0.53	1.19	1.50	1.75	2.00	2.25	2.52
Calculated Runoff Volume =	0.030	0.053	0.106	0.157	0.242	0.305	0.379
OPTIONAL Override Runoff Volume =							
Inflow Hydrograph Volume =	0.030	0.052	0.106	0.156	0.241	0.305	0.379
Time to Drain 97% of Inflow Volume =	39	40	39	35	31	28	26
Time to Drain 99% of Inflow Volume =	51	52	51	47	43	40	38
Maximum Ponding Depth =	0.36	0.60	1.07	1.27	1.59	1.81	2.08
Maximum Ponded Area =	0.080	0.086	0.097	0.102	0.111	0.117	0.124
Maximum Volume Stored =	0.027	0.047	0.090	0.110	0.144	0.169	0.201

Stormwater Detention and Infiltration Design Data Sheet

