Galloway

6162 S. Willow Drive, Suite 320 Greenwood Village, CO 80111 303.770.8884 • GallowayUS.com

March 23, 2020

Kari Parsons Planning and Community Development 2880 International Circle, Suite 110 Colorado Springs, CO 80910

Re: Preliminary Drainage Conformance Letter for King Soopers #147 on Lot 2 & 3 – Falcon Marketplace Subdivision Filing No. 1

This drainage conformance letter has been prepared for Lots 2 & 3 of Falcon Marketplace Subdivision Filing No. 1 located in the southeast quarter of the southeast quarter of Section 1, Township 13 South, Range 65 West of the 6th Principal Meridian, County of El Paso, State of Colorado. The purpose of this letter is to show that the proposed drainage for Lots 2 & 3 conform to the current El Paso County *Drainage Criteria Manual* and the *Final Drainage Report for Falcon Marketplace* prepared by Drexel, Barrell & Co. dated July 22, 2019. A composite runoff coefficient calculation was performed for the subject site and these calculations are attached herein.

The King Soopers grocery store is to be located on Lot 2 of the subdivision, a 9.977-acre lot, and the King Soopers fuel facility is to be located on Lot 3 of the subdivision, a 1.309-acre lot. The project site is located within basins B4, B6, B11, B14, and B15 of the *Final Drainage Report*. Runoff from these lots was designed to be captured on-site and routed to storm sewer stubs provided along the adjacent public street. Detention and water quality will be provided by a pond located on the south side of the development, adjacent to E Woodmen Road.

The proposed site generally slopes from the north to the south and the grading is consistent with the intended grading and drainage pattern proposed in the original *Final Drainage Report* design. The proposed 5-yr and 100-yr runoff coefficients for the site were compared to the those designed in the *Final Drainage Report* to determine that the storm sewer system and detention pond provided by the master infrastructure has adequate capacity. Hydrologic calculations are included herein. All proposed inlets will be sized using UDFCD Street and Inlet Hydraulic spreadsheets. StormCAD will be used to model the proposed storm sewer on-site and calculations will be included with the Final Drainage Conformance Letter.

The proposed drainage plan for Lots 2 & 3 consists of 18 drainage basins (totaling 14.81 acres). Runoff from basins A-1 through A-9 will be collected into multiple curb inlets onsite which will connect to the master infrastructure storm sewer system and be routed to the south pond provided by the master developer. Basins B-1 through B-4 consist of roof areas that will be connected via roof drains to the on-site storm sewer system. Basins OS-1 through OS-5 consist of paved access aprons that sheet flow off-site and route to inlets within the adjacent public street provided by the master developer. The 100-year spillway will remain in the inundation easement, per plat and no structures will be built within this easement. The combined runoff coefficients for basins A-1 through A-9, B-1



through B-4, and OS-1 through OS-5 are estimated to be 0.75 and 0.84 for the 5- and 100- year storms, respectively (see calculations included herein). These runoff coefficients are equal to the planned values designed in the *Final Drainage Report* and thus the runoff will not exceed the anticipated amount. The overall imperviousness of the site after final stabilization has been calculated to be 85%. These findings indicated that this project will have no negative impacts on the existing drainage infrastructure.

Stormwater runoff from the fuel facility on Lot 2 will be collected at the inlet located at design point 8 and routed to Water Quality Pond #2 as determined by the *Final Drainage Report*. Water quality will be provided by Pond #2 and discharged into the open grass-lined channel along the north side of Woodmen Road.

The Four Step Process:

Step 1: Employ Runoff Reduction Practices

This step uses low impact development (LID) practices to reduce runoff at the source. Every attempt was made to reduce impervious areas while also complying with the parking requirements set forth by EI Paso County. The site is comprised of NRCS Type A soils, so all landscaped areas promote infiltration. Grass buffers have been utilized where possible.

Step 2: Implement BMPs that Provide a Water Quality Capture Volume with Slow Release

The proposed storm sewer inlets are scattered around the perimeter of the site, ensuring that the stormwater quality and flood detention is not concentrated in one area.

Step 3: Stabilize Drainageways

Pond S4 The upstream SR4 pond, as constructed by the master developer, reduces downstream runoff, therefore stabilizing the downstream systems.

Step 4: Implement Site Specific and Other Source Control BMPs

The site is compliant with the approved Final Drainage Report for Falcon Marketplace, prepared by Drexel, Barrell & Co. The runoff from this site will be collected via the proposed storm sewer and will tie into the existing storm sewer, which is routed to an existing water quality pond located south of the site.

Drainage Fees:

The El Paso County Drainage Basin Fee is \$305,322.17 based on the 2019 fee schedule of \$29,622/impervious acre with an impervious acreage of 10.31 acres for Lots 2 and 3. This fee is expected to be paid for by the seller of the property due at the time of Final Plat recording.

The El Paso County Bridge Fee is \$41,951.39 based on the 2019 fee schedule of \$4,069/impervious acre and 10.31 acres of imperviousness for Lots 2 and 3. This fee is expected to be paid for by the seller of the property due at the time of Final Plat recording.



Design Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the applicable master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or on sign so n my part in preparing this report.

Jennifer/R. Romano, PE Licensed Professional Engineer, State of Colorado No. 44401



Owner/Developer's Statement:

I, the owner/developer have read and will comply with all of the requirements specified in this drainage report and plan.

Charles Bothm

Charles Boehm King Soopers 800 Ridgelake Boulevard Memphis, TN 38101-1878

<u> 4/2/2020</u> Date

El Paso County:

Filed in accordance with the requirements of the Drainage Criteria Manual, Volumes 1 and 2, El Paso County Engineering Criteria Manual and Land Development Code as amended.

Jennifer Irvine, P.E. County Engineer / ECM Administrator Date

Conditions:



Attachments:

Vicinity Map FEMA Flood Insurance Rate Map Annotated FIRM from CLOMR Master Development Detention Basin Storage Table (by others) Master Development Drainage Plans (by others) Master Development Rational Calculations (by others) Web Soil Survey Developed Hydrologic Calculations Final Drainage Plan



VICINITY MAP

LOTS 2 & 3, BLOCK 1, FALCON MARKETPLACE EL PASO COUNTY, CO



NOT TO SCALE

NOTES TO USERS

This map is for use in administering the National Flood Insurance Program. It does not necessarily identify all areas subject to flooding, particularly from local drainage sources of small size. The community map repository should be consulted for possible updated or additional flood hazard information.

104" 37' 30.00"

3250000 FT

o obtain more detailed information in areas where Base Flood Elevations (BEEs o obtain more detailed information in areas where Base Flood Elevations (BFEs) for floodways have been determined, users are encouraged to consult the Flood forlies and Floodway Data and/or Summary of Stillwater Elevations tables contained infinit the Flood Insurance Study (FIS) report that accompanies this FIRM. Users hould be aware that BFEs shown on the FIRM represent rounded which-dot evations. These BFEs are intended for flood insurance study. Accordingly, nod elevations these BFEs are intended for flood the utilized in *Mccordingly*, and the FIRM for grant and the second the termined interflood the utilized in *Mccordingly*, and the FIRM for grant and the termined in the CIS report thread to utilized in the termined interflood the model and the termined in the CIS report thread to utilized in *Mccordingly*. e FIRM for purposes of construction and/or floodplain management

cestal Base Flood Elevations shown on this map apply only landward of 0.0 orth American Vertical Datum of 1988 (NAVD88). Users of this FiRM should be vare that coastal flood elevations are also provided in the Summary of Sillwater levations table in the Flood Insurance Study report for this jurisdiction. Elevations own in the Summary of Sillwater Elevations table should be used for construction nd/or floodplain management purposes when they are higher than the elevation own on this FIRM.

oundaries of the **floodways** were computed at cross sections and interpolate etween cross sections. The floodways were based on hydraulic considerations wit agard to requirements of the National Flood Insurance Program. Floodway with nd other pertinent floodway data are provided in the Flood Insurance Study repo or this jurisdiction

Certain areas not in Special Flood Hazard Areas may be protected by **flood contr structures**. Refer to section 2.4 "Flood Protection Measures" of the Flood Insurance Study report for information on flood control structures for this jurisdiction.

The projection used in the preparation of this map was Universal Transverse Mercator (UTM) zone 13. The horizontal datum was NAD83, GR880 spheroid. Differences in datum, spherodi, projection or UTM zones zones used in the production of FIRMs for adjacent jurisdictions may result in slight positional differences in map features across jurisdiction boundaries. These differences do not affect the accuracy of this FIRM.

Flood elevations on this map are referenced to the North American Vertical Datum of 1998 (NAVD88). These flood elevations must be compared to structure and ground elevations referenced to the same vertical datum. For information regarding conversion between the National Geodetic Vertical Datum of 1929 and the North American Vertical Datum of 1988, visit the National Geodetic Survey website at http://www.ngs.noaa.gov/ or contact the National Geodetic Survey at the following address:

NGS Information Services NOAA, N/NGS12 National Geodetic Survey SSMC-3, #9202 1315 East-West Highway Silver Spring, MD 20910-3282

o obtain current elevation, description, and/or location information for bench mark hown on this map, please contact the Information Services Branch of the Nation eodetic Survey at (301) 713-3242 or visit its website at http://www.ngs.noaa.gov/.

tase Map information shown on this FIRM was provided in digital format by El Pas bounty, Colorado Springs Utilities, City of Fountain, Bureau of Land Management lational Oceanic and Atmospheric Administration, United States Geological Survey of Anderson Consulting Engineers, Inc. These data are current as of 2006.

This map reflects more detailed and up-to-date stream channel configurations an floodplain delineations than those shown on the previous FIRM for this jurisdictor The floodplains and floodways that were transferred from the previous FIRM ma have been adjusted to conform to these new stream channel configurations. As a result, the Flood Profiles and Floodway Data tables in the Flood insurance Stud esuit, the Flood Profiles and Floodway Data tables in the Flood Insurance Study export (which contains authoritative hydraulic data) may reflect stream channel listances that differ from what is shown on this map. The profile baselines depicted in this map represent the hydraulic modeling baselines that match the flood profiles and Floodway. Data Tables if applicable, in the FIS report. As a result, the profile aselines may deviate significativity from the new base map channel representation nd may appear outside of the floodplain.

Corporate limits shown on this map are based on the best data available at the time of publication. Because changes due to annexations or de-annexations may have occurred after this map was published, may users should contact appropriate community officials to verify current corporate limit locations.

Please refer to the separately printed **Map Index** for an overview map of the county showing the layout of map panels; community map repository addresses; and a using of Communities table containing National Flood Insurance Program dates for each community as well as a listing of the panels on which each community is

Contact FEMA Map Service Center (MSC) via the FEMA Map Information eXchange (FMIX) 1477-336-2627 for information on available products associated with this FIRM. Available products may include previously issued Letters of Map Change, a Flood Insurance Study Report, and/or digital versions of this map. The MSC may also be reached by Fax at 1-800-358-9620 and its website at http://www.msc.fema.gov/.

you have **questions about this map** or questions concerning the National Floc isurance Program in general, please call **1-877-FEMA MAP** (1-877-336-2627) isit the FEMA website at http://www.fema.gov/business/nfip.

El Paso County Vertical Datum Offset Table Vertical Datum Offset (ft) OD INSURANCE ST RSION INFORMATI Flooding Source REFER TO SECTION 3.3 OF THE EL PASO COUN FOR STREAM BY STREAM VERTICAL DATUM Panel Location Map



This Digital Flood Insurance Rate Map (DFIRM) was produced through a Cooperating Technical Partner (CTP) agreement between the State of Colorado Water Conservation Board (CWCB) and the Federal Emergency Management Agency (FEMA).

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Additional Flood Hazard information and resources an available from local communities and the Colorade Water Conservation Board.



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104" 35' 37.50"

			LEGEND
		SPECIAL FLO	OD HAZARD AREAS (SFHAS) SUBJECT TO BY THE 1% ANNUAL CHANCE FLOOD
٣	The 1% annua that has a 1% Hazard Area is Special Floort F	I chance flood (1 chance of being s the area subjec Hazard include Zo	20-year flood), also known as the base flood, is the flood equaled or exceeded in any given year. The Special Flood to flooding by the 1% annual chance flood. Areas of nes A, AE, AH, AO, AR, A9V, V, and VE. The Base Flood
	Elevation is the	water-surface el	evation of the 1% annual chance flood.
	ZONE AE ZONE AH	Base Flood Eleva Flood depths of	ions determined. 1 to 3 feet (usually areas of ponding); Base Flood
	ZONE AO	Elevations detern Flood depths of 1	nined. to 3 feet (usually sheet flow on sloping terrain); average
	1999 M.	depths determin determined.	ed. For areas of alluvial fan flooding, velocities also
	ZONE AR	Special Flood Hat flood by a flood AR indicates the	and Area Formerly protected from the 1% annual chance control system that was subsequently decertified. Zone t the former flood control system is being restored to
	ZONE A99	provide protectio Area to be prote	n from the 1% annual chance or greater flood. scted from 1% annual chance flood by a Federal flood
		protection syste determined.	m under construction; no Base Flood Elevations
	ZONE V	Loastal flood zo Elevations detern	ne with velocity hazard (wave action); no Base Flood nined.
	ZONE VE	Coastal flood z Elevations detern	one with velocity hazard (wave action); Base Flood lined.
	The floodway	FLOODWAY A	REAS IN ZONE AE
	kept free of e substantial incr	ncroachment so f reases in flood he	hat the 1% annual chance flood can be carried without ghts.
		OTHER FLOOI	DAREAS
	ZONE X	Areas of 0.2% ar average depths	nual chance flood; areas of 1% annual chance flood with of less than 1 foot or with drainage areas less than 1
		Square mile; and	areas protected by levees from 1% annual chance flood.
	ZONE X	Areas determined	to be outside the 0.2% annual chance floodplain.
	ZONE D	Areas in which flo	od hazards are undetermined, but possible.
	[[]]	COASTAL BAR	RIER RESOURCES SYSTEM (CBRS) AREAS
	2223	OTHERWISE I	PROTECTED AREAS (OPAs)
	CBRS areas an	d OPAs are norma	IIIy located within or adjacent to Special Flood Hazard Areas.
		Roc	dway boundary
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		Bou Bou	ndary dividing Special Flood Hazard Areas of different Base d Elevations, flood depths or flood velocities.
	~ 513 -	Basi	Flood Elevation line and value; elevation in feet*
	(EL 987)	Basi elev	r rioou clevation value where uniform within zone; ation in feet*:
	* Referenced t	o the North Amer	can vertical Datum of 1988 (NAVD 88) s section line
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	97° 07' 30.0	10" Geo	graphic coordinates referenced to the North American
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DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project:	FALCON MA	RKETPLAC	E	00 000			, 2011	·						
Basin ID:	POND 2		-											
ZOWE 3	2													
	IONE 1	1	-											
VOLUME EUNY WOOV		-		~										
/	/	100-10	RAA .		Depth Increment =		ft							
PERMANENT ORIF	CES	07700			Stars Starses	Ciana	Optional	I an oth	MATE AND	A	Optional	A	Veluese	Veluese
Example Zone	Configurat	tion (Rete	ntion Pond)		Description	(ft)	Stage (ft)	Length (ft)	(ft)	(ft/2)	Area (ft/2)	(acre)	(ft/3)	(ac-ft)
Required Volume Calculation					Top of Micropool		0.00				300	0.007		
Selected BMP Type =	EDB				6873		1.00				1,659	0.038	963	0.022
Watershed Area =	27.80	acres			6874		2.00				4,721	0.108	4,122	0.095
Watershed Length =	1,500	ft			6875		3.00			-	11,225	0.258	12,142	0.279
Watershed Slope =	0.020	ft/ft			6876		4.00				28,592	0.656	32,051	0.736
Watershed Imperviousness =	86.80%	percent			6879		7.00				46,125	1.059	144,126	3.309
Percentage Hydrologic Soil Group A =	100.0%	percent			6879.2		7.20				51,425	1.181	153,881	3.533
Percentage Hydrologic Soil Groups C/D =	0.0%	percent			-									
Desired WQCV Drain Time =	40.0	hours			-				-					
Location for 1-hr Rainfall Depths =	User Input				-									
Water Quality Capture Volume (WQCV) =	0.870	acre-feet	Optional Use	r Override										
Excess Urban Runoff Volume (EURV) =	3.247	acre-feet	1-hr Precipita	ation						-				
2-yr Runoff Volume (P1 = 0.95 in.) =	1.799	acre-feet	0.95	inches						-				
5-yr Runoff Volume (P1 = 1.22 in.) =	2.375	acre-feet	1.22	inches										
10-yr Runoff Volume (P1 = 1.48 in.) =	2.968	acre-feet	1.48	inches										
25-yr Runoff Volume (P1 = 1.86 in.) =	3.832	acre-feet	1.86	inches										
50-yr Runoff Volume (P1 = 2.19 In.) =	4.561	acre-teet	2.19	inches							-			
500-yr Runoff Volume (P1 = 3.46 in) =	7.652	acre-feet	3.46	inches	-									
Approximate 2-vr Detention Volume =	1.708	acre-feet			-									
Approximate 5-yr Detention Volume =	2.258	acre-feet												
Approximate 10-yr Detention Volume =	2.792	acre-feet												
Approximate 25-yr Detention Volume =	3.627	acre-feet								-				
Approximate 50-yr Detention Volume =	4.136	acre-feet												
Approximate 100-yr Detention Volume =	4.593	acre-feet												
Stage-Storage Calculation	0.970	٦									-			
Zone 2 Volume (User Defined - Zone 1) =	0.070	acre-teet	Total datas		-									
Select Zone 3 Storage Volume (Optional) =		acre-feet	is less than	100-year	-									
Total Detention Basin Volume =	1.270	acre-feet	volume.											
Initial Surcharge Volume (ISV) =	user	ft/3								-				
Initial Surcharge Depth (ISD) =	user	ft								-				
Total Available Detention Depth (H _{total}) =	user	ft								-				
Depth of Trickle Channel (H _{TC}) =	user	ft												
Slope of Trickle Channel (S _{TC}) =	user	ft/ft												
Basin Length to Width Ratio (R) =	user	H:V			-									
Eddin Eorgan to Midan Kallo (KE/W) =	0001													
Initial Surcharge Area (A _{ISV}) =	user	ft/9												
Surcharge Volume Length (L _{ISV}) =	user	ft								-				
Surcharge Volume Width (W _{ISV}) =	user	ft								-		-		-
Depth of Basin Floor (H _{FLOOR}) =	user	ft								-				
Length of Basin Floor (L _{FLOOR}) =	user	ft												
Width of Basin Floor (W _{FLOOR}) =	user	ft												
Volume of Basin Floor (V) =	user	11/12								-				
Depth of Main Basin (Hump) =	user	11'0 ft												
Length of Main Basin (L _{MAIN}) =	user	ft												
Width of Main Basin (W _{MAIN}) =	user	ft												
Area of Main Basin (A _{MAIN}) =	user	ft/2								-				
Volume of Main Basin (V _{MAIN}) =	user	ft/3												
Calculated Total Basin Volume (V _{total}) =	user	acre-feet												
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RUNOFF SUMMARY

BASIN	DP	Area (Ac.)	Q_5 (CFS)	Q ₁₀₀ (CFS)
A1	DP1	1.81	3.4	7.7
	DP2	1.81	3.4	7.7
A2		4.82	1.4	10.2
	DP3	6.63	4.6	17.3
B4	DP4	2.35	7.5	14.6
B5		0.63	2.8	5.1
	DP5	2.99	10.0	19.3
B6	DP6	3.19	12.8	23.6
B7		0.46	2.0	3.7
	DP7	6.63	23.8	28.0
B8	DP8	1.04	3.5	6.9
B 9		0.30	1.4	2.5
	DP9	1.35	4.9	9.3
B10		0.18	0.8	1.4
	DP10	8.16	29.2	38.1
B11	DP11	2.01	7.8	14.6
B12		0.18	0.8	1.5
	DP12	10.35	36.4	51.9
B13		0.20	0.9	1.6
	DP13	10.55	37.1	53.2
B14	DP14	2.49	9.1	17.0
B15	DP 15	5.73	20.3	38.0
B16		0.35	1.6	2.9
	DP 16	8.56	30.6	57.1
B17		0.33	1.5	2.7

BASIN	DP	Area (Ac.)	Q ₅ (CFS)	Q ₁₀₀ (CFS)
	DP17	8.89	31.9	59.3
	DP18	19.44	52.1	88.2
B18	DP19	2.18	7.8	15.0
B19	DP20	2.57	10.1	18.8
	DP21	24.19	67.6	117.5
B20	DP22	2.03	5.6	11.4
B21		1.62	0.5	4.0
	DP23	27.85	67.4	121.8
C1	DP24	0.35	1.3	2.6
C2		0.23	0.8	1.5
	DP25	0.59	2.0	3.8
C3		1.88	0.6	4.2
C4		2.19	6.9	13.8
	DP26	4.08	5.4	13.7
C5	DP27	0.64	0.5	1.9
C6		0.45	0.2	1.2
	DP28	5.31	7.4	18.3
C7	DP29	0.19	0.7	1.3
C8		1.14	2.5	5.5
	DP30	1.33	3.1	6.6
C9		3.43	7.3	16.2
D1		2.62	4.1	8.8
D2		0.07	0.3	0.6
D3		0.07	0.3	0.6
	DPO1	32.50	10.3	30.2

Р	REPARED) BY:							
		$\boldsymbol{\boldsymbol{z}}$							
DREX	EL, BARR	ELL & CO.							
Engineers • Surveyors 3 SOUTH 7TH STREET COLORADO SPGS, COLORADO 80905									
COLORADO SPGS, COLORADO 80905 CONTACT: TIM D. McCONNELL, P.E.									
(719)260-0887 BOULDER • COLORADO SPRINGS									
CLIENT:									
HUMM	EL INVESTI	MENTS, LLC							
8117 PF DA	RESTON ROAL	D, SUITE 120 5 75225							
	(214) 416-9	9820							
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RUNOFF	SUMMARY

BASIN	DP	Area (Ac.)	Q ₅ (CFS)	Q ₁₀₀ (CFS)
A1	DP1	1.81	3.4	7.7
	DP2	1.81	3.4	7.7
A2		4.82	1.4	10.2
	DP3	6.63	4.6	17.3
B4	DP4	2.35	7.5	14.6
B5		0.63	2.8	5.1
	DP5	2.99	10.0	19.3
B6	DP6	3.19	12.8	23.6
B7		0.46	2.0	3.7
	DP7	6.63	23.8	28.0
B8	DP8	1.04	3.5	6.9
B9		0.30	1.4	2.5
	DP9	1.35	4.9	9.3
B10		0.18	0.8	1.4
	DP10	8.16	29.2	38.1
B11	DP11	2.01	7.8	14.6
B12		0.18	0.8	1.5
	DP12	10.35	36.4	51.9
B13		0.20	0.9	1.6
	DP13	10.55	37.1	53.2
B14	DP14	2.49	9.1	17.0
B15	DP15	5.73	20.3	38.0
B16		0.35	1.6	2.9
	DP 16	8.56	30.6	57.1
B17		0.33	1.5	2.7

	DP17	8.89	31.9	59.3
	DP18	19.44	52.1	88.2
B18	DP19	2.18	7.8	15.0
B19	DP20	2.57	10.1	<mark>18.</mark> 8
	DP21	24.19	67.6	117.5
B20	DP22	2.03	5.6	11.4
B21		1.62	0.5	4.0
	DP23	27.85	67.4	121.8
C1	DP24	0.35	1.3	2.6
C2		0.23	0.8	1.5
	DP25	0.59	2.0	3.8
C3		1.88	0.6	4.2
C4		2.19	6.9	<mark>13.8</mark>
	DP26	4.08	5.4	13.7
C5	DP27	0.64	0.5	1.9
C6		0.45	0.2	1.2
	DP28	5.31	7.4	18.3
C7	DP29	0.19	0.7	1.3
C8		1.14	2.5	5.5
	DP30	1.33	3.1	6.6
C9		3.43	7.3	<u>16.2</u>
D1		2.62	4.1	8.8
D2		0.07	0.3	0.6
D3		0.07	0.3	0.6
	DPO1	32.50	10.3	30.2

BASIN DP Area (Ac.) Q₅ (CFS) Q₁₀₀ (CFS)

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DREXEL, BARRELL & CO. Engineers • Surveyors								
3 SOUTH 7TH STREET COLORADO SPGS, COLORADO 80905								
COLORADO SPGS, COLORADO 80905 CONTACT: TIM D. McCONNELL, P.E. (719)260-0887								
(719)260-0887 BOULDER • COLORADO SPRINGS								
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SHEET: 2 OF 5

RUNOFF SUMMARY

BASIN	DP	Area (Ac.)	Q ₅ (CFS)	Q ₁₀₀ (CFS)
A1	DP1	1.81	3.4	7.7
	DP2	1.81	3.4	7.7
A2		4.82	1.4	10.2
	DP3	6.63	4.6	17.3
B4	DP4	2.35	7.5	14.6
B5		0.63	2.8	5.1
	DP5	2.99	10.0	19.3
B6	DP6	3.19	12.8	23.6
B7		0.46	2.0	3.7
	DP7	6.63	23.8	28.0
B8	DP8	1.04	3.5	6.9
B9		0.30	1.4	2.5
	DP9	1.35	4.9	9.3
B10		0.18	0.8	1.4
	DP10	8. <mark>1</mark> 6	29.2	38.1
B11	DP11	2.01	7.8	14.6
B12		0.18	0.8	1.5
	DP12	10.35	36.4	5 <mark>1</mark> .9
B13		0.20	0.9	1.6
	DP13	10.55	37.1	53.2
B14	DP14	2.49	9.1	17.0
B15	DP15	5.73	20.3	38.0
B16		0.35	1.6	2.9
	DP16	8.56	30.6	57.1
B17		0.33	1.5	2.7

BASIN	DP	Area (Ac.)	Q ₅ (CFS)	Q ₁₀₀ (CFS)
	DP17	8.89	31.9	59.3
	DP18	19.44	52.1	88.2
B18	DP19	2.18	7.8	15.0
B19	DP20	2.57	10.1	18.8
	DP21	24.19	67.6	117.5
B20	DP22	2.03	5.6	11.4
B21		1.62	0.5	4.0
	DP23	27.85	67.4	121.8
C1	DP24	0.35	1.3	2.6
C2		0.23	0.8	1.5
	DP25	0.59	2.0	3.8
C3		1.88	0.6	4.2
C4		2.19	6.9	13.8
	DP26	4.08	5.4	13.7
C5	DP27	0.64	0.5	1.9
C6		0.45	0.2	1.2
	DP28	5.31	7.4	18.3
C7	DP29	0.19	0.7	1.3
C8		1.14	2.5	5.5
	DP30	1.33	3.1	6.6
C9		3.43	7.3	16.2
D1		2.62	4.1	8.8
D2		0.07	0.3	0.6
D3		0.07	0.3	0.6
	DPO1	32 50	10.3	30.2

SHEET: 4 OF 5

PROJECT INFORMATION

PROJECT: PROJECT NO: DESIGN BY: REV. BY: AGE..... REPORT TYPE: DATE: Falcon Marketplace 20988-00CSCV KGV TDM El Paso County Final 4/17/2019

RATIONAL METHOD CALCULATIONS FOR STORM WATER RUNOFF

DEVELOPED	RUNOFF			-5	YR	STORM				P1=	2.52
				DIRECT RUNOF	F				TOTAL	RUNOFF	
BASIN (S)	Design Point	AREA (AC)	RUNOFF COEFF	t _c (MIN)	C * A	I (IN/HR)	Q (CFS)	t _c (MIN)	S (C * A)	I (IN/HR)	Q (CFS)
	(2)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)
A1	DP1	1.81	0.65	11.2	1.18	6.51	7.7				
A2		4.82	0.35	13.3	1.69	6.04	10.2				
	DP3	6.63	0.43	13.3	2.87	6.04	17.3				
B4	DP4	2.35	0.78	6.3	1.83	7.99	14.6				
B5		0.63	0.93	5.0	0.59	8.55	5.1				
	DP5	2.99	0.81	6.4	2.42	7.96	19.3				
B6	DP6	3.19	0.87	5.0	2.76	8.55	23.6				
B7		0.46	0.94	5.0	0.43	8.55	3.7				
	DP7	6.63	0.53	6.5	3.53	7.92	28.0				
B8	DP8	1.04	0.79	5.4	0.82	8.37	6.9				
В9		0.30	0.96	5.0	0.29	8.55	2.5				
	DP9	1.35	0.83	5.5	1.11	8.33	9.3				
B10		0.18	0.96	5.0	0.17	8.55	1.4				
	DP10	8.16	0.59	6.5	4.81	7.92	38.1				
B11	DP11	2.01	0.85	5.0	1.71	8.55	14.6				
B12		0.18	0.96	5.0	0.17	8.55	1.5				

100-yr

	DP12	10.35	0.65	7.0	6.69	7.76	51.9		
B13		0.20	0.96	5.0	0.19	8.55	1.6		
	DP13	10.55	0.65	7.1	6.88	7.72	53.2		
B14	DP14	2.49	0.84	6.0	2.10	8.11	17.0		
B15	DP15	5.73	0.84	6.6	4.81	7.89	38.0		
B16		0.35	0.96	5.0	0.34	8.55	2.9		
	DP16	8.56	0.85	6.6	7.25	7.88	57.1		
B17		0.33	0.96	5.0	0.32	8.55	2.7		
	DP17	8.89	0.85	6.7	7.56	7.84	59.3		
	DP18	19.44	0.58	6.8	11.30	7.80	88.2		
B18	DP19	2.18	0.80	5.0	1.75	8.55	15.0		
B19	DP20	2.57	0.85	5.0	2.20	8.55	18.8		
	DP21	24.19	0.63	7.1	15.25	7.70	117.5		
B20	DP22	2.03	0.75	7.8	1.53	7.47	11.4		
B21		1.62	0.35	9.3	0.57	7.02	4.0		
POND 2	DP23	27.85	0.62	9.3	17.34	7.02	121.8		
C1	DP24	0.35	0.85	5.1	0.30	8.51	2.6		
C2		0.23	0.85	7.2	0.20	7.66	1.5		
	DP25	0.59	0.85	7.2	0.50	7.66	3.8		
C3		1.88	0.35	12.0	0.66	6.32	4.2		
C4		2.19	0.75	5.2	1.64	8.44	13.8		
	DP26	4.08	0.56	13.6	2.29	5.99	13.7		
C5	DP27	0.64	0.45	10.9	0.29	6.59	1.9		
C6		0.45	0.35	8.1	0.16	7.37	1.2		
POND 3	DP28	5.31	0.58	13.9	3.08	5.93	18.3		
C7	DP29	0.19	0.86	6.8	0.16	7.82	1.3		
C8		1.14	0.69	9.6	0.79	6.94	5.5		
	DP30	1.33	0.72	9.6	0.95	6.94	6.6		
С9		3.43	0.66	8.9	2.28	7.12	16.2		
D1		2.62	0.70	20.9	1.82	4.84	8.8		
D2		0.07	0.96	5.0	0.07	8.55	0.6		
D3		0.07	0.96	5.0	0.07	8.55	0.6		

National Cooperative Soil Survey

Conservation Service

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
8	Blakeland loamy sand, 1 to 9 percent slopes	A	1.4	3.0%
9	Blakeland-Fluvaquentic Haplaquolls	A	16.6	35.7%
19	Columbine gravelly sandy loam, 0 to 3 percent slopes	A	28.5	61.4%
Totals for Area of Intere	est		46.4	100.0%

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition Component Percent Cutoff: None Specified Tie-break Rule: Higher

PROJECT: King Soopers Falcon Marketplace LOCATION: E. Woodsmen Road & Meridian Road Colorado Springs, El Paso County

*PERCENT IMPERVIOUS VALUES	
LANDSCAPE	0
PAVING	100
ROOFING	90
COMMERCIAL	95

* RUNOFF COE	EFFICIENTS U	SED (Type A S	Soils)	
	<u>2-Year</u>	<u>5-Year</u>	<u>10-year</u>	<u>100-Year</u>
LANDSCAPE	0.02	0.08	0.15	0.35
PAVING	0.89	0.90	0.92	0.96
ROOFING	0.71	0.73	0.75	0.81
COMMERCIAL	0.79	0.81	0.83	0.88

Project No.: KSS147

Date: August 26, 2019

Engineer: Natalie Haber

* Table 6-6 in CO Springs, Drainage Criteria Manual Revised May 2014

Composite Runoff Coefficients and Percent Imperviousness for Developed Drainage Basins

BASIN	OVERALL	LANDSCAPE	PAVED	ROOF	COMMERCIAL	2-YEAR	5-YEAR	10-YEAR	100-YEAR	PERCENT
DESIG.	AREA	AREA	AREA	AREA	AREA	COEFF.	COEFF.	COEFF.	COEFF.	IMPERVIOUS
	(sf)	(sf)	(sf)	(sf)	(sf)					
A-1	6,275	0	6,275	0	0	0.89	0.90	0.92	0.96	100%
A-2	18,589	8,629	9,960	0	0	0.49	0.52	0.56	0.68	54%
A-3	14,303	6,725	7,578	0	0	0.48	0.51	0.56	0.67	53%
A-4	22,967	7,038	15,929	0	0	0.62	0.65	0.68	0.77	69%
A-5	49,735	17,988	31,747	0	0	0.58	0.60	0.64	0.74	64%
A-6	18,257	5,209	13,048	0	0	0.64	0.67	0.70	0.79	71%
A-7	71,902	8,126	63,776	0	0	0.79	0.81	0.83	0.89	89%
A-8	233,171	21,075	116,465	0	95,631	0.77	0.79	0.81	0.87	89%
A-9	74,415	3,580	70,835	0	0	0.85	0.86	0.88	0.93	95%
B-1	56,790	0	0	56,790	0	0.71	0.73	0.75	0.81	90%
B-2	64,063	0	0	64,063	0	0.71	0.73	0.75	0.81	90%
B-3	3,742	0	0	3,742	0	0.71	0.73	0.75	0.81	90%
B-4	6,880	0	0	6,880	0	0.71	0.73	0.75	0.81	90%
TOTAL ON-SITE	641,089	78,370	335,613	131,475	95,631	0.73	0.75	0.78	0.84	85%
OS-1	713	0	713	0	0	0.89	0.90	0.92	0.96	100%
OS-2	710	0	710	0	0	0.89	0.90	0.92	0.96	100%
OS-3	1,044	0	1,044	0	0	0.89	0.90	0.92	0.96	100%
OS-4	710	0	710	0	0	0.89	0.90	0.92	0.96	100%
OS-5	712	0	712	0	0	0.89	0.90	0.92	0.96	100%
TOTAL OFF-SITE	3,889	0	3,889	0	0	0.89	0.90	0.92	0.96	100%
TOTAL SITE	644,978	78,370	339,502	131,475	95,631	0.73	0.75	0.78	0.84	85%

FINAL DRAINAGE REPORT FOR FALCON MARKETPLACE

B4	102,436	0.67	0.78
B6	138,913	0.79	0.87
B11	87,628	0.77	0.85
B14	108,260	0.76	0.84
B15	249,501	0.75	0.84
TOTAL SITE	686,738	0.75	0.84

STANDARD FORM SF-2 TIME OF CONCENTRATION

Subdivision:

Location: CO, El Paso County

Project Name: King Soopers Falcon Marketplace

Project No.: KSS147

Calculated By: SMB Checked By:

Date: 11/16/19

		SUB-B/	ASIN			INITIA	AL/OVER	LAND		TR	AVEL TIN	1E					
		DAT	Α				(T _i)				(T _t)			(U	RBANIZED BA	SINS)	FINAL
BASIN	D.A.	Hydrologic	Impervious	C ₁₀₀	C ₅	L	S	Ti	L	S	Cv	VEL.	T _t	COMP. T _c	TOTAL	Urbanized T _c	Τ _c
ID	(AC)	Soils Group	(%)			(FT)	(%)	(MIN)	(FT)	(%)		(FPS)	(MIN)	(MIN)	LENGTH (FT)	(MIN)	(MIN)
A-1	0.14	Α	100	0.96	0.90	95	6.0	2.0						2.0	95.0	10.5	5.0
A-2	0.43	Α	54	0.68	0.52	85	1.0	9.8	105	0.5	20.0	1.4	1.2	11.0	190.0	11.1	11.0
A-3	0.33	A	53	0.67	0.51	45	1.0	7.2	115	0.5	20.0	1.4	1.4	8.5	160.0	10.9	8.5
A-4	0.53	A	69	0.77	0.65	75	4.6	4.3	125	0.5	20.0	1.4	1.5	5.8	200.0	11.1	5.8
A-5	1.14	A	64	0.74	0.60	100	1.8	7.5	120	1.8	20.0	2.7	0.7	8.2	220.0	11.2	8.2
A-6	0.42	A	71	0.79	0.67	95	0.7	8.7	85	2.0	20.0	2.8	0.5	9.2	180.0	11.0	9.2
A-7	1.65	A	89	0.89	0.81	100	2.3	4.1	365	1.5	20.0	2.4	2.5	6.5	465.0	12.6	6.5
A-8	5.35	A	89	0.87	0.79	55	2.9	3.0	480	1.8	20.0	2.7	3.0	5.9	535.0	13.0	5.9
A-9	1.71	A	95	0.93	0.86	100	2.0	3.5	305	2.5	20.0	3.2	1.6	5.1	405.0	12.3	5.1
B-1	1.30	A	90	0.81	0.73												5.0
B-2	1.47	A	90	0.81	0.73												5.0
B-3	0.09	A	90	0.81	0.73												5.0
B-4	0.16	A	90	0.81	0.73												5.0
OS-1	0.02	A	100	0.96	0.90	16	2.0	1.2						1.2	16.0	10.1	5.0
OS-2	0.02	A	100	0.96	0.90	20	2.0	1.3						1.3	20.0	10.1	5.0
OS-3	0.02	A	100	0.96	0.90	22	2.0	1.4						1.4	22.0	10.1	5.0
OS-4	0.02	A	100	0.96	0.90	18	2.0	1.2						1.2	18.0	10.1	5.0
OS-5	0.02	A	100	0.96	0.90	20	2.0	1.3						1.3	20.0	10.1	5.0
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NOTES:

$$\begin{split} T_i &= (0.395^*(1.1 - C_5)^*(L)^{0.5})'((S)^{0.33}), \ S \ in \ ft/ft \\ T_t &= L/60V \ (Velocity \ From \ Fig. \ 501) \\ Velocity \ V &= Cv^*S^{0.5}, \ S \ in \ ft/ft \\ Tc \ Check &= \ 10 + L/180 \end{split}$$

For Urbanized basins a minimum $T_{\rm c}$ of 5.0 minutes is required.

For non-urbanized basins a minimum T_c of 10.0 minutes is required

STANDARD FORM SF-3

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Subdivision:

Project Name: King Soopers Falcon Marketplace Project No.: KSS147

Calculated By: SMB

Location: CO, El Paso County Design Storm: 5-Year

Checked By: Date: 11/16/19

				DIR	ECT RI	JNOFF				TOTAL	RUNOF	F	STE	REET		PIPE		TR/	AVEL T	IME	
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	l (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	l (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
	1	B-1	1.30	0.73	5.0	0.95	5.10	4.8													Roof drain piped to DP 2
	2	A-1	0.14	0.90	5.0	0.13	5.10	0.7													Type 13 Combination Sump Inlet
	2								5.0	1.08	5.10	5.5			5.5						Piped to DP 3
	3	A-2	0.43	0.52	11.0	0.22	3.95	0.9													Type 13 Combination Sump Inlet
	3								5.0	1.30	5.10	6.6			6.6						Piped to DP 5
	4	B-2	1.47	0.73	5.0	1.07	5.10	5.5													Roof drain piped to DP 5
	5	A-3	0.33	0.51	8.5	0.17	4.34	0.7													Type 13 Combination Sump Inlet
	5								8.5	2.54	4.34	11.0			11.0						Piped to DP 6
	6	A-4	0.53	0.65	5.8	0.34	4.91	1.7													Type 13 Combination Sump Inlet
	6								8.5	2.88	4.34	12.5			12.5						Piped to DP 9
	7	B-4	0.16	0.73	5.0	0.12	5.10	0.6													Roof drain piped to DP 8
	8	A-5	1.14	0.60	8.2	0.69	4.40	3.0													10' CDOT Type R Sump Inlet
	8								8.2	0.81	4.40	3.6			3.6						Piped to stub from overall development
	9	A-6	0.42	0.67	9.2	0.28	4.23	1.2													Type 13 Combination Inlet
	9								9.2	3.16	4.23	13.4			13.4						Piped to stub from overall development
	10	A-7	1.65	0.81	6.5	1.33	4.74	6.3							6.3						10' CDOT Type R Sump Inlet Piped to stub from overall development
	11	A-8	5.35	0.79	5.9	4.22	4.87	20.6							20.6						20' CDOT Type R Sump Inlet Piped to stub from overall development
	12	B-3	0.09	0.73	5.0	0.06	5.10	0.3													Roof drain piped to DP 13
	13	A-9	1.71	0.86	5.1	1.47	5.08	7.5													10' CDOT Type R Sump Inlet
	13								5.1	1.53	5.08	7.8			7.8						Piped to stub from overall development
	14	OS-1	0.02	0.90	5.0	0.01	5.10	0.1													Flows directly into roadway
	15	OS-2	0.02	0.90	5.0	0.01	5.10	0.1													Flows directly into roadway
	16	OS-3	0.02	0.90	5.0	0.02	<u>5.1</u> 0	0.1													Flows directly into roadway
															I						

Galloway & Company, Inc.

STANDARD FORM SF-3

STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

Project Name: King Soopers Falcon Marketplace Project No.: KSS147

Location: CO, El Paso County Design Storm: 5-Year Calculated By: SMB Checked By: Date: 11/16/19

				DIR	ECT RI	JNOFF				TOTAL	RUNO	۰F	STR	REET		PIPE		TRA	VEL T	IME	
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	l (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	l (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
	17	OS-5	0.02	0.90	5.0	0.01	5.10	0.1													Flows directly into roadway
	18	OS-4	0.02	0.90	5.0	0.01	5.10	0.1													Flows directly into roadway

Subdivision:

STANDARD FORM SF-3 STORM DRAINAGE SYSTEM DESIGN (RATIONAL METHOD PROCEDURE)

Subdivision: Location: CO, El Paso County Design Storm: 100-Year

Project Name: King Soopers Falcon Marketplace Project No.: KSS147 Calculated By: SMB Checked By:

Date: 11/16/19

				DIRE	CT RUN	OFF				TOTAL	RUNOF	F	STF	REET		PIPE		TRA	VEL T	IME	
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	l (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	l (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS
	1	B-1	1.30	0.81	5.0	1.06	9.09	9.6													Roof drain piped to DP 2
	2	A-1	0.14	0.96	5.0	0.14	9.09	1.3													Type 13 Combination Sump Inlet
	2								5.0	1.20	9.09	10.9			10.9						Piped to DP 3
	3	A-2	0.43	0.68	11.0	0.29	7.03	2.0													Type 13 Combination Sump Inlet
	3								5.0	1.49	9.09	13.5			13.5						Piped to DP 5
	4	B-2	1.47	0.81	5.0	1.19	9.09	10.8													Roof drain piped to DP 5
	5	A-3	0.33	0.67	8.5	0.22	7.73	1.7													Type 13 Combination Sump Inlet
	5								8.5	2.90	7.73	22.4			22.4						Piped to DP 6
	6	A-4	0.53	0.77	5.8	0.41	8.74	3.6													Type 13 Combination Sump Inlet
	6								8.5	3.31	7.73	25.6			25.6						Piped to DP 9
	7	B-4	0.16	0.81	5.0	0.13	9.09	1.2													Roof drain piped to DP 8
	8	A-5	1.14	0.74	8.2	0.84	7.84	6.6													10' CDOT Type R Sump Inlet
	8								8.2	0.97	7.84	7.6			7.6						Piped to stub from overall development
	9	A-6	0.42	0.79	9.2	0.33	7.53	2.5													Type 13 Combination Inlet
	9								9.2	3.64	7.53	27.4			27.4						Piped to stub from overall development
	10	A-7	1.65	0.89	6.5	1.47	8.43	12.4							12.4						10' CDOT Type R Sump Inlet Piped to stub from overall development
	11	A-8	5.35	0.87	5.9	4.67	8.67	40.5							40.5						20' CDOT Type R Sump Inlet Piped to stub from overall development
	12	B-3	0.09	0.81	5.0	0.07	9.09	0.6													Roof drain piped to DP 13
	13	A-9	1.71	0.93	5.1	1.59	9.05	14.4													10' CDOT Type R Sump Inlet
	13								5.1	1.66	9.05	15.0			15.0						Piped to stub from overall development
	14	OS-1	0.02	0.96	5.0	0.02	9.09	0.2													Flows directly into roadway
	15	OS-2	0.02	0.96	5.0	0.02	9.09	0.2													Flows directly into roadway
	16	OS-3	0.02	0.96	5.0	0.02	9.09	0.2													Flows directly into roadway
	17	OS-5	0.02	0.96	5.0	0.02	9.09	0.2													Flows directly into roadway
	18	OS-4	0.02	0.96	5.0	0.02	9.09	0.2													Flows directly into roadway

STANDARD FORM SF-3 STORM DRAINAGE SYSTEM DESIGN (RATIONAL METHOD PROCEDURE)

Subdivision: Location: CO, El Paso County Design Storm: 100-Year

Project Name: King Soopers Falcon Marketplace Project No.: KSS147 Calculated By: SMB Checked By:

Date: 11/16/19

				DIRE	CT RUN	IOFF				TOTAL	RUNOF	F	STR	REET		PIPE		TR/	AVEL 1	ГІМЕ	
STREET	Design Point	Basin ID	Area (Ac)	Runoff Coeff.	Tc (min)	C*A (Ac)	l (in/hr)	Q (cfs)	Tc (min)	C*A (Ac)	l (in/hr)	Q (cfs)	Slope (%)	Street Flow (cfs)	Design Flow (cfs)	Slope (%)	Pipe Size (inches)	Length (ft)	Velocity (fps)	Tt (min)	REMARKS

City Market\CO, Falcon - KSS000147 Woodmen & Meridian\CADD/2-PlankKSS147_CX.X-P-DRAINAGE.dwg - Aaron Johnston

DRAINAGE LEGEND

NOTE

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BASIN SUMMARY TABLE

Tributary	Area			t _c	Q_5	Q ₁₀₀
Sub-basin	(acres)	C ₅	C ₁₀₀	(min)	(cfs)	(cfs)
A-1	0.14	0.90	0.96	5.00	0.7	1.3
A-2	0.43	0.52	0.68	11.04	0.9	2.0
A-3	0.33	0.51	0.67	8.55	0.7	1.7
A-4	0.53	0.65	0.77	5.78	1.7	3.6
A-5	1.14	0.60	0.74	8.22	3.0	6.6
A-6	0.42	0.67	0.79	9.22	1.2	2.5
A-7	1.65	0.81	0.89	6.54	6.3	12.4
A-8	5.35	0.79	0.87	5.94	20.6	40.5
A-9	1.71	0.86	0.93	5.09	7.5	14.4
B-1	1.30	0.73	0.81	5.00	4.9	9.6
B-2	1.47	0.73	0.81	5.00	5.5	10.8
B-3	0.09	0.73	0.81	5.00	0.3	0.6
B-4	0.16	0.73	0.81	5.00	0.6	1.2
OS-1	0.02	0.90	0.96	5.00	0.1	0.1
OS-2	0.02	0.90	0.96	5.00	0.1	0.1
OS-3	0.02	0.90	0.96	5.00	0.1	0.2
OS-4	0.02	0.90	0.96	5.00	0.1	0.1
OS-5	0.02	0.90	0.96	5.00	0.1	0.1

RUNOFF COEFFICIENT TABLE				
	EXISTING COEFFICIENTS		PROPOSED COEFFICIENTS	
	5–YEAR	100-YEAR	5-YEAR	100-YEAR
COMPOSITE COEFFICIENT	0.75	0.84	0.75	0.84

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