

**MASTER DEVELOPMENT DRAINAGE PLAN (MDDP)
AND PRELIMINARY DRAINAGE REPORT
ADDENDUM NO. 1**

for

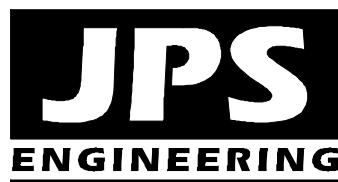
WALDEN PRESERVE 2 PUD

Prepared for:

**Walden Holdings I, LLC
17145 Colonial Park Drive
Monument, CO 80132**

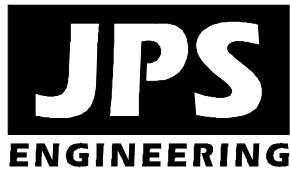
February 26, 2019
Revised May 16, 2019
Revised August 30, 2019
Revised September 30, 2019

Prepared by:



**19 E. Willamette Avenue
Colorado Springs, CO 80903
(719)-477-9429
www.jpsengr.com**

JPS Project No. 040201



19 E. Willamette Avenue
Colorado Springs, CO 80903
(719)-477-9429
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**WALDEN PRESERVE 2 PUD
MASTER DEVELOPMENT DRAINAGE PLAN (MDDP)
ADDENDUM NO. 1**

I. GENERAL

A. Background

Walden Preserve is a residential subdivision located in the Walden community of northeastern El Paso County, Colorado. The Walden Preserve 2 PUD area is located south and west of Woodhaven Drive, north of Pond View Place, and east of Walden Way, as shown in Figure A1 (Appendix F). The currently approved 2014 Walden Preserve 2 PUD includes phased development of 116 new residential lots (1-acre typical lot size), resulting in a total of 211 lots in the Walden Preserve development (including both previously platted lots and proposed lots).

Walden Preserve 2 Filings No. 1-3 have been completed. These filings comprise 43 lots on the south side of the Walden Preserve 2 PUD area.

The proposed Walden Preserve 2 Filing No. 4 consists of 23 residential lots on 45.3 acres within the previously approved Walden Preserve 2 PUD (part of El Paso County Assessor's Parcel Number 61230-01-023), consisting of the existing meadow area located along the proposed extension of Pinehurst Circle on the east side of Walden Way. In conjunction with the final plat for Filing No. 4, Amendments to the Walden Preserve 2 PUD and Preliminary Plan are being processed to update the alignment of Pinehurst Circle and the corresponding overall lot layout, while remaining consistent with the general road circulation pattern and previously approved total number of residential lots.

B. Scope

JPS Engineering prepared the "Master Development Drainage Plan and Preliminary Drainage Report for Walden Preserve 2 PUD" dated September 17, 2014 and the "Final Drainage Report for Walden Preserve 2 - Filings No. 1 and 2" dated November 13, 2014.

This report serves as an Addendum to the previously approved Master Development Drainage Plan (MDDP) for the Walden Preserve 2 PUD. The purpose of this Addendum is to provide an update to the developed drainage basins and general description of developed drainage patterns consistent with the current revisions to the subdivision lot layout.

This Addendum also serves to clarify water quality provisions for the future development areas. The original MDDP identified proposed Grass Buffer Strips along the east side of the future northeasterly residential development area. The updated Master Development Drainage Plan (enclosed in Appendix B), has been revised to include a proposed Water Quality Pond C12 to serve as a permanent Water Quality BMP in accordance with current County MS4 permit requirements. Specific details for future water quality facilities will be provided with future Final Drainage Reports for each development area.

II. DEVELOPED DRAINAGE CONDITIONS

The developed drainage basins and projected flows are shown in Figures D1 and D1.01-D1.04 (Appendix B). In general, the revised alignment of Pinehurst Circle and associated lot layout revisions extending north through the Walden Preserve 2 PUD area does not result in any significant change in the overall drainage plan. In accordance with current County requirements for water quality, developed flows from all residential areas will be routed through water quality treatment facilities.

Appendix A of this Addendum includes updated Rational Method hydrology for the revised Sub-Basin areas. The adjustment in sub-basin areas has no significant impact on the previous major basin hydrology (refer to current SCS model calculations in Appendix B1 of WP2 Filing No. 4 FDR).

As discussed in the approved MDDP, developed Sub-basins A1-A6 are located in the south and southeasterly areas previously developed as Walden Preserve Filings No. 1 and No. 2. Walden Preserve 2 Filings No. 1-3 included development of the areas within Sub-Basins A7-A9 on the north side of Pond View Place between the main drainage channel and Walden Way.

Sub-basins A1-A9 will continue to flow northerly towards the main tributary channel, combining at the existing southerly stock pond (Pond A) with developed peak flows calculated as $Q_5 = 99.6 \text{ cfs}$ and $Q_{100} = 285.5 \text{ cfs}$ (SCS Method) at Design Point #1. While the existing Stock Pond A is not planned for use as a drainage detention pond, the pond will be maintained as an aesthetic feature within the subdivision. The pond was previously upgraded with an engineered overflow spillway during development of Walden Preserve Filing No. 1.

Walden Preserve Filing No. 1 included development of the areas within Sub-basins B1-B4 on the east side of the main drainage channel. Sub-basins B5-B10 include the remaining areas developed as 1-acre lots along the west side of the drainage channel.

Developed Sub-basins B1-B10 will continue to flow northerly towards the main tributary channel, combining at the existing northerly stock pond (Pond B) with developed peak flows of $Q_5 = 189.1 \text{ cfs}$ and $Q_{100} = 508.9 \text{ cfs}$ (SCS Method) at Design Point #2.

The existing stock pond (Pond B) has been upgraded to serve as a sub-regional stormwater detention and water quality pond for the remaining phases of the Walden Preserve Subdivision. Construction plans for Walden Preserve 2 Filing No. 1 included addition of a water quality orifice plate on the Pond B outlet structure, but the outlet structure improvements have not yet been completed. The County has now requested additional improvements to the Pond B outlet structure to meet current full-spectrum detention design standards. The proposed improvements to the Pond B outlet structure are detailed in the Walden Preserve 2 Filing No. 4 Final Drainage Report (enclosed in Appendix D1).

The pond has been designed to “over-detain” to allow for discharges of developed flows from downstream sub-basins, while ensuring that discharges downstream of Walker Road remain below historic levels. An energy dissipation structure has been constructed at the discharge point from Pond B to reduce erosion concerns in the downstream channel.

Sub-basins C1-C4 include areas north of Pond B which will be developed as 1-acre lots. Runoff from these basins will flow northeasterly to the main drainage channel.

Water Quality Rain Garden C2 will be constructed on the east side of Basin C2 to provide water quality enhancement for combined Basins C1 and C2.

Water Quality Pond C4 will be constructed on the east side of Basin C4 to provide water quality enhancement for combined Basins C3 and C4.

While Water Quality Ponds C2 and C4 are not needed for stormwater detention in the overall drainage analysis, these small water quality ponds (or rain gardens) will encourage infiltration of developed drainage and provide water quality mitigation of developed drainage impacts, consistent with an overall low-impact development approach.

Sub-basins C5-C8 include additional areas to be developed as 1-acre lots. These basins will flow northeasterly to the future Detention Pond C8, which will mitigate developed flow impacts prior to discharging through Basin C9 into the main channel upstream of the proposed emergency access and trail crossing at Highview Drive. Total undetained developed peak flows at Design Point #3 are calculated as $Q_5 = 295.8$ cfs and $Q_{100} = 764.9$ cfs (SCS Method).

Sub-basins C10-C12 include additional 1-acre lot areas to be developed in the northwest part of Walden Preserve, and Sub-basin C13 covers the existing Walden Wastewater Treatment Facility at the northwest corner of the Walden property. Water Quality Pond C12 will be constructed in the northeast corner of the future residential development area to mitigate developed flow impacts from Sub-Basins C10-C12. Developed flows from Sub-basins C10-C13 will flow northeasterly, combining with flows in the main tributary channel, and ultimately reaching the existing culvert crossing Walker Road.

Flows from Basins OC1-OC2 on the east side of the channel, along with flows from Basins OD1 and D in vicinity of the Walden Pines Subdivision drain northwesterly across Woodhaven Drive, contributing to the total flow in the main channel. Flows from Basins OA1-OD1 and A1-D1 combine at Design Point #4, with total undetained developed flows calculated as $Q_5 = 324.4$ cfs and $Q_{100} = 845.4$ cfs (SCS Method).

As detailed in the latest hydrologic modeling (Appendix B1, WP2 Filing No. 4 FDR), SCS hydrologic models were developed using the HEC-HMS software package to evaluate the comparison of historic and developed flow conditions, and confirm sizing of the proposed stormwater detention ponds. The detained flow analysis shows that the combination of Pond B and Pond C8 results in detained flows at Design Point #4 calculated as $Q_5 = 226.7$ cfs and $Q_{100} = 601.4$ cfs, which achieves the goal of remaining below historic flows at the downstream boundary of the subdivision.

III. SUMMARY

The proposed amendments to the Walden Preserve 2 PUD and Preliminary Plan conform to the general drainage patterns described in the previously approved MDDP. The overall drainage plans have been updated for consistency with the current subdivision road and lot layout.

Developed drainage impacts from the Walden Preserve 2 PUD will be mitigated through existing and proposed on-site stormwater detention and water quality facilities. The proposed drainage patterns will remain consistent with historic conditions, and new drainage facilities constructed to El Paso County standards will safely convey runoff to suitable outfalls.

The existing and proposed detention ponds have been designed to maintain historic peak flows downstream of the Walden Preserve Subdivision. Proper construction and maintenance of the proposed drainage and erosion control facilities will ensure that this subdivision has no significant adverse drainage impact on downstream or surrounding areas.

APPENDIX A

HYDROLOGIC CALCULATIONS

WALDEN PRESERVE

IMPERVIOUS AREA CALCULATIONS

BASIN	TOTAL AREA (AC)	SOIL TYPE	(AC)	SUB-AREA 1 DEVELOPMENT/ COVER	% IMP.	AREA (AC)	SUB-AREA 2 DEVELOPMENT/ COVER	% IMP.	SUB-AREA 3 DEVELOPMENT/ COVER	% IMP.	WEIGHTED PERCENT IMPERVIOUS
OA1-A1-A9	341.96	B									6.20
OB1-B1-B4	90.90	B									25.00
B5	4.78	B	4.78	2.5-AC LOTS	11.0						11.00
B6	7.91	B	7.91	2.5-AC. LOTS	11.0						11.00
B5,B6	12.69	B									11.00
B7	3.24	B	3.24	1-AC LOTS	20.0						20.00
B8	28.74	B	28.74	1-AC LOTS	20.0						20.00
B5-B8	44.67	B									17.44
B9	10.23	B	10.23	1-AC LOTS	20.0						20.00
B10	16.90	B	16.90	1-AC LOTS	20.0						20.00
B5-B10	71.80	B									18.41
OB1,B1-B10	162.70	B									22.09
OA1,OB1,A,B	504.66	B									11.32
OC1	128.95	B	128.95	1/2-AC. LOTS	25.0						25.00
C1	11.75	B	5.0	1-AC. LOTS	20.0						12.53
C2	11.02	B	11.02	1-AC LOTS	20.0						20.00
C1-C2	22.77	B									16.15
C3	2.04	B	2.04	1-AC LOTS	20.0						20.00
C4	15.59	B	15.59	1-AC LOTS	20.0						20.00
C1-C4	40.40	B									17.83
C5	41.43	B	41.43	1-AC LOTS	20.0						20.00
C6	1.32	B	1.32	1-AC LOTS	20.0						20.00
C8	12.44	B	12.44	1-AC LOTS	20.0						20.00
C5,C6,C8	55.19	B									20.00
C9	4.50	B	4.5	1-AC LOTS	20.0						20.00
OC1,C1-C9	229.04	B									22.43
OA1-OC1,A1-C9	733.70	B									14.79
OC2	81.72	B	81.72	1/2-AC. LOTS	25.0						25.00
C10	2.86	B	2.86	1-AC LOTS	20.0						20.00
C11	8.63	B	8.63	1-AC LOTS	20.0						20.00
C12	17.77	B	17.77	1-AC LOTS	20.0						20.00
C10-C12	29.26	B									18.05
C13	22.44	B	22.44	MEADOW	0.0						0.00
OC2,C11-C13	133.42	B									19.27
OD1	3.30	B	3.30	1-AC LOTS	20.0						20.00
OD2	4.50	B	4.50	FOREST	0.0						0.00
OD3	10.30	B	10.30	FOREST	0.0						0.00
OD4	6.00	B	6.00	FOREST	0.0						0.00
D	10.27	B	10.27	1-AC LOTS	20.0						20.00
OD1-OD4 D	34.37	B									7.90
OA1-OD1,A-D	901.49	B									15.19

**WALDEN PRESERVE
COMPOSITE RUNOFF COEFFICIENTS**
DEVELOPED CONDITIONS**5-YEAR C VALUES**

BASIN	TOTAL AREA (AC)	SOIL TYPE	(AC)	SUB-AREA 1 DEVELOPMENT/ COVER	C	AREA (AC)	SUB-AREA 2 DEVELOPMENT/ COVER	C	(AC)	SUB-AREA 3 DEVELOPMENT/ COVER	C	WEIGHTED C VALUE
OA1,A1-A9	341.96	B										0.266
OB1,B1-B4	90.90	B										0.350
B5	4.78	B	4.78	2.5-AC. LOTS	0.3							0.300
B6	7.91	B	7.91	2.5-AC. LOTS	0.3							0.300
B5,B6	12.69	B										0.300
B7	3.24	B	3.24	1-AC LOTS	0.3							0.300
B8	28.74	B	28.74	1-AC LOTS	0.3							0.300
B5-B8	44.67	B										0.300
B9	10.23	B	10.23	1-AC LOTS	0.3							0.300
B10	16.90	B	71.17	1-AC LOTS	0.3							0.300
B5-B10	71.80	B										0.300
OB1,B1-B10	162.70	B										0.328
OA1,OB1,A,B	504.66	B										0.286
OC1	128.95	B	128.95	1/2-AC. LOTS	0.35							0.350
C1-C4	40.40	B	40.4	1-AC LOTS	0.3							0.300
C5	41.43	B	41.43	1-AC LOTS	0.3							0.300
C6	1.32	B	1.32	1-AC LOTS	0.3							0.300
C8	12.44	B	12.44	1-AC LOTS	0.3							0.300
C5,C6,C8	55.19	B										0.300
C9	4.50	B	4.5	1-AC LOTS	0.3							0.300
OC1,C1-C9	229.04	B										0.328
OA1-OC1,A1-C9	733.70	B										0.299
OC2	81.72	B	81.72	1/2-AC. LOTS	0.35							0.350
C10	2.86	B	2.86	1-AC LOTS	0.3							0.300
C11	8.63	B	8.63	1-AC LOTS	0.3							0.300
C10-C11	11.49	B										0.300
C12	17.77	B	17.77	1-AC LOTS	0.3							0.300
C10-C12	29.26	B										0.300
C13	22.44	B	22.44	MEADOW	0.25							0.250
OC2,C10-C13	133.42	B										0.322
OD1	3.30	B	3.30	1-AC LOTS	0.3							0.300
OD2	4.50	B	4.50	FOREST	0.1							0.100
OD3	10.30	B	10.30	FOREST	0.1							0.100
OD4	6.00	B	6.00	FOREST	0.1							0.100
D	10.27	B	10.27	1-AC LOTS	0.3							0.300
OD1-OD4, D	34.37	B										0.179
OA1-OD1,A-D	901.49	B										0.298

**WALDEN PRESERVE
COMPOSITE RUNOFF COEFFICIENTS**
DEVELOPED CONDITIONS**100-YEAR C VALUES**

BASIN	TOTAL AREA (AC)	SOIL TYPE	(AC)	SUB-AREA 1 DEVELOPMENT/ COVER	C	AREA (AC)	SUB-AREA 2 DEVELOPMENT/ COVER	C	(AC)	SUB-AREA 3 DEVELOPMENT/ COVER	C	WEIGHTED C VALUE
OA1,A1-A9	341.96	B										0.366
OB1,B1-B4	90.90	B										0.450
B5	4.78	B	4.78	2.5-AC. LOTS	0.4							0.400
B6	7.91	B	7.91	2.5-AC. LOTS	0.4							0.400
B5,B6	12.69	B										0.400
B7	3.24	B	3.24	1-AC LOTS	0.4							0.400
B8	28.74	B	28.74	1-AC LOTS	0.4							0.400
B5-B8	44.67	B										0.400
B9	10.23	B	10.23	1-AC LOTS	0.4							0.400
B10	16.90	B	71.17	1-AC LOTS	0.4							0.400
B5-B10	71.80	B										0.400
OB1,B1-B10	162.70	B										0.428
OA1,OB1,A,B	504.66	B										0.386
OC1	128.95	B	128.95	1/2-AC. LOTS	0.45							0.450
C1-C4	40.40	B	40.4	1-AC LOTS	0.4							0.400
C5	41.43	B	41.43	1-AC LOTS	0.4							0.400
C6	1.32	B	1.32	1-AC LOTS	0.4							0.400
C8	12.44	B	12.44	1-AC LOTS	0.4							0.400
C5,C6,C8	55.19	B										0.400
C9	4.50	B	4.5	1-AC LOTS	0.4							0.400
OC1,C1-C9	229.04	B										0.428
OA1-OC1,A1-C9	733.70	B										0.399
OC2	81.72	B	81.72	1/2-AC. LOTS	0.45							0.450
C10	2.86	B	2.86	1-AC LOTS	0.4							0.400
C11	8.63	B	8.63	1-AC LOTS	0.4							0.400
C10-C11	11.49	B										0.400
C12	17.77	B	17.77	1-AC LOTS	0.4							0.400
C10-C12	29.26	B										0.400
C13	22.44	B	22.44	MEADOW	0.35							0.350
OC2,C10-C13	133.42	B										0.422
OD1	3.30	B	3.30	1-AC LOTS	0.4							0.400
OD2	4.50	B	4.50	FOREST	0.15							0.150
OD3	10.30	B	10.30	FOREST	0.15							0.150
OD4	6.00	B	6.00	FOREST	0.15							0.150
D	10.27	B	10.27	1-AC LOTS	0.4							0.400
OD1-OD4, D	34.37	B										0.249
OA1-OD1,A-D	901.49	B										0.397

RATIONAL METHOD - DRAINAGE CALCULATIONS
DEVELOPED FLOWS

BASIN	DESIGN POINT	AREA (AC)	5-YEAR ⁽ⁿ⁾	100-YEAR ⁽ⁿ⁾	OVERLAND LENGTH (FT)	SLOPE (%)	T _{c0} ⁽¹⁾ (MIN)	CHANNEL LENGTH (FT)	CONVEYANCE COEFFICIENT K	SLOPE (%)	SCS ⁽²⁾ VELOCITY (FT/S)	T _c ⁽³⁾ (MIN)	TOTAL INTENSITY ⁽⁴⁾			PEAK FLOW				
													T _c ⁽⁴⁾ (MIN)	T _c ⁽⁵⁾ (MIN)	T _c ⁽⁶⁾ (MIN)	Q ₅ ⁽⁶⁾ (CFS)	Q ₁₀₀ ⁽⁶⁾ (CFS)			
OA1 A1-A9	1	341.96	0.266	0.366										21.1	21.1	2.93	5.22	93.31	213.54	
OB1 B1-B4	B4	90.90	0.350	0.450																
B5	B5	4.78	0.300	0.400	300	3.7	16.1	270	1.50	5.6	3.55	1.3	17.4	3.23	5.75	4.63	10.99			
B6	B6	7.91	0.300	0.400		0.0		380	1.50	10.0	4.74	1.3	5.0	5.10	9.09	12.11	28.75			
B5,B6																		28.14		
B7	B7	3.24	0.300	0.400	300	6.7	13.2	150	1.50	8	4.24	0.6	13.8	13.8	3.59	6.39	3.49	8.28		
WP2 FILING 4:																				
B8	B8	28.74	0.300	0.400		0.0		1250	1.50	4.8	3.29	6.3	6.3	6.3	4.78	8.51	41.24	97.88		
B5-B8	B8	44.67	0.300	0.400		0.0		540	1.50	7.4	4.08	2.2	25.1	25.1	2.68	4.76	35.87	85.13		
B9	B9	10.23	0.300	0.400	300	4.7	14.9	0.0	600	1.50	3.3	2.72	3.7	5.0	17.1	3.26	5.80	9.99		
B5-B10	B10	16.90	0.300	0.400		0.0		1900	1.50	2.4	2.31		28.7	28.7	2.48	4.42	53.46	126.87		
T1 from DP1 to DP2																				
OB1 B1-B10	2	504.66	0.286	0.386											69.5	69.5	1.50	2.65	516.50	516.22
OA1 OB1 A,B																				
OC1 1	C1	121.57	0.350	0.450		0.0		2800	1.50	4.64	3.23	14.4	14.4	14.4	3.52	6.27	149.86	342.97		
OC1 2	C2	7.38	0.350	0.450		0.0		2800	1.50	4.64	3.23	14.4	14.4	14.4	3.52	6.27	9.10	20.82		
OC1 1,OC1 2	OC1	128.95	0.350	0.450		0.0		2800	1.50	4.64	3.23	14.4	14.4	14.4	3.52	6.27	158.96	363.79		
WP2 FILING 5:																				
C1	C1	11.75	0.300	0.400	300	4.0	15.7	550	1.50	8.2	4.30	2.1	17.9	3.19	5.68	11.24	26.68			
C2	C2	11.02	0.300	0.400		0.0		820	1.50	5.5	3.52	3.9	3.9	5.0	5.10	9.09	16.88	40.05		
C3	C3	22.77	0.300	0.400		0.0		500	1.50	3.6	2.85	2.9	14.4	14.4	3.53	6.29	19.73	46.82		
C4	C4	2.04	0.300	0.400	100	2.0	11.4	900	1.50	6.9	3.94	3.8	15.2	15.2	3.44	6.12	16.08	38.15		
C3-C4	C4A	15.59	0.300	0.400	100	2.0	11.4						15.2	15.2	3.44	6.12				
C1-C4	C4	17.63	0.300	0.400		0.0		1900	1.50	2.4	2.31		21.7	21.7	2.89	5.14	35.00	83.07		
WP2 FILING 6:																				
C5	C5	41.43	0.300	0.400	100	4.0	9.1	1800	1.50	4.6	3.22	9.3	18.4	3.14	5.59	39.06	92.69			
C6	C6	1.32	0.300	0.400	100	6.0	7.9	250	1.50	8	4.24	10	8.9	8.9	4.28	7.62	1.70	4.02		
C8	C8	12.44	0.300	0.400	100	8.0	7.2	1200	1.50	5.5	3.52	5.7	12.9	12.9	3.70	6.59	13.82	32.79		
T1 from C5 to C8								550	1.50	3.6	2.85	3.2	3.2	5.0	5.10	9.09	16.89	40.05		
C5,C6,C8 (POND C8)	C8	55.19	0.300	0.400		0.0		360	1.50	3.6	2.85	2.1	21.6	21.6	2.90	5.15	47.95	113.80		
C9	C9	4.50	0.300	0.400		0.0		2700	1.50	1.9	2.04	22.1	22.1	5.0	5.10	9.09	16.36			
T1 from DP2 to DP3																				
OC1 C1-C9	3	733.70	0.299	0.399										91.6	91.6	1.50	2.65	329.06	775.78	

BASIN	DESIGN POINT	AREA (AC)	C 5-YEAR ⁽ⁿ⁾	100-YEAR ⁽ⁿ⁾	OVERLAND LENGTH (FT)	SLOPE (%)	T _{c0} ^(e) (MIN)	CHANNEL LENGTH (FT)	CONVEYANCE COEFFICIENT K	SLOPE (%)	SCS ⁽²⁾ VELOCITY (FT/S)	T _t ⁽³⁾ (MIN)	T _c ⁽⁴⁾ (MIN)	T _c ⁽⁵⁾ (MIN)	T _c ⁽⁶⁾ (MIN)	INTENSITY ⁽⁶⁾ 100-YR (IN/HR)	INTENSITY ⁽⁶⁾ 5-YR (IN/HR)	PEAK FLOW Q ₁₀₀ ⁽⁶⁾ (CFS)
OC22.1		62.48	0.350	0.450		0.0	2800	1.50	4.64	3.23	14.4	14.4	14.4	14.4	3.52	6.27	77.02	176.26
OC22.2		19.24	0.350	0.450		0.0	2800	1.50	4.64	3.23	14.4	14.4	14.4	14.4	3.52	6.27	23.72	54.28
OC22.1-OC22.2		81.72	0.350	0.450		0.0	2800	1.50	4.64	3.23	14.4	14.4	14.4	14.4	3.52	6.27	100.74	230.54
C10	C10	2.98	0.300	0.400	100	6.0	7.9	600	1.50	4.0	3.00	3.3	11.3	11.3	3.92	6.97	3.36	7.97
C11	T _t from C10 to C11	8.63	0.300	0.400	100	6.0	7.9	600	1.50	6.7	3.88	2.6	10.5	10.5	4.03	7.16	10.42	24.73
C11-C12	C11	11.49	0.300	0.400				400	1.50	2.5	2.37	2.8						
C12	C12	17.77	0.300	0.400	100	10.0	6.7	1300	1.50	3.1	2.64	8.2	14.1	14.1	3.56	6.34	12.28	29.15
C10-C12	C12	29.26	0.300	0.400				400	1.50	3.1	2.64	8.2	14.9	14.9	3.47	6.18	18.52	43.95
C13	C13	22.44	0.250	0.350	300	7.3	13.7	430	1.50	8.6	4.40	1.6	15.3	15.3	3.43	6.11	19.18	72.36
OC1-OC2-C1-C13	OC1-OC2-C1-C13	867.12	0.323	0.423				400	1.50	8.6	4.40	1.6	106.5	106.5	1.50	2.95	420.12	972.00
OD1		3.30	0.300	0.400	370	5.4	15.8	450	1.50	4.4	3.15	2.4	18.2	18.2	3.16	5.63	3.13	7.43
OD2		4.50	0.100	0.150	525	8.2	20.5	0			0.0	20.5	20.5	20.5	2.98	5.30	1.34	3.58
OD3		10.30	0.100	0.150	700	5.7	26.7	580	1.50	3.1	2.64	3.7	30.3	30.3	2.41	4.28	2.48	6.62
OD4	OD1-OD4_D	6.00	0.100	0.150	630	5.6	25.5	340	1.50	0.9	1.42	4.0	29.4	29.4	2.45	4.36	1.47	3.92
D	D	24.10	0.300	0.400				750	1.50	3.5	2.81	4.5	5.0	5.0	5.10	9.09	15.73	37.33
OD1-OD4_D	D	10.27	0.179	0.249				400	1.50	3.5	2.81	4.5	34.8	34.8	2.22	3.96	13.67	33.86
G1	G1	3.28	0.300	0.400	100	6.0	7.9	300	1.50	6.7	3.88	1.3	9.2	9.2	4.23	7.53	4.16	9.88
T _t from DP3 to DP4								2750	1.50	1.5	1.81	254	116.9	116.9	1.50	2.65	402.97	948.41
Q ₁₀₀ -OD1-A-D																		

1) OVERLAND FLOW/T_{c0} = (1.87/(1.1-RUNOFF COEFFICIENT)/(OVERLAND FLOW LENGTH^H(0.5)/(SLOPE^(0.333))

2) SCS VELOCITY = K * ((SLOPE(%))^0.5)

K = 0.70 FOR MEADOW / FOREST

K = 1.0 FOR BARE SOIL

K = 1.5 FOR GRASS CHANNEL

K = 2.0 FOR PAVEMENT

I = (A * P) / B + T_{DP/C}3) GUTTER/SWALE FLOW TRAVEL TIME, T_t = (CHANNEL LENGTH/SCS VELOCITY) / 60 SEC4) T_c = T_{c0} + T_t

*** IF TOTAL TIME OF CONCENTRATION IS LESS THAN 5 MINUTES, THEN 5 MINUTES IS USED

5) INTENSITY BASED ON LD-F CURVE IN EL PASO COUNTY DRAINAGE CRITERIA MANUAL, REVISED BY CITY OF COLORADO SPRINGS 1/1/03

I = (A * P) / B + T_{DP/C}

5-YEAR VALUES: A = 26.65; P = 1.5 IN (1-HOUR DEPTH); B = 10.0; C = 0.76

100-YEAR VALUES: A = 29.65; P = 2.67 IN (1-HOUR DEPTH); B = 10.0; C = 0.76

6) Q = CIA

7) WEIGHTED AVERAGE C VALUES FOR COMBINED BASINS

APPENDIX B

FIGURES

SUMMARY HYDROLOGY TABLE

<u>DESIGN POINT</u>	<u>Q₅ (CFS)</u>	<u>Q₁₀₀ (CFS)</u>
1	99.6	285.5
2	189.1	508.9
3	295.8	764.9
4 (DEVELOPED)	324.4	845.4
4 (DETAINED)	226.7	601.4

EXISTING ELEMENTARY SCHOOL

**WALDEN III
FILING
NO. 5**

**WALDEN III
FILING
NO. 7**

EXISTING RETENTION PO

SHEET D1.01

SHEET D1.02

D1.02 PROPOSE WQ RAIN GARDEN

SHEET D1.04

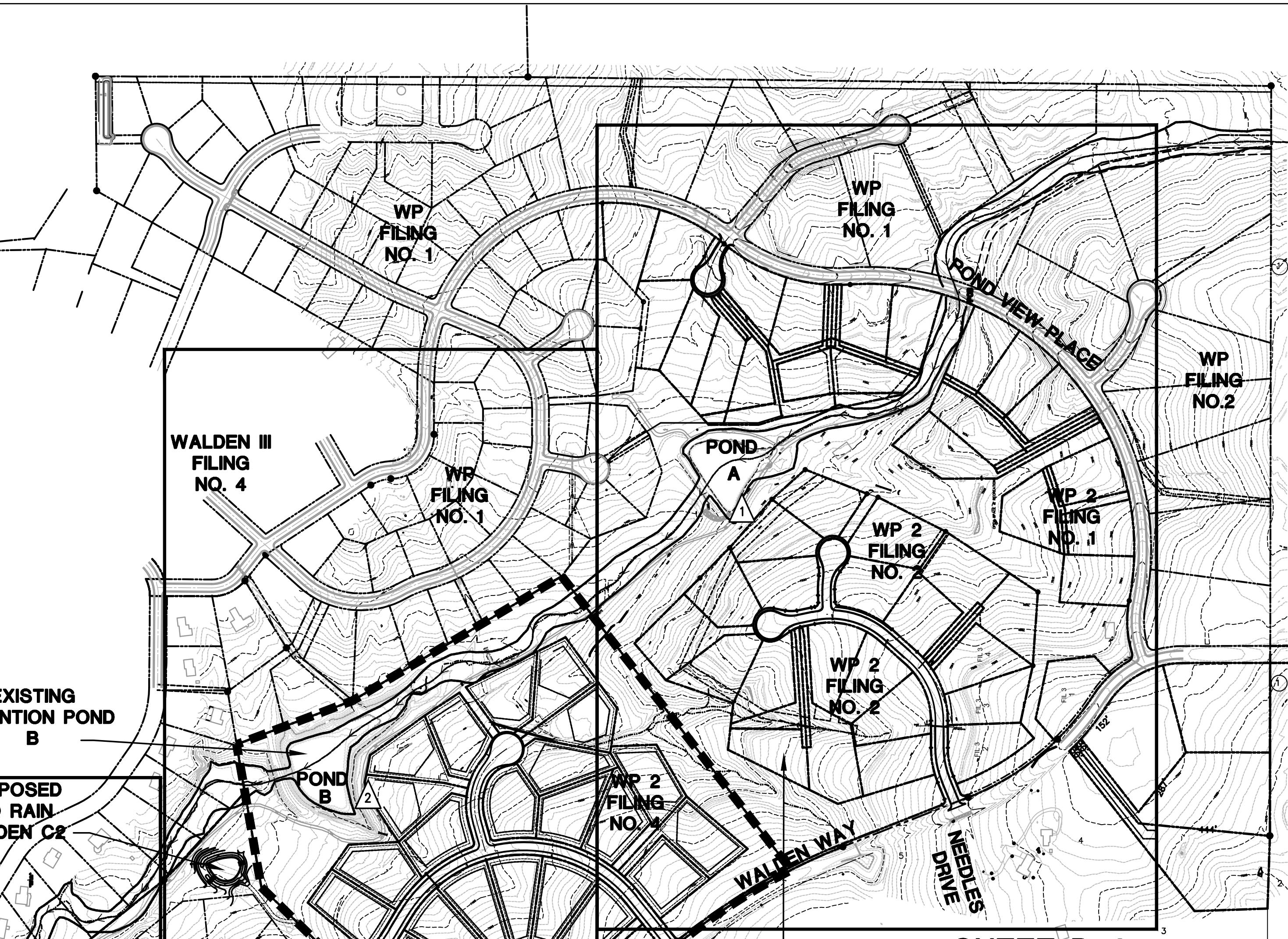
WALKER ROAD

**MONUMENT
ACADEMY
SITE**

EAST TRIBUTARY CHANNEL OF WEST CHERRY CREEK

**16 PROPOSED WATER
QUALITY POND C1**

KEY MAP



300' 0 300' 600'

 ORIGINAL SCALE: 1"=300' (24" x 36" SHEET)
 CONTOUR INTERVAL 5'

WALDEN PRESERVE

MASTER DEVELOPMENT DRAINAGE PLAN

SCALE:	1"=300'	DRAWN:	MJP
SCALE:	N/A	DESIGNED:	JPS
EYED:	RAMPART	CHECKED:	JPS
TED:	10/04/11	LAST MODIFIED:	10/03/19
ECT NO:	040201	MODIFIED BY:	BJJ

D1

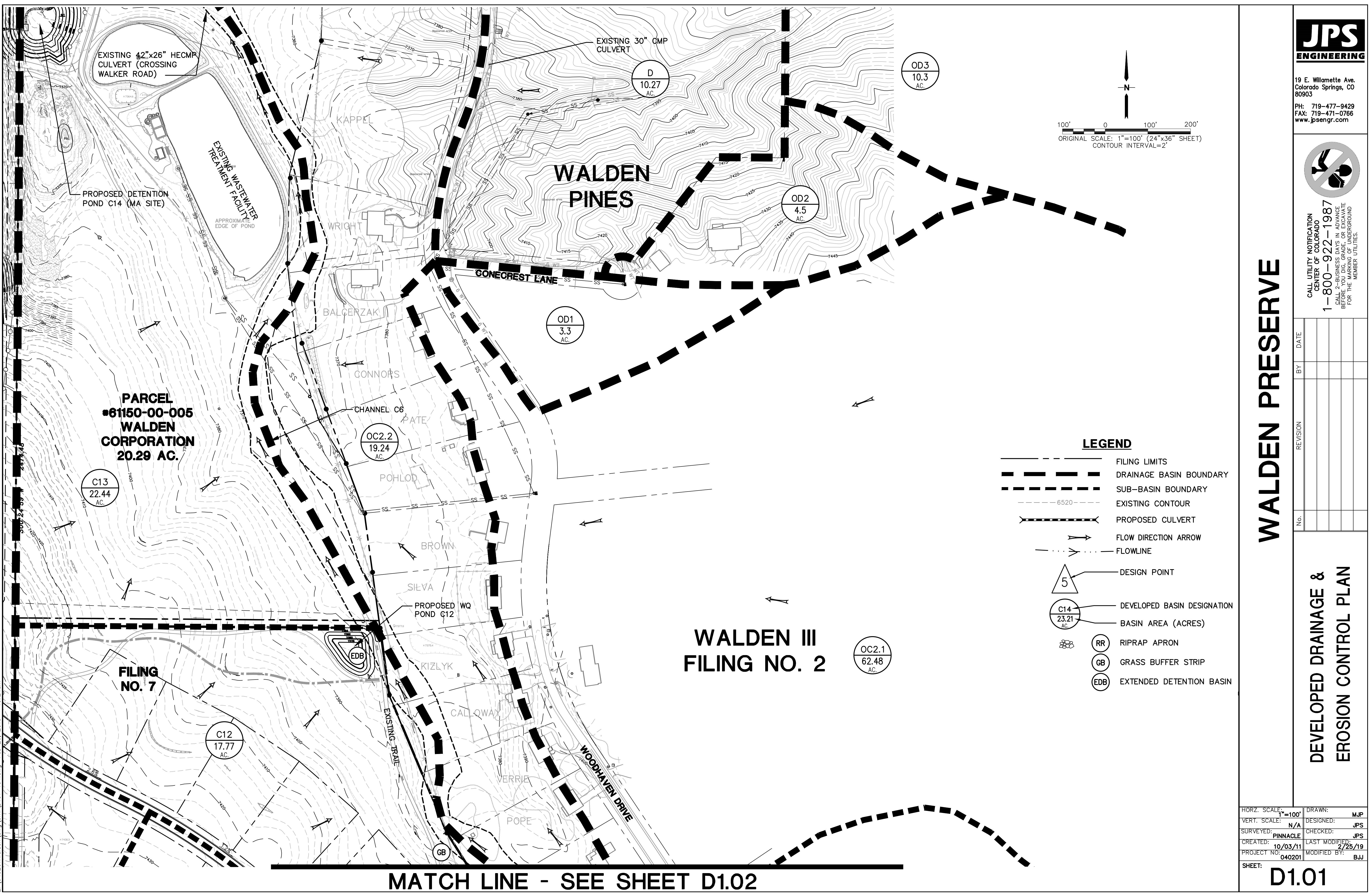
JPS
ENGINEERING

**19 E. Willamette Ave.
Colorado Springs, CO
80903**

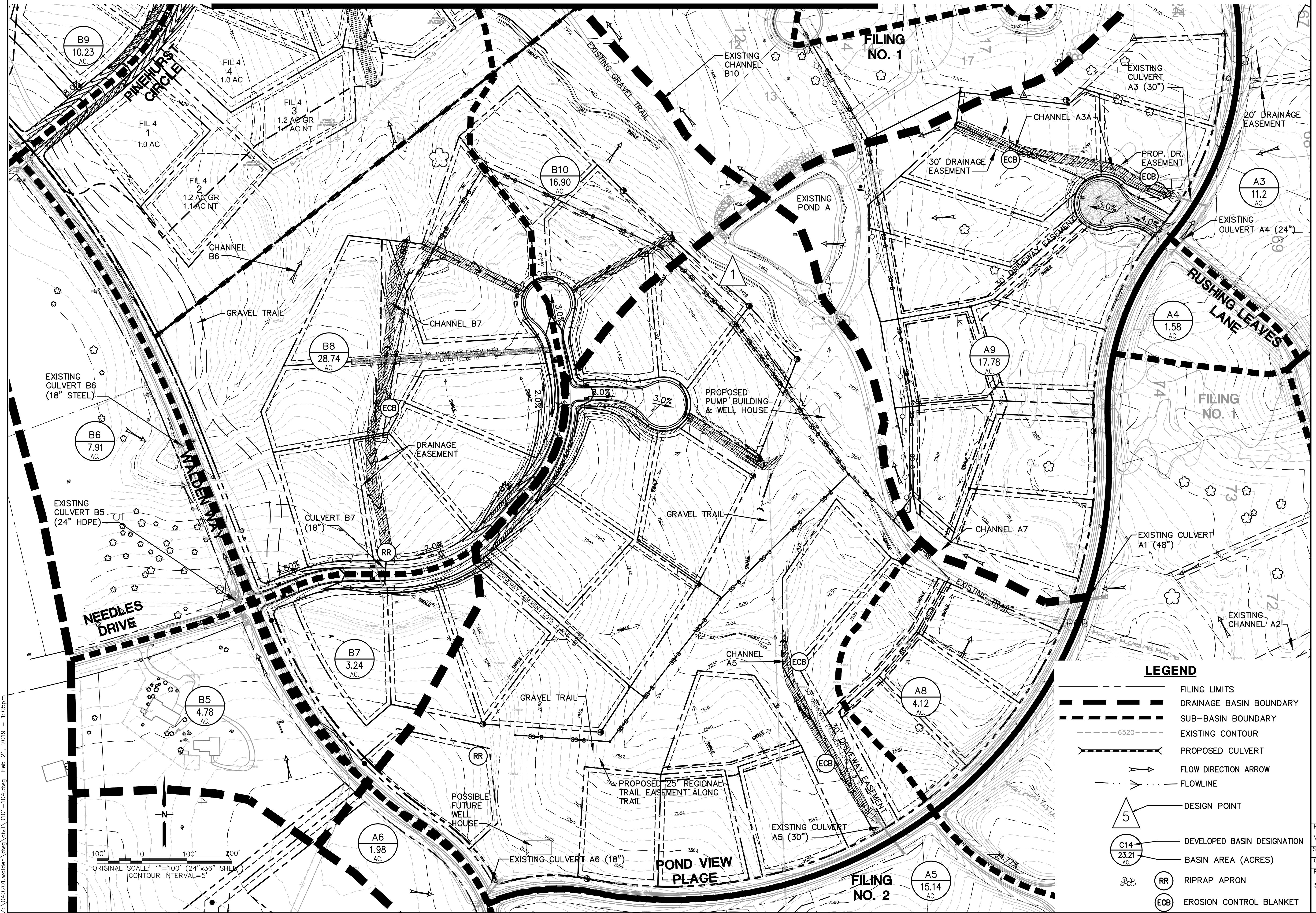


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CENTER OF COLORADO
—800-922-1981**

CALL 2-BUSINESS DAYS IN ADVANCE
BEFORE YOU DIG, GRADE, OR EXCAVATE
FOR THE MARKING OF UNDERGROUND
MEMBER UTILITIES.



MATCH LINE - SEE SHEET D1.03



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Colorado Springs, CO
80903
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