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Walden Preserve 2 Filing No. 4And Traffic Technical Memorandum (LSC #184810)

September 7, 2018

Revise all text from filing no. "4A" to filing no. "4"

Traffic Engineer's Statement

This traffic report and supporting information were prepared under my responsible charge and they comport with the standard of care. So far as is consistent with the standard of care, said report was prepared in general conformance with the criteria established by the County for traffic reports.

Developer's Statement

l, the Developer, have read and will comply with all commitmen	nts made on my behalf within this report.
	Date

Add "PCD File No. SF-18-034"



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September 7, 2018

Mr. Matt Dunston Walden Holdings 1, LLC 17145 Colonial Park Drive Monument, CO 80132

> RE: Walden Preserve 2 Filing No. 4A El Paso County, CO Traffic Technical Memorandum LSC #184810

Dear Matt:

LSC Transportation Consultants, Inc. has prepared this traffic technical memorandum for Filing 4A of the Walden Preserve 2 development. As shown in Figure 1, the site is generally located east of State Highway (SH) 83 and north of Hodgen Road, north of Colorado Springs in unincorporated El Paso County, Colorado. More specifically, the site is located northeast of the intersection of Walden Way and Pinehurst Circle (south intersection).

LSC prepared a traffic impact study (TIS) for the entire Walden Preserve 2 development dated September 14, 2014 and an addendum report for the Colorado Department of Transportation (CDOT) dated November 3, 2014. Since completion of the TIS and addendum report 43 lots for single-family homes have been platted in Filings 1 through 3 and public improvements required for those filings have been completed. An additional 23 lots are currently proposed to be platted as Filing 4A.

An amendment to the PUD Plan is proposed as part of this filing. The proposed amendment would shift one single-family lot from the north end of the development (future Filing 7) to the middle of the development (currently proposed Filing 4A). This change will have a negligible effect on the results/findings of the overall traffic report.

REPORT CONTENTS

This report presents:

Recent/current street and traffic conditions in the vicinity of the site for identification of
existing and planned street widths, lane geometries, traffic controls, posted speed limits,
street classification, etc.

- Intersection sight distance at the proposed access to this filing the intersection of Pinehurst Circle/Walden Way.
- Comparison of the current Walden Preserve 2 land uses to those shown in the PUD Plan TIS for the same land areas.
- The projected average weekday and peak-hour vehicle-trips to be generated by the proposed filing and amendment to the PUD Plan.
- The assignment of the projected trips to the existing and planned street system.
- The recommended street classifications for the internal streets within the proposed development.
- The project's obligation (if any) to the County roadway improvement fee program.
- The project's CDOT requirements per Access Permit No. 215017.

ROADWAY AND TRAFFIC CONDITIONS

Area Streets and Roads

The major roadways in the vicinity of the site are shown in Figure 1 and are described below.

State Highway (SH) 83 extends from Colorado Springs north to Parker and areas of southeast Denver. In the vicinity of the site, SH 83 is classified as a Regional Highway (R-A). At this location SH 83 is a two-lane rural highway with two- to four-foot shoulders and a speed limit of 60 miles per hour (mph). The intersections with Hodgen Road and Walker Road are signalized. The intersection with Walden Way is unsignalized with Stop-sign control for the westbound traffic.

Hodgen Road is a two-lane paved Rural Minor Arterial road that extends west from the intersection of Roller Coaster Road/Baptist Road to Eastonville Road. The speed limit on Hodgen Road is generally 55 mph east of SH 83.

Walden Way is a local roadway that extends southeast from SH 83 to the intersection of Timber Meadows Drive/Pond View Place.

Timber Meadows Drive is a Minor Collector roadway that extends south from the intersection of Walden Way/Pond View Place to just south of Hodgen Road.

Walker Road/Highway 105. Highway 105 west of State Highway 83 is a Principal Arterial and Walker Road east of State Highway 83 is a Collector roadway. Both are currently two-lane roadways but the Major Transportation Corridors Plan (MTCP) shows a future four-lane cross section on Highway 105 west of SH 83.

BACKGROUND

Walker Road Connection

Add a section regarding Timber Meadow Dr. What is it's current condition? Is the cross section to County Standard? Is there any off-site improvement responsibility by the Applicant with respect to Timber Meadow Dr and Timber Meadow Dr/Hodgens

The Walden Preserve 2 PUD plan intersection? re connection north to Walker Road (Pinehurst Circle). This will be a significant improvement to the traffic distribution system of the project and will result in a reduced traffic impact on both the north section of Walden Way just east of SH 83 and Timber Meadow Drive to the south. This connection is not required with this filing. However, the connection is required prior to any additional development beyond this filing.

The developers of Walden Preserve 2 are working with School District 38, which owns a 70-acre parcel on the southeast corner of the intersection of SH 83 and Walker Road. It is anticipated that a school will be built on the site in the short term. It is our understanding that the location of the Walden District wastewater treatment plant (1,400 feet west of Highway 83) has been selected for the intersection of Walker Road and the future connection.

SH 83/Walker/SH 105

At the time the PUD Plan TIS and amendment report were prepared the intersection of SH 83/Walker Road/SH 105 was two-way Stop-sign controlled. The need to signalize this intersection and estimates for fair share contributions towards the cost of the signal were a major focus of both reports. Since completion of the reports a traffic signal has been installed by CDOT. The November 3, 2014 addendum report identified a fair share contribution of 17.6 percent of the total cost for all of Walden Preserve 2. The currently proposed Filing 4A represents 19.8 percent of the total development (23 of 116 lots). CDOT Access Permit No. 215017 established the obligations for future subdivision filings (of which this Filing 4A is one). A copy of Access Permit No. 215017 is attached for reference. A CDOT access permit application will need to be submitted to CDOT for this filing for purposes of processing an amount due of \$6,714.69 for the previously identified fair share contribution associated with this filing to the traffic signal (now in place) at Highway 83 and Walker Road. This amount represents the prorated amount for 23 lots. (The original escrow table included in the Access Permit showed \$6,422.75 for 22 lots for this filing, which was previously called Filing 3.)

SH 83/Walden Way

The PUD Plan TIS assumed the intersection of SH 83/Walden Way would be restricted to right-in/right-out only. Prior to completion of the amendment report the applicant held discussions with the residents along Walden Way. The applicant indicated to LSC that many of the residents were resistant to either closing off the intersection entirely or installing major improvements to it, for instance constructing a raised island to prohibit left-turn movements and converting the intersection to a right in/right out. The amendment report therefore presented a revised analysis assuming no changes to this intersection. The report concluded that no improvements would be needed at this intersection in the foreseeable future.

Was there a CDOT access permit that specified converting to a RIRO? On the original study, what was the trigger which would warrant the change to a RIRO? Coordinate with CDOT for their requirements regarding any proposal to Highway 83.

LAND USE AND ACCESS

The PUD Plan TIS and amendment report assumed Walden Preserve 2 would be developed with 116 lots for single-family homes. The number of lots in the currently proposed amendment for the entire development remains the same but lot line adjustments have resulted in one additional lot in the area currently proposed as Filing No 4A and one less lot in the north end of the development shown as future Filing 7.

The currently proposed Filing 4A is planned to include **23 lots** for single-family homes. Access is proposed to an extension of Pinehurst Circle. The site plan is shown in Figure 2.

INTERSECTION SIGHT DISTANCE

The intersection sight distance at the planned new east leg of the intersection of Pinehurst Circle and Walden Way has been field-checked and meets County standards. The posted speed limit on Walden Way is 30 mph. The ECM-prescribed intersection sight distance is 335 feet. The field-measured sight distance is over 600 feet to the north and south along Walden Way.

TRIP GENERATION

The site-generated vehicle-trips were estimated using the nationally published trip generation rates from *Trip Generation*, *10th Edition*, *2017* by the Institute of Transportation Engineers (ITE). Table 1 shows the current trip generation estimate for the currently proposed Filing 4A and the entire Walden Preserve 2 development at buildout. Table 1 also shows the trip generation estimate from the PUD Plan TIS and amendment report for comparison. The trip generation estimate from the previous report was based on the trip generation rates from the *Trip Generation* 9th edition.

As shown in Table 1 Walden Preserve Filing 4A is projected to generate about 217 new vehicle-trips on the average weekday, with about one-half of the vehicles entering and one-half of the vehicles exiting in a 24-hour period. During the morning peak hour, which generally occurs for one hour between 6:30 and 8:30 a.m., about four vehicles would enter and 13 vehicles would exit the site. During the afternoon peak hour, which generally occurs for one hour between 4:30 and 6:30 p.m., about 14 vehicles would enter and eight vehicles would exit the site.

TRIP DISTRIBUTION AND ASSIGNMENT

The directional distribution of the site-generated traffic volumes on the adjacent roadway system is an important factor in determining the site's traffic impacts. The specific trip distribution estimates are shown in Figure 3. These estimates represent the percentages of the site-generated traffic volumes projected to be oriented to and from the major approaches to the site. The directional distribution estimates are based on the following factors: traffic counts conducted in the area; the location of the site with respect to the Colorado Springs metropolitan

area and other developed areas; the existing and planned roadway system serving the site, particularly SH 83 and Hodgen Road, and Highway 105; and the land uses proposed for the site.

When the distribution percentages (from Figure 3) are applied to the trip generation estimates (from Table 1), the resulting site-generated traffic volumes can be determined. Figures 4 and 5 show the daily and weekday morning and afternoon peak-hour short-term site-generated traffic volume estimates. The short-term site-generated traffic volumes shown in Figure 4 assume Pinehurst Circle has been extended north of Walden Way adjacent to Filing 4A only. The short-term site-generated traffic volumes shown in Figure 5 assume Pinehurst Circle has been extended north to Walker Road. Figure 6 shows the long-term site-generated traffic volumes. The long-term site-generated traffic volumes assume Pinehurst Circle has been extended north to Walker Road.

2040 TOTAL TRAFFIC

Please refer to PUD Plan TIS and amendment report for the 2040 total traffic volumes and level of service analysis.

SUBDIVISION STREET CLASSIFICATIONS

All streets within the currently proposed Walden Preserve 2 Filing 4A including the extension of Pinehurst Circle should be classified as Rural Local.

This contradicts the final plat note which

TRANSPORTATION IMPROVEMENT FEE PROGRAM

states road impact fee to be paid in full.

Verify with the developer and revise either the TIS or the Plat to match.

The proposed subdivision filing will be required to participate in the Countywide Transportation Improvement Fee Program. This project will annex into the 10 mil PID. Based on a per-lot upfront building permit fee of \$923 per dwelling unit, the total building permit fee amount for the 23 lots within Filing 4A would be \$21,229.

CDOT SIGNAL CONTRIBUTION PER ACCESS PERMIT NO. 215017

A CDOT access permit application will need to be submitted to CDOT for purposes of processing an amount due to CDOT of \$6,714.69 for the previously identified contribution associated with this filing to the traffic signal (now in place) at Highway 83 and Walker Road. This amount represents the prorated amount for 23 lots. (The original escrow table included in the Access Permit showed \$6,422.75 for 22 lots for this filing, which was previously called Filing 3.)

Provide an updated Traffic Signal Escrow Table (Exhibit A of the access permit) which includes the filing 3 and this filing 4A.

FYI: Provide an updated tally with each subsequent final plat application.

Please contact me if you have any questions regarding this report.

Respectfully Submitted,

LSC TRANSPORTATION CONSULTANTS, INC.

Jeffrey C. Hodsdon, P.E., PTOE

Principal

JCH:KDF:bjwb

Enclosures: Table 1

Figures 1-6

Walden Preserve 2 Amended PUD Plan

TIS for the overall PUD and CDOT Addendum Report (for reference)

CDOT Access Permit No. 215017 (included for reference)

1. Add a "recommendations and conclusions" section.

2. ECM B.1.3 notes that if the original TIS is older than 3 years, an entirely new TIS shall be prepared. Either comply with this section or submit a deviation request. If the deviation request is approved then a condition of approval will likely be placed to note that an entirely new TIS shall be prepared (w/ new traffic counts) with the subsequent final plat application.

Table 1 Walden Preserve 2 Filing No. 4A Trip Generation Estimate

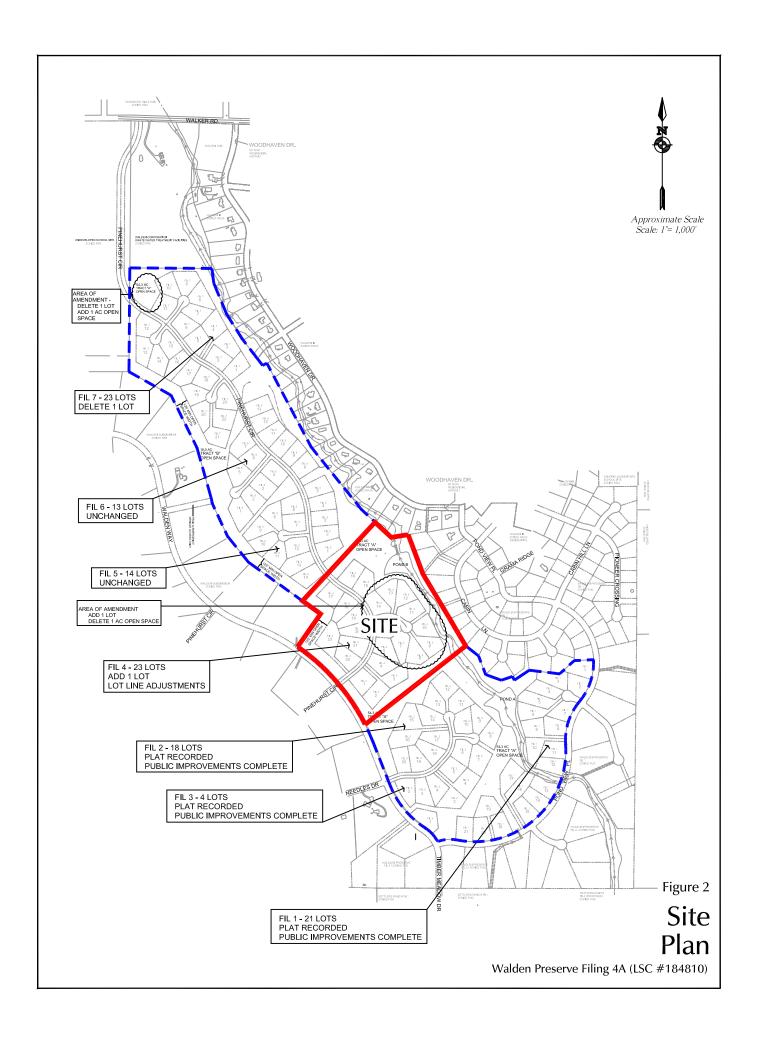
Land Use Description Filing ingle-Family Detached Housing ingle-Family Detached Housing ingle-Family Detached Housing ingle-Family Detached Housing	Trip Generation Units 23 DU ⁽²⁾ 21 DU 18 DU	Average Weekday Traffic 9.44	Mor Peak In 0.19	eration Raning Hour Out 0.56	After	Noon Hour Out	Average Weekday Traffic	Mor	os Genera ning Hour Out	After Peak In	
Description Filing ingle-Family Detached Housing ingle-Family Detached Housing ingle-Family Detached Housing	23 DU ⁽²⁾	Traffic 9.44	In 0.19	Out	ln	Out	Traffic	In	Out	In	Out
Filing ingle-Family Detached Housing ingle-Family Detached Housing ingle-Family Detached Housing	23 DU ⁽²⁾	9.44	0.19								
ingle-Family Detached Housing ingle-Family Detached Housing ingle-Family Detached Housing	21 DU			0.56	0.62	0.37	217	4	13	14	8
ingle-Family Detached Housing	21 DU			0.56	0.62	0.37	217	4	13	14	8
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ingle-Family Detached Housing	_	9.44	0.40								
0 ,	18 DU		0.19	0.56	0.62	0.37	198	4	12	13	8
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ingic-i airiily Detached Housing	4 DU	9.44	0.19	0.56	0.62	0.37	38	1	2	2	1
Total Filings 1, 2 & 3	43 DU						406	8	24	27	16
Total Filings 1-4	66 DU						623	12	37	41	24
ingle-Family Detached Housing	14 DU	9.44	0.19	0.56	0.62	0.37	132	3	8	9	5
ingle-Family Detached Housing	13 DU	9.44	0.19	0.56	0.62	0.37	123	2	7	8	5
ingle-Family Detached Housing	23 DU	9.44	0.19	0.56	0.62	0.37	217	4	13	14	8
Total Filings 5, 6 & 7	50 DU						472	9	28	31	18
Total at Buildout	116 DU						1,095	21	64	72	42
į	ingle-Family Detached Housing ingle-Family Detached Housing ingle-Family Detached Housing Total Filings 5, 6 & 7	ingle-Family Detached Housing 14 DU ingle-Family Detached Housing 13 DU ingle-Family Detached Housing 23 DU Total Filings 5, 6 & 7 50 DU	ingle-Family Detached Housing 14 DU 9.44 ingle-Family Detached Housing 13 DU 9.44 ingle-Family Detached Housing 23 DU 9.44 Total Filings 5, 6 & 7 50 DU	ingle-Family Detached Housing 14 DU 9.44 0.19 ingle-Family Detached Housing 13 DU 9.44 0.19 ingle-Family Detached Housing 23 DU 9.44 0.19 Total Filings 5, 6 & 7 50 DU	ingle-Family Detached Housing 14 DU 9.44 0.19 0.56 ingle-Family Detached Housing 13 DU 9.44 0.19 0.56 ingle-Family Detached Housing 23 DU 9.44 0.19 0.56 Total Filings 5, 6 & 7 50 DU	ingle-Family Detached Housing 14 DU 9.44 0.19 0.56 0.62 ingle-Family Detached Housing 13 DU 9.44 0.19 0.56 0.62 ingle-Family Detached Housing 23 DU 9.44 0.19 0.56 0.62 Total Filings 5, 6 & 7 50 DU	ingle-Family Detached Housing 14 DU 9.44 0.19 0.56 0.62 0.37 ingle-Family Detached Housing 13 DU 9.44 0.19 0.56 0.62 0.37 ingle-Family Detached Housing 23 DU 9.44 0.19 0.56 0.62 0.37 Total Filings 5, 6 & 7 50 DU	ingle-Family Detached Housing 14 DU 9.44 0.19 0.56 0.62 0.37 132 ingle-Family Detached Housing 13 DU 9.44 0.19 0.56 0.62 0.37 123 ingle-Family Detached Housing 23 DU 9.44 0.19 0.56 0.62 0.37 217 Total Filings 5, 6 & 7 50 DU 472	ingle-Family Detached Housing 14 DU 9.44 0.19 0.56 0.62 0.37 132 3 ingle-Family Detached Housing 13 DU 9.44 0.19 0.56 0.62 0.37 123 2 ingle-Family Detached Housing 23 DU 9.44 0.19 0.56 0.62 0.37 217 4 Total Filings 5, 6 & 7 50 DU 472 9	ingle-Family Detached Housing 14 DU 9.44 0.19 0.56 0.62 0.37 132 3 8 ingle-Family Detached Housing 13 DU 9.44 0.19 0.56 0.62 0.37 123 2 7 ingle-Family Detached Housing 23 DU 9.44 0.19 0.56 0.62 0.37 217 4 13 Total Filings 5, 6 & 7 50 DU 472 9 28	ingle-Family Detached Housing 14 DU 9.44 0.19 0.56 0.62 0.37 132 3 8 9 ingle-Family Detached Housing 13 DU 9.44 0.19 0.56 0.62 0.37 123 2 7 8 ingle-Family Detached Housing 23 DU 9.44 0.19 0.56 0.62 0.37 217 4 13 14 Total Filings 5, 6 & 7 50 DU 472 9 28 31

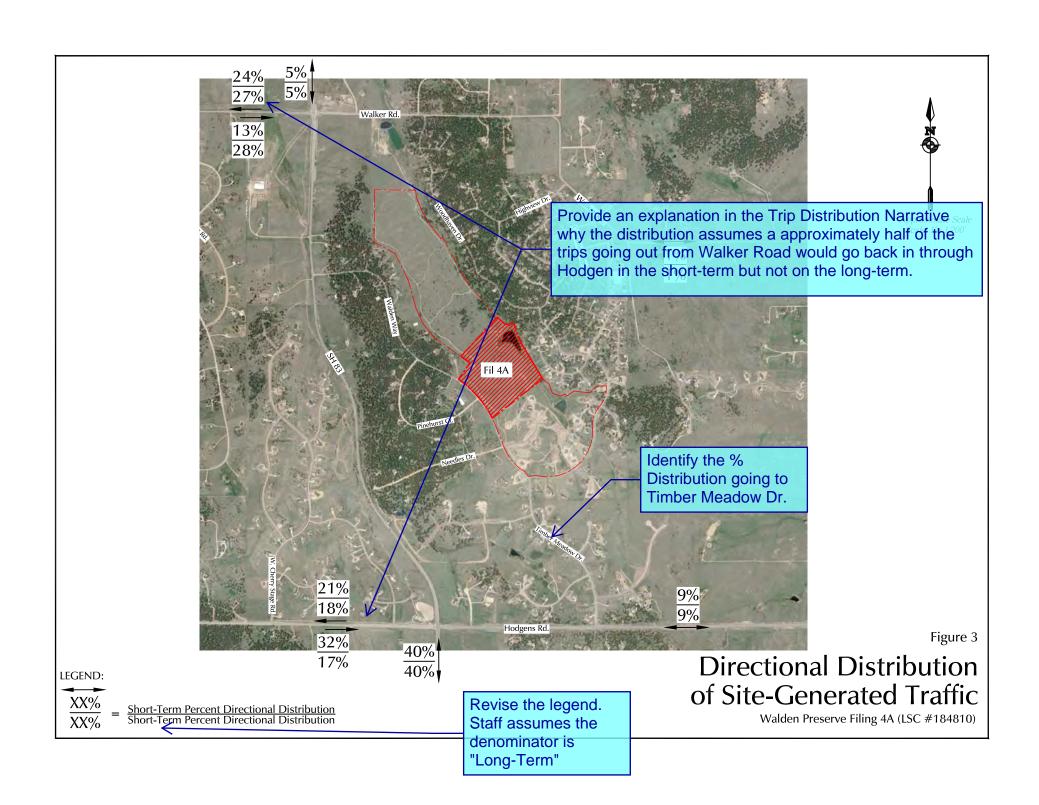
Notes:

(1) Source: "Trip Generation, 10th Edition, 2017" by the Institute of Transportation Engineers (ITE)

(2) DU = Dwelling Units







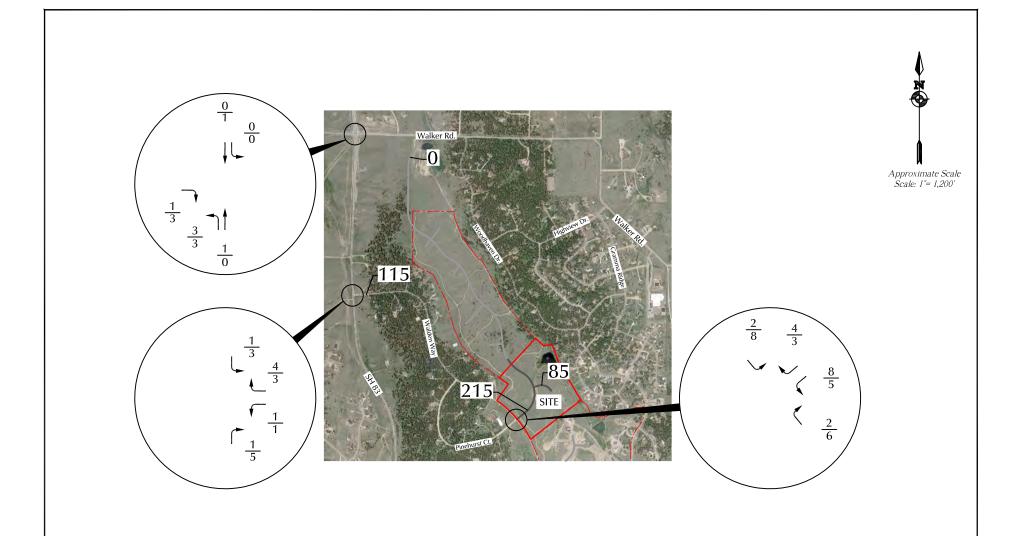


Figure 4

Assignment of Short-Term Site-Generated Traffic Without Pinehurst Extension

Walden Preserve Filing 4A (LSC #184810)

LEGEND:

XX XX = AM Weekday Peak-Hour Traffic (vehicles per hour)
PM Weekday Peak-Hour Traffic (vehicles per hour)

X,XXX= Average Weekday Traffic (vehicles per day)

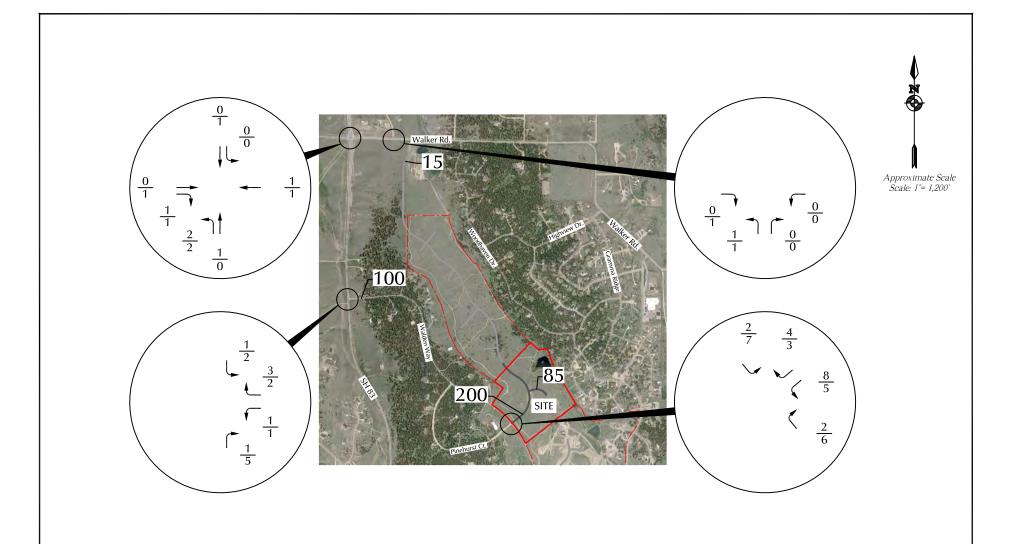


Figure 5

Assignment of Short-Term Site-Generated Traffic With Pinehurst Extension

LEGEND:

XX XX = AM Weekday Peak-Hour Traffic (vehicles per hour)
PM Weekday Peak-Hour Traffic (vehicles per hour)

X,XXX= Average Weekday Traffic (vehicles per day)

Walden Preserve Filing 4A (LSC #184810)

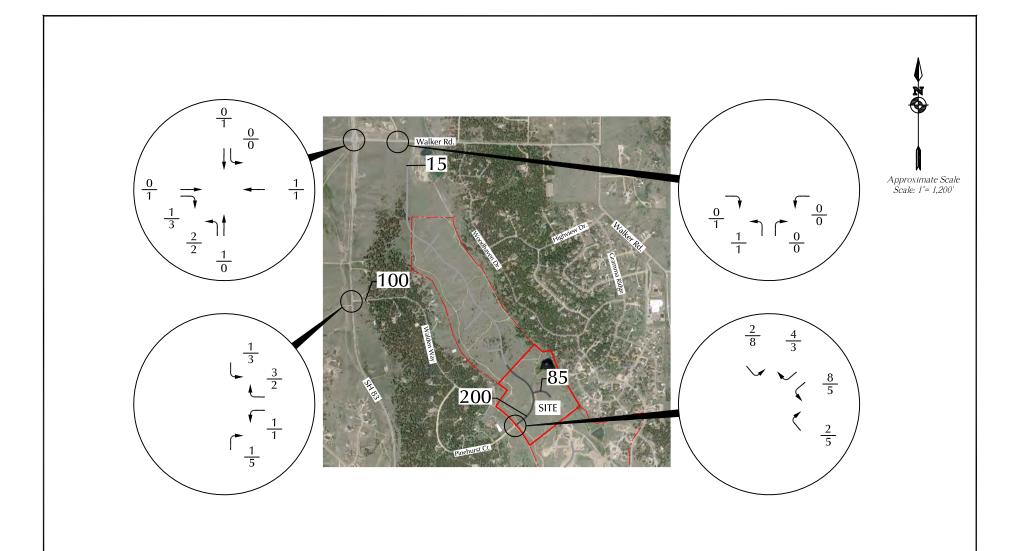


Figure 6

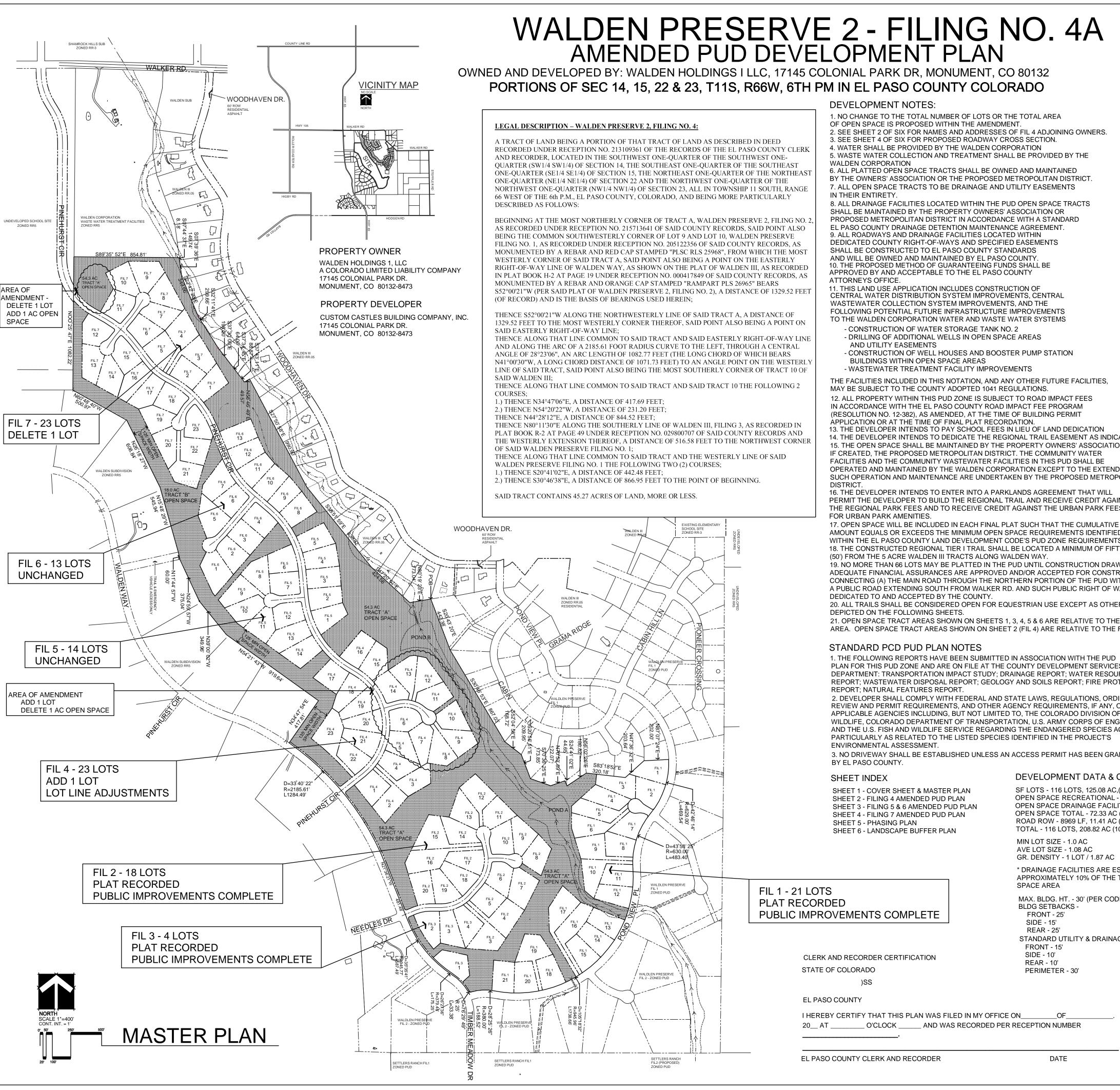
LEGEND:

XX XX = AM Weekday Peak-Hour Traffic (vehicles per hour)
PM Weekday Peak-Hour Traffic (vehicles per hour)

X,XXX= Average Weekday Traffic (vehicles per day)

Assignment of Buildout Site-Generated Traffic

Walden Preserve Filing 4A (LSC #184810)



DEVELOPMENT NOTES:

1. NO CHANGE TO THE TOTAL NUMBER OF LOTS OR THE TOTAL AREA OF OPEN SPACE IS PROPOSED WITHIN THE AMENDMENT.

2. SEE SHEET 2 OF SIX FOR NAMES AND ADDRESSES OF FIL 4 ADJOINING OWNERS. 3. SEE SHEET 4 OF SIX FOR PROPOSED ROADWAY CROSS SECTION. 4. WATER SHALL BE PROVIDED BY THE WALDEN CORPORATION 5. WASTE WATER COLLECTION AND TREATMENT SHALL BE PROVIDED BY THE

WALDEN CORPORATION 6. ALL PLATTED OPEN SPACE TRACTS SHALL BE OWNED AND MAINTAINED BY THE OWNERS' ASSOCIATION OR THE PROPOSED METROPOLITAN DISTRICT 7. ALL OPEN SPACE TRACTS TO BE DRAINAGE AND UTILITY EASEMENTS IN THEIR ENTIRETY.

8. ALL DRAINAGE FACILITIES LOCATED WITHIN THE PUD OPEN SPACE TRACTS SHALL BE MAINTAINED BY THE PROPERTY OWNERS' ASSOCIATION OR PROPOSED METROPOLITAN DISTRICT IN ACCORDANCE WITH A STANDARD EL PASO COUNTY DRAINAGE DETENTION MAINTENANCE AGREEMENT. 9. ALL ROADWAYS AND DRAINAGE FACILITIES LOCATED WITHIN DEDICATED COUNTY RIGHT-OF-WAYS AND SPECIFIED EASEMENTS SHALL BE CONSTRUCTED TO EL PASO COUNTY STANDARDS AND WILL BE OWNED AND MAINTAINED BY EL PASO COUNTY

APPROVED BY AND ACCEPTABLE TO THE EL PASO COUNTY 11. THIS LAND USE APPLICATION INCLUDES CONSTRUCTION OF CENTRAL WATER DISTRIBUTION SYSTEM IMPROVEMENTS, CENTRAL WASTEWATER COLLECTION SYSTEM IMPROVEMENTS, AND THE FOLLOWING POTENTIAL FUTURE INFRASTRUCTURE IMPROVEMENTS

- CONSTRUCTION OF WATER STORAGE TANK NO. 2 - DRILLING OF ADDITIONAL WELLS IN OPEN SPACE AREAS AND UTILITY EASEMENTS

- CONSTRUCTION OF WELL HOUSES AND BOOSTER PUMP STATION BUILDINGS WITHIN OPEN SPACE AREAS

- WASTEWATER TREATMENT FACILITY IMPROVEMENTS

THE FACILITIES INCLUDED IN THIS NOTATION, AND ANY OTHER FUTURE FACILITIES,

12. ALL PROPERTY WITHIN THIS PUD ZONE IS SUBJECT TO ROAD IMPACT FEES IN ACCORDANCE WITH THE EL PASO COUNTY ROAD IMPACT FEE PROGRAM (RESOLUTION NO. 12-382), AS AMENDED, AT THE TIME OF BUILDING PERMIT APPLICATION OR AT THE TIME OF FINAL PLAT RECORDATION. 13. THE DEVELOPER INTENDS TO PAY SCHOOL FEES IN LIEU OF LAND DEDICATION

14. THE DEVELOPER INTENDS TO DEDICATE THE REGIONAL TRAIL EASEMENT AS INDICATED. 15. THE OPEN SPACE SHALL BE MAINTAINED BY THE PROPERTY OWNERS' ASSOCIATION OR, IF CREATED. THE PROPOSED METROPOLITAN DISTRICT. THE COMMUNITY WATER FACILITIES AND THE COMMUNITY WASTEWATER FACILITIES IN THIS PUD SHALL BE OPERATED AND MAINTAINED BY THE WALDEN CORPORATION EXCEPT TO THE EXTEND SUCH OPERATION AND MAINTENANCE ARE UNDERTAKEN BY THE PROPOSED METROPOLITAN

16. THE DEVELOPER INTENDS TO ENTER INTO A PARKLANDS AGREEMENT THAT WILL PERMIT THE DEVELOPER TO BUILD THE REGIONAL TRAIL AND RECEIVE CREDIT AGAINST THE REGIONAL PARK FEES AND TO RECEIVE CREDIT AGAINST THE URBAN PARK FEES FOR URBAN PARK AMENITIES.

AMOUNT EQUALS OR EXCEEDS THE MINIMUM OPEN SPACE REQUIREMENTS IDENTIFIED WITHIN THE EL PASO COUNTY LAND DEVELOPMENT CODE'S PUD ZONE REQUIREMENTS. 18. THE CONSTRUCTED REGIONAL TIER I TRAIL SHALL BE LOCATED A MINIMUM OF FIFTY FEET (50') FROM THE 5 ACRE WALDEN III TRACTS ALONG WALDEN WAY. 19. NO MORE THAN 66 LOTS MAY BE PLATTED IN THE PUD UNTIL CONSTRUCTION DRAWINGS AND ADEQUATE FINANCIAL ASSURANCES ARE APPROVED AND/OR ACCEPTED FOR CONSTRUCTION CONNECTING (A) THE MAIN ROAD THROUGH THE NORTHERN PORTION OF THE PUD WITH (B) A PUBLIC ROAD EXTENDING SOUTH FROM WALKER RD. AND SUCH PUBLIC RIGHT OF WAY HAS BEEN

DEDICATED TO AND ACCEPTED BY THE COUNTY. 20. ALL TRAILS SHALL BE CONSIDERED OPEN FOR EQUESTRIAN USE EXCEPT AS OTHERWISE DEPICTED ON THE FOLLOWING SHEETS.

21. OPEN SPACE TRACT AREAS SHOWN ON SHEETS 1, 3, 4, 5 & 6 ARE RELATIVE TO THE TOTAL PROJECT AREA. OPEN SPACE TRACT AREAS SHOWN ON SHEET 2 (FIL 4) ARE RELATIVE TO THE FILING 4 FINAL PLAT.

STANDARD PCD PUD PLAN NOTES

1. THE FOLLOWING REPORTS HAVE BEEN SUBMITTED IN ASSOCIATION WITH THE PUD PLAN FOR THIS PUD ZONE AND ARE ON FILE AT THE COUNTY DEVELOPMENT SERVICES DEPARTMENT: TRANSPORTATION IMPACT STUDY: DRAINAGE REPORT: WATER RESOURCES REPORT: WASTEWATER DISPOSAL REPORT: GEOLOGY AND SOILS REPORT: FIRE PROTECTION REPORT; NATURAL FEATURES REPORT

2. DEVELOPER SHALL COMPLY WITH FEDERAL AND STATE LAWS, REGULATIONS, ORDINANCES, REVIEW AND PERMIT REQUIREMENTS, AND OTHER AGENCY REQUIREMENTS, IF ANY, OF APPLICABLE AGENCIES INCLUDING, BUT NOT LIMITED TO, THE COLORADO DIVISION OF WILDLIFE, COLORADO DEPARTMENT OF TRANSPORTATION, U.S. ARMY CORPS OF ENGINEERS AND THE U.S. FISH AND WILDLIFE SERVICE REGARDING THE ENDANGERED SPECIES ACT, PARTICULARLY AS RELATED TO THE LISTED SPECIES IDENTIFIED IN THE PROJECT'S ENVIRONMENTAL ASSESSMENT.

3. NO DRIVEWAY SHALL BE ESTABLISHED UNLESS AN ACCESS PERMIT HAS BEEN GRANTED BY EL PASO COUNTY.

SHEET INDEX

SHEET 1 - COVER SHEET & MASTER PLAN SHEET 2 - FILING 4 AMENDED PUD PLAN SHEET 3 - FILING 5 & 6 AMENDED PUD PLAN SHEET 4 - FILING 7 AMENDED PUD PLAN

SHEET 5 - PHASING PLAN SHEET 6 - LANDSCAPE BUFFER PLAN OPEN SPACE RECREATIONAL - 65.10 AC (31%) OPEN SPACE DRAINAGE FACILITIES - 7.23 AC (4%) OPEN SPACE TOTAL - 72.33 AC (35%) ROAD ROW - 8969 LF, 11.41 AC (5%) TOTAL - 116 LOTS, 208.82 AC (100%)

DEVELOPMENT DATA & CRITERIA

SF LOTS - 116 LOTS, 125,08 AC,(60%)

MIN LOT SIZE - 1.0 AC AVE LOT SIZE - 1.08 AC GR. DENSITY - 1 LOT / 1.87 AC * DRAINAGE FACILITIES ARE ESTIMATED TO BE APPROXIMATELY 10% OF THE TOTAL OPEN SPACE AREA

MAX. BLDG. HT. - 30' (PER CODE MEASUREMENT) **BLDG SETBACKS -**FRONT - 25' SIDE - 15'

REAR - 25' STANDARD UTILITY & DRAINAGE EASEMENTS FRONT - 15' SIDE - 10' REAR - 10' PERIMETER - 30'

I HEREBY CERTIFY THAT THIS PLAN WAS FILED IN MY OFFICE ON___ 20__ AT _____ O'CLOCK _____ AND WAS RECORDED PER RECEPTION NUMBER

EL PASO COUNTY CLERK AND RECORDER

- THE PURPOSE AND INTENT OF THE PUD ZONING IS TO PROVIDE FOR THE THE DEVELOPMENT OF 116 SINGLE FAMILY RESIDENTIAL LOTS AT A MINIMUM SIZE OF 1.0 AC. - AUTHORITY

THE PUD IS AUTHORIZED BY CHAPTER 4 OF THE EL PASO COUNTY LAND DEVELOPMENT CODE, AS ADOPTED PURSUANT TO THE COLORADO PLANNED UNIT DEVELOPMENT ACT OF 1972, AS AMENDED. - APPLICABILITY

THE PROVISIONS OF THIS PUD SHALL RUN WITH THE LAND. THE LANDOWNERS, THEIR SUCCESSORS, HEIRS, OR ASSIGNS SHALL BE BOUND BY THIS DEVELOPMENT PLAN, AS AMENDED AND APPROVED BY THE DEVELOPMENT SERVICES DEPARTMENT

- ADOPTION

THE ADOPTION OF THIS DEVELOPMENT PLAN SHALL EVIDENCE THE FINDINGS AND DECISIONS OF THE EL PASO COUNTY BOARD OF COUNTY COMMISSIONERS THAT THIS DEVELOPMENT PLAN FOR WALDEN PRESERVE 2 IS IN GENERAL CONFORMITY WITH THE EL PASO COUNTY MASTER PLAN, EL PASO COUNTY POLICY PLAN AND APPLICABLE SMALL AREA PLAN(S); IS AUTHORIZED UNDER THE PROVISIONS OF THE EL PASO COUNTY LAND DEVELOPMENT CODE; AND THAT THE EL PASO COUNTY LAND DEVELOPMENT CODE AND THIS DEVELOPMENT PLAN COMPLIES WITH THE COLORADO PLANNED UNIT DEVELOPMENT ACT OF 1972, AS AMENDED.

- RELATIONSHIP TO COUNTY REGULATIONS

DIRECTOR OR BOARD OF COUNTY COMMISSIONERS.

THE PROVISIONS OF THIS DEVELOPMENT PLAN SHALL PREVAIL AND GOVERN THE DEVELOPMENT OF WALDEN PRESERVE 2, PROVIDED, THAT WHERE THE PROVISIONS OF THIS DEVELOPMENT PLAN DO NOT ADDRESS A PARTICULAR SUBJECT. THE RELEVANT PROVISIONS OF THE EL PASO COUNTY LAND DEVELOPMENT CODE, AS AMENDED AND IN EFFECT AT THE TIME OF THE PUD PLAN APPROVAL (OR OWNER ACKNOWLEDGE TO PUD CHANGES WITH THE CODE), OR ANY OTHER APPLICABLE RESOLUTIONS OR REGULATIONS OF EL PASO COUNTY, SHALL BE APPLICABLE.

- ENFORCEMENT

TO FURTHER THE MUTUAL INTEREST OF THE RESIDENTS, OCCUPANTS, AND OWNERS OF THE PUD AND OF THE PUBLIC IN PRESERVATION OF THE INTEGRITY OF THIS DEVELOPMENT PLAN, THE PROVISIONS OF THIS PLAN RELATING TO THE USE OF LAND AND THE LOCATION OF COMMON OPEN SPACE SHALL RUN IN FAVOR OF EL PASO COUNTY AND SHALL BE ENFORCEABLE AT LAW OR IN EQUITY BY THE COUNTY WITHOUT LIMITATION ON ANY POWER OR REGULATION OTHERWISE GRANTED BY LAW.

WHERE THERE IS MORE THAN ONE PROVISION WITH THE DEVELOPMENT PLAN THAT COVERS THE SAME SUBJECT MATTER, THE PROVISION WHICH IS MOST RESTRICTIVE OR IMPOSES HIGHER STANDARDS OR REQUIREMENTS SHALL GOVERN.

- MAXIMUM LEVEL OF DEVELOPMENT

THE TOTAL NUMBER OF DWELLINGS OR THE TOTAL COMMERCIAL, BUSINESS, OR INDUSTRIAL INTENSITY SHOWN ON THE DEVELOPMENT PLAN FOR DEVELOPMENT WITHIN THE SPECIFIED PLANNING AREAS IS THE MAXIMUM DEVELOPMENT REQUESTED FOR PLATTING OR CONSTRUCTION (PLUS ANY APPROVED DENSITY TRANSFERS). THE ACTUAL NUMBER OF DWELLINGS OR LEVEL OF DEVELOPMENT MAY BE LESS DUE TO SUBDIVISION OR SITE DEVELOPMENT REQUIREMENTS, LAND CARRYING CAPACITY, OR OTHER REQUIREMENTS OF THE BOARD OF COUNTY COMMISSIONERS.

- PROJECT TRACKING

AT THE TIME OF ANY FINAL PLAT APPLICATION, THE APPLICANT SHALL PROVIDE A SUMMARY OF THE DEVELOPMENT TO DATE. TO DEVELOPMENT SERVICES DEPARTMENT, IN ORDER TO ASSURE MAXIMUM DEVELOPMENT LIMITS ARE NOT EXCEEDED.

"OWNERSHIP"

KNOW ALL MEN BY THESE PRESENTS THAT WALDEN HOLDINGS I, LLC, A COLORADO LIMITED LIABILITY COMPANY IS OWNER OF PROPERTY DESCRIBED AS PARCEL NUMBER 6123001023, WITHIN THE ACCOMPANYING LEGAL DESCRIPTION, WALDEN PRESERVE 2 FIL 4A PUD DEVELOPMENT PLAN.

IN WITNESS WHEREOF:

THE AFOREMENTIONED HAVE EXECUTED THESE PRESENTS THIS

MATTHEW W. DUNSTON, MANAGING MEMBER WALDEN HOLDINGS I, LLC

STATE OF COLORADO)

COUNTY OF EL PASO)

THE ABOVE AND FOREGOING STATEMENT WAS ACKNOWLEDGED BEFORE ME THIS ____ DAY OF __ _, 20__ BY MATTHEW W. DUNSTON

WITNESS MY HAND AND OFFICIAL SEAL: NOTARY PUBLIC

OWNERSHIP CERTIFICATION

MY COMMISSION EXPIRES: __

A QUALIFIED TITLE INSURANCE COMPANY DUELY QUALIFIED, INSURED, OR LICENSED BY THE STATE OF COLORADO, DO HEREBY CERTIFY THAT I/WE HAVE EXAMINED THE TITLE OF ALL LANDS DEPICTED AND DESCRIBED HEREON AND THAT TITLE TO SUCH LAND IS OWNED IN FEE SIMPLE BY WALDEN HOLDINGS I, LLC AT THE TIME OF THIS

STATE OF COLORADO

APPLICATION.

THE ABOVE AND FOREGOING STATEMENT WAS ACKNOWLEDGED BEFORE ME THIS ____ DAY OF __ ____, 20__ BY __

WITNESS MY HAND AND OFFICIAL SEAL NOTARY PUBLIC

MY COMMISSION EXPIRES:

COUNTY CERTIFICATION

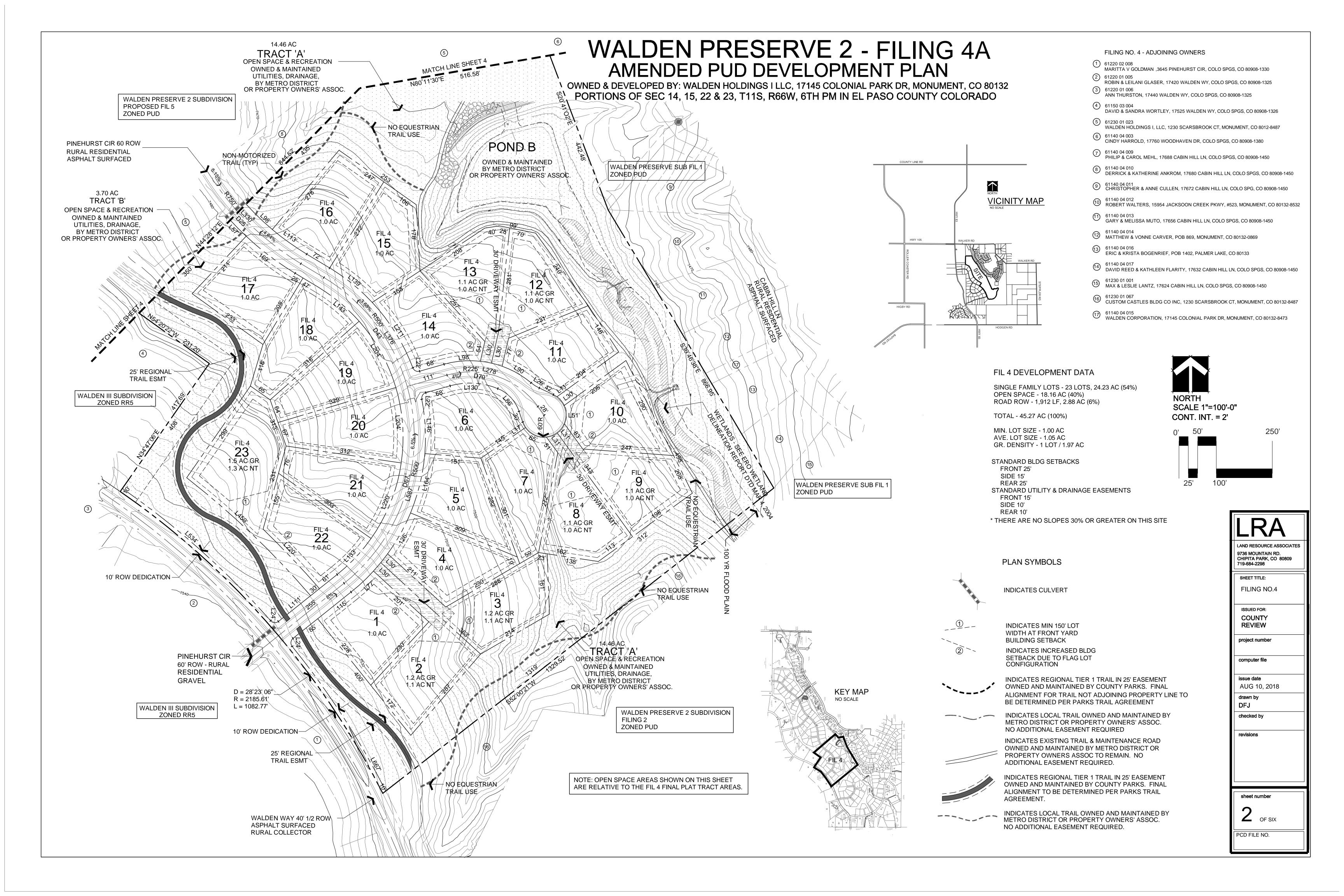
COUNTY REGULATIONS.

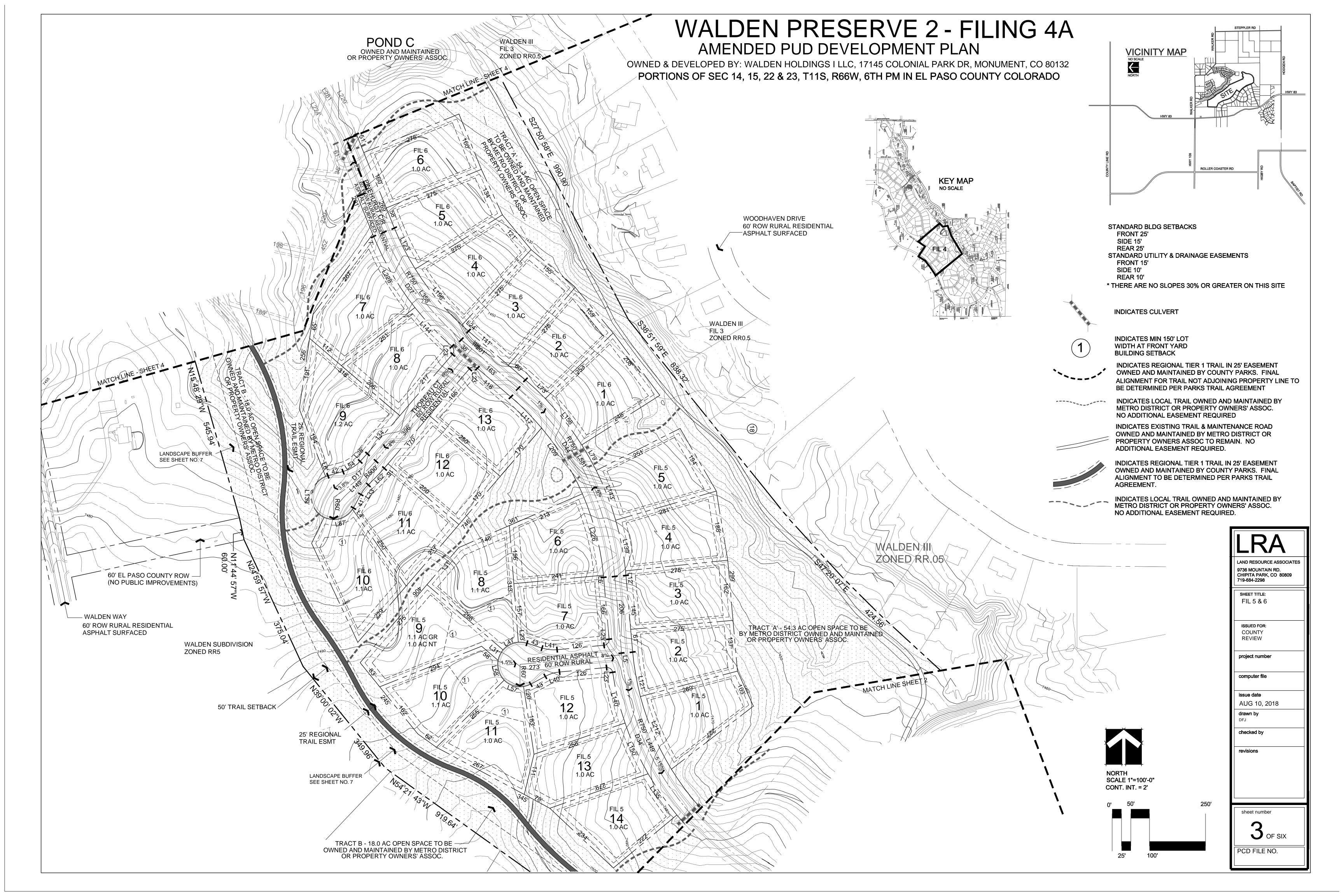
THIS REZONING REQUEST TO PUD HAS BEEN REVIEWED AND FOUND TO BE COMPLETE AND IN ACCORDANCE WITH THE BOARD OF RESOLUTION NO. APPROVING THE PUD AND ALL APPLICABLE EL PASO

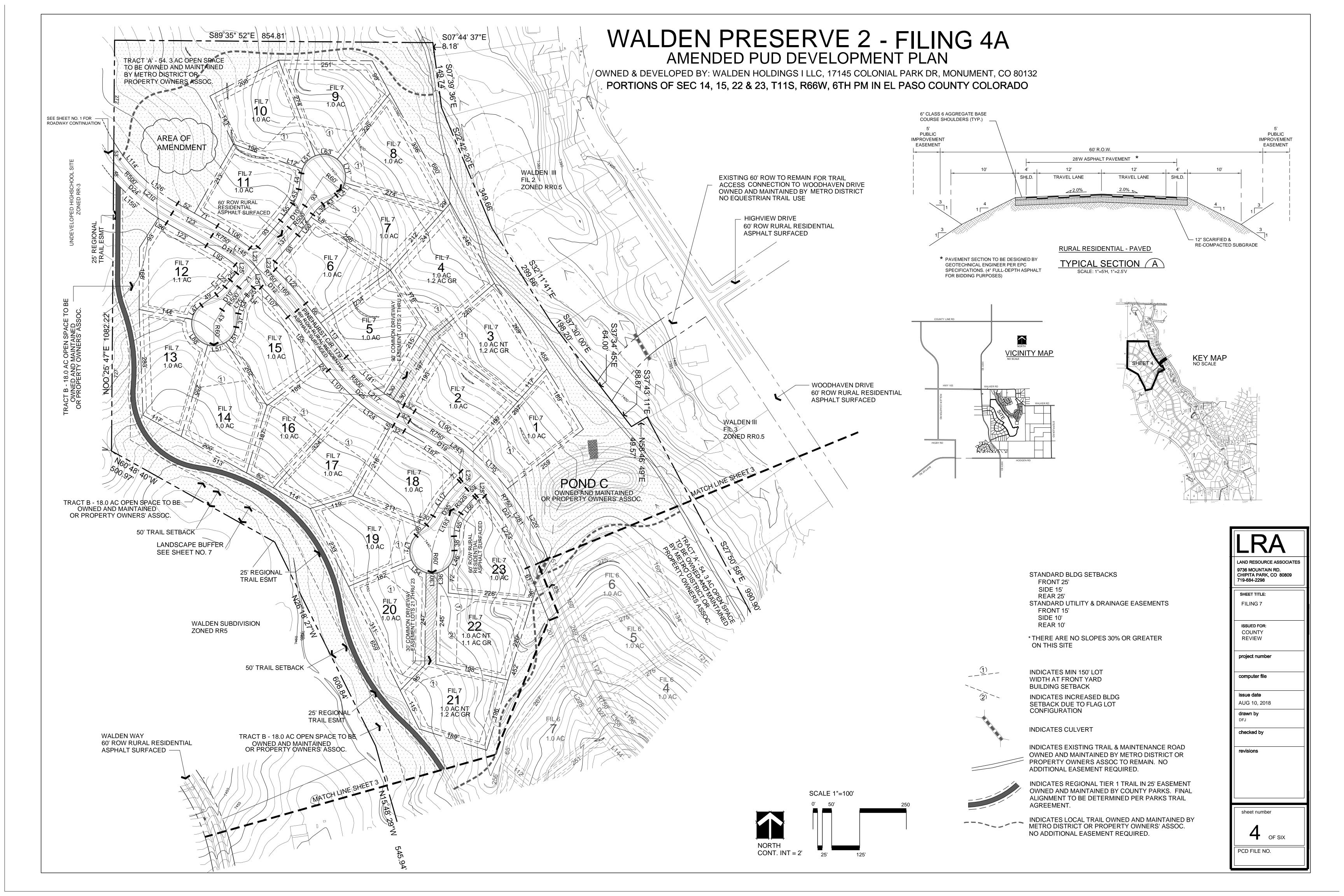
DIRECTOR, PLANNING AND COMMUNITY DEVELOPMENT

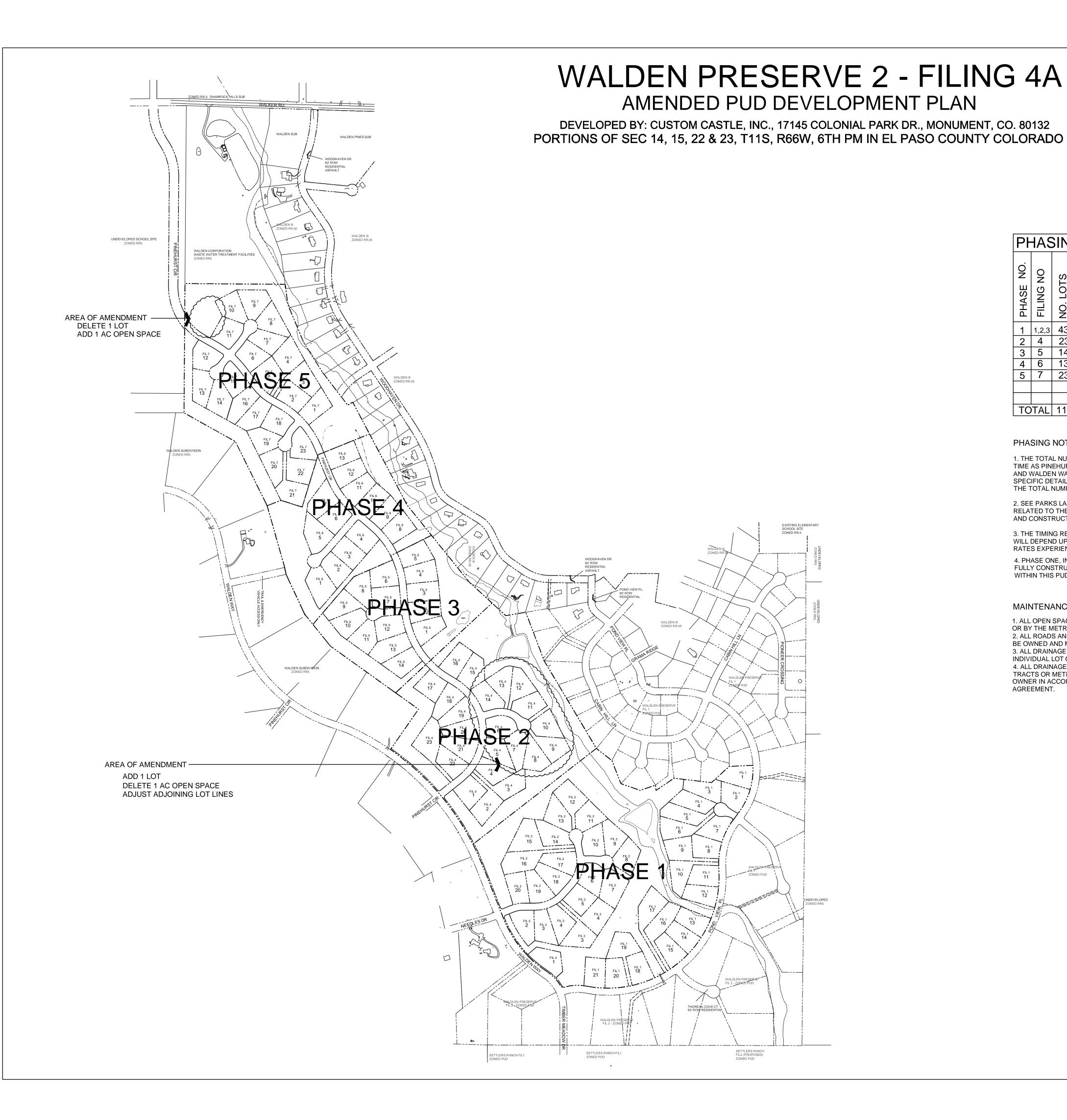
9736 MOUNTAIN RD. CHIPITA PARK, CO 80809 719-684-2298 SHEET TITLE: **COVER SHEET** ISSUED FOR: COUNTY REVIEW project number computer file issue date AUG 10, 2018 drawn by DFJ checked by DFJ revisions sheet number

PCD FILE NO.









Ph	3AF	SINC	3 DAT	A
PHASE NO.	FILING NO	NO. LOTS	OPEN SPACE AREA (AC)	TOTAL AREA (AC)
1	1,2,3	43	21.99	71.67
3	2 4 23		18.16	45.27
3	3 5 14		8.21	24.67
4			10.63	26.09
5	7	23	13.34	41.12
TO	TAL	116	72.33	208.82

PHASING NOTES

1. THE TOTAL NUMBER OF LOTS CANNOT EXCEED 66 UNTIL SUCH TIME AS PINEHURST CIRCLE IS COMPLETED BETWEEN WALKER ROAD AND WALDEN WAY. SEE BOCC PUD APPROVAL CONDITIONS FOR SPECIFIC DETAILS OF AGREEMENT. COMPLETION OF FIL 4 WILL BRING THE TOTAL NUMBER OF PLATTED LOTS TO 66.

2. SEE PARKS LAND AGREEMENT FOR PHASING REQUIREMENTS RELATED TO THE DEDICATION OF THE REGIONAL TRAIL EASEMENT AND CONSTRUCTION OF THE TIER ONE REGIONAL TRAIL.

3. THE TIMING RELATED TO THE CONSTRUCTION OF INDIVIDUAL PHASES WILL DEPEND UPON MARKET FORCES AND ACTUAL ABSORPTION

4. PHASE ONE, INCLUDING FILINGS 1, 2 & 3, HAVE RECORDED PLATS AND FULLY CONSTRUCTED PUBLIC IMPROVEMENTS AND ARE NOT INCLUDED WITHIN THIS PUD DEVELOPMENT PLAN AMENDMENT.

MAINTENANCE STATEMENT

1. ALL OPEN SPACE TRACTS TO BE OWNED AND MAINTAINED BY THE PROPERTY OWNERS' ASSOCIATION OR BY THE METROPOLITAN DISTRICT.

2. ALL ROADS AND DRAINAGE FACILITIES LOCATED WITHIN COUNTY DEDICATED ROWS TO BE OWNED AND MAINTAINED BY EL PASO COUNTY.

3. ALL DRAINAGE FACILITIES LOCATED ON PRIVATELY OWNED LOTS TO BE MAINTAINED BY THE INDIVIDUAL LOT OWNERS.

4. ALL DRAINAGE FACILITIES LOCATED WITHIN PROPERTY OWNERS' ASSOCIATION OWNED OPEN SPACE TRACTS OR METROPOLITAN DISTRICT OWNED OPEN SPACE TRACTS TO BE MAINTAINED BY PROPERTY OWNER IN ACCORDANCE WITH A STANDARD EL PASO COUNTY DRAINAGE DETENTION MAINTENANCE



REVIEW project number

ISSUED FOR:

COUNTY

computer file

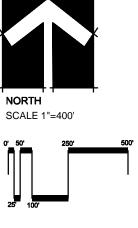
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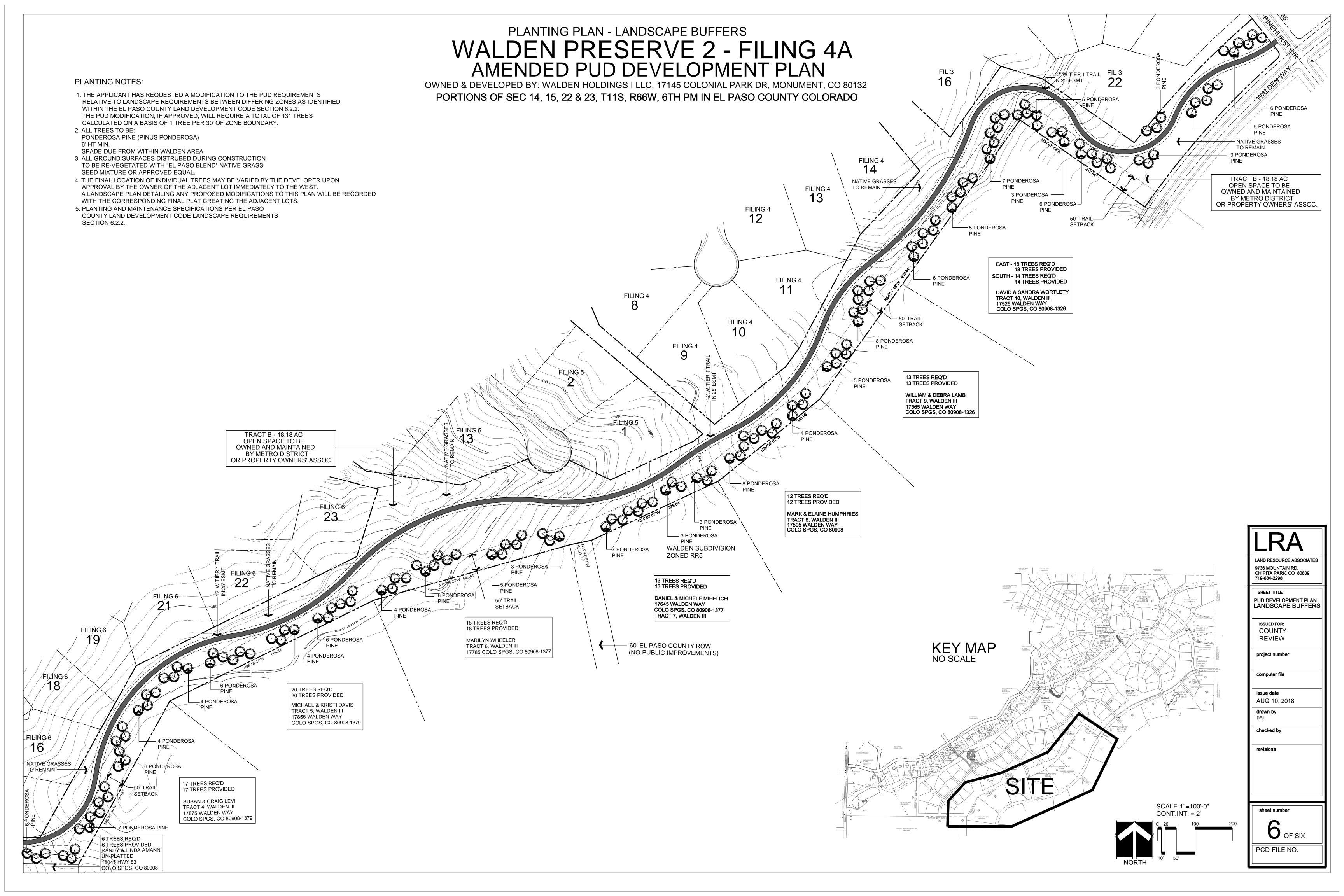
AUG 10, 2018 drawn by

DFJ checked by

revisions

PCD FILE NO.





LSC TRANSPORTATION CONSULTANTS, INC.



516 North Tejon Street Colorado Springs, CO 80903 (719) 633-2868 FAX (719) 633-5430 E-mail: lsc@lsccs.com

September 17, 2014

Mr. Matt Dunston Walden Holdings 1, LLC 17145 Colonial Park Drive Monument, CO 80132

RE: Walden Preserve 2

Preliminary Plan and Filings 1 and 2

Updated Traffic Impact Study

LSC #144380

Dear Mr. Dunston:

LSC has prepared this updated traffic impact report for the planned Walden Preserve 2 residential development. This report has been prepared for submittal with the Preliminary Plan and Filings 1 and 2 plat submittals. The previous traffic impact study was dated May 31, 2013 and was submitted with the PUD. The site is located east of State Highway (SH) 83 and north of Hodgen Road, north of Colorado Springs in unincorporated El Paso County, Colorado. The site is planned to include 116 single-family detached houses at buildout. Primary access will be to SH 83 on the west via Walden Way, to Hodgen Road on the south via Timber Meadows Drive, and to Walker Road via an extension of Pinehurst Circle (following Filing No. 3). The site location is shown on Figure 1.

This updated report is being prepared for submittal to El Paso County Development Services and the Colorado Department of Transportation (CDOT). The report contains an estimate of the vehicle trips to be generated by the proposed development, estimates of the projected site-generated traffic volumes on the area street system, and impacts of additional traffic on the roadway system. The report also includes recommendations for roadway system improvements to mitigate the traffic impacts. The report presents the estimated percentage contribution to the future signal at SH 83 and Walker and improvements at Walden Way/SH 83.

PREVIOUS REPORTS

The previous reports for Walden Preserve are dated December 8, 2005 and May 31, 2013 (the May 2013 report was for Walden Preserve 2). The December 8, 2005 report had originally shown the conversion of the State Highway 83/Walden Way intersection to a right-in/right-out. The May 31, 2013 report also showed this. This report continues to reflect the conversion to a right-in/right-out-

only intersection as shown on the approved PUD plan. A new Colorado State Highway Access Permit will be required for the SH 83/Walden Way intersection.

This report is an update to the May 31, 2013 report reflecting the approved PUD plan including the future connection north to Walker Road following the development of 66 lots. This report also has been updated to 2040 traffic and includes the potential development of the middle school site to the north. Aside from these changes (which, with the PUD plan approved, included the removal of the north connection to Walden Way previously shown) the plan is basically the same as the plan shown in the May 31, 2013 report.

BACKGROUND AND LAND USE PLAN

The development is located in a residential area. There are existing subdivisions surrounding the site. Figure 2 presents a context map of the Walden Preserve development site and the surrounding area. The figure shows the other area developments and vacant parcels.

Appendix Figure 1 shows a plan exhibit from the traffic study dated May 31, 2013 submitted with the PUD plan. This has been provided for reference.

Please refer to the PUD report dated May 31, 2013 for the complete history and explanation of this exhibit. This has been included as the signal warrant analysis included the older PUD Filings 1 and 2.

Phasing and Access

Figure 3 shows the approved PUD plan with the proposed phasing plan. The initial phase (Phases 1 and 2) will include the 42 lots located on the south end of the site. Access to this initial phase is planned on Pond View Place and to Walden Way aligning with Needles Drive.

Access to SH 83 would be at the existing Walden Way intersection. This access point is to be converted to a right-in/right-out.

Following Filing 3 (Phase 3), an extension of Pinehurst Circle would be constructed north from Filing 3 to Walker Road. Phases 3 through 7 would all have access to Pinehurst Circle.

ROADWAY AND TRAFFIC CONDITIONS

Area Streets and Roads

The major roadways in the vicinity of the site are shown in Figure 1 and are described below.

• State Highway (SH) 83 extends from Colorado Springs north to Parker and areas of southeast Denver. In the vicinity of the site, SH 83 is classified as a Regional Highway (R-A). At this location, SH 83 is a two-lane rural highway with two to four-foot shoulders and a speed limit of

60 miles per hour (mph). The intersection with Hodgen Road is signalized. The intersection with Walden Way is unsignalized with Stop-sign control for the westbound traffic. This intersection is planned to be converted to a right-in/right-out.

- **Hodgen Road** is a two-lane paved Rural Minor Arterial road which extends west from the intersection of Roller Coaster Road/Baptist Road to Eastonville Road. The speed limit on Hodgen Road is generally 55 mph east of SH 83.
- Walden Way is a local roadway which extends southeast from SH 83 to the intersection of Timber Meadows Drive/Pond View Place.
- **Timber Meadows Drive** is a Minor Collector roadway which extends south from the intersection of Walden Way/Pond View Place to just south of Hodgen Road.
- Walker Road/Highway 105. Highway 105 west of State Highway 83 is a Principal Arterial and Walker Road east of State Highway 83 is a Collector roadway. Both are currently two-lane roadways but the *Major Transportation Corridors Plan (MTCP)* shows a future four-lane cross section on Highway 105 west of SH 83. The intersection with SH 83 is unsignalized. This report assumes planned CDOT improvements at this intersection.

Existing (2012) Traffic and Lane Geometry

Figure 4 shows the current lane geometry plus weekday morning and afternoon peak-hour traffic count data. Peak-hour traffic volumes are shown for SH 83/Hodgen Road, SH 83/Walden Way, SH 83/Highway 105/Walker Road, Walden Way/Pond View Place, and Timber Meadows Drive/Hodgen Road. The peak-hour volumes are based on data collected by LSC in December 2011, April 2012, and November 2012. Figure 4 also shows the 2013 Average Annual Daily Traffic (AADT) on SH 83 based on data from the Colorado Department of Transportation and the average daily traffic on Pond View Place east of Walden Way based on a machine count by LSC in November 2012. The traffic count reports are attached.

Existing (2012) Levels of Service

The existing (2012) levels of service at the key area intersections are also shown in Figure 4. All of the analyzed intersections are shown to operate at acceptable levels of service. Further discussion and explanation on levels of service is presented later in this report.

Projected Future Background Traffic

Figures 5 and 6 show the projected background traffic volumes for the years 2017 and 2040, respectively, on the area roadway system. Background traffic is the traffic projected to be on the roadway system without consideration of Walden Preserve 2 traffic. The 2017 background traffic volumes include the through traffic and the traffic generated by the development of area vacant

parcels including the original Walden Preserve (not Walden Preserve 2) Filings 1 and 2, Settler's Ranch located just south of the site, and Majestic Pines but assume zero traffic generated by the site. The 2017 background traffic volumes also incorporate the conversion of the SH 83/Walden Way intersection to a right-in/right-out. The 2040 background traffic volumes assume the extension of Pinehurst Circle north to Walker Road. The 2040 background traffic includes traffic estimated to be generated by the development of the parcel located on the southeast corner of Walker/SH 83 as a middle school with access to Pinehurst Circle just south of Walker Road. The background traffic volumes are estimates by LSC based on CDOT 20-year growth factors and previous work completed by LSC in the vicinity of the site.

TRIP GENERATION

The Walden Preserve 2 development will contain 116 single-family detached houses upon completion. The amount of traffic to be generated by Walden Preserve 2 has been estimated using the nationally published trip generation rates found in *Trip Generation*, *9th Edition*, *2012* by the Institute of Transportation Engineers (ITE). The average weekday and peak-hour vehicle-trips have been estimated. Table 1 shows the results of the trip generation estimate.

As shown in Table 1, Phase 1 and Phase 2 combined are expected to generate about 400 vehicle-trips on the average weekday, with about 200 vehicles entering and 200 vehicles exiting during a 24-hour period. During the morning peak hour, about eight vehicles would enter and 24 vehicles would exit the site. During the afternoon peak hour, about 26 vehicles would enter and 16 vehicles would exit the site. The morning peak hour generally occurs for one hour between 6:30 and 8:30 a.m. and the afternoon peak hour generally occurs for one hour between 4:30 and 6:30 p.m.

At buildout, the development is expected to generate about 1,100 vehicle-trips on the average weekday, with about 550 vehicles entering and 550 vehicles exiting during a 24-hour period. During the morning peak hour, about 22 vehicles would enter and 65 vehicles would exit the site. During the afternoon peak hour, about 74 vehicles would enter and 49 vehicles would exit the site.

TRAFFIC DISTRIBUTION AND ASSIGNMENT

The directional distribution of the site-generated traffic volumes on the adjacent roadway system is an important factor in determining the site's traffic impacts. The specific trip distribution estimates are shown in Figure 7. These estimates represent the percentages of the site-generated traffic volumes projected to be oriented to and from the major approaches to the site. The directional distribution estimates are based on the following factors: traffic counts conducted in the area; the location of the site with respect to the Colorado Springs metropolitan area and other developed areas; the existing and planned roadway system serving the site, particularly SH 83 and Hodgen Road, and Highway 105; and the land uses proposed for the site.

TRAFFIC IMPACTS

Site-Generated Traffic

When the distribution percentages (from Figure 7) are applied to the trip generation estimates (from Table 1), the resulting site-generated traffic volumes can be determined. Figures 8 and 9 show the daily and weekday morning and afternoon peak-hour site-generated traffic volume estimates for Phases 1 and 2 and at buildout. The Phases 1 and 2 site-generated traffic represents the additional traffic from the 42 lots located on the south end of the site. The buildout site-generated traffic represents the traffic from the 116 new lots.

Years 2017 and 2040 Total Traffic

The total traffic volumes for the years 2017 and 2040 are shown in Figures 10 and 11, respectively. The 2017 total traffic volumes are the sum of the 2017 background traffic volumes (from Figure 5) plus the initial phase site-generated traffic volumes (from Figure 8). The 2040 total traffic volumes are the sum of the 2040 background traffic volumes (from Figure 6) plus the buildout site-generated traffic volumes (from Figure 9).

Projected Levels of Service

Level of service (LOS) is a quantitative measure of the level of congestion or delay at an intersection. Level of service is indicated on a scale from "A" to "F." LOS A is indicative of very little congestion or delay. LOS F is indicative of a high level of congestion or delay

The SH 83/Hodgen Road, SH 83/Walden Way, Hodgen Road/Timber Meadow Drive, and Walden Way/Timber Meadows/Pond Place intersections have been analyzed to determine the existing and projected levels of service using Synchro. The level of service analysis results are shown in Figures 10 and 11. The level of service reports are attached. Unsignalized intersection levels of service are expressed in terms of the levels of service of specific turning movements/approaches—most notably, the minor street approach or specific turning movements. Signalized intersections also include the level of service for the overall intersection.

- SH 83/Hodgen Road: The intersection of SH 83/Hodgen Road is projected to continue to operate at a satisfactory level of service during the morning and afternoon peak hours based on 2017 and 2040 total traffic volumes.
- SH 83/Walden Way: The right-in/right-out SH 83/Walden Way intersection is projected to operate at LOS B for the westbound right-turn movement during the morning and afternoon peak hours based on the projected 2017 and 2040 total traffic volumes.
- Hodgen Road/Timber Meadow Drive: The Stop-sign-controlled Hodgen Road/Timber Meadow Drive intersection is projected to operate at a satisfactory level of service during the

morning and afternoon peak hours for the side street approaches based on the projected 2017 and 2040 total traffic volumes.

- Timber Meadows Drive/Walden Way/Pond View Place: The Stop-sign-controlled Timber Meadows/Walden Way/Pond View intersection is projected to operate at a satisfactory level of service during peak hours for all approaches based on projected 2017 and 2040 total traffic volumes.
- SH 83/Walker Road/Highway 105: By 2017, with assumed increases in background traffic alone, the minor street approach left and through movements would see delay in the E and F ranges during peak periods. However, these movement levels of service would improve once the intersection is signalized. Once signalized, this intersection is projected to operate at a satisfactory level of service based on projected 2017 and 2040 total traffic volumes.
- Pinehurst Circle/Walker Road: Pinehurst Circle was assumed to be extended north to Walker Road some time after Phases 1 and 2 but before the 67th lot is constructed in Walden Preserve 2. The new intersection of Pinehurst/Waker is projected to operate at a satisfactory level of service as a two-way stop-sign-controlled intersection based on the projected 2040 total traffic volumes.

Traffic Signal Warrant Analysis

LSC has completed a traffic signal warrant analysis for the SH 83/Highway 105/Walker Road intersection to estimate both the timing of the signal (based on the intersection meeting warrants) and the percentage of traffic by this project at the time the signal may become warranted. LSC estimates that warrants may be met by 2018 assuming growth in background traffic, buildout of Walden Preserve (which may not occur by 2048), the restriction of SH 83/Walden Way to right-right-out only and the connection of Pinehurst Circle to Walker Road. The traffic from this project would constitute 15 percent of the eastbound and westbound approach volumes based on the total estimated traffic volumes at the time it is projected to meet a four-hour volume warrant.

The estimated warrants are based on the turning movement counts completed at the SH 83/Highway 105 intersection, estimated growth in through traffic, a projected shift in existing traffic volumes due to the restriction of Walden Way/SH 83 to right-in/right-out only, and the extension of Pinehurst Circle to Walker Road, and site-generated traffic estimates at buildout of Walden Preserve 2.

Table 2 shows the peak-hour traffic volumes for background and site traffic. The attached Appendix Figure 2 shows the specific breakdown of the traffic volumes assumed in this analysis and the total 2018 volumes. Table 3 shows the peak-hour traffic expanded to two hours in the morning peak and two hours in the afternoon peak. Table 4 shows Table 3 volumes with a growth factor applied to the background traffic volumes. The factor was increased from 1.0 until the resulting four hours worth of volumes were shown to meet the thresholds for the four-hour volumes. With the warrants shown to be met with a growth factor of 1.2, this could potentially translate to approximately year 2018.

Table 4 also shows the calculated site-generated percentage of the total at the time the signal is projected to meet a warrant based on minor street approach left and through turning movements only. This percentage is based on a weighted average of the site-generated to total percentages of all four hours analyzed.

Average Daily Traffic Impacts

Each of the figures shows the projected average daily traffic volumes on the roadway sections.

CONCLUSIONS AND RECOMMENDATIONS

Trip Generation

The initial two plats within the Walden Preserve development (Phase 1 and Phase 2) are collectively expected to generate about 400 vehicle-trips on the average weekday, with about 200 vehicles entering and 200 vehicles exiting during a 24-hour period. During the morning peak hour, about eight vehicles would enter and 24 vehicles would exit the site. During the afternoon peak hour, about 26 vehicles would enter and 16 vehicles would exit the site.

At buildout, the Walden Preserve development is expected to generate about 1,100 vehicle-trips on the average weekday, with about 550 vehicles entering and 550 vehicles exiting during a 24-hour period. During the morning peak hour, about 22 vehicles would enter and 65 vehicles would exit the site. During the afternoon peak hour, about 74 vehicles would enter and 49 vehicles would exit the site.

Average Daily Traffic Impacts

The figures show the projected average daily traffic volumes on the roadway sections within the Walden Preserve development.

Projected Levels of Service

Figures 5, 6, 10, and 11 show the level of service analysis results. The recommended traffic control and lane geometry for the years 2017 and 2040 are shown in Figures 10 and 11, respectively. Level of service was discussed in detail previously in this report.

All intersections are projected to operate at acceptable levels of service through the horizon year. The exception is anticipated to be the minor street approaches at the SH 83/Walker Road intersection prior to the traffic signal being warranted. CDOT has indicated that this project will be required to participate in the future traffic signal at this intersection.

Recommendations

SH 83/Walden Way

LSC recommends that the Walden Way/SH 83 intersection be restricted to right-in/right-out with Phase 1. As the southbound left-turn lane is currently warranted, the conversion to right-in/right-out would need to occur with the first phase. New development traffic added to the Walden Way access to SH 83 would need to be approved by CDOT through the access permit process. Also, a right-turn acceleration lane would not be required. The right-in/right-out access would need to be designed to physically prevent left-turning movements. Given the rural, high-speed design of the road, LSC recommends that a right-turn channelizing island be installed using a raised channelizing island with beveled curb (not vertical curb) set back from the edge of the northbound through lane. LSC also recommends that the island include narrow extensions for a short distance to the north and south to further discourage left turning movements. The design should also include pavement markings and breakaway object markers. "No left turn" signs must also be used. The intersection will remain Stopsign controlled. A northbound right-turn deceleration lane is not required by code, however a portion of a right-turn lane will be needed for northbound right-turning traffic to maneuver to the right of the channelizing island. Attached is a preliminary concept for the intersection improvements. The northbound right-turn lane may be shortened in the final design, pending CDOT approval, as a northbound right-turn deceleration lane is not required based on turning volumes.

Hodgen Road/Timber Meadow Drive

No further improvements will be necessary.

Hodgen Road

Hodgen Road east of SH 83 has recently been upgraded with a PPRTA project. No further widening of Hodgen Road would be necessary as a result of this project. The intersection with SH 83 is signalized.

SH 83/Walker Road/Highway 105

CDOT has completed intersection improvements including additional laneage and traffic islands to channelize right-turn movements on the eastbound and westbound intersection approaches. Side-street-traffic-actuated flashing yellow warning beacons have also been installed on the northbound and southbound approaches to the intersection for safety. As growth continues to occur in the area and through traffic increases along SH 83, a traffic signal is expected to be warranted (LSC growth assumptions would translate to a signal being warranted by 2020).

Calculated Percentage Toward the Future Traffic Signal

Based on calculations presented in Table 4, LSC estimates that at buildout, Walden Preserve traffic at this intersection would constitute 15.5 percent of the total volume for the eastbound/westbound

through/left turning movements. The following Table 5 presents the suggested percentages by filing to be included as part of the access permit from CDOT.

Table 5 Calculated Percentage Contribution SH 83/Walker Road Signal					
Filing No. 1	5.88% (including traffic from original Filings 1 and 2)				
Filing No. 2	1.37%				
Filing No. 3	1.53%				
Walker Connection is Installed					
Filing No. 4	1.55%				
Filing No. 5	1.44%				
Filing No. 6	3.81%				
Project Total	15.58%				

Funds escrowed to CDOT for a future traffic signal at this intersection should be eligible for credit and reimbursement through the countywide fee program. The Highway 83 and Highway 105 intersection is the intersection of MTCP roadways. The applicant will need to go before the Fee Advisory Committee to request approval of credit.

Walker Road/Pinehurst Circle

Based on the criteria contained in the El Paso County *Engineering Criteria Manual* and the projected 2040 total traffic volumes, an eastbound right-turn deceleration lane and a westbound left-turn lane will be required on Walker Road approaching Pinehurst Circle. The need for these lanes is primarily due to the assumption that the parcel located southeast of SH 83 and Walker Road will be developed as a middle school with access to Pinehurst Circle.

Pedestrian Trail Crossings

The **trail approaches** (not roadway approaches) to the roadway trail intersections should be posted with Stop-signs if these are intended to be used by cyclists. If the sight distance is limited and the trails are determined to be significant enough to warrant them, advance yellow warning signs for pedestrian crossings could be installed along with pavement markings in the form of crosswalks as appropriate per the *Manual on Uniform traffic Control Devices (MUTCD*).

Roadway Classifications

Figure 12 shows the recommended street classifications in the vicinity of the site. Streets within the first two plats would be rural local streets.

Intersection Cost Sharing Analysis

LSC has prepared a cost sharing analysis for the Walden Way/Highway 83 right-in/right-out improvements. The results of this analysis suggest a contribution of 23 percent from Majestic Pines and the remaining 77 percent from Walden Preserve 2. Either project could file for cost recovery against other future developments if it is anticipated that said other developments' traffic would benefit from the completion of this improvement (i.e., if another projects' traffic would otherwise add turning movements to the southbound left turn at this intersection if the improvement were not completed).

Countywide Roadway Improvement Fee Program

This project will be required to participate in this fee program.

* * * * *

We trust this traffic impact study will assist you in gaining approval of the proposed Walden Preserve 2 Preliminary Plan and first two plats. Please contact me if you have any questions or need further assistance.

Sincerely,

LSC TRANSPORTATION CONSULTANTS, INC.

Jeffrey C. Hodsdon, P.E., PTOI

Principal

JCH:bjwb

Enclosures: Tables 1-4

Figures 1-12

Appendix Figures 1-2 Traffic Count Reports Level of Service Reports

Table 1 Walden Preserve PUD Development Plan Trip Generation Estimate

ITE		Trip Generation Rates (1) Total Trips Generated										
Land Use	Land Use	Trip Generation	Average Weekdav		ning Hour		rnoon Hour	Average Weekday		ning Hour		rnoon Hour
Code	Description	Units	Traffic	ln	Out	ln	Out	Traffic	ln	Out	ln	Out
Phase 1 210	Single-Family Detached Housing	42 DU ⁽²⁾	9.52	0.19	0.56	0.63	0.37	400	8	24	26	16
Buildou 210	t Single-Family Detached Housing	116 DU ⁽²⁾	9.52	0.19	0.56	0.63	0.37	1,104	22	65	73	43

Notes:

(1) Source: "Trip Generation, 9th Edition, 2012" by the Institute of Transportation Engineers (ITE)

(2) DU = Dwelling Units

Table 2 Traffic Signal Warrant Analysis Peak-Hour Volumes Walden Preserve

	Vehicles	Per Hour
	Peak Hour 7:00-8:00 a.m.	Peak Hour 4:45-5:45 p.m.
MINOR STREET TRAFFIC		
Eastbound Site-Generated Traffic ⁽¹⁾		
Left	0	0
Through	9	30
Right	###	##
Background Traffic ⁽²⁾		
Left	11	45
Through	32	68
Right	###	###
Westbound		
Site-Generated Traffic	7	E
Left Through	7 17	5 12
_	",	
Background Traffic Left	52	18
Through	87	52
Eastbound Minor Street	52	143
Westbound Minor Street	163	87
MAJOR STREET TRAFFIC Northbound Site-Generated Traffic Left	13	9
Through	3	2
Right	4	12
Background Traffic		
Left	111	143
Through	130	245
Right Southbound	17	40
Site-Generated Traffic		
Left	2	5
Through	0	1
Right	0	0
Background Traffic		
Left Through	9 237	8 167
Right	47	34
Major Street Totals		
major oneet rotais	7:00-8:00 a.m.	5:00-6:00 p.m.
	573	666

Notes

⁽¹⁾ Includes Filing 1, 2 and Current PUD Amendment Area

⁽²⁾ Based on 2012 traffic volumes with shift in traffic pattern due to the restriction of Walden Way/SH 83 and a new connection to Walker

Table 3 Traffic Signal Warrant Analysis Four-Hour Volumes Walden Preserve

	vvaluen r	Vehicles	Per Hour	
	6:30-7:30 a.m.	7:30-8:30 a.m.	4:00-5:00 p.m.	5:00-6:00 p.m.
MINOR STREET TRAFFIC				
Eastbound				
Site-Generated Traffic ⁽¹⁾		_		
Left	0	0	0	0
Through	8	5	27	30
Right	###			##
Background Traffic ⁽²⁾	44	12	00	47
Left	11	38	32 49	47
Through Right	16 97	104	99	50 98
_	91	104	99	90
Westbound				
Site-Generated Traffic Left	0	0	0	0
Through	15	13	12	11
=			·-	
Background Traffic Left	47	43	35	15
Through	76	59	43	47
Eastbound Minor Street	35	55	108	127
Westbound Minor Street	139	115	90	73
MAJOR STREET TRAFFIC				
Site-Generated Traffic				
Northbound				
Left	12	13	13	12
Through	3	3	3	3
Right	0	4	0	0
Southbound				
Left	2	1	4	6
Through	0	0	1	1
Right	0	0	0	0
Background Traffic	-	-	-	
_				
All NB and SB Approach Traffic	501	518	584	637
Major Street Totals				
	6:30-7:30 a.m.	7:30-8:30 a.m.	4:00-5:00 p.m.	5:00-6:00 p.m.
	517	540	605	658

Notes

- (1) Includes Filing 1, 2 and Current PUD Amendment Area
- (2) Based on 2012 traffic volumes with shift in traffic pattern due to the restriction of Walden Way/SH 83 and a new connection to Walker

Table 4 Traffic Signal Warrant Analysis Future Four Hour Volumes Walden Preserve

		Walden Preserve				
	6:30-7:30 a.m.	7:30-8:30 a.m.	4:00-5:00 p.m.	5:00-6:00 p.m.		
MINOR STREET TRAFFIC						
Eastbound						
Site-Generated Traffic ⁽¹⁾						
Left	0	0	0	0		
Through	8	5	27	30		
Right	###	0	0	##		
Background Traffic ⁽²⁾						
Left	13	14	38	56		
Through	19	46	59	60		
Right	116	125	119	118		
Westbound						
Site-Generated Traffic					l	
Left	0	0	0	0	l	
Through	15	13	12	11	l	
Background Traffic					l	
Left	56	52	42	18	l	
Through	91	71	51	57		
	!					
Eastbound Minor Street	40	66	124	146		
Westbound Minor Street	163	135	105	85		
MAJOR STREET TRAFFIC						
Site-Generated Traffic						
Northbound						
Left	12	13	13	12		
Through	3	3	3	3		
Right	0	4	0	0		
		·	-			
Southbound						
Left	2	1	4	6		
Through	0	0	1	1	l	
•	0	0	0	0	l	
Right	U		U			
Background Traffic						
All NB and SB approach traffic	601	622	701	764		
Major Street Totals						
	7:00-8:00 a.m.	12 noon-1:00 p.m.	4:00-5:00 p.m.	5:00-6:00 p.m.		
	617	644	722	785		
				. 50		
Eastbound			24 400/	20.54%	Avores	45
Eastbound Westbound	0.000/	0.449/	21.48%	20.54%	Average	15.2
	9.38%	9.44%		I	í	
westbound					l	
Major Street + Higher Minor Street	780	779	846	931	Weighted Average	15.5

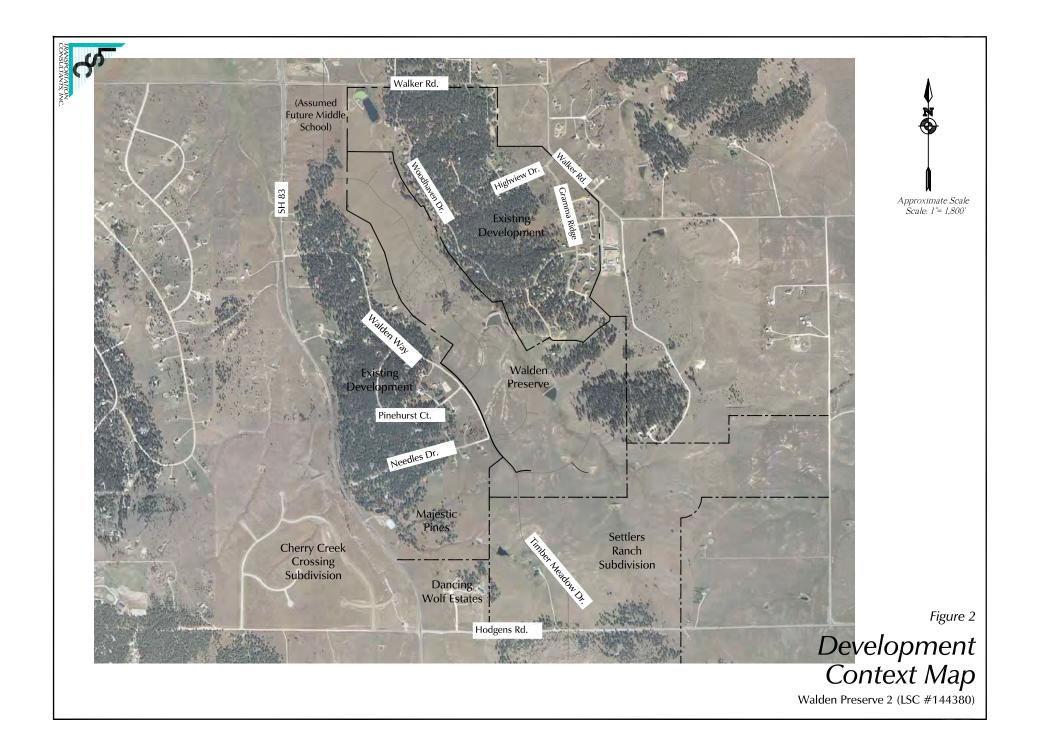
Notes

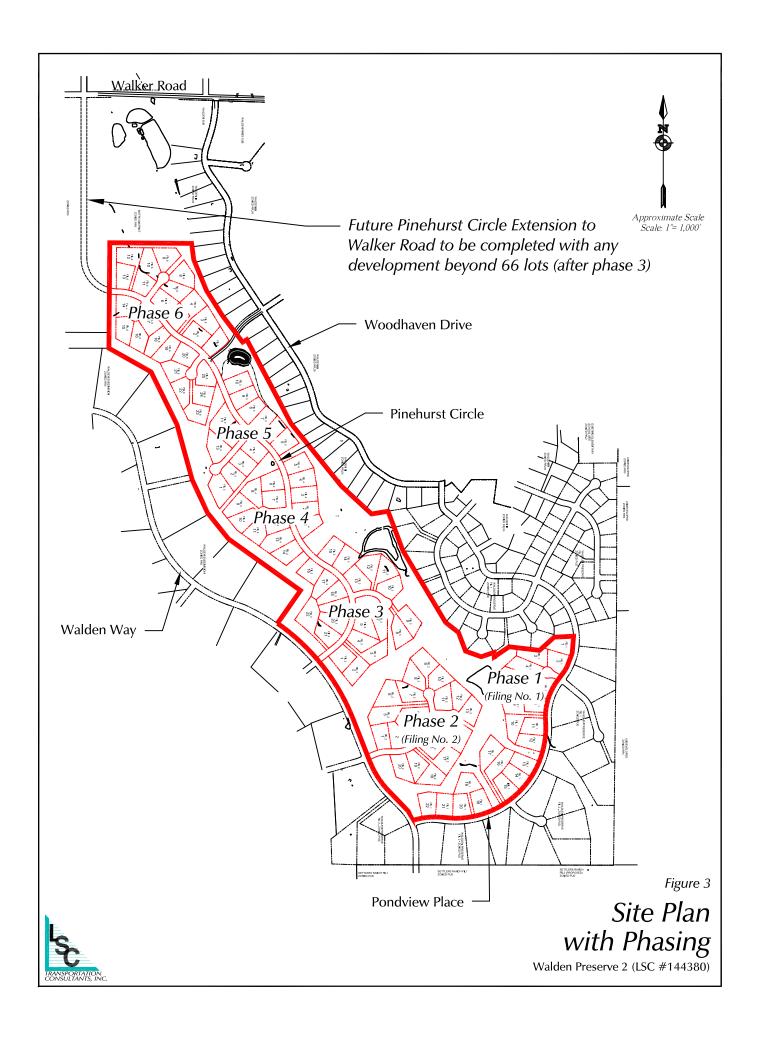
- (1) Includes Filing 1, 2 and Current PUD Amendment Area
- (2) Growth factor of 1.2 applied to current background traffic volumes

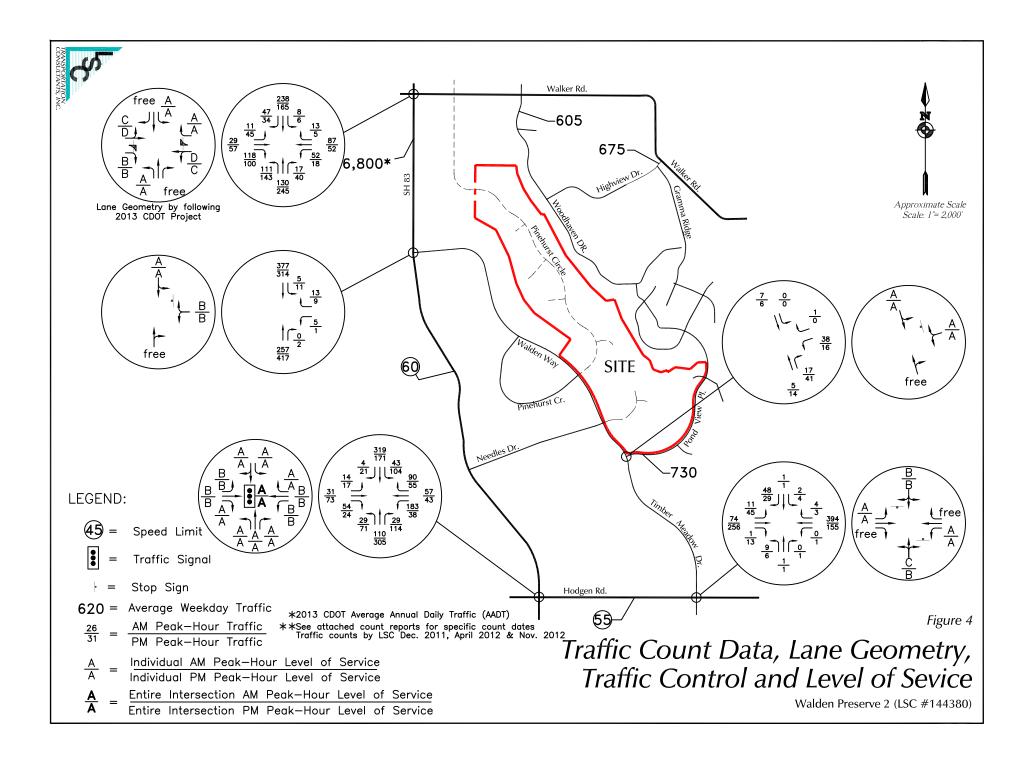


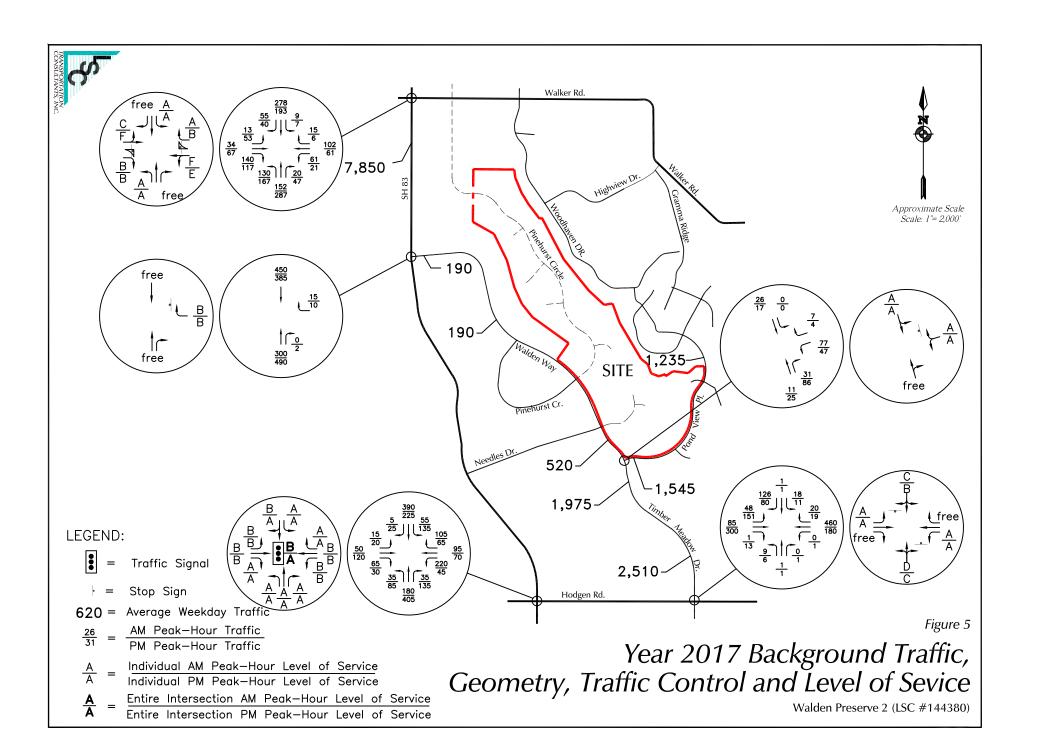


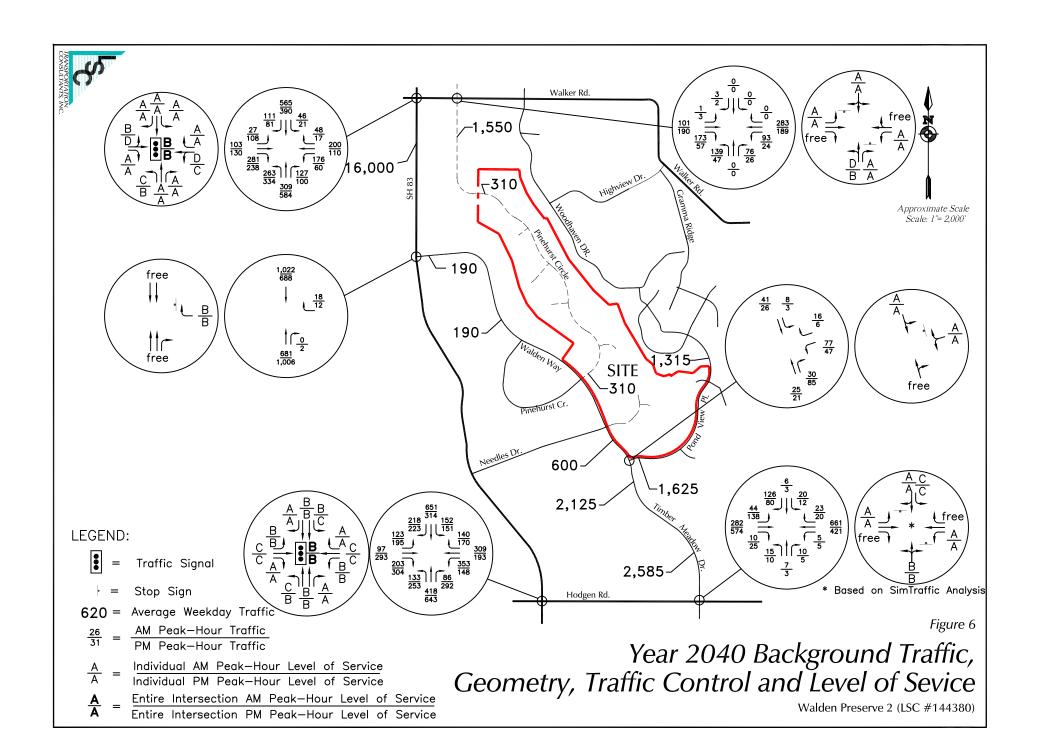
Figure 1 Vicinity
Map
Walden Preserve 2 (LSC #144380)

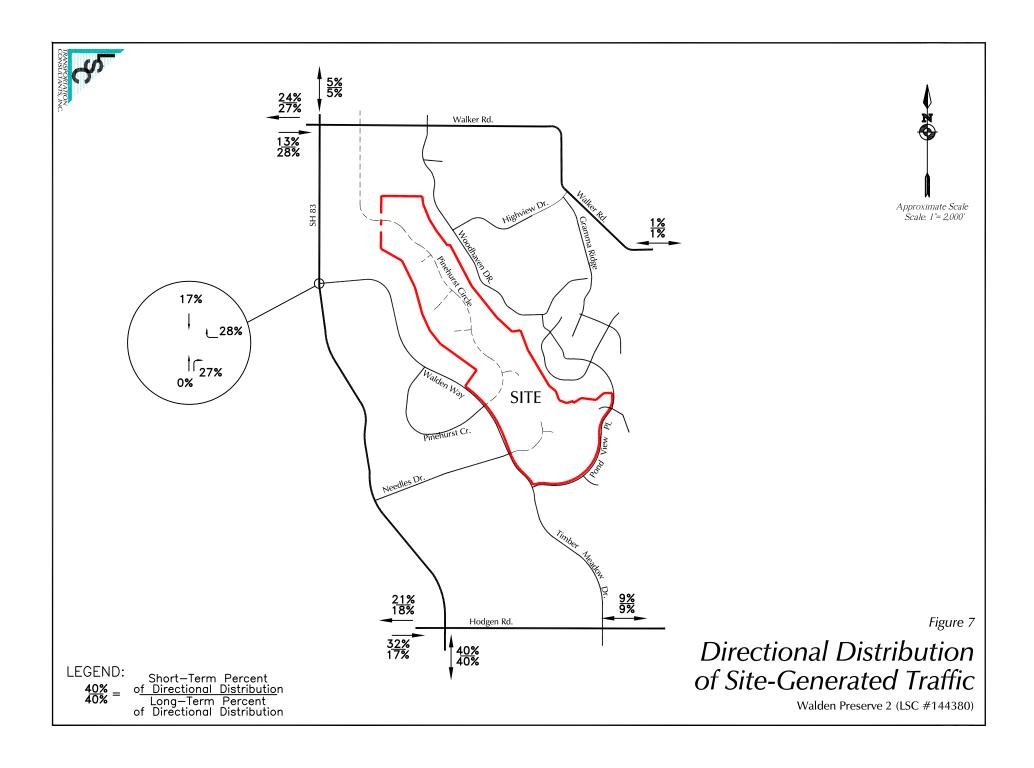




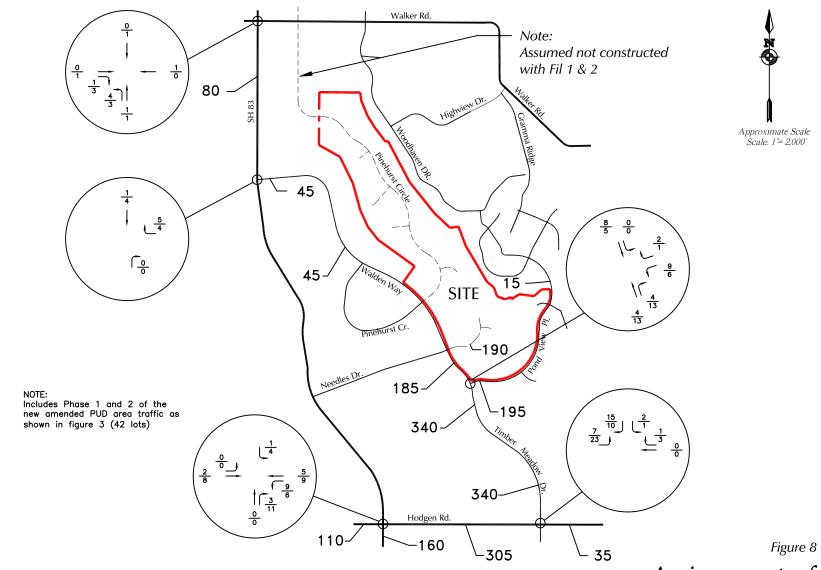












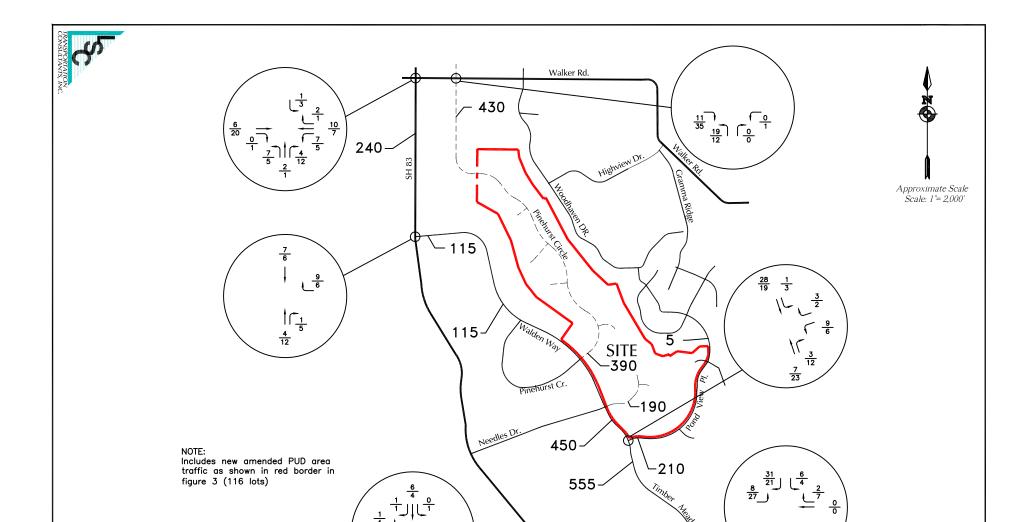
LEGEND:

620 = Average Weekday Traffic

 $\frac{26}{31} = \frac{AM \text{ Peak-Hour Traffic}}{PM \text{ Peak-Hour Traffic}}$

Assignment of Phases 1 & 2 (Fil. 1 & 2) Site-Generated Traffic

Walden Preserve 2 (LSC #144380)



190

LEGEND:

620 = Average Weekday Traffic

 $\frac{26}{31} = \frac{AM \text{ Peak-Hour Traffic}}{PM \text{ Peak-Hour Traffic}}$

Assignment of Buildout Site-Generated Traffic

^{_}100

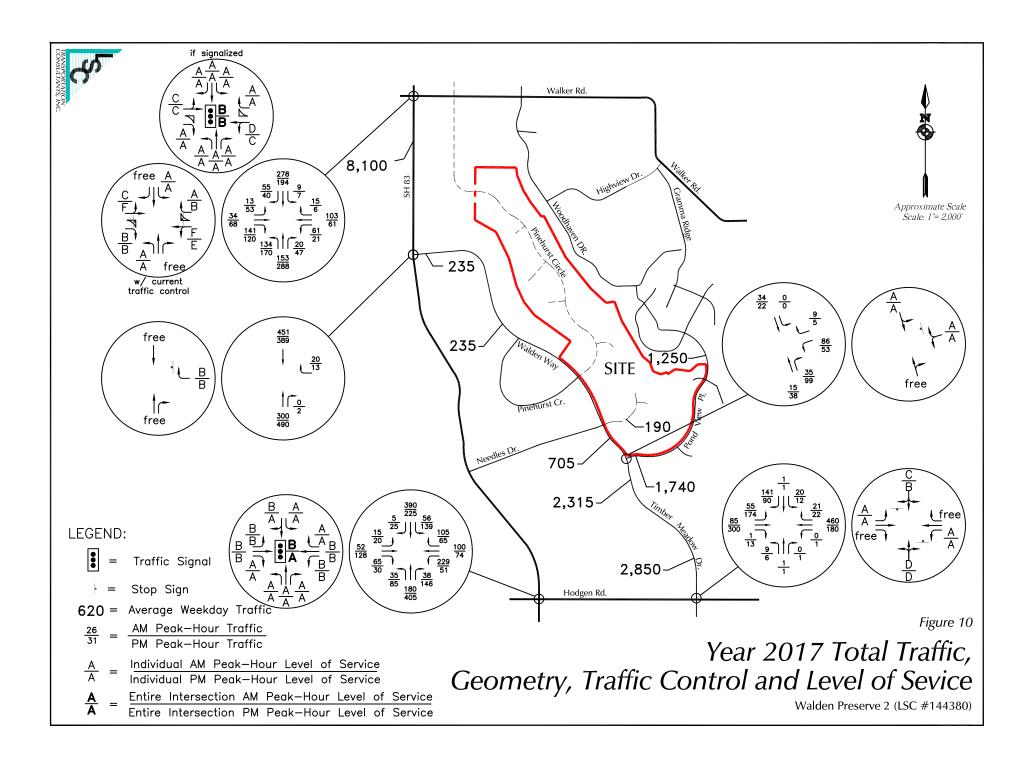
555-

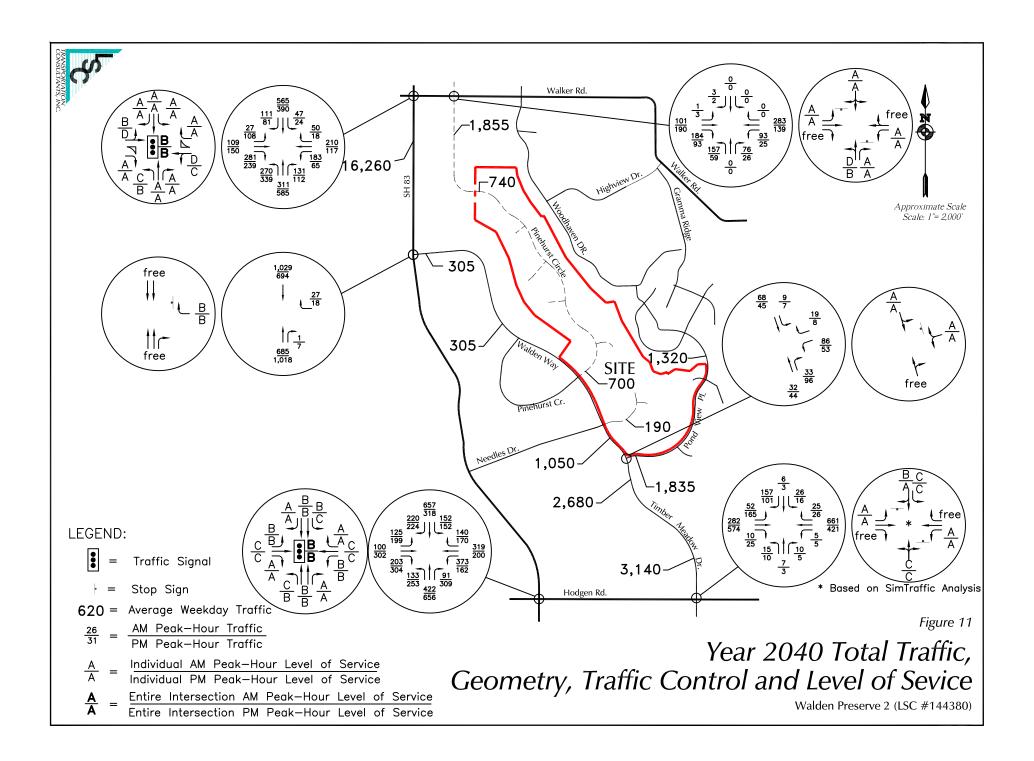
-455

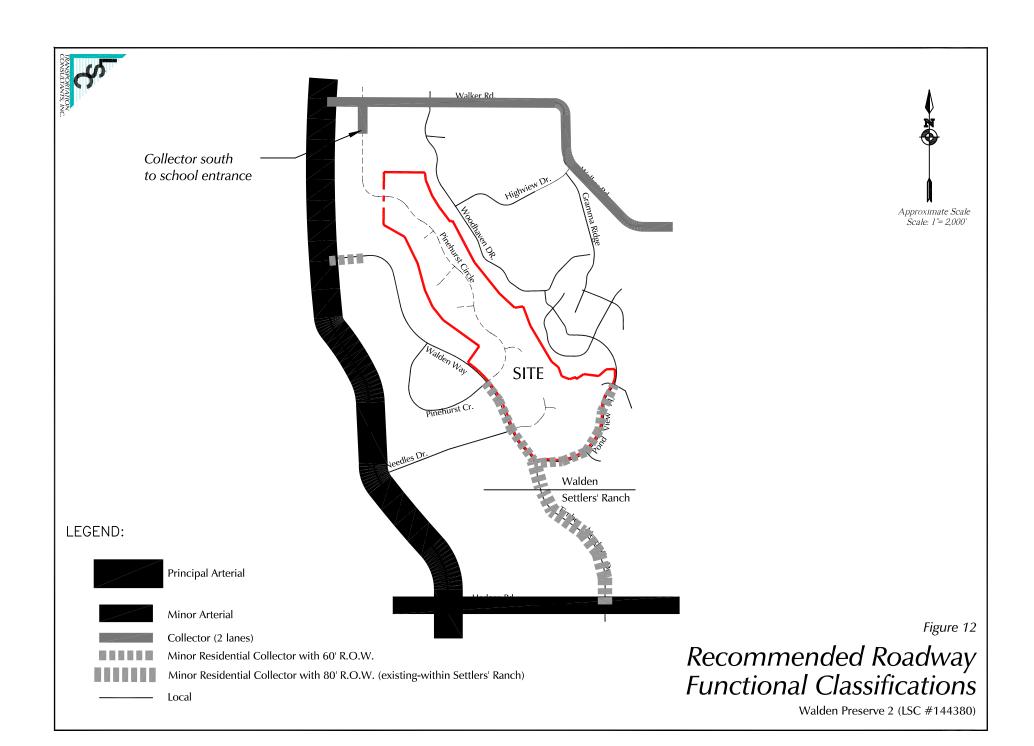
Hodgen Rd.

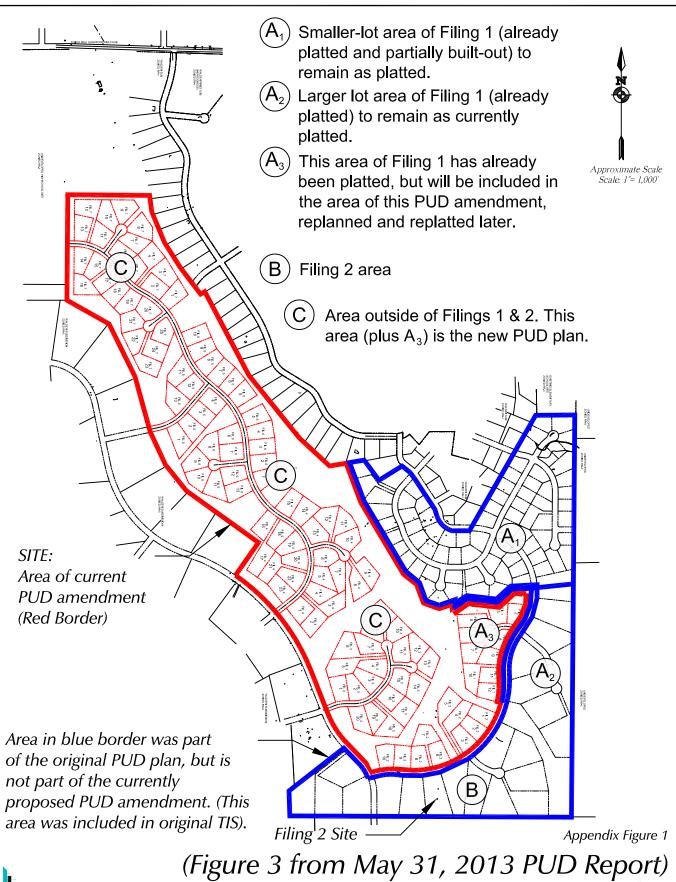
Walden Preserve 2 (LSC #144380)

Figure 9





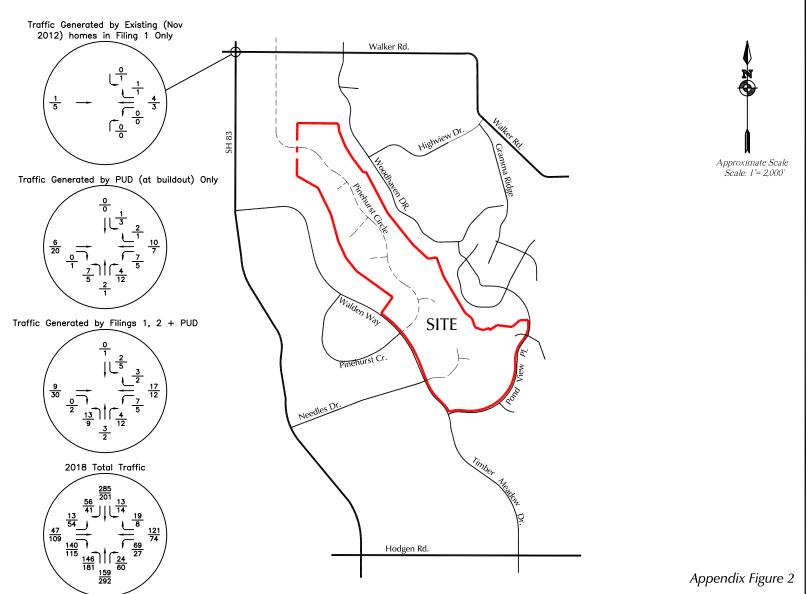




Comparison to Previous Plan

Walden Preserve 2 (LSC #144380)





LEGEND:

 $\frac{26}{31} = \frac{AM \text{ Peak-Hour Traffic}}{PM \text{ Peak-Hour Traffic}}$

SH 83/ Walker Traffic Signal Warrant Analysis Volumes

* Based on SimTraffic Analysis

Walden Preserve 2 (LSC #144380)

516 N. Tejon St.

LSC Transportation Consultants, Inc.

Colorado Springs F@ Name: Timber Meadow Dr - Hodgen Rd AM
(719) 633-286 Bite Code: 00000000
Start Date: 04/19/2012

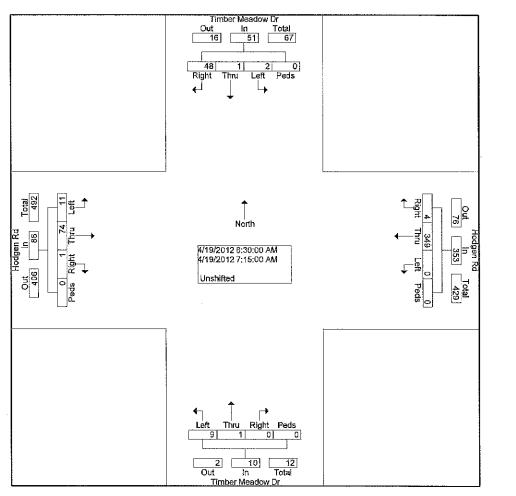
	Unshifted	

		~					Groups	rinteu									
	Tir	nber Me	adow E	or		Hodge	en Rd		Ti	mber M	eadow D)r		Hodge	en Rd		
		From	North			From	East			From	South			From	West		
Start Time	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
06:30 AM	9	0	1	0	0	83	0	0	0	0	0	0	0	20	1	0	114
06:45 AM	8	0	0	0	0	81	0	0	0	1	4	0	0	18	2	0	114
Total	17	0	1	0	0	164	Ó	0	0	1	4	Õ	0	38	3	0	228
07:00 AM	14	1	0	0	0	79	0	0	0	0	0	0	1	21	1	0	117
07:15 AM	17	0	1	0	4	106	0	0	0	0	5	0	0	15	7	0	155
07:30 AM	9	0	2	0	1	70	1	0	1	0	1	0	2	15	3	0	105
07:45 AM	10	0	0	0	2	60	0	0	0	1	0	0	2	25	9	0	109
Total	50	1	3	0	7	315	1	0	1	1	6	0	5	76	20	0	486
08:00 AM	9	1	1	0	0	59	0	0	0	0	2	0	3	19	6	o	100
08:15 AM	6	1	0	0	0	59	0	0	1	0	1	0	1	20	4	0	93
Grand Total	82	3	5	0	7	597	1	0	2	2	13	0	9	153	33	0	907
Apprch %	91.1	3.3	5.6	0.0	1.2	98.7	0.2	0.0	11.8	11.8	76.5	0.0	4.6	78.5	16.9	0.0	
Total %	9.0	0.3	0.6	0.0	0.8	65.8	0.1	0.0	0.2	0.2	1.4	0.0	1.0	16.9	3.6	0.0	

516 N. Tejon St.

Colorado SpringsF@@Name: Timber Meadow Dr - Hodgen Rd AM (719) 633-2868ite Code: 00000000 Start Date: 04/19/2012

		Timbe Fr	r Mea om No		Γ	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,		odgen om Ea					r Meadom So		r			odgen om W			
Start	Rig	Thr	Lef	Рe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	Арр.	Rig	Thr	Lef	Pe	Арр.	Int.
Time	ht	u	t	ds	Total	ht	u	t	ds	Total	ht	u	t	ds	Total	ht	u	t	ds	Total	Total
Peak Hour	From	06:30	AM to	o 08:1	5 AM -	Peak	1 of 1	l													
Intersecti on	06:30	MAC																			
Volume	48	1	2	0	51	4	34 9	0	0	353	0	1	9	0	10	1	74	11	0	86	500
Percent	9 4 . 1	2.0	3.9	0.0		1.1	98. 9	0.0	0.0		0.0	10. D	90. D	0.0		1.2	86. 0	12. 8	0.0		
07:15 Volume Peak	17	0	1	0	18	4	10 6	0	0	110	0	D	5	0	5	0	15	7	0	22	155 0.806
Factor High Int.	07:1	5 AM				07:15	5 AM				06:4	5 AM				07:0	MA 0				0.000
Volume	17	0	1	0	18	4	10 6	0	0	110	0	1	4	0	5	1	21	1	0	23	
Peak Factor					0.70 8					0.80					0.50 0					0.93 5	



516 N. Tejon St.

LSC Transportation Consultants, Inc.

Colorado SpringsF@CName: Timber Meadow Dr - Hodgen Rd PM (719) 633-2868ite Code: 00000000 Start Date: 04/19/2012

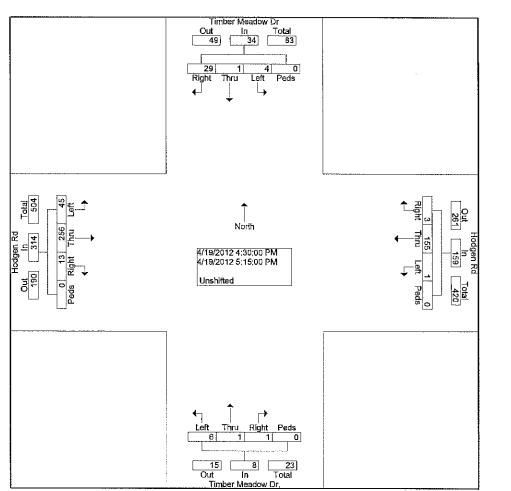
	Unshifted	

		mber Me From		DΓ		Hodge From			Ti		eadow D South	Γ,			en Rd West		
Start Time	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
04:15 PM	4	1	1	0	2	38	0	0	0	3	2	0	2	66	10	0	129
04:30 PM	7	0	2	0	2	37	1	0	0	0	1	0	2	68	11	0	131
04:45 PM	9	0	1	0	0	36	0	0	0	0	2	0	1	60	10	0	119
Total	20	1	4	0	4	111	1	0	0	3	5	0	5	194	31	0	379
05:00 PM	10	0	0	0	1	33	0	0	0	0	3	0	4	57	10	0	118
05:15 PM	3	1	1	0	0	49	0	0	1	1	0	0	6	71	14	0	147
05:30 PM	5	0	0	0	1	45	0	0	0	0	0	0	0	48	9	0	108
05:45 PM	6	0	1	0	1	30	0	0	1	0	1	0	4	50	12	0	106
Total	24	1	2	0	3	157	0	0	2	1	4	0	14	226	45	0	479
06:00 PM	5	0	2	0	1	23	0	0	1	3	2	0	3	50	14	0	104
Grand Total	49	_ 2	. 8	0	- 8	291	_ 1	0	3	7	11	0	22	470	90	0	962
Apprch %	83.1	3.4	13.6	0.0	2.7	97.0	0.3	0.0	14.3	33.3	52.4	0.0	3.8	80.8	15,5	0.0	
Total %	5.1	0.2	8.0	0.0	8.0	30.2	0.1	0.0	0.3	0.7	1.1	0.0	2.3	48.9	9.4	0.0	

516 N. Tejon St.

Colorado Springs F@ Chame: Timber Meadow Dr - Hodgen Rd PM (719) 633-286 Site Code: 00000000 Start Date: 04/19/2012

			r Mea	dow D	r .			odgen rom Ea					r Mead		r,			odgen om W]
Start	Rig	Thr		Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Int.
Time	ht	ш	t	ds	Total	ht	ш	t	ds	Total	ht	Ц	t	ds	Total	ht	u	t	ds	Total	Total
Peak Hour	From	04:15	PM t	o 06:0	0 PM -	Peak	1 of 1	1									•				
Intersecti on	04:30) PM																			
Volume	29	1	4	0	34	3	15 5	1	0	159	1	1	6	0	8	13	25 6	45	0	314	515
Percent	85. 3	2.9	11. 8	0.0		1.9	97. 5	0.6	0.0		12. 5	12. 5	75. 0	0.0		4.1	81. 5	14. 3	0.0		
05:15 Volume Peak	3	1	1	0	5	0	49	0	0	49	1	1	0	0	2	6	71	14	0	91	147 0.876
Factor High Int. Volume Peak Factor	04:4! 9	5 PM 0	1	0	10 0.85 0	05:18 0	5 PM 49	0	0	49 0.81	05:0 0	0 PM 0	3	0	3 0.66	05:1: 6	5 PM 71	14	0	91 0.86 3	



LSC Transportation Consultants, Inc.

516 N. Tejon St. Colorado Springs, CO (719) 633-2868

File Name: Hwy 83-Hodgen AM

Site Code : 00000000 Start Date : 12/14/2011

Page No : 1

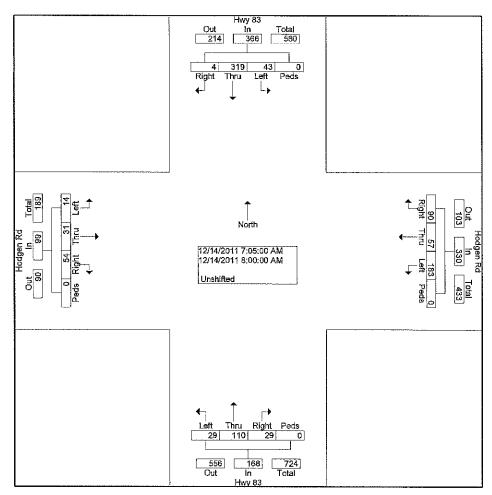
Hwy 83 Hodgen Rd From East From South Hwy 83 Hodgen Rd From West
Start Time Right Thru Left Peds Thru Left
Factor 1.0 1
06:30 AM
06:35 AM 7 22 2 0 3 4 9 0 1 5 0 0 1 4 0 0 58 06:40 AM 0 21 2 0 6 1 15 0 1 7 0 0 1 3 0 0 57 06:45 AM 0 22 5 0 6 5 17 0 2 3 0 0 4 0 0 0 64 06:50 AM 0 20 6 0 7 6 14 0 1 2 3 0 3 2 0 0 64 06:55 AM 1 25 2 0 13 4 16 0 0 5 2 0 4 0 0 0 72 Total 8 129 19 0 44 22 81 0 7 27 5 0 14 12 0 0 368 07:00 AM 0 19 0 0 2 8 12 0 0 8 1 0 7 27 5 0 14 12 0 78 07:10 AM 0 30 3 0 7 9 11 0 3 8 1 0 3 1 0 78 07:15 AM 1 21 2 0 10 10 10 18 0 2 12 3 0 4 4 2 0 89 07:20 AM 0 30 5 0 6 8 18 0 1 6 1 0 7 0 3 0 85 07:25 AM 0 32 2 0 5 6 21 0 2 12 2 0 10 5 0 97 07:30 AM 0 29 3 0 9 3 12 0 2 9 2 0 4 0 1 0 7 74
06:40 AM
06:45 AM
06:50 AM 0 20 6 0 7 6 14 0 1 2 3 0 3 2 0 0 64 06:55 AM 1 25 2 0 13 4 16 0 0 5 2 0 4 0 0 0 72 Total 8 129 19 0 44 22 81 0 7 27 5 0 14 12 0 0 368 07:00 AM 0 19 0 0 2 8 12 0 0 8 1 0 2 0 0 52 07:05 AM 0 25 4 0 9 3 17 0 2 6 4 0 3 4 1 0 78 07:10 AM 0 30 3 0 7 9 11 0 3
06:55 AM 1 25 2 0 13 4 16 0 0 5 2 0 4 0 0 0 72 Total 8 129 19 0 44 22 81 0 7 27 5 0 14 12 0 0 368 07:00 AM 0 19 0 0 2 8 12 0 0 8 1 0 2 0 0 0 52 07:05 AM 0 25 4 0 9 3 17 0 2 6 4 0 3 4 1 0 78 07:10 AM 0 30 3 0 7 9 11 0 3 8 1 0 3 1 0 0 76 07:15 AM 1 21 2 0 10 18 0
Total 8 129 19 0 44 22 81 0 7 27 5 0 14 12 0 0 368 07:00 AM 0 19 0 0 2 8 12 0 0 8 1 0 2 0 0 0 52 07:05 AM 0 25 4 0 9 3 17 0 2 6 4 0 3 4 1 0 78 07:10 AM 0 30 3 0 7 9 11 0 3 8 1 0 3 1 0 0 76 07:15 AM 1 21 2 0 10 18 0 2 12 3 0 4 4 2 0 89 07:20 AM 0 30 5 0 6 8 18 0
07:00 AM
07:05 AM 0 25 4 0 9 3 17 0 2 6 4 0 3 4 1 0 78 07:10 AM 0 30 3 0 7 9 11 0 3 8 1 0 3 1 0 0 76 07:15 AM 1 21 2 0 10 18 0 2 12 3 0 4 4 2 0 89 07:20 AM 0 30 5 0 6 8 18 0 1 6 1 0 7 0 3 0 85 07:25 AM 0 32 2 0 5 6 21 0 2 12 2 0 10 5 0 0 97 07:30 AM 0 29 3 0 9 3 12 0 2 9 2 0 4 0 1 0 74
07:05 AM 0 25 4 0 9 3 17 0 2 6 4 0 3 4 1 0 78 07:10 AM 0 30 3 0 7 9 11 0 3 8 1 0 3 1 0 0 76 07:15 AM 1 21 2 0 10 18 0 2 12 3 0 4 4 2 0 89 07:20 AM 0 30 5 0 6 8 18 0 1 6 1 0 7 0 3 0 85 07:25 AM 0 32 2 0 5 6 21 0 2 12 2 0 10 5 0 0 97 07:30 AM 0 29 3 0 9 3 12 0 2 9 2 0 4 0 1 0 74
07:10 AM 0 30 3 0 7 9 11 0 3 8 1 0 3 1 0 0 76 07:15 AM 1 21 2 0 10 10 18 0 2 12 3 0 4 4 2 0 89 07:20 AM 0 30 5 0 6 8 18 0 1 6 1 0 7 0 3 0 85 07:25 AM 0 32 2 0 5 6 21 0 2 12 2 0 10 5 0 0 97 07:30 AM 0 29 3 0 9 3 12 0 2 9 2 0 4 0 1 0 74
07:15 AM 1 21 2 0 10 10 18 0 2 12 3 0 4 4 2 0 89 07:20 AM 0 30 5 0 6 8 18 0 1 6 1 0 7 0 3 0 85 07:25 AM 0 32 2 0 5 6 21 0 2 12 2 0 10 5 0 0 97 07:30 AM 0 29 3 0 9 3 12 0 2 9 2 0 4 0 1 0 74
07:20 AM 0 30 5 0 6 8 18 0 1 6 1 0 7 0 3 0 85 07:25 AM 0 32 2 0 5 6 21 0 2 12 2 0 10 5 0 97 07:30 AM 0 29 3 0 9 3 12 0 2 9 2 0 4 0 1 0 74
07:25 AM 0 32 2 0 5 6 21 0 2 12 2 0 10 5 0 0 97 07:30 AM 0 29 3 0 9 3 12 0 2 9 2 0 4 0 1 0 74
07:30 AM 0 29 3 0 9 3 12 0 2 9 2 0 4 0 1 0 74
07:35 AM 0 14 6 0 7 6 23 0 3 7 3 0 6 2 2 0 79
07:40 AM 0 25 3 0 8 3 8 0 2 6 5 0 3 2 3 0 68
07:45 AM 0 29 5 0 8 3 19 0 1 16 1 0 2 2 2 0 88
07:50 AM
07:55 AM 1 28 4 0 7 2 15 0 5 10 4 0 4 5 0 0 85
Total 3 307 42 0 87 64 185 0 25 112 28 0 51 28 14 0 946
08:00 AM
08:05 AM 1 15 1 0 11 3 5 0 2 7 1 0 3 2 0 0 51
08:10 AM 1 25 2 0 8 5 5 0 3 8 2 0 5 3 1 0 68
08:15 AM 1 23 2 0 4 2 6 0 7 14 1 0 6 5 3 0 74
08:20 AM 0 9 3 0 4 5 15 0 1 11 2 0 5 3 5 0 63
08:25 AM 2 22 2 0 4 1 7 0 1 20 0 0 5 3 2 0 69
Grand Total 17 561 72 0 167 103 314 0 50 205 41 0 94 59 25 0 1708
Apprch % 2.6 86.3 11.1 0.0 28.6 17.6 53.8 0.0 16.9 69.3 13.9 0.0 52.8 33.1 14.0 0.0
Total % 1.0 32.8 4.2 0.0 9.8 6.0 18.4 0.0 2.9 12.0 2.4 0.0 5.5 3.5 1.5 0.0

LSC Transportation Consultants, Inc. 516 N. Tejon St. Colorado Springs, CO (719) 633-2868

File Name: Hwy 83-Hodgen AM

Site Code : 00000000 Start Date : 12/14/2011

			Hwy 83 rom No					odgen rom Er					Hwy 8					lodgen rom W			
Start Time	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped s	App. Total	Rig ht	Thr u	Left	Ped	App. Total	Int. Total
Peak Hour F			AM to	08:25	AM - Pe	ak 1 o	f 1									 					[
on Volume	07:05 4		43	0	366	90	57	183	Ö	330	29	110	20	0	100	ΕA	21	4.4		00.1	002
Percent	1.1	87. 2	11.	0.0	300	27. 3	17. 3	55. 5	0.0	330	17. 3	65. 5	29 17. 3	0.0	168	54 54. 5	31 31. 3	14 14.	0.0	99	963
07:25 Volume	0	32	2	0	34	5	6	21	0	32	2	12	2	0	16	10	5 5	0	0	15	97
Peak Factor															'					!	0.827
High Int.	07:20					07:15					07:55					07:25	5 AM				
Volume Peak	0	30	5	0	35 0.87	10	10	18	0	38 0,72	5	10	4	0	19 0.73	10	5	0	0	15 0.55	
Factor					1					4					7					0.00	



516 N. Tejon St. Colorado Springs, CO (719) 633-2868

File Name: Hwy 83-Hodgen pM

Site Code : 00000000 Start Date : 12/13/2011

Page No : 1

LSC Transportation Consultants, Inc.

		Hwy From				Hodge From	n Rd	7,11,00	01,011,12	Hwy From				Hodge From			
Start Time	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Right	Thru	Left	Peds	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
04:15 PM	3	47	22	0	17	13	12	0	18	73	21	0	6	27	3	0	262
04:30 PM	5	48	31	0	21	15	12	0	30	73	19	0	6	19	5	1	285
04:45 PM	5	43	27	0	11	10	5	0	29	61	15	0	4	12	0	0	222
Total	13	138	08	0	49	38	29	0	77	207	55	0	16	58	8	1	769
05:00 PM	3	39	27	0	5	9	14	0	24	82	14	0	6	26	4	0	253
05:15 PM	8	41	19	0	18	9	7	2	31	89	23	0	8	16	8	0	279
05:30 PM	7	29	15	0	14	9	10	1	42	82	12	0	12	20	2	0	255
05:45 PM	2	44	14	0	4	14	9	0	26	70	15	0	5	16	6	0	225
Total	20	153	75	0	41	41	40	3	123	323	64	0	31	78	20	0	1012
06:00 PM	2	23	23	0	7	16	8	0	21	84	15	0	3	15	2	0	219
Grand Total	35	314	178	0	97	95	77	3	221	614	134	0	50	151	30	1	2000
Apprch %	6.6	59.6	33.8	0.0	35.7	34.9	28.3	1.1	22.8	63.4	13.8	0.0	21.6	65.1	12.9	0.4	
Total %	1.8	15.7	8.9	0.0	4.9	4.8	3.9	0.2	11. 1	30.7	6.7	0,0	2.5	7.6	1.5	0.1	

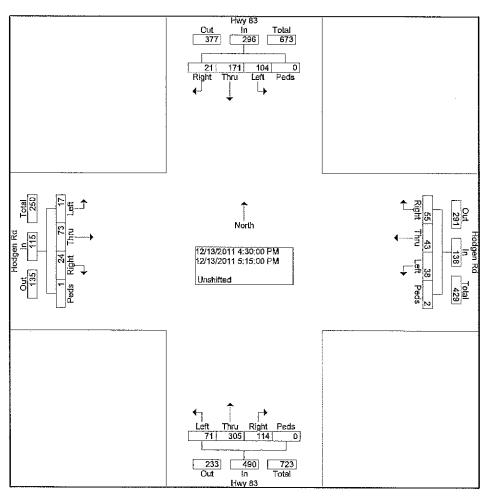
LSC Transportation Consultants, Inc. 516 N. Tejon St. Colorado Springs, CO

Colorado Springs, CO (719) 633-2868

File Name: Hwy 83-Hodgen pM

Site Code : 00000000 Start Date : 12/13/2011

			Hwy 8					odgen rom Ea					Hwy 8					odgen rom W			
Start	Rig	Thr		Ped	App.	Rig	Thr	Left	Ped	App.	Rig	Thr	Left	Ped	Арр.	Rig	Thr	Left	Ped	Арр.	Int.
Time Peak Hour F	ht From 0	<u>ը</u> 4:15 F	: PM to !	s 06:00	Total PM - Pe	ht eak 1 d	<u>u</u> of 1		S	Total	ht i	u	,, <u>.</u>	S	Total	ht]	u		S	lotal	Total
Intersecti on	04:30	PM																			
Volume	21	171	104	0	296	55	43	38	2	138	114	305	71	0	490	24	73	17	1	115	1039
Percent	7.1	57. 8	35. 1	0.0		39. 9	31. 2	27. 5	1.4		23. 3	62. 2	14. 5	0.0		20. 9	63. 5	14. 8	0.9		
04:30 Volume Peak	5	48	31	0	84	21	15	12	0	48	30	73	19	0	122	6	19	5	1	31	285 0.911
Factor High Int.	04:30	PM				04:30	PM				05:15	PM				05:00	PM				
Volume Peak Factor	5	48	31	0	84 0.88 1	21	15	12	0	48 0.71 9	31	89	23	0	143 0.85 7	6	26	4	0	36 0.79 9	



LSC Transportation Consultants, Inc.

516 N. Tejon St. Colorado Springs, CO (719) 633-2868

File Name: Hwy 83 - Walden Way AM

Site Code : 00000000 Start Date : 04/18/2012

Page No : 1

								cuiten.	Onanii								
		Hwy				Walder					/ 83				n Way		
		From	North			From	East			From	South			From	West		
Start Time	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
06:30 AM	0	107	0	0	2	0	0	0	0	37	0	0	1	0	0	0	147
06:45 AM	0	91	0	0	5	0	4	0	0	36	0	0	0	0	0	0	136
Total	0	198	0	0	7	0	4	0	0	73	0	0	1	0	0.	0	283
07:00 A M	0	96	2	0	6	0	0	0	0	57	0	0	0	0	0	0	161
07:15 A M	0	98	0	0	3	0	1	0	0	64	0	0	0	0	0	0	166
07:30 AM	0	94	2	0	. 1	0	2	0	0	82	0	0	0	0	0	0	181
07:45 AM	0	89	1	0	3	0	2	0	0	54	0	0	0	0	0	0	149
Total	0	377	5	0	13	0	5	0	0	257	0	0	0	0	0	0	657
	_		_			_	_		_		_	_				_ 1	
MA 00:80	0	71	0	0	2	0	0	0	2	60	0	0	0	0	0	0	135
08:15 AM	0	66	1	0	2	0	0	0	0	53	0	0	0	0	0	0	122
Grand Total	0	712	6	0	24	0	9	0	2	443	0	0	1	0	0	0	1197
Apprch %	0.0	99.2	8.0	0.0	72.7	0.0	27.3	0.0	0.4	99.6	0.0	0.0	100.	0.0	0.0	0.0	
Total %	0.0	59.5	0.5	0,0	2.0	0.0	0.8	0.0	0.2	37.0	0.0	0.0	0.1	0.0	0.0	0.0	

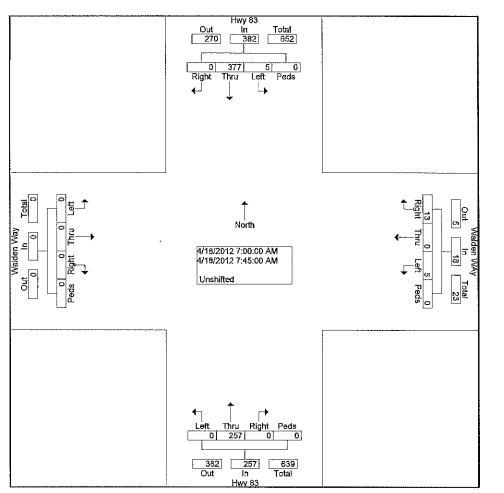
LSC Transportation Consultants, Inc. 516 N. Tejon St.

Colorado Springs, CO (719) 633-2868

File Name : Hwy 83 - Walden Way AM

Site Code : 00000000 Start Date : 04/18/2012

			Hwy 8					lden V					Hwy 8					alden V			
			om No	http				om E	ast				om So	uth			Fr	om W	est		
Start	Rig	Thr	Lef	Pe	App.	Rig	The	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Int.
Time	ht	<u>u</u>	t	ds	Total	ht	u	t	ds	Total	ht	ш	t	ds	Total	ht !	U	t	ds	Total	Total
Peak Hour	From	06:30	AM to	o 08:1	5 AM -	Peak	1 of 1	l													
Intersecti	07:00	S A M				-															[
on	07,00) WIN																			
Volume	0	37 7	5	0	382	13	0	5	0	18	0	25 7	0	0	257	0	0	0	0	0	657
Percent	0.0	98. 7	1.3	0.0		72. 2	0.0	27. 8	0.0		0.0	10 0.0	0.0	0.0		0.0	0.0	0.0	0.0		
07:30 Volume	0	94	2	0	96	1	0	2	0	3	0	82	0	0	82	0	0	0	0	0	181
Peak Factor																					0.907
High Int.	07:00	MAC				07:00	MAC				07:30	MA C				6:15:	1A 00:	И			
Volume	0	96	2	0	98	6	0	0	0	6	0	82	0	0	82	ĺ					ĺ
Peak					0.97					0.75			•		0.78						
Factor					4					0					4						



LSC Transportation Consultants, Inc.

516 N. Tejon St. Colorado Springs, CO (719) 633-2868

File Name: Hwy 83 - Walden Way PM

Site Code : 00000000 Start Date : 04/18/2012

Page No : 1

								rimeu-	· Ousin					~~~~			
		Hwy				Walder				Hwy				Walder			
		From	North			From	East			From	South			From	West		
Start Time	Righ	Thru	Left	Ped	Righ	Thru	Left	Ped	Righ	Thru	Left	Ped	Righ	Thru	Left	Ped	Int.
	t		LCIL	s	t	TIME	LGI	Ş	t	THE	LUIL	5	t	1	Ceit	s	Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
04:15 PM	0	48	2	0	1	0	1	0	3	83	0	0	0	0	0	0	138
04:30 PM	0	80	2	0	1	0	2	0	0	96	0	0	0	0	0	0	181
04:45 PM	0	66	5	0	3	0	0	0	0	113	0	0	0	0	0	0	187
Total	0	194	9	0	5	0	3	0	3	292	0	0	0	0	0	0	506
05:00 PM	0	86	1	0	1	0	1	0	0	96	0	0	0	0	0	0	185
05:15 PM	0	81	2	0	3	0	0	0	2	109	0	0	0	0	0	0	197
05:30 PM	0	81	3	0	2	0	0	0	0	99	0	0	0	0	0	0	185
05:45 PM	0	62	2	0	1	0	0	0	1	105	0	0	0	0	0	0	171
Total	0	310	8	0	7	0	1	0	3	409	0	0	0	0	0	0	738
								- •				_	_			- 1	
06:00 PM	0	56	1	0	4	0	1	0 !	1	55	0	0	0	0	0	0	118
Grand Total	oʻ	560	18	Ō	16	0	5	0	7	756	Ō	Ō	Ō	Ō	Ō	ō	1362
Apprch %	0.0	96.9	3.1	0.0	76.2	0.0	23.8	0.0	0.9	99.1	0.0	0.0	0.0	0.0	0.0	0.0	
Total %	0.0	41.1	1.3	0.0	1.2	0.0	0.4	0.0	0.5	55.5	0.0	0.0	0.0	0.0	0.0	0.0	
i Otal 70	0.0	71.1	۲.5	3.0	1.2	5.0	0.4	3.0	5.5	55.5	0.0	5.0	3.0	5.0	0.0	0.0	

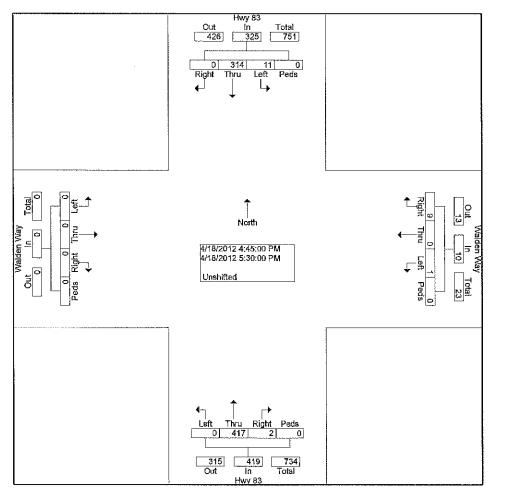
516 N. Tejon St.

Colorado Springs, CO (719) 633-2868

File Name: Hwy 83 - Walden Way PM

Site Code : 00000000 Start Date : 04/18/2012

			Hwy 8					ilden \ rom Ea					Hwy 8 om So					alden V]
Start	Rig		Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Int.
Time	ht		t	ds	Total	ht	''u	1	ds	Total	ht	u	t	ds	Total	ht	u		ds	Total	Total
Peak Hour		04:15	PM to					1	ų,	100	1116	u I			TOTAL	111	<u> </u>			Total	TOtal
Intersecti оп	04;45							•													
Volume	0	31 4	11	0	325	9	0	1	0	10	2	41 7	0	0	419	0	0	0	0	0	754
Percent	0.0	96. 6	3.4	0.0		90. 0	0.0	10. 0	0.0		0.5	99. 5	0.0	0.0		0.0	0.0	0.0	0.0		
05:15 Volume Peak	0	81	2	0	83	3	0	0	0	3	2	10 9	0	0	111	0	0	0	0	0	197 0.957
Factor High Int.	05:00	PM				04:4	5 PM				04:4					4:00	:00 PI	И			
Volume	0	86	1	0	87	3	0	0	0	3	0	11 3	0	0	113						
Peak Factor					0.93 4					0.83 3					0.92 7						



516 N. Tejon St.

LSC Transportation Consultants, Inc.

Colorado Springs, CO (719) 633-2868

File Name: US 83 - CR105 AM

Site Code : 00000000 Start Date : 11/15/2012

Page No : 1

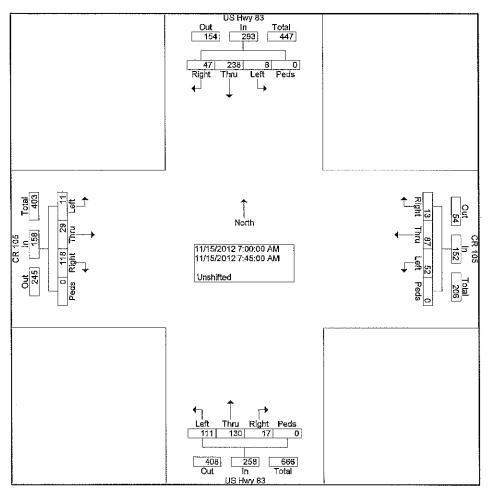
		US H	,			CR				US H	,				105		
	Diab	From	North	Dod	DE-L	From	∟ast	D-3	Diek	From	South	D- 4	Dist	From	West	- B I	1.1
Start Time	Righ t	Thru	Left	Ped s	Rìgh t	Thru	Left	Pedi s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
06:30 AM	8	56	2	0	5	10	7	0	1	37	18	0	20	3	2	0	169
06:45 AM	13	42	0	0	5	18	15	0	2	35	21	0	17	2	4	0	174
Total	21	98	2	0	10	28	22	0	3	72	39	0	37	5	6	0	343
07:00 AM	13	55	1	0	5	28	17	0	2	40	26	0	27	6	1	0	221
07:15 AM	12	56	0	0	2	22	8	0	2	31	29	0	33	6	4	0	205
07:30 AM	10	76	3	0	5	16	12	0	5	24	24	0	35	4	2	0	216
07:45 AM	12	51	4	0	. 1	21	15	0	8	35	32	0	23	13	4	0	219
Total	47	238	8	0	13	87	52	0	17	130	111	0	118	29	11	0	861
MA 00:80	2	48	4	0	2	13	8	0	4	32	24	0	22	12	6	0	177
08:15 AM	6	47	0	0	1	11	8	0	10	37	21	0	24	10	0	0	175
Grand Total	76	431	14	0	26	139	90	0	34	271	195	0	201	56	23	0	1556
Apprch %	14.6	82.7	2.7	0.0	10.2	54.5	35.3	0.0	6.8	54.2	39.0	0.0	71.8	20.0	8.2	0.0	
Total %	4.9	27.7	0.9	0.0	1.7	8.9	5.8	0.0	2.2	17.4	12.5	0.0	12.9	3.6	1.5	0.0	

LSC Transportation Consultants, Inc. 516 N. Tejon St. Colorado Springs, CO (719) 633-2868

File Name: US 83 - CR105 AM

Site Code : 00000000 Start Date : 11/15/2012

			3 Hwy					CR 10					Hwy					CR 10		****	
			om No			ļ <u></u>		om E			L		m So					om W			L
Start	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	Арр.	Int.
Time	ht	u	t]	ds	Total	ht	u	t	ds	Total	ht	ш	t	ds	Total	ht	u	t l	ds	Total	Total
Peak Hour	From	06:30	AM to	o 08:1	5 AM -	Peak	1 of '	j													
Intersecti on	07:00	MA C																			
Volume	47	23 8	8	0	293	13	87	52	0	152	17	13 0	11 1	0	258	11 8	29	11	0	158	861
Percent	16. 0	81. 2	2.7	0.0		8.6	57. 2	34. 2	0.0		6.6	50. 4	43. 0	0.0		74. 7	18. 4	7.0	0.0		
07:00 Volume Peak	13	55	1	0	69	5	28	17	0	50	2	40	26	0	68	27	6	1	0	34	221 0.974
Factor High Int.	07:30	1 A K A				07:00	2 4 5 4				07:4	E AKA				07:1	= ARA				0.974
			_	_	00			47	_		,		20			,		4	_	40	1
Volume	10	76	3	0	89	5	28	17	0	50	8	35	32	0	75	33	6	4	U	43	
Peak					0.82					0.76					0.86					0.91	
Factor					. 3					0	-				0					9	



516 N. Tejon St.

LSC Transportation Consultants, Inc.

Colorado Springs, CO (719) 633-2868

File Name: US 83 - CR105 PM

Site Code : 00000000 Start Date : 11/15/2012

Page No : 1

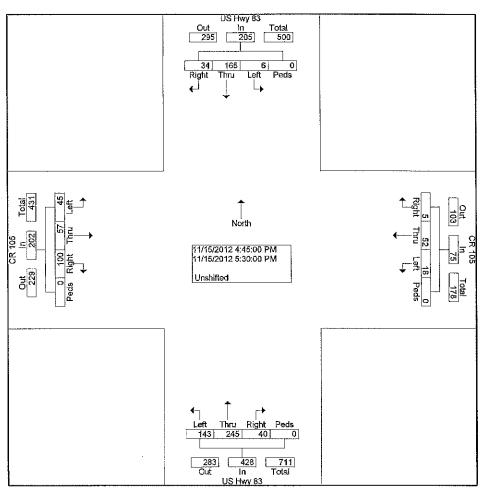
		US H				CR From				US H	•			CR From	105 West		
Start Time	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
04:00 PM	11	44	2	0	5	12	9	0	7	60	27	0	28	14	10	0	229
04:15 PM	7	43	2	0	3	5	10	0	9	52	27	0	22	15	10	0	205
04:30 PM	9	53	7	0	1	14	10	0	6	41	25	0	22	8	4	0	200
04:45 PM	9	34	1_	0	1	13	6	0	11	63	36	0	27	14	8	0	223
Total	36	174	12	0	10	44	35	0	33	216	115	0	99	51	32	0	857
05:00 PM	6	49	2	0	0	8	5	0	9	68	38	0	32	21	15	0	253
05:15 PM	7	29	1	0	1	15	5	0	12	58	34	0	19	11	9	0	201
05:30 PM	12	53	2	0	3	16	2	0	8	56	35	0	22	11	13	0	233
05:45 PM	7	41	5_	0	0	9	3	0	11	62	34	0	25	9	10	0	216
Total	32	172	10	0	4	48	15	0	40	244	141	0	98	52	47	0	903
Grand Total Apprch % Total %	68 15.6 3.9	346 79.4 19.7	22 5.0 1.3	0 0.0 0.0	14 9.0 0.8	92 59.0 5.2	50 32.1 2.8	0.0 0.0	73 9.3 4.1	460 58.3 26.1	256 32.4 14.5	0 0.0 0.0	197 52.0 11.2	103 27.2 5.9	79 20.8 4.5	0.0 0.0	1760

LSC Transportation Consultants, Inc. 516 N. Tejon St. Colorado Springs, CO (719) 633-2868

File Name : US 83 - CR105 PM

Site Code : 00000000 Start Date : 11/15/2012

			Hwy					CR 10 om E					S Hwy					CR 10 om W	_		
Start	Rig	Thr		Pe	Арр.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig		Lef		App.	Int.
Time	ht	u	ŧ	dş	Total	ht	u	ŧ	ds	Total	hŧ	u	t	ds	Total	ht	u	_ t	ds	Total	Total
Peak Hour	From	04:00	PM t	0 05:4	5 PM -	Peak	1 of	1													
Intersecti on	04:45	5 PM																			
Volume	34	16 5	6	0	205	5	52	18	0	75	40	24 5	14 3	0	428	10 0	57	45	0	202	910
Percent	16. 6	80. 5	2.9	0.0		6.7	69. 3	24. 0	0.0		9.3	57. 2	33. 4	0.0		49. 5	28. 2	22. 3	0.0		
05:00 Volume	6	49	2	0	57	0	8	5	0	13	9	68	38	0	115	32	21	15	0	68	253
Peak Factor																					0.899
High Int.	05:30					05:1		_			05:0	_		_		05:00					ļ
Volume	12	53	2	0	67	1	15	5	0	21	9	68	38	0	115	32	21	15	0	68	
Peak Factor					0.7 6 5					0.8 9 3					0.93					0.74 3	



N/S STREET: TIMBER MEADOWS DR E/W STREET: POND VIEW PL CITY: BLACK FOREST COUNTY: EL PASO

1889 YORK ST DENVER, COLORADO 303-333-7409

File Name: TIMBPOND Site Code : 00000005 Start Date : 11/15/2012 Page No : 1

III. LL FAGO	,				O	7-1-4-4 V	ELUAL EA				Pag	je No :	1
——т	TILADE	MEADO	ALDD I	BON			EHICLES						
					D VIEW F	<u>"-</u>		MEADO		-			•
Start Time	Left	uthbound Thru			estbound	D' 11		orthbound			estbound		
Factor			Right	Left	Thru	Right	Left	Thru	Right	Left	Thru	Right	Int. Total
06:30 AM	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
	0	2	0	6	0	0	0	1	0	0	0	0	9
06:45 AM	0	2	0	8	0	0	0		1	0	0	0	12
Total	U	4	0	14	0	0	0	2	1	0	0	0	21
07:00 AM	0	0	0	6	0	1	0	1	6	0.	0	0	14
07:15 AM	Ō	Ì	0	6 ÌÕ	O Õ	Ŏ	õ	i	9	ě	ŏ	ő	21
07:30 AM	0	4	0	14	Ō	0	ō	2	1	ŏ	ŏ	ŏ	21
07:45 AM	0	1	0	4	0	0	Ō	0	3	ō	ō	ō	8
Total	0	6	0	34	0	1	0	4	19	0	0	0	64
MA 00:80	0	1	0	10	0	0	0	0	2	0	0	0	13
08:15 AM	1	1	0	8	0	O	0	4	3	Ö	Ō	0	17
Total	1	2	0	18	0	0	0	4	5	0	0	0	30
04:00 PM	0	0	0	15	0	1	0	3	7	0	0	0	26
04:15 PM	0	2	0	2	Ō	0	Ŏ	. 1	7	Ŏ	ŏ	ō	12
04:30 PM	0	1	o Ì	1	0	1			12			ō	18
04:45 PM	Õ	1	ŏ	Ĝ	Ŏ Ŏ	Ò	0 0	3 3	4	o O	o O	Ò	14
Total	0	4	0	24	0	2	0	10	30	0	0	0	70
05:00 PM	0	1	0	4	0	0	o	4	10	0	0	0	19
05:15 PM	0	3	0	5	0	0	Ō	4	11	ō	Ö	ō	23
05:30 PM	0	1	0	3	Ō	0	Ō	3	11	ō	ō	ō	18
05:45 PM	0	1	a	4	0	0	Ō	3	9	Ŏ	ō	ō	17
Total	Q	6,	Q	16	Q.	Q	0,	14	41	Q	Q	Q	77
Grand Total	1	22	0	106	0	3	0	34	96	0	0	0	262
Apprch %	4.3	95.7	0.0	97.2	0.0	2.8	0.0	26.2	73.8	0.0	0.0	0.0	
Total %	0.4	8.4	0.0										

1889 YORK ST DENVER, COLORADO

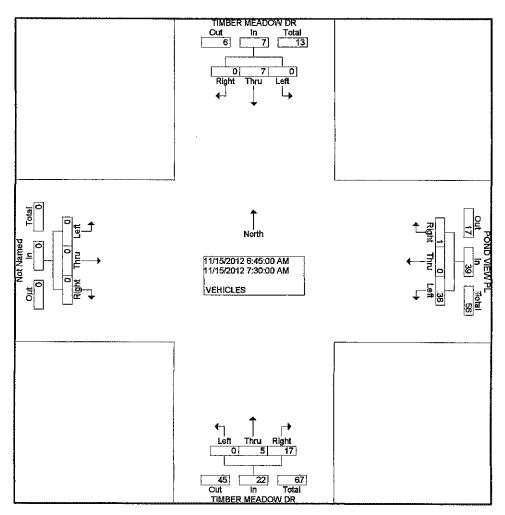
N/S STREET: TIMBER MEADOWS DR E/W STREET: POND VIEW PL CITY: BLACK FOREST

COUNTY: EL PASO

303-333-7409

File Name: TIMBPOND Site Code : 00000005 Start Date : 11/15/2012 Page No : 2

	TIM		EADOV hbound				VIEW P	L	TIM		EADOV	V DR		Êast	bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	int. Total
Peak Hour Fro	m 06:3	O AM to	08:30	AM - Pea	k 1 of 1												
Intersection	06:45	AM														.	
Volume	0	7	0	7	38	0	1	39	0	5	17	22	0	0	0	0	68
Percent	0.0	100. 0	0.0		97.4	0.0	2.6		0.0	22.7	77.3		0.0	0.0	0.0		
07:30 Volume	0	4	0	4	14	0	0	14	0	2	1	3	0	0	0	٥	21
Peak Factor High Int.	07:30	AM			07:30	AM			07:15	AM			6:15:0	0 AM			0.810
Volume Peak Factor	0	4	0	4 0.438	14	0	0	14 0,696	0	1	9	10 0.550					



1889 YORK ST

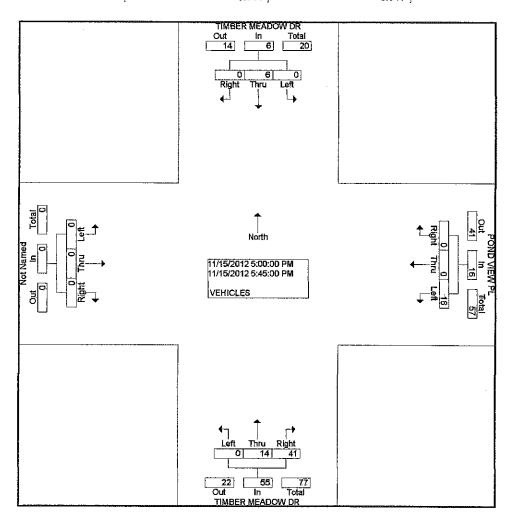
DENVER, COLORADO 303-333-7409

E/W STREET: POND VIEW PL CITY: BLACK FOREST COUNTY: EL PASO

N/S STREET: TIMBER MEADOWS DR

File Name : TIMBPOND Site Code : 00000005 Start Date : 11/15/2012 Page No : 2

	TIM		EADOV nbound				VIEW P	L .	TIM	·	EADOV	V DR		Ëast	bound		
Start Time	Left	Thru	Right	App. Total	Left	Thru	Right	App. Total	Left	Thru	Ríght	App. Total	Left	Thru	Right	App. Total	Int. Total
Peak Hour Fro	om 04:0	0 PM to	05:45	PM - Pea	k 1 of 1							············				***************************************	
Intersection	05;00	PM															
Volume	0	6	0	6	16	0	0	16	0	14	41	55	0	0	0	0	77
Percent	0.0	100. Q	0.0		100. Q	0.0	0,0		0.0	25.5	74.5		0.0	0.0	0.0		
05:15 Volume	0	3	0	3	5	0	0	5	0	4	11	15	0	0	0	0	23
Peak Factor																	0.837
High Int.	05:15	PM			05:15	PM			05:15	PM							
Volume	0	3	0	3	5	0	0	5	0	4	11	15					
Peak Factor				0.500				0.800				0.917					



LSC Transportation Consultants, Inc. 516 N. Tejon St. Colorado Springs, CO 719-633-2868

Timber Meadow NO Hodgen - vol Site Code: Station ID:

Latitude: 0' 0.000 South

Start	16-A	or-12	17-A		18-A	pr-12	19-A	or-12	20-A	pr-12	Weekday	Average	21-A		22-A	
Time	Northbou nd		Northbou nd	Southbo und												
12:00																
AM	*	*	*	*	*	*	*	*	2	0	2	0	3	2	2	
01:00	*	*	*	*	*	*	*	*	1	1	1	1	1	0	0	(
02:00	*	*	*	*	*	*	*	*	6	0	6	0	2	2	1	2
03:00	*	*	*	*	*	*	*	*	0	0	0	0	1	0	4.000	(
04:00	*	*	*	*	*	*	*	*	0	4	0	4	0	2	10	
05:00	*	*	*	*	*	*	*	*	0	6	0	6	0	1	1	6
06:00	*	*	*	*	*	*	*	*	1	31	1	31	1	8	1	6
07:00	*	*	*	*	*	*	*	*	10	54	10	54	10	26	5	10
08:00	*	*	*	*	*	*	*	*	21	43	21	43	13	31	6	32
09:00	*	*	*	*	*	*	*	*	9	39	9	39	22	56	5	27
10:00	*	*	*	*	*	*	21	44	32	42	26	43	19	41	20	33
11:00	*	*	*	*	*	*	30	27	35	52	32	40	22	47	21	32
12:00																
PM	*	*	*	*	*	*	29	30	36	24	32	27	36	35	30	26
01:00	*	*	*	*	*	*	19	28	22	37	20	32	26	33	26	2
02:00	*	*	*	*	*	*	30	23	29	32	30	28	36	26	32	19
03:00	*	*	*	*	*	*	50	61	39	26	44	44	33	41	30	28
04:00	*	*	*	*	*	*	50	60	49	48	50	54	25	39	20	29
05:00	*	*	*	*	*	*	46	40	43	47	44	44	27	31	41	1
06:00	*	*	*	*	*	*	52	26	53	43	52	34	19	29	17	13
07:00	*	*	*	*	*	*	36	29	23	21	30	25	19	10	18	16
08:00	*	*	*	*	*	*	21	14	17	10	19	12	12	7	17	(
09:00	*	*	*	*	*	*	21	1	24	3	22	2	20	5	9	- 5
10:00	*	*	*	*	*	*	7	3	21	3	14	3	13	1	2	(
11:00	*	*	*	*	*	*	0	1	7	6	4	4	3	2	1	(
Total	0	0	0	0	0	0	412	387	480	572	469	570	363	475	318	342
Day	C		0)	()	79	19	10	52	10	39	83		66	
AM Peak							11:00	10:00	11:00	07:00	11:00	07:00	09:00	09:00	11:00	10:00
Vol.							30	44	35	54	32	54	22	56	21	33
PM Peak							18:00	15:00	18:00	16:00	18:00	16:00	12:00	15:00	17:00	16:00
Vol.							52	61	53	48	52	54	36	41	41	29

LSC Transportation Consultants, Inc. 516 N. Tejon St. Colorado Springs, CO 719-633-2868

Timber Meadow NO Hodgen - vol Site Code: Station ID:

Latitude: 0' 0.000 South

Start Time	3-Ap	r-12 Southbo	4-Ap	or-12 Southbo	5-Ap	thbo	6-Ap	or-12 Southbo	Nort	S.	Weekday	\$ is	28-Apr-12 Northbou Sou	50	29-Apr-12 Northbou Southbo	-12 Southbo
40.00	nd	pun	Pu	pun	pu	pun	pu	pun	Б	pun	P	pun	DI.	pun	2	nuq
AM M	c	C	•	0	C	C	~	-	*	*		•	*	*	*	*
01:00	0	0	•	10	0	0	0	0	*	*	0	0	*	*	*	*
02:00	0	0	0	0	0	0	-	0	*	*	0	0	*	*	*	*
03:00		0	0	2	0	2	0	0	*	*	0	-	*	*	*	*
04:00	2	2	2	7	2	0	2	0	*	*	2	•	*	*	*	*
05:00	0	8	0	4	0	7	0	15	*	*	0	8	*	*	*	*
00:90	2	31	~	30	S	47	7	28	*	*	4	34	*	*	*	*
00:20	17	87	1	67	17	82	18	78	*	*	16	78	*	*	*	*
00	26	37	28	54	33	62	32	62	*	*	30	54	*	*	*	*
00:60	22	43	15	38	17	39	15	53	*	*	17	43	*	*	*	*
10:00	23	46	13	29	12	29	*	*	*	*	16	35	*	*	*	*
11:00	20	39	27	46	21	33	*	*	*	*	23	39	*	*	*	*
5:00	00	0,	22	5	00	90	*	*	*	*	22	32	*	*	*	*
01:00	24	28	8.	388	20	21	*	*	*	*	26	29	*	*	*	*
02:00	33	33	34	51	21	24	*	*	*	*	29	35	*	*	*	*
03:00	38	32	49	42	42	29	*	*	*	*	43	34	*	*	*	*
04:00	42	57	43	40	51	44	*	*	*	*	45	47	*	*	*	*
05:00	22	45		51	54	43	*	*	*	*	58	46	*	*	*	*
00:90	4	33		29	55	41	*	*	*	*	49	34	*	*	*	*
00	46	16		26	32	6	*	*	*	*	37	17	*	*	*	*
08:00	26	20		21	21	4	*	*	*	*	24	18	*	*	*	*
00:60	21	5	25	13	10	00	*	*	×	ķ	19	0	*	*	*	*
10:00	7	2	10	~	14	-	*	*	*	*	10	-	*	k	*	¥
11:00	2	0	2	2	2	2	*	*	*	*	က	1	*	*	*	*
Fotal	473	581	499	638	449	563	92	237	0	0	474	265	0	0	0	0
Day	1054	25	1137	8	1012		313		0	0	1071		0	0	0	
AM Peak	08:00	02:00	08:00	00:20	08:00	02:00	08:00	00:20			08:00	02:00				
Vol.	56	87	28	29	33	82	32	78			30	78				
PM Peak Vol.	17:00	16:00 57	17:00 66	14:00 51	18:00 55	16:00					17:00	16:00				
Comb. Total	1054	4	1137	7	1012		1112	12	10	1052	2110	10	838	82	099	
ADT		ADT 959	ď	AADT 959												

Location: HIGHVIEW DR S/O WALKER RD

City:
County: EL PASO
Direction: NORTHBOUND-SOUTHBOUND

1889 YORK ST DENVER,COLORADO 80206 303-333-7409

Site Code: 111305

Start	14-Nov-12	·		
Time	Wed	NB	SB	Total
12:00 AM 01:00 02:00	production of the second of th	1 0 1	0 1 0	
03:00 04:00		0 1		N 1943 - Prince (1940) - Princ
05:00 06:00	in the second	6 26	2 12	38 1997 2011 1997 2011 1997 2011 1997 2011 1997 2011 1997 2011 1997 2011 1997 2011 1997 2011 1997 2011 1997 20 38
07:00 08:00		33 28	16 15	49 43
09:00 10:00		28 14 12	11 12	25) 24
11: 00 12:00 PM		15 17	1 3 13	28) 30
01:00 02:00		22 20	24 18	46) 38
03:00 04:00		25 30	22 28	47) 58
05:00 06:00		38 13	48 19	86 32
07:00 08:00		19 14	15 14	
09:00 10:00		4 3 0	8 5	28 12 8
11:00			0.	
Total		342	298	640
Percent		53.4%	46.6%	
AM Peak		07:00	07:00	07:00
Vol.		33 47.00	16 17:00	49
PM Peak Vol.		17:00 38	17:00 48	17:00 86

Location: HIGHVIEW DR S/O WALKER RD

County: EL PASO
Direction: NORTHBOUND-SOUTHBOUND

1889 YORK ST DENVER,COLORADO 80206 303-333-7409

Site Code: 111305

Start	15-Nov-1	2			
Time	Thu	NB	SE	3	Total
12:00 AM		0		0	0
01:00 02:00		0		D .	
03:00) 1	0 . The results of $oldsymbol{\kappa}_{0}$
04:00		· 5		1	Dentification of the first of the control of the control of the charge territories and the control of the contr
05:00		6 25		3	[] - [] - [] - [] - [] - [] - [] - [] -
06:00 07:00		25 43	12	<u>2</u> ■a	37
07:00 08:00	- " .	32	2 2		70
09:00	;	22 22	1	.	53 - 1888 - 1888 - 1888 - 1888 - 1888 - 1888 - 1888 - 1888 - 1888 - 1888 - 1888 - 1888 - 1888 - 1888 - 1888 - 188
10:00		20	14	į .	34
11,00		19	16	Š	
12:00 PM 01:00		18 27	2	 }} ==================================	39
02:00	•	16	16	3	48
03:00		37	36		
04:00		30	36	}	66
05:00 06:00		27 26	31 26		58 52
07:00		12	19) }.	52 71.43
08:00	* '	5	10)	15
09:00		5	E		
10:00 11:00		3 0	2	Na sangan	n marine programme in the control of
Total		375	340	<u> 28 19 3 2</u> 	715
Percent		52.4%	47.6%		710
AM Peak		07:00	07:00		07:00
Vol.		43	27		70
PM Peak		15:00	15:00		15:00
Vol	·	37	36		73
Grand Total			717	638	135
Percent		52	2.9%	47.1%	
ADT		Α	DT 640		AADT 640

COUNTER MEASURES INC. 1889 YORK ST DENVER, COLORADO 80206 303-333-7409

Location: POND VIEW DR E/O TIMBER MEADOW

City: County: EL PASO Direction: EASTBOUND-WESTBOUND

Site Code: 111307

Start	14-Nov-12			
Time	Wed	EB	WB	Total
12:00 AM 01:00 02:00		1 3 0	0 0 0	TARAN MANAGEMENTAN AND AND AND AND AND AND AND AND AND A
03:00 04:00		0	. 0 1	
05:00 06:00	The state of the s	0 3		7 33
07: <u>00</u> 08:00		22 14	44 40	66 54
09:00 10:00		17 18	26 22	43
11;00 12:00 PM	en e	15 19	14 26	70 29 29 45
01:00 02:00	Maria de la companya	23 24	18	
03:00 04:00		46 2 6	20 30 23	76 49 83
05:00 06:00	0 1971 2 ASS	50 30	33 10	
07:00 08:00		20 16	4 6	40 24 22
09:00 10:00		11 8	3	
11:00		3	. 11.15% . j ev .	<u> A CONTRACTOR O CONTRACTOR O CONTRACTOR DE </u>
Total		369	359	728
Percent		50.7%	49.3%	
AM Peak		07:00	07:00	07:00
Vol.		22	44	66
PM Peak		17:00	17:00	17:00
Vol.		50	33	83

COUNTER MEASURES INC.

Location: POND VIEW DR E/O TIMBER MEADOW City: County: EL PASO Direction: EASTBOUND-WESTBOUND

1889 YORK ST DENVER,COLORADO 80206 303-333-7409

Site Code: 111307

Start	15-Nov-				
Time	Thu	EB	WE	/B Total	i
12:00 AM		3		1	4
01:00 02:00	•	<u>.</u> 0 0 0) (0
03:00 04:00		. <u> </u>) [0
05:00 06:00		2	76 26		_9 29
07:00 08:00		19 25	36	the contract of the contract o	55
09:00 10:00	6 - C	22 22 18	24 21		46
11:00 12:00 PM		27	. 22		69 46 39 49 55
01:00 02:00		18 19	34	學媒 이 시간에 많아가는 일어받아 그 없는 그는 그는 그 가지 않는 사람들이 되는 그를 가는 그를 가는 사람이 받았다. 이 점심장	52 37
03:00 04:00	1.6	40 28	30 21		70 49
05:00 06:00		40 31			56 47
07:00 08:00		1 <u>4</u> 12	6		20 20
09:00 10:00		25 5	2		27 27
11:00		4	õ		4
Total		385	361		46
Percent		51.6%	48.4%		
AM Peak		11:00	08:00		00
Vol.		27	44	· ·	69
PM Peak		15:00	13:00	0 15:0	
Vol		40	34		70
Grand Total Percent			754 51.2% 4	720 48.8%	14
ADT			ADT 728	AADT 728	

COUNTER MEASURES INC.

Location: WOODHAVEN DR S/O WALKER RD

City:
County: EL PASO
Direction: NORTHBOUND-SOUTHBOUND

1889 YORK ST DENVER,COLORADO 80206 303-333-7409

Site Code: 111312

Start	14-Nov-12			
Time	Wed	NB	SB	Total
12:00 AM 01:00 02:00	14. 14.	0 1 0	2 1 0	2 2 2 0
03:00 04:00		<u>0</u> 3	0 1	
05:00 06:00		12 30	2 4	- 14 - 15 15 15 15 15 15 15 15 15 15 15 15 15
07:00 08:00	384. 382.	58 21	10 15	68
09 :00 10:00		17 18	8	36 1
11:00 12:00 PM		11	12 15 13	30 26 29
01:00 02:00	1 1 <u>1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 </u>	18	14 17	29 32 28 41
03:00 04:00	W.S.	13 21	28 45	41 66
05:00 06:00		16	44 17	60
07:00 08:00	ter Miller of the State of the	16 9 4	15 14	33 24 18
09:00 10:00	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	<u>2</u> 1	12	-
11:00		. 1	2	3
Total		299	292	591
Percent		50.6%	49.4%	
AM Peak		07:00	08:00	07:00
Vol.		58	15	68
PM Peak Vol.		16:0 0 21	16:00 45	16:00 66
VOI.		2 1	40	00

COUNTER MEASURES INC.

Location: WOODHAVEN DR S/O WALKER RD City:

County: EL PASO
Direction: NORTHBOUND-SOUTHBOUND

1889 YORK ST DENVER,COLORADO 80206 303-333-7409

Site Code: 111312

Start	15-Nov-12					
Time	Thu N		SB		To	otal
12:00 AM		0	1			1
01:00 02:00		0	1			1
03:00		0	0	±y o	e de la companya de	0
04.00		2	0			2
05:00		10 31				12
06:00		31	2 5	Francisco (Control Control Con	en e	1 <u>2</u> 36
07:00		49	10			59 ∣
08:00			12		· · · · · · · · · · · · · · · · · · ·	
09:00		17	12 8			30 2 9 18
10:00 11:00		10 20	8 10	en en en en e		18
12:00 PM			10 16		and the state of t	30 36 35
01:00			13		in the second second second	30
02:00	Mala Administra	20	34			55 ·
03:00		20 22 20	38			60
04:00		20	28	the section of the se	The second secon	48
05:00		18 21	34			52
06:00 07:00		21	30 19	The second secon		51
08:00		12 2 2	19 17			31
09:00		2	7	and the second of the second of the second	i de la companya de l	19
10:00		0	4	en en mante de la companya del companya del companya de la company	5 July 2 (1997) 21 2111	. 9 4
11:00	The second secon	2	2			- - 4
Total			03			621
Percent	51.2					
AM Peak Vol.	07:0				O	7:00
PM Peak	13:0		12 10		_	59
Vol.	10.0	22 :	38		1	5:00 60
Grand Total		617	595			1212
Percent		50.9%	49.1%			1212
ADT		ADT 591		AADT 591		-

	۶	→	•	•	←	•	4	†	/	>	ļ	
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	7	†	7	ሻ	†	7	7	^	7	ሻ	ĵ»	
Volume (vph)	14	31	54	183	57	90	29	110	29	43	319	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	8.0	8.0	8.0	8.0	8.0	8.0	25.0	25.0	25.0	25.0	25.0	
Minimum Split (s)	15.0	15.0	15.0	15.0	15.0	15.0	36.0	36.0	36.0	36.0	36.0	
Total Split (s)	25.0	25.0	25.0	25.0	25.0	25.0	45.0	45.0	45.0	45.0	45.0	
Total Split (%)	35.7%	35.7%	35.7%	35.7%	35.7%	35.7%	64.3%	64.3%	64.3%	64.3%	64.3%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	Min	Min	Min	Min	Min	
Act Effct Green (s)	15.2	15.2	15.2	15.2	15.2	15.2	27.6	27.6	27.6	27.6	27.6	
Actuated g/C Ratio	0.30	0.30	0.30	0.30	0.30	0.30	0.54	0.54	0.54	0.54	0.54	
v/c Ratio	0.04	0.07	0.13	0.48	0.11	0.18	0.06	0.12	0.04	0.07	0.34	
Control Delay	12.0	12.2	4.5	18.4	12.6	4.2	7.3	7.2	2.1	7.2	8.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	12.0	12.2	4.5	18.4	12.6	4.2	7.3	7.2	2.1	7.2	8.6	
LOS	В	В	Α	В	В	Α	Α	Α	Α	Α	Α	
Approach Delay		8.0			13.5			6.3			8.5	
Approach LOS		Α			В			Α			Α	

Cycle Length: 70

Actuated Cycle Length: 50.8

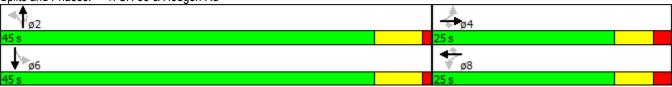
Natural Cycle: 55

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.48 Intersection Signal Delay: 9.7 Intersection Capacity Utilization 58.3%

Intersection LOS: A ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: SH 83 & Hodgen Rd



Intersection							
Intersection	0.4						
Int Delay, s/veh	0.4						
Movement	WBL	WBR		NBT	NBR	SBL	SBT
Vol, veh/h	5	13		257	0	5	377
Conflicting Peds, #/hr	0	0		0	0	0	0
Sign Control	Stop	Stop		Free	Free	Free	Free
RT Channelized	-	None		-	None	-	None
Storage Length	-	-		-	-	-	-
Veh in Median Storage, #	0	-		0	-	-	0
Grade, %	0	-		0	-	-	0
Peak Hour Factor	83	83		78	78	99	99
Heavy Vehicles, %	0	0		5	0	0	5
Mvmt Flow	6	16		329	0	5	381
Major/Minor	Minor1			Major1		Major2	
Conflicting Flow All	720	329		0	0	329	0
Stage 1	329	-		-	-	-	-
Stage 2	391	_		_	_	-	_
Critical Hdwy	6.4	6.2		_	_	4.1	_
Critical Hdwy Stg 1	5.4	-		_	_	-	_
Critical Hdwy Stg 2	5.4	-		_	_	_	_
Follow-up Hdwy	3.5	3.3		_	_	2.2	_
Pot Cap-1 Maneuver	398	717		-	_	1242	_
Stage 1	734	-		_	_	-	_
Stage 2	688	-		-	_	_	_
Platoon blocked, %				_	_		-
Mov Cap-1 Maneuver	396	717		-	-	1242	-
Mov Cap-2 Maneuver	396	-		_	-	-	-
Stage 1	734	-		-	-	-	-
Stage 2	685	-		_	-	_	_
J							
Approach	WB			NB		SB	
HCM Control Delay, s	11.4			0		0.1	
	11.4 B			U		0.1	
HCM LOS	В						
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT			
Capacity (veh/h)	-		1242	-			
HCM Lane V/C Ratio	-	- 0.037		-			
HCM Control Delay (s)	-	- 11.4	7.9	0			
HCM Lane LOS	-	- B	Α	Α			
HCM 95th %tile Q(veh)	-	- 0.1	0	-			

Intersection														
Int Delay, s/veh	8.9													
ine Boldy, or von	0.0													
Movement	EBL	EBT	EBR		WBL	WBT	WBR		NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	11	29	118		52	87	13		111	130	17	8	238	47
Conflicting Peds, #/hr	0	0	0		0	0	0		0	0	0	0	0	0
Sign Control	Stop	Stop	Stop		Stop	Stop	Stop		Free	Free	Free	Free	Free	Free
RT Channelized	-	-	Yeild		-	-	Yeild		-	-	None	-	-	None
Storage Length	-	-	200		-	-	200		100	-	100	100	-	100
Veh in Median Storage, #	+ -	0	-		-	0	-		-	0	-	-	0	-
Grade, %	-	0	-		-	0	-		-	0	-	-	0	-
Peak Hour Factor	100	100	100		76	76	76		95	95	95	100	100	100
Heavy Vehicles, %	2	2	2		2	2	2		2	5	2	2	5	2
Mvmt Flow	11	29	118		68	114	17		117	137	18	8	238	47
Major/Minor	Minor2			N	Minor1			1	Major1			Major2		
Conflicting Flow All	682	625	238		640	625	137		238	0	0	137	0	0
Stage 1	254	254	_		371	371	-		-	-	-	-	-	_
Stage 2	428	371	_		269	254	-		-	-	_	-	-	-
Critical Hdwy	7.12	6.52	6.22		7.12	6.52	6.22		4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-		6.12	5.52	-		-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-		6.12	5.52	-		-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318		3.518	4.018	3.318		2.218	-	-	2.218	-	-
Pot Cap-1 Maneuver	364	401	801		388	401	911		1329	-	-	1447	-	-
Stage 1	750	697	-		649	620	-		-	-	-	-	-	-
Stage 2	605	620	-		737	697	-		-	-	-	-	-	_
Platoon blocked, %										-	-		-	-
Mov Cap-1 Maneuver	252	364	801		289	364	911		1329	-	-	1447	-	-
Mov Cap-2 Maneuver	252	364	-		289	364	-		-	-	-	-	-	-
Stage 1	684	693	-		592	565	-		-	-	-	-	-	-
Stage 2	432	565	-		599	693	-		-	-	-	-	-	-
Approach	EB				WB				NB			SB		
HCM Control Delay, s	12.2				26.7				3.4			0.2		
HCM LOS	В				D									
Minor Lane/Major Mvmt	NBL	NBT	NBR I	EBLn1 E	EBLn2\	WBLn1V	VBLn2	SBL	SBT	SBR				
Capacity (veh/h)	1329	_	-	324	801	332	911	1447	-	-				
HCM Lane V/C Ratio	0.088	-	_	0.123				0.006	_	_				
HCM Control Delay (s)	8	-	_	17.7	10.3	28.4	9	7.5	-	_				
HCM Lane LOS	A	-	-	С	В	D	A	Α	-	-				_
HCM 95th %tile Q(veh)	0.3	-	-	0.4	0.5	3.1	0.1	0	-	-				

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	7	†	7	7	†	7	ሻ	^	7	ሻ	₽	
Volume (vph)	17	73	24	38	43	55	71	305	114	104	171	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	8.0	8.0	8.0	8.0	8.0	8.0	25.0	25.0	25.0	25.0	25.0	
Minimum Split (s)	15.0	15.0	15.0	15.0	15.0	15.0	36.0	36.0	36.0	36.0	36.0	
Total Split (s)	25.0	25.0	25.0	25.0	25.0	25.0	45.0	45.0	45.0	45.0	45.0	
Total Split (%)	35.7%	35.7%	35.7%	35.7%	35.7%	35.7%	64.3%	64.3%	64.3%	64.3%	64.3%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	Min	Min	Min	Min	Min	
Act Effct Green (s)	10.7	10.7	10.7	10.7	10.7	10.7	30.8	30.8	30.8	30.8	30.8	
Actuated g/C Ratio	0.23	0.23	0.23	0.23	0.23	0.23	0.67	0.67	0.67	0.67	0.67	
v/c Ratio	0.06	0.19	0.07	0.14	0.11	0.15	0.11	0.29	0.12	0.18	0.18	
Control Delay	14.0	15.2	3.7	15.0	14.4	5.8	4.9	5.4	1.5	5.5	4.6	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	14.0	15.2	3.7	15.0	14.4	5.8	4.9	5.4	1.5	5.5	4.6	
LOS	В	В	Α	В	В	Α	Α	Α	Α	Α	Α	
Approach Delay		12.6			11.1			4.4			4.9	
Approach LOS		В			В			Α			Α	

Cycle Length: 70

Actuated Cycle Length: 45.9

Natural Cycle: 55

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.29 Intersection Signal Delay: 6.3 Intersection Capacity Utilization 60.4%

Intersection LOS: A ICU Level of Service B

Analysis Period (min) 15

Splits and Phases: 1: SH 83 & Hodgen Rd



Synchro 8 Report Existing (2012) Traffic PM Peak Hour KDF

Intersection								
Int Delay, s/veh	0.3							
Movement	WBL	WBR			NBT	NBR	SBL	SBT
Vol, veh/h	1	9			417	2	11	314
Conflicting Peds, #/hr	0	0			0	0	0	0
Sign Control	Stop	Stop			Free	Free	Free	Free
RT Channelized	- -	None			-	None	-	None
Storage Length	-	-			-	-	-	-
Veh in Median Storage, #	0	-			0	-	-	0
Grade, %	0	-			0	-	-	0
Peak Hour Factor	83	83			94	94	97	97
Heavy Vehicles, %	0	0			5	0	0	5
Mvmt Flow	1	11			444	2	11	324
Major/Minor	Minor1				Major1		Major2	
Conflicting Flow All	791	445			0	0	446	0
Stage 1	445	-			-	-	-	-
Stage 2	346	-			-	=	-	-
Critical Hdwy	6.4	6.2			-	-	4.1	-
Critical Hdwy Stg 1	5.4	-			-	-	-	-
Critical Hdwy Stg 2	5.4	-			-	-	-	-
Follow-up Hdwy	3.5	3.3			-	-	2.2	-
Pot Cap-1 Maneuver	361	617			-	-	1125	-
Stage 1	650	-			-	-	-	-
Stage 2	721	-			-	-	-	-
Platoon blocked, %					-	-		-
Mov Cap-1 Maneuver	357	617			-	-	1125	-
Mov Cap-2 Maneuver	357	-			-	-	-	-
Stage 1	650	-			-	-	-	-
Stage 2	712	-			-	-	-	-
Approach	WB				NB		SB	
HCM Control Delay, s	11.4				0		0.3	
HCM LOS	В							
Minor Lane/Major Mvmt	NBT	NBR WBLn1	SBL	SBT				
Capacity (veh/h)	-	- 575	1125	-				
HCM Lane V/C Ratio	_	- 0.021	0.01	-				
HCM Control Delay (s)	-	- 11.4	8.2	0				
HCM Lane LOS	_	- B	Α	A				
HCM 95th %tile Q(veh)	_	- 0.1	0	_				

Interposition											
Intersection	8.5										
Int Delay, s/veh	0.5										
Movement	EBL	EBT	EBR		WBL	WBT	WBR		NBL	NBT	NBR
Vol., veh/h	45	57	100		18	52	5		143	245	40
Conflicting Peds, #/hr	0	0	0		0	0	0		0	0	C
Sign Control	Stop	Stop	Stop		Stop	Stop	Stop		Free	Free	Free
RT Channelized	· -	' <u>-</u>	Yeild		' <u>-</u>	-	Yeild		-	-	None
Storage Length	-	-	200		=	-	200		100	-	100
Veh in Median Storage, #	-	0	-		-	0	-		-	0	-
Grade, %	-	0	-		-	0	-		-	0	-
Peak Hour Factor	74	74	74		100	100	100		93	93	93
Heavy Vehicles, %	2	2	2		2	2	2		2	5	2
Mvmt Flow	61	77	135		18	52	5		154	263	43
Major/Minor	Minor2				Minor1				Major1		
Conflicting Flow All	794	768	183		806	768	263		183	0	0
Stage 1	197	197	-		571	571	-		-	-	-
Stage 2	597	571	-		235	197	-		-	-	-
Critical Hdwy	7.12	6.52	6.22		7.12	6.52	6.22		4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-		6.12	5.52	-		-	-	-
Critical Hdwy Stg 2	6.12	5.52	-		6.12	5.52	-		-	-	-
Follow-up Hdwy	3.518	4.018	3.318		3.518	4.018	3.318		2.218	-	-
Pot Cap-1 Maneuver	306	332	859		300	332	776		1392	-	-
Stage 1	805	738	-		506	505	-		-	-	-
Stage 2	490	505	-		768	738	-		-	-	-
Platoon blocked, %										-	-
Mov Cap-1 Maneuver	240	294	859		185	294	776		1392	-	-
Mov Cap-2 Maneuver	240	294	-		185	294	-		-	-	-
Stage 1	716	734	-		450	449	-		-	-	-
Stage 2	383	449	-		576	734	-		-	-	-
A	ED.				WD				ND		
Approach	EB 21.1				WB				NB 2.6		
HCM Control Delay, s	21.1				23.4				2.6		
HCM LOS	С				С						
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	FRI n2	WBLn1	WBLn2	SBL	SBT	SBR	
Capacity (veh/h)	1392	-	HUIT	267	859	255	776	1301		- ODIN	
HCM Lane V/C Ratio	0.11	_	-	0.516	0.157	0.275	0.006	0.005	<u>-</u>	<u>-</u>	
HCM Control Delay (s)	7.9	_	-	32	10	24.4	9.7	7.8	-	_	
HCM Lane LOS	7.5 A	_	-	D	В	24.4 C	Α.	Α.	_		
HCM 95th %tile Q(veh)	0.4	_	_	2.7	0.6	1.1	0	0	_	_	
TOWN JOHN JOHN Q(VOII)	0.4	_	_	۷.۱	0.0	1.1	0	J	_	_	

Intersection			
Int Delay, s/veh			
Movement	SBL	SBT	SBR
Vol, veh/h	6	165	34
Conflicting Peds, #/hr	0	0	0
Sign Control	Free	Free	Free
RT Channelized	-	-	None
Storage Length	100	-	100
Veh in Median Storage, #	-	0	-
Grade, %	_	0	-
Peak Hour Factor	90	90	90
Heavy Vehicles, %	2	5	2
Mymt Flow	7	183	38
WWITET IOW	'	100	30
Major/Minor	Major2		
Conflicting Flow All	263	0	0
Stage 1	-	-	-
Stage 2	_	-	-
Critical Hdwy	4.12	_	_
Critical Hdwy Stg 1		_	_
Critical Hdwy Stg 2	_	_	_
Follow-up Hdwy	2.218	_	_
Pot Cap-1 Maneuver	1301	_	-
Stage 1	1301		_
		_	
Stage 2	-	-	-
Platoon blocked, %	4004	-	-
Mov Cap-1 Maneuver	1301	-	-
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-
Approach	SB		
HCM Control Delay, s	0.2		
HCM LOS			
Minor Lane/Major Mvmt			
Willion Land Major WWITE			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	†	7	ሻ	†	7	7	^	7	ሻ	î.	
Volume (vph)	15	52	65	229	100	105	35	180	38	56	390	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	4	4	4	8	8	8						
Switch Phase												
Minimum Initial (s)	8.0	8.0	8.0	8.0	8.0	8.0	25.0	25.0	25.0	25.0	25.0	
Minimum Split (s)	15.0	15.0	15.0	15.0	15.0	15.0	36.0	36.0	36.0	36.0	36.0	
Total Split (s)	25.0	25.0	25.0	25.0	25.0	25.0	45.0	45.0	45.0	45.0	45.0	
Total Split (%)	35.7%	35.7%	35.7%	35.7%	35.7%	35.7%	64.3%	64.3%	64.3%	64.3%	64.3%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	Min	Min	Min	Min	Min	
Act Effct Green (s)	17.0	17.0	17.0	17.0	17.0	17.0	27.2	27.2	27.2	27.2	27.2	
Actuated g/C Ratio	0.33	0.33	0.33	0.33	0.33	0.33	0.52	0.52	0.52	0.52	0.52	
v/c Ratio	0.04	0.10	0.14	0.56	0.17	0.19	0.09	0.20	0.05	0.10	0.44	
Control Delay	11.7	12.2	4.2	19.9	12.9	4.0	8.3	8.3	2.9	8.1	10.4	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	11.7	12.2	4.2	19.9	12.9	4.0	8.3	8.3	2.9	8.1	10.4	
LOS	В	В	Α	В	В	Α	Α	Α	Α	Α	В	
Approach Delay		8.2			14.4			7.5			10.1	
Approach LOS		Α			В			Α			В	

Cycle Length: 70

Actuated Cycle Length: 52.2

Natural Cycle: 55

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.56 Intersection Signal Delay: 10.8 Intersection Capacity Utilization 71.0%

Intersection LOS: B ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: SH 83 & Hodgen Rd



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4	7		ર્ન	7	ሻ	↑	7	7	↑	7
Volume (vph)	13	34	141	61	103	15	134	153	20	9	278	55
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0
Total Split (s)	20.0	20.0	20.0	20.0	20.0	20.0	51.0	51.0	51.0	51.0	51.0	51.0
Total Split (%)	28.2%	28.2%	28.2%	28.2%	28.2%	28.2%	71.8%	71.8%	71.8%	71.8%	71.8%	71.8%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Lost Time (s)		5.0	5.0		5.0	5.0	5.0	5.0	5.0	5.0	5.0	5.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)		13.3	13.3		13.3	13.3	47.7	47.7	47.7	47.7	47.7	47.7
Actuated g/C Ratio		0.19	0.19		0.19	0.19	0.67	0.67	0.67	0.67	0.67	0.67
v/c Ratio		0.15	0.34		0.72	0.06	0.19	0.13	0.02	0.01	0.23	0.05
Control Delay		24.4	7.4		41.6	7.1	5.6	4.9	1.4	4.6	5.4	1.6
Queue Delay		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay		24.4	7.4		41.6	7.1	5.6	4.9	1.4	4.6	5.4	1.6
LOS		С	Α		D	Α	Α	Α	Α	Α	Α	Α
Approach Delay		11.6			38.7			5.0			4.8	
Approach LOS		В			D			Α			Α	

Cycle Length: 71

Actuated Cycle Length: 71

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 40

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.72

Intersection Signal Delay: 13.4 Intersection LOS: B
Intersection Capacity Utilization 50.0% ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 5: SH 83 & Walker Road



nt Delay, s/veh	0.3								
, ,									
Movement	WBL	WBR		NE	BT N	BR	SBL	SBT	
Vol, veh/h	0	20		30	00	0	0	451	
Conflicting Peds, #/hr	0	0			0	0	0	0	
Sign Control	Stop	Stop		Fr	ee F	ree	Free	Free	
RT Channelized	·-	None			- No	ne	-	None	
Storage Length	0	-			- 5	500	-	-	
/eh in Median Storage, #	0	-			0	-	-	0	
Grade, %	0	-			0	-	-	0	
Peak Hour Factor	83	83			78	78	99	99	
Heavy Vehicles, %	0	0			5	0	0	5	
Mvmt Flow	0	24		3	85	0	0	456	
Major/Minor	Minor1			Majo	r1		Major2		ı
Conflicting Flow All	841	385			0	0	385	0	
Stage 1	385	-			_	-	-	-	
Stage 2	456	_			-	-	_	_	
Critical Hdwy	6.4	6.2			_	_	4.1	_	
Critical Hdwy Stg 1	5.4	-			-	-	-	_	
Critical Hdwy Stg 2	5.4	<u>-</u>			_	_	_	_	
Follow-up Hdwy	3.5	3.3			-	-	2.2	_	
Pot Cap-1 Maneuver	338	667			_	_	1185	_	
Stage 1	692	-			_	-	-	_	
Stage 2	643	<u>-</u>			_	_	_	_	
Platoon blocked, %	0.0				_	_		_	
Mov Cap-1 Maneuver	338	667			_	_	1185	_	
Mov Cap-2 Maneuver	338	-			_	_	-	_	
Stage 1	692	<u>-</u>			_	_	_	_	
Stage 2	643	-			_	_	_	_	
Olugo Z	040								
Approach	WB			N	I B		SB		
HCM Control Delay, s	10.6				0		0		
HCM LOS	В				-				
	_								
Minor Lane/Major Mvmt	NBT	NBR WBLn1	SBL	SBT					
Capacity (veh/h)	-	- 667	1185	-					
HCM Lane V/C Ratio	_	- 0.036	-	-					
HCM Control Delay (s)	-	- 10.6	0	-					
HCM Lane LOS	_	- 10.0	A	-					
HCM 95th %tile Q(veh)	<u>-</u>	- 0.1	0	-					

Intersection											
Int Delay, s/veh	5.5										
Movement	EBL	EBT	EBR	,	WBL	WBT	WBR		NBL	NBT	NBR
Vol, veh/h	55	85	1		0	460	21		9	1	0
Conflicting Peds, #/hr	0	0	0		0	0	0		0	0	0
Sign Control	Free	Free	Free		Free	Free	Free		Stop	Stop	Stop
RT Channelized	-	-	None		-	-	None		-	-	None
Storage Length	300	-	100		0	-	100		-	-	_
Veh in Median Storage, #	-	0	-		-	0	-		-	0	
Grade, %	-	0	-		-	0	-		-	0	-
Peak Hour Factor	98	98	98		80	80	80		50	50	50
Heavy Vehicles, %	0	2	0		0	2	0		0	0	0
Mvmt Flow	56	87	1		0	575	26		18	2	0
Major/Minor	Major1			Ma	ajor2				Minor1		
Conflicting Flow All	575	0	0		87	0	0		874	774	87
Stage 1	-	-	-		-	-	-		199	199	-
Stage 2	-	_	-		-	-	-		675	575	-
Critical Hdwy	4.1	-	-		4.1	-	-		7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-		-	-	-		6.1	5.5	-
Critical Hdwy Stg 2	-	-	-		-	-	-		6.1	5.5	_
Follow-up Hdwy	2.2	-	-		2.2	-	-		3.5	4	3.3
Pot Cap-1 Maneuver	1008	-	-		1522	-	-		272	332	977
Stage 1	-	-	-		-	-	-		807	740	-
Stage 2	-	-	-		-	-	-		447	506	
Platoon blocked, %		-	-			-	-				
Mov Cap-1 Maneuver	1008	_	-		1522	-	-		161	314	977
Mov Cap-2 Maneuver	-	-	-		-	-	-		161	314	-
Stage 1	-	-	-		-	-	-		762	699	-
Stage 2	-	-	-		-	-	-		276	506	-
Approach	EB				WB				NB		
HCM Control Delay, s	3.4				0				29.1		
HCM LOS									D		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT		WBL	WBT	WBR	SBLn1			
Capacity (veh/h)	169	1008	-	- '	1522	-	-	477			
HCM Lane V/C Ratio	0.118	0.056	-	-	-	-	-	0.478			
HCM Control Delay (s)	29.1	8.8	-	-	0	-	-	19.3			
HCM Lane LOS	D	Α	-	-	Α	-	-	С			
HCM 95th %tile Q(veh)	0.4	0.2	-	-	0	-	-	2.5			

Intersection			
Int Delay, s/veh			
Movement	SBL	SBT	SBR
Vol, veh/h	20	1	141
Conflicting Peds, #/hr	0	0	0
Sign Control	Stop	Stop	Stop
RT Channelized	310p -	Stop -	None
Storage Length	<u>-</u>	-	-
Veh in Median Storage, #	- -	0	<u>-</u>
Grade, %	_	0	
Peak Hour Factor	71	71	- 71
	0	0	0
Heavy Vehicles, %	28	-	199
Mvmt Flow	28	1	199
Major/Minor	Minor2		
Conflicting Flow All	775	774	575
Stage 1	575	575	-
Stage 2	200	199	-
Critical Hdwy	7.1	6.5	6.2
Critical Hdwy Stg 1	6.1	5.5	-
Critical Hdwy Stg 2	6.1	5.5	-
Follow-up Hdwy	3.5	4	3.3
Pot Cap-1 Maneuver	318	332	521
Stage 1	507	506	-
Stage 2	806	740	_
Platoon blocked, %			
Mov Cap-1 Maneuver	303	314	521
Mov Cap-2 Maneuver	303	314	-
Stage 1	479	506	_
Stage 2	759	699	_
Jugo L		- 000	
Approach	SB		
HCM Control Delay, s	19.3		
HCM LOS	С		
Minor Lane/Major Mvmt			
willor Lane/iviajor ivivint			

Intersection											
Int Delay, s/veh	15.1										
Movement	EBL	EBT	EBR		WBL	WBT	WBR		NBL	NBT	NBR
Vol, veh/h	13	34	141		61	103	15		134	153	20
Conflicting Peds, #/hr	0	0	0		0	0	0		0	0	0
Sign Control	Stop	Stop	Stop		Stop	Stop	Stop		Free	Free	Free
RT Channelized	·-	·-	Yeild		-	·-	Yeild		-	-	None
Storage Length	-	-	200		-	-	200		100	-	100
Veh in Median Storage, #	-	0	-		-	0	-		-	0	-
Grade, %	-	0	-		-	0	-		-	0	-
Peak Hour Factor	100	100	100		76	76	76		95	95	95
Heavy Vehicles, %	2	2	2		2	2	2		2	5	2
Mvmt Flow	13	34	141		80	136	20		141	161	21
Major/Minor	Minor2				Minor1				Major1		
Conflicting Flow All	807	739	278		756	739	161		278	0	0
Stage 1	296	296	-		443	443	-		-	-	-
Stage 2	511	443	_		313	296	-		-	-	_
Critical Hdwy	7.12	6.52	6.22		7.12	6.52	6.22		4.12	_	_
Critical Hdwy Stg 1	6.12	5.52	-		6.12	5.52	-		-	-	-
Critical Hdwy Stg 2	6.12	5.52	-		6.12	5.52	-		-	-	-
Follow-up Hdwy	3.518	4.018	3.318		3.518	4.018	3.318		2.218	=	_
Pot Cap-1 Maneuver	300	345	761		325	345	884		1285	-	_
Stage 1	712	668	-		594	576	-		-	-	-
Stage 2	545	576	-		698	668	-		-	-	-
Platoon blocked, %										-	-
Mov Cap-1 Maneuver	176	305	761		221	305	884		1285	-	-
Mov Cap-2 Maneuver	176	305	-		221	305	-		-	-	-
Stage 1	634	664	-		529	513	-		-	-	-
Stage 2	349	513	-		536	664	-		-	=	-
Approach	EB				WB				NB		
HCM Control Delay, s	13.7				53.5				3.6		
HCM LOS	В				F				0.0		
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	FRI n2	WBLn1	WBLn2	SBL	SBT	SBR	
Capacity (veh/h)	1285	-	-	254	761	267	884	1418	- 001	-	
HCM Lane V/C Ratio	0.11	<u>-</u>	-	0.185	0.185	0.808	0.022	0.006	<u>-</u>	<u>-</u>	
HCM Control Delay (s)	8.1	-	-	22.4	10.8	57.6	9.2	7.6	-	-	
HCM Lane LOS	A	_	_	22.4 C	В	57.0	9.Z A	7.0 A	_		
HCM 95th %tile Q(veh)	0.4	_	-	0.7	0.7	6.3	0.1	0	-	_	
HOW JOHN JOHNE Q(VEII)	0.4	-	-	0.7	0.1	0.5	0.1	U	-	-	

Intersection			
Int Delay, s/veh			
Movement	SBL	SBT	SBR
Vol, veh/h	9	278	55
•	0	2/8	55 0
Conflicting Peds, #/hr			
Sign Control	Free	Free	Free
RT Channelized	-	-	None
Storage Length	100	-	100
Veh in Median Storage, #	-	0	-
Grade, %	-	0	-
Peak Hour Factor	100	100	100
Heavy Vehicles, %	2	5	2
Mvmt Flow	9	278	55
Major/Minor	Major2		
Conflicting Flow All	161	0	0
Stage 1	-	-	-
Stage 2	-	-	-
Critical Hdwy	4.12	-	-
Critical Hdwy Stg 1	-	-	-
Critical Hdwy Stg 2	-	-	-
Follow-up Hdwy	2.218	_	_
Pot Cap-1 Maneuver	1418	_	_
Stage 1	-	_	_
Stage 2	<u>-</u>	_	_
Platoon blocked, %	<u>-</u>		_
	1418	-	-
Mov Cap-1 Maneuver		-	
Mov Cap-2 Maneuver	-	-	-
Stage 1	-	-	-
Stage 2	-	-	-
Approach	SB		
HCM Control Delay, s	0.2		
HCM LOS			
Minor Lane/Major Mvmt			

Intersection							
nt Delay, s/veh	4.9						
Movement	WBL	WBR		NBT	NBR	SBL	SBT
Vol, veh/h	86	9		15	35	0	34
Conflicting Peds, #/hr	0	0		0	0	0	0
Sign Control	Stop	Stop		Free	Free	Free	Free
RT Channelized	·-	None		-	None	-	None
Storage Length	0	-		-	-	-	-
/eh in Median Storage, #	0	-		0	-	-	0
Grade, %	0	-		0	-	-	0
Peak Hour Factor	92	92		92	92	92	92
Heavy Vehicles, %	1	1		1	1	1	1
Mvmt Flow	93	10		16	38	0	37
Major/Minor	Minor1			Major1		Major2	
Conflicting Flow All	72	35		0	0	54	0
Stage 1	35	-		<u>-</u>	-	<u>-</u>	-
Stage 2	37	-		-	-	-	-
Critical Hdwy	6.41	6.21		-	-	4.11	-
Critical Hdwy Stg 1	5.41	-		-	=	-	=
Critical Hdwy Stg 2	5.41	-		-	-	-	-
Follow-up Hdwy	3.509	3.309		-	-	2.209	-
ot Cap-1 Maneuver	935	1041		-	-	1558	-
Stage 1	990	-		-	-	-	-
Stage 2	988	-		-	-	-	-
Platoon blocked, %				-	-		-
Mov Cap-1 Maneuver	935	1041		-	-	1558	-
Mov Cap-2 Maneuver	935	-		-	-	-	-
Stage 1	990	-		-	-	-	-
Stage 2	988	-		-	-	-	-
Approach	WB			NB		SB	
HCM Control Delay, s	9.3			0		0	
HCM LOS	A						
Minor Lane/Major Mvmt	NBT	NBR WBLn1	SBL	SBT			
Capacity (veh/h)	-	- 944	1558	-			
HCM Lane V/C Ratio	-	- 0.109	-	-			
HCM Control Delay (s)	-	- 9.3	0	-			
HCM Lane LOS	-	- A	A	=			
HCM 95th %tile Q(veh)	-	- 0.4	0	-			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	†	7	7	†	7	ሻ	^	7	ሻ	₽	
Volume (vph)	20	128	30	51	74	65	85	405	146	139	225	
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2		2	6		
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	
Switch Phase												
Minimum Initial (s)	8.0	8.0	8.0	8.0	8.0	8.0	25.0	25.0	25.0	25.0	25.0	
Minimum Split (s)	15.0	15.0	15.0	15.0	15.0	15.0	36.0	36.0	36.0	36.0	36.0	
Total Split (s)	25.0	25.0	25.0	25.0	25.0	25.0	45.0	45.0	45.0	45.0	45.0	
Total Split (%)	35.7%	35.7%	35.7%	35.7%	35.7%	35.7%	64.3%	64.3%	64.3%	64.3%	64.3%	
Yellow Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	5.0	5.0	5.0	5.0	5.0	
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	
Total Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	Min	Min	Min	Min	Min	
Act Effct Green (s)	12.0	12.0	12.0	12.0	12.0	12.0	31.0	31.0	31.0	31.0	31.0	
Actuated g/C Ratio	0.25	0.25	0.25	0.25	0.25	0.25	0.66	0.66	0.66	0.66	0.66	
v/c Ratio	0.07	0.30	0.08	0.18	0.18	0.16	0.14	0.39	0.16	0.30	0.24	
Control Delay	13.4	15.9	4.4	15.0	14.4	5.2	5.9	7.0	1.6	7.8	5.7	
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total Delay	13.4	15.9	4.4	15.0	14.4	5.2	5.9	7.0	1.6	7.8	5.7	
LOS	В	В	Α	В	В	Α	Α	Α	Α	Α	Α	
Approach Delay		13.7			11.4			5.6			6.4	
Approach LOS		В			В			Α			Α	

Cycle Length: 70

Actuated Cycle Length: 47.3

Natural Cycle: 55

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.39 Intersection Signal Delay: 7.6 Intersection Capacity Utilization 68.9%

Intersection LOS: A ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 1: SH 83 & Hodgen Rd



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		ર્ન	7	ሻ	†	7	*	†	7
Volume (vph)	53	68	120	21	61	6	170	288	47	7	194	40
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0	9.0
Total Split (s)	20.0	20.0	20.0	20.0	20.0	20.0	50.0	50.0	50.0	50.0	50.0	50.0
Total Split (%)	28.6%	28.6%	28.6%	28.6%	28.6%	28.6%	71.4%	71.4%	71.4%	71.4%	71.4%	71.4%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)		4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Max	C-Max	C-Max	C-Max	C-Max	C-Max
Act Effct Green (s)		13.0	13.0		13.0	13.0	49.0	49.0	49.0	49.0	49.0	49.0
Actuated g/C Ratio		0.19	0.19		0.19	0.19	0.70	0.70	0.70	0.70	0.70	0.70
v/c Ratio		0.58	0.38		0.26	0.02	0.23	0.24	0.05	0.01	0.17	0.04
Control Delay		33.8	7.1		25.5	0.2	5.2	4.9	1.5	4.1	4.5	1.6
Queue Delay		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay		33.8	7.1		25.5	0.2	5.2	4.9	1.5	4.1	4.5	1.6
LOS		С	Α		С	Α	Α	Α	Α	Α	Α	Α
Approach Delay		20.5			23.7			4.6			4.0	
Approach LOS		С			С			Α			Α	

Cycle Length: 70

Actuated Cycle Length: 70

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 40

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.58

Intersection Signal Delay: 10.1 Intersection LOS: B
Intersection Capacity Utilization 42.8% ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 5: SH 83 & Walker Road



Intersection							
nt Delay, s/veh	0.2						
Movement	WBL	WBR		NBT	NBR	SBL	SBT
Vol, veh/h	0	13		490	2	0	389
Conflicting Peds, #/hr	0	0		0	0	0	0
Sign Control	Stop	Stop		Free	Free	Free	Free
RT Channelized	-	None		-	None	-	None
Storage Length	0	-		-	-	-	-
Veh in Median Storage, #	0	-		0	-	-	0
Grade, %	0	-		0	-	-	0
Peak Hour Factor	83	83		94	94	97	97
Heavy Vehicles, %	0	0		5	0	0	5
Mvmt Flow	0	16		521	2	0	401
Major/Minor	Minor1			Major1		Major2	
Conflicting Flow All	923	522		0	0	523	0
Stage 1	522	<u>-</u>		- -	-	-	-
Stage 2	401	-		-	-	-	-
Critical Hdwy	6.4	6.2		-	-	4.1	-
Critical Hdwy Stg 1	5.4	-		-	=	-	=
Critical Hdwy Stg 2	5.4	-		-	-	-	-
Follow-up Hdwy	3.5	3.3		-	-	2.2	-
Pot Cap-1 Maneuver	302	559		-	-	1054	-
Stage 1	599	=		=	-	=	-
Stage 2	681	-		-	-	-	-
Platoon blocked, %				-	-		-
Mov Cap-1 Maneuver	302	559		-	-	1054	-
Mov Cap-2 Maneuver	302	-		-	-	-	-
Stage 1	599	-		-	-	-	-
Stage 2	681	-		-	-	-	-
Approach	WB			NB		SB	
HCM Control Delay, s	11.6			0		0	
HCM LOS	В						
Minor Lane/Major Mvmt	NBT	NBR WBLn1	SBL	SBT			
Capacity (veh/h)	-	- 559	1054	-			
HCM Lane V/C Ratio	-	- 0.028	-	-			
HCM Control Delay (s)	-	- 11.6	0	-			
HCM Lane LOS	-	- B	Α	-			
HCM 95th %tile Q(veh)	-	- 0.1	0	-			

Intersection											
Int Delay, s/veh	4										
, , , , , , , , , , , , , , , , , , ,											
Movement	EBL	EBT	EBR		WBL	WBT	WBR		NBL	NBT	NBR
Vol, veh/h	174	300	13		1	180	22		6	1	1
Conflicting Peds, #/hr	0	0	0		0	0	0		0	0	0
Sign Control	Free	Free	Free		Free	Free	Free		Stop	Stop	Stop
RT Channelized	-	-	None		-	-	None		-	-	None
Storage Length	300	-	100		0	-	100		-	-	-
Veh in Median Storage, #	-	0	-		-	0	-		-	0	-
Grade, %	-	0	-		-	0	-		-	0	_
Peak Hour Factor	86	86	86		81	81	81		50	50	50
Heavy Vehicles, %	0	2	0		0	2	0		0	0	0
Mvmt Flow	202	349	15		1	222	27		12	2	2
Major/Minor	Major1			М	ajor2				Minor1		
Conflicting Flow All	222	0	0		349	0	0		1042	978	349
Stage 1	-	-	-		-	-	-		753	753	_
Stage 2	-	-	-		-	-	-		289	225	_
Critical Hdwy	4.1	-	-		4.1	-	-		7.1	6.5	6.2
Critical Hdwy Stg 1	-	-	-		-	-	=		6.1	5.5	_
Critical Hdwy Stg 2	-	-	-		-	-	-		6.1	5.5	-
Follow-up Hdwy	2.2	-	-		2.2	-	-		3.5	4	3.3
Pot Cap-1 Maneuver	1359	-	-		1221	-	-		210	252	699
Stage 1	-	-	-		-	-	-		405	420	-
Stage 2	-	-	-		-	-	-		723	721	_
Platoon blocked, %		-	-			-	-				
Mov Cap-1 Maneuver	1359	-	-		1221	-	-		157	214	699
Mov Cap-2 Maneuver	-	-	-		-	-	-		157	214	-
Stage 1	-	-	-		-	-	-		345	358	-
Stage 2	-	-	-		-	-	-		610	720	-
Approach	EB				WB				NB		
HCM Control Delay, s	2.9				0				26.8		
HCM LOS									D		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1			
Capacity (veh/h)	181	1359	-	-	1221	-	=	595			
HCM Lane V/C Ratio	0.088	0.149	-		0.001	-	-	0.244			
HCM Control Delay (s)	26.8	8.1	-	-	8	-	-	13			
HCM Lane LOS	D	Α	-	-	Α	-	-	В			
HCM 95th %tile Q(veh)	0.3	0.5	-	-	0	-	-	1			

Intersection			
Int Delay, s/veh			
in boldy, 3/ven			
Movement	SBL	SBT	SBR
Vol, veh/h	12	1	90
Conflicting Peds, #/hr	0	0	0
Sign Control	Stop	Stop	Stop
RT Channelized	-	-	None
Storage Length	-	-	-
Veh in Median Storage, #	-	0	-
Grade, %	-	0	-
Peak Hour Factor	71	71	71
Heavy Vehicles, %	0	0	0
Mvmt Flow	17	1	127
Major/Minor	Minor2		
		070	000
Conflicting Flow All	980	978	222
Stage 1	225	225	-
Stage 2	755	753	-
Critical Hdwy	7.1	6.5	6.2
Critical Hdwy Stg 1	6.1	5.5	-
Critical Hdwy Stg 2	6.1	5.5	-
Follow-up Hdwy	3.5	4	3.3
Pot Cap-1 Maneuver	231	252	823
Stage 1	782	721	-
Stage 2	404	420	-
Platoon blocked, %			
Mov Cap-1 Maneuver	203	214	823
Mov Cap-2 Maneuver	203	214	-
Stage 1	666	720	-
Stage 2	341	358	-
- V			
Approach	SB		
HCM Control Delay, s	13		
HCM LOS	В		
Minor Lane/Major Mvmt			
or Lano, major wwint			

Intersection											
Int Delay, s/veh	14.3										
Movement	EBL	EBT	EBR		WBL	WBT	WBR		NBL	NBT	NBR
Vol, veh/h	53	68	120		21	61	6		170	288	47
Conflicting Peds, #/hr	0	0	0		0	0	0		0	0	0
Sign Control	Stop	Stop	Stop		Stop	Stop	Stop		Free	Free	Free
RT Channelized	· -	-	Yeild		· -	-	Yeild		-	-	None
Storage Length	-	-	200		-	-	200		100	-	100
Veh in Median Storage, #	-	0	-		-	0	-		-	0	-
Grade, %	-	0	-		-	0	-		-	0	-
Peak Hour Factor	74	74	74		100	100	100		93	93	93
Heavy Vehicles, %	2	2	2		2	2	2		2	5	2
Mvmt Flow	72	92	162		21	61	6		183	310	51
Major/Minor	Minor2				Minor1				Major1		
Conflicting Flow All	937	906	216		952	906	310		216	0	0
Stage 1	231	231	-		675	675	-		-	-	-
Stage 2	706	675	-		277	231	-		-	-	-
Critical Hdwy	7.12	6.52	6.22		7.12	6.52	6.22		4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-		6.12	5.52	-		-	-	-
Critical Hdwy Stg 2	6.12	5.52	-		6.12	5.52	-		-	-	-
Follow-up Hdwy	3.518	4.018	3.318		3.518	4.018	3.318		2.218	-	-
Pot Cap-1 Maneuver	245	276	824		239	276	730		1354	-	-
Stage 1	772	713	-		444	453	-		-	-	-
Stage 2	427	453	-		729	713	-		-	-	-
Platoon blocked, %										-	-
Mov Cap-1 Maneuver	175	237	824		121	237	730		1354	-	-
Mov Cap-2 Maneuver	175	237	-		121	237	-		-	-	-
Stage 1	668	708	-		384	392	-		-	-	-
Stage 2	309	392	-		506	708	-		-	-	-
Approach	EB				WB				NB		
HCM Control Delay, s	39.6				35.7				2.7		
HCM LOS	Е				Е						
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2	WBLn1	WBLn2	SBL	SBT	SBR	
Capacity (veh/h)	1354	-	-	205	824	190	730	1250	-	_	
HCM Lane V/C Ratio	0.135	-	_	0.798	0.197	0.432	0.008	0.006	_	-	
HCM Control Delay (s)	8.1	_	-	68.6	10.4	37.6	10	7.9	_	-	
HCM Lane LOS	A	-	-	F	В	E	В	Α	-	-	
HCM 95th %tile Q(veh)	0.5	-	-	5.7	0.7	2	0	0	-	-	

nt Delay, s/veh	2.1							
, ,								
Movement	WBL	WBR		NE	BT NI	BR SBL	SBT	
/ol, veh/h	53	5			66	99 0	41	
Conflicting Peds, #/hr	0	0			0	0 0	0	
Sign Control	Stop	Stop		Fr	ee Fr	ree Free	Free	
RT Channelized	·-	None			- No	ne -	None	
Storage Length	0	-			-		-	
/eh in Median Storage, #	0	-			0		0	
Grade, %	0	-			0		0	
Peak Hour Factor	92	92			92	92 92		
Heavy Vehicles, %	1	1			1	1 1		
Nymt Flow	58	5		•	72 1	08 0	45	
//ajor/Minor	Minor1			Majo	r1	Major2		
Conflicting Flow All	171	126			0	0 179		
Stage 1	126	-			-		_	
Stage 2	45	_			-		_	
Critical Hdwy	6.41	6.21			_	- 4.11	_	
Critical Hdwy Stg 1	5.41	-			-	-		
Critical Hdwy Stg 2	5.41	<u>-</u>			_		_	
Follow-up Hdwy	3.509	3.309			-	- 2.209	_	
Pot Cap-1 Maneuver	821	927			_	- 1403		
Stage 1	902	-			_			
Stage 2	980	<u>-</u>			_		_	
Platoon blocked, %					-	-	_	
Mov Cap-1 Maneuver	821	927			_	- 1403	_	
Mov Cap-2 Maneuver	821	-			_			
Stage 1	902	_			_		_	
Stage 2	980	-			_	_	_	
Olugo Z	000							
Approach	WB			N	I B	SB		
ICM Control Delay, s	9.7				0	0		
HCM LOS	A					,		
10.111 200								
Minor Lane/Major Mvmt	NBT	NBR WBLn1	SBL	SBT				
Capacity (veh/h)	-	- 829	1403	-				
ICM Lane V/C Ratio	-	- 0.076	-	-				
HCM Control Delay (s)	-	- 9.7	0	-				
ICM Lane LOS	_	- A	A	-				
ICM 95th %tile Q(veh)	<u>-</u>	- 0.2	0	-				

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	†	7	7	†	7	ሻ	^	7	7	44	7
Volume (vph)	123	97	203	353	309	140	133	418	86	152	651	218
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	4.0	8.0	8.0	4.0	8.0	8.0	25.0	25.0	25.0	25.0	25.0	25.0
Minimum Split (s)	9.0	15.0	15.0	9.0	15.0	15.0	36.0	36.0	36.0	36.0	36.0	36.0
Total Split (s)	10.0	15.0	15.0	20.0	25.0	25.0	55.0	55.0	55.0	55.0	55.0	55.0
Total Split (%)	11.1%	16.7%	16.7%	22.2%	27.8%	27.8%	61.1%	61.1%	61.1%	61.1%	61.1%	61.1%
Yellow Time (s)	3.0	4.0	4.0	3.0	4.0	4.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	0.0
Total Lost Time (s)	3.0	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	6.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	None	None	None	None	None	Min	Min	Min	Min	Min	Min
Act Effct Green (s)	18.8	10.7	10.7	30.5	21.6	21.6	28.4	28.4	28.4	28.4	28.4	26.4
Actuated g/C Ratio	0.29	0.16	0.16	0.46	0.33	0.33	0.43	0.43	0.43	0.43	0.43	0.40
v/c Ratio	0.34	0.34	0.49	0.57	0.53	0.24	0.56	0.29	0.12	0.43	0.45	0.30
Control Delay	14.8	29.2	8.8	16.4	23.4	5.0	24.8	13.0	8.0	17.8	14.6	3.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	14.8	29.2	8.8	16.4	23.4	5.0	24.8	13.0	0.8	17.8	14.6	3.2
LOS	В	С	Α	В	С	Α	С	В	Α	В	В	Α
Approach Delay		15.2			17.1			13.8			12.6	
Approach LOS		В			В			В			В	

Cycle Length: 90

Actuated Cycle Length: 65.9

Natural Cycle: 60

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.57 Intersection Signal Delay: 14.5 Intersection Capacity Utilization 81.2%

Intersection LOS: B ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: SH 83 & Hodgen Rd



1: SH 83 & Hodgen Rd Synchro 8 Report KDF

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		ર્ન	7	ሻ	44	7	7	44	7
Volume (vph)	27	103	281	176	200	48	263	309	127	46	565	111
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0
Total Split (s)	25.0	25.0	25.0	25.0	25.0	25.0	45.0	45.0	45.0	45.0	45.0	45.0
Total Split (%)	35.7%	35.7%	35.7%	35.7%	35.7%	35.7%	64.3%	64.3%	64.3%	64.3%	64.3%	64.3%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)		4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Min	C-Min	C-Min	C-Min	C-Min	C-Min
Act Effct Green (s)		23.6	23.6		23.6	23.6	38.4	38.4	38.4	38.4	38.4	38.4
Actuated g/C Ratio		0.34	0.34		0.34	0.34	0.55	0.55	0.55	0.55	0.55	0.55
v/c Ratio		0.27	0.42		0.83	0.09	0.68	0.17	0.14	0.08	0.32	0.13
Control Delay		19.0	5.1		39.6	6.3	21.7	8.1	1.8	7.8	9.2	1.8
Queue Delay		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay		19.0	5.1		39.6	6.3	21.7	8.1	1.8	7.8	9.2	1.8
LOS		В	Α		D	Α	С	Α	Α	Α	A	Α
Approach Delay		9.5			35.8			12.1			7.9	
Approach LOS		Α			D			В			Α	

Cycle Length: 70

Actuated Cycle Length: 70

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 50

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 14.8 Intersection LOS: B Intersection Capacity Utilization 69.0% ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 5: SH 83 & Walker Road



Synchro 8 Report 5: SH 83 & Walker Road KDF

Silli Tallic Pellollia	ance ne	ρυπ									AWI Gak Hou
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Inte	rval #1	7:00		
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	Т	R	L	T	R	LTR	L	TR		
Stop Del/Veh (s)	4.2	0.3	0.0	0.0	0.0	0.0	8.9	10.1	9.3	1.7	
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Inte	rval #2	7:15		
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	T	R	L	T	R	LTR	L	TR		
Stop Del/Veh (s)	4.7	0.4	0.0	0.0	0.0	0.0	13.7	24.3	9.8	2.2	
3: Timber Meadow	Drive &	Hodge EB	en Rd EB	Perfor	mance WB	by lar	ne Inte	rval #3	3 7:30 SB	All	
Movements Served	I	<u></u>	R	L	T	R	LTR	L	TR	7 41	
Stop Del/Veh (s)	4.6	0.4	0.0	1.4	0.0	0.0	9.3	12.5	8.7	1.8	
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Inte	rval #4	7:45		
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	T	R	L	Т	R	LTR	L	TR		
Stop Del/Veh (s)	3.6	0.4	0.0	0.0	0.0	0.0	12.3	11.6	8.2	1.6	
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Enti	re Rur)		
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	Т	R	L	T	R	LTR	L	TR		
Stop Del/Veh (s)	4.4	0.4	0.0	0.5	0.0	0.0	11.4	15.7	9.6	1.9	

2040 Background Traffic SimTraffic Report AM Peak Hour KDF

Intersection							
	0.1						
ini Delay, S/Ven	0.1						
Movement	WBL	WBR		NBT	NBR	SBL	SBT
Vol, veh/h	0	18		681	0	0	1022
Conflicting Peds, #/hr	0	0		0	0	0	0
Sign Control	Stop	Stop		Free	Free	Free	Free
RT Channelized	-	None		-	None	-	None
Storage Length	0	-		-	500	-	-
Veh in Median Storage, #	0	-		0	-	-	0
Grade, %	0	-		0	-	-	0
Peak Hour Factor	83	83		95	78	99	95
Heavy Vehicles, %	0	0		5	0	0	5
Mvmt Flow	0	22		717	0	0	1076
Major/Minor	Minor1			Major1		Major2	
Conflicting Flow All	1255	358		0	0	717	0
Stage 1	717	-		-	-	-	-
Stage 2	538	_		_	_	_	_
Critical Hdwy	6.8	6.9		_	_	4.1	_
Critical Hdwy Stg 1	5.8	-		-	_	-	_
Critical Hdwy Stg 2	5.8	-		-	_	<u>-</u>	_
Follow-up Hdwy	3.5	3.3		_	_	2.2	-
Pot Cap-1 Maneuver	166	644		_	_	893	_
Stage 1	450	-		_	_	-	-
Stage 2	555	-		-	_	_	_
Platoon blocked, %				-	_		-
Mov Cap-1 Maneuver	166	644		-	_	893	_
Mov Cap-2 Maneuver	166	-		_	_	-	-
Stage 1	450	-		-	_	<u>-</u>	_
Stage 2	555	-		_	_	_	-
Olago Z							
Annroach	WB			NB		SB	
Approach	10.8			0		0	
HCM LOS	10.8 B			U		U	
HCM LOS	В						
			05:	0.5.			
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT			
Capacity (veh/h)	-	- 644	893	-			
HCM Lane V/C Ratio	-	- 0.034	-	-			
HCM Control Delay (s)	-	- 10.8	0	-			
HCM Lane LOS	-	- B	Α	-			
HCM 95th %tile Q(veh)	-	- 0.1	0	-			

Intersection							
	4.7						
in Bolay, or von	1.1						
Movement	WBL	WBR		NBT	NBR	SBL	SBT
Vol, veh/h	77	16		25	30	8	41
Conflicting Peds, #/hr	0	0		0	0	0	0
Sign Control	Stop	Stop		Free	Free	Free	Free
RT Channelized	-	None		-	None	-	None
Storage Length	0	-		-	-	-	-
Veh in Median Storage, #	0	-		0	-	-	0
Grade, %	0	-		0	-	-	0
Peak Hour Factor	92	92		92	92	92	92
Heavy Vehicles, %	1	1		1	1	1	1
Mvmt Flow	84	17		27	33	9	45
Major/Minor	Minor1			Major1		Major2	
Conflicting Flow All	105	43		0	0	60	0
Stage 1	43	-		-	-	-	-
Stage 2	62	-		-	-	-	_
Critical Hdwy	6.41	6.21		-	-	4.11	_
Critical Hdwy Stg 1	5.41	-		-	-	-	-
Critical Hdwy Stg 2	5.41	-		-	-	-	-
Follow-up Hdwy	3.509	3.309		-	-	2.209	-
Pot Cap-1 Maneuver	895	1030		-	-	1550	-
Stage 1	982	-		-	-	-	-
Stage 2	963	-		-	-	-	-
Platoon blocked, %				-	-		-
Mov Cap-1 Maneuver	890	1030		-	-	1550	-
Mov Cap-2 Maneuver	890	-		-	-	-	-
Stage 1	982	-		-	-	-	-
Stage 2	957	-		-	-	-	-
Approach	WB			NB		SB	
HCM Control Delay, s	9.4			0		1.2	
HCM LOS	A					1,2	
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT			
Capacity (veh/h)	-		1550	-			
HCM Lane V/C Ratio	_	- 0.111		-			
HCM Control Delay (s)	_	- 9.4	7.3	0			
HCM Lane LOS	_	- A	A	Å			
HCM 95th %tile Q(veh)	-	- 0.4	0	-			

Vol. veh/h	Intersection												
Vol, veh/h 1 101 173 93 283 0 139 0 76 0 0 Conflicting Peds, #/hr 0 - None - - 0 - - 0 - - 0 0 - - 0 0	Int Delay, s/veh	6.5											
Vol, veh/h 1 101 173 93 283 0 139 0 76 0 0 Conflicting Peds, #/hr 0 - None - - 0 - - 0 - - 0 0 - - 0 0													
Conflicting Peds, #/hr	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Sign Control Free RTC Free None Free None Free None Free None Free None Stop None Stop None Stop None Stop None Stop None Non	Vol, veh/h	1	101	173	93	283	0	139	0	76	0	0	3
RT Channelized	Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Storage Length	Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
Veh in Median Storage, # - 0 - - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 0 - 0 - 0 - 0 - 0 0 - 0 0 2 92 2 <td>RT Channelized</td> <td>-</td> <td>-</td> <td>None</td> <td>-</td> <td>-</td> <td>None</td> <td>-</td> <td>-</td> <td>None</td> <td>-</td> <td>-</td> <td>None</td>	RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Grade, % - 0 - - 0 - - 0 - - 0 - 0 - 0 0 - 0 0 99 92 92 75 75 75 92 92 92 75 75 75 92 92 92 75 75 75 92 92 92 75 75 75 92 92 22 3 55 556 668 668 668 668 868 868 868 868 868 868 868 868 868 868 868 868 <	Storage Length	0	-	250	250	-	-	-	-	0	-	-	-
Peak Hour Factor 92 92 75 75 92 92 75 75 75 75 92 92 Heavy Vehicles, % 2 3 0 0 110 0 0 669 668 110 668 668 868 868 8110 668 668 868 810 2 2 2 2 2 2 2 2 2 2 2 2	Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2	Grade, %	-		-	-	0	-	-	0	-	-		-
Mejor/Minor Major1 Major2 Minor1 Minor2 Conflicting Flow All 308 0 110 0 0 669 668 110 668 668 Stage 1 - - - - - 112 112 - 556 556 Stage 2 - - - - - 557 556 - 112 112 - 556 552 - 612 552 - 612 552 - 612 552 - 612 552 - 612 552 - 612 552 - 612 552 - 612 552 - 612 <	Peak Hour Factor		92	75	75		92	75			92	92	92
Major/Minor Major1	Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Conflicting Flow All 308	Mvmt Flow	1	110	231	124	308	0	185	0	101	0	0	3
Conflicting Flow All 308													
Conflicting Flow All 308	Major/Minor	Major1			Major2			Minor1			Minor2		
Stage 1 - - - - - 112 112 - 556 556 Stage 2 - - - - 557 556 - 112			0	0		0	0	669	668	110	668	668	308
Stage 2	•	-	_	-	-	-	-	112		-	556		_
Critical Hdwy 4.12 - 4.12 - 7.12 6.52 6.22 7.12 6.52 6 Critical Hdwy Stg 1 - - - - - 6.12 5.52 - 6.12 5.52 Critical Hdwy Stg 2 - - - - - 6.12 5.52 - 6.12 5.52 Follow-up Hdwy 2.218 - - 2.218 - 3.518 4.018 3.318 3.518 4.018 3. Pot Cap-1 Maneuver 1253 - 1480 - 371 379 943 372 379 Stage 1 - - - - 893 803 - 515 513 893 803 Platoon blocked, % - - - - - 345 347 943 311 347 Mov Cap-1 Maneuver 1253 - 1480 - 345 347 943 311 347 Stage 1 - - - - - 892		-	_	-	-	-	-	557	556	-	112	112	-
Critical Hdwy Stg 2 - - - - 6.12 5.52 - 6.12 5.52 Follow-up Hdwy 2.218 - - 2.218 - 3.518 4.018 3.318 3.518 4.018 3. Pot Cap-1 Maneuver 1253 - 1480 - 371 379 943 372 379 Stage 1 - - - - 893 803 - 515 513 513 515 513 Stage 2 - - - - 515 513 - 893 803 - 515 513 Stage 2 -	J	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Follow-up Hdwy 2.218 2.218 3.518 4.018 3.318 3.518 4.018 3. Pot Cap-1 Maneuver 1253 1480 371 379 943 372 379 Stage 1 893 803 - 515 513 Stage 2 515 513 - 893 803 Platoon blocked, % Mov Cap-1 Maneuver 1253 1480 345 347 943 311 347 Mov Cap-2 Maneuver 345 347 943 311 347 Stage 1 345 347 - 311 347 Stage 1 892 802 - 515 470 Stage 2 470 470 - 796 802 Approach EB WB NB SB HCM Control Delay, s 0 2.2 20.7 9.9 HCM LOS C A Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 345 943 1253 - 1480 - 732 HCM Lane V/C Ratio 0.537 0.107 0.001 - 0.0084 - 0.004 HCM Control Delay (s) 26.9 9.3 7.9 - 7.7 - 9.9	Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Pot Cap-1 Maneuver 1253 - 1480 - 371 379 943 372 379 Stage 1 - - - - - 893 803 - 515 513 Stage 2 - - - - - 515 513 - 893 803 Platoon blocked, % -	Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Stage 1 - - - - - 893 803 - 515 513 Stage 2 - - - - - 515 513 - 893 803 Platoon blocked, % -<	Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Stage 2 - - - - 515 513 - 893 803 Platoon blocked, % - 345 347 943 311 347 Mov Cap-2 Maneuver - - - - - - 345 347 - 311 347 Stage 1 - - - - - - 892 802 - 515 470 Stage 2 - - - - - - 470 470 - 796 802 Approach EB WB WB NB SB BB HCM Control Delay, s 0 2.2 20.7 9.9 9.9 HCM Los C A A A A A A A A A A <	Pot Cap-1 Maneuver	1253	-	-	1480	-	-	371	379	943	372	379	732
Platoon blocked, %	Stage 1	-	-	-	-	-	-	893	803	-	515	513	-
Mov Cap-1 Maneuver 1253 - 1480 - 345 347 943 311 347 Mov Cap-2 Maneuver - - - - - - 345 347 - 311 347 Stage 1 - - - - - - 892 802 - 515 470 Stage 2 - - - - - 470 470 - 796 802 Approach EB WB NB	Stage 2	-	-	-	-	-	-	515	513	-	893	803	-
Mov Cap-2 Maneuver - - - - 345 347 - 311 347 Stage 1 - - - - - 892 802 - 515 470 Stage 2 - - - - - 470 470 - 796 802 Approach EB WB NB NB SB HCM Control Delay, s 0 2.2 20.7 9.9 HCM Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 345 943 1253 - 1480 - - 732 HCM Lane V/C Ratio 0.537 0.107 0.001 - 0.084 - - 0.004 HCM Control Delay (s) 26.9 9.3 7.9 - 7.7 - 9.9	Platoon blocked, %		-	-		-	-						
Stage 1 - - - - - 515 470 Stage 2 - - - - - 470 470 - 796 802 Approach EB WB NB NB SB HCM Control Delay, s 0 2.2 20.7 9.9 HCM LOS C A Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 345 943 1253 - 1480 - 732 HCM Lane V/C Ratio 0.537 0.107 0.001 - 0.084 - 0.004 HCM Control Delay (s) 26.9 9.3 7.9 - 7.7 - 9.9 NB SB NB N	Mov Cap-1 Maneuver	1253	-	-	1480	-	-	345	347	943	311	347	732
Stage 2 - - - - - - 470 470 - 796 802 Approach EB WB NB SB HCM Control Delay, s 0 2.2 20.7 9.9 HCM LOS C A Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 345 943 1253 - - 1480 - - 732 HCM Lane V/C Ratio 0.537 0.107 0.001 - - 0.004 HCM Control Delay (s) 26.9 9.3 7.9 - 7.7 - 9.9	Mov Cap-2 Maneuver	-	-	-	-	-	-	345	347	-	311	347	-
Approach EB WB NB SB HCM Control Delay, s 0 2.2 20.7 9.9 HCM LOS C A Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 345 943 1253 - - 1480 - - 732 HCM Lane V/C Ratio 0.537 0.107 0.001 - - 0.004 HCM Control Delay (s) 26.9 9.3 7.9 - - 7.7 - 9.9	Stage 1	-	-	-	-	-	-	892	802	-	515	470	-
HCM Control Delay, s 0 2.2 20.7 9.9 HCM LOS C A Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 345 943 1253 1480 732 HCM Lane V/C Ratio 0.537 0.107 0.001 0.084 0.004 HCM Control Delay (s) 26.9 9.3 7.9 7.7 - 9.9	Stage 2	-	-	-	-	-	-	470	470	-	796	802	-
HCM Control Delay, s 0 2.2 20.7 9.9 HCM LOS C A Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 345 943 1253 1480 732 HCM Lane V/C Ratio 0.537 0.107 0.001 0.084 0.004 HCM Control Delay (s) 26.9 9.3 7.9 7.7 - 9.9													
HCM Control Delay, s 0 2.2 20.7 9.9 HCM LOS C A	Approach	EB			WB			NB			SB		
Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 345 943 1253 - - 1480 - - 732 HCM Lane V/C Ratio 0.537 0.107 0.001 - - 0.004 HCM Control Delay (s) 26.9 9.3 7.9 - 7.7 - 9.9		0									9.9		
Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 345 943 1253 - - 1480 - - 732 HCM Lane V/C Ratio 0.537 0.107 0.001 - - 0.084 - - 0.004 HCM Control Delay (s) 26.9 9.3 7.9 - - 7.7 - - 9.9													
Capacity (veh/h) 345 943 1253 1480 732 HCM Lane V/C Ratio 0.537 0.107 0.001 0.084 0.004 HCM Control Delay (s) 26.9 9.3 7.9 7.7 - 9.9													
Capacity (veh/h) 345 943 1253 1480 732 HCM Lane V/C Ratio 0.537 0.107 0.001 0.084 0.004 HCM Control Delay (s) 26.9 9.3 7.9 7.7 - 9.9	Minor Lane/Maior Mymt	NBLn11	NBLn2	EBL	EBT EBR	WBL	WBT	WBR SBLn1					
HCM Lane V/C Ratio 0.537 0.107 0.001 0.084 0.004 HCM Control Delay (s) 26.9 9.3 7.9 7.7 9.9													
HCM Control Delay (s) 26.9 9.3 7.9 7.7 9.9													
	HCM Lane LOS	D	Α.	Α.		A	_	- A					
HCM 95th %tile Q(veh) 3 0.4 0 0.3 0													

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	†	7	7	↑	7	ሻ	^	7	ሻ	^	7
Volume (vph)	195	293	304	148	193	170	253	643	292	151	314	223
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	4.0	8.0	8.0	4.0	8.0	8.0	25.0	25.0	25.0	25.0	25.0	25.0
Minimum Split (s)	9.0	15.0	15.0	9.0	15.0	15.0	36.0	36.0	36.0	36.0	36.0	36.0
Total Split (s)	10.0	20.0	20.0	10.0	20.0	20.0	60.0	60.0	60.0	60.0	60.0	60.0
Total Split (%)	11.1%	22.2%	22.2%	11.1%	22.2%	22.2%	66.7%	66.7%	66.7%	66.7%	66.7%	66.7%
Yellow Time (s)	3.0	4.0	4.0	3.0	4.0	4.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	0.0
Total Lost Time (s)	3.0	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	6.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	None	None	None	None	None	Min	Min	Min	Min	Min	Min
Act Effct Green (s)	24.1	16.1	16.1	24.1	16.1	16.1	29.8	29.8	29.8	29.8	29.8	27.8
Actuated g/C Ratio	0.38	0.25	0.25	0.38	0.25	0.25	0.47	0.47	0.47	0.47	0.47	0.43
v/c Ratio	0.46	0.68	0.51	0.44	0.45	0.35	0.58	0.42	0.35	0.58	0.21	0.29
Control Delay	17.4	32.2	6.3	17.4	25.0	6.1	18.0	12.1	2.4	22.1	10.2	2.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	17.4	32.2	6.3	17.4	25.0	6.1	18.0	12.1	2.4	22.1	10.2	2.6
LOS	В	С	Α	В	С	Α	В	В	Α	С	В	Α
Approach Delay		18.6			16.5			11.0			10.4	
Approach LOS		В			В			В			В	

Cycle Length: 90

Actuated Cycle Length: 64

Natural Cycle: 60

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.68 Intersection Signal Delay: 13.6

Intersection LOS: B Intersection Capacity Utilization 78.6% ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: SH 83 & Hodgen Rd



Synchro 8 Report KDF

	•	→	•	•	←	•	4	†	<i>></i>	>	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		4	7	ሻ	^	7	7	^	7
Volume (vph)	108	130	238	60	110	17	334	584	100	21	390	81
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0
Total Split (s)	25.0	25.0	25.0	25.0	25.0	25.0	45.0	45.0	45.0	45.0	45.0	45.0
Total Split (%)	35.7%	35.7%	35.7%	35.7%	35.7%	35.7%	64.3%	64.3%	64.3%	64.3%	64.3%	64.3%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)		4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Min	C-Min	C-Min	C-Min	C-Min	C-Min
Act Effct Green (s)		17.9	17.9		17.9	17.9	44.1	44.1	44.1	44.1	44.1	44.1
Actuated g/C Ratio		0.26	0.26		0.26	0.26	0.63	0.63	0.63	0.63	0.63	0.63
v/c Ratio		0.79	0.43		0.56	0.04	0.60	0.29	0.10	0.05	0.19	0.08
Control Delay		42.5	5.3		28.6	4.7	14.1	6.8	1.8	6.5	6.3	1.9
Queue Delay		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay		42.5	5.3		28.6	4.7	14.1	6.8	1.8	6.5	6.3	1.9
LOS		D	Α		С	Α	В	Α	Α	Α	Α	Α
Approach Delay		23.9			26.4			8.7			5.6	
Approach LOS		С			С			Α			Α	

Cycle Length: 70

Actuated Cycle Length: 70

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.79

Intersection Signal Delay: 12.9 Intersection LOS: B Intersection Capacity Utilization 64.5% ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 5: SH 83 & Walker Road



Synchro 8 Report 5: SH 83 & Walker Road 2040 Background Traffic PM Peak Hour KDF

Simi raffic Performa	ance Re	port									PIVI Peak Hour
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Inte	rval #1	5:00		
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	Τ	R	L	T	R	LTR	L	TR		
Stop Del/Veh (s)	2.3	0.3	0.0		0.0	0.0	13.2	16.0	4.4	1.0	
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Inte	rval #2	2 5:15		
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	Т	R	L	Т	R	LTR	L	TR		_
Stop Del/Veh (s)	2.0	0.3	0.0	1.7	0.0	0.0	11.9	14.0	5.5	1.0	
3: Timber Meadow	Drive &	Hodge EB	en Rd EB	Perfor WB	mance WB	by lar	ne Inte	rval #3	3 5:30 SB	All	
Movements Served	L	<u></u>	R	I	T	R	LTR	L	TR	7 (11	
Stop Del/Veh (s)	2.0	0.3	0.0	1.0	0.0	0.0	9.0	17.2	4.8	1.0	
3: Timber Meadow											
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	T	R	L	T	R	LTR	L	TR		
Stop Del/Veh (s)	1.6	0.3	0.0	1.3	0.0	0.0	11.0	13.9	5.5	1.0	
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Enti	re Rur	1		
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	Т	R	L	Т	R	LTR	L	TR		
Stop Del/Veh (s)	2.0	0.3	0.0	1.0	0.0	0.0	11.2	15.3	5.5	1.1	

2040 Background Traffic SimTraffic Report M Peak Hour KDF

ntersection nt Delay, s/veh	0.1							
nt Delay, S/Ven	0.1							
Movement	WBL	WBR		NB	T NBR	SBL	SBT	
Vol, veh/h	0	12		100	6 2	. 0	688	
Conflicting Peds, #/hr	0	0			0 0	0	0	
Sign Control	Stop	Stop		Fre	e Free	Free	Free	
RT Channelized	· -	None			- None	-	None	
Storage Length	0	-			- 500	-	-	
/eh in Median Storage, #	0	-			0 -	<u>-</u>	0	
Grade, %	0	-			0 -	-	0	
Peak Hour Factor	83	83		9		97	97	
Heavy Vehicles, %	0	0			5 0	0	5	
Mvmt Flow	0	14		107			709	
Major/Minor	Minor1			Major	1	Major2		
Conflicting Flow All	1425	535			0 0		0	
Stage 1	1070	-					_	
Stage 2	355	_				<u>-</u>	_	
Critical Hdwy	6.8	6.9				4.1	_	
Critical Hdwy Stg 1	5.8	-				····	_	
Critical Hdwy Stg 2	5.8	-					_	
Follow-up Hdwy	3.5	3.3				2.2	_	
Pot Cap-1 Maneuver	129	495				659	_	
Stage 1	295	-				-	_	
Stage 2	686	_				-	_	
Platoon blocked, %	000						_	
Mov Cap-1 Maneuver	129	495				659	_	
Mov Cap-2 Maneuver	129	-				_	_	
Stage 1	295	<u>-</u>					_	
Stage 2	686						_	
Olage 2	000							
Approach	WB			N	В	SB		
HCM Control Delay, s	12.5				0	0		
HCM LOS	В							
Minor Lane/Major Mvmt	NBT	NBR WBLn1	SBL	SBT				
Capacity (veh/h)	-	- 495	659	-				
HCM Lane V/C Ratio	-	- 0.029	-	-				
HCM Control Delay (s)	-	- 12.5	0	-				
ICM Lane LOS	-	- B	Α	-				
HCM 95th %tile Q(veh)	-	- 0.1	0	-				

Intersection							
Int Delay, s/veh	2.7						
Movement	WBL	WBR		NBT	NBR	SBL	SBT
Vol, veh/h	47	6		21		3	26
Conflicting Peds, #/hr	0	0		0		0	0
Sign Control	Stop	Stop		Free		Free	Free
RT Channelized	-	None		-		-	None
Storage Length	0	-		-	-	-	-
Veh in Median Storage, #	0	-		0	-	-	0
Grade, %	0	-		0	-	-	0
Peak Hour Factor	92	92		92	92	92	92
Heavy Vehicles, %	1	1		1	1	1	1
Mvmt Flow	51	7		23	92	3	28
Major/Minor	Minor1			Major1		Major2	
Conflicting Flow All	104	69		0		115	0
Stage 1	69	-		-		-	-
Stage 2	35	-		-	_	-	-
Critical Hdwy	6.41	6.21		-	<u>-</u>	4.11	-
Critical Hdwy Stg 1	5.41	-		-	-	-	-
Critical Hdwy Stg 2	5.41	-		-		-	-
Follow-up Hdwy	3.509	3.309		-	-	2.209	=
Pot Cap-1 Maneuver	896	997		-	-	1480	-
Stage 1	956	-		-	-	-	-
Stage 2	990	-		-	-	-	-
Platoon blocked, %				-	-		-
Mov Cap-1 Maneuver	894	997		-	-	1480	-
Mov Cap-2 Maneuver	894	-		-	-	-	-
Stage 1	956	-		-	-	-	-
Stage 2	988	-		-	-	-	-
Approach	WB			NB		SB	
HCM Control Delay, s	9.2			0		0.8	
HCM LOS	A					3.0	
Minor Lane/Major Mvmt	NBT	NBR WBLn1	SBL	SBT			
Capacity (veh/h)	_	- 905	1480	-			
HCM Lane V/C Ratio	_	- 0.064	0.002	-			
HCM Control Delay (s)	-	- 9.2	7.4	0			
HCM Lane LOS	<u>-</u>	- A	Α	A			
HCM 95th %tile Q(veh)	-	- 0.2	0	-			

Intersection									
Int Delay, s/veh	2.2								
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR
Vol, veh/h	3	190	57	24	139	0	47	0	26
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None
Storage Length	250	-	250	250	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2
Mvmt Flow	3	207	62	26	151	0	51	0	28
Major/Minor	Major1			Major2			Minor1		
Conflicting Flow All	151	0	0	207	0	0	417	416	207
Stage 1	-	-	-	-	-	-	213	213	-
Stage 2	-	_	_	_	_	=	204	203	_
Critical Hdwy	4.12	_	_	4.12	_	_	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	_	-	-	_	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318
Pot Cap-1 Maneuver	1430	-	-	1364	-	-	546	527	833
Stage 1	-	_	-	-	-	-	789	726	-
Stage 2	-	-	-	-	-	-	798	733	-
Platoon blocked, %		-	-		-	-			
Mov Cap-1 Maneuver	1430	-	-	1364	-	-	536	516	833
Mov Cap-2 Maneuver	-	-	-	-	-	-	536	516	-
Stage 1	-	-	-	-	-	-	787	724	-
Stage 2	-	-	-	-	-	-	781	719	-
Approach	EB			WB			NB		
HCM Control Delay, s	0.1			1.1			11.4		
HCM LOS	0.1			1.1			В		
Minor Lane/Major Mvmt	NBLn1	NBLn2	EBL	EBT EBR	WBL	WBT	WBR SBLn1		
Capacity (veh/h)	536	833	1430		1364	-	- 895		
HCM Lane V/C Ratio	0.095	0.034	0.002		0.019	_	- 0.002		
HCM Control Delay (s)	12.4	9.5	7.5		7.7	-	- 0.002		
HCM Lane LOS	12.4 B	9.5 A	7.5 A		Α.		- A		
HCM 95th %tile Q(veh)	0.3	0.1	0		0.1	-	- A		
HOW SOUL WILL (VEII)	0.3	U. I	U		0.1	-	- 0		

Intersection			
Int Delay, s/veh			
Movement	SBL	SBT	SBR
Vol, veh/h	0	0	2
Conflicting Peds, #/hr	0	0	0
Sign Control	Stop	Stop	Stop
RT Channelized	-	- Otop	None
Storage Length	-	=	-
Veh in Median Storage, #	-	0	_
Grade, %	-	0	_
Peak Hour Factor	92	92	92
Heavy Vehicles, %	2	2	2
Mymt Flow	0	0	2
	Ţ.		_
Major/Minor	Minor2		
Conflicting Flow All	416	416	151
Stage 1	203	203	-
Stage 2	213	213	-
Critical Hdwy	7.12	6.52	6.22
Critical Hdwy Stg 1	6.12	5.52	-
Critical Hdwy Stg 2	6.12	5.52	-
Follow-up Hdwy	3.518	4.018	3.318
Pot Cap-1 Maneuver	547	527	895
Stage 1	799	733	-
Stage 2	789	726	-
Platoon blocked, %			
Mov Cap-1 Maneuver	520	516	895
Mov Cap-2 Maneuver	520	516	-
Stage 1	797	719	-
Stage 2	761	724	-
Approach	SB		
HCM Control Delay, s	9		
HCM LOS	Α		
Minor Long/Main Mary			
Minor Lane/Major Mvmt			

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	7	†	7	7	†	7	*	^	7	*	^	7
Volume (vph)	125	100	203	373	319	140	133	422	91	152	657	220
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	4.0	8.0	8.0	4.0	8.0	8.0	25.0	25.0	25.0	25.0	25.0	25.0
Minimum Split (s)	9.0	15.0	15.0	9.0	15.0	15.0	36.0	36.0	36.0	36.0	36.0	36.0
Total Split (s)	10.0	15.0	15.0	20.0	25.0	25.0	55.0	55.0	55.0	55.0	55.0	55.0
Total Split (%)	11.1%	16.7%	16.7%	22.2%	27.8%	27.8%	61.1%	61.1%	61.1%	61.1%	61.1%	61.1%
Yellow Time (s)	3.0	4.0	4.0	3.0	4.0	4.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	0.0
Total Lost Time (s)	3.0	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	6.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	None	None	None	None	None	Min	Min	Min	Min	Min	Min
Act Effct Green (s)	18.8	10.8	10.8	30.9	22.0	22.0	28.5	28.5	28.5	28.5	28.5	26.4
Actuated g/C Ratio	0.28	0.16	0.16	0.47	0.33	0.33	0.43	0.43	0.43	0.43	0.43	0.40
v/c Ratio	0.36	0.35	0.49	0.60	0.54	0.24	0.57	0.29	0.12	0.43	0.46	0.30
Control Delay	15.1	29.5	8.8	17.1	23.6	5.0	25.5	13.1	0.9	18.0	14.7	3.2
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	15.1	29.5	8.8	17.1	23.6	5.0	25.5	13.1	0.9	18.0	14.7	3.2
LOS	В	С	Α	В	С	Α	С	В	Α	В	В	Α
Approach Delay		15.5			17.6			13.9			12.8	
Approach LOS		В			В			В			В	

Cycle Length: 90

Actuated Cycle Length: 66.4

Natural Cycle: 60

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.60 Intersection Signal Delay: 14.8 Intersection Capacity Utilization 82.3%

Intersection LOS: B ICU Level of Service E

Analysis Period (min) 15

Splits and Phases: 1: SH 83 & Hodgen Rd



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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		ર્ન	7	ሻ	^	7	ሻ	^	7
Volume (vph)	27	109	281	183	210	50	271	311	131	47	565	111
Turn Type	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0
Total Split (s)	25.0	25.0	25.0	25.0	25.0	25.0	45.0	45.0	45.0	45.0	45.0	45.0
Total Split (%)	35.7%	35.7%	35.7%	35.7%	35.7%	35.7%	64.3%	64.3%	64.3%	64.3%	64.3%	64.3%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)		4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Min	C-Min	C-Min	C-Min	C-Min	C-Min
Act Effct Green (s)		24.8	24.8		24.8	24.8	37.2	37.2	37.2	37.2	37.2	37.2
Actuated g/C Ratio		0.35	0.35		0.35	0.35	0.53	0.53	0.53	0.53	0.53	0.53
v/c Ratio		0.26	0.41		0.83	0.09	0.73	0.18	0.15	0.09	0.33	0.13
Control Delay		18.7	5.0		39.4	6.3	25.1	8.4	1.8	7.7	9.6	1.8
Queue Delay		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay		18.7	5.0		39.4	6.3	25.1	8.4	1.8	7.7	9.6	1.8
LOS		В	Α		D	Α	С	Α	Α	Α	Α	Α
Approach Delay		9.4			35.7			13.6			8.3	
Approach LOS		Α			D			В			Α	

Cycle Length: 70

Actuated Cycle Length: 70

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.83

Intersection Signal Delay: 15.5 Intersection LOS: B
Intersection Capacity Utilization 69.7% ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 5: SH 83 & Walker Road



Onititalite i chomic		port									7 IIII T GUIT TTGUI
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Inte	rval #1	1 7:00		
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	Т	R	L	Т	R	LTR	L	TR		
Stop Del/Veh (s)	5.2	0.4	0.0	0.9	0.0	0.0	9.6	14.6	10.0	2.3	
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Inte	rval #2	2 7:15		
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	T	R	L	T	R	LTR	L	TR		
Stop Del/Veh (s)	7.2	0.4	0.0	0.9	0.0	0.0	19.3	21.9	12.2	2.9	
3: Timber Meadow	Drive &	Hodge EB	en Rd EB	Perfor	mance WB	by lar	ne Inte	rval #3	3 7:30 SB	All	
Movements Served	LD	T	R		T	R	LTR	JD I	TR	All	
Stop Del/Veh (s)	5.1	0.4	0.0	0.0	0.0	0.0	13.5	20.3	9.2	2.5	
3: Timber Meadow											
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	Т	R	L	Т	R	LTR	L	TR		
Stop Del/Veh (s)	4.7	0.4	0.0	2.4	0.0	0.0	26.9	12.4	11.5	2.7	
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Enti	re Rur	า		
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All	
Movements Served	L	T	R	L	T	R	LTR	L	TR		
Stop Del/Veh (s)	5.6	0.4	0.0	1.0	0.0	0.0	17.5	17.5	11.6	2.7	

Intersection							
Int Delay, s/veh	0.2						
- y ,							
Movement	WBL	WBR		NBT	NBR	SBL	SBT
Vol, veh/h	0	27		685	1	0	1029
Conflicting Peds, #/hr	0	0		003	0	0	0
Sign Control	Stop	Stop		Free	Free	Free	Free
RT Channelized	- -	None		-	None		None
Storage Length	0	-		_	500	_	-
Veh in Median Storage, #		-		0	-	_	0
Grade, %	0	-		0	_	-	0
Peak Hour Factor	83	83		95	78	99	95
Heavy Vehicles, %	0	0		5	0	0	5
Mvmt Flow	0	33		721	1	0	1083
Major/Minor	Minor1			Major1		Major2	
	1263	361			0	721	0
Conflicting Flow All Stage 1	721	301		<u> </u>	<u>-</u>	121	-
Stage 1	542	-		-	-	-	-
Critical Hdwy	6.8	6.9		-	-	4.1	-
Critical Hdwy Stg 1	5.8	0.9		-	-	4.1	-
Critical Hdwy Stg 2	5.8	_		_	_	-	-
Follow-up Hdwy	3.5	3.3		-	-	2.2	-
Pot Cap-1 Maneuver	164	641		<u>-</u>	_	890	<u>-</u>
Stage 1	448	-		_	_	-	_
Stage 2	553	<u>-</u>		<u>-</u>	_	_	_
Platoon blocked, %	000			-	-		-
Mov Cap-1 Maneuver	164	641		-	_	890	-
Mov Cap-2 Maneuver	164	-		-	-	-	-
Stage 1	448	-		-	-	-	-
Stage 2	553	<u>-</u>		_	-	-	-
Approach	WB			NB		SB	
HCM Control Delay, s	10.9			0		0	
HCM LOS	В			U		U	
I IOW LOO	D						
Mariana I and Andrew Maria	NDT	NIDDWDL 4	001	ODT			
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT			
Capacity (veh/h)	-	- 641	890	-			
HCM Lane V/C Ratio	-	- 0.051	-	-			
HCM Control Delay (s)	-	- 10.9	0	-			
HCM Lane LOS	-	- B	Α	-			
HCM 95th %tile Q(veh)	-	- 0.2	0	-			

Intersection							
	.4						
THE BOILDY, OF VOIT	.,						
Movement	WBL	WBR		NBT	NBR	SBL	SBT
Vol, veh/h	86	19		32	33	9	68
Conflicting Peds, #/hr	0	0		0	0	0	0
Sign Control	Stop	Stop		Free	Free	Free	Free
RT Channelized	-	None		-	None	-	None
Storage Length	0	-		-	-	-	_
Veh in Median Storage, #	0	-		0	-	-	0
Grade, %	0	-		0	-	-	0
Peak Hour Factor	92	92		92	92	92	92
Heavy Vehicles, %	1	1		1	1	1	1
Mvmt Flow	93	21		35	36	10	74
Major/Minor	Minor1			Major1		Major2	
Conflicting Flow All	146	53		0	0	71	0
Stage 1	53	-		-	-	-	-
Stage 2	93				_		_
Critical Hdwy	6.41	6.21		<u>-</u>	-	4.11	_
Critical Hdwy Stg 1	5.41	0.21		=	_	-	_
Critical Hdwy Stg 2	5.41	_		_	_	_	_
Follow-up Hdwy	3.509	3.309		_	_	2.209	_
Pot Cap-1 Maneuver	849	1017		_	_	1536	_
Stage 1	972	-		_	_	-	_
Stage 2	933	_		_	_	_	_
Platoon blocked, %				_	_		-
Mov Cap-1 Maneuver	843	1017		-	_	1536	-
Mov Cap-2 Maneuver	843	-		-	-	-	-
Stage 1	972	-		-	-	-	-
Stage 2	926	-		-	-	_	-
Approach	WB			NB		SB	
HCM Control Delay, s	9.8			0		0.9	
HCM LOS	9.0 A			U		0.9	
HOW LOS	A						
NA' I (NA - ' NA I	NDT	NDDWDL 4	ODI	ODT			
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT			
Capacity (veh/h)	-		1536	-			
HCM Lane V/C Ratio	-	- 0.131		-			
HCM Control Delay (s)	-	- 9.8	7.4	0			
HCM Lane LOS	-	- A	Α	Α			
HCM 95th %tile Q(veh)	-	- 0.5	0	-			

Intersection												
Int Delay, s/veh	7.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Vol, veh/h	1	101	184	93	283	0	157	0	76	0	0	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	0	-	0	0	-	-	-	-	0	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	75	75	92	92	75	75	75	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	1	110	245	124	308	0	209	0	101	0	0	3
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	308	0	0	110	0	0	669	668	110	668	668	308
Stage 1	-	-	-	-	-	-	112	112	-	556	556	-
Stage 2	-	_	_	-	-	-	557	556	_	112	112	_
Critical Hdwy	4.12	-	-	4.12	-	_	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1253	-	-	1480	-	-	371	379	943	372	379	732
Stage 1	-	-	-	-	-	-	893	803	-	515	513	-
Stage 2	-	-	-	-	-	-	515	513	-	893	803	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1253	-	-	1480	-	-	345	347	943	311	347	732
Mov Cap-2 Maneuver	-	-	-	-	-	-	345	347	-	311	347	-
Stage 1	-	-	-	_	-	-	892	802	-	515	470	-
Stage 2	-	-	-	-	-	-	470	470	-	796	802	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			2.2			23.5			9.9		
HCM LOS	•						C			A		
Minor Lane/Major Mvmt	NBLn1	NRI n2	EBL	EBT EBR	WBL	WBT	WBR SBLn1					
Capacity (veh/h)	345	943	1253			1101	- 732					
HCM Lane V/C Ratio	0.607		0.001		0.084	-	- 0.004					
HCM Control Delay (s)	30.3	9.3	7.9		7.7	-	- 9.9					
HCM Lane LOS	30.3 D	9.3 A	7.9 A		Α.	_	- 9.9 - A					
HCM 95th %tile Q(veh)	3.8	0.4	0		0.3	-	- A					
HOW JOHN /OHIE Q(VEH)	3.0	0.4	U		0.5	-	- 0					

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	ሻ	†	7	7	↑	7	ሻ	^	7	ሻ	^	7
Volume (vph)	199	302	304	162	200	170	253	656	309	152	318	224
Turn Type	pm+pt	NA	Perm	pm+pt	NA	Perm	Perm	NA	Perm	Perm	NA	Perm
Protected Phases	7	4		3	8			2			6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	7	4	4	3	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	4.0	8.0	8.0	4.0	8.0	8.0	25.0	25.0	25.0	25.0	25.0	25.0
Minimum Split (s)	9.0	15.0	15.0	9.0	15.0	15.0	36.0	36.0	36.0	36.0	36.0	36.0
Total Split (s)	10.0	20.0	20.0	10.0	20.0	20.0	60.0	60.0	60.0	60.0	60.0	60.0
Total Split (%)	11.1%	22.2%	22.2%	11.1%	22.2%	22.2%	66.7%	66.7%	66.7%	66.7%	66.7%	66.7%
Yellow Time (s)	3.0	4.0	4.0	3.0	4.0	4.0	5.0	5.0	5.0	5.0	5.0	5.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	1.0	1.0	1.0	1.0	1.0	1.0
Lost Time Adjust (s)	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	-2.0	0.0
Total Lost Time (s)	3.0	4.0	4.0	3.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	6.0
Lead/Lag	Lead	Lag	Lag	Lead	Lag	Lag						
Lead-Lag Optimize?	Yes	Yes	Yes	Yes	Yes	Yes						
Recall Mode	None	None	None	None	None	None	Min	Min	Min	Min	Min	Min
Act Effct Green (s)	24.1	16.1	16.1	24.1	16.1	16.1	30.2	30.2	30.2	30.2	30.2	28.2
Actuated g/C Ratio	0.37	0.25	0.25	0.37	0.25	0.25	0.47	0.47	0.47	0.47	0.47	0.44
v/c Ratio	0.48	0.71	0.51	0.50	0.47	0.35	0.58	0.43	0.37	0.59	0.21	0.29
Control Delay	18.2	33.7	6.4	19.1	25.7	6.2	17.9	12.1	2.4	22.6	10.2	2.6
Queue Delay	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay	18.2	33.7	6.4	19.1	25.7	6.2	17.9	12.1	2.4	22.6	10.2	2.6
LOS	В	С	Α	В	С	Α	В	В	Α	С	В	Α
Approach Delay		19.6			17.5			10.8			10.4	
Approach LOS		В			В			В			В	

Cycle Length: 90

Actuated Cycle Length: 64.4

Natural Cycle: 60

Control Type: Semi Act-Uncoord Maximum v/c Ratio: 0.71 Intersection Signal Delay: 14.0 Intersection Capacity Utilization 79.9%

Intersection LOS: B ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 1: SH 83 & Hodgen Rd



	۶	→	•	•	←	*	4	†	/	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		ર્ન	7	7	^	7	J.	^	7
Volume (vph)	108	150	239	65	117	18	339	585	112	24	390	81
Turn Type	Perm	NA	Perm									
Protected Phases		4			8			2			6	
Permitted Phases	4		4	8		8	2		2	6		6
Detector Phase	4	4	4	8	8	8	2	2	2	6	6	6
Switch Phase												
Minimum Initial (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Split (s)	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0	21.0
Total Split (s)	25.0	25.0	25.0	25.0	25.0	25.0	45.0	45.0	45.0	45.0	45.0	45.0
Total Split (%)	35.7%	35.7%	35.7%	35.7%	35.7%	35.7%	64.3%	64.3%	64.3%	64.3%	64.3%	64.3%
Yellow Time (s)	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0	3.0
All-Red Time (s)	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0
Lost Time Adjust (s)		-1.0	-1.0		-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0	-1.0
Total Lost Time (s)		4.0	4.0		4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None	None	None	None	C-Min	C-Min	C-Min	C-Min	C-Min	C-Min
Act Effct Green (s)		18.7	18.7		18.7	18.7	43.3	43.3	43.3	43.3	43.3	43.3
Actuated g/C Ratio		0.27	0.27		0.27	0.27	0.62	0.62	0.62	0.62	0.62	0.62
v/c Ratio		0.82	0.42		0.61	0.04	0.62	0.29	0.12	0.06	0.20	0.08
Control Delay		44.3	5.2		30.2	5.3	15.0	7.1	1.8	6.7	6.6	2.0
Queue Delay		0.0	0.0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total Delay		44.3	5.2		30.2	5.3	15.0	7.1	1.8	6.7	6.6	2.0
LOS		D	Α		С	Α	В	Α	Α	Α	Α	Α
Approach Delay		25.4			28.0			9.1			5.8	
Approach LOS		С			С			Α			Α	

Cycle Length: 70

Actuated Cycle Length: 70

Offset: 0 (0%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 55

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.82

Intersection Signal Delay: 13.8 Intersection LOS: B
Intersection Capacity Utilization 66.5% ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 5: SH 83 & Walker Road



Sillitanic Penonia	ance Ne	ροπ									I WIT CAN HOU	
3: Timber Meadow Drive & Hodgen Rd Performance by lane Interval #1 5:00												
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All		
Movements Served	L	T	R	L	Т	R	LTR	L	TR			
Stop Del/Veh (s)	2.3	0.3	0.0	0.0	0.0	0.0	19.9	12.1	4.2	1.3		
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Inte	rval #2	5:15			
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All		
Movements Served	L	T	R	L	T	R	LTR	L	TR			
Stop Del/Veh (s)	2.6	0.3	0.0	1.8	0.0	0.0	9.3	22.2	5.9	1.3		
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Inte	rval #3	5:30			
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All		
Movements Served	L	Т	R	L	Т	R	LTR	L	TR			
Stop Del/Veh (s)	2.0	0.3	0.0	1.3	0.0	0.0	13.2	16.2	5.0	1.1		
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Inte	rval #4	5:45			
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All		
Movements Served	L	T	R	L	T	R	LTR	L	TR			
Stop Del/Veh (s)	2.8	0.3	0.0	2.2	0.0	0.0	16.8	14.1	4.6	1.2		
3: Timber Meadow	Drive &	Hodge	en Rd	Perfor	mance	by lar	ne Enti	ire Rur	1			
Lane	EB	EB	EB	WB	WB	WB	NB	SB	SB	All		
Movements Served	L	T	R	L	Т	R	LTR	L	TR			
Stop Del/Veh (s)	2.4	0.3	0.0	1.4	0.0	0.0	15.7	16.7	5.2	1.2		

nt Delay, s/veh	0.2							
,								
Movement	WBL	WBR		NB	T NBR	SBL	SBT	
/ol, veh/h	0	18		1018	8 7	0	694	
Conflicting Peds, #/hr	0	0		(0 0	0	0	
Sign Control	Stop	Stop		Free	e Free	Free	Free	
RT Channelized	-	None			- None	-	None	
Storage Length	0	-			- 500	-	-	
/eh in Median Storage, #	0	-		() -	-	0	
Grade, %	0	-		() -	-	0	
Peak Hour Factor	83	83		94	4 94	97	97	
Heavy Vehicles, %	0	0		Į	5 0	0	5	
Mvmt Flow	0	22		1083		0	715	
Major/Minor	Minor1			Major	1	Major2		
Conflicting Flow All	1441	541			0 0	1083	0	
Stage 1	1083	-				-	-	
Stage 2	358	_				-	_	
Critical Hdwy	6.8	6.9				4.1	_	
Critical Hdwy Stg 1	5.8	-				-	_	
Critical Hdwy Stg 2	5.8	<u>-</u>				-	_	
Follow-up Hdwy	3.5	3.3				2.2	_	
Pot Cap-1 Maneuver	126	491				652	_	
Stage 1	291	<u>-</u>				-	_	
Stage 2	684	<u>-</u>				-	_	
Platoon blocked, %	001						_	
Mov Cap-1 Maneuver	126	491				652	_	
Mov Cap-2 Maneuver	126	-				-	_	
Stage 1	291	<u>-</u>				_	_	
Stage 2	684							
Olugo Z	004							
Approach	WB			NE	3	SB		
HCM Control Delay, s	12.7)	0		
HCM LOS	В				-	, and the second		
.5 200								
Minor Lane/Major Mvmt	NBT	NBR WBLn1	SBL	SBT				
Capacity (veh/h)	-	- 491	652	-				
ICM Lane V/C Ratio	_	- 0.044	-	<u> </u>				
HCM Control Delay (s)	-	- 12.7	0	<u>-</u>				
HCM Lane LOS	_	- 12.7 - B	A	<u>-</u>				
HCM 95th %tile Q(veh)	<u>-</u>	- 0.1	0	<u>-</u>				

Intersection							
Int Delay, s/veh	2.5						
Movement	WBL	WBR		NE	BT NBR	SBL	SBT
Vol, veh/h	53	8			14 96	7	45
Conflicting Peds, #/hr	0	0			0 0	0	0
Sign Control	Stop	Stop		Fre	ee Free	Free	Free
RT Channelized	· -	None			- None	-	None
Storage Length	0	-				-	-
/eh in Median Storage, #	0	-			0 -	-	0
Grade, %	0	-			0 -	-	0
Peak Hour Factor	92	92		(92 92	92	92
Heavy Vehicles, %	1	1			1 1	1	1
Mvmt Flow	58	9		4	18 104	8	49
Major/Minor	Minor1			Majo	r1	Major2	
Conflicting Flow All	164	100			0 0	152	0
Stage 1	100	-				-	-
Stage 2	64	-				-	-
Critical Hdwy	6.41	6.21				4.11	-
Critical Hdwy Stg 1	5.41	-				-	-
Critical Hdwy Stg 2	5.41	-				-	-
Follow-up Hdwy	3.509	3.309				2.209	-
ot Cap-1 Maneuver	829	958				1435	-
Stage 1	927	-				-	-
Stage 2	961	-				-	-
Platoon blocked, %							-
Mov Cap-1 Maneuver	824	958				1435	-
Mov Cap-2 Maneuver	824	-				-	-
Stage 1	927	-				-	-
Stage 2	955	-				-	-
Approach	WB				IB	SB	
HCM Control Delay, s	9.7				0	1	
HCM LOS	Α					' 	
	, (
Minor Lane/Major Mvmt	NBT	NBR WBLn1	SBL	SBT			
Capacity (veh/h)	-	- 839	1435	-			
HCM Lane V/C Ratio	_	- 0.079	0.005	-			
HCM Control Delay (s)	- -	- 9.7	7.5	0			
HCM Lane LOS	_	- A	Α.5	A			
HCM 95th %tile Q(veh)		- 0.3	0	-			

Vol, vehi/h 3 190 93 24 139 0 59 0 26 Conflicting Peds, #/hr 0	Intersection									
Vol, vehirh 3 190 93 24 139 0 59 0 26 Conflicting Peds, #/hr 0 20 92 92 92 92 92 92 92 92 92 92 92 92 92 92 92 92 92	Int Delay, s/veh	2.3								
Vol, vehirh 3 190 93 24 139 0 59 0 26 Conflicting Peds, #/hr 0 20 92 92 92 92 92 92 92 92 92 92 92 92 92 92 92 92 92										
Conflicting Peds, #hr	Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR
Sign Control Free None - None Chall Combit Combit All A	Vol, veh/h	3	190	93	24	139	0	59	0	26
RT Channelized	Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0
Storage Length	Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop
Veh in Median Storage, # - 0 - - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 - 0 0 - 0 0 - 0 0 2 <td>RT Channelized</td> <td>-</td> <td>-</td> <td>None</td> <td>-</td> <td>-</td> <td>None</td> <td>·-</td> <td>-</td> <td>None</td>	RT Channelized	-	-	None	-	-	None	·-	-	None
Grade, % - 0 - - 0 - 0 - 0 - 0 9 92	Storage Length	250	-	250	250	-	-	-	-	0
Peak Hour Factor 92	Veh in Median Storage, #	-	0	-	-	0	-	-	0	-
Heavy Vehicles, % 2 2 2 2 2 2 2 2 2	Grade, %	-	0	-	-	0	-	-	0	-
Major/Minor Major Major	Peak Hour Factor	92	92	92	92	92	92	92	92	92
Major/Minor Major Major	Heavy Vehicles, %	2	2	2	2	2	2	2	2	2
Conflicting Flow All	Mvmt Flow	3	207	101	26	151	0	64	0	28
Conflicting Flow All										
Conflicting Flow All	Major/Minor	Major1			Major2			Minor1		
Stage 1			0	0		0	0		416	207
Stage 2 - - - - - 204 203 - Critical Hdwy 4.12 - 4.12 - 7.12 6.52 6.22 Critical Hdwy Stg 1 - - - - - 6.12 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - Follow-up Hdwy 2.218 - - - 6.12 5.52 - Follow-up Hdwy 2.218 - 2.218 - 3.518 4.018 3.318 Pot Cap-1 Maneuver 1430 - - 1364 - - 789 726 - Stage 2 -		-	_	_	-	-	-	213	213	_
Critical Hdwy 4.12 - 4.12 - 7.12 6.52 6.22 Critical Hdwy Stg 1 - - - - - 6.12 5.52 - Critical Hdwy Stg 2 - - - - - 6.12 5.52 - Follow-up Hdwy 2.218 - - - 6.12 5.52 - Follow-up Hdwy 2.218 - - - 6.12 5.52 - Follow-up Hdwy 2.218 - - 2.218 - - 5.52 - Follow-up Hdwy 2.218 - - 1364 - - 546 527 833 Stage 1 - - - - - 789 723 - Stage 1 - - - - - - 787 724 - Stage 2 - - - - - - 781 719 - Approach EB WB WB NB </td <td></td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td>-</td> <td></td> <td></td> <td>_</td>		-	-	-	-	-	-			_
Critical Hdwy Stg 1 - - - - 6.12 5.52 - Critical Hdwy Stg 2 - - - - 6.12 5.52 - Follow-up Hdwy 2.218 - - 2.218 - 3.518 4.018 3.318 Pot Cap-1 Maneuver 1430 - - 1364 - - 789 726 833 Stage 2 - - - - 798 733 - 1364 - - 798 733 - 1364 - - - 798 733 -		4.12	_	_	4.12	-	-			6.22
Critical Hdwy Stg 2 - - - - - 6.12 5.52 - Follow-up Hdwy 2.218 - - 2.218 - 3.518 4.018 3.318 Pot Cap-1 Maneuver 1430 - - 1364 - - 546 527 833 Stage 1 - - - - - 789 726 - Stage 2 - - - - - 798 733 - Platoon blocked, % -	Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-
Follow-up Hdwy 2.218 2.218 3.518 4.018 3.318 Pot Cap-1 Maneuver 1430 1364 546 527 833 Stage 1 789 726 - 546 527 833 Stage 2 798 733 - 726 Platoon blocked, % 536 516 833 Mov Cap-1 Maneuver 1430 1364 536 516 833 Mov Cap-2 Maneuver 787 724 - 536 516 516 Stage 1 787 724 - 536 516 516 Stage 2 781 719 - 536 Stage 2 781 719 - 536 Approach EB WB NB HCM Control Delay, s 0.1 1.1 1.1 11.7 HCM LOS B Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 536 833 1430 - 1364 - 895 HCM Lane V/C Ratio 0.12 0.034 0.002 - 0.019 - 0.002 HCM Control Delay (s) 12.6 9.5 7.5 - 7.7 - 9 HCM Lane LOS B A A A - A	Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	_
Stage 1 - - - - - 726 - Stage 2 - - - - - 798 733 - Platoon blocked, % -	Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318
Stage 2 - - - - - 798 733 - Platoon blocked, % -<	Pot Cap-1 Maneuver	1430	-	-	1364	-	-	546	527	833
Platoon blocked, %	Stage 1	-	-	-	-	-	-	789	726	-
Mov Cap-1 Maneuver 1430 - 1364 - 536 516 833 Mov Cap-2 Maneuver - - - - - - 536 516 - Stage 1 - - - - - - 787 724 - Stage 2 - - - - - - 781 719 - Approach EB WB NB	Stage 2	-	-	-	-	-	-	798	733	-
Mov Cap-2 Maneuver - - - - - 536 516 - Stage 1 - - - - - - 787 724 - Stage 2 - - - - - - 781 719 - Approach EB WB NB	Platoon blocked, %		-	-		-	-			
Stage 1 - - - - - 724 - Stage 2 - - - - - - 781 719 - Approach EB WB NB NB HCM Control Delay, s 0.1 1.1 11.7 11.7 HCM LOS B B WBL WBT WBR SBLn1 Capacity (veh/h) 536 833 1430 - - 1364 - - 895 HCM Lane V/C Ratio 0.12 0.034 0.002 - - 0.019 - - 0.002 HCM Control Delay (s) 12.6 9.5 7.5 - 7.7 - - 9 HCM Lane LOS B A A - A - - A	Mov Cap-1 Maneuver	1430	-	-	1364	-	-			833
Stage 2	Mov Cap-2 Maneuver	-	-	-	-	-	-			-
Approach EB WB NB HCM Control Delay, s 0.1 1.1 11.7 HCM LOS B B B B Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBR WBL WBR SBLn1 Capacity (veh/h) 536 833 1430 - - 1364 - - 895 HCM Lane V/C Ratio 0.12 0.034 0.002 - - 0.019 - - 0.002 HCM Control Delay (s) 12.6 9.5 7.5 - 7.7 - - 9 HCM Lane LOS B A A - - A - - A	Stage 1	-	-	-	-	-	-			_
HCM Control Delay, s	Stage 2	-	-	-	-	-	-	781	719	-
HCM Control Delay, s										
HCM Control Delay, s	Approach	EB			WB			NB		
Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 536 833 1430 - - 1364 - - 895 HCM Lane V/C Ratio 0.12 0.034 0.002 - - 0.019 - - 0.002 HCM Control Delay (s) 12.6 9.5 7.5 - - 7.7 - - 9 HCM Lane LOS B A A - - A - - A		0.1			1.1			11.7		
Minor Lane/Major Mvmt NBLn1 NBLn2 EBL EBT EBR WBL WBT WBR SBLn1 Capacity (veh/h) 536 833 1430 - - 1364 - - 895 HCM Lane V/C Ratio 0.12 0.034 0.002 - - 0.019 - - 0.002 HCM Control Delay (s) 12.6 9.5 7.5 - - 7.7 - - 9 HCM Lane LOS B A A - - A - - A								В		
Capacity (veh/h) 536 833 1430 - - 1364 - - 895 HCM Lane V/C Ratio 0.12 0.034 0.002 - - 0.019 - - 0.002 HCM Control Delay (s) 12.6 9.5 7.5 - - 7.7 - - 9 HCM Lane LOS B A A - A - - A										
Capacity (veh/h) 536 833 1430 - - 1364 - - 895 HCM Lane V/C Ratio 0.12 0.034 0.002 - - 0.019 - - 0.002 HCM Control Delay (s) 12.6 9.5 7.5 - - 7.7 - - 9 HCM Lane LOS B A A - A - - A	Minor Lane/Maior Mymt	NBI n1	NBLn2	EBI	EBT FBR	WBI	WBT	WBR SBI n1		
HCM Lane V/C Ratio 0.12 0.034 0.002 - - 0.019 - - 0.002 HCM Control Delay (s) 12.6 9.5 7.5 - - 7.7 - - 9 HCM Lane LOS B A A - - A - - A										
HCM Control Delay (s) 12.6 9.5 7.5 - - 7.7 - - 9 HCM Lane LOS B A A - - A - - A										
HCM Lane LOS B A A A A										
	HCM 95th %tile Q(veh)	0.4	0.1	0		0.1	_			

Intersection			
Int Delay, s/veh			
Movement	SBL	SBT	SBR
Vol, veh/h	0	0	2
Conflicting Peds, #/hr	0	0	0
Sign Control	Stop	Stop	Stop
RT Channelized	-	- Otop	None
Storage Length	-	=	-
Veh in Median Storage, #	-	0	_
Grade, %	-	0	_
Peak Hour Factor	92	92	92
Heavy Vehicles, %	2	2	2
Mymt Flow	0	0	2
	Ţ.		_
Major/Minor	Minor2		
Conflicting Flow All	416	416	151
Stage 1	203	203	-
Stage 2	213	213	-
Critical Hdwy	7.12	6.52	6.22
Critical Hdwy Stg 1	6.12	5.52	-
Critical Hdwy Stg 2	6.12	5.52	-
Follow-up Hdwy	3.518	4.018	3.318
Pot Cap-1 Maneuver	547	527	895
Stage 1	799	733	-
Stage 2	789	726	-
Platoon blocked, %			
Mov Cap-1 Maneuver	520	516	895
Mov Cap-2 Maneuver	520	516	-
Stage 1	797	719	-
Stage 2	761	724	-
Approach	SB		
HCM Control Delay, s	9		
HCM LOS	Α		
Minor Long/Main Mary			
Minor Lane/Major Mvmt			

LSC TRANSPORTATION CONSULTANTS, INC.



516 North Tejon Street Colorado Springs, CO 80903 (719) 633-2868 FAX (719) 633-5430 E-mail: lsc@lsccs.com

November 3, 2014

Mr. Matt Dunston Walden Holdings 1, LLC 17145 Colonial Park Drive Monument, CO 80132

RE: Walden Preserve 2

Preliminary Plan and Filings 1 and 2

Addendum Report for CDOT

LSC #144380

Dear Mr. Dunston:

LSC has prepared this letter addendum to the September 17, 2014 traffic report for Walden Preserve 2. This letter has been prepared due to the results of updated traffic counts at the Walden Way/State Highway 83 intersection and changes to the internal road layout of the subdivision which will shift some site-generated traffic from the north section of Walden Way to the north to Walker Road.

This addendum report concludes that: 1) no improvements are warranted now or are projected to be warranted in the foreseeable future at the intersection of Walden Way/SH 83, and 2) a Four-Hour Volume traffic signal warrant appears to already be warranted at the intersection of SH83 and Walker/Highway 105.

The site location is shown in Figure 1. The development context map is shown in Figure 2. This report presents a proposed change **from** the future right-in/right-out configuration shown in the traffic report for the intersection of Walden Way/State Highway 83 **to** keeping the intersection as a full-movement intersection. This addendum letter also provides a status update regarding the proposed future Walden Preserve 2 connection to Walker Road.

This addendum report contains the following:

- An introduction to the proposed September 17, 2014 traffic report modification regarding the Walden Way/State Highway 83 intersection.
- Status update regarding the proposed Walden Preserve 2 connection to Walker Road.

- Current morning and afternoon peak-hour traffic volume counts at the intersection of Walden Way/State Highway 83. These update the April 2012 volumes contained in the September 17, 2014 traffic report. Based on these counts, this report also presents estimated adjustments to the November 2012 counts at Walker/SH 83. It is based on these adjustments that a traffic signal warrant appears to be met currently.
- Status of nearby/adjacent subdivisions relative to the number of constructed homes.
- Updates to the projected background traffic volumes at the Walden Way/State Highway 83 intersection contained in the September 17, 2014 traffic report. Updates to the projected background traffic volumes at Walker/State Highway 83.
- Updates to the projected site-generated traffic volumes at the Walden Way/State Highway 83 intersection contained in the September 17, 2014 traffic report and updates to the projected site-generated traffic volumes at the Walker/State Highway 83 intersection.
- Updated projections of future total traffic at these two intersections and levels of service.
- Update to the traffic signal warrant analysis and anticipated percentages of participation by this project at Walker Road/State Highway 105.
- Updates to recommendations for the Walden Way/State Highway 83 intersection with the proposal to keep the intersection as it currently exists, full movement, and concentrate efforts on the Walker Road/SH 83 intersection.

INTRODUCTION/BACKGROUND

During the development review process of the Walden Preserve 2 PUD plan, the applicant proposed a future connection north to Walker Road. This will be a significant improvement to the traffic distribution system of the project and will result in a reduced traffic impact on both the north section of Walden Way just east of SH 83 and Timber Meadow Drive to the south. The other significant change to the plan was the removal of the northern connection between Walden Preserve 2 and Walden Way. This change will further discourage Walden Preserve 2 trips from using the north section of Walden Way in favor of the future connection to the north to Walker Road. These two changes from the previous plan iterations are illustrated in Figure 3.

The applicant has held discussions with the residents along Walden Way. The applicant has indicated to LSC that many of the residents are resistant to either closing off the intersection entirely or installing major improvements to it, for instance constructing a raised island to prohibit left-turn movements and converting the intersection to a right-in-right-out. The applicant has indicated to LSC from their discussions that residents are not dissatisfied with the current configuration and do not see a need for improvements. Given the views expressed by the Walden Way residents and the proposed investment in the Walker Road connection and other efforts by the applicant to discourage Walden Preserve 2 traffic from using the north end of Walden Way, the applicant would prefer to focus resources on improvements to Walker Road in the vicinity of the proposed location of the Walden PUD connection to Walker Road and the signalization of Walker Road/State Highway 83.

The developers of Walden Preserve 2 are working with School District 38, which owns a 70-acre parcel on the southeast corner of the intersection. It is anticipated that eventually a school of some sort, not a high school, will be built on the site and will contribute more traffic at the intersection of Walker Road/SH 83. Therefore, the developers are proposing to concentrate their efforts at Walker/SH 83, where funding will be most beneficial as the traffic signal is warranted. The Walker Road intersection is identified in the County *Major Thoroughfares Transportation Plan* as a Major Collector. The west leg of the intersection (Highway 105) is a Principal Arterial. It is also important to note that Highway 105 west of Highway 83 is a PPRTA project and PPRTA funds may be available to match developer contributions for future signalization.

The applicant met with CDOT on October 3, 2014 to discuss the concept of focusing efforts on the improvements to Walker Road and signalization of Walker Road/State Highway 105 rather than toward construction costs to restrict Walden Way to right-in/right out. CDOT was receptive to the concept of focusing efforts at Walker/SH 83 and requested this proposal/letter update from LSC on behalf of the applicant. LSC has been requested by the applicant to reevaluate the originally proposed right-in/right-out at Walden Way. CDOT requested an analysis of the Walden Way intersection assuming the current full-movement configuration and the effects of this change at the SH 83/Highway 105/Walker Road intersection. The effects of this change at SH83/Walker Road of interest to CDOT is primarily the change in signal percentage contribution by Walden Preserve 2.

WALKER ROAD CONNECTION STATUS

The applicant and consultant team met with the County Engineer on-location in the field on Walker Road to discuss options for the proposed future intersection of Walker Road/Walden Preserve 2 north connection. Options discussed included an intersection aligning with Shannon Road (890 feet west of Highway 83) and an intersection at the location of the Walden District wastewater treatment plant (1,400 feet west of Highway 83). The potential for the need for both access points depending of the size and circulation/capacity needs of the future school at the southeast corner of Highway 83/ Walker Road was also discussed. The applicant will be conducting some initial design work to evaluate these options relative to sight distance and potential future right-of-way and turn lane needs. As preliminary design concepts are developed, these would be sent to CDOT for review as although this project will primarily involve the County road, we anticipate interest by CDOT because of the proximity to the SH 83 intersection. The developers plan to have the street connection north to Walker Road constructed by the time half the homes are built within Walden Preserve 2.

UPDATED TRAFFIC VOLUME COUNTS

LSC has completed updated traffic counts during the morning and afternoon peak hour at the Walden Way/SH 83 intersection. The count data sheets are attached for reference. The attached Figure 4 shows the count results from the new counts taken in October 2014. Through traffic on SH 83 has increased since the previous count. Regarding the southbound left-turn movement, the previous count from 2012 indicated an afternoon southbound peak-hour left-turn volume of 11 vehicles per hour, which was over the 10 vph maximum volume before a left-turn lane would be required (RA Classification in the State Highway Access Code). The recent October 2014 count shows a southbound left-turn volume of four vehicles per hour.

STATUS OF ADJACENT SUBDIVISIONS

Figure 2 shows the existing and planned area subdivisions. Figure 3 shows the adjacent subdivisions, the total number of lots within Walden III, Walden Preserve Filing 2 and the large lot area of Filing No. 1 and the number of homes built.

The purpose of compiling these data is 1) to evaluate the current and previous intersection turning volumes at Walden Way against the area trip generation and 2) to estimate the added turning movements at the intersection that would be generated by the future, yet-to-be-built homes in these areas. This information has been used to estimate both the background and site-generated turning movements at the Walden Way/SH 83 intersection.

The primary current users of the Walden Way intersection and the north end of Walden Way are the 41 homes in Walden III (excepting lots with access directly to SH 83). Also, homes have been built on most of the lots in Walden Preserve Filing 2, the next closest subdivision to the Walden Way/SH 83 intersection.

The southbound left-turn volume and the westbound right-turn volume have decreased from the 2012 traffic count despite additional homes having been constructed in the original Walden Preserve Filings 1 and 2 near the Walden Way/Pond View Place intersection to the south. There are a couple of possible reasons for the turn movement reductions despite the additional homes: 1) The general commuter peak distribution to/from this area may have shifted slightly from north to south since 2012 possibly due to improved economic conditions in the Colorado Springs area. 2) Through traffic has increased on State Highway 83 and traffic from the subdivisions in the Walden/Settlers Ranch area and resident motorists are opting to utilize the more major intersections of Hodgen/SH 83 and Walker/SH 83 to either travel eastbound straight across SH 83 and use Walker and Hodgen and the local/collector street network as a route to their homes rather than using the Walden Way/SH 83 intersection.

UPDATED TRAFFIC PROJECTIONS AND ANALYSIS

Figures 5 and 6 show revised background traffic estimates. Figure 7 shows the directional distribution and Figures 8 and 9 show the revised estimates of site-generated turning movement volumes at the Walden Way/SH 83 and Walker Road/Highway 83 intersections. Figures 10 and 11 show the resulting updated total traffic volumes, levels of service, and laneage.

Based on the estimated turning movements and the State Highway Access Code turning volume threshold, auxiliary turn lane improvements at Walden Way/SH 83 would not be required.

Tables 1 and 2 show the revised signal warrant analysis and estimates of signal warrant fair share percentage for Walden Preserve. Based on estimated existing traffic at Highway 83/Walker Road, a signal is currently warranted at this intersection.

SUMMARY

The data and projections contained in this addendum report support leaving the intersection of Walden Way/SH 83 as it currently exists. The applicant's updated percentage toward SH 83/Walker signalization is 17.6 percent. The signal appears to be warranted now, earlier than previously anticipated, primarily due to significant increases in peak-hour through traffic on SH 83 since 2012.

Please contact me if you have any questions or need further assistance.

Sincerely,

LSC TRANSPORTATION CONSULTANTS, INC.

By

Jeffrey C. Hodsdon, P.E., PTOE

Principal

JCH:bjwb

Enclosures: Tables 1 and 2

Figures 1-11

Traffic Count Reports

Table 1 Traffic Signal Warrant Analysis Peak-Hour Volumes Walden Preserve 2

	Vehicles	Per Hour
	Peak Hour	Peak Hour
	7:00-8:00 a.m.	4:45-5:45 p.m.
MINOR STREET TRAFFIC		
Westbound		
Site-Generated Traffic ⁽¹⁾		
Left	6	4
Through	20	14
Right	###	##
Background Traffic ⁽²⁾		
Left	55	25
Through	90	55
Right	###	###
Eastbound		
Site-Generated Traffic		
Left	0	0
Through	8	27
Right	###	###
Background Traffic		
Left	15	45
Through	30	60
Right	###	###
Westbound Minor Street	171	98
Eastbound Minor Street	53	132
MAJOR STREET TRAFFIC		
Northbound		
Site-Generated Traffic		
Left	10	7
Through	3	2
Right	4	12
Background Traffic		
Left	125	150
Through	190	240
Right	20	50
Southbound		
Site-Generated Traffic		_
Left	1	5
Through	1	1
Right	0	0
Background Traffic Left	10	10
Through	250	315
Right	50	40
-	50	40
Major Street Totals		
	7:00-8:00 a.m.	5:00-6:00 p.m.
	664	832

Notes:

- (1) Includes original Filings 1, 2 and buildout of Walden Preserve 2
- (2) Based on Existing (2014) Traffic Volumes

Source: LSC Transportation Consultants, Inc.

Table 2 Traffic Signal Warrant Analysis Four-Hour Volumes - Baseline + Original Filings 1, 2 and buildout of Walden Preserve 2 Walden Preserve 2

		Vehicles	Per Hour	
	6:30-7:30 a.m.	7:30-8:30 a.m.	4:00-5:00 p.m.	5:00-6:00 p.m.
MINOR STREET TRAFFIC				
Westbound				
Site-Generated Traffic ⁽¹⁾	_		_	_
Left	5	3	3	5
Through Right	16 ###	10 ###	12 ###	16 ###
	****	*****	###	###
Background Traffic ⁽²⁾ Left	50	45	41	23
Through	81	63	41	23 51
Right	###	###	###	###
Eastbound				
Site-Generated Traffic				
Left	0	0	0	0
Through	7	7	22	30
Right	###	###	###	###
Background Traffic				
Left	14	19	32	47
Through	18	40	53	55
-				
Westbound Minor Street	152	121	102	95
Eastbound Minor Street	39	66	107	132
MAJOR STREET TRAFFIC Northbound				
Site-Generated Traffic				
Left	9	6	6	8
Through	3	2	2	2
Right	3	2	10	10
Background Traffic	400	400	404	4.40
Left	103 194	108 160	121 212	149 238
Through Right	194	29	41	236 51
Southbound	10	23	7.	31
Site-Generated Traffic				
Left	1	1	4	5
Through	1	1	1	1
Right	0	0	0	0
Background Traffic				
Left	5	12	15	15
Through	219	228	299	343
Right	49	32	41	39
Major Street Totals			400 5	F 00 0
	6:30-7:30 a.m.	7:30-8:30 a.m.	4:00-5:00 p.m.	5:00-6:00 p.m.
	597	581	752	861
Higher Minor Street	0.4	40	20	20
Site-Generated (vhp) Site-Generated (% of Total)	21 13.8%	13	22	30 22.7%
Total	13.8%	10.7% 121	20.6% 107	22.7% 132
. otai	102	121	107	102
Weighted Average		ı 17.€ 	60% 	
4-Hour Vehicular Volume Traffic				
Signal Warrant Threshold ⁽³⁾ Met?	91 YES	97 YES	65 YES	60 YES
Notes:				

Source: LSC Transportation Consultants, Inc.

⁽¹⁾ Includes original Filings 1, 2 and buildout of Walden Preserve 2

⁽²⁾ Based on Existing (2014) Traffic Volumes

(3) Manual on Uniform Traffic Control Devices, Figure 4C-2

Warrant 2, Four-Hour Vehicular Volume (70% Factor) (1 lane & 1 lane)

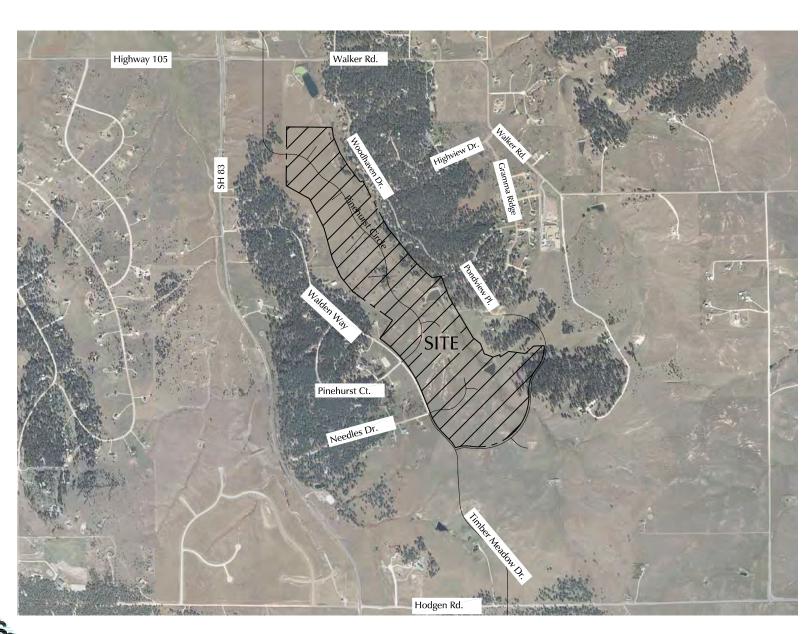
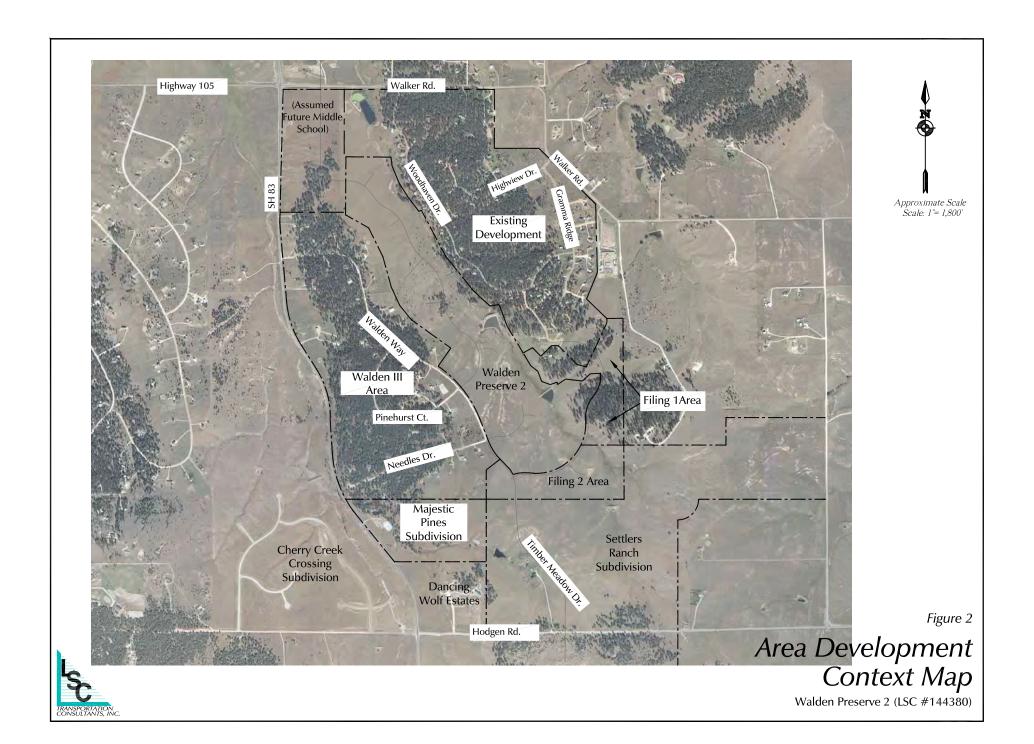
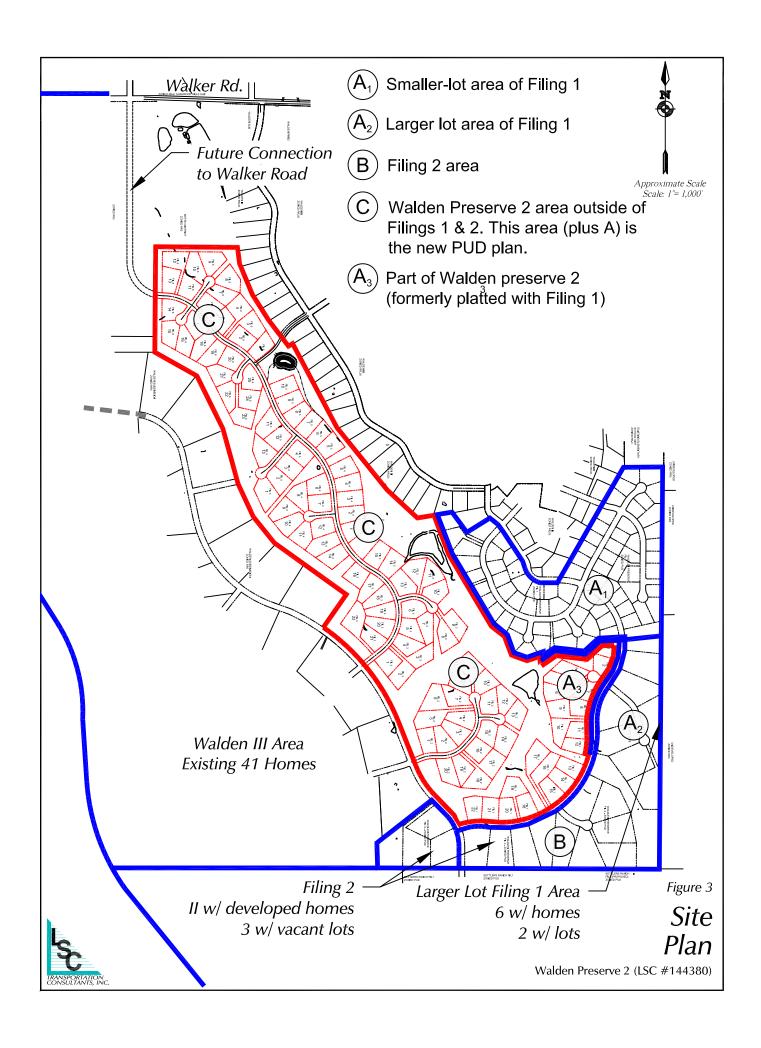
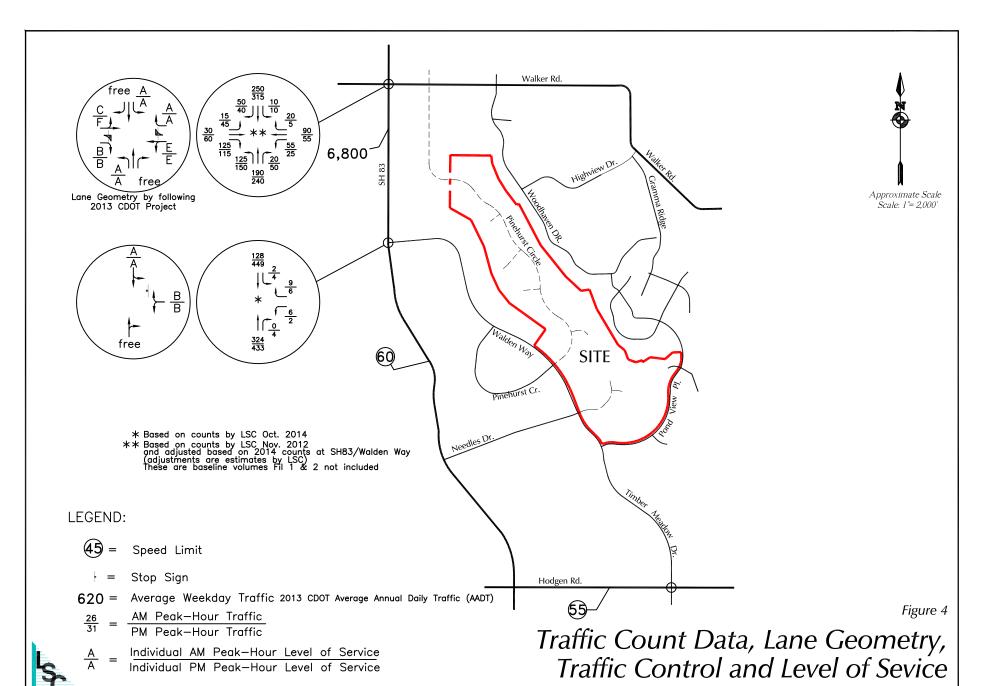


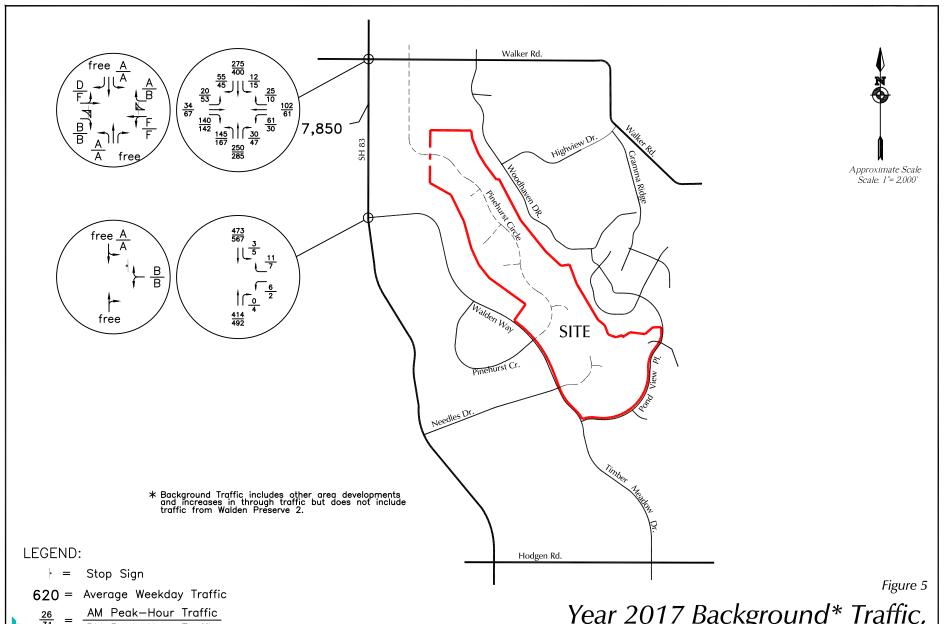


Figure 1 Vicinity
Map
Walden Preserve 2 (LSC #144380)



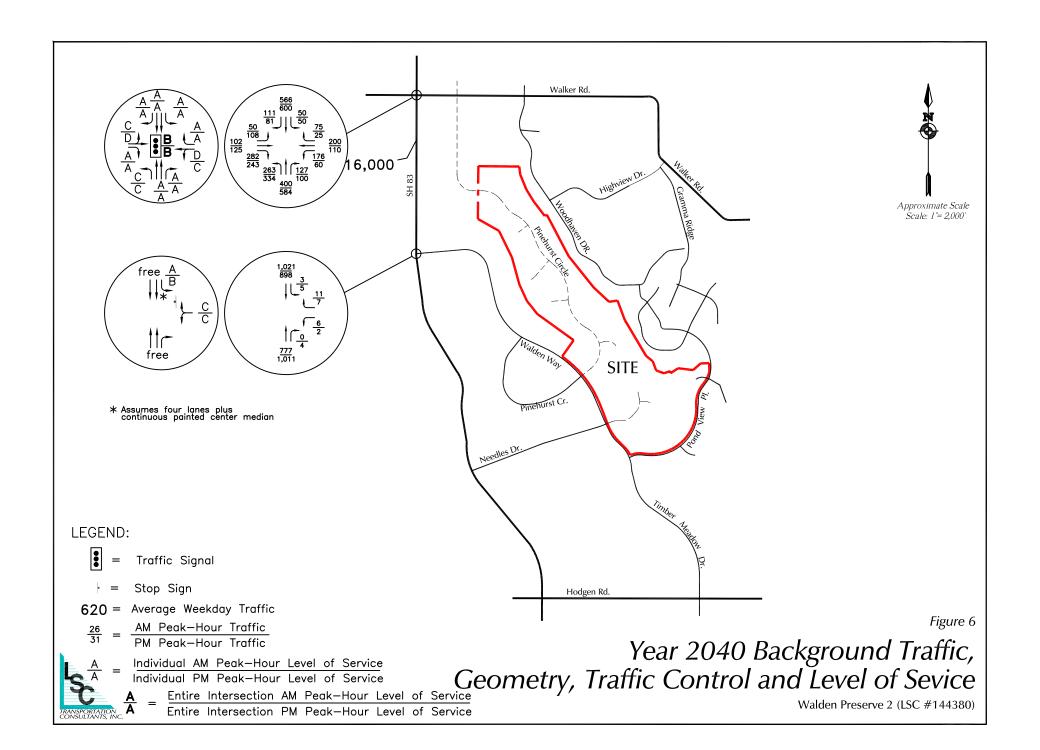


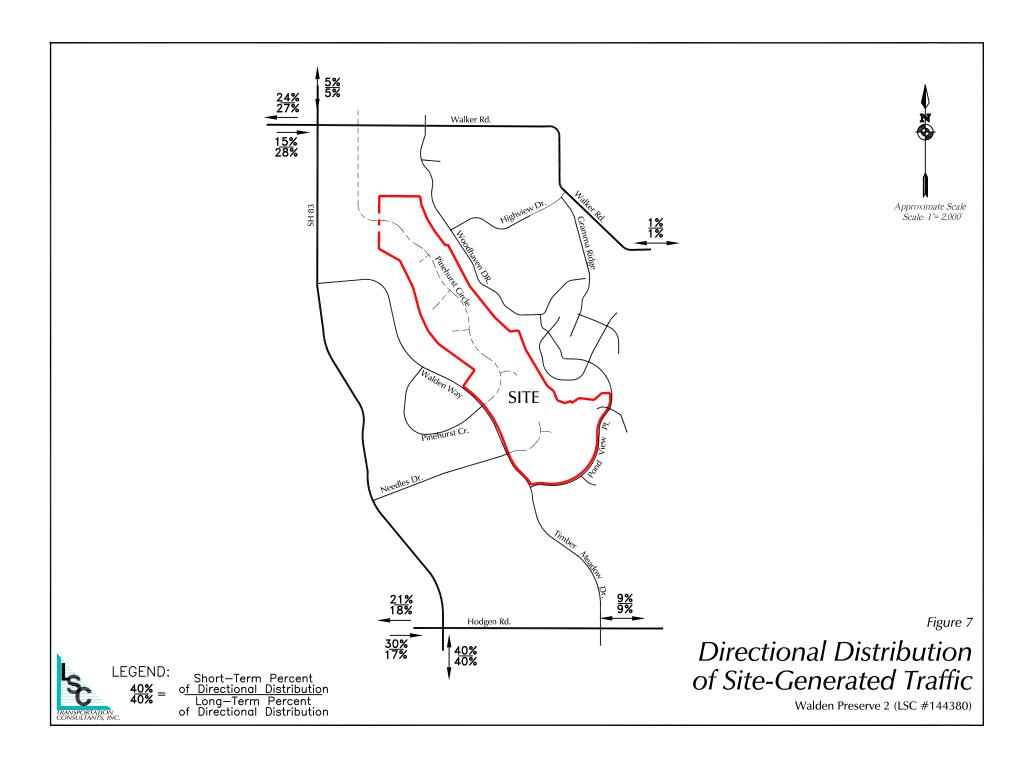


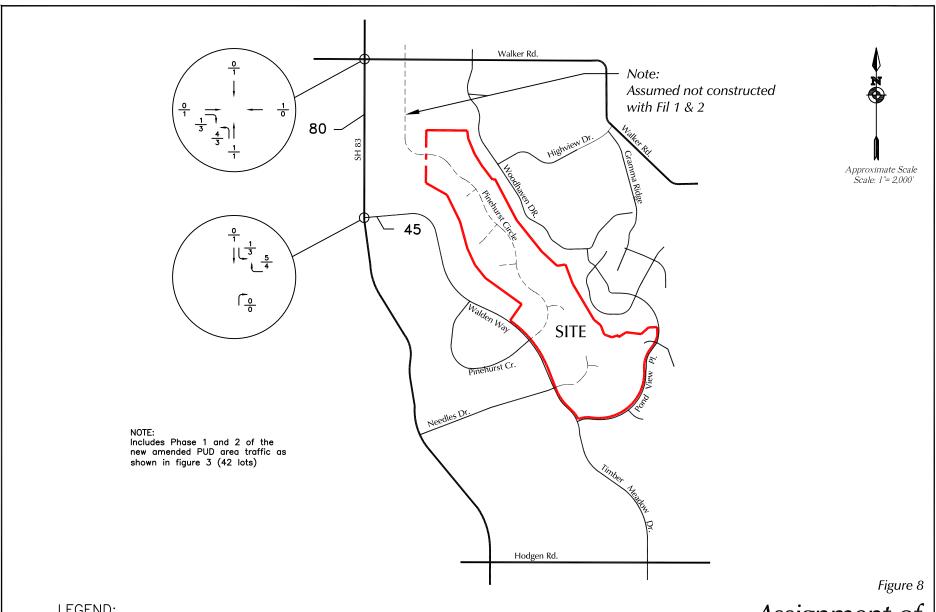


Individual AM Peak—Hour Level of Service Individual PM Peak—Hour Level of Service

Year 2017 Background* Traffic, Geometry, Traffic Control and Level of Sevice





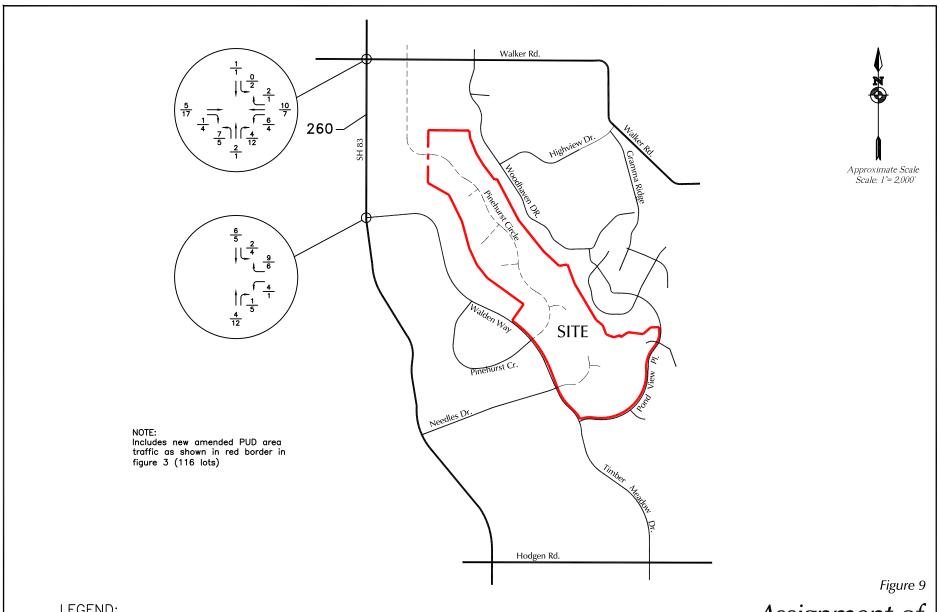


LEGEND:

620 = Average Weekday Traffic

AM Peak-Hour Traffic
PM Peak-Hour Traffic

Assignment of Phases 1 & 2 (Fil. 1 & 2) Site-Generated Traffic

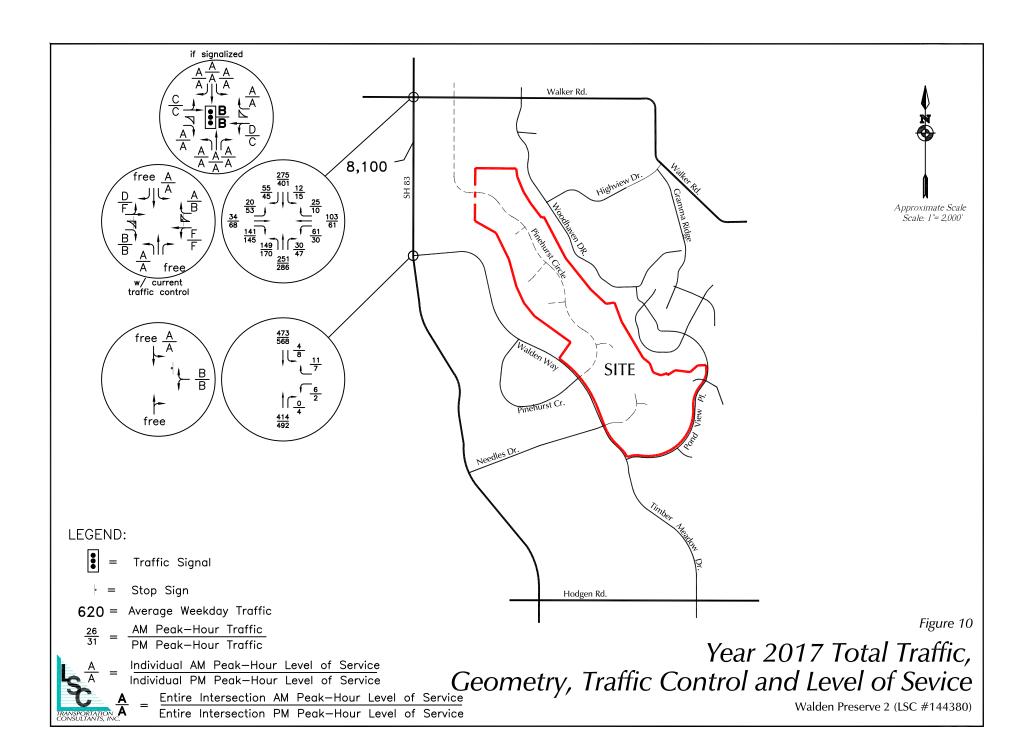


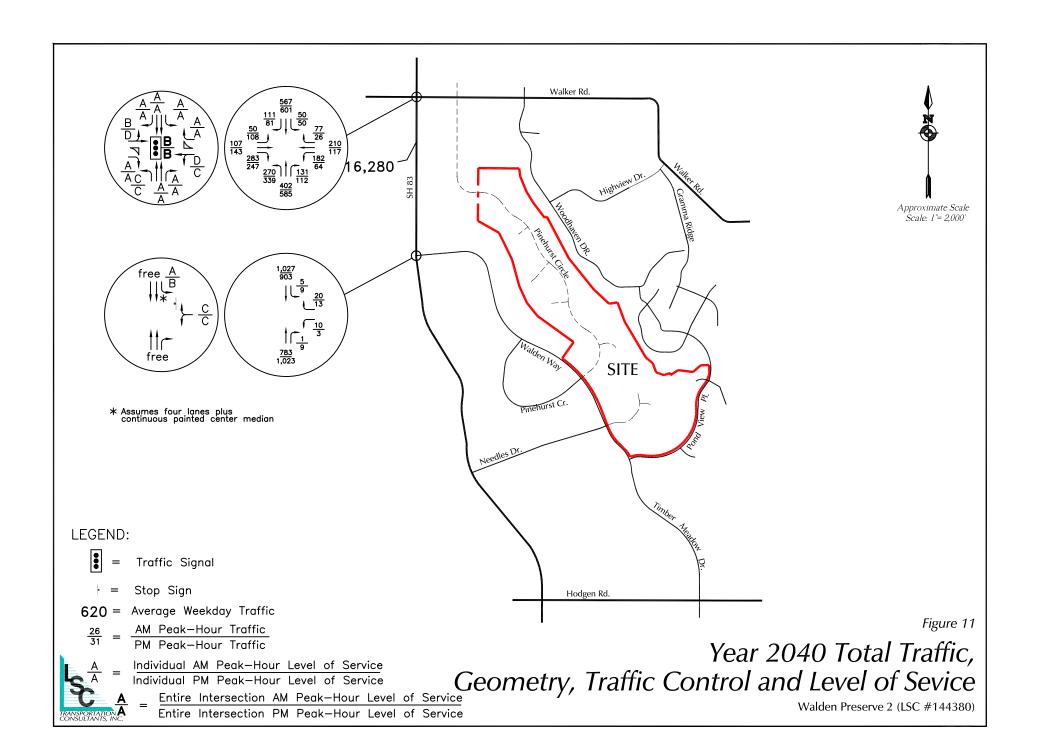
LEGEND:

620 = Average Weekday Traffic

AM Peak-Hour Traffic
PM Peak-Hour Traffic

Assignment of Buildout Site-Generated Traffic





LSC Transportation Consultants, Inc.

LSC Transportation Consultants, Inc.

516 N. Tejon St. Colorado Springs, CO (719) 633-2868

File Name: Hwy 83 - Walden Way AM2

Site Code : 00000000 Start Date : 10/08/2014

Page No : 1

Groups Printed- Unshifted

		Hwy	83			Walde			Hwy 83								
		-															
	~	From	Norm		From East				From South								
Start Time	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
06:30 AM	0	70	0	0	2	0	0	0	0	61	0	0	0	0	0	0	133
06:45 AM	0	95	. 1	0	2	0	. 0	0	0	77	0	0	0	0	0	0	175
Total	0	165	1	0	4	0	0	0	0	138	0	0	0	0	0	0	308
07:00 AM	0	105	0	0	4	0	0	0	0	70	0	0	0	0	0	0	179
07:15 AM	0	111	0	0	1	0	3	0	0	82	0	0	0	0	0	0	197
07:30 AM	0	113	1	0	1	0	3	0	0	92	0	0	0	0	0	0	210
07:45 AM	0	99	1	0	3	0	0	0	0	80	0	0	0	0	0	0	183
Total	0	428	2	0	9	0	6	0	0	324	0	0	0	0	0	0	769
			_														
08:00 AM	0	78	0	0	1	0	0	0	0	71	0	0	0	0	0	0	150
08:15 AM	0	98	2	0	1	0	0	0	0	71	0	0	0	0	0	0	172
Grand Total	0	769	5	0	15	0	6	0	0	604	0	0	0	0	0	0	1399
Apprch %	0.0	99.4	0.6	0.0	71.4	0.0	28.6	0.0	0.0	100. 0	0.0	0.0	0.0	0.0	0.0	0.0	
Total %	0.0	55.0	0.4	0.0	1.1	0.0	0.4	0.0	0.0	43.2	0.0	0.0	0.0	0.0	0.0	0.0	

LSC Transportation Consultants, Inc.

516 N. Tejon St.

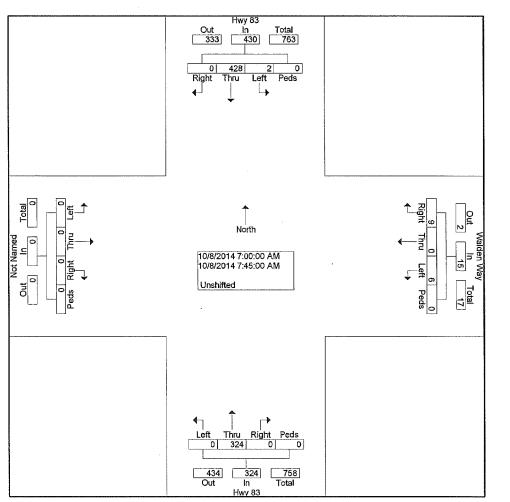
Colorado Springs, CO (719) 633-2868

File Name: Hwy 83 - Walden Way AM2

Site Code : 00000000 Start Date : 10/08/2014

Page No : 2

	1		lwy 8				Walden Way						Hwy 83									
			om No				From East						From South					From West				
Start	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Рe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Int.	
Time	ht	u	t	ds	Total	ht	u	t	ds	Total	ht	u	t l	ds	Total	ht	ш	t [ds	Total	Total	
Peak Hour	From	06:30	AM to	o 08:1	5 AM -	Peak	1 of	i														
Intersecti on	07:00	MA																				
Volume	0	42 8	2	0	430	9	0	6	0	15	0	32 4	0	0	324	0	0	0	0	0	769	
Percent	0.0	99. 5	0.5	0.0		60. 0	0.0	40. 0	0.0		0.0	10 0.0	0.0	0.0		0.0	0.0	0.0	0.0			
07:30 Volume	0	11 3	1	0	114	1	0	3	0	4	0	92	0	0	92	0	0	0	0	0	210	
Peak Factor																					0.915	
High Int.	07:30) AM				07:00	MA C				07:30	MAC				6:15	1A 00:	VI.				
Volume	0	11 3	1	0	114	4	0	0	0	4	0	92	0	0	92							
Peak					0.94					0.93					0.88							
Factor					3	[8					0							



LSC Transportation Consultants, Inc.

516 N. Tejon St.

LSC Transportation Consultants, Inc.

Colorado Springs, CO (719) 633-2868 File Name : Hwy 83 - Walden Way PM3 Site Code : 00000000

Site Code : 00000000 Start Date : 10/14/2014

Page No : 1

Groups Printed- Unshifted

		Hwy From			Walden Way From East			Hwy 83 From South			Walden Way From West						
Start Time	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Righ t	Thru	Left	Ped s	Int. Total
Factor	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	1.0	
04:00 PM	0	95	3	0	0	0	0	0	2	91	0	0	0	0	0	0	191
04:15 PM	0	91	0	0	1	0	0	0	2	120	0	0	0	0	0	0	214
04:30 PM	0	91	0	0	2	0	0	0	1	114	0	0	0	0	0	0	208
04:45 PM	0	129	0	0	1	0	0	0	3	117	0	0	0	0	0	0	250
Total	0	406	3	0	4	0	0	0	8	442	0	0	0	0	0	0	863
05:00 PM	0	102	1	0	0	0	1	0	1	111	0	0	0	0	0	0	216
05:15 PM	0	110	2	0	2	0	1	0	0	98	0	0	0	0	0	0	213
05:30 PM	0	108	1	0	3	0	0	0	0	107	0	0	0	0	0	0	219
05:45 PM	0	94	2	0	1	0	0	0	2	99	0	0	0	0	0	0	198
Total	0	414	6	0	6	0	2	0	3	415	0	0	0	0	0	0	846
Grand Total Apprch % Total %	0 0.0 0.0	820 98.9 48.0	9 1.1 0.5	0 0.0 0.0	10 83.3 0.6	0 0.0 0.0	2 16.7 0.1	0.0 0.0	11 1.3 0.6	857 98.7 50.1	0 0.0 0.0	0 0.0 0.0	0.0 0.0	0.0 0.0	0 0.0 0.0	0.0 0.0	1709

LSC Transportation Consultants, Inc.

516 N. Tejon St.

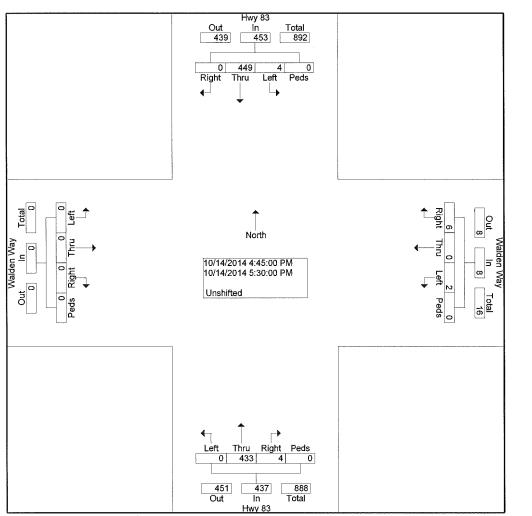
Colorado Springs, CO (719) 633-2868

File Name: Hwy 83 - Walden Way PM3

Site Code : 00000000 Start Date : 10/14/2014

Page No : 2

	Hwy 83 From North			Walden Way From East			Hwy 83 From South			Walden Way From West											
Start	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	App.	Rig	Thr	Lef	Pe	Арр.	Int.
Time	ht	u	t	ds	Total	ht	u	t	ds	Total	ht	u	t	ds	Total	ht	u	t	ds	Total	Total
Peak Hour	From (04:00	PM to	05:4	5 PM -	Peak	1 of 1	1													
Intersecti on	04:45	PM																			
Volume	0	44 9	4	0	453	6	0	2	0	8	4	43 3	0	0	437	0	0	0	0	0	898
Percent	0.0	99. 1	0.9	0.0		75. 0	0.0	25. 0	0.0		0.9	99. 1	0.0	0.0		0.0	0.0	0.0	0.0		
04:45 Volume Peak Factor	0	12 9	0	0	129	1	0	0	0	1	3	11 7	0	0	120	0	0	0	0	0	250 0.898
High Int.	04:45	PM				05:1	5 PM				04:4	5 PM				3:45	:00 Pi	√I			
Volume	0	12 9	0	0	129	2	0	1	0	3	3	11 7	0	0	120						
Peak Factor					0.87 8					0.66 7					0.91 0						





Region 2 Traffic Section 905 Erie Ave., P.O. Box 536 Pueblo, Colorado 81002 (719) 546-5407 Fax:(719) 562-5523

May 28, 2015

ATTN: Jeff Hodsdon LSC Transportation Consultants 516 North Tejon Street Colorado Springs, CO 80903

State Highway Access Permit No. 215017, Located on Highway 83, Milepost 28.0, in El Paso RE:

County

Dear Jeff,

The Colorado Department of Transportation (CDOT) has received your signed permit and application fee. A copy of the issued permit enclosed. CDOT has issued a Notice to Proceed for this permit since the permit did not require any additional construction of the access. Please keep a copy of the access permit and the notice to proceed for your files.

If you have any questions or need more information, please contact me at the office listed above.

Respectfully,

Valerie Sword

Region 2 Access Manager

XC: Andre Brackin, El Paso County

> Karami Lollar

Quintana/Patrol 21 Jagow/Lewis/file



COLORADO DEPARTMENT OF TRANSPORTATION

STATE HIGHWAY ACCESS CODE NOTICE TO PROCEED

CDOT Permit No.	2	215017					
SH/S/MP							
83 A /	28.000	/ R					
Local Jurisdiction							
El Paso County							

D-		:44 -	- 1	$\overline{}$	
re	rm	itte	er:	SI	•

El Paso County Public Svc Dept

Andre Brackin
3275 Akers Drive

Colorado Springs, CO 80922

Applicant:

LSC Transportation Consultants

Jeff Hodsdon

516 North Tejon Street

Colorado Springs, CO 80903

The permittee is hereby authorized to proceed with access construction within state highway right-of-way in accordance with the above referenced State Highway Access Permit and this Notice to Proceed.

This Notice to Proceed is valid only if the referenced Access Permit has not expired. Access Permits expire one year from date of issue if not under construction, or completed. Access Permits may be extended in accordance with Section 2.3(11)(d), of the Access Code.

Adequate advance warning is required at all times during access construction, in conformance with the Manual on Uniform Traffic Control Devices for Streets and Highways.

All construction shall be completed in an expeditious and safe manner and shall be finished within 45 days from initiation. The permittee or applicant shall notify the Department prior to commencing construction as indicated on the Access Permit.

Both the Access Permit and this Notice To Proceed shall be available for review at the construction site.

This Notice to Proceed is conditional. The following items shall be addressed prior to or during construction as appropriate.

No new construction required. A Letter of Credit has been received for \$39,996.20.

Municipality or County Approval (When the appropriate local authority retains issuing authority)							
By (X)	Title	Date					
This Notice is not valid until signed by a du Colorado Department of Transporta	•	nent					
(x) Vales is Inord	Title Access Mar	Date 5/28/2015					

Copy distribution:

Required:
Region (original)
Applicant
Staff Access Section

Make copies as necessary for: Local Authority Ins MTCE Patrol Tra Quintana/21

Inspector Todd Ausbun
Traffic Engineer

Form 1265 8/98, 6/99

COLORADO DEPARTMENT OF TRANSPORTATION CDOT Permit No. 215017 STATE HIGHWAY ACCESS PERMIT State Highway No/Mp/Side 83 A / 28,000 Pennit fee Date of transmittal Region/Section/Patrol Local Jurisdiction \$300.00 4/30/2015 2 / 04 / Quintana/21 El Paso County The Permittee(s): Ref No. Applicant: El Paso County Public Svc Dept LSC Transportation Consultants Andre Brackin Jeff Hodsdon 5275 Akers Drive 516 North Tejon Street Colorado Springs, CO 80922 Colorado Springs, CO 80903 719-520-6460 719-633-2868 is hereby granted permission to have an access to the state highway at the location noted below. The access shall be constructed, maintained and used in accordance with this permit, including the State Highway Access Code and any attachments, terms, conditions and exhibits. This permit may be revoked by the issuing authority if at any time the permitted access and its use violate any parts of this permit. The issuing authority, the Department and their duly appointed agents and employees shall be neld harmless against any action for personal injury or property damage sustained by reason of the exercise of the permit Location: Walden Preserve 2 Filings 1 and 2 Parcel is located East of Hwy 83 and South of HWY 50 (Walker Rd.) Access to Provide Service to: Land Lise Code -(Size or Count) (Units) 210 - Single-Family Detached Housing 42 EACH Additional Information: Escrow of \$39,996.20 for future signal at Walker Road is required. MUNICIPALITY OR COUNTY APPROVAL Required only when the appropriate local authority retains issuing authority Signature Print Name Date NDIZE P. BRACKIN COUNTY ENGINEER Upon the signing of this permit the permittee agrees to the terms and conditions and referenced attachments contained herein. All construction shall be completed in an expeditious and safe manner and shall be finished within 45 days from initiation. The permitted access shall be completed in accordance with the terms and conditions of the permit prior to being used. The permittee shall notify Todd Ausbun with the Colorado Department of Transportation in Pueblo, Colorado at (719) 696-1403, at least 48 hours prior to commencing construction within the State Highway right-of-way. The person signing as the permittee must be the owner or legal representative of the property served by the permitted access and have full authority to accept the permit and its terms and conditions Permittee Signature Pont Name Date This permit is not valid until signed by a duly authorized representative of the Department. COLORADO DEPARTMENT OF TRANSPORTATION Print Name Date (of issue

Copy Distribution:

Regunad 1 Region 2 Applicant

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MTCE Patrol

Inspector Traffic Engineer Previous editions are obsolete and may not be used Page 1 of 3 CDOT Form #101 5/07 State Highway Access Permit Form 101, Page 2

The following paragraphs are excerpts of the State Highway Access Code. These are provided for your convenience but do not alleviate compliance with all sections of the Access Code. A copy of the State Highway Access Code is available from your local issuing authority (local government) or the Colorado Department of Transportation (Department). When this permit was issued, the issuing authority made its decision based in part on information submitted by the applicant, on the access category which is assigned to the highway, what alternative access to other public roads and streets is available, and safety and design standards. Changes in use or design not approved by the permit or the issuing authority may cause the revocation or suspension of the permit.

APPEALS

- 1. Should the permittee or applicant object to the denial of a permit application by the Department or object to any of the terms or conditions of a permit placed there by the Department, the applicant and permittee (appellant) have a right to appeal the decision to the [Transportation] Commission [of Colorado]. To appeal a decision, submit a request for administrative hearing to the Transportation Commission of Colorado within 60 days of transmittal of notice of denial or transmittal of the permit for signature. Submit the request to the Transportation Commission of Colorado, 4201 East Arkansas Avenue, Denver, Colorado 80222-3400. The request shall include reasons for the appeal and may include changes, revisions, or conditions that would be acceptable to the permittee or applicant.
- 2. Any appeal by the applicant or permittee of action by a local issuing authority shall be filed with the local authority and be consistent with the appeal procedures of the local authority.
- 3. In submitting the request for administrative hearing, the appellant has the option of including within the appeal a request for a review by the Department's internal administrative review committee pursuant to [Code] subsection 2.10. When such committee review is requested, processing of the appeal for formal administrative hearing, 2.9(5) and (6), shall be suspended until the appellant notifies the Commission to proceed with the administrative hearing, or the appellant submits a request to the Commission or the administrative law judge to withdraw the appeal. The two administrative processes, the internal administrative review committee, and the administrative hearing, may not run concurrently.
- 4. Regardless of any communications, meetings, administrative reviews or negotiations with the Department or the internal administrative review Committee regarding revisions or objections to the permit or a denial, if the permittee or applicant wishes to appeal the Department's decision to the Commission for a hearing, the appeal must be brought to the Commission within 60 days of transmittal of notice of denial or transmittal of the permit.

PERMIT EXPIRATION

1. A permit shall be considered expired if the access is not under construction within one year of the permit issue date or before the expiration of any authorized extension. When the permittee is unable to commence construction within one year after the permit issue date, the permittee may request a one year extension from the issuing authority. No more than two one-year extensions may be granted under any circumstances. If the access is not under construction within three years from date of issue the permit will be considered expired. Any request for an extension must be in writing and submitted to the issuing authority before the permit expires. The request should state the reasons why the extension is necessary, when construction is anticipated, and include a copy of page 1 (face of permit) of the access permit. Extension approvals shall be in writing. The local issuing authority shall obtain the concurrence of the Department prior to the approval of an extension, and shall notify the Department of all denied extensions within ten days. Any person wishing to reestablish an access permit that has expired may begin again with the application procedures. An approved Notice to Proceed, automatically renews the access permit for the period of the Notice to Proceed.

CONSTRUCTION

- 1. Construction may not begin until a Notice to Proceed is approved. (Code subsection 2.4)
- 2. The construction of the access and its appurtenances as required by the terms and conditions of the permit shall be completed at the expense of the permittee except as provided in subsection 2.14. All materials used in the construction of the access within the highway right-of-way or on permanent easements, become public property. Any materials removed from the highway right-of-way will be disposed of only as directed by the Department. All fencing, guard rail, traffic control devices and other equipment and materials removed in the course of access construction shall be given to the Department unless otherwise instructed by the permit or the Department inspector.
- 3. The permittee shall notify the individual or the office specified on the permit or Notice to Proceed at least two working days prior to any construction within state highway right-of-way. Construction of the access shall not proceed until both the access permit and the Notice to Proceed are issued. The access shall be completed in an expeditious and safe manner and shall be finished within 45 days from initiation of construction within the highway right-of-way. A construction time extension not to exceed 30 working days may be requested from the individual or office specified on the permit.
- 4. The issuing authority and the Department may inspect the access during construction and upon completion of the access to ensure that all terms and conditions of the permit are met. Inspectors are authorized to enforce the conditions of the permit during construction and to halt any activities within state right-of-way that do not comply with the provisions of the permit, that conflict with concurrent highway construction or maintenance work, that endanger

highway property, natural or cultural resources protected by law, or the health and safety of workers or the public.

- 5. Prior to using the access, the permittee is required to complete the construction according to the terms and conditions of the permit. Failure by the permittee to abide by all permit terms and conditions shall be sufficient cause for the Department or issuing authority to initiate action to suspend or revoke the permit and close the access. If in the determination of the Department or issuing authority the failure to comply with or complete the construction requirements of the permit create a highway safety hazard, such shall be sufficient cause for the summary suspension of the permit. If the permittee wishes to use the access prior to completion, arrangements must be approved by the issuing authority and Department and included in the permit. The Department or issuing authority may order a halt to any unauthorized use of the access pursuant to statutory and regulatory powers. Reconstruction or improvement of the access may be required when the permittee has failed to meet required specifications of design or materials. If any construction element fails within two years due to improper construction or material specifications, the permittee shall be responsible for all repairs. Failure to make such repairs may result in suspension of the permit and closure of the access.
- 6. The permittee shall provide construction traffic control devices at all times during access construction, in conformance with the M.U.T.C.D. as required by section 42-4-104, C.R.S., as amended.
- 7. A utility permit shall be obtained for any utility work within highway right-of-way. Where necessary to remove, relocate, or repair a traffic control device or public or private utilities for the construction of a permitted access, the relocation, removal or repair shall be accomplished by the permittee without cost to the Department or issuing authority, and at the direction of the Department or utility company. Any damage to the state highway or other public right-of-way beyond that which is allowed in the permit shall be repaired immediately. The permittee is responsible for the repair of any utility damaged in the course of access construction, reconstruction or repair.
- 8. In the event it becomes necessary to remove any rightof-way fence, the posts on either side of the access shall be securely braced with an approved end post before the fence is cut to prevent any slacking of the remaining fence. All posts and wire removed are Department property and shall be turned over to a representative of the Department.
- 9. The permittee shall ensure that a copy of the permit is available for review at the construction site at all times. The permit may require the contractor to notify the individual or office specified on the permit at any specified phases in construction to allow the field inspector to inspect various aspects of construction such as concrete forms, subbase, base course compaction, and materials specifications. Minor changes and additions may be ordered by the Department or local authority field inspector to meet unanticipated site conditions.
- 10. Each access shall be constructed in a manner that shall not cause water to enter onto the roadway or shoulder, and shall not interfere with the existing drainage system on the

right-of-way or any adopted municipal system and drainage plan.

11. By accepting the permit, permittee agrees to save, indemnify, and hold harmless to the extent allowed by law, the issuing authority, the Department, its officers, and employees from suits, actions, claims of any type or character brought because of injuries or damage sustained by any person resulting from the permittee's use of the access permit during the construction of the access.

CHANGES IN ACCESS USE AND PERMIT VIOLATIONS

- 1. It is the responsibility of the property owner and permittee to ensure that the use of the access to the property is not in violation of the Code, permit terms and conditions or the Act. The terms and conditions of any permit are binding upon all assigns, successors-in-interest, heirs and occupants. If any significant changes are made or will be made in the use of the property which will affect access operation, traffic volume and or vehicle type, the permittee or property owner shall contact the local issuing authority or the Department to determine if a new access permit and modifications to the access are required.
- 2. When an access is constructed or used in violation of the Code, section 43-2-147(5)(c), C.R.S., of the Act applies. The Department or issuing authority may summarily suspend an access permit and immediately order closure of the access when its continued use presents an immediate threat to public health; welfare or safety. Summary suspension shall comply with article 4 of title 24, C.R.S.

MAINTENANCE

1. The permittee, his or her heirs, successors-in-interest, assigns, and occupants of the property serviced by the access shall be responsible for meeting the terms and conditions of the permit, the repair and maintenance of the access beyond the edge of the roadway including any cattle guard and gate, and the removal or clearance of snow or ice upon the access even though deposited on the access in the course of Department snow removal operations. Within unincorporated areas the Department will keep access culverts clean as part of maintenance of the highway drainage system. However, the permittee is responsible for the repair and replacement of any access-related culverts within the right-of-way. Within incorporated areas, drainage responsibilities for municipalities are determined by statute and local ordinance. The Department will maintain the roadway including auxiliary lanes and shoulders, except in those cases where the access installation has failed due to improper access construction and/or failure to follow permit requirements and specifications in which case the permittee shall be responsible for such repair. Any significant repairs such as culvert replacement, resurfacing, or changes in design or specifications, requires authorization from the Department,

- 1. A NOTICE TO PROCEED TO CONSTRUCTION, CDOT Form 1265, is required before beginning the construction of the access or any activity in the highway right-of-way. All submittals, documents, plans, and other items that must be completed shall be submitted and approved by the Department before a NOTICE TO PROCEED to construction will be issued.
- 2. The access is located on the east side of State Highway 83, at Walker Road or approximately milepost 28.13.
- 3. This section of highway is a Category R-A highway.
- 4. The Permittee/Applicant shall provide the Department with the following submittals, documents, plans and other items for review prior to the issuance of a NOTICE TO PROCEED to construction:
 - a) A written request for a NOTICE TO PROCEED including the access permit number listed above.
 - b) The Permittee/Applicant shall provide the Department with an Escrow document in the amount of \$39,996.20 for the future installation of a signal at Walker Road.
- 5. This Access Permit is issued to allow access to State Highway 83 for a change in use of the property. The previous use of the access was to serve the County road Walker Rd. The access will now serve Walker Rd and a 42-lot residential subdivision Walden Preserve 2 Filings 1 & 2.
- 6. No new construction or improvements are required by the issuance of this Access Permit.
- 7. The Permittee shall refer to all additional standard requirements attached to this permit. This includes CDOT Form 101b, enclosed additional terms, conditions, exhibits, and noted attachments.
- 8. The following criteria were used to establish this Access Permit:
 - a) The Application for Access Permit (CDOT Form 137) dated February 18, 2015 and accepted by the regional office on April 6, 2015 and all attachments.
 - b) State Highway Access Code, Volume 2, CCR-601-1; Effective date August 31, 1998
 - c) The State Highway Access Category Assignment Schedule, as revised.
 - d) The Colorado Department of Transportation (CDOT) M&S Standard Plans
 - e) Vicinity Map
 - f) Exhibit A, Traffic Signal Escrow Table prepared by LSC Transportation Consultants
 - g) Approved Traffic Report, signed and sealed by Jeff Hodsdon, PE #31684, dated November 3, 2015.
- 9. This Access Permit is issued in accordance with the 1998 State Highway Access Code (2CCR 601-1), and is based in part upon the information submitted by the Permittee. This Access Permit is only for the use and purpose stated in the Application and on the Permit. Any changes, based upon existing and/or anticipated future conditions in traffic volumes, drainage, types of traffic, or other operational aspects may render this permit void, requiring a new Application for Access Permit to be submitted for review by the Department and/or Issuing Authority.
- 10. If necessary, minor changes, corrections and/or additions to the Permit may be ordered by the Department Inspector, other Department representative, or the local authority, to meet unanticipated site conditions. Changes may not be in violation of the State Highway Access Code. All major changes to the permit must be approved in writing by the Department prior to commencement of any work on or within the State Highway right-of-way.
- 11. Backing maneuvers within and into the State Highway right-of-way are strictly prohibited. All vehicles shall enter and exit the highway right-of-way in a forward movement. Backing into the right-of-way shall be considered a violation of the Terms and Conditions of the Access Permit and may result in the revocation of the Permit by the Department and/or Issuing Authority.
- 12. This access will be allowed a full movement. However, left turn movements in and out of this access may be prohibited at some future date.
- 13. Any additional permits and clearances required by other Federal, State, Local Government Agencies or Ditch Companies is the responsibility of the Permittee and/or Applicant.

- 14. The Permittee is responsible for obtaining any necessary additional federal, state and/or local government agency permits or clearances required for construction of the access. Approval of this access permit does not constitute verification of this action by the Permittee.
- 15. All access permit requirements shall be met prior to the herein-authorized use of this access.
- 16. The Permittee is responsible for any utilities and/or traffic control devices disrupted by the construction of this access and all expense incurred for repair. There are existing utilities on the highway right-of-way by permit. Owners of those utilities must be contacted. Any work necessary to protect existing permitted utilities, such as encasements, bulwarks, etc. will be the responsibility of the Permittee.
 - a) The Permittee is hereby advised that other utilities may exist within the proposed permit area. Permittee shall implement any and all measures to protect any existing utilities from damage.
 - b) Non-Destructive Air-vacuum Excavation (potholing) to expose the utilities being surveyed to determine their exact depth and location maybe necessary before any work commences. A core hole saw cut is the recommended method of entry through pavement for potholing. Flowfill is required for backfill of the core hole under the pavement or on the roadway.
 - c) The vacuum excavation technique is used not only to expose utilities but also for other uses that are benefited by the non-invasive/non-destructive, environmentally friendly technology such as dewatering or drill fluid/saw cutting fluid removal.
 - d) The Contractor shall utilize a spotter to assist in the visual inspection of all excavation work as it progresses near existing CDOT intelligent Transportation Systems fiber optic line conduits, pull boxes and manholes. The Contractor shall provide a spotter to aid equipment operators when construction activities are near marked or unmarked fiber lines.
 - e) The spotter shall observe all excavation work as it progresses to ensure that no damage occurs to existing underground fiber lines. When the spotter has visual sight of the underground conduit, the spotter shall notify the equipment operator of the proximity to the conduit and begin to guide the excavation work. The spotter shall guide all excavation work around the conduit to ensure no damage occurs.
- 17. Additional CDOT permits are required for work involving water, sanitary sewer, gas, electrical, telephone and landscaping within the right-of-way.
- 18. The Permittee shall maintain adequate, unobstructed sight distance in both directions from the access. When determining the distance between accesses, the point of tangent shall be used where a radius is present, or the beginning of the curb cut. The minimum sight distance that shall be maintained along the highway for the access shall be 450 feet. The minimum sight distance that shall be maintained for the vehicle entering the highway shall be 550 feet.
- 19. Any landscaping or potentially obstructing objects such as but not limited to advertising signs, structures, trees, and bushes, shall be designed, placed, and maintained at a height not to interfere with the sight distance needed by any vehicle using the access. Planting of tree(s), which will be over 4 inches in caliper at maturity, will not be allowed within 30 feet of the edge of the traveled way. All other objects shall not exceed a total height of thirty inches from the top of final grade. The Department will require any object or landscaping that becomes unsightly or is considered to be a traffic hazard to be removed by the Permittee at no cost to the Department.
- 20. It is the responsibility of the Permittee to prevent all livestock from entering the State Highway right of way at this access location. Any livestock that does enter the highway right of way shall be the sole responsibility of the Permittee.
- 21. The access width, for an access without curbs, shall be measured exclusive of the radii or flares. The width of any non-traversal median is not counted as part of the access width. Only the travel portion is measured.

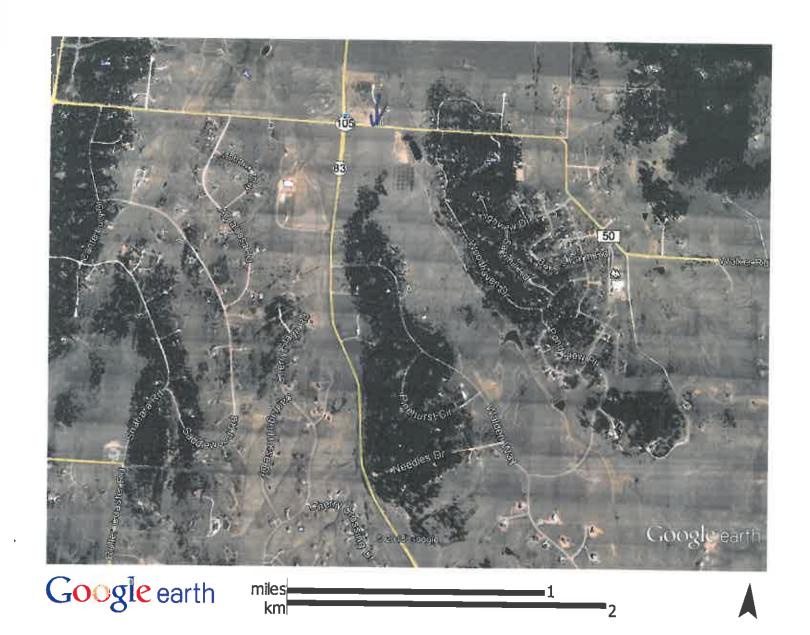
- 22. All discharges to the CDOT highway drainage system must comply with the applicable provisions of the Colorado Water Quality Control Act and the Colorado Discharge Permit Regulations, and are subject to inspection by the CDOT and CDPHE. CDOT recommends this development devise and implement a permanent plan for periodic removal and disposal of sediment from detention facilities and for maintenance of development detention facilities. Attached is the CDOT Environmental Clearances Information Summary listing some of the more commonly encountered environmental permits/clearances that may apply to activities and contacts for questions regarding these permits/clearances.
- 23. Within unincorporated areas, the Department will keep access culverts clean as part of maintenance of the highway drainage system. However, the Permittee is responsible for the repair and replacement of any access-related culverts within the right-of-way. Within incorporated areas, drainage responsibilities for municipalities are determined by statute and local ordinance.
- 24. The highway drainage system is for the protection of the state highway right-of-way, structures, and appurtenances. It is not designed nor intended to serve the drainage requirement of abutting or other properties beyond undeveloped historical flow. Drainage to the state highway right-of-way shall not exceed the undeveloped historical rate of flow.
- 25. All drainage appurtenances required for detention and release shall be located and fully maintainable cutside the highway right-of-way.
- 26. This Permit hereby replaces all previous access permit(s) for this ownership, which now become null and void.
- 27. CDOT retains the right to perform any necessary maintenance work in this area.
- 28. A "Notice to Proceed" (CDOT Form 1265) is required to complete the access permitting process, even when construction is not required.

Exhibit A

Traffic Signal Escrow Amounts State Highway 83/Walker Road Intersection

Walden Preserve 2 Subdivision

Filing	Number of Lots	Status	Portion of total cost estimate of \$61,600	
Original Walden Preserve Filing 1 81		Platted & Recorded	\$23,647.39	
Original Walden Preserve Filing 2	14	Platted & Recorded	\$4,087.20	
Walden Preserve 2 Filing No. 1	22	Approved; They will be recording ASAP. The county just needs a document from CDOT R2 Access evidencing compliance with CDOT requirements.	\$6,422.75	
Walden Preserve 2 Filing No. 2	20	Approved; They will be recording ASAP. The county just needs a document from CDOT R2 Access evidencing compliance with CDOT requirements.		
Subtotal - Current Access Permit for			\$5,838.86	
	THE LOCK PHEZ		\$39,996.20	
Future Filings			Future Amounts	
Walden Preserve 2 Filing No. 3	22	Bull to Bloom the same	\$6,422.75	
Walden Preserve 2 Filing No. 4 Walden Preserve 2 Filing No. 5	14	Preliminary Plan Approved; Plat not	\$4,087.20	
Walden Preserve 2 Filing No. 6	13 25	submitted yet.	\$3,795.26	
Subtotal - Fu			\$7,298.58	
Total			\$21,603.80 \$61,600.00	
Source: LSC Transportation Consultants, I		Di	ate: March 31, 2015	
Walden Preserve 2 lots 116	-	Total Signal Cost % by Walden Pres Cost to Walden	\$350,000.00 17.6% \$61,600.00	



Vicinity Map Access Permit #215017

COLORADO DEPARTMENT OF TRANSPORTATION STATE HIGHWAY ACCESS PERMIT APPLICATION

Issuing authority application 15 76 acceptance date:

- Contact the issuing a - Complete this form (Please print - Submit an applicatio	 Contact the Colorado Department of Transportation (CDOT) or your local government to determine your issuing authority? Contact the issuing authority to determine what plans and other documents are required to be submitted with your application. Complete this form (some questions may not apply to you) and attach all necessary documents and Submit it to the issuing authority. Submit an application for each access affected. If you have any questions contact the issuing authority. 								
- For additional informa	- For additional information see CDOT's Access Management website at http://www.dot.state.co.us/AccessPermits/Index.htm								
1) Property owner (Permittee) El Paso County Public Svc	DptAndre Brackin	2) Agent for permittee (if did LSC Transporat	fferent from proper tation Cons	tyowner) ultJeff F	Iodsdon				
Street address 3275 Akers Drive		Mailing address 516 North Tejo							
CO. Springs, CO. 80922	Phone # (719) 520-6460	City, state & zip CO. Springs, C	80903	Phone # (required (719) 633					
E-mail address AndreBrackin@ElPasoCo.com		E-mail address if available Jeff@LSCTrans.							
3) Address of property to be served by permit (re El Paso County Parcel Numb	per 6123001022								
4) Legal description of property: If within jurisdic county subdivision El Paso See Attach. Pla	block	and/or County, which one?	township	гапде					
5) What State Highway are you requesting acce SH 83A		6) What side of the highway	E\	w	_				
7) How many feet is the proposed access from the 253	<u> </u>	y feet is the proposed access							
8) What is the approximate date you intend to be 5/1/2015		feet 🗐N 🔲 S 🔲 E [_IW) from:	er aq.					
change in access use	new access temporary access (duration anticipated:								
10) Provide existing property use vacant land									
	e permit number(s) and provide co	opies:		a property interes or, permit date:	it?				
12) Does the property owner own or have any interpretation no yes, if yes - please description	terests in any adjacent property? cribe: Parcel 611500000				-				
13) Are there other existing or dedicated public s no yes, if yes - list them on		ss easements bordering or with	hin the property?						
14) If you are requesting agricultural field access N/A					-				
 If you are requesting commercial or industrial business/land use 	al access please indicate the type square footage	es and number of businesses busine		or area square foo	otage of each. square footage				
N/A									
16) If you are requesting residential developement			use) and number of	units?					
type Single Family Detached	number of units	type		-	number of units				
(Walden Preserve 2, Filin			-						
17) Provide the following vehicle count estimate					<u> </u>				
Indicate if your counts are peak hour volumes or average daily volur	# of passenger cars and light truck mes. 1	ks at peak hour volumes	# of multi unit trucks at	t peak hour volumes					
# of single unit vehicles in excess of 30 ft.	# of farm vehicles (field equipment)	Total count of all vehicles							

18) Check with the issuing authority to determine which of the following doc	suments are required to co	omplete the review of your	application.				
 a) Property map indicating other access, bordering roads and str b) Highway and driveway plan profile. c) Drainage plan showing impact to the highway right-of-way. d) Map and letters detailing utility locations before and after development in and along the right-of-way. 	f) Propose g) Parcel a h) Traffic s	ision, zoning, or developme ed access design. and ownership maps includ studies. f ownership.					
1- It is the applicant's responsibility to contact appropriate to their activities. Such clearances may include Corps of permits, or ecological, archeological, historical or cultural Information Summary presents contact information for agree prohibited discharges, and may be obtained from Region CDOT Planning/Construction-Environmental-Guidance well-construction with the State Highway right of way sna procedures, and all applicable U.S. Occupational Safety a limited to the applicable sections of 29 CFR Part 1910 - Construction and Health Regulations for Construction.	Engineers 404 Perroll resource clearance gencies administering that CDOT Utility/Sperebpage http://www.sull comply with their cand Health Administ	mits or Colorado Dises. The CDOT Envir ng certain clearances ocial Use Permit office c.dot.state.co.us/enverses employer's safety ar tration (OSHA) regul	scharge Permit System ronmental Clearances s, information about ses or accessed via the vironmental/Forms.asp. and health policies/ lations - including, but not				
Personal protective equipment (e.g. head protection, footwear, high visibility apparel, safety glasses, hearing protection, respirators, gioves, etc.) shall be worn as appropriate for the work being performed, and as specified in regulation. At a minimum, all workers in the State Highway right of way, except when in their vehicles, shall wear the following personal protective equipment: High visibility apparel as specified in the Traffic Control provisions of the documentation accompanying the Notice to Proceed related to this permit (at a minimum, ANSI/ISEA 107-1999, class 2); head protection that complies with the ANSI Z89.1-1997 standard; and at all construction sites or whenever there is danger of injury to feet, workers shall comply with OSHA's PPE requirements for foot protection per 29 CFR 1910.136, 1926.95, and 1926.96. If required, such footwear shall meet the requirements of ANSI Z41-1999.							
Where any of the above-referenced ANSI standards have apply.	e been revised, the r	most recent version	of the standard shall				
3- The Permittee is responsible for complying with the Revised Guidelines that have been adopted by the Access Board under the American Disabilities Act (ADA). These guidelines define traversable slope requirements and prescribe the use of a defined pattern of truncated domes as detectable warnings at street crossings. The new Standards Plans and can be found on the Design and Construction Project Support web page at: http://www.dot.state.co.us/DesignSupport/ , then click on Design Bulletins.							
If an access permit is issued to you, it will state the terms permitted access not consistent with the terms and condipermit.	If an access permit is issued to you, it will state the terms and conditions for its use. Any changes in the use of the permitted access not consistent with the terms and conditions listed on the permit may be considered a violation of the permit.						
The applicant declares under penalty of perjury in the second degree, and any other applicable state or federal laws, that all information provided on this form and submitted attachments are to the best of their knowledge true and complete.							
I understand receipt of an access permit does not cor	nstitute permission	to start access co	nstruction work.				
pplicant's signature	Print name		Date				
Jeffrey C. Hodsdon 1/18/15							
the applicant is not the owner of the property, we require this application also to be signed by the property owner or heir legally authorized representative (or other acceptable written evidence). This signature shall constitute agreement with this application by all owners-of-interest unless stated in writing. If a permit is issued, the property owner, in most cases, will be listed as the permittee.							

Print name

Andre P Brackin

Property owner signature

Feb. 9, 2015



IRREVOCABLE STANDBY LETTER OF CREDIT NO. 2015-3

DATE: May 15, 2015 AMOUNT: \$39,996.20

EXPIRATION DATE: None

TO: STATE OF COLORADO
DEPARTMENT OF
TRANSPORTATION
REGION 2 TRAFFIC AND
SAFETY
P.O. BOX 536
PUEBLO, CO 81002

RE: Colorado State Highway Access Permit No. 215017

ATTENTION: Valerie Sword. Access Manager, CDOT Region 2

We hereby issue an Irrevocable Standby Letter of Credit in your favor for the account of Custom Castles Building Company, Inc. for the development of Walden Preserve 2, Filing No. 1 and Filing No. 2 as per access permit No. 215017. Requests to draw on this letter of credit will require a written draft presented to us and must be accompanied with the following documents:

- 1. Your officially signed statement that Custom Castles Building Company, Inc.'s payment of the amount on the referenced access permit is due but unpaid after 30 days' notice to Custom Castles Building Company, Inc.
- 3. The original letter of credit issued by the undersigned bank.

The issuer shall not be in any way responsible for performance by any beneficiary of its obligations, nor for the form, sufficiency, correctness, genuineness, authority of any person signing, falsification or legal effect of any documents called for if such documents on their face appear in order.

This Letter of Credit is subject to the law and customs and practices of the trade existing in the area where the beneficiary is located, said Letter of Credit shall be subject to the Uniform Customs and Practice of Documentary Credits (1983 Revision,) International Chamber of Commerce, Publication No. 400.

Integrity Bank & Trust - Bank

By: Michael Casarez, Commercial Loan Officer

Monument · 1275 Village Ridge Point · Monument, CO 80132 · Phone 719.484.0077 · Fax 719.488.9160

Research & Powers · 5550 Powers Center Point · Colorado Springs, CO 80920 · Phone 719.495.3700 · Fax 719.282.8100

Northgate & Voyager · 13475 Voyager Parkway · Colorado Springs, CO 80921 · Phone 719.487.3034 · Fax 719.481.3521

Toll Free 877.677.2265 · Telephone Banking 877.317.2265 · www.integritybankandtrust.com

LSC TRANSPORTATION CONSULTANTS, INC.



516 North Tejon Street Colorado Springs, CO 80903 (719) 633-2868 FAX (719) 633-5430 E-mail: lsc@lsccs.com

May 15, 2015

Valerie Sword Region 2 Access Manager CDOT - Region 2 Traffic Section 905 Erie Avenue Pueblo, CO 81002



RE: Notice to Proceed Request Walden Preserve 2 Filings 1 & 2

LSC #144380

Dear Valerie:

The purpose of this letter is to request the Notice to Proceed (NTP) for Access Permit number 201017 along with the finalized access permit. Please find enclosed the signed access permit (signed by the permittee, the County Engineer), the permit fee, and the letter of credit from Integrity Bank. From our previous discussions, it is our understanding that a letter of credit would be acceptable to CDOT in lieu of a cash escrow to satisfy permit condition number 4b.

Regarding the letter of credit process, the following is our understanding. When CDOT starts a project to install the signal referenced in the access permit, assigns a project code to the project, and opens a project financial account, CDOT will send a letter requesting payment of the \$39,996.20 by Custom Castles Building Company, Inc., and Custom Castles Building Company, Inc. agrees to pay that amount within 30 days of receipt of that request. If CDOT receives that payment within that time, CDOT will return the letter of credit to Custom Castles Building Company, Inc. in exchange for the payment and deposit the funds in the signalization project account. If CDOT does not receive that payment within that time, but not otherwise, CDOT may draw on the letter of credit and will deposit the funds in the signalization project account.

Provided the attached letter of credit is in a form acceptable to CDOT, it is our understanding that the terms requirements for issuance of the NTP have been met. Please prepare and issue the final permit and NTP as soon as possible as these items are required for subdivision plat recordation.

Sincerely,

LSC TRANSPORTATION CONSULTANTS, INC.

Jeffrey C. Hodsdo

C. Hodsdon, P.E., PTOE

rrmeipai

JCH:bjwb

Enclosed: Executed Access Permit

Permit Fee Check

Letter of Credit from Integrity Bank

CUSTOM CASTLES BUILDING CO., INC. PH. 719-488-2598 1230 SCARSBROOK CT MONUMENT, CO 80132	6626 82-699/1070
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integrity BANK & TRUST PO. B	Martin Ment