FINAL DRAINAGE REPORT

for

WALDEN PRESERVE 2 – FILING NO. 4

Prepared for:

Walden Holdings I, LLC
1230 Scarsbrook Court
Monument, CO 80132

September 5, 2018
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Prepared by:

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JPS Project No. 040201
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FINAL DRAINAGE REPORT - EXECUTIVE SUMMARY

A. Background

- Walden Preserve is a residential subdivision of a 345-acre parcel located in northeastern El Paso County. Walden Preserve 2 Filing No. 4 consists of 23 residential lots in the northwesterly area of the Walden Preserve development.
- The proposed Filing No. 4 conforms to the previously approved Walden Preserve 2 PUD.
- Walden Preserve is located within the Cherry Creek Drainage Basin, which comprises a total drainage area in excess of 30 square miles. The Walden Preserve property represents less than 2 percent of the total basin area.

B. General Drainage Concept

- Developed drainage within the site will be conveyed through paved streets with roadside ditches and culverts, as well as grass-lined channels through open space areas.
- Developed flows from Walden Preserve will be detained to historic levels through on-site stormwater detention ponds.
- The existing tributary channel of West Cherry Creek running through the site will be preserved as a greenway and trail corridor. Main channel improvements have previously been completed with development of Walden Preserve Filing No. 1, including upgrade of two existing stock ponds.

C. Drainage Impacts

- The proposed detention ponds will release historic flows at the downstream property boundary, ensuring no significant adverse impact from developed drainage flows.
- Drainage facilities within public road rights-of-way and open space areas will be designed and constructed to El Paso County standards, and dedicated to the County for maintenance. The proposed stormwater detention ponds will be owned and maintained by the HOA or Metropolitan District.

DRAINAGE STATEMENTS

Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the County for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for liability caused by negligent acts, errors or omissions on my part in preparing this report.

____

John P. Schwab, P.E. #29891

Developer's Statement:

I, the developer have read and will comply with all of the requirements specified in this drainage report and plan.

By:

____

Printed Name: Date

Title:

Custom Castles Building Company, Inc.
1230 Scarsbrook Court
Monument, CO 80132

El Paso County's Statement:

Filed in accordance with the requirements of the El Paso County Land Development Code, Drainage Criteria Manual, Volumes 1 and 2, and Engineering Criteria Manual as amended.

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Jennifer Irvine, P.E. Date

County Engineer / ECM Administrator

Conditions:

I. GENERAL LOCATION AND DESCRIPTION

A. Background

Walden Preserve is a residential subdivision located in the Walden community of northeastern El Paso County, Colorado. The Walden Preserve 2 PUD area is located south and west of Woodhaven Drive, north of Pond View Place, and east of Walden Way, as shown in Figure A1 (Appendix F). The originally approved 2004 Walden Preserve PUD consisted of a combination of medium-density residential lots (0.5-acre average size) and low-density residential lots (2.5-acre minimum size) on a 345-acre property. The currently approved 2014 Walden Preserve 2 PUD includes phased development of 116 new residential lots (1-acre typical lot size), resulting in a total of 211 lots in the Walden Preserve development (including both previously platted lots and proposed lots). All of the proposed lots in the amended PUD area will be served by the existing central water and sewer system.

Walden Preserve 2 Filings No. 1-3 have been completed. These filings comprise 43 lots on the south side of the Walden Preserve 2 PUD area.

The proposed Walden Preserve 2 Filing No. 4 consists of 23 residential lots on 45.3 acres within the previously approved Walden Preserve 2 PUD (part of El Paso County Assessor's Parcel Number 61230-01-023), consisting of the existing meadow area located along the proposed extension of Pinehurst Circle on the east side of Walden Way.

B. Scope

This report is intended to fulfill the El Paso County requirements for a Final Drainage Report (FDR) in support of the final plat application for the proposed Walden Preserve 2 Filing No. 4. The report will provide a summary of site drainage issues impacting the proposed development, including analysis of impacts from upstream drainage areas, site-specific developed drainage patterns, and impacts on downstream facilities. This Final Drainage Report was prepared based on the guidelines and criteria presented in the El Paso County Drainage Criteria Manual.

C. Site Location and Description

The Walden Preserve parcel is located in parts of Sections 14, 15, 22, and 23, Township 11 South, Range 66 West of the 6th Principal Meridian. The site has historically been a vacant forest and meadow tract, with the exception of existing utility facilities (i.e. wastewater lagoon and well sites) serving the Walden Subdivision. The Walden Preserve property was re-zoned to PUD in 2005, with an approved development plan including a band of low-density lots along the south and west sides of the parcel, transitioning to medium-density lots adjacent to the existing platted areas of the Walden Subdivision, which is zoned RR1 for ½-acre lots.

Access to the areas on the west side of the Walden Preserve will be provided by extension of several existing local roads from Walden Way, including Needles Drive, and Pinehurst Circle. Associated site improvements will include paving of new public roadways through the site, as well as grading, drainage, and utility service improvements for the proposed residential lots. Interior roads will all be classified as rural local roads, with 60-feet rights-of-way and paved widths of 28-feet.

The Walden Preserve parcel is bordered by Walker Road to the north, developed areas of the Walden III Subdivision (Zoned RR1) to the east, the Settlers Ranch Subdivision to the south, and developed areas of the Walden Subdivision (Zoned RR5) to the west. Ground elevations within the parcel range from a low point of approximately 7,350 feet above mean sea level at the north boundary of the parcel, to a high point of 7,600 feet at the south boundary.

Surface drainage from this area flows northerly towards a tributary of West Cherry Creek on the west side of Woodhaven Drive. The terrain is rolling with south to north slopes ranging from 1% to 10%. Existing vegetation is typical eastern Colorado prairie grass in the meadow areas and evergreen pines in the forest areas towards the easterly property boundary.

D. General Soil Conditions

According to the Soil Survey of El Paso County prepared by the Soil Conservation Service, the majority of the parcel is classified as "Tomah" series loamy sand, and characterized as hydrologic soils group B. On-site soils are comprised primarily of the following soil types (see Appendix A):

- Type 92 - "Tomah-Crawfoot loamy sands": slow surface runoff, slight to moderate erosion hazard (Hydrologic Group B)
- Type 71 – "Pring sandy loam": medium surface runoff, moderate erosion hazard (Hydrologic Group B)
- Type 25 – "Elbeth sandy loam": slow to medium surface runoff, moderate erosion hazard (Hydrologic group B)

E. References

City of Colorado Springs & El Paso County "Drainage Criteria Manual," revised October 12, 1994.

City of Colorado Springs "Drainage Criteria Manual, Volumes 1 and 2," revised May, 2014.

CDOT, "CDOT Drainage Design Manual," July, 1995.

El Paso County "Engineering Criteria Manual," January 9, 2006.

ERO Resources Corporation, "Wetland Delineation Report for Walden Village," March 4, 2003.

Guenther Polok, "Drainage Report, Walden III, Filings 5, 6, and 7," July, 1983.

JPS Engineering, Inc., "Final Drainage Report for Settlers Ranch Subdivision Filing No. 1," October 18, 2005 (approved October 19, 2005).

JPS Engineering, Inc., "Final Drainage Report for Settlers Ranch Subdivision Filing No. 2," May 30, 2008 (approved March 31, 2009).

JPS Engineering, Inc., "Final Drainage Report for Walden Preserve Subdivision Filing No. 1," May 11, 2005.

JPS Engineering, Inc., "Master Development Drainage Plan (MDDP) and Final Drainage Report for Monument Academy," April 30, 2019.

JPS Engineering, Inc., "Master Development Drainage Plan (MDDP) and Preliminary Drainage Report for Walden Preserve Subdivision," December 10, 2004.

JPS Engineering, Inc., "Master Development Drainage Plan (MDDP) and Preliminary Drainage Report for Walden Preserve 2 PUD," September 17, 2014 (approved November 6, 2014).

JPS Engineering, Inc., "Preliminary Drainage Report for Walden Pines Subdivision," December 29, 2003.

JPS Engineering, Inc., "Final Drainage Report for Walden Pines Subdivision," March 24, 2004.

Kiowa Engineering Corporation, "Final Drainage Plan and Erosion Control Plan, Walden III Filing 6 Phase 1, Filing 6 Phase 2 and Filing 7," February 19, 2002.

M.V.E., Inc., "Settlers Ranch Preliminary Drainage Report," December 13, 2004.

USDA/NRCS, "Soil Survey of El Paso County Area, Colorado," June, 1981.

II. DRAINAGE BASINS AND SUB-BASINS

A. Major Basin Description

The proposed development lies within the West Cherry Creek Drainage Basin (CYCY 0400) as classified by El Paso County. Drainage from this site flows northerly to an eastern tributary of West Cherry Creek, which flows to a confluence with the main channel north of Walker Road. Downstream agricultural areas generally drain northerly towards the main channel of West Cherry Creek.

The major drainage basins lying in and around the proposed development are depicted in Figure EX1. The Walden Preserve parcel is located near the southerly limits of the West Cherry Creek Basin, which comprises a total drainage area in excess of 30 square miles. As such, the proposed 349-acre Walden Preserve subdivision represents less than 2 percent of the total basin area, which is primarily agricultural land.

B. Floodplain Impacts

The proposed development area is located beyond the limits of any 100-year floodplain delineated by the Federal Emergency Management Agency (FEMA). The floodplain limits in the vicinity of the site are shown in Flood Insurance Rate Map (FIRM) Numbers 08041C0285-F and 08041C0325-F, dated March 17, 1997, as shown in Figure A2 (Appendix F). As shown on Figure A2, the FEMA floodplain limit extends slightly south of Walker Road in the vicinity of the existing culvert crossing.

No impacts to the existing drainage channel are proposed as part of this subdivision filing.

C. Sub-Basin Description

The existing drainage basins lying in and around the proposed development are depicted in Figure EX1 (Appendix F). The site is impacted by off-site drainage basins of approximately 515 acres to the south and east, which flow to the existing natural drainage channel that bisects the property. The existing on-site topography has been delineated as five drainage basins. Basins A, B, and C drain into the main tributary channel flowing northerly to the existing culvert crossing Walker Road.

The developed drainage basins lying within the proposed development are depicted in Figures D1 and D1.03 (Appendix F). The interior site layout has been divided into drainage basins based on the road layout and topography within the site. The natural drainage patterns will be impacted through development by site grading and concentration of runoff in subdivision roadside ditches and channels. The majority of sub-basins drain to the north, collecting in the interior roads and drainage channels. On-site flows will be diverted to natural swales draining towards the main tributary channel running through the property, following historic drainage paths.

III. DRAINAGE DESIGN CRITERIA

A. Development Criteria Reference

No Drainage Basin Planning Study (DBPS) has been completed for the West Cherry Creek Drainage Basin. JPS Engineering, Inc. prepared the "Master Development Drainage Plan (MDDP) and Preliminary Drainage Report for Walden Preserve 2 PUD" dated September 17, 2014, which was approved by El Paso County on November 6, 2014 in support of the Walden Preserve 2 PUD and Preliminary Plan. The proposed drainage plan for Walden Preserve 2 Filing No. 4 is in full conformance with the previously approved MDDP.

B. Hydrologic Criteria

SCS procedures were utilized for analysis of the major off-site basin flows impacting the site. In accordance with El Paso County drainage criteria, SCS hydrologic calculations were based on the following assumptions:

• Design storm (minor)	5-year
• Design storm (major)	100-year
• Storm distribution	SCS Type II
• 100-year, 24-hour rainfall	4.4 inches (NOAA isopluvial map)
• 5-year, 24-hour rainfall	2.6 inches (NOAA isopluvial map)
• Hydrologic soil type	B
• SCS curve number - undeveloped conditions	61 (pasture / range)
• SCS curve number – 1-ac to 2.5-ac lots	68
• SCS curve number – 0.5-acre lots	70

Rational Method procedures were utilized for calculation of peak flows within the on-site drainage basins. Rational Method hydrologic calculations were based on the following assumptions:

• Design storm (minor)	5-year	
• Design storm (major)	100-year	
• Rainfall Intensities	El Paso County I-D-F Curve	
• Hydrologic soil type	B	
	<u>C5</u>	<u>C100</u>
• Runoff Coefficients - undeveloped:		
Existing pasture/range areas	0.25	0.35
Existing forest areas	0.1	0.15
• Runoff Coefficients - developed:		
Proposed lot areas (1-2.5-acre lots)	0.30	0.40
Proposed lot areas (1/2-acre lots)	0.35	0.45

To provide for conservative drainage design, the developed flow hydrologic calculations were performed using conservative assumptions for runoff curve numbers and coefficients associated with the 1-acre lots proposed throughout the Walden Preserve 2 PUD. For example, the SCS curve number of 68 for 1-acre lots has been applied without reduction to the majority of developed areas throughout the Walden Preserve 2 PUD, while in fact there are significant open space areas in the subdivision layout. As noted in Table 5-5 of Appendix B, the SCS curve number of 68 is recommended for densities of 1 acre per dwelling unit. The Walden Preserve 2 PUD has an actual gross density of approximately 1.8 acres per dwelling unit, providing for a significant factor of safety in the subdivision drainage design.

Hydrologic calculations are detailed in Appendix B, and peak design flows are identified on the drainage plan drawings in Appendix F.

IV. DRAINAGE PLANNING FOUR STEP PROCESS

El Paso County Drainage Criteria require drainage planning to include a Four Step Process for receiving water protection that focuses on reducing runoff volumes, treating the water quality capture volume (WQCV), stabilizing drainageways, and implementing long-term source controls. The Four Step Process is applicable to all new and re-development projects with construction activities that disturb 1 acre or greater or that disturb less than 1 acre but are part of a larger common plan of development. The Four Step Process has been implemented as follows in the planning of this project:

<u>Step 1: Employ Runoff Reduction Practices</u>

- Minimize Directly Connected Impervious Areas (MDCIA): The rural residential development will have roadside ditches along all roads, providing for impervious areas to drain across pervious areas. Based on the roadside ditches throughout the subdivision, the subdivision is classified as MDCIA Level One.
- Grass Swales: The proposed roadside ditches will drain to existing and proposed grass-lined drainage swales following historic drainage patterns through the property.

<u>Step 2: Stabilize Drainageways</u>

- Proper erosion control measures will be implemented along the roadside ditches and grass-lined drainage channels to provide stabilized drainageways within the site.

<u>Step 3: Provide Water Quality Capture Volume (WQCV)</u>

- Detention Ponds and Rain Gardens: The developed site will drain through stormwater detention ponds and rain gardens which will capture and slowly release the WQCV over an extended release period.

<u>Step 4: Consider Need for Industrial and Commercial BMPs</u>

- No industrial or commercial land uses are proposed within this rural residential subdivision.
- On-site drainage will be routed through private detention ponds and rain gardens to minimize introduction of contaminants to the County's public drainage system.

V. GENERAL DRAINAGE RECOMMENDATIONS

The developed drainage plan for the site is to provide and maintain positive drainage away from structures and conform to the established drainage patterns for the overall site. JPS Engineering recommends that positive drainage be established and maintained away from all structures within the site in conformance with applicable building codes and geotechnical engineering recommendations.

Site grading and drainage improvements performed as a part of subdivision infrastructure development includes overlot grading and subdivision drainage improvements depicted on the subdivision construction drawings. Individual lot grading is the sole responsibility of the individual builders and property owners. Final grading of each home site should establish proper protective slopes and positive drainage in accordance with HUD guidelines and building codes. In general, main floor elevations for each home should be established approximately 2 feet above the top of curb of the adjoining street.

In general, we recommend a minimum of 6 inches clearance from the top of concrete foundation walls to adjacent finished site grades. Positive drainage slopes should be maintained away from all structures, with a minimum recommended slope of 5 percent for the first 10 feet away from buildings in landscaped areas, a minimum recommended slope of 2 percent for the first 10 feet away from buildings in paved areas, and a minimum slope of 1 percent for paved areas beyond buildings.

VI. DRAINAGE FACILITY DESIGN

A. General Concept

Development of the Walden Preserve Subdivision will require site grading and paving, resulting in additional impervious areas throughout the site. The general drainage pattern will consist of grading away from home sites to swales and roadside ditches along the internal roads within the subdivision, conveying runoff flows through the site. Runoff from the site will flow by roadside ditches to cross culverts at low points in the road profiles, and grass-lined channels connecting to existing natural swales at the site boundaries.

The stormwater management concept for the Walden Preserve development will be to provide roadside ditches and grass-lined swales as required to convey developed drainage through the site to existing natural outfalls. Individual lot grading will provide positive drainage away from building sites, and direct developed flows into the system of roadside ditches and drainage swales running through the subdivision.

One of the existing stock ponds along the main channel within the site (“Pond B”) has previously been upgraded to serve as a stormwater retention pond to mitigate the impact of developed flows and ensure that historic flows are maintained downstream of the proposed subdivision. Development of the balance of the western part of the Walden Preserve property will ultimately include construction of one additional detention pond (“Pond C8”), along with other low impact development approaches to further mitigate developed flow impacts.

B. Specific Details

1. Existing Drainage Conditions

Historic drainage conditions within the site are depicted in Figure EX2. The existing tributary channel of West Cherry Creek runs northerly through the site, collecting drainage from numerous natural swales on each side of the main channel.

Off-site flows from Basin OA1 combine with Basin A at the south end of the site, flowing into an existing stock pond ("Pond A") at Design Point #1, with historic peak flows of Q_5 = 33.1 cfs and Q_{100} = 213.4 cfs (SCS Method).

Off-site flows from Basin OB1 combine with flows from Basin B near the center of the existing parcel, adding to the flow in the main tributary channel at the second existing stock pond ("Pond B") located at Design Point #2. Historic peak flows at this location are estimated to be Q_5 = 65.8 cfs and Q_{100} = 287.9 cfs (SCS Method).

Off-site flows from Basins OC1 and OD1 combine with flows from Basins C and D, joining with flows in the main tributary channel at the existing culvert crossing Walker Road at the northerly property boundary of the Walden Wastewater Treatment Plant property. Historic peak flows at Design Point #4 are calculated as Q_5 = 222.5 cfs and Q_{100} = 582.9 cfs (SCS Method).

2. Developed Drainage Conditions

<u>Master Development Drainage Plan</u>

The developed drainage basins and projected flows are shown in Figures D1 and D1.03 (Appendix F). As discussed in the approved MDDP, developed Sub-basins A1-A6 are located in the south and southeasterly areas previously developed as Walden Preserve Filings No. 1 and No. 2. Walden Preserve 2 Filings No. 1-3 included development of the areas within Sub-Basins A7-A9 on the north side of Pond View Place between the main drainage channel and Walden Way.

Sub-basins A1-A9 will continue to flow northerly towards the main tributary channel, combining at the existing southerly stock pond (Pond A) with developed peak flows calculated as Q_5 = 92.1 cfs and Q_{100} = 261.3 cfs (SCS Method) at Design Point #1 (per approved MDDP). While the existing Stock Pond A is not planned for use as a drainage detention pond, the pond will be maintained as an aesthetic feature within the subdivision. The pond was previously upgraded with an engineered overflow spillway during development of Walden Preserve Filing No. 1.

Walden Preserve Filing No. 1 included development of the areas within Sub-basins B1-B4 on the east side of the main drainage channel. Sub-basins B5-B10 include the remaining areas developed as 1-acre lots along the west side of the drainage channel.

Developed Sub-basins B1-B10 will continue to flow northerly towards the main tributary channel, combining at the existing northerly stock pond (Pond B) with developed peak flows of Q_5 = 208.0 cfs and Q_{100} = 551.4 cfs (SCS Method) at Design Point #2 (per approved MDDP). The existing stock pond (Pond B) has been upgraded to serve as a sub-regional stormwater detention and water quality pond for the remaining phases of the Walden Preserve Subdivision. Construction plans for Walden Preserve 2 Filing No. 1 included addition of a water quality orifice plate on the Pond B outlet structure, but the outlet structure improvements have not yet been completed. The County has now requested additional improvements to the Pond B outlet structure to meet current full-spectrum detention design standards. The proposed improvements to the Pond B outlet structure are detailed in an Addendum to the Walden Preserve 2 Filing No. 1 Final Drainage Report (enclosed in Appendix D1).

The pond has been designed to "over-detain" to allow for discharges of developed flows from downstream sub-basins, while ensuring that discharges downstream of Walker Road remain below historic levels. An energy dissipation structure has been constructed at the discharge point from Pond B to reduce erosion concerns in the downstream channel.

Sub-basins C1-C4 include areas north of Pond B which will be developed as 1-acre lots. Runoff from these basins will flow northeasterly to the main drainage channel. Proposed Water Quality Ponds C2 and C4 will be constructed on the downstream side of the new extension of Pinehurst Circle through the Walden Preserve 2 PUD area. While Ponds C2 and C4 are not needed for stormwater detention in the overall drainage analysis, these small water quality ponds will encourage infiltration of developed drainage and provide water quality mitigation of developed drainage impacts, consistent with an overall low-impact development approach.

Sub-basins C5-C8 include additional areas to be developed as 1-acre lots. These basins will flow northeasterly to the future Detention Pond C8, which will mitigate developed flow impacts prior to discharging through Basin C9 into the main channel upstream of the proposed emergency access and trail crossing at Highview Drive. Total undetained developed peak flows at Design Point #3 are calculated as Q_5 = 280.7 cfs and Q_{100} = 734.2 cfs (SCS Method; per approved MDDP).

Sub-basins C10-C12 include additional 1-acre lot areas to be developed in the northwest part of Walden Preserve, and Sub-basin C13 covers the existing Walden Wastewater Treatment Facility at the northwest corner of the Walden property. Water Quality Pond C12 will be constructed in the northeast corner of the future residential development area to mitigate developed flow impacts from Sub-Basins C10-C12. Developed flows from Sub-basins C10-C13 will flow northeasterly, combining with flows in the main tributary channel, and ultimately reaching the existing culvert crossing Walker Road.

Flows from Basins OC1-OC2 on the east side of the channel, along with flows from Basins OD1 and D in vicinity of the Walden Pines Subdivision drain northwesterly across Woodhaven Drive, contributing to the total flow in the main channel. Flows from Basins OA1-OD1 and A1-D1 combine at Design Point #4, with total undetained developed flows calculated as Q_5 = 298.7 cfs and Q_{100} = 785.0 cfs (SCS Method; per approved MDDP).

As detailed in the previously approved MDDP, SCS hydrologic models were developed using the HEC-HMS software package to evaluate the comparison of historic and developed flow conditions, and confirm sizing of the proposed stormwater detention ponds. The detained flow analysis shows that the combination of Pond B and Pond C8 results in detained flows at Design Point #4 calculated as Q_5 = 213.5 cfs and Q_{100} = 577.1 cfs, which achieves the goal of remaining below historic flows at the downstream boundary of the subdivision.

<u>WP2 Filing No. 4 Drainage Plan</u>

Walden Preserve 2 Filing No. 4 is located in parts of Basins B8-B10 and C1-C2 as depicted in the enclosed drainage plans (Sh. D1 and D1.03, Appendix F). Developed Sub-basins B5-B8 comprise the areas along the southeast side of the extension of Pinehurst Circle through Filing No. 4. These basins combine at Design Point #B8, with developed peak flows calculated as Q_5 = 35.9 cfs and Q_{100} = 85.1 cfs (Rational Method). Drainage from these basins will continue to flow northeasterly through grass-lined drainage Channel B8 to Pond B at the east boundary of Filing No. 4.

Developed Sub-basin B9 comprises the area along the northwest side of the extension of Pinehurst Circle through Filing No. 4, flowing northeasterly along the roadside ditch to Culvert B9 (24” RCP) at the low point in the road profile. Developed peak flows at Design Point #B9 are calculated as Q_5 = 10.0 cfs and Q_{100} = 23.7 cfs (Rational Method). Culvert B9 will convey the flow from this basin easterly through grass-lined drainage Channel B9 to Pond B within Tract A at the east boundary of Filing No. 4.

Developed Sub-basin B10 comprises the area on the easterly downstream side of the extension of Pinehurst Circle through Filing No. 4, sheet flowing northeasterly to Pond B. Developed flows from Sub-basins B5-B10 combine at Design Point #B10, with peak flows calculated as Q_5 = 53.5 cfs and Q_{100} = 126.9 cfs (Rational Method).

Developed Sub-basin C1 comprises the area along the west side of the extension of Pinehurst Circle at the north end of Filing No. 4. Sub-basin C1 will generally flow southeasterly along the roadside ditch to Culvert C1 at the low point in the road profile. Developed peak flows at Design Point #C1 are calculated as Q_5 = 11.2 cfs and Q_{100} =

26.7 cfs (Rational Method). Culvert C1 (24" RCP) will convey the flow from this basin easterly across Pinehurst Circle, draining through Channel C1 into Water Quality Rain Garden C2.

Developed Sub-basin C2 comprises the area on the easterly downstream side of the extension of Pinehurst Circle at the north end of Filing No. 4, sheet flowing easterly to Water Quality Pond C2. Developed flows from Sub-basins C1-C2 combine at Design Point #C2, with peak flows calculated as Q_5 = 19.7 cfs and Q_{100} = 46.8 cfs (Rational Method). Water Quality Pond C2 will be constructed on the east side of Basin C2 to provide water quality enhancement for combined Basins C1 and C2.

A 15-inch discharge pipe will release flows from Rain Garden C2 to Channel C2 (ECB-lined) which will flow northwesterly to a concrete crosspan crossing the trail alongside the existing downstream channel.

C. Comparison of Developed to Historic Discharges

Based on the hydrologic calculations in Appendix B, the proposed development will result in developed flows exceeding historic flows from the parcel. The increase in developed flows will be mitigated through on-site stormwater detention facilities.

The comparison of developed to historic discharges at key design points is summarized as follows (per approved MDDP):

Design Point	Historic Flow			Developed Flow			Comparison of Developed to Historic Flow (Q_5%/Q_{100}%)
	Area (ac)	Q_5 (cfs)	Q_{100} (cfs)	Area (ac)	Q_5 (cfs)	Q_{100} (cfs)	
1	337.1	33.1	213.4	342.0	92.1	261.3	278% / 122% (increase)
2 (Developed)	490.5	65.8	287.9	504.7	208.0	551.4	316% / 192% (increase)
2 (Detained)	490.5	65.8	287.9	504.7	3.5	155.8	5% / 54% (decrease)
3 (Developed)				737.8	280.7	734.2	
3 (Detained)				737.8	175.2	459.8	62% / 63% (decrease)
4 (Developed)	902.7	222.5	582.9	902.7	298.7	785.0	134% / 135% (increase)
4 (Detained)	**902.7**	**222.5**	**582.9**	**902.7**	**213.5**	**577.1**	**96% / 99% (decrease)**

D. Detention Ponds

The total developed storm runoff downstream of the proposed subdivision will be maintained at historic levels by routing flows through an existing detention pond (Pond B) located along the main tributary channel running through the property, along with ultimate construction of four additional ponds (Rain Garden C2 and Detention Ponds C4, C8, and C12) serving the northern parts of the Walden 2 PUD area. Rain Garden C2 has been designed as a stormwater quality facility in

Since Pond B is to be constructed meeting current criteria for FSD, update the table to identify the historic and developed flow for the other storm event to show historic levels are maintained for the other design storms (2-yr, 10-yr, 25-yr, 50-yr, EURV).

accordance with County and UDFCD criteria, and future Ponds C4 and C12 will also serve as water quality ponds. In accordance with the approved MDDP, future Pond C8 will be designed as a "full-spectrum" detention pond.

Pond B is an existing Retention Pond with a stormwater detention volume of 13.6 acre-feet above the minimum "active" storage volume in the pond. As detailed in the approved MDDP, Detention Ponds B and C8 will provide sufficient stormwater detention to result in a net release at or below historic flows at Design Point #4 downstream (crossing Walker Road), discharging to the existing natural swale downstream.

Outlet structure improvements to Pond B will be implemented to upgrade the pond to current full-spectrum detention standards. The proposed outlet structure improvements are detailed in an Addendum to the Walden Preserve 2 Filing No. 1 Final Drainage Report (enclosed in Appendix D1).

Development of Walden Preserve 2 Filing No. 4 will include construction of Water Quality Rain Garden C2. Final pond design calculations utilizing the Denver Urban Drainage and Flood Control District (UDFCD) "UD-BMP_v3.06" software package are enclosed in Appendix D2. Design parameters for WQ Rain Garden C2 are summarized as follows:

Pond	Tributary Area (ac)	Percent Impervious (%)	Tributary Impervious Area (ac)	Required WQCV (ac-ft)	Design Volume (ac-ft)
C2	22.9	16.2%	3.7	0.15	0.19

The existing and proposed detention ponds will be owned and maintained by the Walden Property Owners Association.

E. On-Site Drainage Facility Design

Developed sub-basins and proposed drainage improvements are depicted in the enclosed Drainage Plans (Sheets D1 and D1.03). In accordance with El Paso County standards, new roadways will be graded with a minimum longitudinal slope of 1.0 percent. The typical local road section will consist of a 28-foot paved width with 2-foot gravel shoulders and 4:1 slopes to 2.5-foot deep ditches. On-site drainage facilities will consist of roadside ditches, grass-lined channels, and culverts. Hydraulic calculations for preliminary sizing of major on-site drainage facilities are enclosed in Appendix C, and design criteria are summarized as follows:

1. Culverts

The internal road system will be graded to drain roadside ditches to low points along the road profile, where cross-culverts will convey developed flows into grass-lined channels following historic drainage paths. Culvert pipes have been specified as reinforced concrete pipe (RCP) with a minimum diameter of 18-inches. Final culvert design calculations were performed utilizing the FHWA HY-8 software package to perform a detailed analysis of inlet and outlet control conditions, meeting El Paso County criteria for allowable headwater depths and overtopping. Riprap outlet protection will be provided at all culverts.

2. Open Channels

Drainage easements will be dedicated along major drainage channels following historic drainage paths through the subdivision. These channels have generally been designed as grass-lined channels designed to convey 100-year flows, with a trapezoidal cross-section, variable bottom width and depth, 4:1 maximum side slopes, 1-foot freeboard, and a minimum slope of 0.5 percent.

The proposed drainage channels have been sized utilizing Manning's equation for open channel flow, assuming a friction factor ("n") of 0.030 for dry-land grass channels. Maximum allowable velocities will be evaluated based on El Paso County drainage criteria, typically allowing for a maximum 100-year velocity of 4-5 feet per second. Erosion control mats have been specified for channel segments with maximum 100-year velocities up to 8 feet per second. The proposed channels will generally be seeded with native grasses for erosion control. Erosion control mats, ditch checks, and/or riprap channel lining will be provided where required based on erosive velocities. Ditch flows will be diverted to drainage channels at the nearest practical location to minimize excessive roadside ditch sizes.

The proposed development plan for Walden Preserve provides a substantial open space buffer along the existing Tributary Channel of West Cherry Creek running through the site, with the intention of protecting this existing greenway and providing an active trail connection along the drainage corridor. The existing main channel upstream of Pond B is a grass-lined channel in stable condition, and the major channel will generally be protected from development. The existing stock ponds have previously been upgraded with engineered outlet structures and spillways.

Primary drainage swales crossing proposed lots have been placed in drainage easements, with variable widths based on the required channel sections.

F. Analysis of Existing and Proposed Downstream Facilities

The general concept of the proposed drainage plan is to mitigate developed flow impacts and attenuate peak flows in the main channel by routing developed flows through on-site detention ponds. Detention Pond B has been designed to "over-detain" to account for release of developed flows from some downstream parts of the Walden Preserve Subdivision.

Downstream of Detention Pond B, the existing channel runs through the back yards of a block of existing developed lots. Detention Pond B has been designed to reduce peak flows below historic levels, so there is no anticipated increase in peak flows to be discharged to the existing channel running through these downstream properties. The channel downstream of Pond B is an existing deficient condition from a drainage design standpoint. No structural improvements (i.e. riprap lining, drop structures) were constructed to stabilize this segment of channel when the original subdivision was developed, and as a result this segment of channel does not meet County standards for public drainage facilities. The pond runs through a row of private backyards, and a number of existing private structures are located along the drainage channel. The channel shows signs of erosion in several areas, and limited maintenance access is available along this private reach of channel.

The proposed drainage plan recognizes the existing deficiencies in the channel downstream of Pond B, and for this reason a primary focus of the proposed drainage plan is to reduce flows to a level below historic flows at Detention Pond B prior to reaching this existing deficient reach of channel. An engineered energy dissipation structure has been constructed at the pond discharge point to further mitigate the impact of flows from the Walden Preserve Subdivision.

The existing 43”x27” elliptical CMP culvert at Walker Road has an estimated capacity of 55 cfs. The existing culvert is undersized based on the calculated historic flows from the basin. While the existing roadway would be expected to overtop during major storm events, we are not aware of any reported drainage problems at this crossing and we are not aware of any County plans for replacement of the existing culvert. The existing stock ponds upstream in the main channel provide a significant level of stormwater detention/retention in their current condition, which likely has minimized historic concerns with flows overtopping the roadway at this crossing.

Combined flows in the East Tributary of West Cherry Creek continue flowing northerly through a grass-lined channel following a 100-foot wide drainage easement through the Shamrock Hills Subdivision north of Walker Road, ultimately reaching a confluence with West Cherry Creek. The existing channel downstream of the culvert crossing Walker Road consists of a broad grass-lined swale with no signs of active erosion. On-site detention ponds mitigate the developed drainage impacts from the Walden Preserve Subdivision, so there is no need for this development to upgrade existing downstream facilities.

G. Anticipated Drainage Problems and Solutions

Stormwater detention ponds have been designed to mitigate the impacts of developed drainage from the overall Walden Preserve 2 PUD project. The previously completed construction and upgrades to Pond B are sufficient to provide stormwater detention for WP2 Filings No. 1-4. Water Quality Rain Garden C2 will provide stormwater quality mitigation for Filing No. 4.

The overall drainage plan for the subdivision includes a system of roadside ditches, channels, and culverts to convey developed flows through the site. The primary drainage problems anticipated within this development will consist of maintenance of these drainage channels, culverts, and detention pond facilities. Care will need to be taken to implement proper erosion control measures in the proposed roadside ditches, channels, and swales.

Ditches have been designed to meet allowable velocity criteria. Erosion control mats will be installed where necessary to minimize erosion concerns. Proper construction and maintenance of the proposed detention facility will minimize downstream drainage impacts.

VII. EROSION / SEDIMENT CONTROL

The Contractor will be required to implement Best Management Practices (BMP's) for erosion control through the course of construction. Sediment control measures will include installation of silt fence at the toe of disturbed slopes and hay bales protecting drainage ditches. Cut slopes will be stabilized during excavation as necessary and vegetation will be established for stabilization of disturbed areas as soon as possible. All ditches will be designed to meet El Paso County criteria for slope and velocity.

VIII. COST ESTIMATE AND DRAINAGE FEES

A cost estimate for proposed drainage improvements is enclosed in Appendix E.

The developer will finance all construction costs for proposed roadway and drainage improvements, and public facilities will be owned and maintained by El Paso County upon final acceptance. The existing and proposed detention ponds will ultimately be owned and maintained by the Walden Metropolitan District.

This parcel is located in the West Cherry Creek drainage basin. No drainage and bridge fees will be due at time of recordation of the final plat as the subject site is not located in a fee basin.

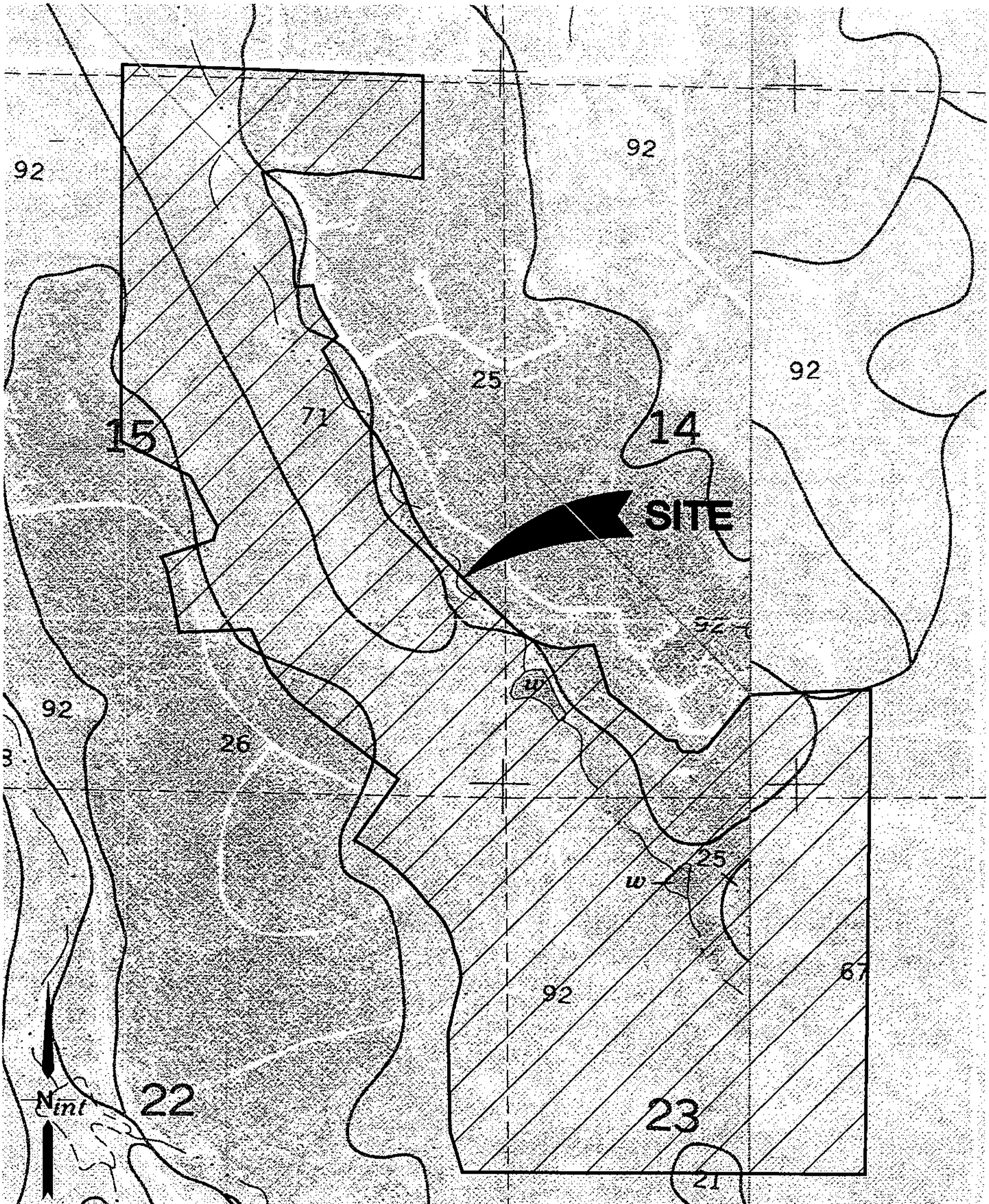
IX. SUMMARY

Walden Preserve 2 Filing No. 4 consists of 23 proposed lots along the new extension of Pinehurst Circle east of Walden Way. Developed drainage impacts from the Walden Preserve 2 PUD will be mitigated through existing and proposed on-site stormwater detention and water quality facilities. The proposed drainage patterns will remain consistent with historic conditions, and new drainage facilities constructed to El Paso County standards will safely convey runoff to suitable outfalls. Based on the on-site stormwater detention facilities, no downstream drainage improvements are proposed.

The existing and proposed detention ponds have been designed to maintain historic peak flows downstream of the Walden Preserve Subdivision. Proper construction and maintenance of the proposed drainage and erosion control facilities will ensure that this subdivision has no significant adverse drainage impact on downstream or surrounding areas.

APPENDIX A

SOILS INFORMATION



SCALE: 1"=1000'

(EL PASO COUNTY SCS MAP SHEET NO. 1 & 2)

J:\jpsprojects\040201.walden\WALDEN PRESERVE 2\DWG\CIVIL\SCS-1.dwg Sep 17, 2014 - 9:42am

SCS SOILS MAP	JPS ENGINEERING	WALDEN PRESERVE FIGURE SCS-1 JPS PROJ NO. 040201

The presence of princesplume, two-groove milkvetch, and Fremont goldenweed indicates that selenium-bearing plants are in the stand.

Good grazing management is essential to maintain desirable grasses on the Kutch soil. Deferment of grazing early in spring helps to maintain the health and vigor of cool-season grasses. Proper location of livestock watering facilities helps to control grazing.

If the range has deteriorated, blue grama, junegrass, and native bluegrasses increase. Sleepygrass and annuals replace these grasses if the range has seriously deteriorated. Proper range management and proper location of livestock watering facilities are essential to maintain the potential vegetation on these soils. Seeding is advisable if the range is in poor condition. Seeding of the native vegetation is desirable, but the range can also be seeded with tame species of grasses such as Nordan crested wheatgrass, Russian wildrye, pubescent wheatgrass, or intermediate wheatgrass.

This complex is suited to wildlife habitat. It is best suited to habitat for openland and rangeland wildlife. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

The main limitations of these soils for homesites are depth to sandstone and shale, permeability, and the shrink-swell potential of the subsoil. Dwellings and roads need to be designed to offset these limitations. Septic tank absorption fields do not function properly because of the depth to shale and permeability. Capability subclass VIe.

25—Elbeth sandy loam, 3 to 8 percent slopes. This deep, well drained soil formed in material transported from arkose deposits on uplands. Elevation ranges from 7,300 to 7,600 feet. The average annual precipitation is about 18 inches, the average annual air temperature is about 43 degrees F, and the average frost-free period is about 120 days.

Typically the surface layer is very dark grayish brown sandy loam about 3 inches thick. The subsurface layer is light gray loamy sand about 20 inches thick. The subsoil is brown sandy clay loam about 45 inches thick. The substratum is light brown sandy clay loam.

Included with this soil in mapping are small areas of Tomah-Crowfoot loamy sands, 3 to 8 percent slopes; Kettle gravelly loamy sand, 3 to 8 percent slopes; and Peyton-Pring complex, 3 to 8 percent slopes.

Permeability of this Elbeth soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is high. Surface runoff is slow to medium, and the hazard of erosion is moderate.

This soil is used for woodland, limited livestock grazing, recreation, wildlife habitat, and homesites.

This soil is suited to the production of ponderosa pine. It is capable of producing about 2,240 cubic feet, or 4,900 board feet (International rule), of merchantable timber per acre from a fully stocked, even-aged stand of 80-year-old trees. Conventional methods can be used for harvesting, but operations may be restricted during wet periods. Reforestation, after harvesting, must be carefully managed to reduce competition of undesirable understory plants.

Woodland wildlife, such as mule deer and wild turkey, is attracted to this soil because of its potential to produce ponderosa pine, Gambel oak, and various grasses and shrubs. Water developments, such as guzzlers, would enhance populations of wild turkey as well as other kinds of wildlife. Where wildlife and livestock share the same range, proper grazing management is needed to prevent overuse and to reduce competition. Livestock watering facilities would also benefit wildlife on this soil.

This soil has good potential for homesites. The main limitation is the moderate shrink-swell potential in the subsoil. Special road design is necessary on this soil to overcome the limitations of shrink-swell potential and frost action. Special planning is needed on this soil to minimize site disturbance and tree and seedling damage. During seasons of low precipitation, fire may become a hazard to homesites on this soil. The hazard can be minimized by installing firebreaks and reducing the amount of potential fuel on the forest floor. Capability subclass VIe.

26—Elbeth sandy loam, 8 to 15 percent slopes. This deep, well drained soil formed in material transported from arkose deposits on uplands. Elevation ranges from 7,300 to 7,600 feet. The average annual precipitation is about 18 inches, the average annual air temperature is about 43 degrees F, and the average frost-free season is about 120 days.

Typically, the surface layer is very dark grayish brown sandy loam about 3 inches thick. The subsurface layer is light gray loamy sand about 20 inches thick. The subsoil is brown sandy clay loam about 45 inches thick. The substratum is light brown.

Included with this soil in mapping are small areas of Tomah-Crowfoot loamy sand, 8 to 15 percent slopes; Peyton-Pring complex, 8 to 15 percent slopes; Kettle gravelly loamy sand, 8 to 40 percent slopes; and Kettle-Rock outcrop complex.

Permeability of this Elbeth soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is high. Surface runoff is slow to medium, and the hazard of erosion is moderate.

This soil is used for woodland, limited livestock grazing, recreation, wildlife habitat, and homesites.

This soil is suited to the production of ponderosa pine. It is capable of producing about 2,240 cubic feet, or 4,900 board feet (International rule), of merchantable timber per acre from a fully stocked, even-aged stand of 80-year-old trees. Conventional methods can be used for harvesting, but operations may be restricted during wet periods. Reforestation, after harvesting, must be carefully managed to reduce competition of undesirable understory plants.

survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

These soils are suited to habitat for openland and rangeland wildlife. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

These soils have a good potential for homesites. The main limitations, especially on the Peyton soil, are low bearing strength and frost-action potential. Buildings and roads can be designed to overcome these limitations. Access roads should have adequate cut-slope grade and be provided with drains to control surface runoff and keep soil losses to a minimum. Capability subclass VIe.

69—Peyton-Pring complex, 8 to 15 percent slopes. These gently to moderately sloping soils are on valley side slopes and on uplands. Elevation ranges from 6,800 to 7,600 feet. The average annual precipitation is about 17 inches, the average annual air temperature is about 43 degrees F, and the average frost-free period is about 120 days.

The Peyton soil makes up about 40 percent of the complex, the Pring soil about 30 percent, and other soils about 30 percent.

Included with these soils in mapping are areas of Holderness loam, 8 to 15 percent slopes; Tomah-Crowfoot loamy sands, 8 to 15 percent slopes; Kettle gravelly loamy sand, 8 to 40 percent slopes; and a few areas of Rock outcrop.

The Peyton soil is commonly on the less sloping part of the landscape. It is deep, noncalcareous, and well drained. It formed in alluvium and residuum derived from weathered, arkosic, sedimentary rock. Typically, the surface layer is grayish brown sandy loam about 12 inches thick. The subsoil, about 23 inches thick, is pale brown sandy clay loam in the upper 13 inches and pale brown sandy loam in the lower 10 inches. The substratum is pale brown sandy loam to a depth of 60 inches or more.

Permeability of the Peyton soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is high. Surface runoff is medium to rapid, and the hazard of erosion is moderate to high. Some gullies have developed along drainageways and livestock trails.

The Pring soil is deep, noncalcareous, and well drained. It formed in sandy sediment derived from weathered, arkosic, sedimentary rock. Typically, the surface layer is dark grayish brown coarse sandy loam about 4 inches thick. The substratum is dark grayish brown coarse sandy loam about 10 inches thick over pale brown gravelly sandy loam that extends to a depth of 60 inches or more.

Permeability of the Pring soil is rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is medium to rapid, and the hazard of erosion is moderate to high. Some gullies have developed along drainageways and livestock trails.

The soils in this complex are used as rangeland, for wildlife habitat, and for homesites.

These soils are well suited to the production of native vegetation suitable for grazing. The dominant native species are mountain muhly, bluestem grasses, needleandthread, and blue grama. These soils are subject to invasion of Kentucky bluegrass and Gambel oak. Common forbs are hairy goldenrod, geranium, milkvetch, low larkspur, fringed sage, and buckwheat.

Properly locating livestock watering facilities helps to control grazing. Timely deferment of grazing is needed to protect the plant cover.

Windbreaks and environmental plantings generally are suited to these soils. Soil blowing is the main limitation to the establishment of trees and shrubs. This limitation can be overcome by cultivating only in the tree rows and leaving a strip of vegetation between the rows. Supplemental irrigation may be needed when planting and during dry periods. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

These soils are well suited to wildlife habitat. They are best suited to habitat for openland and rangeland wildlife. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

These soils have good potential for use as homesites. The main limitations are steepness of slope, limited ability to support a load, and frost-action potential. Buildings and roads can be designed to overcome these limitations. These soils also require special site or building designs because of the slope. Access roads should have adequate cut-slope grade, and drains should be provided to control surface runoff and keep soil losses to a minimum. Capability subclass VIe.

70—Pits, gravel. Gravel pits are in nearly level to rolling areas. They are open excavations several feet deep and commonly 5 acres or less in size.

Gravel pits are very low in natural fertility and are highly susceptible to soil blowing. A cover of weeds or straw helps to control erosion.

Windbreaks and environmental plantings generally are not suited to these areas. Onsite investigation is needed to determine if plantings are feasible. Capability subclass VIIIs.

71—Pring coarse sandy loam, 3 to 8 percent slopes. This deep, noncalcareous, well drained soil formed in sandy sediment derived from arkosic sedimentary rock on valley side slopes and on uplands. Elevation ranges from 6,800 to 7,600 feet. The average annual precipitation is about 17 inches, the average annual air temperature is about 43 degrees F, and the average frost-free period is about 120 days.

Typically, the surface layer is dark grayish brown coarse sandy loam about 4 inches thick. The substratum is dark grayish brown coarse sandy loam about 10 inches thick over pale brown gravelly sandy loam that extends to a depth of 60 inches or more.

Included with this soil in mapping are small areas of Alamosa loam, 1 to 3 percent slopes, along drainageways; Cruckton sandy loam, 1 to 9 percent slopes; Peyton sandy loam, 1 to 5 percent slopes; Peyton sandy loam, 5 to 9 percent slopes; and Tomah-Crowfoot loamy sands, 3 to 8 percent slopes. In some places arkose beds of sandstone and shale are at a depth of 0 to 40 inches.

Permeability of this Pring soil is rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is medium, and the hazard of erosion is moderate.

Almost all areas of this soil are used as rangeland. Some areas previously cultivated have been reseeded to grass. This soil is also used for wildlife habitat and homesites.

This soil is well suited to the production of native vegetation suitable for grazing by cattle and sheep. Rangeland vegetation is mainly mountain muhly, little bluestem, needleandthread, Parry oatgrass, and junegrass.

Deferment of grazing in spring helps to maintain vigor and production of the cool-season bunchgrasses. Fencing and properly locating livestock watering facilities help to control grazing.

Windbreaks and environmental plantings generally are suited to this soil. The hazard of soil blowing is the main limitation to the establishment of trees and shrubs. This limitation can be overcome by cultivating only in the tree rows and leaving a strip of vegetation between the rows. Supplemental irrigation may be needed when planting and during dry periods. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

This soil is suited to habitat for openland and rangeland wildlife. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

This soil is well suited for use as homesites. Erosion control practices are needed to control soil blowing and water erosion on construction sites where the ground cover has been removed. Capability subclass IVe.

72—Pring coarse sandy loam, 8 to 15 percent slopes. This deep, noncalcareous, well drained soil formed in sandy sediment derived from arkosic sedimentary rock on valley side slopes and on uplands. Elevation ranges from 6,800 to 7,600 feet. The average annual precipitation is about 17 inches, the average annual air temperature is about 43 degrees F, and the average frost-free period is about 120 days.

Typically, the surface layer is dark grayish brown coarse sandy loam about 4 inches thick. The substratum is dark grayish brown coarse sandy loam about 10 inches thick over pale brown gravelly sandy loam that extends to a depth of 60 inches or more.

Included with this soil in mapping are small areas of Cruckton sandy loam, 1 to 9 percent slopes; Peyton sandy loam, 5 to 9 percent slopes; and Tomah-Crowfoot loamy sands, 8 to 15 percent slopes. Arkose beds of sandstone and shale are at a depth of 0 to 40 inches in some places.

Permeability of this Pring soil is rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is medium, and the hazard of erosion is moderate. Some gullies have developed along drainageways.

Almost all areas of this soil are used as rangeland. Some areas previously cultivated have been reseeded to grass. This soil is also used for wildlife habitat and as homesites.

This soil is well suited to the production of native vegetation suitable for grazing by cattle and sheep. The native vegetation is mainly mountain muhly, little bluestem, needleandthread, Parry oatgrass, and junegrass.

Deferment of grazing in spring helps to maintain the vigor and production of the cool-season bunchgrasses. Fencing and properly locating livestock watering facilities help to control grazing.

Windbreaks and environmental plantings generally are suited to this soil. The hazard of soil blowing is the main limitation to the establishment of trees and shrubs. This limitation can be overcome by cultivating only in the tree rows and leaving a strip of vegetation between the rows. Supplemental irrigation may be needed when planting and during dry periods. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, Siberian elm, Russian-olive, and hackberry. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

This soil is suited to habitat for openland and rangeland wildlife habitat. Rangeland wildlife, such as pronghorn antelope, can be encouraged by developing livestock watering facilities, properly managing livestock grazing, and reseeding range where needed.

This soil has good potential for urban uses. The main limitation is slope. Special site or building designs are needed because of the slope. Access roads must have adequate cut-slope grade and be provided with drains to control surface runoff. Capability subclass VIe.

73—Razor clay loam, 3 to 9 percent slopes. This moderately deep, well drained, clayey soil formed in residuum derived from calcareous shale on uplands. Elevation ranges from 5,300 to 6,100 feet. The average annual precipitation is about 13 inches, the average annual air temperature is about 49 degrees F, and the average frost-free period is about 145 days.

Typically, the surface layer is light brownish gray clay loam about 3 inches thick. The subsoil is grayish brown heavy clay loam or clay about 15 inches thick. The substratum is grayish brown clay that grades to calcareous shale at a depth of about 31 inches. Visible lime is in the lower part of the subsoil and in the substratum.

Included with this soil in mapping are small areas of Midway clay loam, 3 to 25 percent slopes; Heldt clay loam, 0 to 3 percent slopes; and Stoneham sandy loam, 3 to 8 percent slopes.

strength. Special designs for buildings and roads are required to offset these limitations. Methods of sewage disposal other than septic tank absorption fields are needed because of the limited depth to bedrock. Capability subclass VIe.

92—Tomah-Crowfoot loamy sands, 3 to 8 percent slopes. These gently sloping to moderately sloping soils are on alluvial fans, hills, and ridges in the uplands. Elevation ranges from about 7,300 to 7,600 feet. The average annual precipitation is about 17 inches, the average annual air temperature is about 42 degrees F, and the average frost-free period is about 120 days.

The Tomah soil makes up about 50 percent of the complex, the Crowfoot soil about 30 percent, and other soils about 20 percent.

Included with these soils in mapping are areas of Elbeth sandy loam, 3 to 8 percent slopes; Kettle gravelly loamy sand, 3 to 8 percent slopes; and Pring coarse sandy loam, 3 to 8 percent slopes.

The Tomah soil is deep and well drained. It formed in alluvium or residuum derived from arkose beds. Typically, the surface layer is dark grayish brown loamy sand about 10 inches thick. The subsurface layer is very pale brown coarse sand about 12 inches thick. The subsoil, about 26 inches thick, is a matrix of very pale brown coarse sand in which are embedded many thin bands and lamellae of pale brown coarse sandy clay loam. The substratum is very pale brown coarse sand to a depth of 60 inches or more.

Permeability of the Tomah soil is moderately rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is slow, and the hazard of erosion is slight to moderate.

The Crowfoot soil is deep and well drained. It formed in sediment weathered from arkosic sandstone. Typically, the surface layer is grayish brown loamy sand about 12 inches thick. The subsurface layer is very pale brown sand about 11 inches thick. The subsoil is light yellowish brown sandy clay loam about 13 inches thick. The substratum is very pale brown coarse sand to a depth of about 68 inches.

Permeability of the Crowfoot soil is moderate. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is slow, and the hazard of erosion is slight to moderate.

This complex is used as rangeland, for wildlife habitat, and as homesites.

Native vegetation is mainly mountain muhly, bluestem, mountain brome, needleandthread, and blue grama. These soils are subject to invasion by Kentucky bluegrass and Gambel oak. Noticeable forbs are hairy goldenrod, geranium, milkvetch, low larkspur, fringed sage, and buckwheat.

Properly locating livestock watering facilities helps to control grazing. Timely deferment of grazing is needed to protect the plant cover.

Windbreaks and environmental plantings are fairly well suited to these soils. Blowing sand and moderate available water capacity are the principal limitations for the establishment of trees and shrubs. The soils are so loose that trees need to be planted in shallow furrows and plant cover needs to be maintained between the rows. Supplemental irrigation may be needed to insure survival. Trees that are best suited and have good survival are Rocky Mountain juniper, eastern redcedar, ponderosa pine, and Siberian elm. Shrubs that are best suited are skunkbush sumac, lilac, and Siberian peashrub.

These soils are best suited to habitat for openland wildlife such as pronghorn antelope and sharp-tailed grouse. Although sharp-tailed grouse are not plentiful, they could be encouraged on these soils, especially where brush species are interspersed with grasses and forbs. If these soils are used as rangeland, wildlife production can be increased by managing livestock grazing to preclude overuse of the more desirable grass species and depletion of the various brush species.

These soils have good potential for use as homesites. The main limitation of the Crowfoot soil is frost-action potential. Roads and streets need to be designed to minimize frost-heave damage. Maintaining the existing vegetation on building sites during construction helps to control erosion. Capability subclass IVe.

93—Tomah-Crowfoot loamy sands, 8 to 15 percent slopes. These moderately sloping to strongly sloping soils are on alluvial fans, hills, and ridges in the uplands. Elevation ranges from about 7,300 to 7,600 feet. The average annual precipitation is about 17 inches, the average annual air temperature is about 42 degrees F, and the average frost-free period is about 120 days.

The Tomah soil makes up about 50 percent of the complex, the Crowfoot soil about 30 percent, and other soils about 20 percent.

Included with these soils in mapping are areas of Elbeth sandy loam, 8 to 15 percent slopes; Peyton-Pring complex, 8 to 15 percent slopes; and Kettle gravelly loamy sand, 8 to 40 percent slopes.

The Tomah soil is deep and well drained. It formed in alluvium or residuum derived from arkose beds. Typically, the surface layer is dark grayish brown loamy sand about 10 inches thick. The subsurface layer is very pale brown coarse sand about 12 inches thick. The subsoil, about 26 inches thick, consists of a matrix of very pale brown coarse sandy clay loam. The substratum is very pale brown coarse sand to a depth of 60 inches or more.

Permeability of the Tomah soil is moderately rapid. Effective rooting depth is 60 inches or more. Available water capacity is moderate. Surface runoff is medium, and the hazard of erosion is moderate. Some gullies are present in some drainageways and along stock trails.

The Crowfoot soil is deep and well drained. It formed in sediment weathered from arkosic sandstone. Typically, the surface layer is grayish brown loamy sand about 12 inches thick. The subsurface layer is very pale brown sand about 11 inches thick. The subsoil is light yellowish brown sandy clay loam about 13 inches thick. The substratum is very pale brown coarse sand to a depth of about 68 inches.

TABLE 16.--SOIL AND WATER FEATURES

[Absence of an entry indicates the feature is not a concern. See "flooding" in Glossary for definition of terms as "rare," "brief," and "very brief." The symbol > means greater than]

Soil name and map symbol	Hydro-logic group	Flooding			Bedrock		Potential frost action
		Frequency	Duration	Months	Depth	Hardness	
					In		
Alamosa: 1	C	Frequent	Brief	May-Jun	>60	---	High.
Ascalon: 2, 3	B	None	---	---	>60	---	Moderate.
Badland: 4	D	---	---	---	---	---	---
Bijou: 5, 6, 7	B	None	---	---	>60	---	Low.
Blakeland: 8	A	None	---	---	>60	---	Low.
9: Blakeland part	A	None	---	---	>60	---	Low.
Fluvaquentic Haplaquolls part	D	Common	Very brief	Mar-Aug	>60	---	High.
Blendon: 10	B	None	---	---	>60	---	Moderate.
Bresser: 11, 12, 13	B	None	---	---	>60	---	Low.
Brussett: 14, 15	B	None	---	---	>60	---	Moderate.
Chaseville: 16, 17	A	None	---	---	>60	---	Low.
18: Chaseville part	A	None	---	---	>60	---	Low.
Midway part	D	None	---	---	10-20	Rippable	Moderate.
Columbine: 19	A	None to rare	---	---	>60	---	Low.
Connerton: 20: Connerton part	B	None	---	---	>60	---	High.
Rock outcrop part	D	---	---	---	---	---	---
Cruckton: 21	B	None	---	---	>60	---	Moderate.
Cushman: 22, 23	C	None	---	---	20-40	Rippable	Moderate.
24: Cushman part	C	None	---	---	20-40	Rippable	Moderate.
Kutch part	C	None	---	---	20-40	Rippable	Moderate.
Elbeth: 25, 26	B	None	---	---	>60	---	Moderate.
27: Elbeth part	B	None	---	---	>60	---	Moderate.

See footnote at end of table.

TABLE 16.--SOIL AND WATER FEATURES--Continued

Soil name and map symbol	Hydrologic group	Flooding			Bedrock		Potential frost action
		Frequency	Duration	Months	Depth	Hardness	
					In		
Manvel:							
50	C	None	---	---	>60	---	High.
Manzanola:							
51, 52, 53	C	None to rare	---	---	>60	---	Moderate.
Midway:							
54	D	None	---	---	10-20	Rippable	Moderate.
Nederland:							
55	B	None	---	---	>60	---	Moderate.
Nelson:							
[1]56:							
Nelson part	B	None	---	---	20-40	Rippable	Low.
Tassel part	D	None	---	---	10-20	Rippable	Low.
Neville:							
57	B	None	---	---	>60	---	High.
[1]58:							
Neville part	B	None	---	---	>60	---	High.
Rednun part	C	None	---	---	>60	---	Moderate.
Nunn:							
59	C	None	---	---	>60	---	Moderate.
Olney:							
60, 61	B	None	---	---	>60	---	Moderate.
[1]62:							
Olney part	B	None	---	---	>60	---	Moderate.
Vona part	B	None	---	---	>60	---	Moderate.
Paunsaugunt:							
[1]63:							
Paunsaugunt part	D	None	---	---	10-20	Hard	Moderate.
Rock outcrop part	D	---	---	---	---	---	---
Penrose:							
[1]64:							
Penrose part	D	None	---	---	10-20	Rippable	Low.
Manvel part	C	None	---	---	>60	---	High.
Perrypark:							
65	B	None	---	---	>60	---	Moderate.
Peyton:							
66, 67	B	None	---	---	>60	---	Moderate.
[1]68, [1]69:							
Peyton part	B	None	---	---	>60	---	Moderate.
Pring part	B	None	---	---	>60	---	Moderate.
Pits, gravel:							
70	A	---	---	---	---	---	---
Pring:							
71, 72	B	None	---	---	>60	---	Moderate.
Razor:							
73, 74	C	None	---	---	20-40	Rippable	Moderate.

[1] See footnote at end of table.

TABLE 16.--SOIL AND WATER FEATURES--Continued

Soil name and map symbol	Hydrologic group	Flooding			Bedrock		Potential frost action
		Frequency	Duration	Months	Depth	Hardness	
					In		
Tomah:							
[1]92, [1]93:							
Tomah part	B	None	---	---	>60	---	Moderate.
Crowfoot part	B	None	---	---	>60	---	Moderate.
Travessilla:							
[1]94:							
Travessilla part	D	None	---	---	6-20	Hard	Low.
Rock outcrop part	D	---	---	---	---	---	---
Truckton:							
95, 96, 97	B	None	---	---	>60	---	Moderate.
[1]98:							
Truckton part	B	None	---	---	>60	---	Moderate.
Blakeland part	A	None	---	---	>60	---	Low.
[1]99, [1]100:							
Truckton part	B	None	---	---	>60	---	Moderate.
Bresser part	B	None	---	---	>60	---	Low.
Ustic Torrifluvents:							
101	B	Occasional	Very brief	Mar-Aug	>60	---	Moderate.
Valent:							
102, 103	A	None	---	---	>60	---	Low.
Vona:							
104, 105	B	None	---	---	>60	---	Moderate.
Wigton:							
106	A	None	---	---	>60	---	Low.
Wiley:							
107, 108	B	None	---	---	>60	---	Low.
Yoder:							
109, 110	B	None	---	---	>60	---	Low.

[1]This map unit is made up of two or more dominant kinds of soil. See map unit description for the composition and behavior characteristics of the map unit.

APPENDIX B1

HYDROLOGIC CALCULATIONS – SCS METHOD

(FROM APPROVED 2014 WALDEN PRESERVE MDDP)

TABLE 5-5
RUNOFF CURVE NUMBERS FOR HYDROLOGIC SOIL COVER COMPLEXES - URBAN AND SUBURBAN CONDITIONS 1/
(Antecedent Moisture Condition II)
(From: U.S. Dept. of Agriculture, Soil Conservation Service, 1977)

Land Use		Hydrologic Soil Group A	B	C	D
Open spaces, lawns, parks, golf courses, cemeteries, etc.					
Good condition: grass cover on 75% or more of the area		39*	61	74	80
Fair condition: grass cover on 50% to 75% of the area		49*	69	79	84
Commercial and Business areas (85% Impervious)		89*	92	94	95
Industrial Districts 72% Impervious)		81*	88	91	93
Residential: 2/					
Acres per Dwelling Unit	Average % Impervious 3/				
1/8 acre or less	65	77*	85	90	92
1/4 acre	38	61*	75	83	87
1/3 acre	30	57*	72	81	86
1/2 acre	25	54*	70	80	85
1 acre	20	51*	68	79	84
Paved parking lots, roofs, driveways, etc.		98	98	98	98
Streets and Roads:					
paved with curbs and storm sewers		98	98	98	98
gravel		76*	85	89	91
dirt		72*	82	87	89

1/ For a more detailed description of agricultural land use curve numbers, refer to the National Engineering Handbook (U.S. Dept. of Agriculture, Soil Conservation Service, 1972).
2/ Curve numbers are computed assuming the runoff from the house and driveway is directed towards the street with a minimum of roof water directed to lawns where additional infiltration could occur.
3/ The remaining pervious areas (lawn) are considered to be in good pasture condition for these curve numbers.

* Not to be used wherever overlot grading or filling is to occur.

WALDEN PRESERVE
IMPERVIOUS AREA CALCULATIONS

DEVELOPED CONDITIONS

IMPERVIOUS AREAS

BASIN	TOTAL AREA (AC)	SOIL TYPE	(AC)	SUB-AREA 1 DEVELOPMENT/ COVER	IMP. %	AREA (AC)	SUB-AREA 2 DEVELOPMENT/ COVER	IMP. %	(AC)	SUB-AREA 3 DEVELOPMENT/ COVER	IMP. %	WEIGHTED % IMP.
OA1.1	83.8	B	83.8	PASTURE	2.00							2.00
OA1.2	108.2	B	108.2	PASTURE	2.00							2.00
A1	28.25	B	28.25	2.5-AC. LOTS	11.00							11.00
OA1.3	14.8	B	14.8	PASTURE	2.00							2.00
A2	2.34	B	2.34	2.5-AC. LOTS	11.00							11.00
OA1.3,A2	17.14	B										3.23
OA1,A1,A2	237.39	B										3.16
OA1.4	25.2	B	25.2	PASTURE	2.00							2.00
A3	11.22	B	11.22	2.5-AC. LOTS	11.00							11.00
A4	1.58	B	1.58	2.5-AC. LOTS	11.00							11.00
OA1.4,A3,A4	38.0	B										5.03
A5	15.14	B	15.14	2.5-AC. LOTS	11.00							11.00
A6	1.98	B	1.98	2.5-AC. LOTS	11.00							11.00
A5,A6	17.12	B										11.00
A7	27.55	B	27.55	1-AC. LOTS	20.00							20.00
A8	4.12	B	4.12	1-AC. LOTS	20.00							20.00
A9	17.78	B	17.78	1-AC. LOTS	20.00							20.00
OA1.4,A3,A4,A9	55.78	B										9.80
OA1,A1-A9	341.96	B										6.20
OB1	35.4	B	35.4	1/2-AC. LOTS	25.00							25.00
B1-B4	55.5	B	55.5	1/2-AC. LOTS	25.00							25.00
OB1,B1-B4	90.90	B										25.00
B5-B10	71.80	B	71.8	1-AC. LOTS	20.00							20.00
OB1,B1-B10	162.70	B										22.79
OA1,OB1,A,B	504.66	B										11.55
OC1	129.00	B	129	1/2-AC. LOTS	25.00							25.00
C1-C4	40.30	B	40.3	1-AC. LOTS	20.00							20.00
C5-C8	59.30	B	59.3	1-AC. LOTS	20.00							20.00
C9	4.50	B	4.5	1-AC. LOTS	20.00							20.00
OC1,C1-C9	233.10	B										22.77
OA1-OC1,A-C9	737.76	B										15.09
OC2	81.70	B	81.7	1/2-AC. LOTS	25.00							25.00
C10-C12	26.40	B	26.4	1-AC. LOTS	20.00							20.00
C13	22.40	B	22.4	MEADOW	2.00							2.00
OC2,C10-C13	130.50	B										20.04
OD1	3.30	B	3.30	1/2-AC. LOTS	25.00							25.00
OD2	4.50	B	4.50	FOREST	2.00							2.00
OD3	10.30	B	10.30	FOREST	2.00							2.00
OD4	6.00	B	6.00	FOREST	2.00							2.00
D	10.27	B	10.27	1/2-AC. LOTS	25.00							25.00
OD1-OD4, D	34.37	B										11.08
OA1-OD1,A-D	902.63	B										15.65

HISTORIC FLOWS

BASIN	DESIGN POINT	AREA (AC)	AREA (SM)	CURVE No. (CN)	S	Ia	PERCENT IMPERVIOUS (%)	Time of Concentration Tc (1) (MIN)	Total Lag Time Tl (2) (MIN)	Peak Flow SCS Q5 (3) (CFS)	Peak Flow SCS Q100 (3) (CFS)
OA1		232.0	0.3625	61	6.39	1.28	2	34.80	20.88		
A		105.1	0.1642	61	6.39	1.28	2	18.80	11.28		
OA1,A	1	337.1	0.5267	61	6.39	1.28	2	53.60	32.16	33.1	213.4
OB1		35.4	0.0553	70	4.29	0.86	25	8.20	4.92		
B		118.0	0.1844	61	6.39	1.28	2	13.70	8.22		
OA1-OAB,A-B	2	490.5	0.7664	61	6.39	1.28	2	67.30	40.38	65.8	287.9
OC1		187.6	0.2931	70	4.29	0.86	25	14.40	8.64		
C		190.2	0.2972	61	6.39	1.28	2	47.10	28.26		
OD1		24.1	0.0377	61	6.39	1.28	2	30.30	18.18		
D		10.27	0.0160	61	6.39	1.28	2	4.50	2.70		
OA1-OC1,A-D	4	902.7	1.4104	61	6.39	1.28	2	114.40	68.64	222.5	582.9

DEVELOPED FLOWS

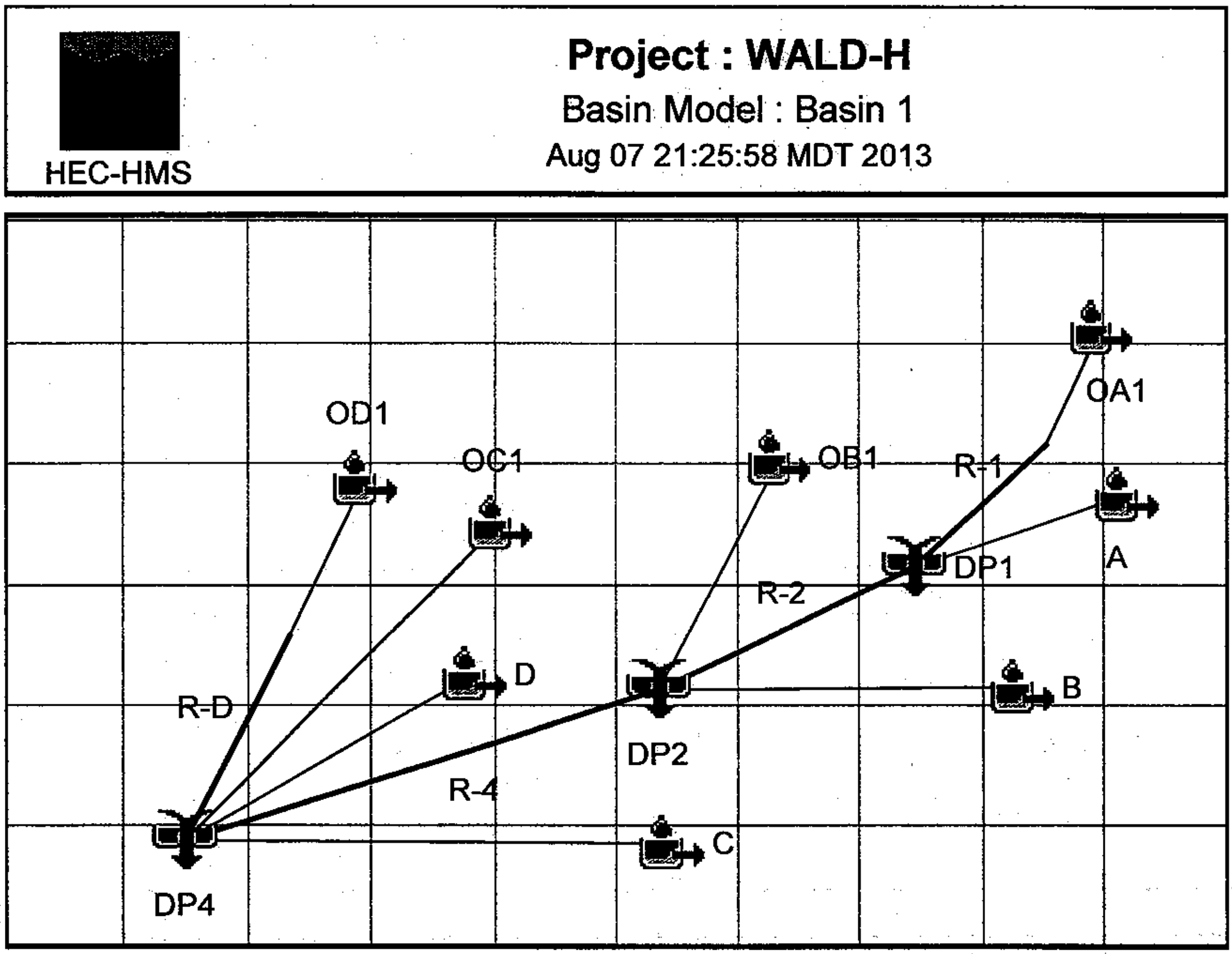
BASIN	DESIGN POINT	AREA (AC)	AREA (SM)	CURVE No. (CN)	S	Ia	PERCENT IMPERVIOUS (%)	Time of Concentration Tc (1) (HR)	Total Lag Time Tl (2) (HR)	Peak Flow SCS Q5 (3) (CFS)	Peak Flow SCS Q100 (3) (CFS)
OA1		232.0	0.3625	61	6.39	1.28	2	36.80	22.08		
A9 (A1-A9)		110.0	0.1719	68	4.71	0.94	20	19.00	11.40		
OA1,A1-A9	1	342.0	0.5344					55.80	33.48	92.1	261.3
OB1		35.4	0.0553	70	4.29	0.86	25	20.60	12.36		
B4 (B1-B4)		55.5	0.0867	70	4.29	0.86	25	21.10	12.66		
B10 (B5-B10)		71.8	0.1122	68	4.71	0.94	20	28.70	17.22		
Tt DP1 to DP2								13.70	8.22		
OA1-OB1,A-B	2	504.7	0.7886					69.50	41.70	208.0	551.4
OC1		129	0.2016	69.5	4.39	0.88	23.6	14.40	8.64		
C4 (C1-C4)		40.3	0.0630	68	4.71	0.94	20	21.70	13.02		
C8 (C5-C8)		59.3	0.0927	68	4.71	0.94	20	35.00	21.00		
C9		4.5	0.0070	61	6.39	1.28	2	2.10	1.26		
Tt DP2 to DP3								22.10	13.26		
OA1-OC1,A,B,C1-C8	3	737.8	1.1528					91.60	54.96	280.7	734.2
OC2		81.7	0.1277	67.9	4.73	0.95	19.1	14.40	8.64		
C12 (C10-C12)		26.4	0.0413	68	4.71	0.94	20	19.10	11.46		
C13		22.4	0.0350	61	6.39	1.28	2	15.30	9.18		
OD1		24.1	0.0377	61	6.39	1.28	2	30.30	18.18		
D		10.3	0.0160	70	4.29	0.86	25	4.50	2.70		
Tt DP3 to DP4								25.40	15.24		
OA1-OC2,A-D	4	902.7	1.4104					117.00	70.20	298.7	785.0

1) Tc = Tco + Tt (from Rational Method Calculation Spreadsheet)
2) SCS LAG TIME, Tl = 0.6 * Tt
3) PEAK FLOWS CALCULATED BY HEC-HMS 3.5; 5-YR RAINFALL DEPTH = 2.6 IN; 100-YR RAINFALL DEPTH = 4.4 IN

DETAINED FLOWS

BASIN	DESIGN POINT	AREA (AC)	AREA (SM)	CURVE No. (CN)	S	Ia	PERCENT IMPERVIOUS (%)	Time of Concentration T_c (1) (HR)	Total Lag Time T_l (2) (HR)	Peak Flow SCS Q5 (3) (CFS)	Peak Flow SCS Q100 (3) (CFS)
OA1		232.0	0.3625	61	6.39	1.28	2	36.80	22.08		
A9 (A1-A9)		110.0	0.1719	68	4.71	0.94	20	19.00	11.40		
OA1,A1-A9	1	342.0	0.5344					55.80	33.48	92.1	261.3
OB1		35.4	0.0553	70	4.29	0.86	25	20.60	12.36		
B4 (B1-B4)		55.5	0.0867	70	4.29	0.86	25	21.10	12.66		
B10 (B5-B10)		71.8	0.1122	68	4.71	0.94	20	28.70	17.22		
Tt DP1 to DP2								13.70	8.22		
OA1-OB1,A-B	2	504.7	0.7886					69.50	41.70	82.3	311.1
OC1		129	0.2016	69.5	4.39	0.88	23.6	14.40	8.64		
C4 (C1-C4)		40.3	0.0630	68	4.71	0.94	20	21.70	13.02		
C8 (C5-C8)		59.3	0.0927	68	4.71	0.94	20	35.00	21.00		
C9		4.5	0.0070	61	6.39	1.28	2	2.10	1.26		
Tt DP2 to DP3								22.10	13.26		
OA1-OC1,A,B,C1-C8	3	737.8	1.1528					91.60	54.96	175.2	459.8
OC2		81.7	0.1277	67.9	4.73	0.95	19.1	14.40	8.64		
C12 (C10-C12)		26.4	0.0413	68	4.71	0.94	20	19.10	11.46		
C13		22.4	0.0350	61	6.39	1.28	2	15.30	9.18		
OD1		24.1	0.0377	61	6.39	1.28	2	30.30	18.18		
D		10.3	0.0160	70	4.29	0.86	25	4.50	2.70		
Tt DP3 to DP4								25.40	15.24		
OA1-OC2,A-D	4	902.7	1.4104					117.00	70.20	213.5	577.1

1) $T_c = T_{co} + T_t$ (from Rational Method Calculation Spreadsheet)
2) SCS LAG TIME, $T_l = 0.6 * T_t$
3) PEAK FLOWS CALCULATED BY HEC-HMS 3.5; 5-YR RAINFALL DEPTH = 2.6 IN; 100-YR RAINFALL DEPTH = 4.4 IN



HEC-HMS
Project : WALD-H
Basin Model : Basin 1
Aug 07 21:25:58 MDT 2013
OA1
OD1
OC1
OB1
R-1
A
DP1
R-2
D
B
R-D
DP2
R-4
C
DP4

100-YK

Project: WALD-H   Simulation Run: Run 1

Start of Run: 01Jan3000, 01:00   Basin Model: Basin 1

End of Run: 02Jan3000, 01:30   Meteorologic Model: Met 1

Compute Time: 07Aug2013, 21:24:43   Control Specifications: Control 1

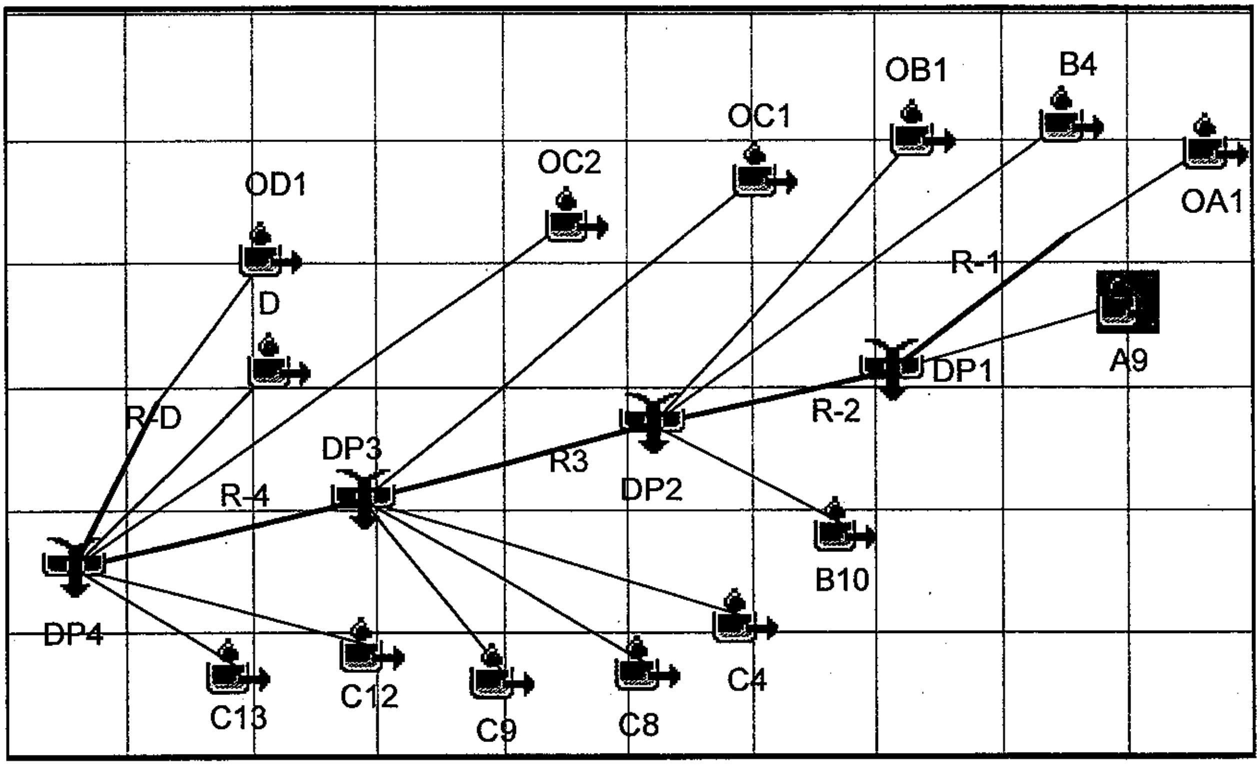
Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
OA1	0.3625	176.1	01Jan3000, 13:16	21.1
R-1	0.3625	176.1	01Jan3000, 13:27	21.0
A	0.1642	118.2	01Jan3000, 13:06	9.6
DP1	0.5267	213.4	01Jan3000, 13:25	30.6
R-2	0.5267	213.4	01Jan3000, 13:33	30.5
B	0.1844	154.8	01Jan3000, 13:03	10.7
OB1	0.0553	113.5	01Jan3000, 12:58	6.8
DP2	0.7664	287.9	01Jan3000, 13:02	48.0
R-4	0.7664	287.9	01Jan3000, 13:30	47.6
C	0.2972	116.4	01Jan3000, 13:24	17.2
OC1	0.2931	517.1	01Jan3000, 13:02	36.0
OD1	0.0377	20.2	01Jan3000, 13:13	2.2
R-D	0.0377	20.2	01Jan3000, 13:15	2.2
D	0.0160	17.4	01Jan3000, 12:57	0.9
DP4	1.4104	582.9	01Jan3000, 13:03	103.9

HEC-HMS

Project : WALD_D

Basin Model : Basin 1

Aug 07 21:29:09 MDT 2013



5-YR

Project: WALD_D Simulation Run: Run 1

Start of Run: 01Jan3000, 01:00 Basin Model: Basin 1
End of Run: 02Jan3000, 01:30 Meteorologic Model: Met 1
Compute Time: 07Aug2013, 21:31:24 Control Specifications: Control 1

OA1	0.3625	24.9	01Jan3000, 13:21	5.3
R-1	0.3625	24.9	01Jan3000, 13:32	5.3
A9	0.1719	88.9	01Jan3000, 13:05	7.9
DP1	0.5344	92.1	01Jan3000, 13:06	13.2
R-2	0.5344	92.1	01Jan3000, 13:14	13.2
B10	0.1122	46.2	01Jan3000, 13:11	5.2
B4	0.0867	52.2	01Jan3000, 13:06	4.7
OB1	0.0553	33.7	01Jan3000, 13:06	3.0
DP2	0.7886	208.0	01Jan3000, 13:11	26.1
R3	0.7886	208.0	01Jan3000, 13:24	26.1
OC1	0.2016	137.0	01Jan3000, 13:03	10.6
C8	0.0927	33.6	01Jan3000, 13:14	4.3
C4	0.0630	30.5	01Jan3000, 13:07	2.9
C9	0.0070	1.3	01Jan3000, 12:58	0.1
DP3	1.1529	280.7	01Jan3000, 13:22	43.9
R-4	1.1529	280.7	01Jan3000, 13:37	43.8
OC2	0.1277	72.3	01Jan3000, 13:03	5.7
C12	0.0413	21.3	01Jan3000, 13:05	1.9
OD1	0.0377	2.9	01Jan3000, 13:16	0.5
R-D	0.0377	2.9	01Jan3000, 13:18	0.5
C13	0.0350	4.1	01Jan3000, 13:06	0.5
D	0.0160	14.6	01Jan3000, 12:56	0.9
DP4	1.4106	298.7	01Jan3000, 13:37	53.4

100-YR

Project: WALD_D Simulation Run: Run 1

Start of Run: 01Jan3000, 01:00 Basin Model: Basin 1
End of Run: 02Jan3000, 01:30 Meteorologic Model: Met 1
Compute Time: 07Aug2013, 21:30:41 Control Specifications: Control 1

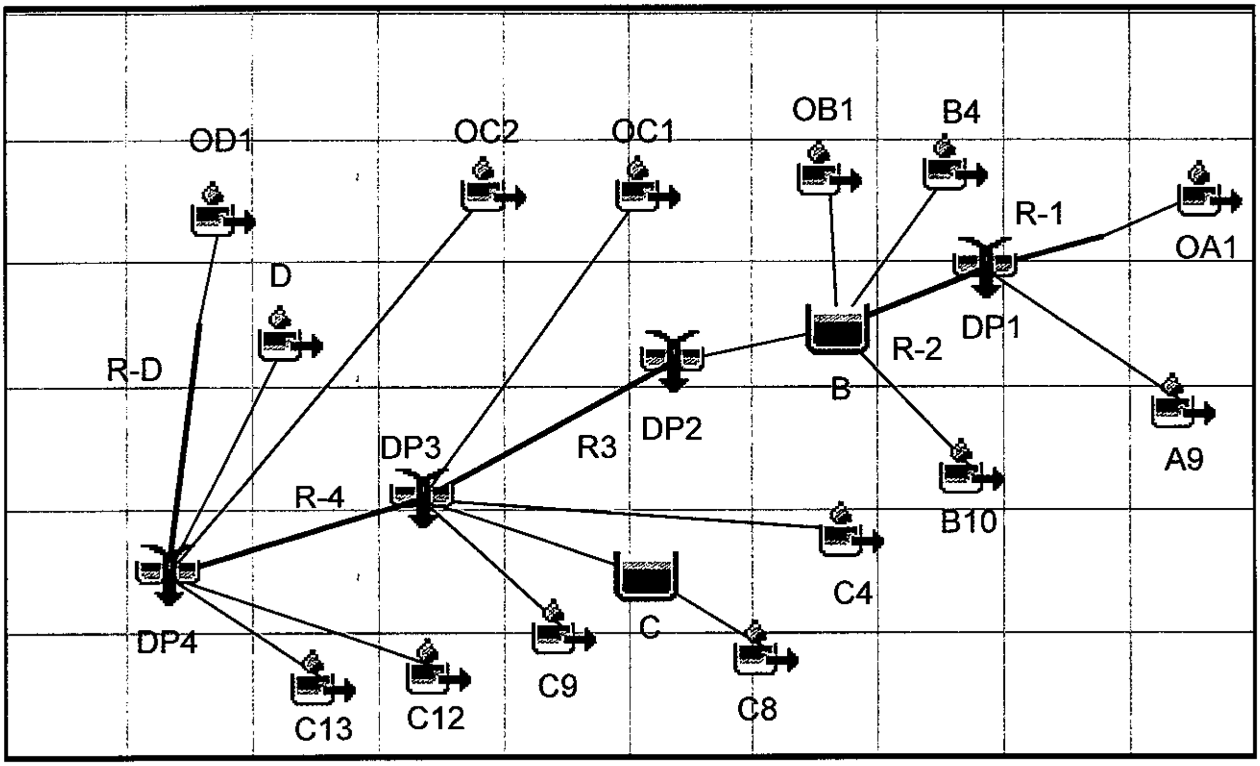
Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
OA1	0.3625	169.4	01Jan3000, 13:17	21.0
R-1	0.3625	169.4	01Jan3000, 13:28	21.0
A9	0.1719	240.1	01Jan3000, 13:05	18.8
DP1	0.5344	261.3	01Jan3000, 13:06	39.8
R-2	0.5344	261.3	01Jan3000, 13:14	39.7
B10	0.1122	125.0	01Jan3000, 13:10	12.3
B4	0.0867	129.2	01Jan3000, 13:06	10.6
OB1	0.0553	83.3	01Jan3000, 13:06	6.8
DP2	0.7886	551.4	01Jan3000, 13:11	69.4
R3	0.7886	551.4	01Jan3000, 13:24	69.3
OC1	0.2016	345.7	01Jan3000, 13:02	24.0
C8	0.0927	91.0	01Jan3000, 13:14	10.1
C4	0.0630	82.3	01Jan3000, 13:06	6.9
C9	0.0070	8.0	01Jan3000, 12:55	0.4
DP3	1.1529	734.2	01Jan3000, 13:22	110.7
R-4	1.1529	734.2	01Jan3000, 13:37	110.4
OC2	0.1277	197.7	01Jan3000, 13:02	13.7
C12	0.0413	57.5	01Jan3000, 13:05	4.5
OD1	0.0377	20.2	01Jan3000, 13:13	2.2
R-D	0.0377	20.2	01Jan3000, 13:15	2.2
C13	0.0350	27.9	01Jan3000, 13:04	2.0
D	0.0160	35.8	01Jan3000, 12:56	2.0
DP4	1.4106	785.0	01Jan3000, 13:36	134.8

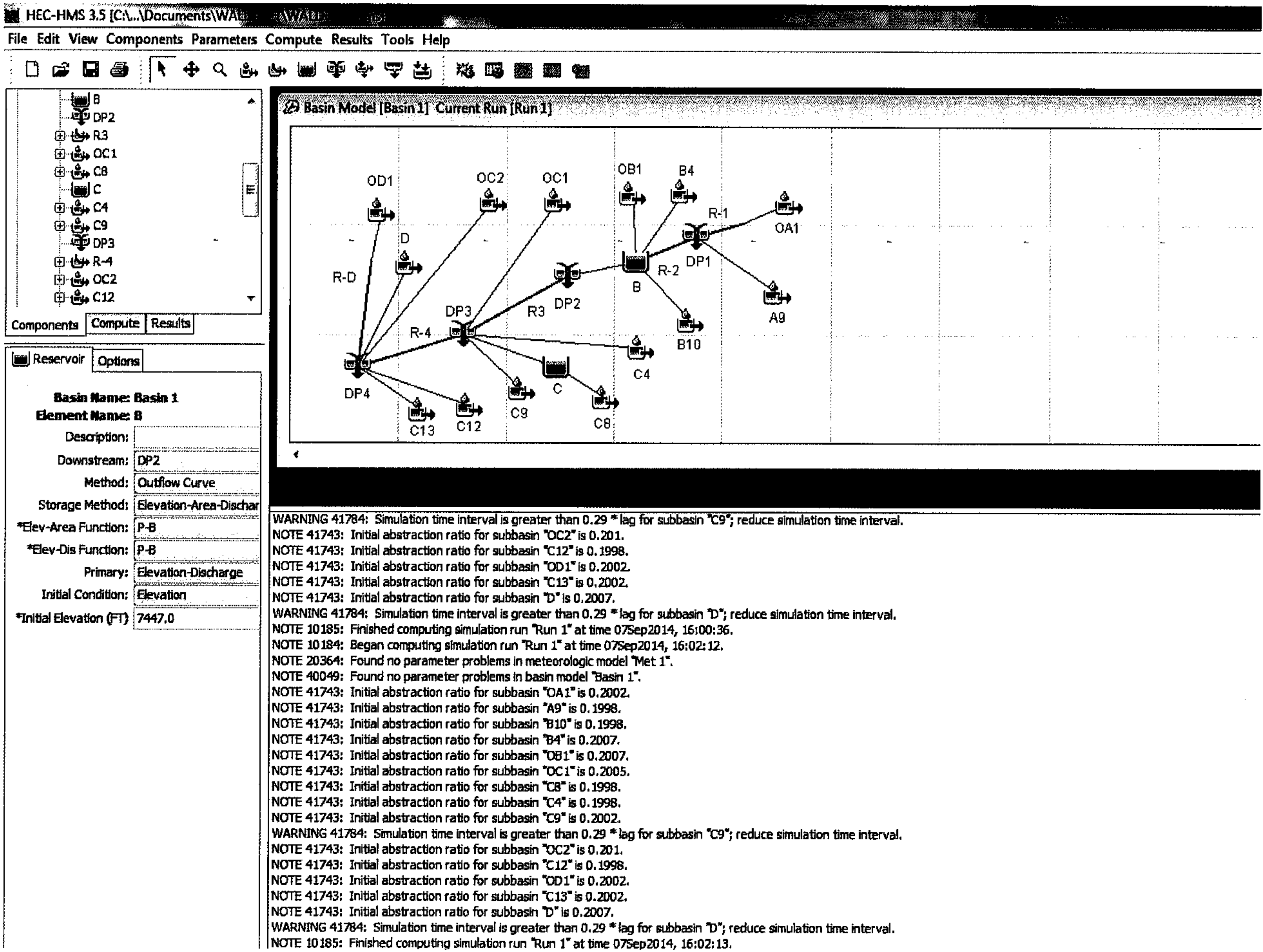
HEC-HMS

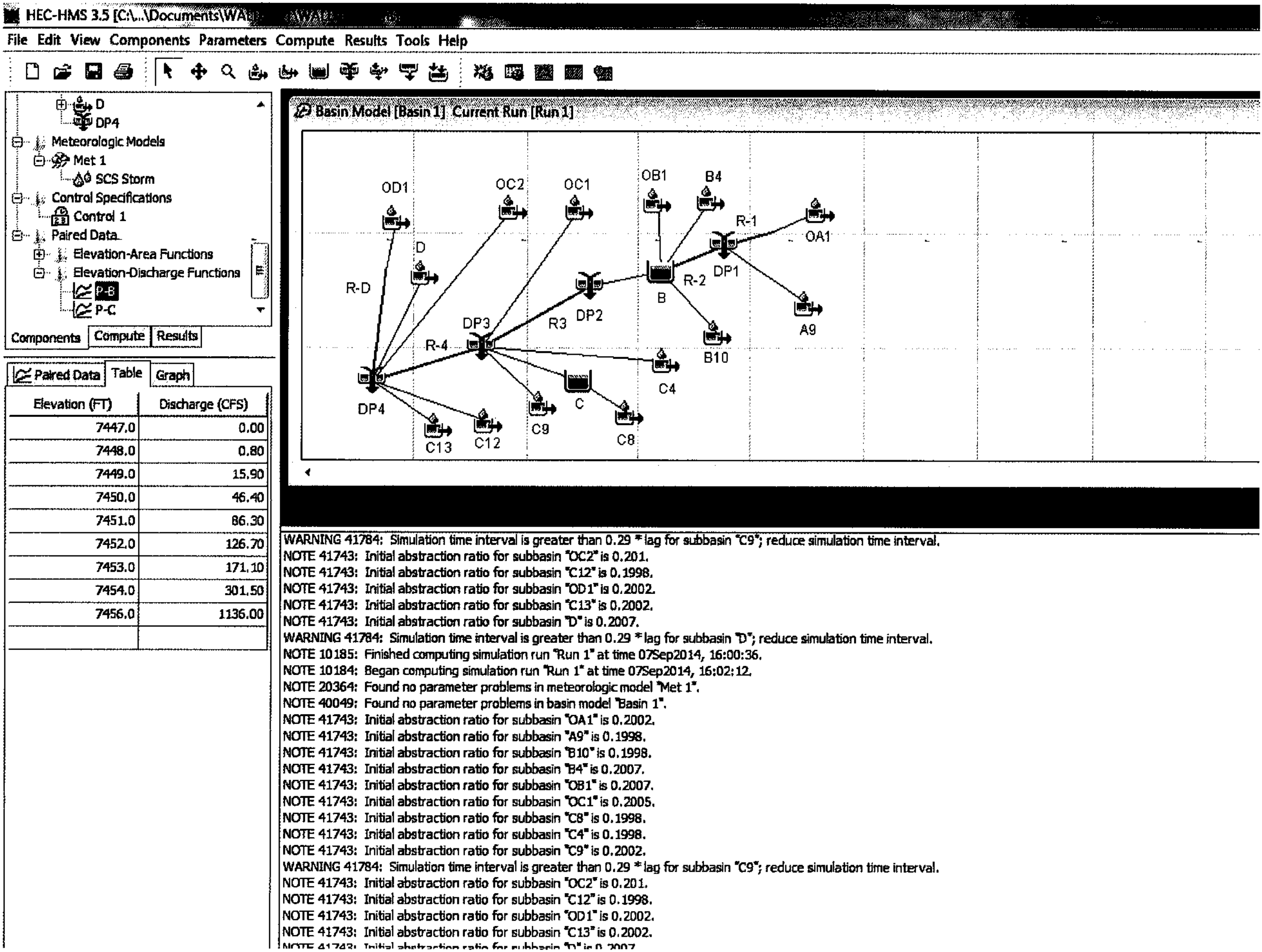
Project : WALD_DET

Basin Model : Basin 1

Sep 07 16:02:43 MDT 2014




HEC-HMS 3.5 [C:\...\Documents\WA
File Edit View Components Parameters Compute Results Tools Help
B
DP2
R3
OC1
C8
C
C4
C9
DP3
R-4
OC2
C12
Components
Compute
Results
Reservoir
Options
Basin Name: Basin 1
Element Name: B
Description:
Downstream: DP2
Method: Outflow Curve
Storage Method: Elevation-Area-Dischar
*Elev-Area Function: P-B
*Elev-Dis Function: P-B
Primary: Elevation-Discharge
Initial Condition: Elevation
*Initial Elevation (FT) 7447.0
Basin Model [Basin 1] Current Run [Run 1]
OD1
OC2
OC1
OB1
B4
R-1
OA1
D
R-D
DP1
R-2
B
DP2
R3
DP3
R-4
A9
B10
C4
C
DP4
C9
C8
C13
C12
WARNING 41784: Simulation time interval is greater than 0.29 * lag for subbasin "C9"; reduce simulation time interval.
NOTE 41743: Initial abstraction ratio for subbasin "OC2" is 0.201.
NOTE 41743: Initial abstraction ratio for subbasin "C12" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "OD1" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "C13" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "D" is 0.2007.
WARNING 41784: Simulation time interval is greater than 0.29 * lag for subbasin "D"; reduce simulation time interval.
NOTE 10185: Finished computing simulation run "Run 1" at time 07Sep2014, 16:00:36.
NOTE 10184: Began computing simulation run "Run 1" at time 07Sep2014, 16:02:12.
NOTE 20364: Found no parameter problems in meteorologic model "Met 1".
NOTE 40049: Found no parameter problems in basin model "Basin 1".
NOTE 41743: Initial abstraction ratio for subbasin "OA1" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "A9" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "B10" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "B4" is 0.2007.
NOTE 41743: Initial abstraction ratio for subbasin "OB1" is 0.2007.
NOTE 41743: Initial abstraction ratio for subbasin "OC1" is 0.2005.
NOTE 41743: Initial abstraction ratio for subbasin "C8" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "C4" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "C9" is 0.2002.
WARNING 41784: Simulation time interval is greater than 0.29 * lag for subbasin "C9"; reduce simulation time interval.
NOTE 41743: Initial abstraction ratio for subbasin "OC2" is 0.201.
NOTE 41743: Initial abstraction ratio for subbasin "C12" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "OD1" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "C13" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "D" is 0.2007.
WARNING 41784: Simulation time interval is greater than 0.29 * lag for subbasin "D"; reduce simulation time interval.
NOTE 10185: Finished computing simulation run "Run 1" at time 07Sep2014, 16:02:13.


HEC-HMS 3.5 [C:\...\Documents\WA
File Edit View Components Parameters Compute Results Tools Help
D
DP4
Meteorologic Models
Met 1
SCS Storm
Control Specifications
Control 1
Paired Data
Elevation-Area Functions
Elevation-Discharge Functions
P-B
P-C
Components
Compute
Results
Paired Data
Table
Graph
Elevation (FT)
Discharge (CFS)
7447.0 0.00
7448.0 0.80
7449.0 15.90
7450.0 46.40
7451.0 86.30
7452.0 126.70
7453.0 171.10
7454.0 301.50
7456.0 1136.00
Basin Model [Basin 1] Current Run [Run 1]
OD1
OC2
OC1
OB1
B4
R-1
OA1
D
R-D
DP1
R-2
B
DP2
R3
DP3
R-4
A9
B10
C4
DP4
C
C8
C9
C13
C12
WARNING 41784: Simulation time interval is greater than 0.29 * lag for subbasin "C9"; reduce simulation time interval.
NOTE 41743: Initial abstraction ratio for subbasin "OC2" is 0.201.
NOTE 41743: Initial abstraction ratio for subbasin "C12" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "OD1" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "C13" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "D" is 0.2007.
WARNING 41784: Simulation time interval is greater than 0.29 * lag for subbasin "D"; reduce simulation time interval.
NOTE 10185: Finished computing simulation run "Run 1" at time 07Sep2014, 16:00:36.
NOTE 10184: Began computing simulation run "Run 1" at time 07Sep2014, 16:02:12.
NOTE 20364: Found no parameter problems in meteorologic model "Met 1".
NOTE 40049: Found no parameter problems in basin model "Basin 1".
NOTE 41743: Initial abstraction ratio for subbasin "OA1" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "A9" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "B10" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "B4" is 0.2007.
NOTE 41743: Initial abstraction ratio for subbasin "OB1" is 0.2007.
NOTE 41743: Initial abstraction ratio for subbasin "OC1" is 0.2005.
NOTE 41743: Initial abstraction ratio for subbasin "C8" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "C4" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "C9" is 0.2002.
WARNING 41784: Simulation time interval is greater than 0.29 * lag for subbasin "C9"; reduce simulation time interval.
NOTE 41743: Initial abstraction ratio for subbasin "OC2" is 0.201.
NOTE 41743: Initial abstraction ratio for subbasin "C12" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "OD1" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "C13" is 0.2002.

HEC-HMS 3.5 [C:\..\Documents\WA...

File Edit View Components Parameters Compute Results Tools Help

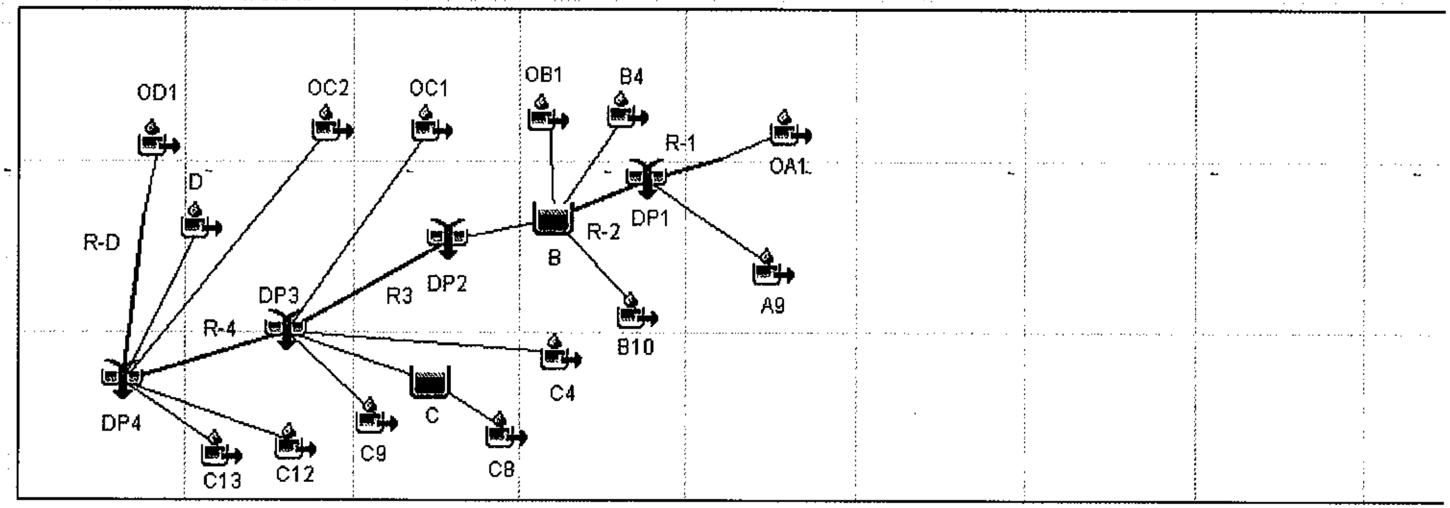
- D
- DP4
- Meteorologic Models
  - Met 1
    - SCS Storm
- Control Specifications
  - Control 1
- Paired Data
  - Elevation-Area Functions
    - P-B
    - P-C
  - Elevation-Discharge Functions

Components | Compute | Results

Paired Data | Table | Graph

Elevation (FT)	Area (AC)
7447.0	1.76
7448.0	1.95
7449.0	2.12
7450.0	2.27
7451.0	2.43
7452.0	2.61
7453.0	3.17
7454.0	3.79
7456.0	5.30

Basin Model [Basin 1] Current Run [Run 1]



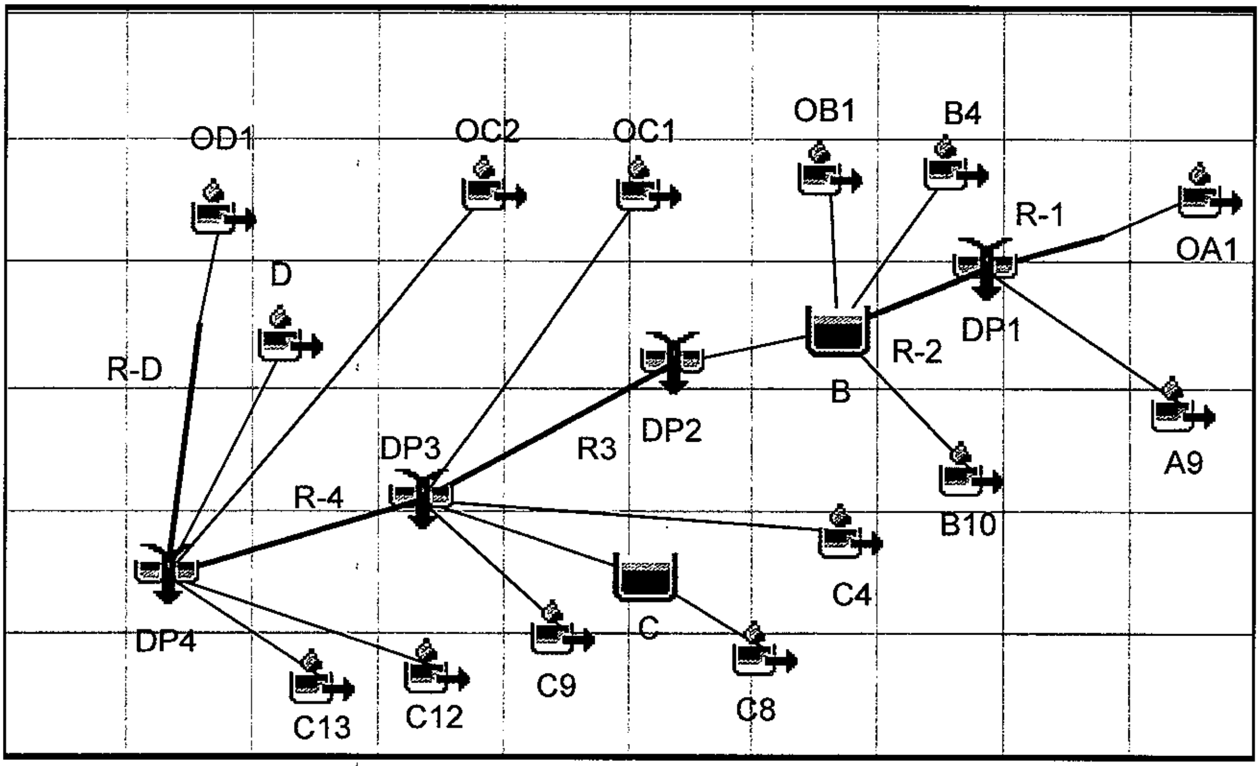
WARNING 41784: Simulation time interval is greater than 0.29 * lag for subbasin "C9"; reduce simulation time interval.
NOTE 41743: Initial abstraction ratio for subbasin "OC2" is 0.201.
NOTE 41743: Initial abstraction ratio for subbasin "C12" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "OD1" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "C13" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "D" is 0.2007.
WARNING 41784: Simulation time interval is greater than 0.29 * lag for subbasin "D"; reduce simulation time interval.
NOTE 10185: Finished computing simulation run "Run 1" at time 07Sep2014, 16:00:36.
NOTE 10184: Began computing simulation run "Run 1" at time 07Sep2014, 16:02:12.
NOTE 20364: Found no parameter problems in meteorologic model "Met 1".
NOTE 40049: Found no parameter problems in basin model "Basin 1".
NOTE 41743: Initial abstraction ratio for subbasin "OA1" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "A9" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "B10" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "B4" is 0.2007.
NOTE 41743: Initial abstraction ratio for subbasin "OB1" is 0.2007.
NOTE 41743: Initial abstraction ratio for subbasin "OC1" is 0.2005.
NOTE 41743: Initial abstraction ratio for subbasin "C8" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "C4" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "C9" is 0.2002.
WARNING 41784: Simulation time interval is greater than 0.29 * lag for subbasin "C9"; reduce simulation time interval.
NOTE 41743: Initial abstraction ratio for subbasin "OC2" is 0.201.
NOTE 41743: Initial abstraction ratio for subbasin "C12" is 0.1998.
NOTE 41743: Initial abstraction ratio for subbasin "OD1" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "C13" is 0.2002.
NOTE 41743: Initial abstraction ratio for subbasin "D" is 0.2007.

HEC-HMS

Project : WALD_DET

Basin Model : Basin 1

Sep 07 16:03:12 MDT 2014



5-YR

Project: WALD_DET Simulation Run: Run 1

Start of Run: 01Jan3000, 01:00 Basin Model: Basin 1
End of Run: 02Jan3000, 01:30 Meteorologic Model: Met 1
Compute Time: 07Sep2014, 16:05:27 Control Specifications: Control 1

Hydrologic Element	Drainage Area (MI2)	Peak Discharge (CFS)	Time of Peak	Volume (AC-FT)
OA1	0.3625	24.9	01Jan3000, 13:21	5.3
R-1	0.3625	24.9	01Jan3000, 13:32	5.3
A9	0.1719	88.9	01Jan3000, 13:05	7.9
DP1	0.5344	92.1	01Jan3000, 13:06	13.2
R-2	0.5344	92.1	01Jan3000, 13:14	13.2
B10	0.1122	46.2	01Jan3000, 13:11	5.2
B4	0.0867	52.1	01Jan3000, 13:06	4.7
OB1	0.0553	33.6	01Jan3000, 13:06	3.0
B	0.7886	82.3	01Jan3000, 13:36	23.5
DP2	0.7886	82.3	01Jan3000, 13:36	23.5
R3	0.7886	82.3	01Jan3000, 13:49	23.3
OC1	0.2016	137.0	01Jan3000, 13:03	10.6
C8	0.0927	33.6	01Jan3000, 13:14	4.3
C	0.0927	14.5	01Jan3000, 13:40	3.4
C4	0.0630	30.5	01Jan3000, 13:07	2.9
C9	0.0070	1.3	01Jan3000, 12:58	0.1
DP3	1.1529	175.2	01Jan3000, 13:03	40.3
R-4	1.1529	175.2	01Jan3000, 13:18	40.1
OC2	0.1277	72.3	01Jan3000, 13:03	5.7
C12	0.0413	21.3	01Jan3000, 13:05	1.9
OD1	0.0377	2.9	01Jan3000, 13:16	0.5
R-D	0.0377	2.9	01Jan3000, 13:18	0.5
C13	0.0350	4.1	01Jan3000, 13:06	0.5
D	0.0160	14.6	01Jan3000, 12:56	0.9
DP4	1.4106	213.5	01Jan3000, 13:17	49.7

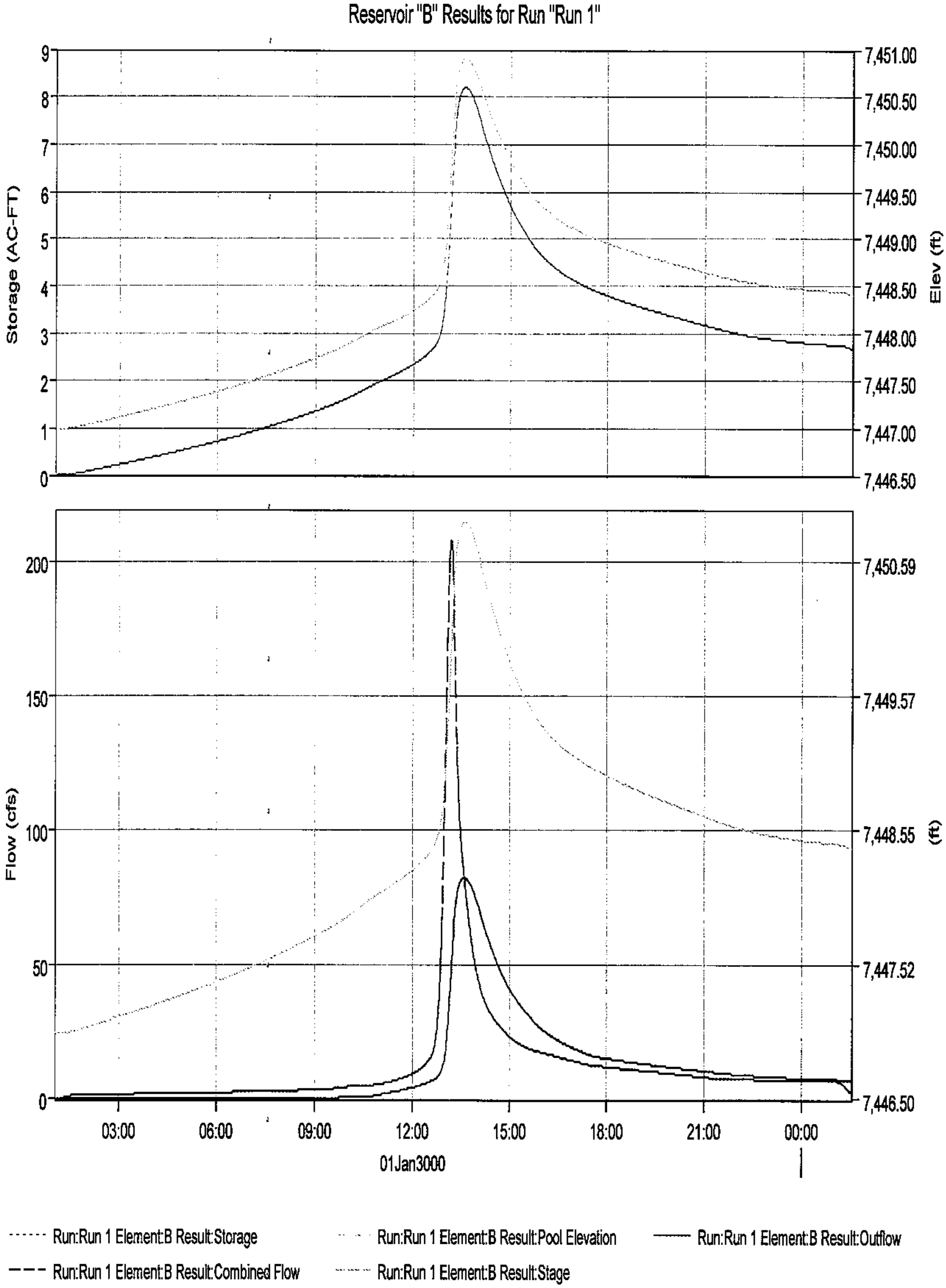
Project: WALD_DET
Simulation Run: Run 1 Reservoir: B

Start of Run:	01Jan3000, 01:00	Basin Model:	Basin 1
End of Run:	02Jan3000, 01:30	Meteorologic Model:	Met 1
Compute Time:	07Sep2014, 16:05:27	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	208.1 (CFS)	Date/Time of Peak Inflow :	01Jan3000, 13:11
Peak Outflow :	82.3 (CFS)	Date/Time of Peak Outflow :	01Jan3000, 13:36
Total Inflow :	26.1 (AC-FT)	Peak Storage :	8.2 (AC-FT)
Total Outflow :	23.5 (AC-FT)	Peak Elevation :	7450.9 (FT)


Reservoir "B" Results for Run "Run 1"
Storage (AC-FT)
Elev (ft)
Flow (cfs)
(ft)
0
1
2
3
4
5
6
7
8
9
7,446.50
7,447.00
7,447.50
7,448.00
7,448.50
7,449.00
7,449.50
7,450.00
7,450.50
7,451.00
0
50
100
150
200
7,446.50
7,447.52
7,448.55
7,449.57
7,450.59
03:00
06:00
09:00
12:00
15:00
18:00
21:00
00:00
01Jan3000
Run:Run 1 Element:B Result:Storage
Run:Run 1 Element:B Result:Pool Elevation
Run:Run 1 Element:B Result:Outflow
Run:Run 1 Element:B Result:Combined Flow
Run:Run 1 Element:B Result:Stage

100-YR

Project: WALD_DET Simulation Run: Run 1

Start of Run: 01Jan3000, 01:00 Basin Model: Basin 1
End of Run: 02Jan3000, 01:30 Meteorologic Model: Met 1
Compute Time: 07Sep2014, 16:02:13 Control Specifications: Control 1

OA1	0.3625	169.4	01Jan3000, 13:17	21.0
R-1	0.3625	169.4	01Jan3000, 13:28	21.0
A9	0.1719	240.1	01Jan3000, 13:05	18.8
DP1	0.5344	261.3	01Jan3000, 13:06	39.8
R-2	0.5344	261.3	01Jan3000, 13:14	39.7
B10	0.1122	125.0	01Jan3000, 13:10	12.3
B4	0.0867	129.0	01Jan3000, 13:06	10.6
OB1	0.0553	83.2	01Jan3000, 13:06	6.8
B	0.7886	311.1	01Jan3000, 13:36	65.6
DP2	0.7886	311.1	01Jan3000, 13:36	65.6
R3	0.7886	311.1	01Jan3000, 13:49	65.3
OC1	0.2016	345.7	01Jan3000, 13:02	24.0
C8	0.0927	91.0	01Jan3000, 13:14	10.1
C	0.0927	33.6	01Jan3000, 13:42	9.2
C4	0.0630	82.3	01Jan3000, 13:06	6.9
C9	0.0070	8.0	01Jan3000, 12:55	0.4
DP3	1.1529	459.8	01Jan3000, 13:03	105.8
R-4	1.1529	459.8	01Jan3000, 13:18	105.4
OC2	0.1277	197.7	01Jan3000, 13:02	13.7
C12	0.0413	57.5	01Jan3000, 13:05	4.5
OD1	0.0377	20.2	01Jan3000, 13:13	2.2
R-D	0.0377	20.2	01Jan3000, 13:15	2.2
C13	0.0350	27.9	01Jan3000, 13:04	2.0
D	0.0160	35.8	01Jan3000, 12:56	2.0
DP4	1.4106	577.1	01Jan3000, 13:17	129.9

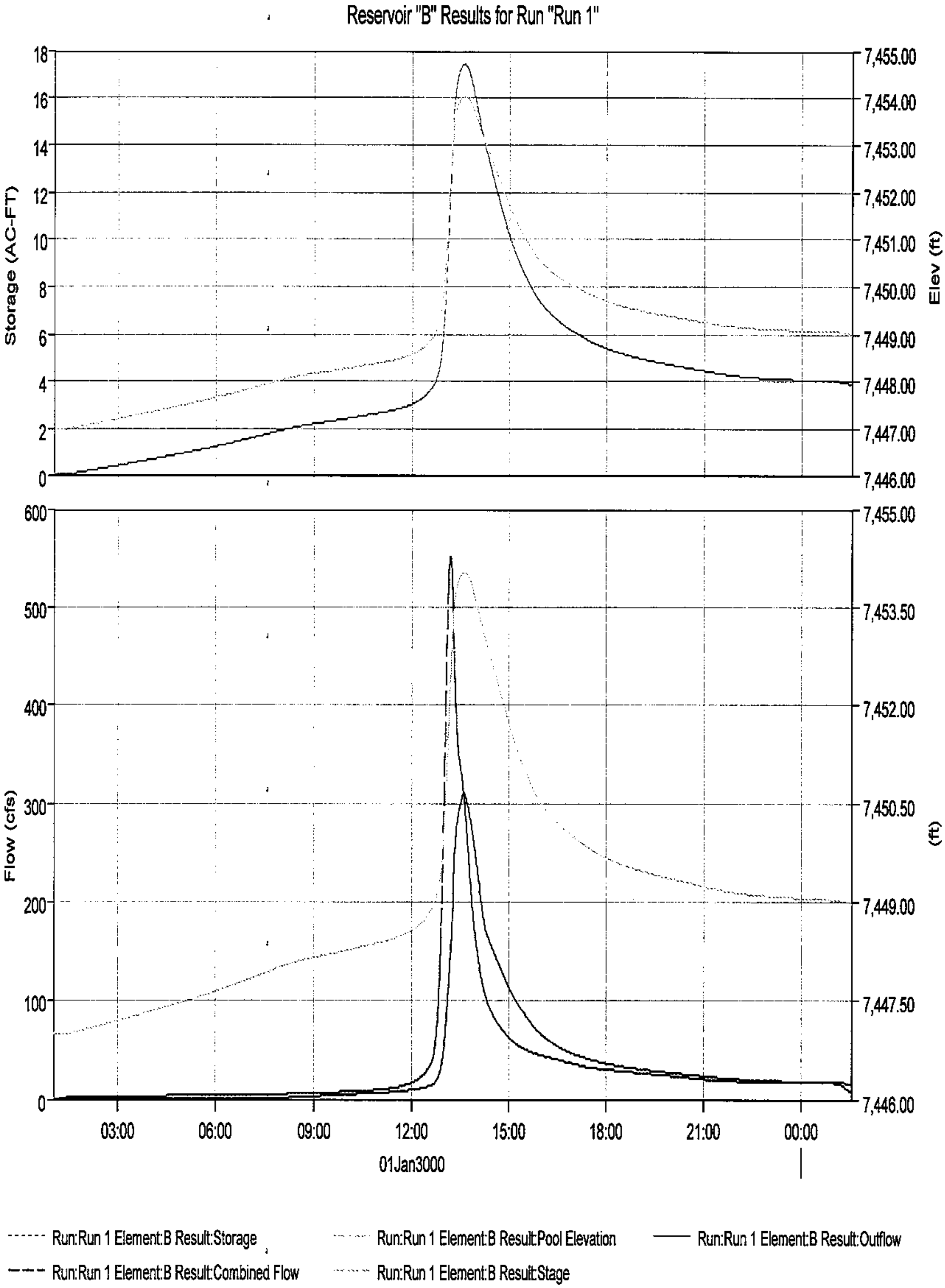
Project: WALD_DET
Simulation Run: Run 1 Reservoir: B

Start of Run:	01Jan3000, 01:00	Basin Model:	Basin 1
End of Run:	02Jan3000, 01:30	Meteorologic Model:	Met 1
Compute Time:	07Sep2014, 16:02:13	Control Specifications:	Control 1

Volume Units: AC-FT

Computed Results

Peak Inflow :	551.7 (CFS)	Date/Time of Peak Inflow :	01Jan3000, 13:11
Peak Outflow :	311.1 (CFS)	Date/Time of Peak Outflow :	01Jan3000, 13:36
Total Inflow :	69.4 (AC-FT)	Peak Storage :	17.4 (AC-FT)
Total Outflow :	65.6 (AC-FT)	Peak Elevation :	7454.0 (FT)



Reservoir "B" Results for Run "Run 1"
Storage (AC-FT)
Elev (ft)
Flow (cfs)
(ft)
0
2
4
6
8
10
12
14
16
18
7,446.00
7,447.00
7,448.00
7,449.00
7,450.00
7,451.00
7,452.00
7,453.00
7,454.00
7,455.00
0
100
200
300
400
500
600
7,446.00
7,447.50
7,449.00
7,450.50
7,452.00
7,453.50
7,455.00
03:00
06:00
09:00
12:00
15:00
18:00
21:00
00:00
01Jan3000
Run:Run 1 Element:B Result:Storage
Run:Run 1 Element:B Result:Pool Elevation
Run:Run 1 Element:B Result:Outflow
Run:Run 1 Element:B Result:Combined Flow
Run:Run 1 Element:B Result:Stage

APPENDIX B2

HYDROLOGIC CALCULATIONS – RATIONAL METHOD

(CU-Denver)

Determination of Time of Concentration

The time of concentration is the period of time required for stormwater to travel from the most remote point in the basin to the design point. In general, drainageways in an urban area may be composed of overland flow, gully (swale) flow, and channel (gutter) flow. There are many empirical formulas developed for estimating the time of concentration such as Kirpich, Kinematic Wave, Kerby etc.. They indicate that the time of concentration is a function of basin slope, length of waterway, roughness of waterway and rainfall amount. For an urban area, the airport empirical formula is recommended for estimating overland flow time:

* "Airport"

$$T_o = \frac{1.8\,(1.1 - C)\,L_o^{0.5}}{S_o^{0.33}} = \frac{1.8\,(1.1-C)\sqrt{L_o}}{\sqrt[3]{S_o}} \quad (4)$$

in which To = overland flow time in minutes, C5 = runoff coefficient for a 5-year storm, Lo = overland flow length in feet, So = overland flow slope in percent.

The U.S. Soil Conservation Service (SCS) upland method is recommended for estimating gully (swale) flow time and channel (gutter) flow time. The SCS upland method classifies the linings of drainageways into six categories. They are: (1) meadow, (2) tillage, (3) lawn, (4) bare soil, (5) grass, and (6) pavement. Flow velocity is approximated by

* "SCS Upland"

$$V = K\,S^{0.5} \;\rightarrow\; T_F = \frac{L_F}{K\sqrt{S_F}\,(60\ \text{s/m})} \quad (5)$$

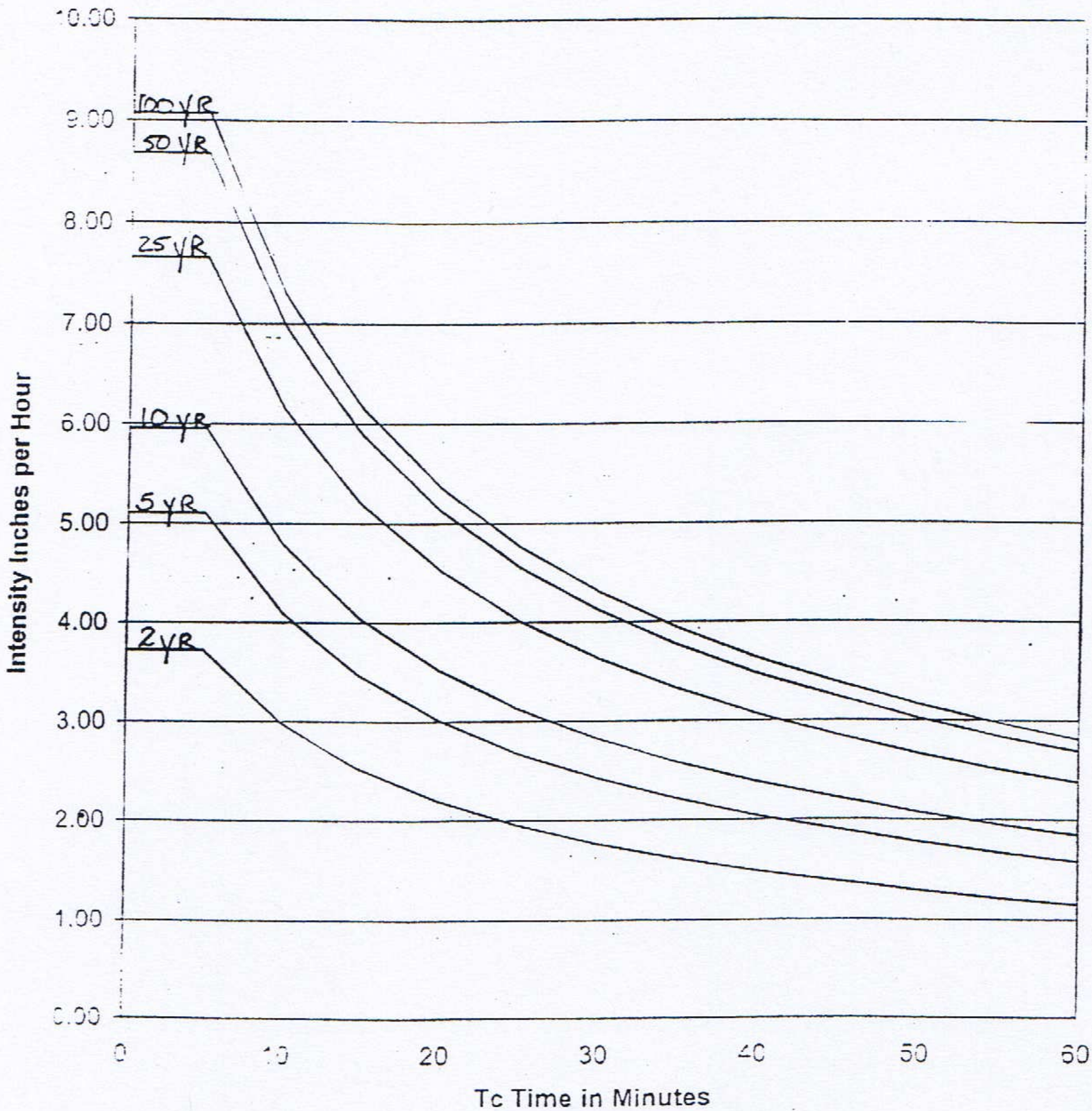
in which V = flow velocity in fps, S = drainageway slope in percent, and K = conveyance coefficient. The values of K for various lining materials are listed as follows: (2 impervious factor)

Linings	meadow	tillage	lawn	bare soil	grass	pavement
Value of K	.25	.45	.70	1.0	1.5	2.0

Applying the SCS upland method to swale and channel flows, we have

Ls

Storm Rainfall Time Intensity-Frequency Curves



Rainfall Depth - Duration - Frequency Table derived from Rainfall Atlas III for Colorado
Resource: Guo, James C.Y., (200?) "Urban Storm Water Modeling", Chapter 5: Runoff Prediction for Small Catchment, published by Auraria Campus Book Company, University of Colorado at Denver, Denver, Colorado.

TABLE 5-1

RECOMMENDED AVERAGE RUNOFF COEFFICIENTS AND PERCENT IMPERVIOUS

LAND USE OR SURFACE CHARACTERISTICS	PERCENT IMPERVIOUS	"C" FREQUENCY 10 A&B*	10 C&D*	100 A&B*	100 C&D*
Business					
Commercial Areas	95	0.90	0.90	0.90	0.90
Neighborhood Areas	70	0.75	0.75	0.80	0.80
Residential					
1/8 Acre or less	65	0.60	0.70	0.70	0.80
1/4 Acre	40	0.50	0.60	0.60	0.70
1/3 Acre	30	0.40	0.50	0.55	0.60
1/2 Acre	25	0.35	0.45	0.45	0.55
1 Acre	20	0.30	0.40	0.40	0.50
Industrial					
Light Areas	80	0.70	0.70	0.80	0.80
Heavy Areas	90	0.80	0.80	0.90	0.90
Parks and Cemeteries	7	0.30	0.35	0.55	0.60
Playgrounds	13	0.30	0.35	0.60	0.65
Railroad Yard Areas	40	0.50	0.55	0.60	0.65
Undeveloped Areas					
Historic Flow Analysis- Greenbelts, Agricultural	2	0.15	0.25	0.20	0.30
Pasture/Meadow	0	0.25	0.30	0.35	0.45
Forest	0	0.10	0.15	0.15	0.20
Exposed Rock	100	0.90	0.90	0.95	0.95
Offsite Flow Analysis (when land use not defined)	45	0.55	0.60	0.65	0.70
Streets					
Paved	100	0.90	0.90	0.95	0.95
Gravel	80	0.80	0.80	0.85	0.85
Drive and Walks	100	0.90	0.90	0.95	0.95
Roofs	90	0.90	0.90	0.95	0.95
Lawns	0	0.25	0.30	0.35	0.45

* Hydrologic Soil Group

9/30/90

WALDEN PRESERVE
COMPOSITE RUNOFF COEFFICIENTS

HISTORIC CONDITIONS

5-YEAR C VALUES

BASIN	TOTAL AREA (AC)	SOIL TYPE	(AC)	SUB-AREA 1 DEVELOPMENT/ COVER	C	AREA (AC)	SUB-AREA 2 DEVELOPMENT/ COVER	C	(AC)	SUB-AREA 3 DEVELOPMENT/ COVER	C	WEIGHTED C VALUE
OA1	232	B	232	PASTURE	0.25							0.250
A	105.1	B	31.5	FOREST	0.1	73.6	PASTURE	0.25				0.205
OA1,A	337.1											0.236
OB1	35.4	B	35.4	1/2-AC. LOTS	0.35							0.350
B	118.0	B	23.6	FOREST	0.1	94.4	PASTURE	0.25				0.220
OB1,B	153.4											0.250
OA1,OB1,A,B	490.5											0.240
OC1	122.5		122.5	1/2-AC. LOTS	0.35							0.350
OC2	61.4		61.4	1/2-AC. LOTS	0.35							0.350
C	190.2		190.2	FOREST	0.1							0.100
OC1,OC2,C	374.1											0.223
OA1-OC1,A-C	864.6											0.233
OD1	3.30	B	3.30	1-AC LOTS	0.3							0.300
OD2	4.50	B	4.50	FOREST	0.1							0.100
OD3	10.30	B	10.30	FOREST	0.1							0.100
OD4	6.00	B	6.00	FOREST	0.1							0.100
D	10.27	B	10.27	FOREST	0.1							0.100
OD1-OD4, D	34.37	B										0.119
OA1-OD1,A-D	898.9											0.228
E	14.7		14.74	PASTURE	0.25							0.250
F	7.7		7.71	PASTURE	0.25							0.250

100-YEAR C VALUES

BASIN	TOTAL AREA (AC)	SOIL TYPE	(AC)	SUB-AREA 1 DEVELOPMENT/ COVER	C	AREA (AC)	SUB-AREA 2 DEVELOPMENT/ COVER	C	(AC)	SUB-AREA 3 DEVELOPMENT/ COVER	C	WEIGHTED C VALUE
OA1	232	B	232	PASTURE	0.35							0.350
A	105.1	B	31.5	FOREST	0.15	73.6	PASTURE	0.35				0.290
OA1,A	337.1											0.331
OB1	35.4	B	35.4	1/2-AC. LOTS	0.45							0.450
B	118.0	B	23.6	FOREST	0.15	94.4	PASTURE	0.35				0.310
OB1,B	153.4											0.342
OA1,OB1,A,B	490.5											0.335
OC1	122.5		122.5	1/2-AC. LOTS	0.45							0.450
OC2	61.4		61.4	1/2-AC. LOTS	0.45							0.450
C	190.2		190.2	FOREST	0.15							0.150
OC1,OC2,C	374.1											0.297
OA1-OC1,A-C	864.6											0.319
OD1	3.30	B	3.30	1-AC LOTS	0.4							0.400
OD2	4.50	B	4.50	FOREST	0.15							0.150
OD3	10.30	B	10.30	FOREST	0.15							0.150
OD4	6.00	B	6.00	FOREST	0.15							0.150
D	10.27	B	10.27	FOREST	0.15							0.150
OD1-OD4, D	34.37	B										0.174
OA1-OD1,A-D	898.9											0.313
E	14.7		14.74	PASTURE	0.35							0.350
F	7.7		7.71	PASTURE	0.35							0.350

WALDEN PRESERVE
RATIONAL METHOD

HISTORIC FLOWS

BASIN	DESIGN POINT	AREA (AC)	C 5-YEAR[7]	C 100-YEAR[7]	OVERLAND LENGTH (FT)	SLOPE (%)	Tco[1] (MIN)	CHANNEL LENGTH (FT)	CONVEYANCE COEFFICIENT K	SLOPE (%)	SCS[2] VELOCITY (FT/S)	Tt[3] (MIN)	TOTAL Tc[4] (MIN)	INTENSITY[5] 5-YR (IN/HR)	INTENSITY[5] 100-YR (IN/HR)	PEAK FLOW Q5[6] (CFS)	PEAK FLOW Q100[6] (CFS)
OA1		232.0	0.250	0.350	500	6.0	18.8	2900	1.50	4.1	3.04	15.9	34.8	2.22	3.96	129.00	321.47
A		105.1	0.205	0.290			0.0	2650	1.50	2.45	2.35	18.8	18.8	3.11	5.53	66.97	168.63
OA1,A	1	337.1	0.236	0.331									53.6	1.70	3.03	135.53	338.34
OB1		35.4	0.350	0.450			0.0	1600	1.50	4.7	3.25	8.2	8.2	4.41	7.84	54.60	124.96
B		118.0	0.220	0.310			0.0	1900	1.50	2.37	2.31	13.7	13.7	3.60	6.42	93.56	234.67
OB1,B		153.4	0.250	0.342									13.7	3.60	6.42	138.22	336.56
OA1,OB1,A,B	2	490.5	0.240	0.335									67.3	1.47	2.61	172.87	429.51
OC1		122.5	0.350	0.450			0.0	2800	1.50	4.64	3.23	14.4	14.4	3.52	6.27	151.01	345.59
OC2		61.4	0.350	0.450			0.0	2800	1.50	4.64	3.23	14.4	14.4	3.52	6.27	75.69	173.22
C		190.2	0.100	0.150			0.0	5450	1.50	1.65	1.93	47.1	47.1	1.85	3.29	35.12	93.78
OC1,OC2,C		374.1	0.223	0.297									47.1	1.85	3.29	154.08	365.26
OA1-OC2,A-C		864.6	0.233	0.319									114.4	1.02	1.82	205.98	501.97
OD1		3.30	0.30	0.40	370	5.4	15.8	450	1.50	4.4	3.15	2.4	18.2	3.16	5.63	3.13	7.43
OD2		4.50	0.10	0.15	525	8.2	20.5	0				0.0	20.5	2.98	5.30	1.34	3.58
OD3		10.30	0.10	0.15	700	5.7	26.7	580	1.50	3.1	2.64	3.7	30.3	2.41	4.28	2.48	6.62
OD4		6.00	0.10	0.15	630	5.6	25.5	340	1.50	0.9	1.42	4.0	29.4	2.45	4.36	1.47	3.92
D		10.27	0.10	0.15			0.0	750	1.50	3.5	2.81	4.5	5.0	5.10	9.09	5.24	14.00
OD1-OD4, D	4	34.37	0.12	0.17									34.8	2.22	3.96	9.17	23.12
OA1-OD1,A-D	3	898.9	0.228	0.313									114.4	1.02	1.82	209.57	512.11
E	5	14.7	0.250	0.350	800	4.4	26.4	0				0.0	26.4	2.60	4.63	9.58	23.89
F	6	7.7	0.250	0.350	1000	7.0	25.3	0				0.0	25.3	2.66	4.74	5.13	12.79

1) OVERLAND FLOW Tco = (1.87*(1.1-RUNOFF COEFFICIENT)*(OVERLAND FLOW LENGTH^(0.5)/(SLOPE^(0.333))
2) SCS VELOCITY = K * ((SLOPE(%))^0.5)
K = 0.25 FOR MEADOW
K = 1.5 FOR GRASS CHANNEL
K = 2.0 FOR PAVEMENT
3) GUTTER/SWALE FLOW, Tcg = (GUTTER LENGTH/ SCS VELOCITY) / 60 SEC
4) Tc = Tco + Tcg
*** IF TOTAL TIME OF CONCENTRATION IS LESS THAN 5 MINUTES, THEN 5 MINUTES IS USED
5) INTENSITY BASED ON I-D-F CURVE IN EL PASO COUNTY DRAINAGE CRITERIA MANUAL, REVISED BY CITY OF COLORADO SPRINGS 1/1/03
I = (A * P) / B + Td)^C
5-YEAR VALUES: A = 26.65; P1 = 1.5 IN (1-HOUR DEPTH); B = 10.0; C = 0.76
100-YEAR VALUES: A = 26.65; P = 2.67 IN (1-HOUR DEPTH); B = 10.0; C = 0.76
6) Q = CiA
7) WEIGHTED AVERAGE C VALUES FOR COMBINED BASINS

WALDEN PRESERVE
COMPOSITE RUNOFF COEFFICIENTS

DEVELOPED CONDITIONS

5-YEAR C VALUES

BASIN	TOTAL AREA (AC)	SOIL TYPE	(AC)	SUB-AREA 1 DEVELOPMENT/ COVER	C	AREA (AC)	SUB-AREA 2 DEVELOPMENT/ COVER	C	(AC)	SUB-AREA 3 DEVELOPMENT/ COVER	C	WEIGHTED C VALUE
OA1,A1-A9	341.96	B										0.266
OB1,B1-B4	90.90	B										0.350
B5	4.78	B	4.78	2.5-AC. LOTS	0.3							0.300
B6	7.91	B	7.91	2.5-AC. LOTS	0.3							0.300
B5,B6	12.69	B										0.300
B7	3.24	B	3.24	1-AC LOTS	0.3							0.300
B8	28.74	B	28.74	1-AC LOTS	0.3							0.300
B5-B8	44.67	B										0.300
B9	10.23	B	10.23	1-AC LOTS	0.3							0.300
B10	16.90	B	71.17	1-AC LOTS	0.3							0.300
B5-B10	71.80	B										0.300
OB1,B1-B10	162.70	B										0.328
OA1,OB1,A,B	504.66	B										0.286
OC1	128.95	B	128.95	1/2-AC. LOTS	0.35							0.350
C1-C4	40.40	B	40.4	1-AC LOTS	0.3							0.300
C5	41.43	B	41.43	1-AC LOTS	0.3							0.300
C6	1.32	B	1.32	1-AC LOTS	0.3							0.300
C8	12.44	B	12.44	1-AC LOTS	0.3							0.300
C5,C6,C8	55.19	B										0.300
C9	4.50	B	4.5	1-AC LOTS	0.3							0.300
OC1,C1-C9	229.04	B										0.328
OA1-OC1,A1-C9	733.70	B										0.299
OC2	81.72	B	81.72	1/2-AC. LOTS	0.35							0.350
C10	2.86	B	2.86	1-AC LOTS	0.3							0.300
C11	8.63	B	8.63	1-AC LOTS	0.3							0.300
C10-C11	11.49	B										0.300
C12	17.77	B	17.77	1-AC LOTS	0.3							0.300
C10-C12	29.26	B										0.300
C13	22.44	B	22.44	MEADOW	0.25							0.250
OC2,C10-C13	133.42	B										0.322
OD1	3.30	B	3.30	1-AC LOTS	0.3							0.300
OD2	4.50	B	4.50	FOREST	0.1							0.100
OD3	10.30	B	10.30	FOREST	0.1							0.100
OD4	6.00	B	6.00	FOREST	0.1							0.100
D	10.27	B	10.27	1-AC LOTS	0.3							0.300
OD1-OD4, D	34.37	B										0.179
OA1-OD1,A-D	901.49	B										0.298

WALDEN PRESERVE
COMPOSITE RUNOFF COEFFICIENTS

DEVELOPED CONDITIONS

100-YEAR C VALUES

BASIN	TOTAL AREA (AC)	SOIL TYPE	(AC)	SUB-AREA 1 DEVELOPMENT/ COVER	C	AREA (AC)	SUB-AREA 2 DEVELOPMENT/ COVER	C	(AC)	SUB-AREA 3 DEVELOPMENT/ COVER	C	WEIGHTED C VALUE
OA1,A1-A9	341.96	B										0.366
OB1,B1-B4	90.90	B										0.450
B5	4.78	B	4.78	2.5-AC. LOTS	0.4							0.400
B6	7.91	B	7.91	2.5-AC. LOTS	0.4							0.400
B5,B6	12.69	B										0.400
B7	3.24	B	3.24	1-AC LOTS	0.4							0.400
B8	28.74	B	28.74	1-AC LOTS	0.4							0.400
B5-B8	44.67	B										0.400
B9	10.23	B	10.23	1-AC LOTS	0.4							0.400
B10	16.90	B	71.17	1-AC LOTS	0.4							0.400
B5-B10	71.80	B										0.400
OB1,B1-B10	162.70	B										0.428
OA1,OB1,A,B	504.66	B										0.386
OC1	128.95	B	128.95	1/2-AC. LOTS	0.45							0.450
C1-C4	40.40	B	40.4	1-AC LOTS	0.4							0.400
C5	41.43	B	41.43	1-AC LOTS	0.4							0.400
C6	1.32	B	1.32	1-AC LOTS	0.4							0.400
C8	12.44	B	12.44	1-AC LOTS	0.4							0.400
C5,C6,C8	55.19	B										0.400
C9	4.50	B	4.5	1-AC LOTS	0.4							0.400
OC1,C1-C9	229.04	B										0.428
OA1-OC1,A1-C9	733.70	B										0.399
OC2	81.72	B	81.72	1/2-AC. LOTS	0.45							0.450
C10	2.86	B	2.86	1-AC LOTS	0.4							0.400
C11	8.63	B	8.63	1-AC LOTS	0.4							0.400
C10-C11	11.49	B										0.400
C12	17.77	B	17.77	1-AC LOTS	0.4							0.400
C10-C12	29.26	B										0.400
C13	22.44	B	22.44	MEADOW	0.35							0.350
OC2,C10-C13	133.42	B										0.422
OD1	3.30	B	3.30	1-AC LOTS	0.4							0.400
OD2	4.50	B	4.50	FOREST	0.15							0.150
OD3	10.30	B	10.30	FOREST	0.15							0.150
OD4	6.00	B	6.00	FOREST	0.15							0.150
D	10.27	B	10.27	1-AC LOTS	0.4							0.400
OD1-OD4, D	34.37	B										0.249
OA1-OD1,A-D	901.49	B										0.397

RATIONAL METHOD - DRAINAGE CALCULATIONS
DEVELOPED FLOWS

BASIN	DESIGN POINT	AREA (AC)	C 5-YEAR[7]	C 100-YEAR[7]	OVERLAND LENGTH (FT)	SLOPE (%)	Tco[1] (MIN)	CHANNEL LENGTH (FT)	CONVEYANCE COEFFICIENT K	SLOPE (%)	SCS[2] VELOCITY (FT/S)	Tt[3] (MIN)	TOTAL Tc[4] (MIN)	TOTAL Tc[4] (MIN)	INTENSITY[5] 5-YR (IN/HR)	INTENSITY[5] 100-YR (IN/HR)	PEAK FLOW Q5[6] (CFS)	PEAK FLOW Q100[6] (CFS)
OA1,A1-A9	**1**	**341.96**	**0.266**	**0.366**									**55.8**	**55.8**	1.66	2.95	**150.94**	**369.67**
OB1,B1-B4	B4	90.90	0.350	0.450									21.1	21.1	2.93	5.22	93.31	213.54
B5	B5	4.78	0.300	0.400	300	3.7	16.1	270	1.50	5.6	3.55	1.3	17.4	17.4	3.23	5.75	4.63	10.99
B6		7.91	0.300	0.400			0.0	380	1.50	10.0	4.74	1.3	1.3	5.0	5.10	9.09	12.11	28.75
B5,B6	B6	12.69	0.300	0.400									18.7	18.7	3.11	5.54	11.86	28.14
B7	B7	3.24	0.300	0.400	300	6.7	13.2	150	1.50	8	4.24	0.6	13.8	13.8	3.59	6.39	3.49	8.28
WP2 FILING 4:																		
B8		28.74	0.300	0.400			0.0	1250	1.50	4.8	3.29	6.3	6.3	6.3	4.78	8.51	41.24	97.88
B5-B8	B8	44.67	0.300	0.400									25.1	25.1	2.68	4.76	35.87	85.13
B9	B9	10.23	0.300	0.400	300	4.7	14.9	540	1.50	7.4	4.08	2.2	17.1	17.1	3.26	5.80	9.99	23.72
B10		16.90	0.300	0.400			0.0	600	1.50	3.3	2.72	3.7	3.7	5.0	5.10	9.09	25.88	61.42
B5-B10	B10	71.80	0.300	0.400									28.7	28.7	2.48	4.42	53.46	126.87
Tt from DP1 to DP2								1900	1.50	2.4	2.31	13.7	13.7					
OB1,B1-B10		162.70	0.328	0.428									25.1	25.1	2.68	4.76	142.84	331.77
OA1,OB1,A,B	**2**	**504.66**	**0.286**	**0.386**									**69.5**	**69.5**	1.50	2.65	**216.50**	**516.22**
OC1.1		121.57	0.350	0.450			0.0	2800	1.50	4.64	3.23	14.4	14.4	14.4	3.52	6.27	149.86	342.97
OC1.2		7.38	0.350	0.450			0.0	2800	1.50	4.64	3.23	14.4	14.4	14.4	3.52	6.27	9.10	20.82
OC1.1,OC1.2	OC1	**128.95**	0.350	0.450			0.0	2800	1.50	4.64	3.23	14.4	14.4	14.4	3.52	6.27	158.96	363.79
WP2 FILING 5:																		
C1	C1	11.75	0.300	0.400	300	4.0	15.7	550	1.50	8.2	4.30	2.1	17.9	17.9	3.19	5.68	11.24	26.68
C2		11.02	0.300	0.400			0.0	820	1.50	5.5	3.52	3.9	3.9	5.0	5.10	9.09	16.88	40.05
C1,C2	C2	22.77	0.300	0.400									21.7	21.7	2.89	5.14	19.73	46.82
C3	C3	2.04	0.300	0.400	100	2.0	11.4	500	1.50	3.6	2.85	2.9	14.4	14.4	3.53	6.29	2.16	5.13
C4		15.59	0.300	0.400	100	2.0	11.4	900	1.50	6.9	3.94	3.8	15.2	15.2	3.44	6.12	16.08	38.15
C3,C4	C4A	17.63	0.300	0.400									15.2	15.2	3.44	6.12	18.18	43.15
C1-C4	C4	**40.40**	0.300	0.400									21.7	21.7	2.89	5.14	35.00	83.07
WP2 FILINGS 6-7:																		
C5	C5	41.43	0.300	0.400	100	4.0	9.1	1800	1.50	4.6	3.22	9.3	18.4	18.4	3.14	5.59	39.06	92.69
C6	C6	1.32	0.300	0.400	100	6.0	7.9	250	1.50	8	4.24	1.0	8.9	8.9	4.28	7.62	1.70	4.02
C8		12.44	0.300	0.400	100	8.0	7.2	1200	1.50	5.5	3.52	5.7	12.9	12.9	3.70	6.59	13.82	32.79
Tt from C5 to C8								550	1.50	3.6	2.85	3.2	3.2					
C5,C6,C8 (POND C8)	C8	55.19	0.300	0.400									21.6	21.6	2.90	5.15	47.95	113.80
C9		4.50	0.300	0.400			0.0	360	1.50	3.6	2.85	2.1	2.1	5.0	5.10	9.09	6.89	16.36
Tt from DP2 to DP3								2700	1.50	1.9	2.04	22.1	22.1					
OC1,C1-C9	**3**	**733.70**	**0.299**	**0.399**									**91.6**	**91.6**	1.50	2.65	**329.06**	**775.78**

BASIN	DESIGN POINT	AREA (AC)	C 5-YEAR[7]	C 100-YEAR [7]	OVERLAND LENGTH (FT)	SLOPE (%)	Tco [1] (MIN)	CHANNEL LENGTH (FT)	CONVEYANCE COEFFICIENT K	SLOPE (%)	SCS [2] VELOCITY (FT/S)	Tt [3] (MIN)	TOTAL Tc [4] (MIN)	TOTAL Tc [4] (MIN)	INTENSITY [5] 5-YR (IN/HR)	INTENSITY [5] 100-YR (IN/HR)	PEAK FLOW Q5 [6] (CFS)	PEAK FLOW Q100 [6] (CFS)
OC2.1		62.48	0.350	0.450			0.0	2800	1.50	4.64	3.23	14.4	14.4	14.4	3.52	6.27	77.02	176.26
OC2.2		19.24	0.350	0.450			0.0	2800	1.50	4.64	3.23	14.4	14.4	14.4	3.52	6.27	23.72	54.28
OC2.1,OC2.2		81.72	0.350	0.450			0.0	2800	1.50	4.64	3.23	14.4	14.4	14.4	3.52	6.27	100.74	230.54
C10	C10	2.86	0.300	0.400	100	6.0	7.9	600	1.50	4.0	3.00	3.3	11.3	11.3	3.92	6.97	3.36	7.97
C11		8.63	0.300	0.400	100	6.0	7.9	600	1.50	6.7	3.88	2.6	10.5	10.5	4.03	7.16	10.42	24.73
Tt from C10 to C11								400	1.50	2.5	2.37	2.8	2.8					
C11-C12	C11	11.49	0.300	0.400									14.1	14.1	3.56	6.34	12.28	29.15
C12		17.77	0.300	0.400	100	10.0	6.7	1300	1.50	3.1	2.64	8.2	14.9	14.9	3.47	6.18	18.52	43.95
C10-C12	C12	29.26	0.300	0.400									14.9	14.9	3.47	6.18	30.49	72.36
C13	C13	22.44	0.250	0.350	300	7.3	13.7	430	1.50	8.6	4.40	1.6	15.3	15.3	3.43	6.11	19.25	47.97
OC1,OC2,C1-C13		867.12	0.323	0.423									106.5	106.5	1.50	2.65	420.12	972.00
OD1		3.30	0.300	0.400	370	5.4	15.8	450	1.50	4.4	3.15	2.4	18.2	18.2	3.16	5.63	3.13	7.43
OD2		4.50	0.100	0.150	525	8.2	20.5	0				0.0	20.5	20.5	2.98	5.30	1.34	3.58
OD3		10.30	0.100	0.150	700	5.7	26.7	580	1.50	3.1	2.64	3.7	30.3	30.3	2.41	4.28	2.48	6.62
OD4		6.00	0.100	0.150	630	5.6	25.5	340	1.50	0.9	1.42	4.0	29.4	29.4	2.45	4.36	1.47	3.92
OD1-OD4		**24.10**											30.3	30.3				
D		**10.27**	0.300	0.400			0.0	750	1.50	3.5	2.81	4.5	5.0	5.0	5.10	9.09	15.73	37.33
OD1-OD4, D	D	**34.37**	0.179	0.249									34.8	34.8	2.22	3.96	13.67	33.86
G1	G1	3.28	0.300	0.400	100	6.0	7.9	300	1.50	6.7	3.88	1.3	9.2	9.2	4.23	7.53	4.16	9.88
Tt from DP3 to DP4								2750	1.50	1.5	1.81	25.4	25.4					
OA1-OD1,A-D	**4**	**901.49**	**0.298**	**0.397**									**116.9**	**116.9**	1.50	2.65	**402.97**	**948.41**

1) OVERLAND FLOW Tco = (1.87*(1.1-RUNOFF COEFFICIENT)*(OVERLAND FLOW LENGTH^(0.5)/(SLOPE^(0.333))
2) SCS VELOCITY = K * ((SLOPE(%))^0.5)
K = 0.70 FOR MEADOW / FOREST
K = 1.0 FOR BARE SOIL
K = 1.5 FOR GRASS CHANNEL
K = 2.0 FOR PAVEMENT
3) GUTTER/SWALE FLOW, TRAVEL TIME, Tt = (CHANNEL LENGTH/ SCS VELOCITY) / 60 SEC
4) Tc = Tco + Tt
*** IF TOTAL TIME OF CONCENTRATION IS LESS THAN 5 MINUTES, THEN 5 MINUTES IS USED
5) INTENSITY BASED ON I-D-F CURVE IN EL PASO COUNTY DRAINAGE CRITERIA MANUAL, REVISED BY CITY OF COLORADO SPRINGS 1/1/03
I = (A * P) / B + Td)^C
5-YEAR VALUES: A = 26.65; P1 = 1.5 IN (1-HOUR DEPTH); B = 10.0; C = 0.76
100-YEAR VALUES: A = 26.65; P = 2.67 IN (1-HOUR DEPTH); B = 10.0; C = 0.76
6) Q = CiA
7) WEIGHTED AVERAGE C VALUES FOR COMBINED BASINS

APPENDIX C1

HYDRAULIC CALCULATIONS – OPEN CHANNELS

TABLE 10-4

MAXIMUM PERMISSIBLE VELOCITIES FOR EARTH CHANNELS WITH VARIED GRASS LININGS AND SLOPES

Channel Slope	Lining	Permissible Mean Channel Velocity * (ft/sec)
0 - 5%	Sodded grass	7
	Bermudagrass	6
	Reed canarygrass	5
	Tall fescue	5
	Kentucky bluegrass	5
	Grass-legume mixture	4
	Red fescue	2.5
	Redtop	2.5
	Sericea lespedeza	2.5
	Annual lespedeza	2.5
	Small grains (temporary)	2.5
5 - 10%	Sodded grass	6
	Bermudagrass	5
	Reed canarygrass	4
	Tall fescue	4
	Kentucky bluegrass	4
	Grass-legume mixture	3
Greater than 10%	Sodded grass	5
	Bermudagrass	4
	Reed canarygrass	3
	Tall fescue	3
	Kentucky bluegrass	3

* For highly erodible soils, decrease permissible velocities by 25%.

* Grass lined channels are dependent upon assurances of continuous growth and maintenance of grass.

ALLOWABLE VELOCITY AND MAXIMUM SHEAR STRESS
Streambank and Shoreland Protection Code 580

Type of Treatment	Allowable Shear lb/sq ft	Velocity ft/sec
Brush Mattresses[1]		
Staked only w/ rock riprap toe (initial)	0.8 - 4.1	5
Staked only w/ rock riprap toe (grown)	4.0 - 8.0	12
Coir Geotextile Roll[2]		
Roll with coir rope mesh staked only without rock riprap toe	0.2 - 0.8	< 5
Roll with Polypropylene rope mesh staked only without rock riprap toe	0.8 - 3.0	< 8
Roll with Polypropylene rope mesh staked and with rock riprap toe	3.0 - 4.0	< 12
Live Fascine[3]		
LF Bundle w/ rock riprap toe	2.0 - 3.1	8
Soils[4]		
Fine colloidal sand	0.02-0.03	1.5
Sandy loam (noncolloidal)	0.03-0.04	1.75
Alluvial silt (noncolloidal)	0.045-0.05	2
Silty loam (noncolloidal)	0.045-0.05	1.75-2.25
Firm loam	0.075	2.5
Fine gravels	0.075	2.5
Stiff clay	0.26	3-4.5
Alluvial silt (colloidal)	0.26	3.75
Graded loam to cobbles	0.38	3.75
Graded silts to cobbles	0.43	4
Shales and hardpan	0.67	6
Gravel/Cobble[4]		
1-inch	0.33	2.5-5
2-inch	0.67	3-6
6-inch	2	4-7.5
12-inch	4	5.5-12
Vegetation[4]		
Class A turf (ret class)	3.7	6-8
Class B turf (ret class)	2.1	4-7
Class C turf (ret class)	1	3.5
Retardance Class D	0.6	Design of roadside channels HEC-15
Retardance Class E	0.35	
Long native grasses	1.2-1.7	4-6
Short native and bunch grass	0.7-0.95	3-4

The complete line of RollMax™ products offers a variety of options for both short-term and permanent erosion control needs. Reference the RollMax Products Chart below to find the right solution for your next project.

RollMax Product Selection Chart

	TEMPORARY						
	ERONET						BIONET
							
	DS75	**DS150**	**S75**	**S150**	**SC150**	**C125**	**S75BN**
Longevity	45 days	60 days	12 mo.	12 mo.	24 mo.	36 mo.	12 mo.
Applications	Low Flow Channels 4:1-3:1 Slopes	Moderate Flow Channels 3:1-2:1 Slopes	Low Flow Channels 4:1-3:1 Slopes	Moderate Flow Channels 3:1-2:1 Slopes	Medium Flow Channels 2:1-1:1 Slopes	High-Flow Channels 1:1 and Greater Slopes	Low Flow Channels 4:1-3:1 Slopes
Design Permissible Shear Stress lbs/ft² (Pa)	Unvegetated 1.55 (74)	Unvegetated 1.75 (84)	Unvegetated 1.55 (74)	Unvegetated 1.75 (84)	Unvegetated 2.00 (96)	Unvegetated 2.25 (108)	Unvegetated 1.60 (76)
Design Permissible Velocity ft/s (m/s)	Unvegetated 5.00 (1.52)	Unvegetated 6.00 (1.52)	Unvegetated 5.00 (1.2)	Unvegetated 6.00 (1.83)	Unvegetated 8.00 (2.44)	Unvegetated 10.00 (3.05)	Unvegetated 5.00 (1.52)
Top Net	Lightweight accelerated photodegradable polypropylene 1.50 lbs/1000 ft² (0.73 kg/100 m²) approx wt	Lightweight accelerated photodegradable polypropylene 1.50 lbs/1000 ft² (0.73 kg/100 m²) approx wt	Lightweight photodegradable polypropylene 1.50 lbs/1000 ft² (0.73 kg/100 m²) approx wt	Lightweight photodegradable polypropylene 1.50 lbs/1000 ft² (0.73 kg/100 m²) approx wt	Heavyweight UV-stabilized polypropylene 2.9 lbs/1000 ft² (1.47 kg/100 m²) approx wt	Heavyweight UV-stabilized polypropylene 2.9 lbs/1000 ft² (1.47 kg/100 m²) approx wt	Leno woven. 100% biodegradable jute fiber 9.30 lbs/1000 ft² (4.53 kg/100 m²) approx wt
Center Net	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Fiber Matrix	Straw fiber 0.50 lbs/yd² (0.27 kg/m²)	Straw fiber 0.50 lbs/yd² (0.27 kg/m²)	Straw fiber 0.50 lbs/yd² (0.27 kg/m²)	Straw fiber 0.50 lbs/yd² (0.27 kg/m²)	Straw/coconut matrix 70% Straw 0.35 lbs/yd² (0.19 kg/m²) 30% Coconut 0.15 lbs/yd² (0.08 kg/m²)	Coconut fiber 0.50 lbs/yd² (0.27 kg/m²)	Straw fiber 0.50 lbs/yd² (0.27 kg/m²)
Bottom Net	N/A	Lightweight accelerated photodegradable polypropylene 1.50 lbs/1000 ft² (0.73 kg/100 m²) approx wt	N/A	Lightweight photodegradable polypropylene 1.50 lbs/1000 ft² (0.73 kg/100 m²) approx wt	Lightweight photodegradable polypropylene 1.50 lbs/1000 ft² (0.73 kg/100 m²) approx wt	Heavyweight UV-stabilized polypropylene 2.9 lbs/1000 ft² (1.47 kg/100 m²) approx wt	N/A
Thread	Accelerated degradable	Accelerated degradable	Degradable	Degradable	Degradable	UV-stabilized polypropylene	Biodegradable

WALDEN PRESERVE 2 - FILING NO. 4
DITCH CALCULATION SUMMARY

PROPOSED ROADSIDE DITCHES

ROADWAY	FROM STA	TO STA	SIDE	PROPOSED SLOPE (%)	SIDE SLOPE (Z)	CHANNEL DEPTH (FT)	FRICTION FACTOR (n)	ROW WIDTH (ft)	BASIN	Q100 FLOW (CFS)	DITCH FLOW % OF BASIN	DITCH FLOW (CFS)	Q100 DEPTH (FT)	Q100 VELOCITY (FT/S)	DITCH LINING
PINEHURST CIRCLE	54+42	61+00	N	8.00	4:1/3:1	2.5	0.030	60	B9	23.7	5	1.2	0.3	3.8	GRASS
PINEHURST CIRCLE	54+42	61+00	S	8.00	4:1/3:1	2.5	0.030	60	B8	97.9	5	4.9	0.5	5.5	**GRASS / ECB**
PINEHURST CIRCLE	51+40	54+42	W	4.00	4:1/3:1	2.5	0.030	60	B9	23.7	85	20.1	1.0	6.0	**GRASS / ECB**
PINEHURST CIRCLE	51+40	54+42	E	4.00	4:1/3:1	2.5	0.030	60	B10	61.4	5	3.1	0.5	3.8	GRASS
PINEHURST CIRCLE	49+01	51+40	W	1.62	4:1/3:1	2.5	0.030	60	B9	23.7	15	3.6	0.6	2.8	GRASS
PINEHURST CIRCLE	49+01	51+40	E	1.62	4:1/3:1	2.5	0.030	60	B10	61.4	5	3.1	0.6	2.7	GRASS
PINEHURST CIRCLE	46+93	49+01	W	3.66	4:1/3:1	2.5	0.030	60	C1	30.8	15	4.6	0.6	4.0	**GRASS / ECB**
PINEHURST CIRCLE	46+93	49+01	E	3.66	4:1/3:1	2.5	0.030	60	C2	42.7	5	2.1	0.4	3.3	GRASS
DEBOODT COURT	10+00	10+50	N	2.00	4:1/3:1	2.5	0.030	60	B10	61.4	5	3.1	0.6	2.9	GRASS
DEBOODT COURT	10+00	10+50	S	2.00	4:1/3:1	2.5	0.030	60	B8	97.9	10	9.8	0.9	3.9	GRASS
DEBOODT COURT	10+50	14+67	N	1.16	4:1/3:1	2.5	0.030	60	B10	61.4	5	3.1	0.6	2.4	GRASS
DEBOODT COURT	10+50	14+67	S	1.16	4:1/3:1	2.5	0.030	60	B8	97.9	15	14.7	1.1	3.5	GRASS

1) Channel flow calculations based on Manning's Equation
2) n = 0.03 for grass-lined non-irrigated channels
3) Vmax = 4.0 fps for 100-year flows w/ native grass-lined channels (per ECM Table 10-4 & NRCS Companion Document 580-10)
4) Vmax = 8.0 fps for 100-year flows w/ Erosion Control Blankets (Tensar Eronet SC150 or equal)

Hydraulic Analysis Report

Project Data

Project Title: Walden-WP2-F4-Roadside-Ditches

Designer: JPS

Project Date: Monday, January 21, 2019

Project Units: U.S. Customary Units

Notes:

Channel Analysis: Pinehurst-5442-6100-N

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0800 ft/ft

Manning's n: 0.0300

Flow: 1.2000 cfs

Result Parameters

Depth: 0.2988 ft

Area of Flow: 0.3124 ft^2

Wetted Perimeter: 2.1767 ft

Hydraulic Radius: 0.1435 ft

Average Velocity: 3.8408 ft/s

Top Width: 2.0914 ft

Froude Number: 1.7512

Critical Depth: 0.3754 ft

Critical Velocity: 2.4332 ft/s

Critical Slope: 0.0237 ft/ft

Critical Top Width: 2.68 ft

Calculated Max Shear Stress: 1.4915 lb/ft^2

Calculated Avg Shear Stress: 0.7165 lb/ft^2

Channel Analysis: Pinehurst-5442-6100-S

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0800 ft/ft

Manning's n: 0.0300

Flow: 4.9000 cfs

Result Parameters

Depth: 0.5064 ft

Area of Flow: 0.8975 ft^2

Wetted Perimeter: 3.6892 ft

Hydraulic Radius: 0.2433 ft

Average Velocity: 5.4597 ft/s

Top Width: 3.5447 ft

Froude Number: 1.9121

Critical Depth: 0.6590 ft

Critical Velocity: 3.2238 ft/s

Critical Slope: 0.0196 ft/ft

Critical Top Width: 4.71 ft

Calculated Max Shear Stress: 2.5279 lb/ft^2

Calculated Avg Shear Stress: 1.2144 lb/ft^2

Channel Analysis: Pinehurst-5140-5442-W

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0400 ft/ft

Manning's n: 0.0300

Flow: 20.1000 cfs

Result Parameters

Depth: 0.9790 ft

Area of Flow: 3.3548 ft^2

Wetted Perimeter: 7.1326 ft

Hydraulic Radius: 0.4703 ft

Average Velocity: 5.9915 ft/s

Top Width: 6.8532 ft

Froude Number: 1.5091

Critical Depth: 1.1590 ft

Critical Velocity: 4.2754 ft/s

Critical Slope: 0.0163 ft/ft

Critical Top Width: 8.28 ft

Calculated Max Shear Stress: 2.4437 lb/ft^2

Calculated Avg Shear Stress: 1.1740 lb/ft^2

Channel Analysis: Pinehurst-5140-5442-E

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0400 ft/ft

Manning's n: 0.0300

Flow: 3.1000 cfs

Result Parameters

Depth: 0.4857 ft

Area of Flow: 0.8256 ft^2

Wetted Perimeter: 3.5384 ft

Hydraulic Radius: 0.2333 ft

Average Velocity: 3.7547 ft/s

Top Width: 3.3998 ft

Froude Number: 1.3427

Critical Depth: 0.5487 ft

Critical Velocity: 2.9418 ft/s

Critical Slope: 0.0209 ft/ft

Critical Top Width: 3.92 ft

Calculated Max Shear Stress: 1.2123 lb/ft^2

Calculated Avg Shear Stress: 0.5824 lb/ft^2

Channel Analysis: Pinehurst-4901-5140-W

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0162 ft/ft

Manning's n: 0.0300

Flow: 3.6000 cfs

Result Parameters

Depth: 0.6086 ft

Area of Flow: 1.2963 ft^2

Wetted Perimeter: 4.4337 ft

Hydraulic Radius: 0.2924 ft

Average Velocity: 2.7772 ft/s

Top Width: 4.2600 ft

Froude Number: 0.8872

Critical Depth: 0.5825 ft

Critical Velocity: 3.0311 ft/s

Critical Slope: 0.0205 ft/ft

Critical Top Width: 4.16 ft

Calculated Max Shear Stress: 0.6152 lb/ft^2

Calculated Avg Shear Stress: 0.2955 lb/ft^2

Channel Analysis: Pinehurst-4901-5140-E

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0162 ft/ft

Manning's n: 0.0300

Flow: 3.1000 cfs

Result Parameters

Depth: 0.5754 ft

Area of Flow: 1.1587 ft^2

Wetted Perimeter: 4.1919 ft

Hydraulic Radius: 0.2764 ft

Average Velocity: 2.6753 ft/s

Top Width: 4.0277 ft

Froude Number: 0.8790

Critical Depth: 0.5487 ft

Critical Velocity: 2.9418 ft/s

Critical Slope: 0.0209 ft/ft

Critical Top Width: 3.92 ft

Calculated Max Shear Stress: 0.5816 lb/ft^2

Calculated Avg Shear Stress: 0.2794 lb/ft^2

Channel Analysis: Pinehurst-4693-4901-W

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0366 ft/ft

Manning's n: 0.0300

Flow: 4.6000 cfs

Result Parameters

Depth: 0.5726 ft

Area of Flow: 1.1476 ft^2

Wetted Perimeter: 4.1717 ft

Hydraulic Radius: 0.2751 ft

Average Velocity: 4.0083 ft/s

Top Width: 4.0083 ft

Froude Number: 1.3201

Critical Depth: 0.6425 ft

Critical Velocity: 3.1834 ft/s

Critical Slope: 0.0198 ft/ft

Critical Top Width: 4.59 ft

Calculated Max Shear Stress: 1.3078 lb/ft^2

Calculated Avg Shear Stress: 0.6283 lb/ft^2

Channel Analysis: Pinehurst-4693-4901-E

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0366 ft/ft

Manning's n: 0.0300

Flow: 2.1000 cfs

Result Parameters

Depth: 0.4267 ft

Area of Flow: 0.6374 ft^2

Wetted Perimeter: 3.1090 ft

Hydraulic Radius: 0.2050 ft

Average Velocity: 3.2948 ft/s

Top Width: 2.9872 ft

Froude Number: 1.2570

Critical Depth: 0.4696 ft

Critical Velocity: 2.7213 ft/s

Critical Slope: 0.0220 ft/ft

Critical Top Width: 3.36 ft

Calculated Max Shear Stress: 0.9746 lb/ft^2

Calculated Avg Shear Stress: 0.4682 lb/ft^2

Channel Analysis: Deboodt-1000-1050-N

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0200 ft/ft

Manning's n: 0.0300

Flow: 3.1000 cfs

Result Parameters

Depth: 0.5531 ft

Area of Flow: 1.0707 ft^2

Wetted Perimeter: 4.0295 ft

Hydraulic Radius: 0.2657 ft

Average Velocity: 2.8953 ft/s

Top Width: 3.8717 ft

Froude Number: 0.9702

Critical Depth: 0.5487 ft

Critical Velocity: 2.9418 ft/s

Critical Slope: 0.0209 ft/ft

Critical Top Width: 3.92 ft

Calculated Max Shear Stress: 0.6903 lb/ft^2

Calculated Avg Shear Stress: 0.3316 lb/ft^2

Channel Analysis: Deboodt-1000-1050-S

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0200 ft/ft

Manning's n: 0.0300

Flow: 9.8000 cfs

Result Parameters

Depth: 0.8516 ft

Area of Flow: 2.5385 ft^2

Wetted Perimeter: 6.2044 ft

Hydraulic Radius: 0.4091 ft

Average Velocity: 3.8606 ft/s

Top Width: 5.9614 ft

Froude Number: 1.0426

Critical Depth: 0.8695 ft

Critical Velocity: 3.7032 ft/s

Critical Slope: 0.0179 ft/ft

Critical Top Width: 6.21 ft

Calculated Max Shear Stress: 1.0628 lb/ft^2

Calculated Avg Shear Stress: 0.5106 lb/ft^2

Channel Analysis: Deboodt-1050-1467-N

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0120 ft/ft

Manning's n: 0.0300

Flow: 3.1000 cfs

Result Parameters

Depth: 0.6087 ft

Area of Flow: 1.2968 ft^2

Wetted Perimeter: 4.4346 ft

Hydraulic Radius: 0.2924 ft

Average Velocity: 2.3905 ft/s

Top Width: 4.2608 ft

Froude Number: 0.7636

Critical Depth: 0.5487 ft

Critical Velocity: 2.9418 ft/s

Critical Slope: 0.0209 ft/ft

Critical Top Width: 3.92 ft

Calculated Max Shear Stress: 0.4558 lb/ft^2

Calculated Avg Shear Stress: 0.2190 lb/ft^2

Channel Analysis: Deboodt-1050-1467-S

Notes:

Input Parameters

Channel Type: Triangular

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 3.0000 ft/ft

Longitudinal Slope: 0.0120 ft/ft

Manning's n: 0.0300

Flow: 14.7000 cfs

Result Parameters

Depth: 1.0911 ft

Area of Flow: 4.1671 ft^2

Wetted Perimeter: 7.9494 ft

Hydraulic Radius: 0.5242 ft

Average Velocity: 3.5277 ft/s

Top Width: 7.6380 ft

Froude Number: 0.8417

Critical Depth: 1.0226 ft

Critical Velocity: 4.0160 ft/s

Critical Slope: 0.0170 ft/ft

Critical Top Width: 7.31 ft

Calculated Max Shear Stress: 0.8170 lb/ft^2

Calculated Avg Shear Stress: 0.3925 lb/ft^2

WALDEN PRESERVE 2 - FILING NO. 4
CHANNEL CALCULATIONS
DEVELOPED FLOWS

PROPOSED CHANNELS

CHANNEL	DESIGN POINT	PROPOSED MAX. SLOPE (%)	BOTTOM WIDTH (B, FT)	SIDE SLOPE (Z)	CHANNEL DEPTH (FT)	FRICTION FACTOR (n)	EASEMENT WIDTH (ft)	Q100 FLOW (CFS)	Q100 DEPTH (FT)	Q100 VELOCITY (FT/S)	CHANNEL LINING
B8	B8	4.0	8	4:1	2.0	0.030	24	85.1	0.9	7.8	**ECB**
B9	B9	10.0	8	4:1	2.0	0.030	24	23.7	0.4	7.1	**ECB**
C1	C1	6.5	8	4:1	2.0	0.030		26.7	0.4	6.4	**ECB**
C2	C2	9.5	8	4:1	2.0	0.030		46.8	0.3	8.5	**ECB**

1) Channel flow calculations based on Manning's Equation
2) Channel depth includes 1' minimum freeboard
3) n = 0.03 for grass-lined non-irrigated channels (minimum)
4) n = 0.035 for riprap-lined channels
5) Vmax = 5.0 fps for 100-year flows w/ grass-lined channels
6) Vmax = 8.0 fps for 100-year flows w/ Erosion Control Blankets (Tensar Eronet SC150 or equal)
7) Vmax = 10.0 fps for 100-year flows w/ Erosion Control Blankets (Tensar Eronet C125 or equal)

Hydraulic Analysis Report

Project Data

Project Title: Walden-WP2-F4-Channels

Designer: JPS

Project Date: Thursday, September 6, 2018

Project Units: U.S. Customary Units

Notes:

Channel Analysis: Channel Analysis-B8

Notes:

Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 4.0000 ft/ft

Channel Width: 8.0000 ft

Longitudinal Slope: 0.0400 ft/ft

Manning's n: 0.0300

Flow: 85.1000 cfs

Result Parameters

Depth: 0.9321 ft

Area of Flow: 10.9316 ft^2

Wetted Perimeter: 15.6861 ft

Hydraulic Radius: 0.6969 ft

Average Velocity: 7.7848 ft/s

Top Width: 15.4566 ft

Froude Number: 1.6313

Critical Depth: 1.2305 ft

Critical Velocity: 5.3522 ft/s

Critical Slope: 0.0139 ft/ft

Critical Top Width: 17.84 ft

Calculated Max Shear Stress: 2.3265 lb/ft^2

Calculated Avg Shear Stress: 1.7395 lb/ft^2

Channel Analysis: Channel Analysis-B9

Notes:

Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 4.0000 ft/ft

Channel Width: 8.0000 ft

Longitudinal Slope: 0.1000 ft/ft

Manning's n: 0.0300

Flow: 23.7000 cfs

Result Parameters

Depth: 0.3544 ft

Area of Flow: 3.3377 ft^2

Wetted Perimeter: 10.9226 ft

Hydraulic Radius: 0.3056 ft

Average Velocity: 7.1006 ft/s

Top Width: 10.8353 ft

Froude Number: 2.2546

Critical Depth: 0.5847 ft

Critical Velocity: 3.9202 ft/s

Critical Slope: 0.0171 ft/ft

Critical Top Width: 12.68 ft

Calculated Max Shear Stress: 2.2115 lb/ft^2

Calculated Avg Shear Stress: 1.9068 lb/ft^2

Channel Analysis: Channel Analysis-C1

Notes:

Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 4.0000 ft/ft

Channel Width: 8.0000 ft

Longitudinal Slope: 0.0650 ft/ft

Manning's n: 0.0300

Flow: 26.7000 cfs

Result Parameters

Depth: 0.4291 ft

Area of Flow: 4.1695 ft^2

Wetted Perimeter: 11.5386 ft

Hydraulic Radius: 0.3614 ft

Average Velocity: 6.4036 ft/s

Top Width: 11.4329 ft

Froude Number: 1.8687

Critical Depth: 0.6281 ft

Critical Velocity: 4.0436 ft/s

Critical Slope: 0.0167 ft/ft

Critical Top Width: 13.02 ft

Calculated Max Shear Stress: 1.7405 lb/ft^2

Calculated Avg Shear Stress: 1.4656 lb/ft^2

Channel Analysis: Channel Analysis-C2

Notes:

Input Parameters

Channel Type: Trapezoidal

Side Slope 1 (Z1): 4.0000 ft/ft

Side Slope 2 (Z2): 4.0000 ft/ft

Channel Width: 8.0000 ft

Longitudinal Slope: 0.0950 ft/ft

Manning's n: 0.0300

Flow: 46.8000 cfs

Result Parameters

Depth: 0.5291 ft

Area of Flow: 5.3525 ft^2

Wetted Perimeter: 12.3630 ft

Hydraulic Radius: 0.4329 ft

Average Velocity: 8.7436 ft/s

Top Width: 12.2327 ft

Froude Number: 2.3294

Critical Depth: 0.8751 ft

Critical Velocity: 4.6499 ft/s

Critical Slope: 0.0153 ft/ft

Critical Top Width: 15.00 ft

Calculated Max Shear Stress: 3.1364 lb/ft^2

Calculated Avg Shear Stress: 2.5665 lb/ft^2

APPENDIX C2

HYDRAULIC CALCULATIONS – CULVERTS

WALDEN PRESERVE 2 FILING NO. 4
CULVERT DESIGN SUMMARY

BASIN	DESIGN POINT	RD CL ELEV	INV IN ELEV	INV OUT ELEV	PIPE LENGTH (FT)	# of CULVERTS	PIPE DIA (FT)	TOTAL Q_5 (CFS)	PER PIPE Q_5 (CFS)	Q_5 MAX ALLOWABLE HEADWATER [1]	CALC Q_5 HW ELEV	TOTAL Q_{100} (CFS)	PER PIPE Q_{100} (CFS)	Q_{100} MAX ALLOWABLE HEADWATER [2]	CALC Q_{100} HW ELEV
B8	B8	7475.26	7469.20	7467.00	216.0	1	**3.5**	35.9	35.9	7472.7	7472.0	85.1	85.10	7475.4	7475.3
B9	B9	7479.26	7475.57	7474.78	80.0	1	**2.0**	10.0	10.0	7477.6	7477.2	23.7	23.70	7479.4	7478.7
C1	C1	7477.39	7473.00	7472.18	80.0	1	**2.0**	13.0	13.0	7475.0	7474.9	30.8	30.80	7477.6	7477.4

[1] Q_5 MAX. ALLOWABLE HEADWATER, HW/D = 1.0

[2] Q_{100} MAX. ALLOWABLE HEADWATER = 6" DEPTH AT SHOULDER (PER DCM TABLE 6-1)

HY-8 Culvert Analysis Report – Culvert B8

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 15 cfs

Design Flow: 35.9 cfs

Maximum Flow: 85.1 cfs

Table 1 - Summary of Culvert Flows at Crossing: Crossing B8

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert B8 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
7470.91	15.00	15.00	0.00	1
7471.32	22.01	22.01	0.00	1
7471.67	29.02	29.02	0.00	1
7472.00	35.90	35.90	0.00	1
7472.34	43.04	43.04	0.00	1
7472.71	50.05	50.05	0.00	1
7473.12	57.06	57.06	0.00	1
7473.59	64.07	64.07	0.00	1
7474.11	71.08	71.08	0.00	1
7474.69	78.09	78.09	0.00	1
7475.27	85.10	84.55	0.35	38
7475.26	84.42	84.42	0.00	Overtopping

Rating Curve Plot for Crossing: Crossing B8

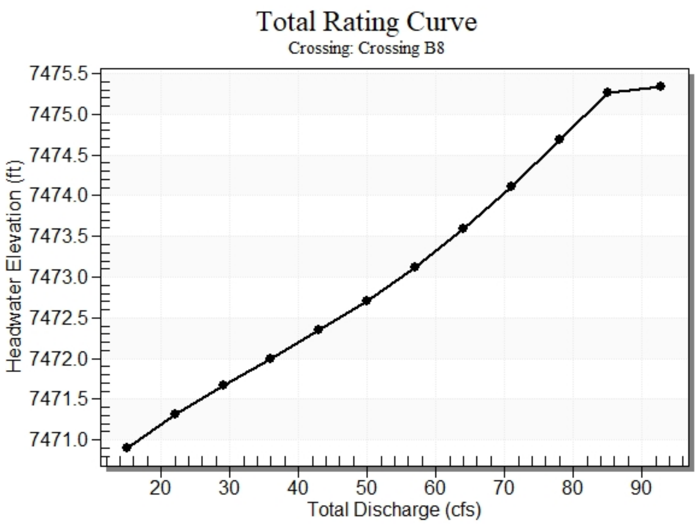


Table 2 - Culvert Summary Table: Culvert B8

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
15.00	15.00	7470.91	1.707	0.0*	1-S2n	0.901	1.176	0.901	0.594	7.617	3.963
22.01	22.01	7471.32	2.117	0.0*	1-S2n	1.097	1.434	1.097	0.723	8.500	4.415
29.02	29.02	7471.67	2.471	0.0*	1-S2n	1.279	1.663	1.279	0.831	9.123	4.766
35.90	35.90	7472.00	2.797	0.297	1-S2n	1.431	1.856	1.431	0.924	9.690	5.049
43.04	43.04	7472.34	3.143	0.760	1-S2n	1.589	2.039	1.598	1.010	10.057	5.302
50.05	50.05	7472.71	3.511	1.253	5-S2n	1.731	2.207	1.731	1.086	10.547	5.520
57.06	57.06	7473.12	3.923	1.783	5-S2n	1.874	2.363	1.901	1.157	10.686	5.716
64.07	64.07	7473.59	4.388	2.348	5-S2n	2.015	2.506	2.042	1.223	11.004	5.895
71.08	71.08	7474.11	4.909	3.382	5-S2n	2.156	2.638	2.185	1.284	11.259	6.058
78.09	78.09	7474.69	5.489	3.963	5-S2n	2.302	2.760	2.314	1.342	11.580	6.210
85.10	84.55	7475.27	6.071	4.536	5-S2n	2.441	2.862	2.453	1.397	11.752	6.351

* Full Flow Headwater elevation is below inlet invert.

****

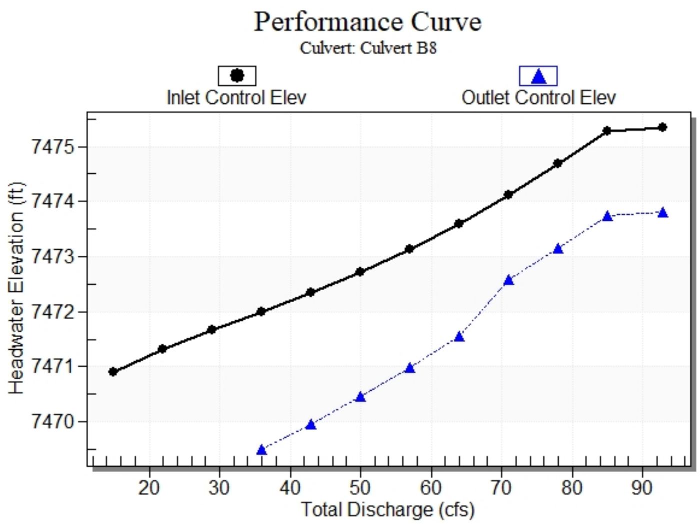
Straight Culvert

Inlet Elevation (invert): 7469.20 ft, Outlet Elevation (invert): 7467.00 ft

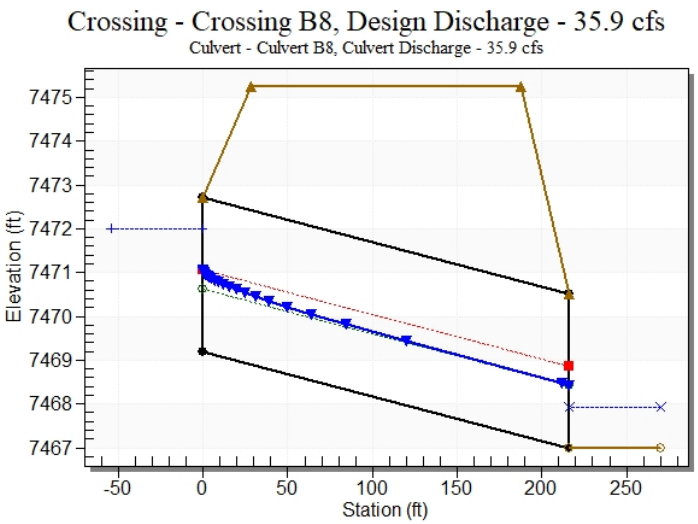
Culvert Length: 216.01 ft, Culvert Slope: 0.0102

****

Culvert Performance Curve Plot: Culvert B8



Water Surface Profile Plot for Culvert: Culvert B8



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Inlet Station: 0.00 ft

Inlet Elevation: 7469.20 ft

Outlet Station: 216.00 ft

Outlet Elevation: 7467.00 ft

Number of Barrels: 1

Culvert Data Summary - Culvert B8

Barrel Shape: Circular

Barrel Diameter: 3.50 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0130

Culvert Type: Straight

Inlet Configuration: Mitered to Conform to Slope

Inlet Depression: NONE

Table 3 - Downstream Channel Rating Curve (Crossing: Crossing B8)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)	Velocity (ft/s)	Shear (psf)	Froude Number
15.00	7467.59	0.59	3.96	0.74	1.06
22.01	7467.72	0.72	4.41	0.90	1.09
29.02	7467.83	0.83	4.77	1.04	1.11
35.90	7467.92	0.92	5.05	1.15	1.13
43.04	7468.01	1.01	5.30	1.26	1.14
50.05	7468.09	1.09	5.52	1.36	1.15
57.06	7468.16	1.16	5.72	1.44	1.16
64.07	7468.22	1.22	5.89	1.53	1.17
71.08	7468.28	1.28	6.06	1.60	1.18
78.09	7468.34	1.34	6.21	1.68	1.18
85.10	7468.40	1.40	6.35	1.74	1.19

Tailwater Channel Data - Crossing B8

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 4.00 ft

Side Slope (H:V): 4.00 (_:1)

Channel Slope: 0.0200

Channel Manning's n: 0.0300

Channel Invert Elevation: 7467.00 ft

Roadway Data for Crossing: Crossing B8

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 100.00 ft

Crest Elevation: 7475.26 ft

Roadway Surface: Paved

Roadway Top Width: 160.00 ft

HY-8 Culvert Analysis Report – Culvert B9

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 5 cfs

Design Flow: 10 cfs

Maximum Flow: 23.7 cfs

Table 1 - Summary of Culvert Flows at Crossing: Crossing B9

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert B9 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
7476.66	5.00	5.00	0.00	1
7476.88	6.87	6.87	0.00	1
7477.08	8.74	8.74	0.00	1
7477.20	10.00	10.00	0.00	1
7477.43	12.48	12.48	0.00	1
7477.60	14.35	14.35	0.00	1
7477.79	16.22	16.22	0.00	1
7477.99	18.09	18.09	0.00	1
7478.22	19.96	19.96	0.00	1
7478.46	21.83	21.83	0.00	1
7478.74	23.70	23.70	0.00	1
7479.26	26.90	26.90	0.00	Overtopping

Rating Curve Plot for Crossing: Crossing B9

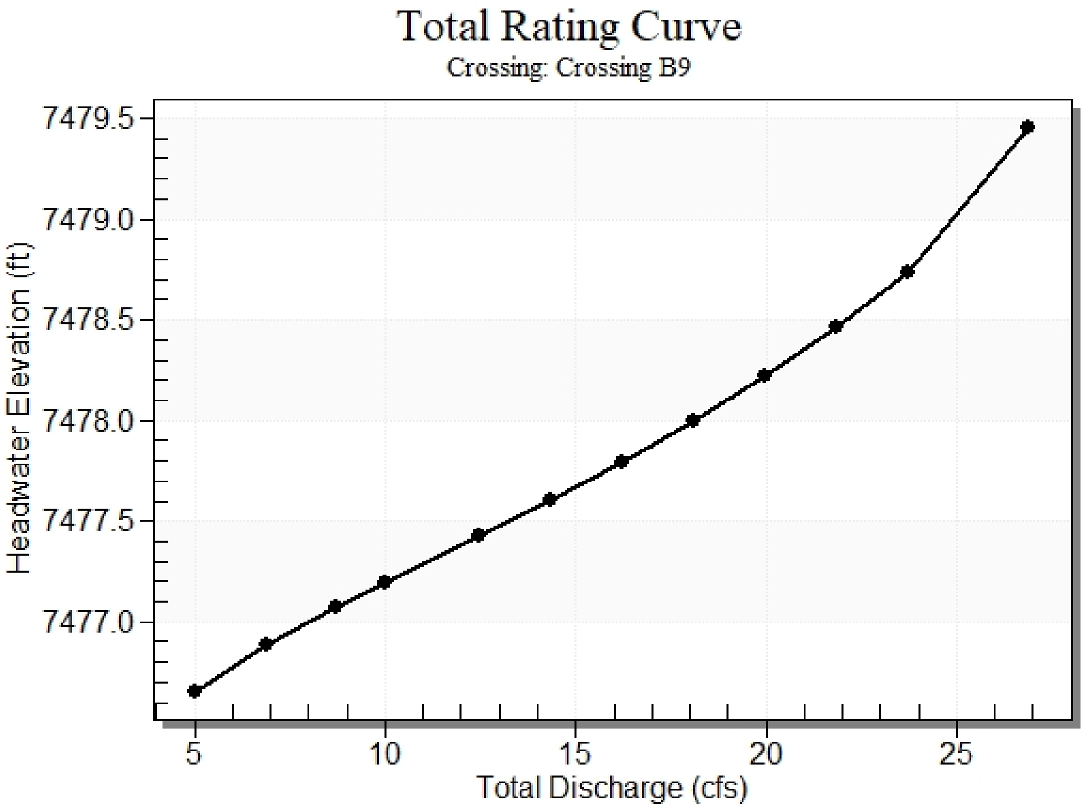


Table 2 - Culvert Summary Table: Culvert B9

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
5.00	5.00	7476.66	1.087	0.0*	1-S2n	0.624	0.783	0.624	0.329	5.769	2.858
6.87	6.87	7476.88	1.311	0.302	1-S2n	0.738	0.930	0.738	0.392	6.301	3.151
8.74	8.74	7477.08	1.506	0.526	1-S2n	0.842	1.053	0.871	0.446	6.435	3.387
10.00	10.00	7477.20	1.627	0.0*	1-S2n	0.909	1.126	0.939	0.480	6.675	3.524
12.48	12.48	7477.43	1.856	1.014	1-S2n	1.036	1.268	1.072	0.539	7.044	3.759
14.35	14.35	7477.60	2.033	1.280	5-S2n	1.130	1.361	1.168	0.580	7.301	3.913
16.22	16.22	7477.79	2.220	1.563	5-S2n	1.224	1.447	1.265	0.618	7.510	4.052
18.09	18.09	7477.99	2.423	2.102	5-S2n	1.322	1.530	1.366	0.654	7.688	4.179
19.96	19.96	7478.22	2.647	2.383	5-S2n	1.427	1.603	1.470	0.688	7.847	4.296
21.83	21.83	7478.46	2.895	2.685	5-S2n	1.546	1.668	1.591	0.720	7.952	4.405
23.70	23.70	7478.74	3.166	2.983	7-M2c	2.000	1.726	1.726	0.751	8.221	4.507

* Full Flow Headwater elevation is below inlet invert.

****

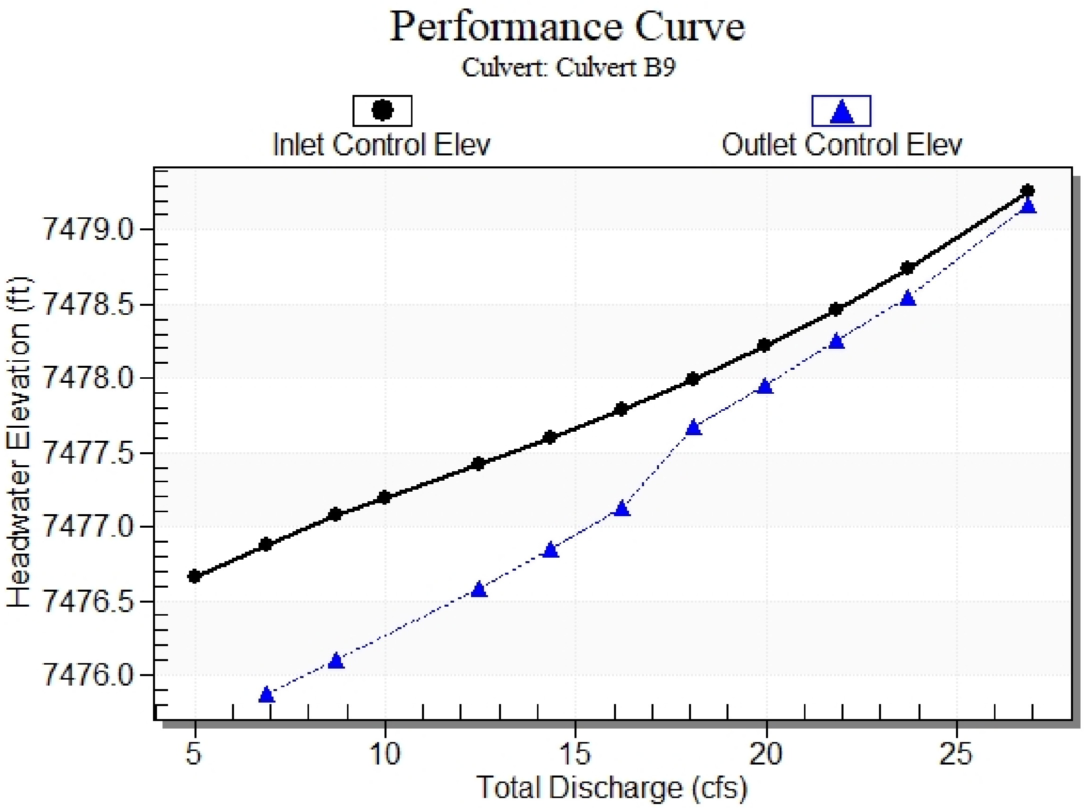
Straight Culvert

Inlet Elevation (invert): 7475.57 ft,   Outlet Elevation (invert): 7474.78 ft

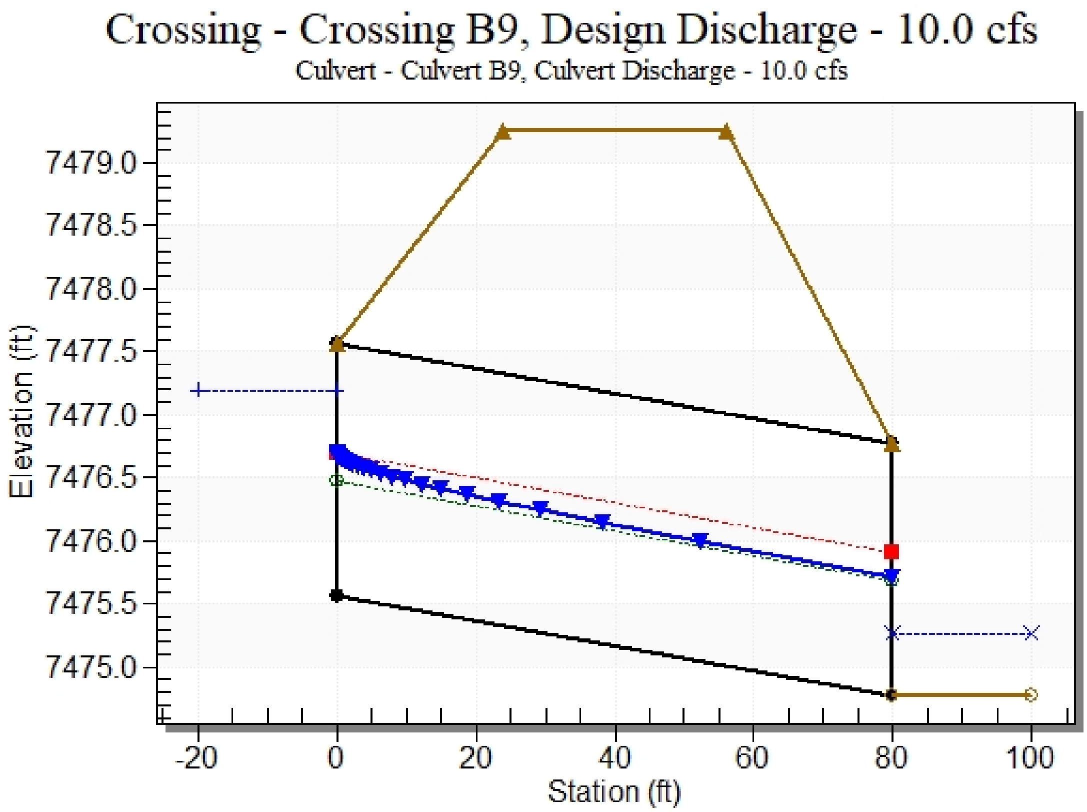
Culvert Length: 80.00 ft,   Culvert Slope: 0.0099

****

Culvert Performance Curve Plot: Culvert B9



Water Surface Profile Plot for Culvert: Culvert B9



Site Data - Culvert B9

Site Data Option: Culvert Invert Data

Inlet Station: 0.00 ft

Inlet Elevation: 7475.57 ft

Outlet Station: 80.00 ft

Outlet Elevation: 7474.78 ft

Number of Barrels: 1

Culvert Data Summary - Culvert B9

Barrel Shape: Circular

Barrel Diameter: 2.00 ft

Barrel Material:

Embedment: 0.00 in

Barrel Manning's n: 0.0130

Culvert Type: Straight

Inlet Configuration: Grooved End Projecting

Inlet Depression: None

Table 3 - Downstream Channel Rating Curve (Crossing: Crossing B9)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)	Velocity (ft/s)	Shear (psf)	Froude Number
5.00	7475.11	0.33	2.86	0.41	0.98
6.87	7475.17	0.39	3.15	0.49	1.00
8.74	7475.23	0.45	3.39	0.56	1.02
10.00	7475.26	0.48	3.52	0.60	1.03
12.48	7475.32	0.54	3.76	0.67	1.05
14.35	7475.36	0.58	3.91	0.72	1.06
16.22	7475.40	0.62	4.05	0.77	1.07
18.09	7475.43	0.65	4.18	0.82	1.08
19.96	7475.47	0.69	4.30	0.86	1.08
21.83	7475.50	0.72	4.40	0.90	1.09
23.70	7475.53	0.75	4.51	0.94	1.10

Tailwater Channel Data - Crossing B9

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 4.00 ft

Side Slope (H:V): 4.00 (_:1)

Channel Slope: 0.0200

Channel Manning's n: 0.0300

Channel Invert Elevation: 7474.78 ft

Roadway Data for Crossing: Crossing B9

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 100.00 ft

Crest Elevation: 7479.26 ft

Roadway Surface: Paved

Roadway Top Width: 32.00 ft

HY-8 Culvert Analysis Report – Culvert C1

Crossing Discharge Data

Discharge Selection Method: Specify Minimum, Design, and Maximum Flow

Minimum Flow: 5 cfs

Design Flow: 13 cfs

Maximum Flow: 30.8 cfs

Table 4 - Summary of Culvert Flows at Crossing: Crossing C1

Headwater Elevation (ft)	Total Discharge (cfs)	Culvert C1 Discharge (cfs)	Roadway Discharge (cfs)	Iterations
7474.09	5.00	5.00	0.00	1
7474.39	7.58	7.58	0.00	1
7474.64	10.16	10.16	0.00	1
7474.88	12.74	12.74	0.00	1
7474.90	13.00	13.00	0.00	1
7475.40	17.90	17.90	0.00	1
7475.71	20.48	20.48	0.00	1
7476.07	23.06	23.06	0.00	1
7476.47	25.64	25.64	0.00	1
7476.93	28.22	28.22	0.00	1
7477.42	30.80	30.80	0.00	1
7477.65	31.89	31.89	0.00	Overtopping

Rating Curve Plot for Crossing: Crossing C1

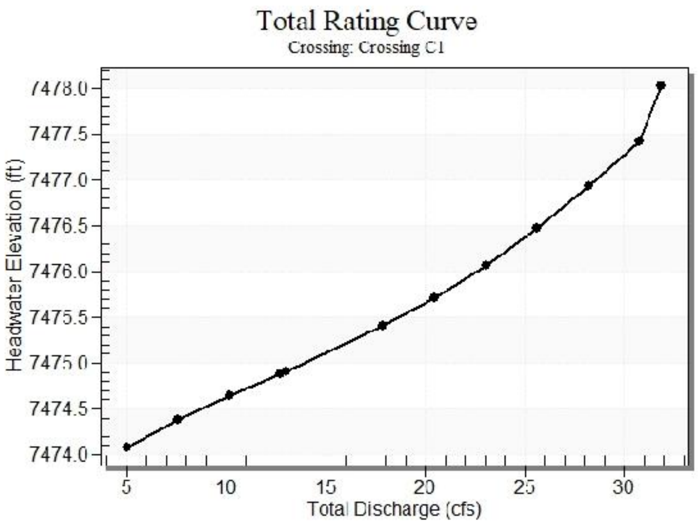


Table 5 - Culvert Summary Table: Culvert C1

Total Discharge (cfs)	Culvert Discharge (cfs)	Headwater Elevation (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)	Tailwater Depth (ft)	Outlet Velocity (ft/s)	Tailwater Velocity (ft/s)
5.00	5.00	7474.09	1.086	0.0*	1-S2n	0.629	0.783	0.637	0.268	5.779	2.638
7.58	7.58	7474.39	1.388	0.357	1-S2n	0.789	0.979	0.796	0.340	6.489	3.028
10.16	10.16	7474.64	1.641	0.670	1-S2n	0.931	1.135	0.939	0.401	7.020	3.330
12.74	12.74	7474.88	1.880	1.020	1-S2n	1.064	1.281	1.074	0.455	7.415	3.579
13.00	13.00	7474.90	1.904	0.0*	1-S2n	1.077	1.294	1.087	0.460	7.451	3.601
17.90	17.90	7475.40	2.401	2.044	5-S2n	1.331	1.522	1.338	0.549	8.024	3.976
20.48	20.48	7475.71	2.713	2.435	5-S2n	1.475	1.622	1.479	0.591	8.216	4.143
23.06	23.06	7476.07	3.070	2.864	5-S2n	1.653	1.707	1.652	0.630	8.302	4.293
25.64	25.64	7476.47	3.475	3.255	7-M2c	2.000	1.777	1.777	0.667	8.691	4.431
28.22	28.22	7476.93	3.927	3.840	7-M2c	2.000	1.833	1.833	0.703	9.358	4.558
30.80	30.80	7477.42	4.425	4.393	7-M2c	2.000	1.877	1.877	0.736	10.061	4.677

* Full Flow Headwater elevation is below inlet invert.

****

Straight Culvert

Inlet Elevation (invert): 7473.00 ft, Outlet Elevation (invert): 7472.18 ft

Culvert Length: 80.00 ft, Culvert Slope: 0.0102

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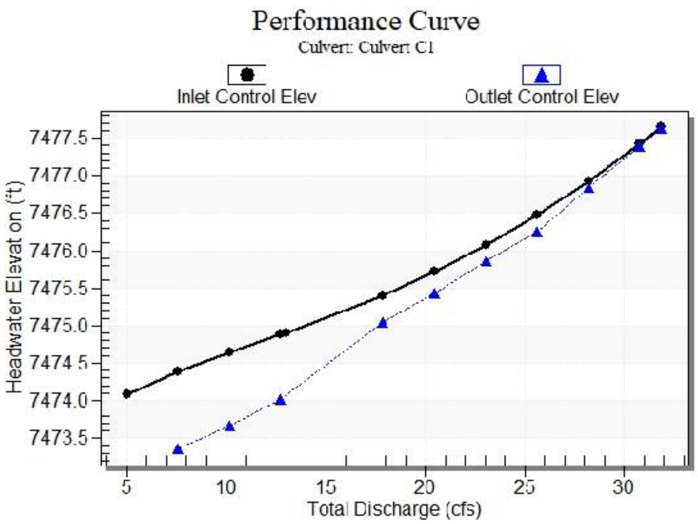
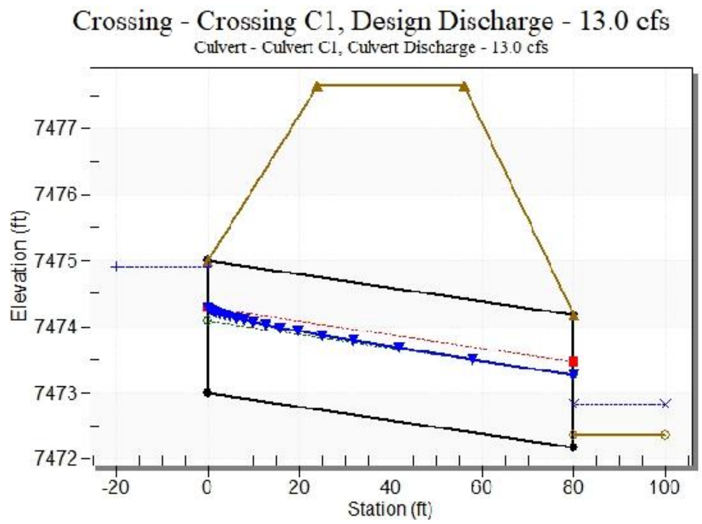
Culvert Performance Curve Plot: Culvert C1



Water Surface Profile Plot for Culvert: Culvert C1



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Inlet Station: 0.00 ft

Inlet Elevation: 7473.00 ft

Outlet Station: 80.00 ft

Outlet Elevation: 7472.18 ft

Number of Barrels: 1

Culvert Data Summary - Culvert C1

Barrel Shape: Circular

Barrel Diameter: 2.00 ft

Barrel Material: Concrete

Embedment: 0.00 in

Barrel Manning's n: 0.0130

Culvert Type: Straight

Inlet Configuration: Grooved End Projecting

Inlet Depression: NONE

Table 6 - Downstream Channel Rating Curve (Crossing: Crossing C1)

Flow (cfs)	Water Surface Elev (ft)	Depth (ft)	Velocity (ft/s)	Shear (psf)	Froude Number
5.00	7472.63	0.27	2.64	0.33	0.96
7.58	7472.70	0.34	3.03	0.42	1.00
10.16	7472.76	0.40	3.33	0.50	1.02
12.74	7472.82	0.46	3.58	0.57	1.04
13.00	7472.82	0.46	3.60	0.57	1.04
17.90	7472.91	0.55	3.98	0.69	1.06
20.48	7472.95	0.59	4.14	0.74	1.08
23.06	7472.99	0.63	4.29	0.79	1.08
25.64	7473.03	0.67	4.43	0.83	1.09
28.22	7473.06	0.70	4.56	0.88	1.10
30.80	7473.10	0.74	4.68	0.92	1.11

Tailwater Channel Data - Crossing C1

Tailwater Channel Option: Trapezoidal Channel

Bottom Width: 6.00 ft

Side Slope (H:V): 4.00 (_:1)

Channel Slope: 0.0200

Channel Manning's n: 0.0300

Channel Invert Elevation: 7472.36 ft

Roadway Data for Crossing: Crossing C1

Roadway Profile Shape: Constant Roadway Elevation

Crest Length: 200.00 ft

Crest Elevation: 7477.65 ft

Roadway Surface: Paved

Roadway Top Width: 32.00 ft

Culvert B8

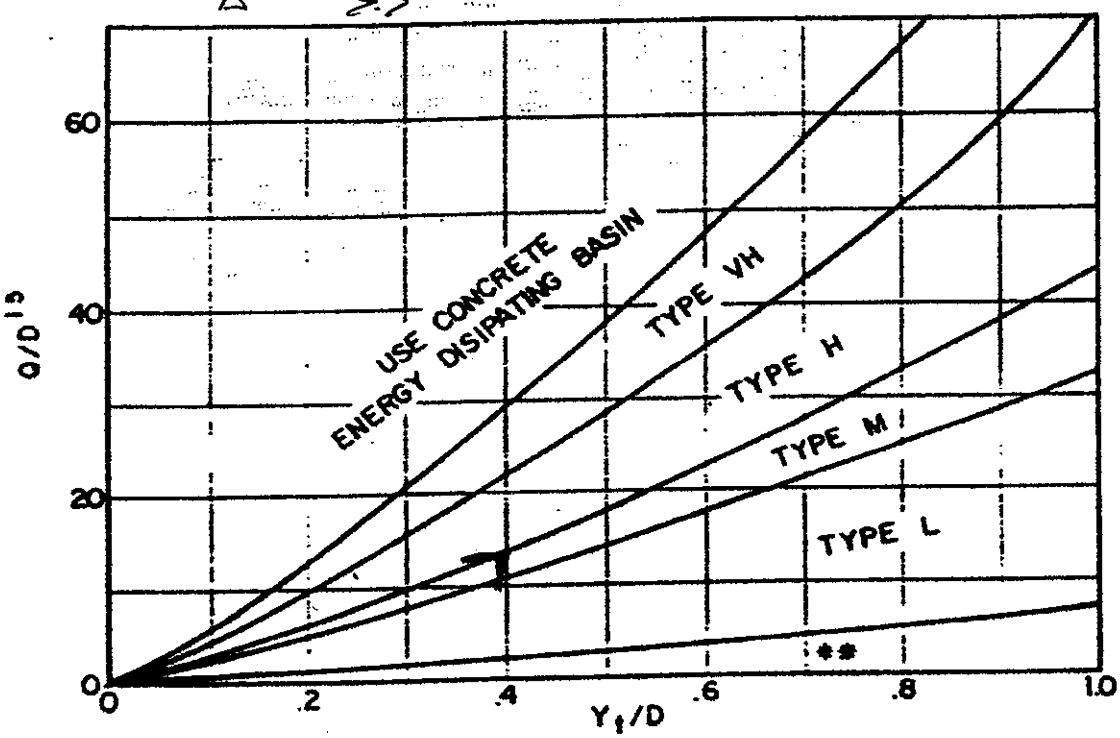
$Q_{100} = 85.1$ cfs

$\Delta = 3.5'$

$$\frac{Q}{\Delta^{1.5}} = \frac{85.1}{(3.5)^{1.5}} = 13.0$$

$Y_t = 1.4'$

$$\frac{Y_t}{\Delta} = \frac{1.4}{3.5} = 0.4$$



Use D_a instead of D whenever flow is supercritical in the barrel.
** Use Type L for a distance of 3D downstream.

→ Use Type M

FIGURE 5-7. RIPRAP EROSION PROTECTION AT CIRCULAR CONDUIT OUTLET.

Culvert B9

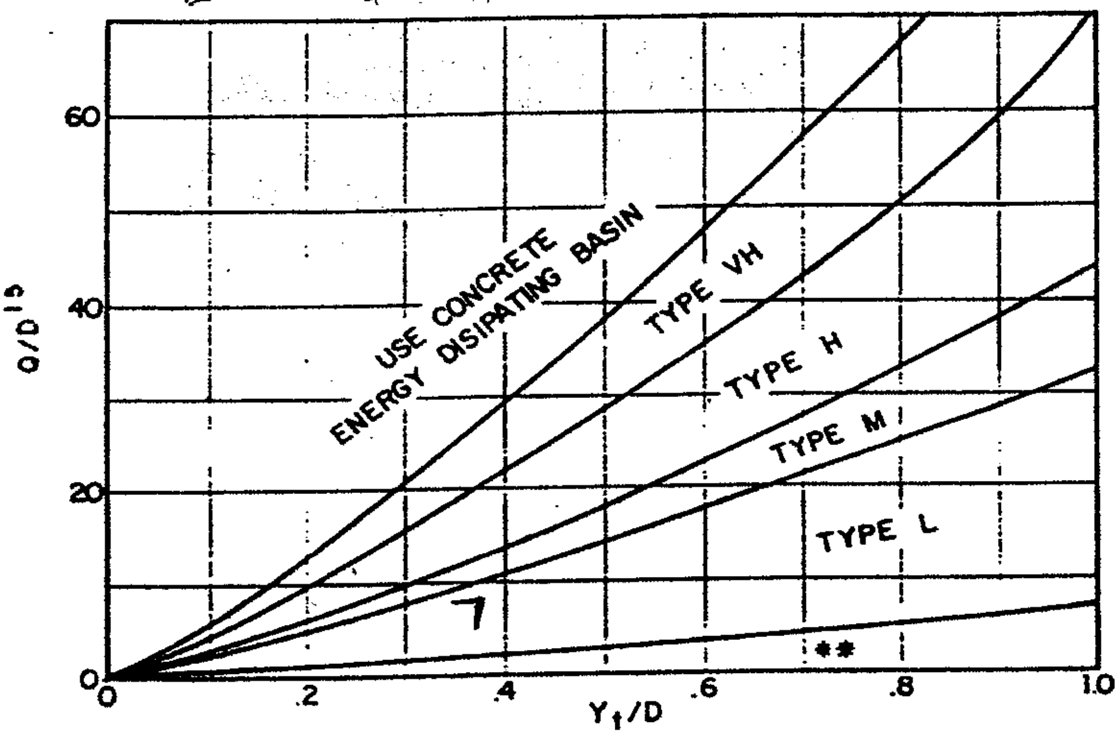
$Q_{100} = 23.7$ cfs

$D = 2'$

$\frac{Q}{D^{1.5}} = \frac{23.7}{(2)^{1.5}} = 8.4$

$Y_t = 0.75'$

$\frac{Y_t}{D} = \frac{0.75}{2} = 0.38$



Use D_a instead of D whenever flow is supercritical in the barrel.
** Use Type L for a distance of 3D downstream.

→ Use Type M

FIGURE 5-7. RIPRAP EROSION PROTECTION AT CIRCULAR CONDUIT OUTLET.

Culvert C1

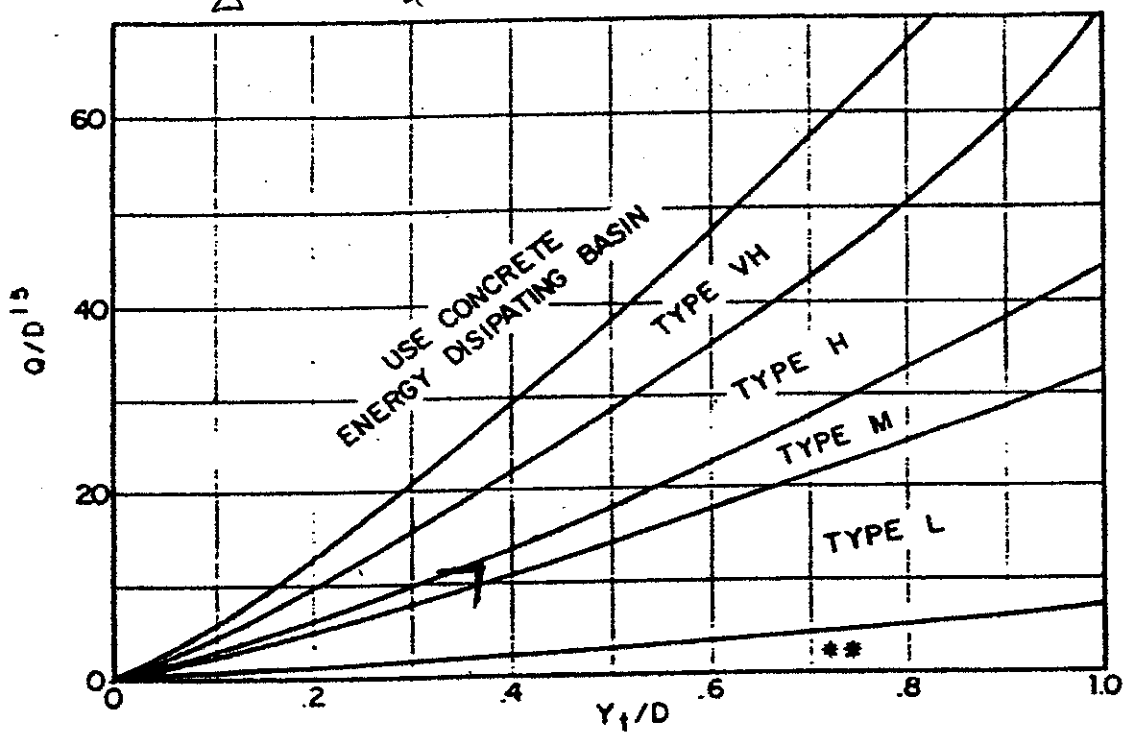
$Q_{100} = 30.8$ cfs

$\Delta = 2'$

$$\frac{Q}{\Delta^{1.5}} = \frac{30.8}{(2)^{1.5}} = 10.9$$

$Y_t = 0.74'$

$$\frac{Y_t}{\Delta} = \frac{0.74}{2} = 0.37$$



Use D_a instead of D whenever flow is supercritical in the barrel.
** Use Type L for a distance of 3D downstream.

→ Use Type M

FIGURE 5-7. RIPRAP EROSION PROTECTION AT CIRCULAR CONDUIT OUTLET.

Rain Garden C2- Outlet Pipe

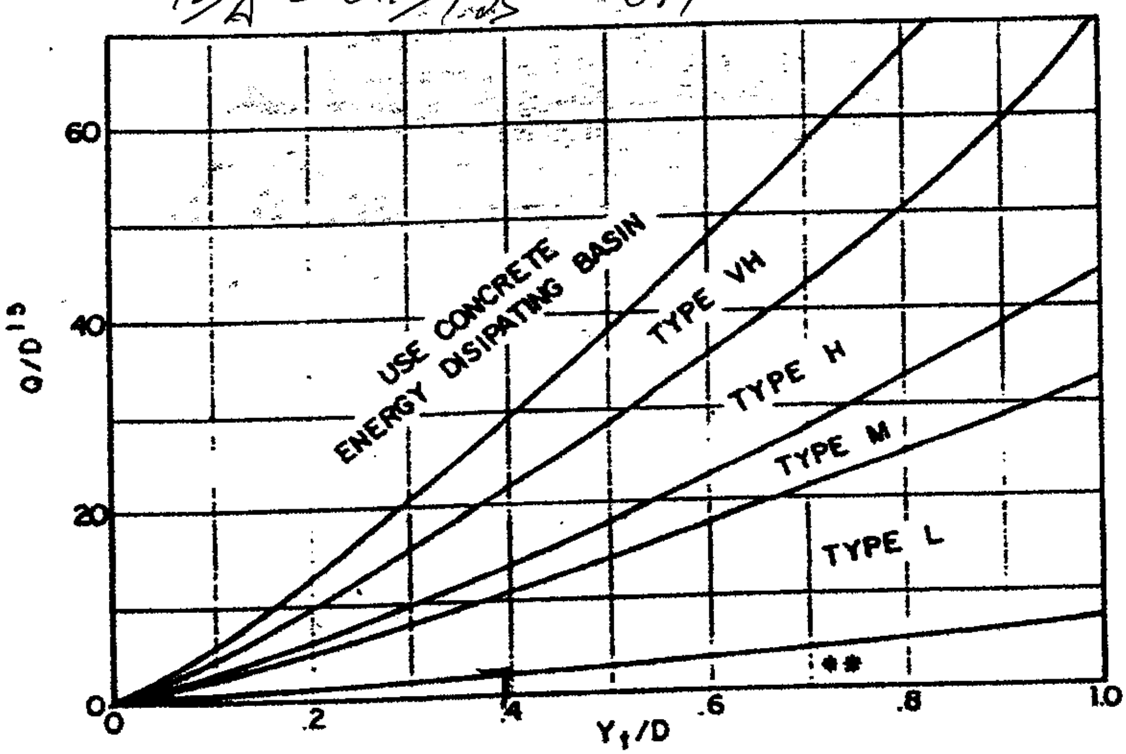
$Q_5 = 3.6$ cfs

$D = 15'' = 1.25'$

$\frac{Q}{D^{1.5}} = \frac{3.6}{(1.25)^{1.5}} = 2.6$

$Y_t = 0.5$

$Y_t/D = 0.5/1.25 = 0.4$



Use D_a instead of D whenever flow is supercritical in the barrel.
** Use Type L for a distance of 3D downstream.

→ Use Type M
(conservative design)

FIGURE 5-7. RIPRAP EROSION PROTECTION AT CIRCULAR CONDUIT OUTLET.

WALDEN PRESERVE 2 - FILING NO. 4
DRIVEWAY CULVERT SUMMARY

LOT	DRAINAGE BASIN	Q5 FLOW (CFS)	CULVERT FLOW % OF BASIN	CULVERT FLOW (CFS)		CULVERT SIZE (IN)
1	B8	35.9	10	3.6		18"
2	B8	35.9	10	3.6		18"
3	B8	35.9	10	3.6		18"
4	B8	35.9	10	3.6		18"
5	B8	35.9	15	5.4		18"
6	B8	35.9	15	5.4		18"
7	B8	35.9	20	7.2		18"
8	B8	35.9	100	35.9		42"
9	B8	35.9	100	35.9		42"
10	B8	35.9	100	35.9		42"
11	B10	25.9	5	1.3		18"
12	B10	25.9	5	1.3		18"
13	B10	25.9	5	1.3		18"
14	B10	25.9	5	1.3		18"
15	B10	25.9	5	1.3		18"
16	C2	18.0	10	1.8		18"
17	C1	13.0	20	2.6		18"
18	B9	10.0	10	1.0		18"
19	B9	10.0	60	6.0		18"
20	B9	10.0	50	5.0		18"
21	B9	10.0	30	3.0		18"
22	B9	10.0	20	2.0		18"
23	B9	10.0	10	1.0		18"

ASSUMPTIONS:
1) Driveway Culverts sized based on Max HW/D = 1.5 for Q5 Flows
2) Culvert Capacity based on Inlet Control Nomographs (DCM Fig. 9-32)
3) Driveway Culverts assumed to be RCP or HDPE

APPENDIX D1

DETENTION POND CALCULATIONS – POND B

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WALDEN PRESERVE 2 - FILING NO. 1
FINAL DRAINAGE REPORT – ADDENDUM NO. 1

August 10, 2019

This Final Drainage Report Addendum has been prepared in response to El Paso County's request to upgrade the outlet structure of Walden Preserve Pond B to current full-spectrum detention design standards.

Background

JPS Engineering previously prepared the "Master Development Drainage Plan and Preliminary Drainage Report for Walden Preserve 2 PUD" dated September 17, 2014 and the "Final Drainage Report for Walden Preserve 2 - Filings No. 1 and 2" dated November 13, 2014. Walden Preserve Pond B is an existing Retention Pond which serves as an aesthetic amenity for the Walden community while also providing significant stormwater detention volume above the "active" storage volume in the pond. The historic stock pond was originally upgraded with a new outlet structure during construction of Walden Preserve Filing No. 1. Plans for Walden Preserve 2 Filing No. 1 included further upgrades to the Pond B outlet structure to incorporate a new water quality control orifice plate, though these improvements have not yet been constructed. El Paso County has now requested improvements to the outlet structure to meet current full-spectrum detention design standards.

Detention Pond Calculations

As detailed in the previous MDDP and Final Drainage Reports, off-site flows from the southeast combine with flows from the southern part of Walden Preserve (Basins A and B) in a main tributary channel of West Cherry Creek which flows northwesterly through the Walden property. The combined flows entering Pond B at Design Point #2 comprise a tributary drainage area of approximately 500 acres. Historic peak flows at Design Point #2 have been calculated as Q_5 = 65.8 cfs and Q_{100} = 287.9 cfs (SCS Method) in the previously approved drainage reports.

The pond outlet structure for Pond B has been designed to release detained flows below historic flows at Design Point #2, resulting in a net release at or below historic flows at Design Point #4 downstream (crossing Walker Road), discharging to the existing downstream channel. The pond outlet structure has been designed to maintain the calculated pond discharge below the target outflow, while holding the 100-year water surface elevation below the allowable maximum level.

In accordance with current County drainage criteria, Retention Pond B has been evaluated based on "Full-Spectrum" detention pond design guidance provided by the Denver Urban Drainage and Flood Control District (UDFCD). Design calculations have been performed for upgrade of the pond outlet structure to current full-spectrum detention standards, utilizing the UDFCD "UD-Detention_v3.07" and "UD-BMP_v3.06" software packages (see calculations in Appendix A).

For consistency with the previously accepted major basin hydrology, the "UD-Detention" model has been calibrated by adjusting the percentage of hydrologic soils group A and B soils so that the calculated predevelopment 100-year peak flow in the UD-Detention model matches the 100-year flow of 287.9 cfs calculated in the HEC-HMS model.

The existing Pond B outlet structure consists of a 6'x6.3' concrete catch basin with a lower 16"x16" orifice entering the west face of the box and an upper 36"x36" orifice entering the south face of the box. An existing 48" RCP discharge pipe extends north from the outlet structure to a riprap energy dissipation structure flowing into the downstream channel.

Based on the enclosed detention calculations, we recommend upgrading the existing pond outlet structure to current full-spectrum design standards by closing the west orifice, installing a new trash rack at the south face of the box and constructing a new interior wall and chamber within the outlet structure, providing a trash screen and orifice plate to meter flows through the structure. The proposed internal orifice plate will have a lower slot opening and two higher orifice openings sized based on the UD-Detention model. Additionally, a steel restrictor plate will be installed on the inside face of the box at the 48" discharge pipe opening.

As detailed in the enclosed calculations, the proposed outlet structure improvements result in calculated peak outflows below predevelopment flows for the full spectrum of storm events. The calculated 100-year discharge is 155.8 cfs, providing a significant reduction in comparison to predevelopment flows.

Summary

Retention Pond B provides stormwater detention and water quality enhancement to mitigate developed flow impacts from a large part of the Walden Preserve development. The proposed Pond B outlet structure improvements will improve the metering of discharge flows and upgrade the pond to current full-spectrum detention design standards.

APPENDIX A

DETENTION POND CALCULATIONS – POND B

HISTORIC FLOWS

BASIN	DESIGN POINT	AREA (AC)	AREA (SM)	CURVE No. (CN)	S	Ia	PERCENT IMPERVIOUS (%)	Time of Concentration Tc[1] (MIN)	Total Lag Time Tl[2] (MIN)	Peak Flow SCS Q5[3] (CFS)	Peak Flow SCS Q100[3] (CFS)
OA1		232.0	0.3625	61	6.39	1.28	2	34.80	20.88		
A		105.1	0.1642	61	6.39	1.28	2	18.80	11.28		
OA1,A	1	337.1	0.5267	61	6.39	1.28	2	53.60	32.16	33.1	213.4
OB1		35.4	0.0553	70	4.29	0.86	25	8.20	4.92		
B		118.0	0.1844	61	6.39	1.28	2	13.70	8.22		
OA1-OAB,A-B	2	490.5	0.7664	61	6.39	1.28	2	67.30	40.38	65.8	287.9
OC1		187.6	0.2931	70	4.29	0.86	25	14.40	8.64		
C		190.2	0.2972	61	6.39	1.28	2	47.10	28.26		
OD1		24.1	0.0377	61	6.39	1.28	2	30.30	18.18		
D		10.27	0.0160	61	6.39	1.28	2	4.50	2.70		
OA1-OC1,A-D	4	902.7	1.4104	61	6.39	1.28	2	114.40	68.64	222.5	582.9

DEVELOPED FLOWS

BASIN	DESIGN POINT	AREA (AC)	AREA (SM)	CURVE No. (CN)	S	Ia	PERCENT IMPERVIOUS (%)	Time of Concentration Tc[1] (HR)	Total Lag Time Tl[2] (HR)	Peak Flow SCS Q5[3] (CFS)	Peak Flow SCS Q100[3] (CFS)
OA1		232.0	0.3625	61	6.39	1.28	2	36.80	22.08		
A9 (A1-A9)		110.0	0.1719	68	4.71	0.94	20	19.00	11.40		
OA1,A1-A9	1	342.0	0.5344					55.80	33.48	92.1	261.3
OB1		35.4	0.0553	70	4.29	0.86	25	20.60	12.36		
B4 (B1-B4)		55.5	0.0867	70	4.29	0.86	25	21.10	12.66		
B10 (B5-B10)		71.8	0.1122	68	4.71	0.94	20	28.70	17.22		
Tt DP1 to DP2								13.70	8.22		
OA1-OB1,A-B	2	504.7	0.7886					69.50	41.70	208.0	551.4
OC1		129	0.2016	69.5	4.39	0.88	23.6	14.40	8.64		
C4 (C1-C4)		40.3	0.0630	68	4.71	0.94	20	21.70	13.02		
C8 (C5-C8)		59.3	0.0927	68	4.71	0.94	20	35.00	21.00		
C9		4.5	0.0070	61	6.39	1.28	2	2.10	1.26		
Tt DP2 to DP3								22.10	13.26		
OA1-OC1,A,B,C1-C8	3	737.8	1.1528					91.60	54.96	280.7	734.2
OC2		81.7	0.1277	67.9	4.73	0.95	19.1	14.40	8.64		
C12 (C10-C12)		26.4	0.0413	68	4.71	0.94	20	19.10	11.46		
C13		22.4	0.0350	61	6.39	1.28	2	15.30	9.18		
OD1		24.1	0.0377	61	6.39	1.28	2	30.30	18.18		
D		10.3	0.0160	70	4.29	0.86	25	4.50	2.70		
Tt DP3 to DP4								25.40	15.24		
OA1-OC2,A-D	4	902.7	1.4104					117.00	70.20	298.7	785.0

1) Tc = Tco + Tt (from Rational Method Calculation Spreadsheet)
2) SCS LAG TIME, Tl = 0.6 * Tt
3) PEAK FLOWS CALCULATED BY HEC-HMS 3.5; 5-YR RAINFALL DEPTH = 2.6 IN; 100-YR RAINFALL DEPTH = 4.4 IN

WALDEN PRESERVE

IMPERVIOUS AREA CALCULATIONS

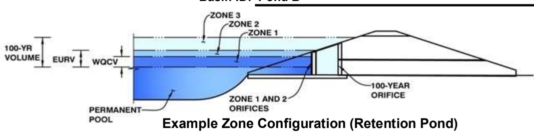
BASIN	TOTAL AREA (AC)	(AC)	SUB-AREA 1 DEVELOPMENT/ COVER	% IMP.	AREA (AC)	SUB-AREA 2 DEVELOPMENT/ COVER	% IMP.	(AC)	SUB-AREA 3 DEVELOPMENT/ COVER	% IMP.	WEIGHTED PERCENT IMPERVIOUS
OA1.1	83.8	83.8	PASTURE	0.00							0.00
OA1.2	108.2	108.2	PASTURE	0.00							0.00
A1	28.25	28.25	2.5-AC. LOTS	11.00							11.00
OA1.3	14.8	14.8	PASTURE	0.00							0.00
A2	2.34	2.34	2.5-AC. LOTS	11.00							11.00
OA1.3,A2	17.14										1.50
OA1,A1,A2	237.39										1.42
OA1.4	25.2	25.2	PASTURE	0.00							0.00
A3	11.22	11.22	2.5-AC. LOTS	11.00							11.00
A4	1.58	1.58	2.5-AC. LOTS	11.00							11.00
OA1.4,A3,A4	38.0										3.71
A5	15.14	15.14	2.5-AC. LOTS	11.00							11.00
A6	1.98	1.98	2.5-AC. LOTS	11.00							11.00
A5,A6	17.12										11.00
A7	27.55	27.55	1-AC. LOTS	20.00							20.00
A8	4.12	4.12	1-AC. LOTS	20.00							20.00
A9	17.78	17.78	1-AC. LOTS	20.00							20.00
OA1.4,A3,A4,A9	55.78										8.90
OA1,A1-A9	341.96										4.84
OB1,B1-B4	90.90										25.00
B5	4.78	4.78	2.5-AC. LOTS	11.0							11.00
B6	7.91	7.91	2.5-AC. LOTS	11.0							11.00
B5,B6	12.69										11.00
B7	3.24	3.24	1-AC LOTS	20.0							20.00
B8	28.74	28.74	1-AC LOTS	20.0							20.00
B5-B8	44.67										17.44
B9	10.23	10.23	1-AC LOTS	20.0							20.00
B10	16.90	71.17	1-AC LOTS	20.0							20.00
B5-B10	71.80										18.41
OB1,B1-B10	162.70										22.09
OA1,OB1,A,B	**504.66**										**10.40**

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: Walden Preserve

Basin ID: Pond B



Example Zone Configuration (Retention Pond)

Required Volume Calculation

Selected BMP Type =	RP			
Watershed Area =	504.70	acres		
Watershed Length =	6,870	ft		
Watershed Slope =	0.029	ft/ft		
Watershed Imperviousness =	10.40%	percent		
Percentage Hydrologic Soil Group A =	91.0%	percent		
Percentage Hydrologic Soil Group B =	9.0%	percent		
Percentage Hydrologic Soil Groups C/D =	0.0%	percent		
Desired WQCV Drain Time =	12.0	hours		
Location for 1-hr Rainfall Depths =	User Input			
Water Quality Capture Volume (WQCV) =	2.331	acre-feet	Optional User Override 1-hr Precipitation	
Excess Urban Runoff Volume (EURV) =	3.994	acre-feet		
2-yr Runoff Volume (P1 = 1.19 in.) =	2.613	acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	3.634	acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	5.233	acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	8.926	acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	14.939	acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	26.083	acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 3.14 in.) =	52.161	acre-feet	3.14	inches
Approximate 2-yr Detention Volume =	2.415	acre-feet		
Approximate 5-yr Detention Volume =	3.366	acre-feet		
Approximate 10-yr Detention Volume =	4.747	acre-feet		
Approximate 25-yr Detention Volume =	6.445	acre-feet		
Approximate 50-yr Detention Volume =	8.245	acre-feet		
Approximate 100-yr Detention Volume =	12.813	acre-feet		

Stage-Storage Calculation

Zone 1 Volume (WQCV) =	2.331	acre-feet
Zone 2 Volume (EURV - Zone 1) =	1.663	acre-feet
Zone 3 Volume (100-year - Zones 1 & 2) =	8.820	acre-feet
Total Detention Basin Volume =	12.813	acre-feet

Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Permanent Pool	--	0.00	--	--	--	76,766	1.762		
EL=7448.0	--	1.00	--	--	--	84,736	1.945	79,904	1.834
	--	3.00	--	--	--	91,954	2.111	257,441	5.910
	--	5.00	--	--	--	116,593	2.677	465,988	10.698
Spillway=7453.0	--	6.00	--	--	--	134,165	3.080	591,367	13.576
	--	7.00	--	--	--	151,736	3.483	734,317	16.858
Top EL=7456.0	--	9.00	--	--	--	199,982	4.591	1,086,035	24.932
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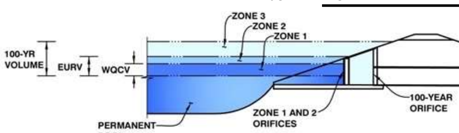
Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: WALDEN PRESERVE
Basin ID: POND B



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.25	2.331	Orifice Plate
Zone 2 (EURV)	2.08	1.663	Orifice Plate
Zone 3 (100-year)	5.75	8.820	Weir&Pipe (Restrict)
		12.813	Total

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft^2
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	2.08	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	8.30	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft^2
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft^2

How is the hydraulics for the existing 36"x36" upstream of the overflow weir accounted for since the overflow weir is located downstream?

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	0.69	1.39					
Orifice Area (sq. inches)	81.00	0.12	0.12					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =	N/A	N/A	ft^2
Vertical Orifice Centroid =	N/A	N/A	feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	2.08	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	6.00	N/A	feet
Overflow Weir Slope =	0.00	N/A	H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	0.00	N/A	feet
Overflow Grate Open Area % =	70%	N/A	%, grate open area/total area
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	2.08	N/A	feet
Over Flow Weir Slope Length =	0.00	N/A	feet
Grate Open Area / 100-yr Orifice Area =	0.00	N/A	should be ≥ 4
Overflow Grate Open Area w/o Debris =	0.00	N/A	ft^2
Overflow Grate Open Area w/ Debris =	0.00	N/A	ft^2

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	4.17	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	48.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	36.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected	
Outlet Orifice Area =	10.11	N/A	ft^2
Outlet Orifice Centroid =	1.66	N/A	feet
Half-Central Angle of Restrictor Plate on Pipe =	2.09	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	6.00	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	10.00	feet
Spillway End Slopes =	3.00	H:V
Freeboard above Max Water Surface =	2.00	feet

Calculated Parameters for Spillway

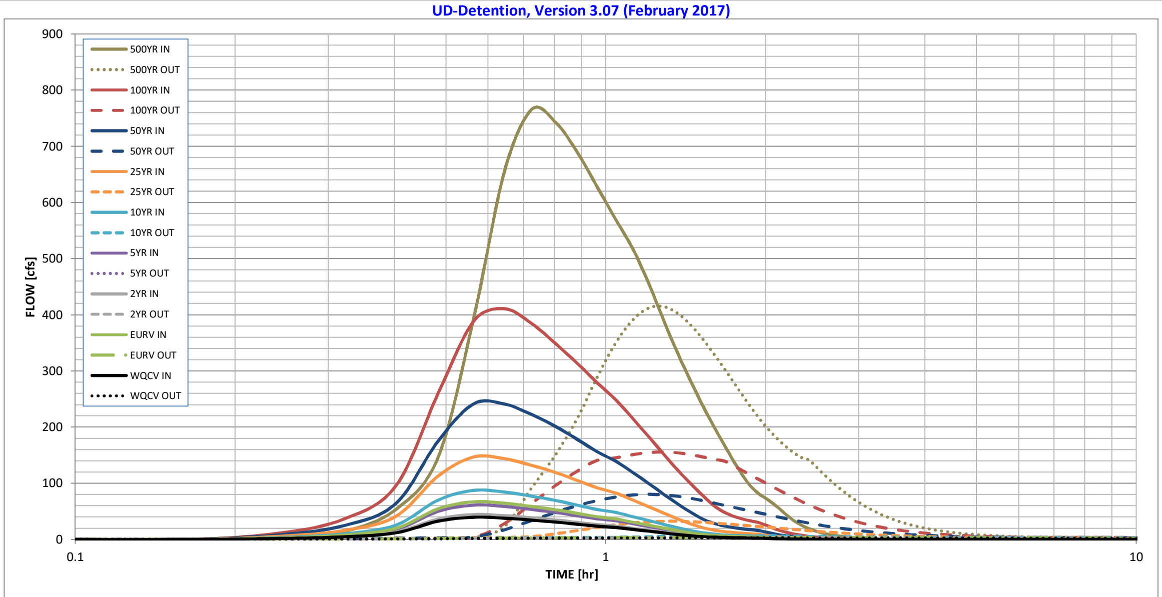
Spillway Design Flow Depth=	3.73	feet
Stage at Top of Freeboard =	11.73	feet
Basin Area at Top of Freeboard =	4.59	acres

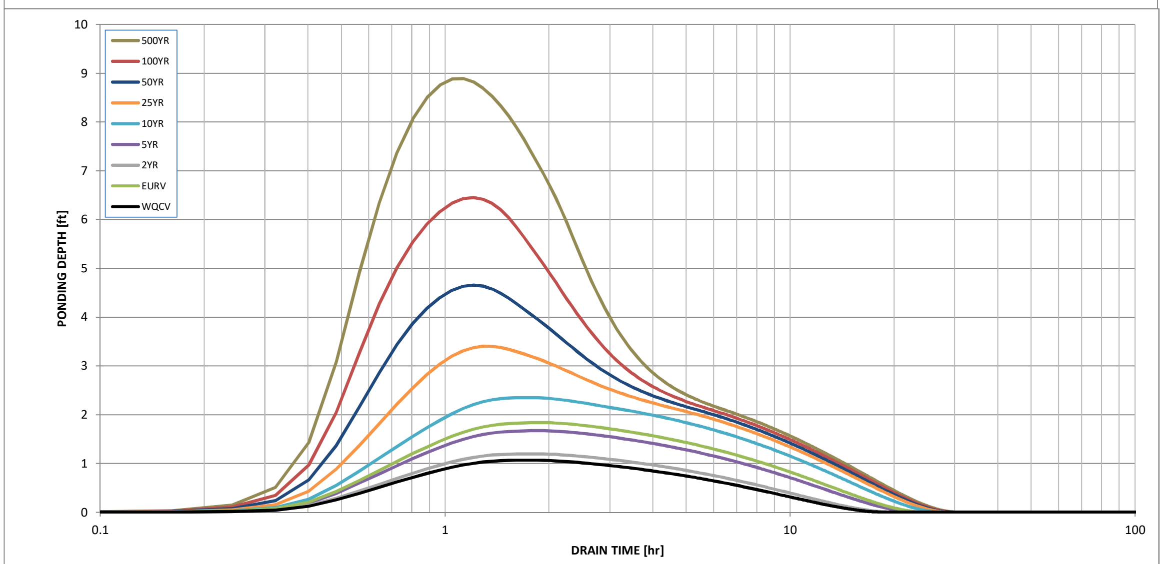
Routed Hydrograph Results

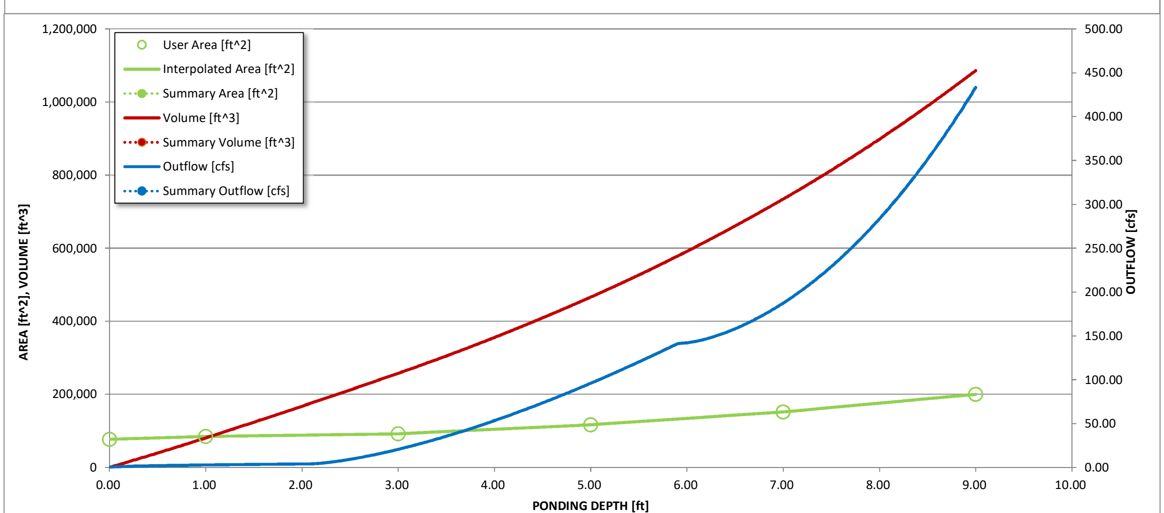
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.14
Calculated Runoff Volume (acre-ft) =	2.331	3.994	2.613	3.634	5.233	8.926	14.939	26.083	52.161
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	2.329	3.992	2.612	3.633	5.230	8.923	14.930	26.071	52.140
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.00	0.01	0.03	0.09	0.27	0.57	1.19
Predevelopment Peak Q (cfs) =	0.0	0.0	0.9	3.4	15.3	44.4	138.1	287.6	602.4
Peak Inflow Q (cfs) =	39.4	67.0	44.1	61.1	87.3	146.9	241.3	410.7	765.3
Peak Outflow Q (cfs) =	2.8	3.7	3.0	3.5	6.7	32.4	80.3	155.8	414.3
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	1.0	0.4	0.7	0.6	0.5	0.7
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Overflow Grate 1	Overflow Grate 1	Overflow Grate 1	Spillway	Spillway
Max Velocity through Grate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	16	21	17	20	23	23	22	20	16
Time to Drain 99% of Inflow Volume (hours) =	17	22	18	21	25	26	25	24	22
Maximum Ponding Depth (ft) =	1.07	1.84	1.20	1.67	2.35	3.40	4.66	6.45	8.89
Area at Maximum Ponding Depth (acres) =	1.95	2.01	1.96	2.00	2.06	2.22	2.58	3.26	4.53
Maximum Volume Stored (acre-ft) =	1.971	3.497	2.225	3.176	4.555	6.777	9.778	15.003	24.430

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename:

Storm Inflow Hydrographs **UD-Detention, Version 3.07 (February 2017)**

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	4.83 min
Hydrograph Constant	1.035

SOURCE	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK
TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0:04:50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0:09:40	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0:14:29	1.69	2.79	1.88	2.56	3.54	5.51	8.10	10.97	14.27
0:19:19	4.62	7.74	5.16	7.08	9.95	16.08	24.98	36.70	53.80
0:24:09	11.86	19.88	13.24	18.18	25.54	41.30	64.26	95.86	146.00
0:28:59	32.54	54.50	36.34	49.86	70.00	112.94	175.18	259.88	393.03
0:33:49	39.42	67.01	44.13	61.08	87.28	146.86	241.27	388.32	654.58
0:38:38	37.77	64.44	42.30	58.67	84.31	143.83	241.02	410.71	765.26
0:43:28	34.38	58.68	38.50	53.40	76.93	131.92	222.54	384.62	742.25
0:48:18	30.89	52.85	34.61	48.09	69.33	119.02	200.99	348.55	688.21
0:53:08	26.89	46.22	30.16	42.02	60.75	104.71	177.55	312.64	626.08
0:57:58	23.39	40.35	26.26	36.66	53.09	91.68	155.62	278.05	568.30
1:02:47	21.19	36.43	23.77	33.12	47.84	82.23	138.83	247.70	516.02
1:07:37	17.69	30.52	19.85	27.73	40.20	69.65	118.73	214.43	453.67
1:12:27	14.60	25.29	16.40	22.96	33.35	57.93	98.94	181.80	388.59
1:17:17	11.47	20.02	12.91	18.15	26.52	46.45	80.01	151.00	330.40
1:22:07	8.77	15.46	9.89	13.99	20.55	36.23	62.75	122.78	279.94
1:26:56	6.48	11.54	7.32	10.43	15.41	27.40	47.87	98.31	235.19
1:31:46	4.90	8.66	5.53	7.84	11.52	20.29	35.43	76.53	196.23
1:36:36	3.98	6.97	4.48	6.31	9.23	16.12	27.82	58.16	160.68
1:41:26	3.36	5.86	3.78	5.32	7.76	13.51	23.18	46.31	128.56
1:46:16	2.93	5.10	3.30	4.63	6.74	11.69	19.97	38.98	101.28
1:51:05	2.63	4.56	2.95	4.14	6.02	10.42	17.73	34.03	83.19
1:55:55	2.41	4.18	2.71	3.80	5.51	9.51	16.14	30.50	71.90
2:00:45	1.78	3.11	2.00	2.82	4.13	7.28	12.65	24.79	60.01
2:05:35	1.30	2.25	1.46	2.04	2.99	5.24	9.12	18.31	46.54
2:10:25	0.95	1.67	1.07	1.51	2.21	3.89	6.76	13.31	34.59
2:15:14	0.71	1.24	0.80	1.12	1.64	2.89	5.02	9.91	25.74
2:20:04	0.51	0.91	0.58	0.82	1.21	2.13	3.72	7.47	19.13
2:24:54	0.37	0.65	0.41	0.59	0.87	1.54	2.70	5.57	14.52
2:29:44	0.27	0.47	0.30	0.43	0.63	1.12	1.95	4.11	11.00
2:34:34	0.18	0.33	0.21	0.30	0.44	0.79	1.40	3.06	8.35
2:39:23	0.12	0.21	0.13	0.19	0.29	0.52	0.94	2.22	6.40
2:44:13	0.06	0.12	0.07	0.11	0.17	0.31	0.57	1.51	4.85
2:49:03	0.03	0.05	0.03	0.05	0.08	0.15	0.29	0.94	3.52
2:53:53	0.01	0.01	0.01	0.01	0.02	0.05	0.11	0.50	2.41
2:58:43	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.20	1.50
3:03:32	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.03	0.81
3:08:22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.32
3:13:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.06
3:18:02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:22:52	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:27:41	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:32:31	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:37:21	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:42:11	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:47:01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:51:50	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:56:40	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:01:30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:06:20	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:11:10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:15:59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:20:49	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:25:39	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:30:29	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:35:19	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:40:08	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:44:58	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:49:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:54:38	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:59:28	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:04:17	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:09:07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:13:57	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:18:47	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:23:37	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:28:26	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:33:16	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:38:06	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:42:56	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:47:46	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Design Procedure Form: Retention Pond (RP)

UD-BMP (Version 3.06, November 2016)

Designer:	**JPS**
Company:	**JPS**
Date:	**August 10, 2019**
Project:	**Walden Preserve**
Location:	**Pond B**

1. Baseflow A) Is the permanent pool established by groundwater?	Choose One ○ YES ◉ NO **THE NET INFLUX OF WATER MUST BE AVAILABLE THROUGH A PERENNIAL BASEFLOW AND MUST EXCEED THE LOSSES.**
2. Surcharge Volume	
A) Effective Imperviousness of Tributary Area, I_a	I_a = 10.4 %
B) Tributary Area's Imperviousness Ratio ($i = I_a / 100$)	i = 0.104
C) Contributing Watershed Area	Area = 504.660 ac
D) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm	d_6 = ______ in
E) Design Concept (Select EURV when also designing for flood control)	Choose One ○ Water Quality Capture Volume (WQCV) ◉ Excess Urban Runoff Volume (EURV)
F) Water Quality Capture Volume (WQCV) Based on 12-hour Drain Time ($V_{WQCV} = (0.8 * (0.91 * i^3 - 1.19 * i^2 + 0.78 * i) / 12 * Area)$	V_{WQCV}= 2.331 ac-ft
G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) ($V_{WQCV\ OTHER} = (d_6*(V_{WQCV}/0.43))$	$V_{WQCV\ OTHER}$= ______ ac-ft
H) User Input of Water Quality Capture Volume (WQCV) (Only if a different WQCV Design Volume is desired)	$V_{WQCV\ USER}$= ______ ac-ft
I) Predominant Watershed NRCS Soil Group	Choose One ◉ A ○ B ○ C / D
J) Excess Urban Runoff Volume (EURV) Design Volume For HSG A: $EURV_A = 1.68 * i^{1.28}$ For HSG B: $EURV_B = 1.36 * i^{1.08}$ For HSG C/D: $EURV_{C/D} = 1.20 * i^{1.08}$	EURV = 3.899 ac-f t
3. Basin Shape (It is recommended to have a basin length-to-width ratio between 2:1 and 3:1)	L : W = 2.0 : 1
4. Permanent Pool A) Minimum Permanent Pool Volume	V_{POOL} = 2.331 ac-ft
B) Depth of the Safety Wetland Bench (Depth between 6 to 12 inches recommended)	D_{LZ} = 6 in
C) Depth of the Open Water Zone (Maximum depth of 12 feet)	D_{OWZ} = 7.0 ft
5. Side Slopes	
A) Maximum Side Slopes Above the Safety Wetland Bench (Horiz. dist. per unit vertical, should be no steeper than 4:1)	Z_{PP} = 4.00 ft / ft
B) Maximum Side Slopes Below the Safety Wetland Bench (Horiz. dist. per unit vertical, should be no steeper than 3:1)	Z_{OWZ} = 3.00 ft / ft

Design Procedure Form: Retention Pond (RP)

Designer:	**JPS**
Company:	**JPS**
Date:	**August 10, 2019**
Project:	**Walden Preserve**
Location:	**Pond B**

6. Inlet

A) Describe means of providing energy dissipation at concentrated inflow locations: Riprap Aprons

7. Forebay

A) Minimum Forebay Volume (V_{FMIN} = 3% of the WQCV) — V_{FMIN} = 0.070 ac-ft

B) Actual Forebay Volume — V_F = ______ ac-ft

8. Outlet

A) Outlet Type — Choose One: ◉ Orifice Plate ○ Other (Describe):

C) Smallest Dimension of Orifice Opening Based on Hydrograph Routing (Use UD-Detention) — $D_{orifice}$ = 0.375 inches

D) Total Outlet Area (A_{ot}) — A_{ot} = 81.240 square inches

9. Trash Rack

A) Water Quality Screen Open Area: A_t = Aot * 38.5*($e^{-0.095D}$) — A_t = 3018 square inches

B) Type of Screen (If specifying an alternative to the materials recommended in the USDCM, indicate "other" and enter the ratio of the total open are to the total screen are for the material specified.) — *Other (Please describe below)*

Amico Klemp SR Series Aluminum Bar Grate; 3/16" Width Bars on 1-3/16" Centers
Cross Rods on 4" Centers

Other (Y/N): Y

C) Ratio of Total Open Area to Total Area (only for type 'Other') — User Ratio = 0.77

D) Total Water Quality Screen Area (based on screen type) — A_{total} = 3920 square inches

E) Inundated Depth of Water Quality Screen below Permanent Pool — $D_{inundated}$ = 4.0 ft

F) Depth of Design Volume (EURV or WQCV) Based on the Design Concept Chosen Under 1.E — H= 2.1 ft

G) Height of Water Quality Screen (H_{TR}) — H_{TR}= 72.96 inches

H) Width of Water Quality Screen Opening ($W_{opening}$) (Minimum of 12 inches is recommended) — $W_{opening}$ = 53.7 inches

Design Procedure Form: Retention Pond (RP)

Designer:	**JPS**
Company:	**JPS**
Date:	**August 10, 2019**
Project:	**Walden Preserve**
Location:	**Pond B**

10. Overflow Embankment	
A) Describe embankment protection for 100-year and greater overtopping:	Riprap Spillway
B) Maximum Embankment Side Slopes (Horiz. dist. per unit vertical, should be no steeper than 4:1)	0.08 **TOO STEEP (< 4)**
11. Maintenance Considerations	
A) Describe Means of Draining the Pond	Pumping to drain water below active level into pond outlet pipe
12. Vegetation	Choose One ○ Irrigated ◉ Not Irrigated
13. Access	
A) Describe Sediment Removal Procedures	Periodic inspection and sediment removal as needed; Draining / pumping as needed to lower pond level for excavation / hauling off sediment

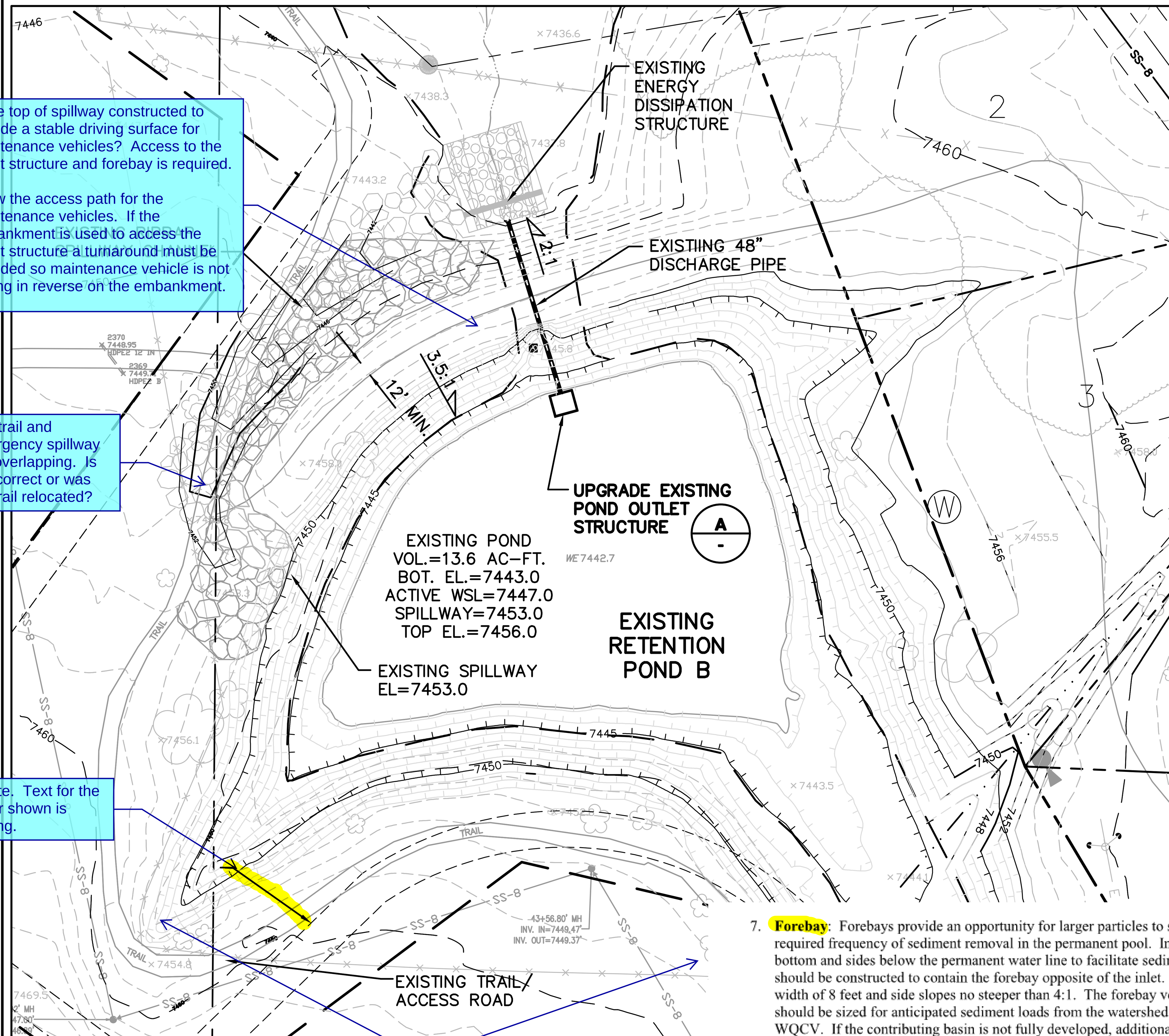
Notes:

Is the top of spillway constructed to provide a stable driving surface for maintenance vehicles? Access to the outlet structure and forebay is required.

Show the access path for the maintenance vehicles. If the embankment is used to access the outlet structure a turnaround must be provided so maintenance vehicle is not driving in reverse on the embankment.

The trail and emergency spillway are overlapping. Is this correct or was the trail relocated?

Update. Text for the leader shown is missing.



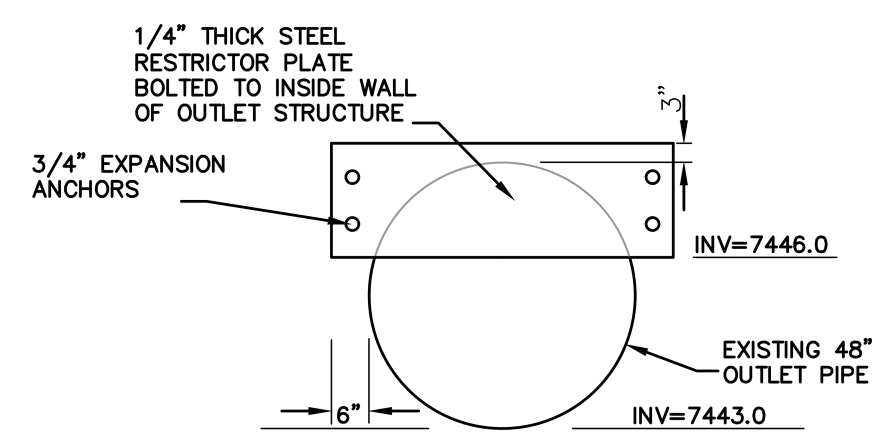
POND B PLAN

Provide Forebay for the inflow. See the snippet to the right for additional design requirements.

7. **Forebay**: Forebays provide an opportunity for larger particles to settle out, which will reduce the required frequency of sediment removal in the permanent pool. Install a solid driving surface on the bottom and sides below the permanent water line to facilitate sediment removal. A soil riprap berm should be constructed to contain the forebay opposite of the inlet. This should have a minimum top width of 8 feet and side slopes no steeper than 4:1. The forebay volume within the permanent pool should be sized for anticipated sediment loads from the watershed and should be at least 3% of the WQCV. If the contributing basin is not fully developed, additional measures should be taken to maintain a relatively clean forebay. This includes more frequent maintenance of the forebay and/or providing and maintaining temporary erosion control.

N

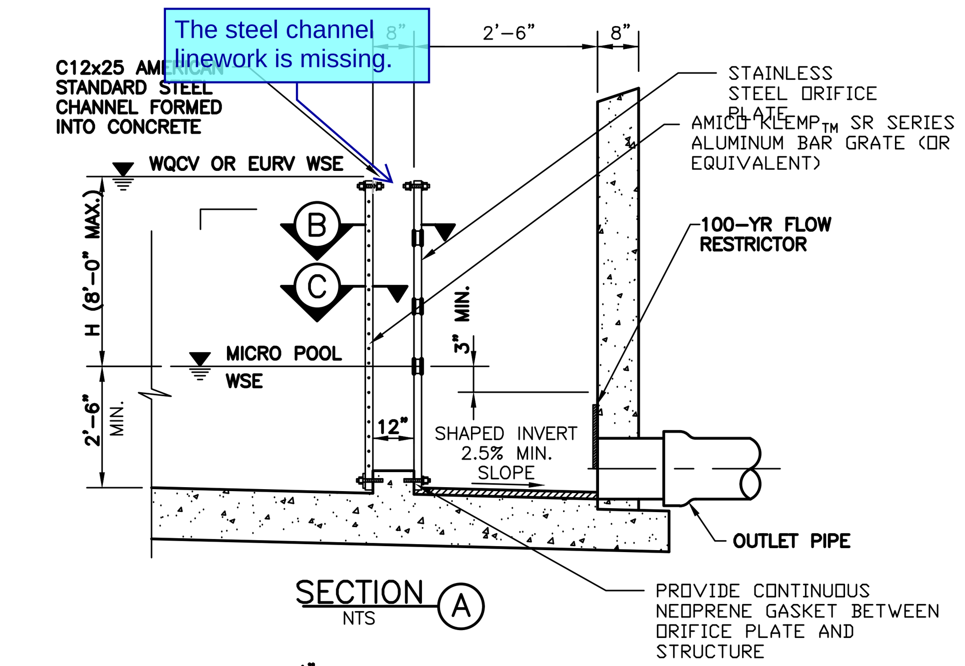
50' 0 50' 100'
5' 0 5' 10'
ORIGINAL SCALE: 1"=50' H, 1"=5' V
CONTOUR INTERVAL=2'



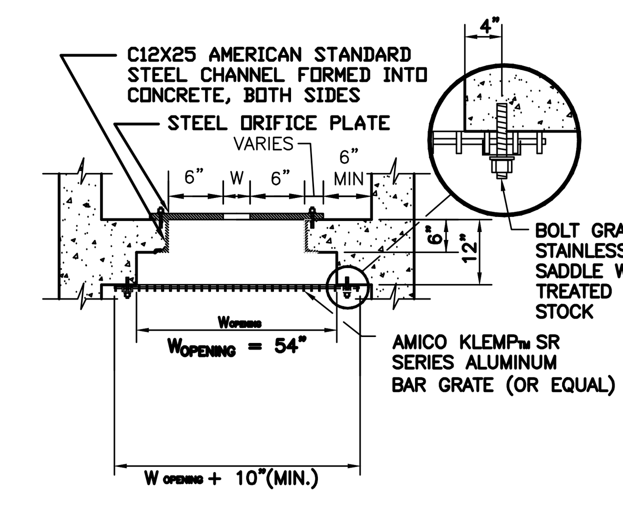
RESTRICTOR PLATE DETAIL (B)
NTS

Include the pond outlet construction plan in the grading and erosion control plan.

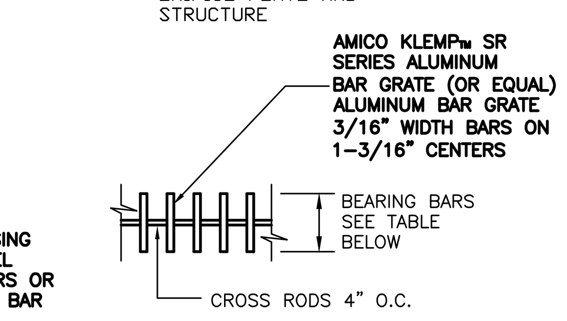
The steel channel linework is missing.



SECTION (A)
NTS



SECTION (B)
NTS



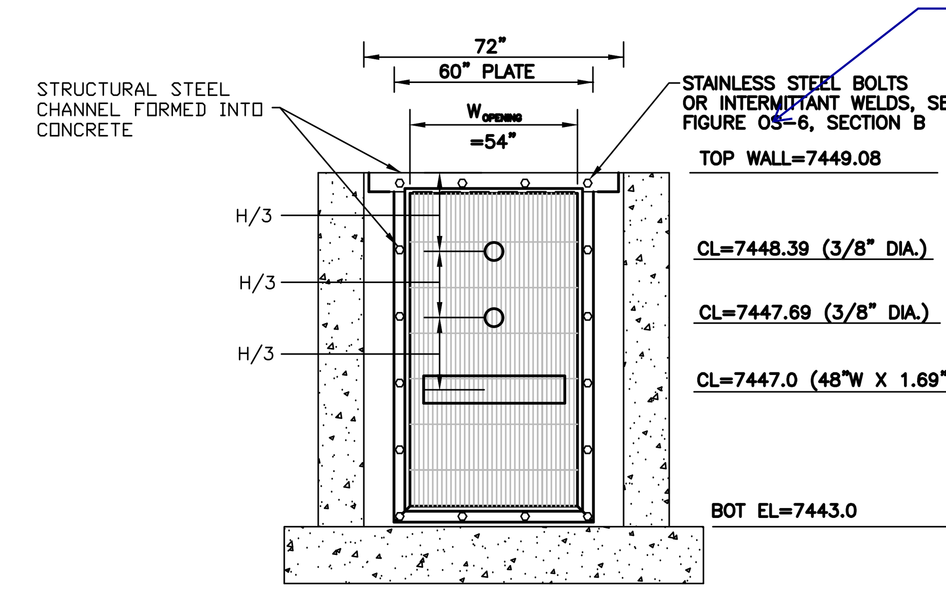
WATER DEPTH ABOVE LOWEST OPENING, H	MINIMUM BEARING BAR SIZE, BARS ALIGNED VERTICALLY
4.0 FT.	1-3/4" x 3/16"

R VALUE=(NET OPEN AREA)/GROSS RACK AREA) =0.77 FOR CROSS RODS ON 4" CENTERS

SECTION (C)
NTS

TRASH RACK & ORIFICE PLATE DETAIL (C)
(BASED ON UDFCD VOL. 3 FIGURE OS-6)

Update reference.

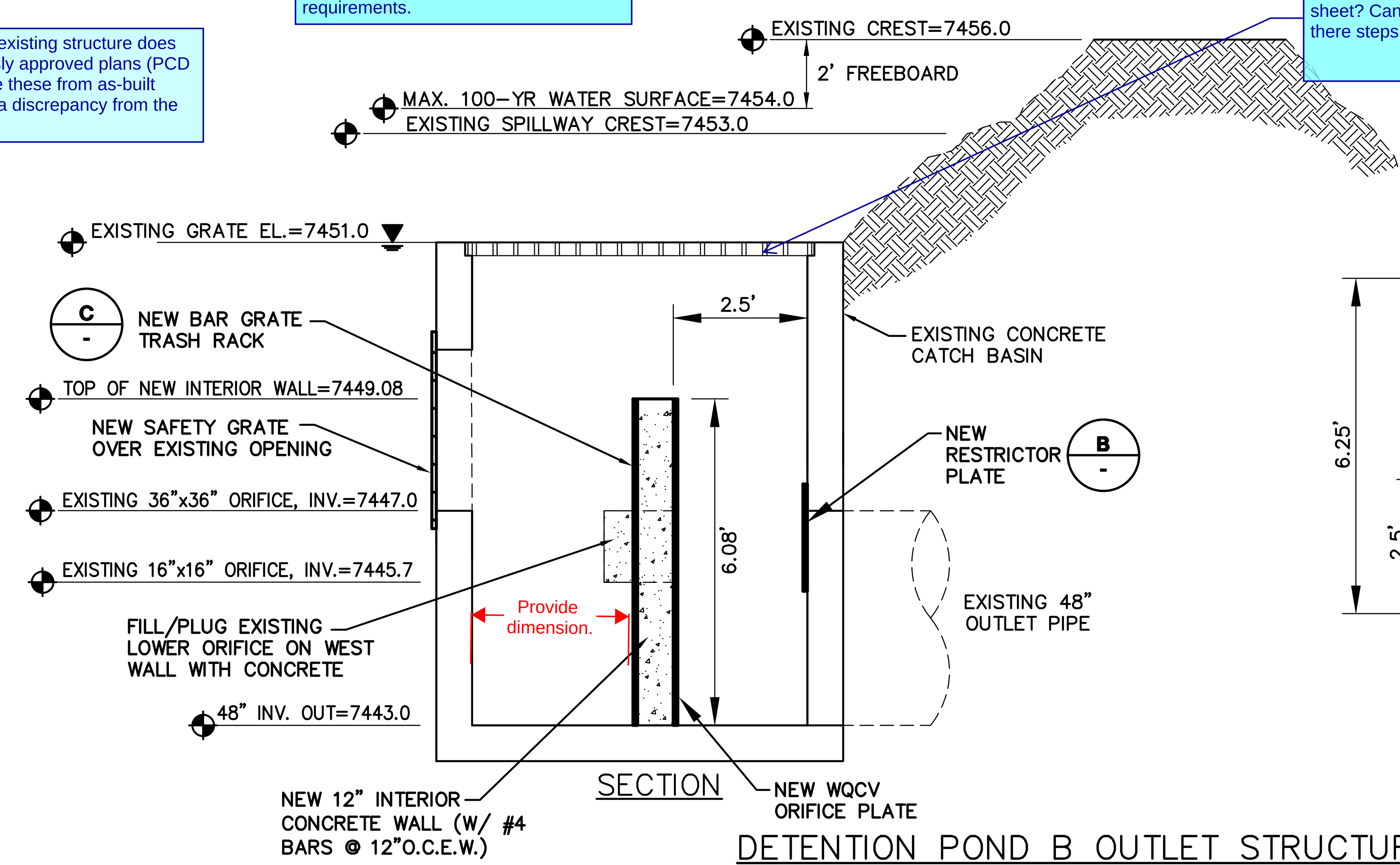


ELEVATION
NTS

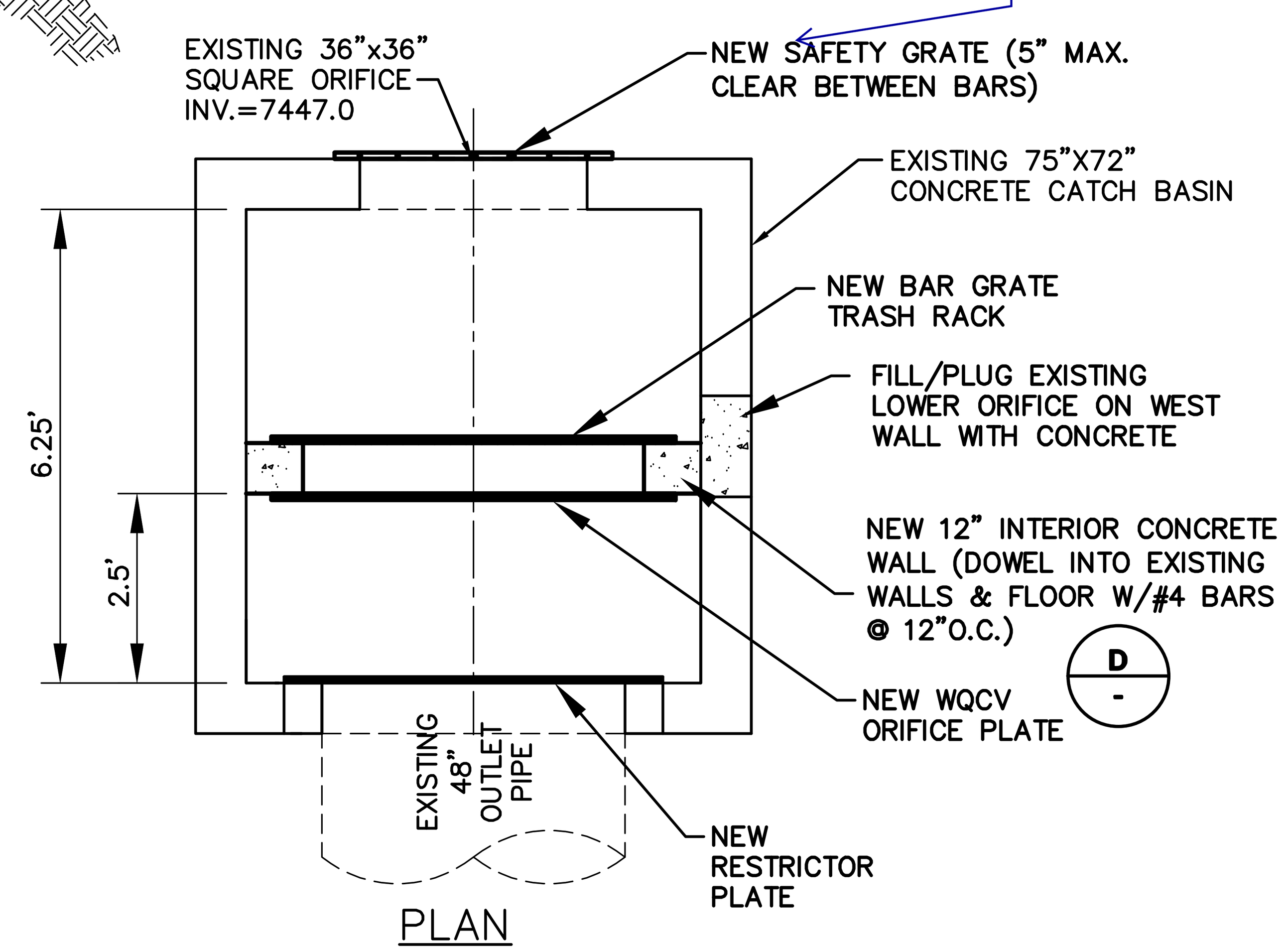
Provide construction level detail for the safety grate. What's the size of the safety bars? How is this installed to the face of the outlet structure? How much more does this extend beyond the opening?

How is the existing grate mounted? Does this meet the comply with the overflow safety grate note on this sheet? Can this be opened for maintenance? Are there steps installed?

The elevations for the existing structure does not match the previously approved plans (PCD File No. SF1416). Are these from as-built survey? Why is there a discrepancy from the previous plans?



SECTION



PLAN

DETENTION POND B OUTLET STRUCTURE (A)
SCALE: 1/2"=1'-0"

ORIFICE PLATE NOTES:

1. PROVIDE CONTINUOUS NEOPRENE GASKET MATERIAL BETWEEN THE ORIFICE PLATE AND CON[CRETE]
2. BOLT PLATE TO CONCRETE 12" MAX. ON CENTER. SEE TABLE OS-2 FOR PLATE THICKNESS

Either include Table OS-2 and highlight the required plate thickness or directly identify on the plans the required plate thickness.

EURV AND WQCV TRASH RACKS:

1. WELL-SCREEN TRASH RACKS SHALL BE STAINLESS STEEL AND SHALL BE ATTACHED BY INTERMITTENT WELDS ALONG THE EDGE OF THE MOUNTING FRAME.
2. BAR GATE TRASH RACKS SHALL BE ALUMINUM AND SHALL BE BOLTED USING STAINLESS STEEL HARDWARE.
3. TRASH RACK OPEN AREAS ARE FOR SPECIFIED TRASH RACK MATERIALS. TOTAL TRASH RACK SIZE MAY NEED TO BE ADJUSTED FOR MATERIALS HAVING DIFFERENT OPEN AREA/GROSS AREA RATIO (R VALUE).
4. STRUCTURAL DESIGN OF TRASH RACKS SHALL BE BASED ON FULL HYDROSTATIC HEAD WITH ZERO HEAD DOWNSTREAM OF THE RACK.

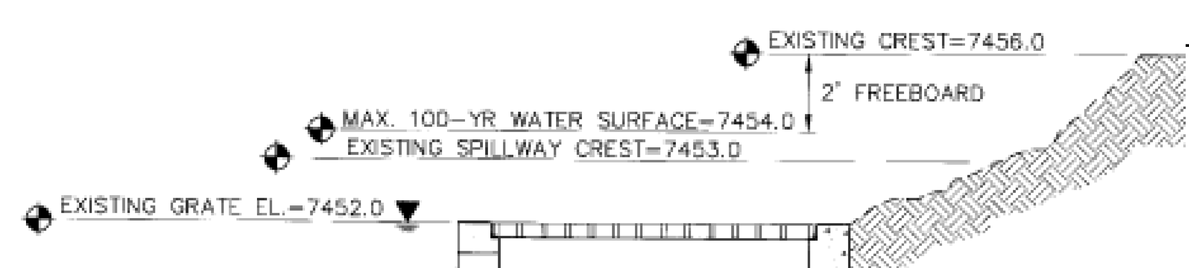
OVERFLOW SAFETY GRATES:

1. ALL SAFETY GRATES SHALL BE MOUNTED USING STAINLESS STEEL HARDWARE AND PROVIDED WITH HINGED AND LOCKABLE OR BOLTABLE ACCESS PANELS.
2. SAFETY GRATES SHALL BE STAINLESS STEEL, ALUMINUM, OR STEEL. STEEL GRATES SHALL BE HOT DIP GALVANIZED AND MAY BE HOT POWDER COATED AFTER GALVANIZING.
3. SAFETY GRATES SHALL BE DESIGNED SUCH THAT THE DIAGONAL DIMENSION OF EACH OPENING IS SMALLER THAN THE DIAMETER OF THE OUTLET PIPE.
4. STRUCTURAL DESIGN OF SAFETY GRATES SHALL BE BASED ON FULL HYDROSTATIC HEAD WITH ZERO HEAD DOWNSTREAM OF THE RACK.

Remove notes 3 & 4. These are design notes, not construction notes. Notes 3 & 4 should have already been accounted for by the design engineer in drafting the construction details.

Similar comment for note 3 & 4 on the overflow safety grates notes.

ORIFICE PLATE AND TRASH RACK DETAIL & NOTES (D)
(BASED ON UDFCD VOL. 3 FIGURE OS-4)



Z:\040201.walden\dwg\civil\WALDEN PRESERVE 2\FLG-4\C5.dwg Aug 10, 2019 - 7:02pm

JPS ENGINEERING
19 E. Willamette Ave.
Colorado Springs, CO 80903
PH: 719-477-9429
FAX: 719-471-0766
www.jpsengr.com

CALL UTILITY NOTIFICATION CENTER OF COLORADO
1-800-922-1987
CALL 2-BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE, OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES.

No.	REVISION	BY	DATE
1	FINAL PLAT SUBMITTAL	JPS	7/23/14
2	EPC COMMENTS	JPS	9/17/14
3	FSD OUTLET STRUCTURE UPGRADE	JPS	8/9/19

WALDEN PRESERVE 2 - FILING NO. 1

POND B - OUTLET STRUCTURE UPGRADE DETAILS

HORZ. SCALE:	1"=50'	DRAWN:	MJP
VERT. SCALE:	N/A	DESIGNED:	JPS
SURVEYED:	RAMPART	CHECKED:	JPS
CREATED:	7/17/14	LAST MODIFIED:	8/9/19
PROJECT NO:	040201	MODIFIED BY:	BJJ

SHEET: C5

APPENDIX D2

STORMWATER QUALITY CALCULATIONS – RAIN GARDEN C2

WALDEN PRESERVE

IMPERVIOUS AREA CALCULATIONS

BASIN	TOTAL AREA (AC)	SOIL TYPE	(AC)	SUB-AREA 1 DEVELOPMENT/ COVER	% IMP.	AREA (AC)	SUB-AREA 2 DEVELOPMENT/ COVER	% IMP.	(AC)	SUB-AREA 3 DEVELOPMENT/ COVER	% IMP.	WEIGHTED PERCENT IMPERVIOUS
OA1,A1-A9	341.96	B										6.20
OB1,B1-B4	90.90	B										25.00
B5	4.78	B	4.78	2.5-AC. LOTS	11.0							11.00
B6	7.91	B	7.91	2.5-AC. LOTS	11.0							11.00
B5,B6	12.69	B										11.00
B7	3.24	B	3.24	1-AC LOTS	20.0							20.00
B8	28.74	B	28.74	1-AC LOTS	20.0							20.00
B5-B8	44.67	B										17.44
B9	10.23	B	10.23	1-AC LOTS	20.0							20.00
B10	16.90	B	71.17	1-AC LOTS	20.0							20.00
B5-B10	71.80	B										18.41
OB1,B1-B10	162.70	B										22.09
OA1,OB1,A,B	504.66	B										11.32
OC1	128.95	B	128.95	1/2-AC. LOTS	25.0							25.00
C1	11.75	B	5.0	1-AC. LOTS	20.0	6.8	5-AC. LOTS	7.0				12.53
C2	11.02	B	11.02	1-AC LOTS	20.0							20.00
C1-C2	**22.77**	**B**										**16.15**
C3	2.04	B	2.04	1-AC LOTS	20.0							20.00
C4	15.59	B	15.59	1-AC LOTS	20.0							20.00
C1-C4	40.40	B										17.83
C5	41.43	B	41.43	1-AC LOTS	20.0							20.00
C6	1.32	B	1.32	1-AC LOTS	20.0							20.00
C8	12.44	B	12.44	1-AC LOTS	20.0							20.00
C5,C6,C8	**55.19**	**B**										**20.00**
C9	4.50	B	4.5	1-AC LOTS	20.0							20.00
OC1,C1-C9	229.04	B										22.43
OA1-OC1,A1-C9	733.70	B										14.79
OC2	81.72	B	81.72	1/2-AC. LOTS	25.0							25.00
C10	2.86	B	2.86	1-AC LOTS	20.0							20.00
C11	8.63	B	8.63	1-AC LOTS	20.0							20.00
C12	17.77	B	17.77	1-AC LOTS	20.0							20.00
C10-C12	**29.26**	**B**										**18.05**
C13	22.44	B	22.44	MEADOW	0.0							0.00
OC2,C11-C13	133.42	B										19.27
OD1	3.30	B	3.30	1-AC LOTS	20.0							20.00
OD2	4.50	B	4.50	FOREST	0.0							0.00
OD3	10.30	B	10.30	FOREST	0.0							0.00
OD4	6.00	B	6.00	FOREST	0.0							0.00
D	10.27	B	10.27	1-AC LOTS	20.0							20.00
OD1-OD4, D	34.37	B										7.90
OA1-OD1,A-D	901.49	B										15.19

Design Procedure Form: Rain Garden (RG)

UD-BMP (Version 3.06, November 2016)

Designer: JPS
Company: JPS
Date: July 11, 2019
Project: Walden Preserve 2
Location: Rain Garden C2

1. Basin Storage Volume	
A) Effective Imperviousness of Tributary Area, I_a (100% if all paved and roofed areas upstream of rain garden)	I_a = 16.2 %
B) Tributary Area's Imperviousness Ratio ($i = I_a/100$)	i = 0.162
C) Water Quality Capture Volume (WQCV) for a 12-hour Drain Time (WQCV= 0.8 * (0.91* i^3 - 1.19 * i^2 + 0.78 * i)	WQCV = 0.08 watershed inches
D) Contributing Watershed Area (including rain garden area)	Area = 993,168 sq ft
E) Water Quality Capture Volume (WQCV) Design Volume Vol = (WQCV / 12) * Area	V_{WQCV} = 6,555 cu ft
F) For Watersheds Outside of the Denver Region, Depth of Average Runoff Producing Storm	d_6 = in
G) For Watersheds Outside of the Denver Region, Water Quality Capture Volume (WQCV) Design Volume	$V_{WQCV\ OTHER}$ = cu ft
H) User Input of Water Quality Capture Volume (WQCV) Design Volume (Only if a different WQCV Design Volume is desired)	$V_{WQCV\ USER}$ = cu ft
2. Basin Geometry	
A) WQCV Depth (12-inch maximum)	D_{WQCV} = 12 in
B) Rain Garden Side Slopes (Z = 4 min., horiz. dist per unit vertical) (Use "0" if rain garden has vertical walls)	Z = 4.00 ft / ft
C) Mimimum Flat Surface Area	A_{Min} = 3218 sq ft
D) Actual Flat Surface Area	A_{Actual} = 7441 sq ft
E) Area at Design Depth (Top Surface Area)	A_{Top} = 8871 sq ft
F) Rain Garden Total Volume (V_T= ((A_{Top} + A_{Actual}) / 2) * Depth)	V_T= 8,156 cu ft
3. Growing Media	Choose One ◉ 18" Rain Garden Growing Media ○ Other (Explain):
4. Underdrain System	
A) Are underdrains provided?	Choose One ◉ YES ○ NO
B) Underdrain system orifice diameter for 12 hour drain time	
i) Distance From Lowest Elevation of the Storage Volume to the Center of the Orifice	y = 2.0 ft
ii) Volume to Drain in 12 Hours	Vol_{12} = 6,555 cu ft
iii) Orifice Diameter, 3/8" Minimum	D_O = 1 7/8 in

Design Procedure Form: Rain Garden (RG)

Designer: JPS
Company: JPS
Date: July 11, 2019
Project: Walden Preserve 2
Location: Rain Garden C2

5. Impermeable Geomembrane Liner and Geotextile Separator Fabric A) Is an impermeable liner provided due to proximity of structures or groundwater contamination?	Choose One ○ YES ◉ NO
6. Inlet / Outlet Control A) Inlet Control	Choose One ○ Sheet Flow- No Energy Dissipation Required ◉ Concentrated Flow- Energy Dissipation Provided
7. Vegetation	Choose One ◉ Seed (Plan for frequent weed control) ○ Plantings ○ Sand Grown or Other High Infiltration Sod
8. Irrigation A) Will the rain garden be irrigated?	Choose One ○ YES ○ NO

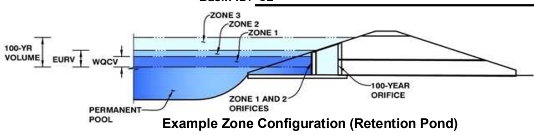
Notes:

DETENTION BASIN STAGE-STORAGE TABLE BUILDER

UD-Detention, Version 3.07 (February 2017)

Project: Walden Preserve 2

Basin ID: C2



Example Zone Configuration (Retention Pond)

Required Volume Calculation

			Optional User Override 1-hr Precipitation	
Selected BMP Type =	RG			
Watershed Area =	22.77	acres		
Watershed Length =	1,670	ft		
Watershed Slope =	0.061	ft/ft		
Watershed Imperviousness =	16.15%	percent		
Percentage Hydrologic Soil Group A =	0.0%	percent		
Percentage Hydrologic Soil Group B =	100.0%	percent		
Percentage Hydrologic Soil Groups C/D =	0.0%	percent		
Desired WQCV Drain Time =	12.0	hours		
Location for 1-hr Rainfall Depths =	User Input			
Water Quality Capture Volume (WQCV) =	0.150	acre-feet		
Excess Urban Runoff Volume (EURV) =	0.359	acre-feet		
2-yr Runoff Volume (P1 = 1.19 in.) =	0.259	acre-feet	1.19	inches
5-yr Runoff Volume (P1 = 1.5 in.) =	0.388	acre-feet	1.50	inches
10-yr Runoff Volume (P1 = 1.75 in.) =	0.721	acre-feet	1.75	inches
25-yr Runoff Volume (P1 = 2 in.) =	1.556	acre-feet	2.00	inches
50-yr Runoff Volume (P1 = 2.25 in.) =	2.086	acre-feet	2.25	inches
100-yr Runoff Volume (P1 = 2.52 in.) =	2.779	acre-feet	2.52	inches
500-yr Runoff Volume (P1 = 3.14 in.) =	4.134	acre-feet	3.14	inches
Approximate 2-yr Detention Volume =	0.241	acre-feet		
Approximate 5-yr Detention Volume =	0.363	acre-feet		
Approximate 10-yr Detention Volume =	0.627	acre-feet		
Approximate 25-yr Detention Volume =	0.806	acre-feet		
Approximate 50-yr Detention Volume =	0.851	acre-feet		
Approximate 100-yr Detention Volume =	1.065	acre-feet		

Stage-Storage Calculation

Zone 1 Volume (WQCV) =	0.150	acre-feet
Zone 2 Volume (User Defined - Zone 1) =	0.035	acre-feet
Select Zone 3 Storage Volume (Optional) =		acre-feet
Total Detention Basin Volume =	0.185	acre-feet

Total detention volume is less than 100-year volume.

Depth Increment = ft

Stage - Storage Description	Stage (ft)	Optional Override Stage (ft)	Length (ft)	Width (ft)	Area (ft^2)	Optional Override Area (ft^2)	Area (acre)	Volume (ft^3)	Volume (ac-ft)
Media Surface	--	0.00	--	--	--	7,441	0.171		
WQCV EL=7441.0	--	1.00	--	--	--	8,871	0.204	8,067	0.185
Top EL=7442.0	--	2.00	--	--	--	10,300	0.236	17,639	0.405
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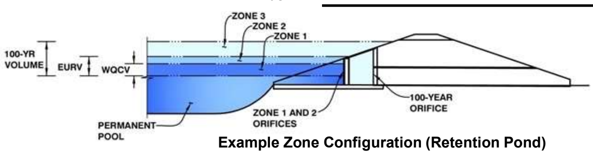
Update pond to include stage/area information up to the top of bank. Per the outlet structure design the required top of bank elevation is at elevation 7442.9.

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)

Project: Walden Preserve 2

Basin ID: C2



Example Zone Configuration (Retention Pond)

	Stage (ft)	Zone Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	0.82	0.150	Filtration Media
Zone 2 (User)	0.99	0.035	Weir&Pipe (Restrict)
Zone 3			
		0.185	Total

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	2.00	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	1.88	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	0.0	ft^2
Underdrain Orifice Centroid =	0.08	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Invert of Lowest Orifice =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	N/A	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft^2
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft^2

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (optional)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Orifice Area (sq. inches)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =			ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =			ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =			inches

Calculated Parameters for Vertical Orifice

	Not Selected	Not Selected	
Vertical Orifice Area =			ft^2
Vertical Orifice Centroid =			feet

User Input: Overflow Weir (Dropbox) and Grate (Flat or Sloped)

	Zone 2 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	0.82		ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	3.00		feet
Overflow Weir Slope =	0.00		H:V (enter zero for flat grate)
Horiz. Length of Weir Sides =	6.00		feet
Overflow Grate Open Area % =	70%		%, grate open area/total area
Debris Clogging % =	50%		%

Calculated Parameters for Overflow Weir

	Zone 2 Weir	Not Selected	
Height of Grate Upper Edge, H_t =	0.82		feet
Over Flow Weir Slope Length =	6.00		feet
Grate Open Area / 100-yr Orifice Area =	1478.51		should be $\geq$ 4
Overflow Grate Open Area w/o Debris =	12.60		ft^2
Overflow Grate Open Area w/ Debris =	6.30		ft^2

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 2 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	2.00		ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	1.25		inches
Restrictor Plate Height Above Pipe Invert =	6.00		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 2 Restrictor	Not Selected	
Outlet Orifice Area =	0.01		ft^2
Outlet Orifice Centroid =	0.05		feet
Half-Central Angle of Restrictor Plate on Pipe =	3.14	N/A	radians

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage=	1.00	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	15.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

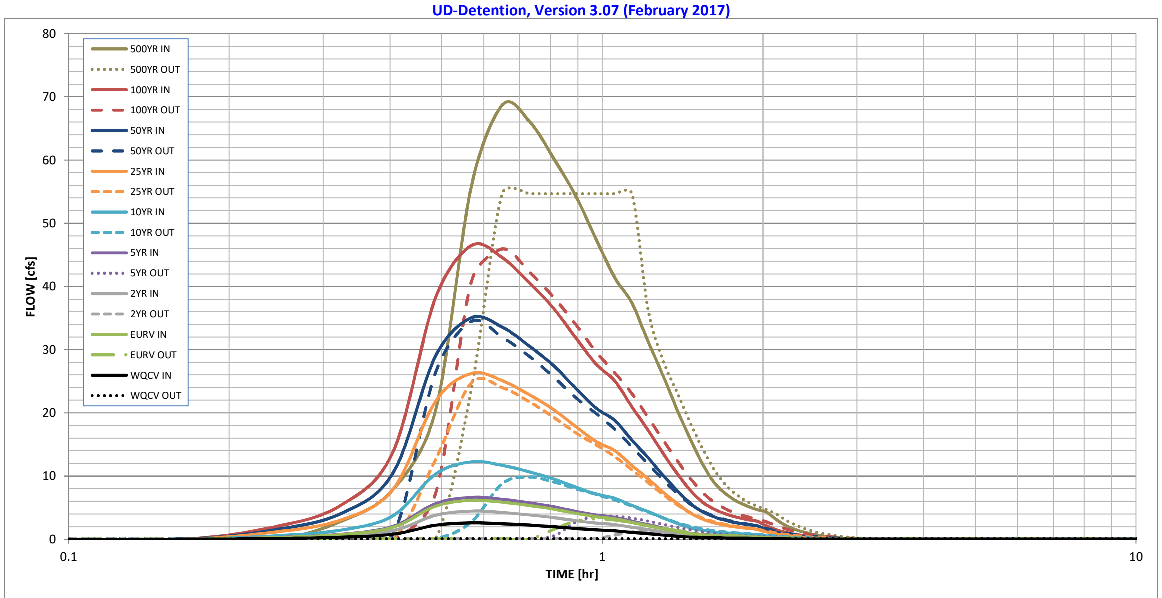
Spillway Design Flow Depth=	0.90	feet
Stage at Top of Freeboard =	2.90	feet
Basin Area at Top of Freeboard =	0.24	acres

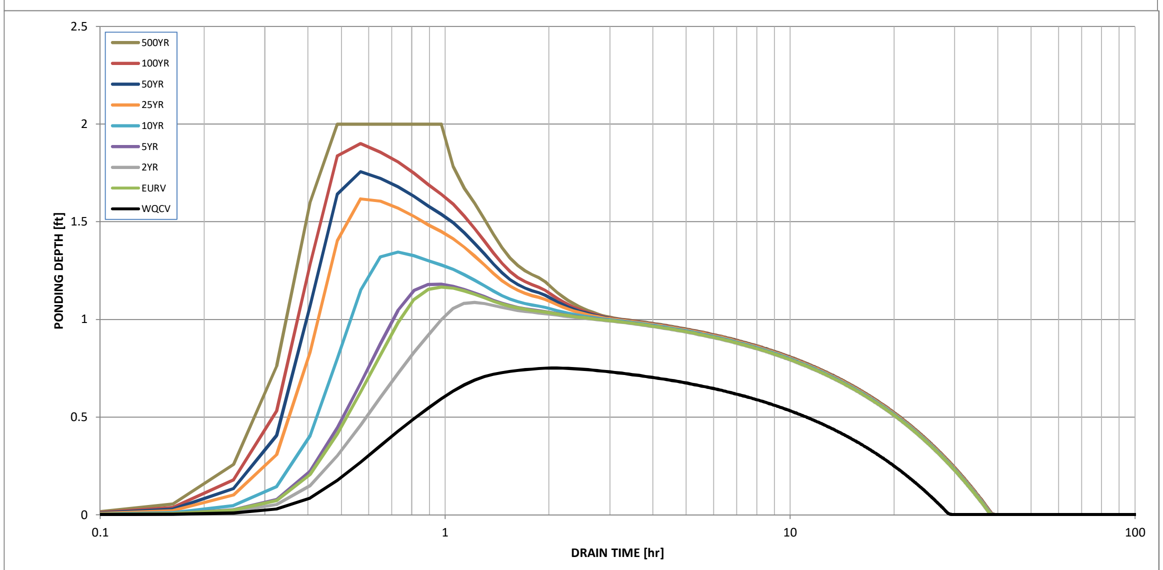
Routed Hydrograph Results

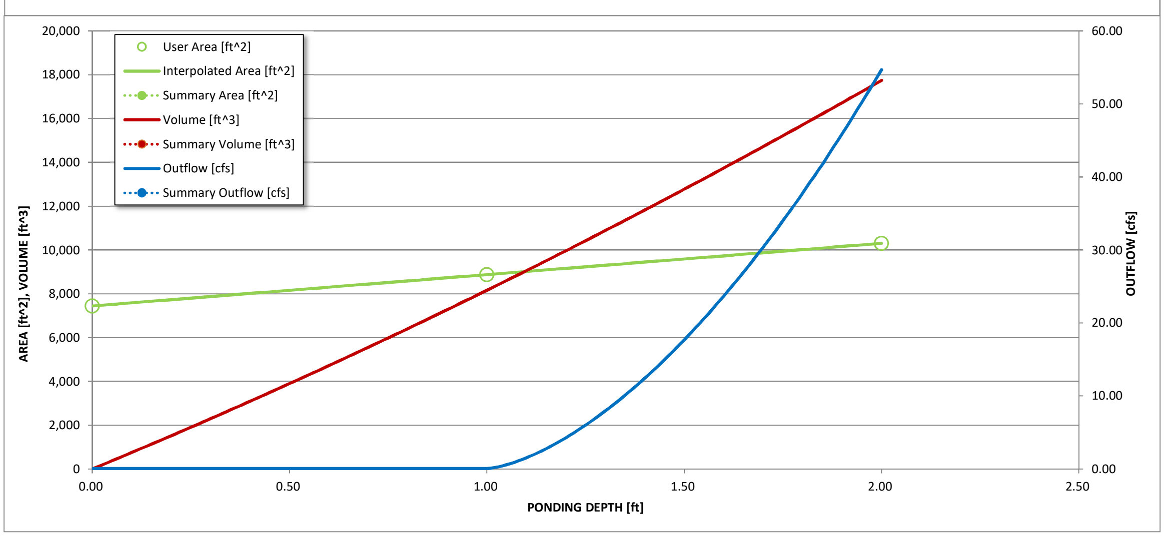
Design Storm Return Period =	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year	500 Year
One-Hour Rainfall Depth (in) =	0.53	1.07	1.19	1.50	1.75	2.00	2.25	2.52	3.14
Calculated Runoff Volume (acre-ft) =	0.150	0.359	0.259	0.388	0.721	1.556	2.086	2.779	4.134
OPTIONAL Override Runoff Volume (acre-ft) =									
Inflow Hydrograph Volume (acre-ft) =	0.149	0.359	0.258	0.387	0.719	1.555	2.084	2.776	4.130
Predevelopment Unit Peak Flow, q (cfs/acre) =	0.00	0.00	0.01	0.02	0.20	0.66	0.91	1.23	1.82
Predevelopment Peak Q (cfs) =	0.0	0.0	0.3	0.5	4.5	15.1	20.8	28.0	41.4
Peak Inflow Q (cfs) =	2.6	6.2	4.4	6.6	12.2	26.2	35.0	46.5	68.7
Peak Outflow Q (cfs) =	0.1	3.2	1.2	3.6	9.8	24.7	34.5	45.9	54.7
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	7.7	2.2	1.6	1.7	1.6	1.3
Structure Controlling Flow =	Outlet Plate 1	Spillway	Spillway	Spillway	Spillway	Spillway	Spillway	Spillway	Spillway
Max Velocity through Grate 1 (fps) =	N/A	-0.01	-0.01	0.0	0.0	0.0	0.0	0.0	0.0
Max Velocity through Grate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	28	36	37	36	34	29	26	22	15
Time to Drain 99% of Inflow Volume (hours) =	**29**	37	38	37	37	35	34	33	30
Maximum Ponding Depth (ft) =	0.75	1.17	1.09	1.18	1.34	1.62	1.76	1.90	2.00
Area at Maximum Ponding Depth (acres) =	0.20	0.21	0.21	0.21	0.21	0.22	0.23	0.23	0.24
Maximum Volume Stored (acre-ft) =	0.137	0.220	0.204	0.224	0.258	0.318	0.349	0.384	0.405

Detention Basin Outlet Structure Design

UD-Detention, Version 3.07 (February 2017)







S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

Detention Basin Outlet Structure Design

Outflow Hydrograph Workbook Filename:

Storm Inflow Hydrographs UD-Detention, Version 3.07 (February 2017)

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval: 4.87 min

Hydrograph Constant: 1.026

SOURCE	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK	WORKBOOK
TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0:04:52	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0:09:44	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0:14:37	0.12	0.28	0.20	0.30	0.54	1.14	1.50	1.97	2.85
0:19:29	0.31	0.74	0.54	0.80	1.46	3.09	4.11	5.43	7.93
0:24:21	0.81	1.90	1.38	2.05	3.75	7.94	10.56	13.93	20.35
0:29:13	2.22	5.23	3.79	5.62	10.30	21.81	28.98	38.23	55.79
0:34:05	2.59	6.16	4.45	6.63	12.25	26.23	35.04	46.48	68.70
0:38:58	2.46	5.87	4.23	6.32	11.69	25.10	33.56	44.56	66.10
0:43:50	2.23	5.34	3.85	5.75	10.65	22.85	30.55	40.56	60.20
0:48:42	1.98	4.75	3.42	5.12	9.51	20.49	27.43	36.47	54.23
0:53:34	1.69	4.09	2.93	4.41	8.22	17.79	23.86	31.80	47.44
0:58:26	1.48	3.57	2.56	3.85	7.16	15.46	20.74	27.70	41.41
1:03:19	1.34	3.23	2.32	3.48	6.49	14.02	18.80	25.07	37.38
1:08:11	1.09	2.65	1.90	2.86	5.36	11.66	15.68	20.94	31.33
1:13:03	0.87	2.15	1.54	2.32	4.38	9.60	12.93	17.31	25.96
1:17:55	0.66	1.64	1.16	1.77	3.38	7.50	10.15	13.64	20.57
1:22:47	0.47	1.21	0.85	1.31	2.52	5.69	7.74	10.46	15.89
1:27:40	0.35	0.88	0.62	0.95	1.82	4.16	5.70	7.75	11.86
1:32:32	0.28	0.69	0.49	0.74	1.41	3.17	4.32	5.85	8.90
1:37:24	0.23	0.57	0.40	0.61	1.16	2.59	3.51	4.73	7.16
1:42:16	0.19	0.48	0.34	0.52	0.99	2.19	2.97	3.99	6.02
1:47:08	0.17	0.42	0.30	0.46	0.86	1.91	2.59	3.48	5.24
1:52:01	0.16	0.38	0.27	0.41	0.78	1.72	2.32	3.12	4.69
1:56:53	0.14	0.35	0.25	0.38	0.72	1.58	2.13	2.86	4.29
2:01:45	0.11	0.26	0.19	0.28	0.53	1.16	1.57	2.12	3.20
2:06:37	0.08	0.19	0.14	0.21	0.39	0.85	1.15	1.54	2.32
2:11:29	0.06	0.14	0.10	0.15	0.28	0.62	0.84	1.13	1.71
2:16:22	0.04	0.10	0.07	0.11	0.21	0.46	0.63	0.84	1.27
2:21:14	0.03	0.07	0.05	0.08	0.15	0.33	0.45	0.61	0.93
2:26:06	0.02	0.05	0.04	0.06	0.11	0.24	0.32	0.44	0.67
2:30:58	0.01	0.04	0.03	0.04	0.08	0.17	0.24	0.32	0.49
2:35:50	0.01	0.02	0.02	0.03	0.05	0.12	0.16	0.22	0.34
2:40:43	0.00	0.01	0.01	0.01	0.03	0.07	0.10	0.14	0.22
2:45:35	0.00	0.01	0.00	0.01	0.01	0.04	0.06	0.08	0.12
2:50:27	0.00	0.00	0.00	0.00	0.00	0.02	0.02	0.03	0.06
2:55:19	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.02
3:00:11	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:05:04	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:09:56	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:14:48	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:19:40	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:24:32	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:29:25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:34:17	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:39:09	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:44:01	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:48:53	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:53:46	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3:58:38	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:03:30	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:08:22	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:13:14	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:18:07	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:22:59	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:27:51	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:32:43	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:37:35	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:42:28	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:47:20	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:52:12	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
4:57:04	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:01:56	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:06:49	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:11:41	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:16:33	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:21:25	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:26:17	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:31:10	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:36:02	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:40:54	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5:45:46	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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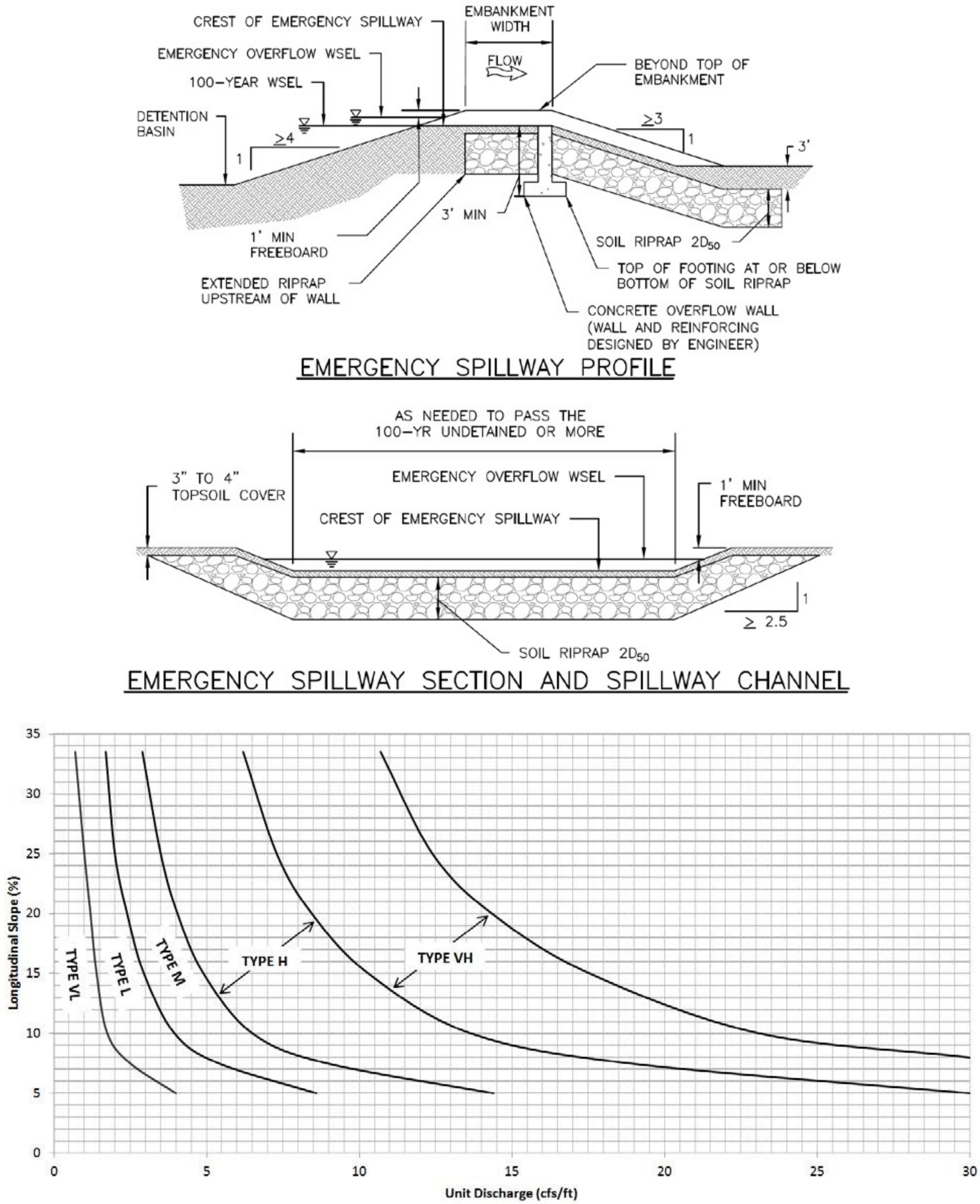


Figure 12-21. Embankment protection details and rock sizing chart (adapted from Arapahoe County)

RAIN GARDEN C2: UNIT DISCHARGE = (46.8 CFS / 15.0 FT) = 3.1; SLOPE = 25.0%
USE TYPE M RIPRAP

APPENDIX E

DRAINAGE COST ESTIMATE

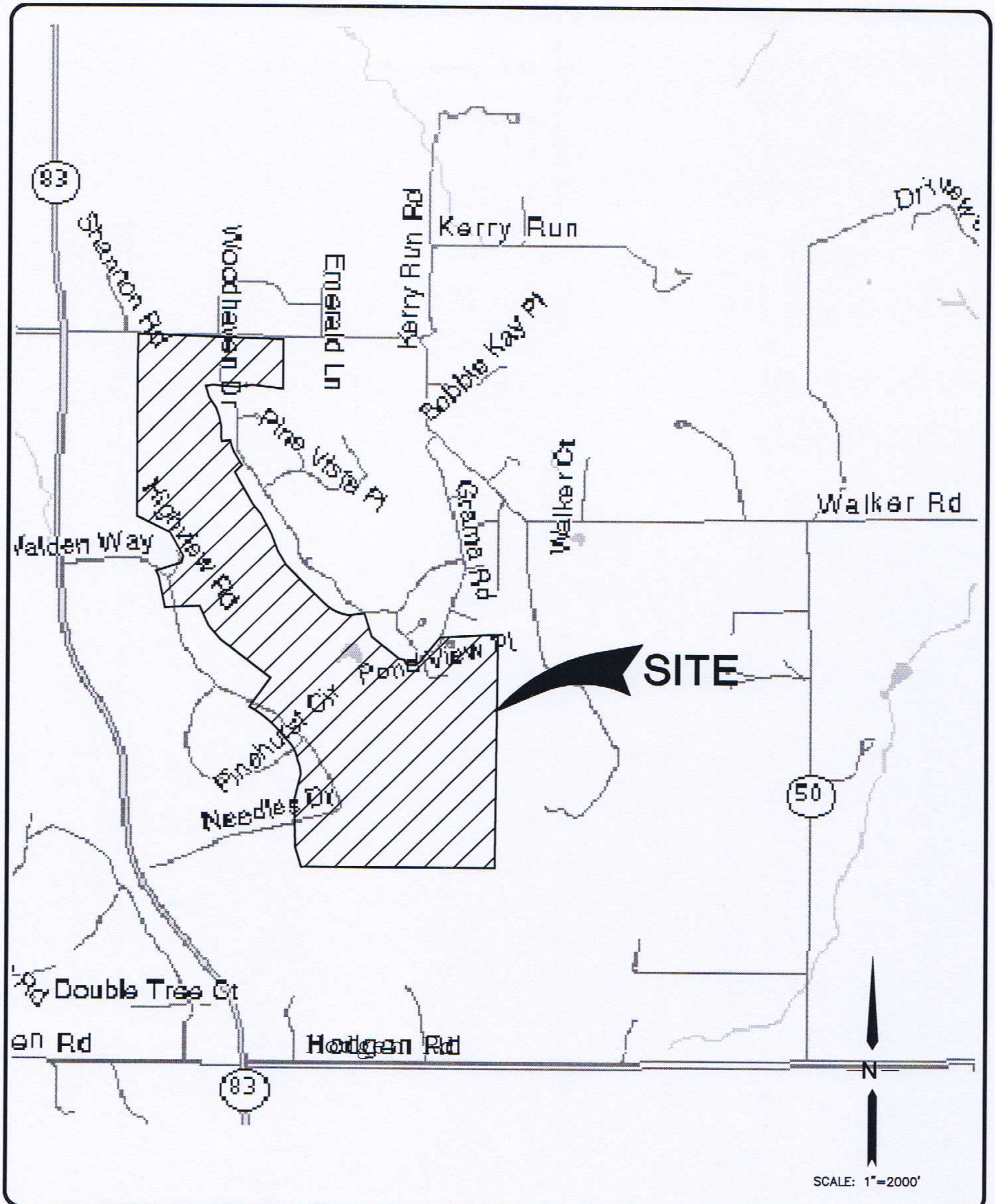
WALDEN PRESERVE 2 FILING NO. 4
DRAINAGE IMPROVEMENTS COST ESTIMATE

Item No.	Description	Quantity	Unit	Unit Cost ($$$)	Total Cost ($$$)
	PUBLIC DRAINAGE IMPROVEMENTS (NON-REIMBURSABLE)				
	24" RCP Culvert	160	LF	$84	$13,440
	24" RCP FES	4	EA	$504	$2,016
	Riprap (d_{50} = 12")	10	CY	$98	$980
	SUBTOTAL				**$16,436**
	Contingency @ 15%				$2,465
	TOTAL				**$18,901**
	PRIVATE DRAINAGE IMPROVEMENTS (NON-REIMBURSABLE)				
	15" HDPE Culvert	56	LF	$55	$3,080
	24" HDPE Culvert	25	LF	$84	$2,100
	42" HDPE Culvert	216	LF	$134	$28,944
	15" FES	1	EA	$330	$330
	24" FES	2	EA	$504	$1,008
	42" FES	2	EA	$804	$1,608
	Riprap (d_{50} = 12")	15	CY	$98	$1,470
	Channel Lining (ECB)	1800	SY	$3	$5,400
	Rain Garden C2 Outlet Structure	1	LS	$8,000	$8,000
	Rain Garden Spillway	1	LS	$6,000	$6,000
	SUBTOTAL				**$57,940**
	Contingency @ 15%				$8,691
	TOTAL				**$66,631**
	TOTAL DRAINAGE IMPROVEMENTS				**$85,532**
	Note: This estimate does not include costs for street improvements (curb & gutter, crosspans, etc.)				

The cost estimate submitted herein is based on time-honored practices within the construction industry. As such the engineer does not control the cost of labor, materials, equipment or a contractor's method of determining prices and competitive bidding practices or market conditions. The estimate represents our best judgement as design professionals using current information available at the time of the preparation. The engineer cannot guarantee that proposals, bids and/or construction costs will not vary from this cost estimate.

APPENDIX F

FIGURES



VICINITY MAP

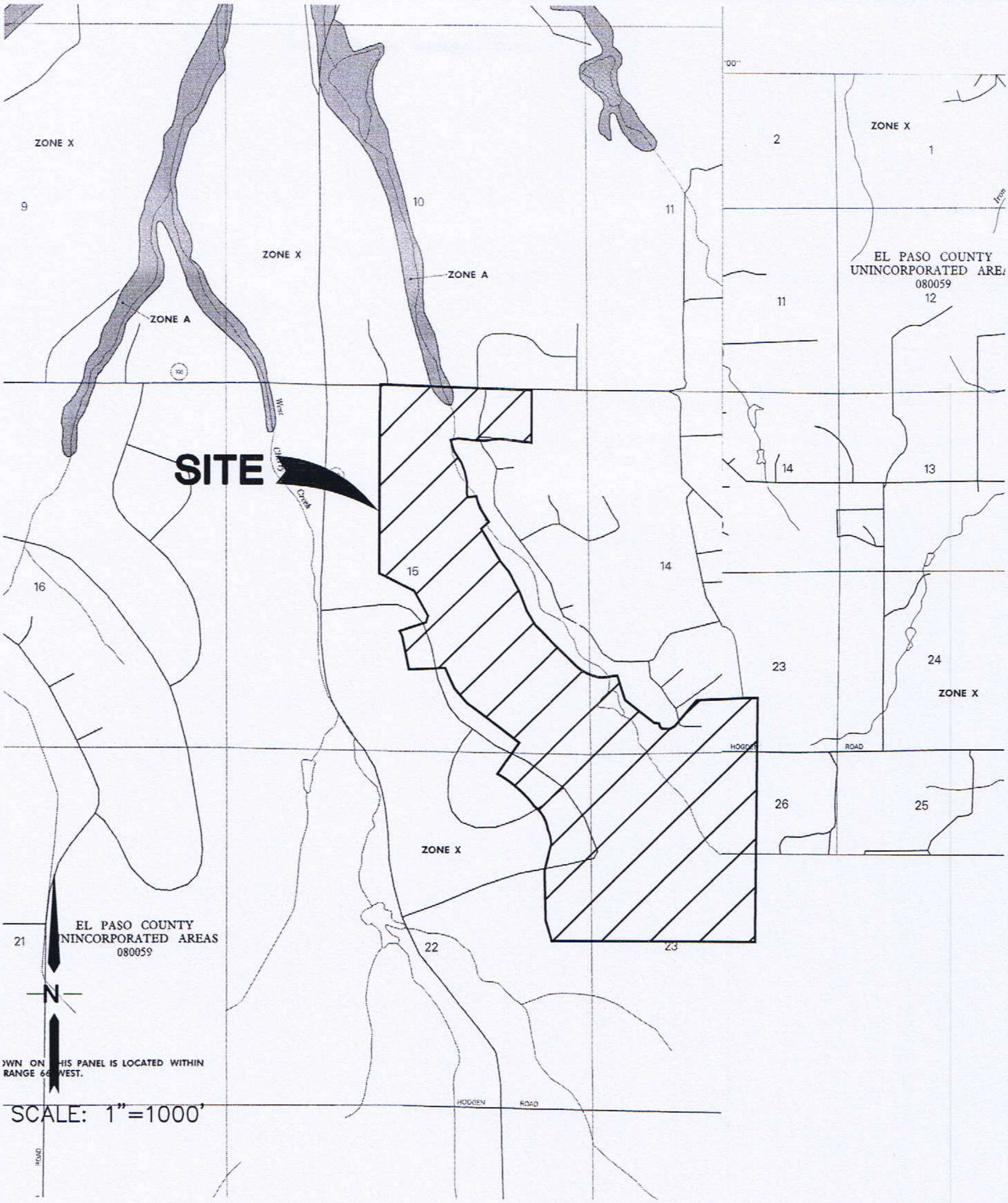
JPS ENGINEERING

WALDEN PRESERVE

FIGURE A1

JPS PROJ NO. 040201

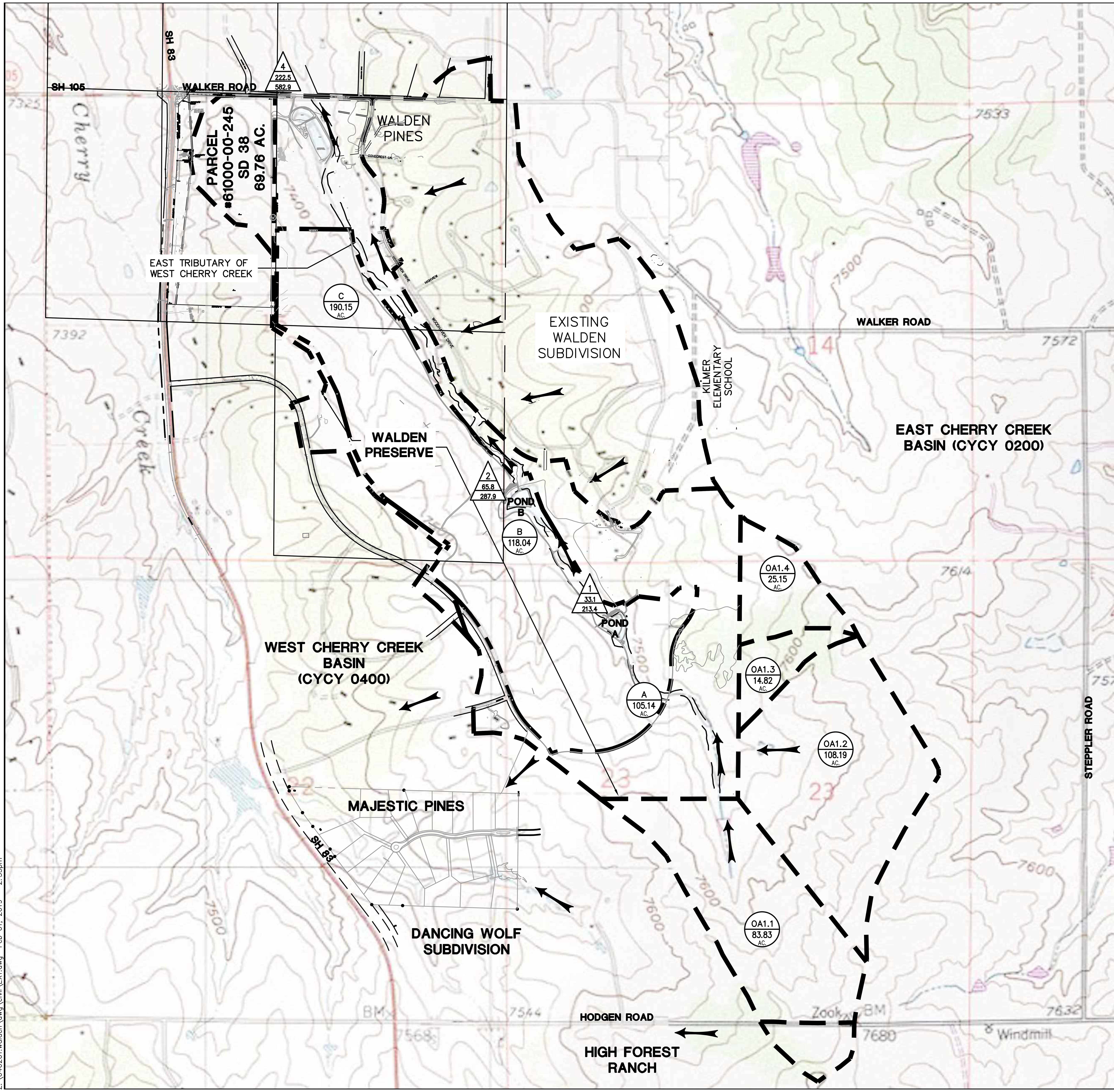
J:\jpsprojects\040201.walden\WALDEN PRESERVE 2\DWG\CIVIL\A1.dwg, 8/14/2014 2:51:45 PM



FIRM PANELS 08041C0285F & 08041C0325F (MARCH 17, 1997)

FLOODPLAIN MAP	JPS ENGINEERING	WALDEN PRESERVE FIGURE FP-1 JPS PROJ NO. 040201

J:\jpsprojects\040201.walden\WALDEN PRESERVE 2\DWG\CIVIL\FP-1.dwg Sep 17, 2014 - 9:41am



LEGEND

- FILING LIMITS
- MAJOR BASIN BOUNDARY
- 6520 EXISTING CONTOUR
- FLOWLINE
- DESIGN POINT (1); Q5 (cfs) (20.6); Q100 (cfs) (50.3)
- BASIN DESIGNATION (BD); BASIN AREA (ACRES) (524 AC)

GRAPHIC SCALE

600 0 300 600 1200

ORIGINAL SCALE: 1"=600'
CONTOUR INTERVAL=20'

N

Z:\040201.walden\dwg\civil\EX1.dwg Feb 01, 2019 - 2:55pm

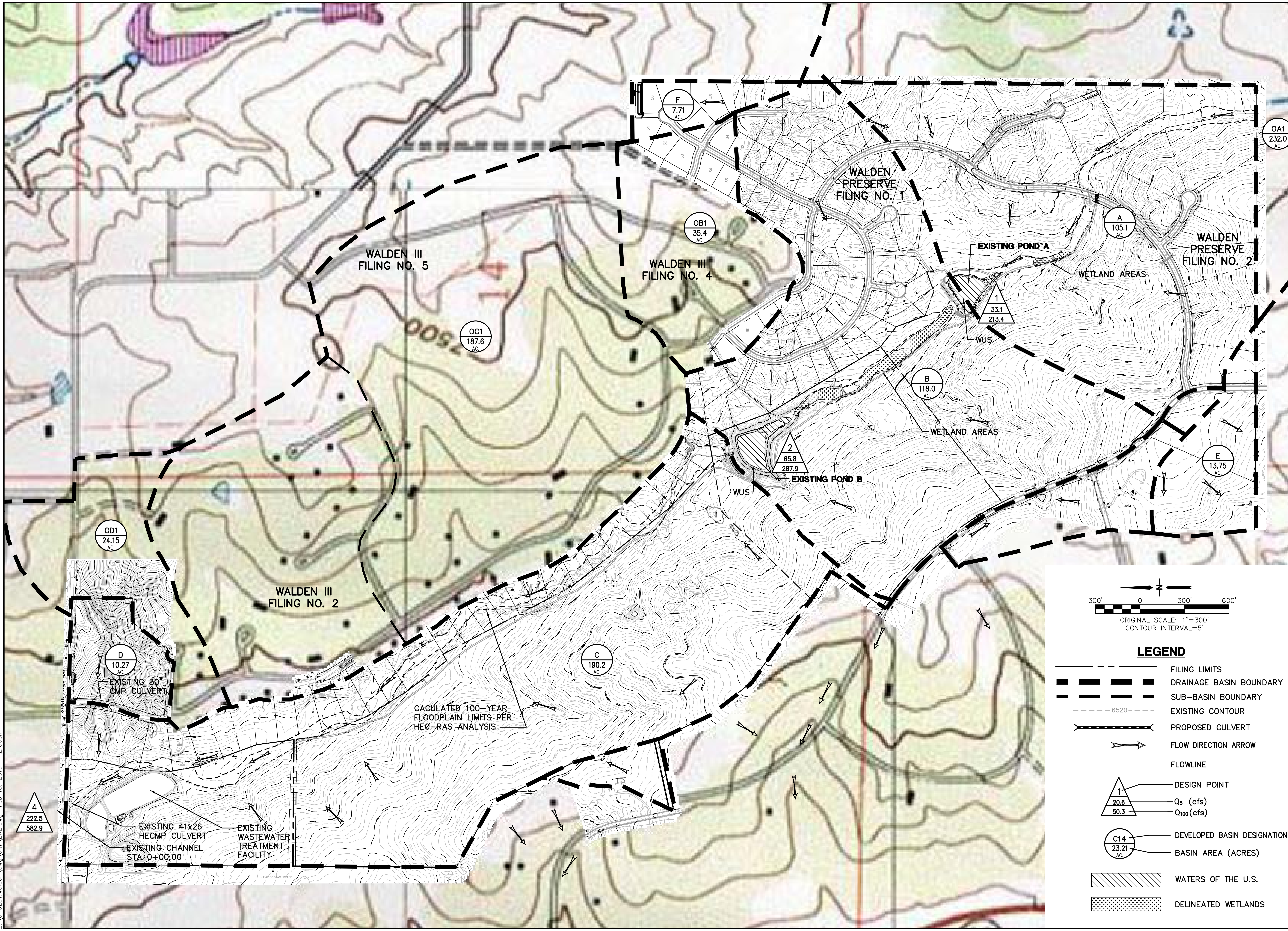
WALDEN PRESERVE SUBDIVISION

MAJOR BASIN / HISTORIC DRAINAGE PLAN

JPS ENGINEERING
19 E. Willamette Ave.
Colorado Springs, CO 80903
PH: 719-477-9429
FAX: 719-471-0766
www.jpsengr.com

No.	REVISION	BY	DATE

HORZ. SCALE:	1"=600'	DRAWN:	MJP
VERT. SCALE:	N/A	DESIGNED:	JPS
SURVEYED:	PINNACLE	CHECKED:	JPS
CREATED:	7/22/02	LAST MODIFIED:	2/01/19
PROJECT NO:	040201	MODIFIED BY:	BJJ
SHEET:	EX1		



JPS ENGINEERING
19 E. Willamette Ave.
Colorado Springs, CO 80903
PH: 719-477-9429
FAX: 719-471-0766
www.jpsengr.com

CALL UTILITY NOTIFICATION CENTER OF COLORADO
1-800-922-1987
CALL 2-BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE, OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES.

No.	REVISION	BY	DATE

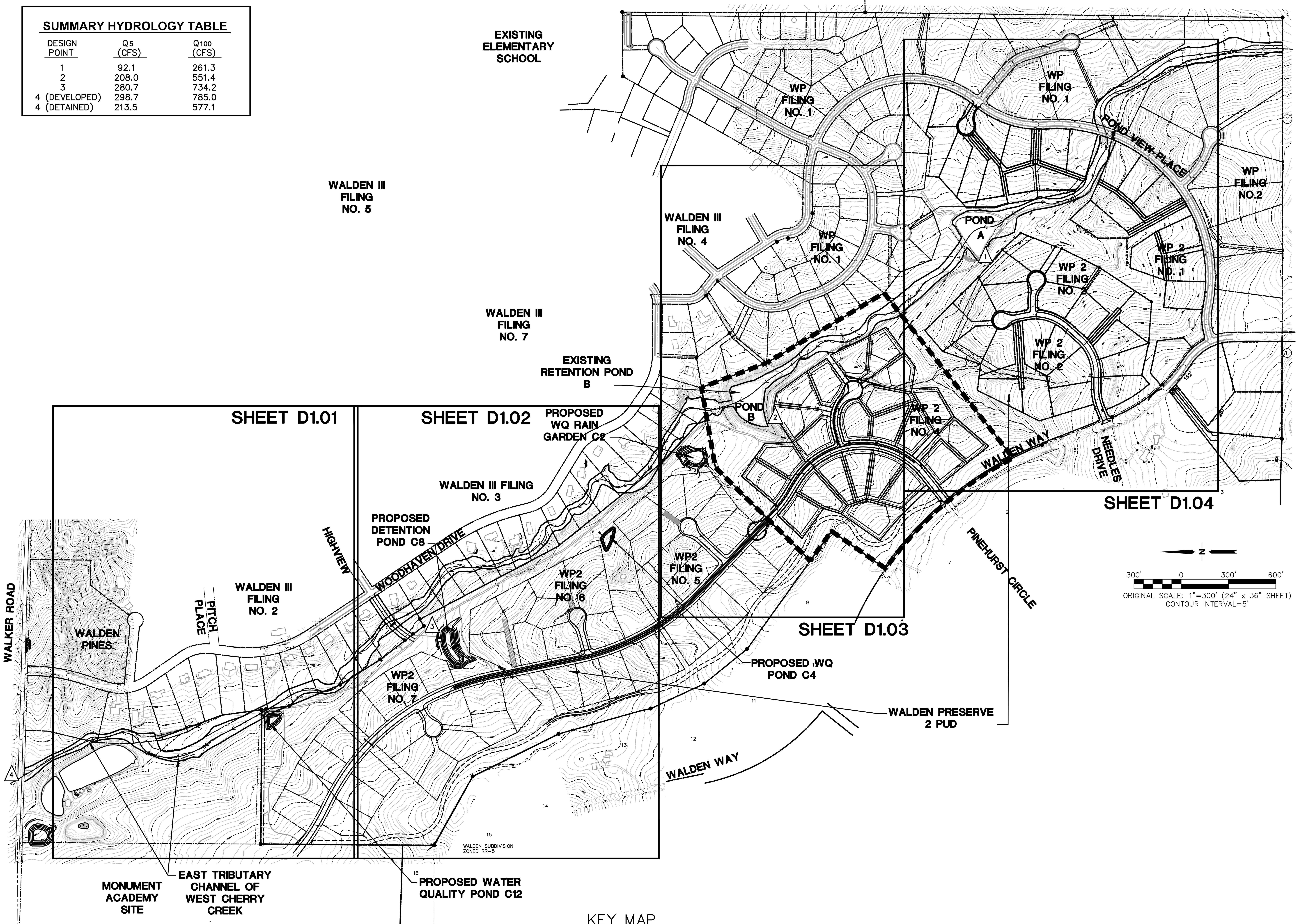
WALDEN PRESERVE

HISTORIC DRAINAGE PLAN

HORZ. SCALE:	1"=300'	DRAWN:	MJP
VERT. SCALE:	N/A	DESIGNED:	JPS
SURVEYED:	PINNACLE	CHECKED:	JPS
CREATED:	9/30/11	LAST MODIFIED:	2/15/19
PROJECT NO:	040201	MODIFIED BY:	BJJ
SHEET:		EX2	

SUMMARY HYDROLOGY TABLE

DESIGN POINT	Q_5 (CFS)	Q_{100} (CFS)
1	92.1	261.3
2	208.0	551.4
3	280.7	734.2
4 (DEVELOPED)	298.7	785.0
4 (DETAINED)	213.5	577.1



KEY MAP

Z:\040201.walden\dwg\civil\D1.DWG Jul 12, 2019 - 2:19pm

JPS ENGINEERING

19 E. Willamette Ave.
Colorado Springs, CO 80903

PH: 719-477-9429
FAX: 719-471-0766
www.jpsengr.com

CALL UTILITY NOTIFICATION CENTER OF COLORADO
1-800-922-1987
CALL 2-BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE, OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES.

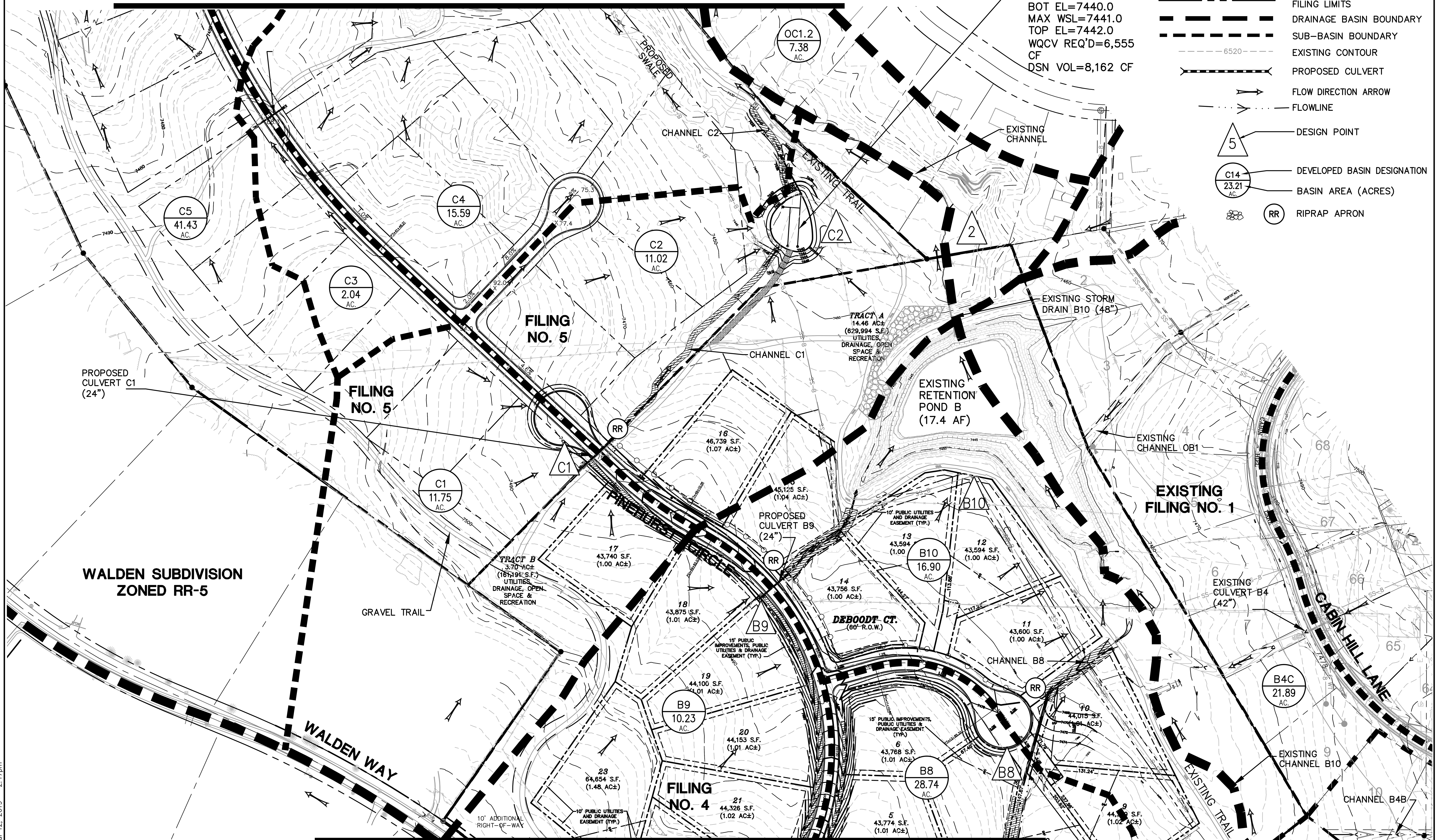
No.	REVISION	BY	DATE

WALDEN PRESERVE

MASTER DEVELOPMENT DRAINAGE PLAN

HORZ. SCALE: 1"=300'	DRAWN: MJP
VERT. SCALE: N/A	DESIGNED: JPS
SURVEYED: RAMPART	CHECKED: JPS
CREATED: 10/04/11	LAST MODIFIED: 7/11/19
PROJECT NO: 040201	MODIFIED BY: BJJ
SHEET:	D1

MATCH LINE - SEE SHEET D1.02



MATCH LINE - SEE SHEET D1.04

SUMMARY HYDROLOGY TABLE

DESIGN POINT	Q_5 (CFS)	Q_{100} (CFS)
B8	35.9	85.1
B9	10.0	23.7
B10	53.5	126.9
C1	11.2	26.7
C2	19.7	46.8

Z:\040201.walden\dwg\civil\D101-104.dwg Jul 12, 2019 - 2:17pm

JPS ENGINEERING
19 E. Willamette Ave.
Colorado Springs, CO 80903
PH: 719-477-9429
FAX: 719-471-0766
www.jpsengr.com

CALL UTILITY NOTIFICATION CENTER OF COLORADO
1-800-922-1987
CALL 2-BUSINESS DAYS IN ADVANCE BEFORE YOU DIG, GRADE, OR EXCAVATE FOR THE MARKING OF UNDERGROUND MEMBER UTILITIES.

No.	REVISION	BY	DATE

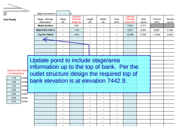
WALDEN PRESERVE

DEVELOPED DRAINAGE & EROSION CONTROL PLAN

HORZ. SCALE:	1"=100'	DRAWN:	BJJ
VERT. SCALE:	N/A	DESIGNED:	JPS
SURVEYED:	PINNACLE	CHECKED:	JPS
CREATED:	10/02/13	LAST MODIFIED:	7/11/19
PROJECT NO:	040201	MODIFIED BY:	BJJ
SHEET:	D1.03		

Markup Summary

dsdlaforce (20)



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Page Index: 134
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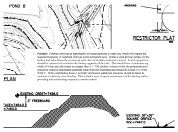
Author: dsdlaforce
Date: 8/20/2019 2:24:08 PM
Color:

Update pond to include stage/area information up to the top of bank. Per the outlet structure design the required top of bank elevation is at elevation 7442.9.



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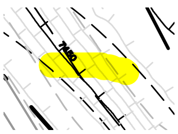

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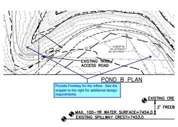

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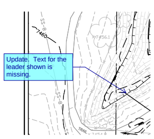
Color:

Provide Forebay for the inflow. See the snippet to the right for additional design requirements.



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Author: dsdlaforce
Date: 8/21/2019 2:52:34 PM
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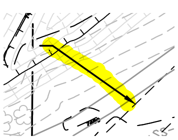
Update. Text for the leader shown is missing.



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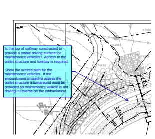
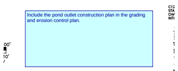


Subject: Callout
Page Index: 129
Lock: Unlocked
Author: dsdlaforce
Date: 8/21/2019 3:01:55 PM
Color:

Is the top of spillway constructed to provide a stable driving surface for maintenance vehicles? Access to the outlet structure and forebay is required.

Show the access path for the maintenance vehicles. If the embankment is used to access the outlet structure a turnaround must be provided so maintenance vehicle is not driving in reverse on the embankment.



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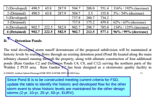
Author: dsdlaforce
Date: 8/21/2019 3:05:31 PM
Color:

Include the pond outlet construction plan in the grading and erosion control plan.



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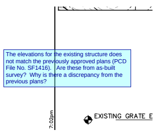
Color:

Since Pond B is to be constructed meeting current criteria for FSD, update the table to identify the historic and developed flow for the other storm event to show historic levels are maintained for the other design storms (2-yr, 10-yr, 25-yr, 50-yr, EURV).



Subject: Text Box
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Author: dsdlaforce
Date: 8/21/2019 3:57:51 PM
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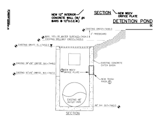
The elevations for the existing structure does not match the previously approved plans (PCD File No. SF1416). Are these from as-built survey? Why is there a discrepancy from the previous plans?



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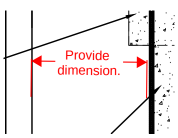

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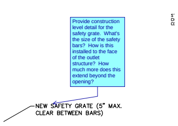
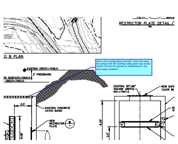
Page Index: 129
Lock: Unlocked
Author: dsdlaforce
Date: 8/21/2019 4:08:19 PM
Color:

Provide dimension.



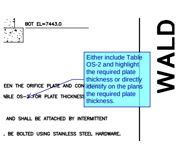
Subject: Callout
Page Index: 129
Lock: Unlocked
Author: dsdlaforce
Date: 8/21/2019 4:18:51 PM
Color:

Provide construction level detail for the safety grate. What's the size of the safety bars? How is this installed to the face of the outlet structure? How much more does this extend beyond the opening?



Subject: Callout
Page Index: 129
Lock: Unlocked
Author: dsdlaforce
Date: 8/21/2019 4:25:50 PM
Color:

How is the existing grate mounted? Does this meet the comply with the overflow safety grate note on this sheet? Can this be opened for maintenance? Are there steps installed?



Subject: Callout
Page Index: 129
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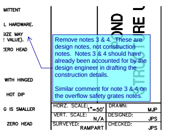
Author: dsdlaforce
Date: 8/21/2019 4:27:54 PM
Color:

Either include Table OS-2 and highlight the required plate thickness or directly identify on the plans the required plate thickness.



Subject: Callout
Page Index: 129
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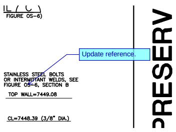
Author: dsdlaforce
Date: 8/21/2019 4:33:11 PM
Color:

Remove notes 3 & 4. These are design notes, not construction notes. Notes 3 & 4 should have already been accounted for by the design engineer in drafting the construction details.

Similar comment for note 3 & 4 on the overflow safety grates notes.



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Page Index: 129
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Author: dsdlaforce
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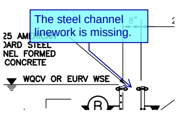
Color:

Update reference.



Subject: Callout
Page Index: 129
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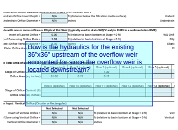
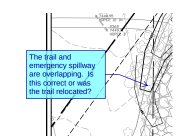
Author: dsdlaforce
Date: 8/21/2019 4:36:05 PM
Color:

The steel channel linework is missing.



Subject: Text Box
Page Index: 123
Lock: Unlocked
Author: dsdlaforce
Date: 8/21/2019 4:43:52 PM
Color:

How is the hydraulics for the existing 36"x36" upstream of the overflow weir accounted for since the overflow weir is located downstream?



Subject: Callout
Page Index: 129
Lock: Unlocked
Author: dsdlaforce
Date: 8/21/2019 4:52:47 PM
Color:

The trail and emergency spillway are overlapping. Is this correct or was the trail relocated?