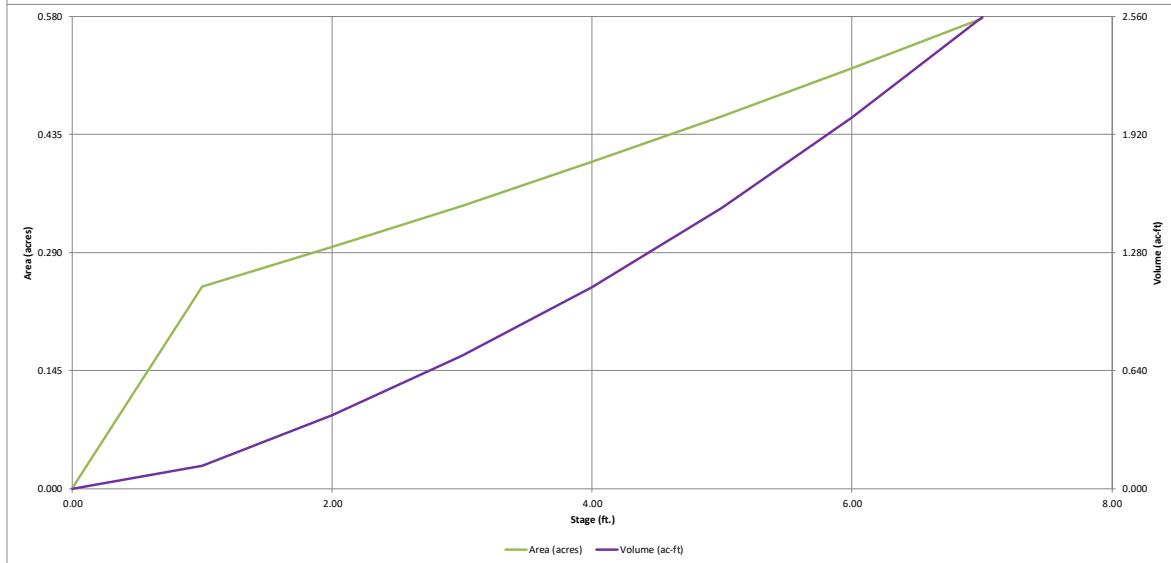
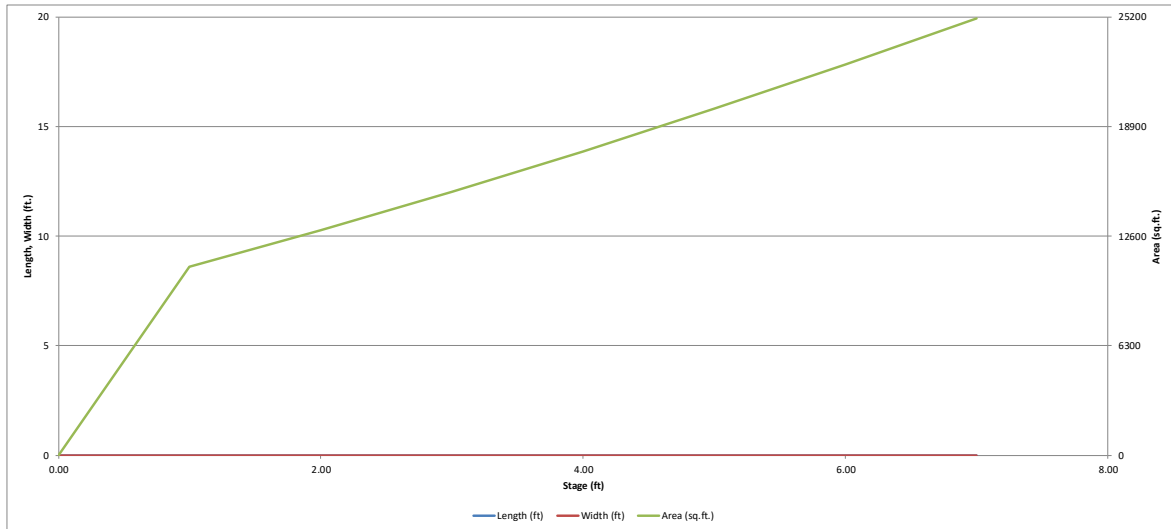


POND A: ULTIMATE DESIGN FOR REFERENCE ONLY

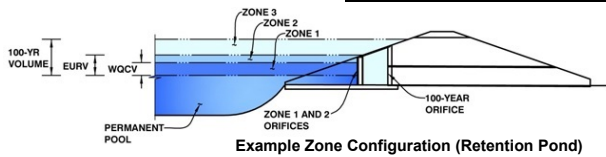


DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

POND A: ULTIMATE
DESIGN FOR
REFERENCE ONLY

Project: Falcon Field Filing 2
Basin ID: Pond A (with overdetention)



Example Zone Configuration (Retention Pond)

	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	1.86	0.354	Orifice Plate
Zone 2 (EURV)	4.36	0.884	Orifice Plate
Zone 3 (100-year)	5.85	0.694	Weir&Pipe (Restrict)
Total (all zones)		1.932	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain

Underdrain Orifice Area =	N/A	ft ²
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	4.36	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	sq. inches

Calculated Parameters for Plate

WQ Orifice Area per Row =	N/A	ft ²
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft ²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.42	2.84					
Orifice Area (sq. inches)	2.95	2.95	2.00					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orific

	Not Selected	Not Selected
Vertical Orifice Area =	N/A	N/A
Vertical Orifice Centroid =	N/A	N/A

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	4.56	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	4.30	N/A	feet
Overflow Weir Gate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	4.30	N/A	feet
Overflow Gate Type =	Type C Gate	N/A	
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir

	Zone 3 Weir	Not Selected
Height of Gate Upper Edge, H ₁ =	4.56	N/A
Overflow Weir Slope Length =	4.30	N/A
Gate Open Area / 100-yr Orifice Area =	9.42	N/A
Overflow Gate Open Area w/o Debris =	12.87	N/A
Overflow Gate Open Area w/ Debris =	6.43	N/A

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	18.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	13.00	N/A	inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate

	Zone 3 Restrictor	Not Selected
Outlet Orifice Area =	1.37	N/A
Outlet Orifice Centroid =	0.60	N/A
Half-Central Angle of Restrictor Plate on Pipe =	2.03	N/A

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	5.44	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	40.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

Spillway Design Flow Depth =	0.56	feet
Stage at Top of Freeboard =	7.00	feet
Basin Area at Top of Freeboard =	0.58	acres
Basin Volume at Top of Freeboard =	2.56	acre-ft

Routed Hydrograph Results

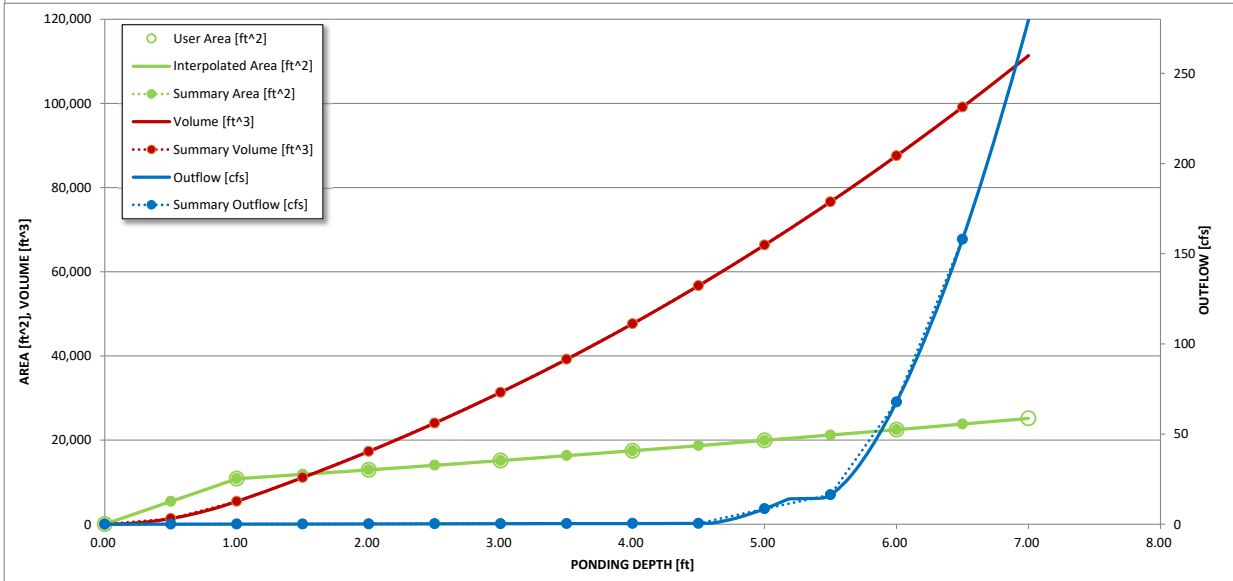
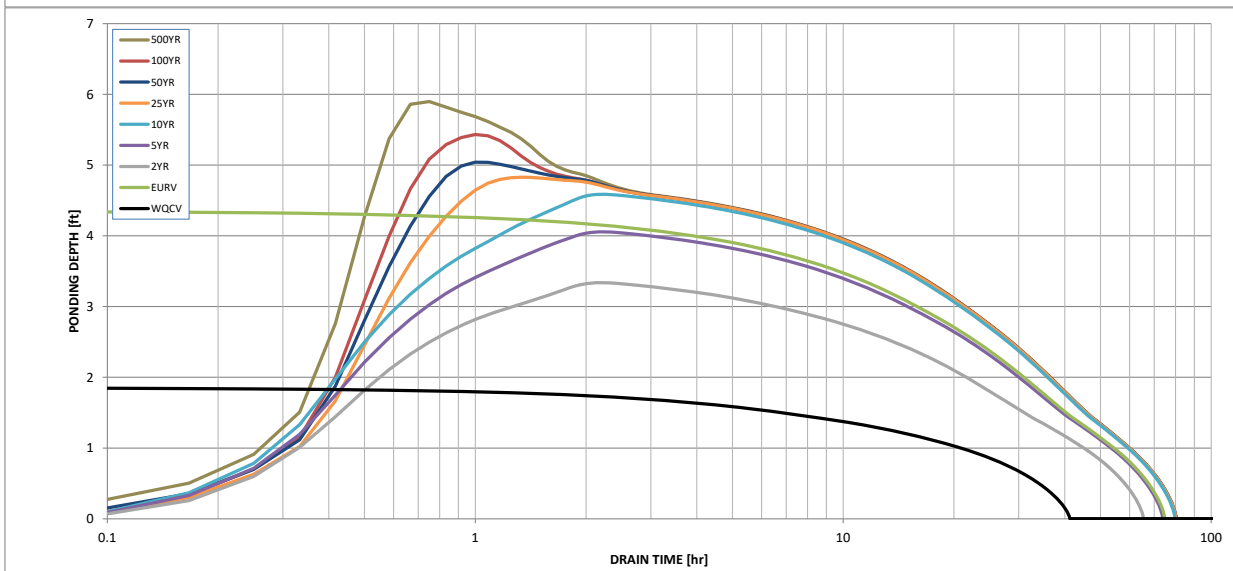
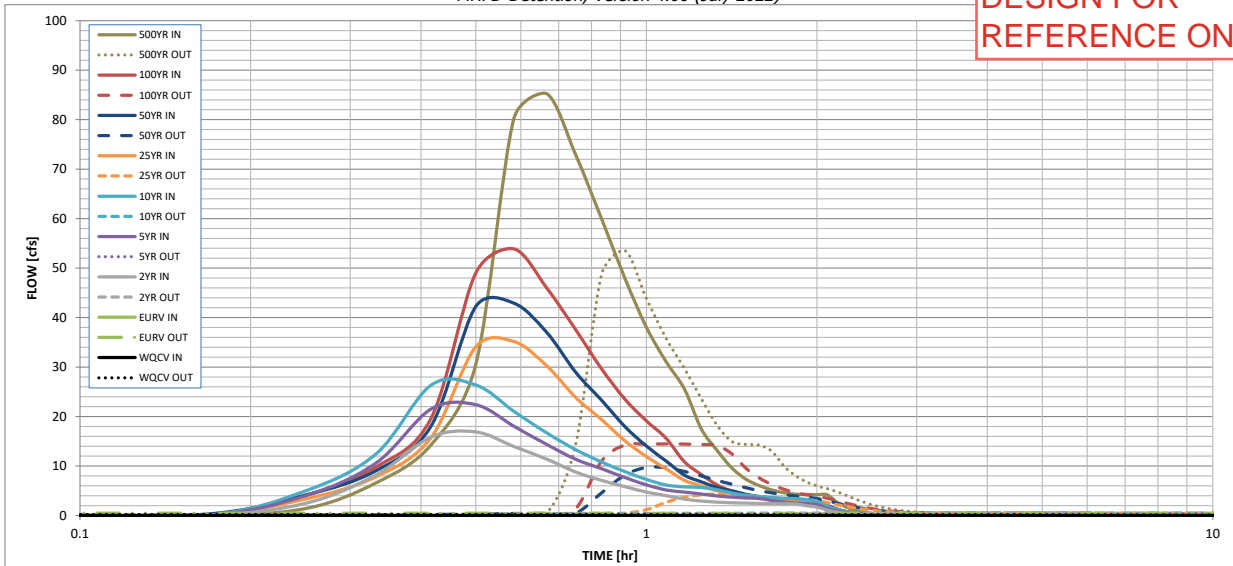
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF)

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
One-Hour Rainfall Depth (in)	0.354	1.238	0.890	1.179	1.410	1.757	2.096	2.523
CUHP Runoff Volume (acre-ft)	N/A	N/A	0.890	1.179	1.410	1.757	2.096	2.523
Inflow Hydrograph Volume (acre-ft)	N/A	N/A	0.2	0.4	0.6	5.0	9.8	16.1
CUHP Predevelopment Peak Q (cfs)	N/A	N/A	0.01	0.02	0.03	0.26	0.50	0.82
OPTIONAL Override Predevelopment Peak Q (cfs)	N/A	N/A	16.9	22.4	26.4	35.2	42.9	53.9
Predevelopment Unit Peak Flow, q (cfs/acre)	N/A	N/A	0.4	0.4	0.6	4.3	9.7	14.5
Peak Inflow Q (cfs)	N/A	N/A	1.0	1.0	1.0	0.9	1.0	0.9
Peak Outflow Q (cfs)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Ratio Peak Outflow to Predevelopment Q	Plate	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1
Structure Controlling Flow	N/A	N/A	N/A	N/A	0.0	0.3	0.7	1.1
Max Velocity through Gate 1 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Max Velocity through Gate 2 (fps)	38	67	59	66	71	70	68	66
Time to Drain 97% of Inflow Volume (hours)	40	71	63	70	76	76	75	74
Time to Drain 99% of Inflow Volume (hours)	1.86	4.36	3.34	4.05	4.59	4.83	5.04	5.43
Maximum Ponding Depth (ft)	0.29	0.42	0.37	0.40	0.43	0.45	0.46	0.48
Area at Maximum Ponding Depth (acres)	0.356	1.242	0.837	1.114	1.336	1.442	1.537	1.725
Maximum Volume Stored (acre-ft)								

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

POND A: ULTIMATE DESIGN FOR REFERENCE ONLY



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

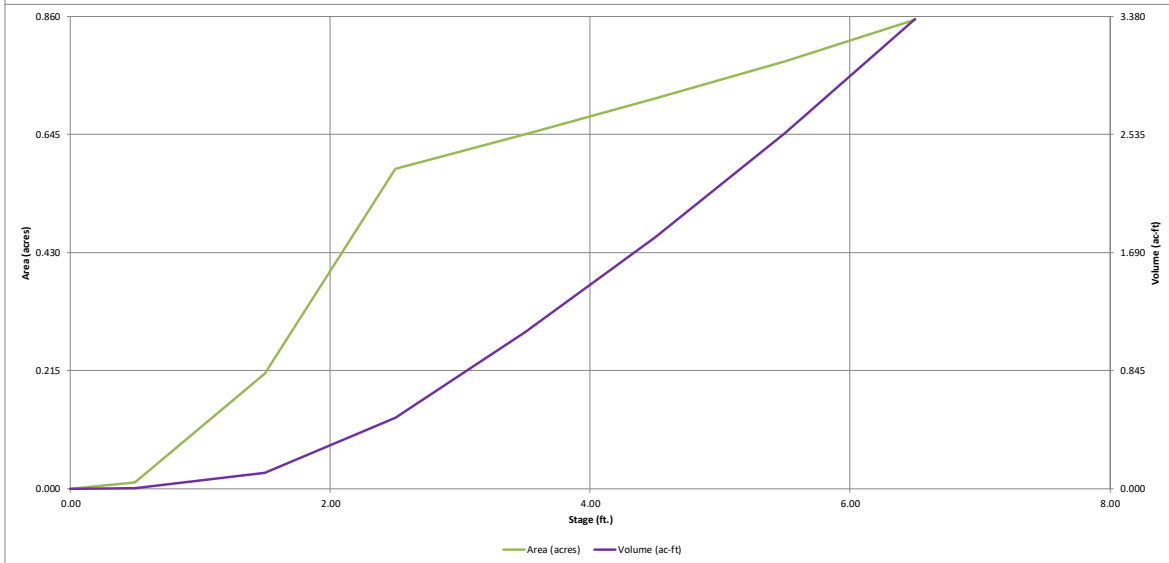
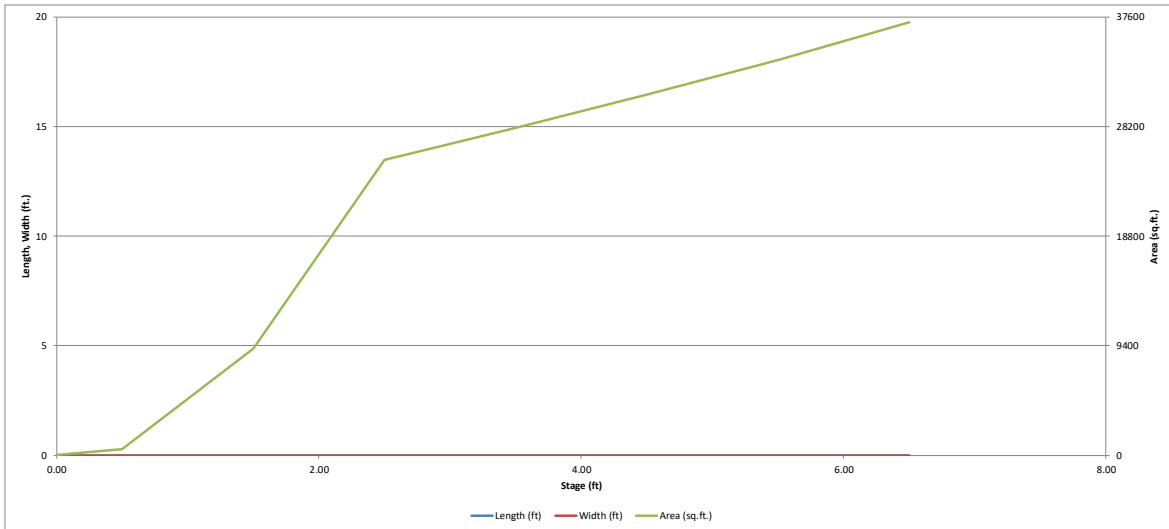
Outflow Hydrograph Workbook Filename: _____

POND A: ULTIMATE
DESIGN FOR
REFERENCE ONLY

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

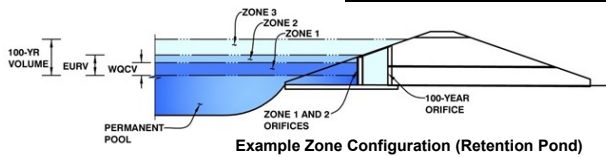
Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.28	0.03	1.40
	0:15:00	0.00	0.00	2.46	4.00	4.97	3.35	4.12	4.09	6.61
	0:20:00	0.00	0.00	8.24	10.64	12.47	7.84	9.07	9.82	14.28
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	0:35:00	0.00	0.00	13.95	18.04	21.07	35.15	42.93	53.88	85.25
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	0:45:00	0.00	0.00	8.79	11.37	13.31	23.86	28.94	37.55	59.95
	0:50:00	0.00	0.00	7.10	9.46	10.82	19.40	23.29	29.64	47.89
	0:55:00	0.00	0.00	5.82	7.66	8.88	15.07	17.95	23.48	38.04
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	1:10:00	0.00	0.00	3.30	4.74	5.79	7.11	8.16	10.87	17.51
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	1:30:00	0.00	0.00	2.50	3.55	4.10	3.79	4.27	4.12	6.03
	1:35:00	0.00	0.00	2.44	3.45	3.81	3.40	3.83	3.61	5.16
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	1:45:00	0.00	0.00	2.39	2.73	3.49	2.99	3.36	3.12	4.33
	1:50:00	0.00	0.00	2.39	2.54	3.40	2.90	3.26	3.06	4.24
	1:55:00	0.00	0.00	1.98	2.42	3.23	2.84	3.20	3.04	4.21
	2:00:00	0.00	0.00	1.70	2.25	2.89	2.82	3.17	3.04	4.21
	2:05:00	0.00	0.00	1.10	1.46	1.88	1.83	2.06	1.97	2.73
	2:10:00	0.00	0.00	0.69	0.92	1.20	1.18	1.32	1.26	1.74
	2:15:00	0.00	0.00	0.43	0.57	0.74	0.73	0.82	0.78	1.08
	2:20:00	0.00	0.00	0.24	0.34	0.44	0.44	0.49	0.47	0.64
	2:25:00	0.00	0.00	0.13	0.20	0.25	0.26	0.28	0.27	0.37
	2:30:00	0.00	0.00	0.05	0.09	0.11	0.12	0.13	0.13	0.17
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	2:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00



DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: **Falcon Field Filing 1**
Basin ID: **Pond B INTERIM**



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.10	0.305	Orifice Plate
Zone 2 (EURV)	3.49	0.809	Orifice Plate
Zone 3 (100-year)	4.37	0.589	Weir&Pipe (Restrict)
Total (all zones)		1.702	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain	
Underdrain Orifice Area =	N/A ft ²
Underdrain Orifice Centroid =	N/A feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	4.37	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	sq. inches

Calculated Parameters for Plate	
WQ Orifice Area per Row =	N/A ft ²
Elliptical Half-Width =	N/A feet
Elliptical Slot Centroid =	N/A feet
Elliptical Slot Area =	N/A ft ²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.00	2.00					
Orifice Area (sq. inches)	1.42	1.75	3.50					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orific	
Vertical Orifice Area =	N/A
Vertical Orifice Centroid =	N/A

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	4.40	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	5.00	N/A	feet
Overflow Weir Gate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	5.00	N/A	feet
Overflow Gate Type =	Type C Gate	N/A	
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir	
Height of Gate Upper Edge, H ₁ =	4.40
Overflow Weir Slope Length =	5.00
Gate Open Area / 100-yr Orifice Area =	16.26
Overflow Gate Open Area w/o Debris =	17.40
Overflow Gate Open Area w/ Debris =	8.70

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	18.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	10.50		inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate	
Outlet Orifice Area =	1.07
Outlet Orifice Centroid =	0.50
Half-Central Angle of Restrictor Plate on Pipe =	1.74

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	4.97	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	50.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway		
Spillway Design Flow Depth =	0.53	feet
Stage at Top of Freeboard =	6.50	feet
Basin Area at Top of Freeboard =	0.85	acres
Basin Volume at Top of Freeboard =	3.36	acre-ft

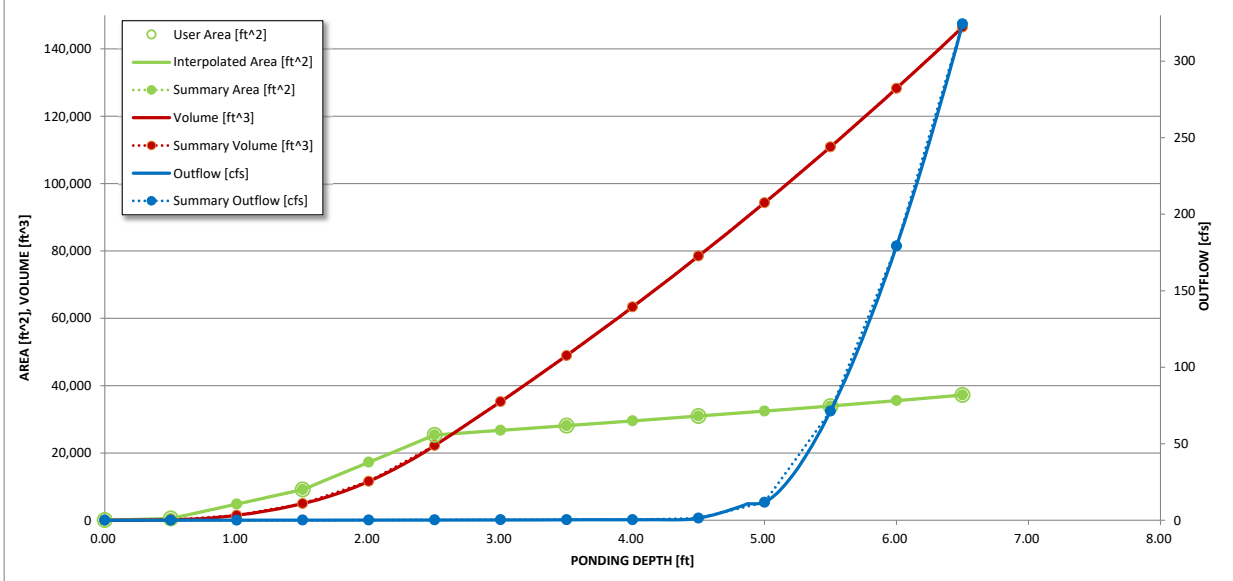
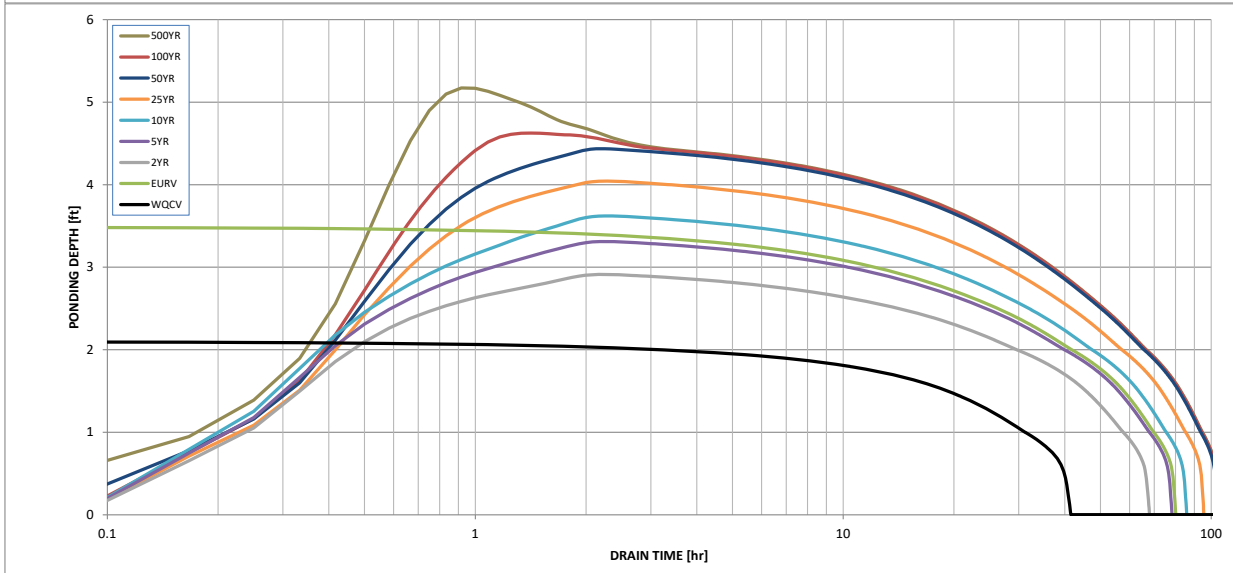
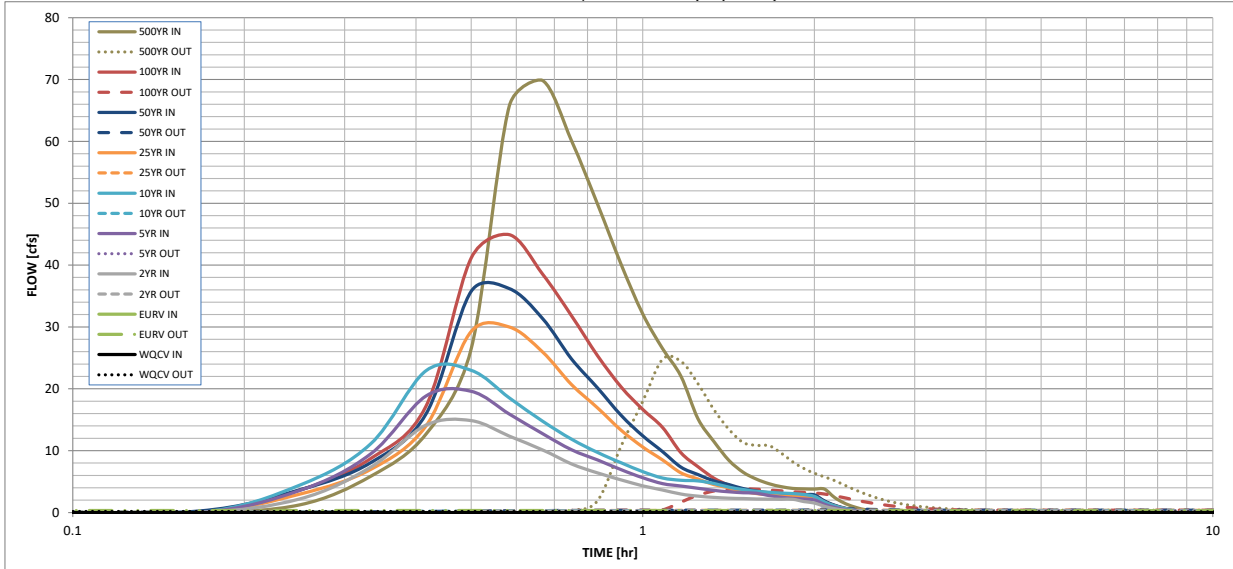
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF)

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
One-Hour Rainfall Depth (in)	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
CUHP Runoff Volume (acre-ft)	0.305	1.114	0.796	1.050	1.254	1.539	1.820	2.167
Inflow Hydrograph Volume (acre-ft)	N/A	N/A	0.796	1.050	1.254	1.539	1.820	2.167
CUHP Predevelopment Peak Q (cfs)	N/A	N/A	0.2	0.3	0.4	3.8	7.5	12.3
OPTIONAL Override Predevelopment Peak Q (cfs)	N/A	N/A						
Predevelopment Unit Peak Flow, q (cfs/acre)	N/A	N/A	0.01	0.02	0.03	0.24	0.47	0.77
Peak Inflow Q (cfs)	N/A	N/A	14.9	19.6	23.0	30.0	36.2	44.9
Peak Outflow Q (cfs)	0.2	0.3	0.3	0.3	0.3	0.4	0.6	3.8
Ratio Peak Outflow to Predevelopment Q	N/A	N/A	N/A	1.0	0.8	0.1	0.1	0.3
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1
Max Velocity through Gate 1 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	0.0	0.2
Max Velocity through Gate 2 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours)	38	70	60	69	75	83	90	89
Time to Drain 99% of Inflow Volume (hours)	40	76	65	74	81	90	98	98
Maximum Ponding Depth (ft)	2.10	3.49	2.91	3.31	3.62	4.04	4.43	4.62
Area at Maximum Ponding Depth (acres)	0.43	0.64	0.61	0.63	0.65	0.68	0.71	0.72
Maximum Volume Stored (acre-ft)	0.307	1.117	0.754	1.002	1.201	1.481	1.752	1.887

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

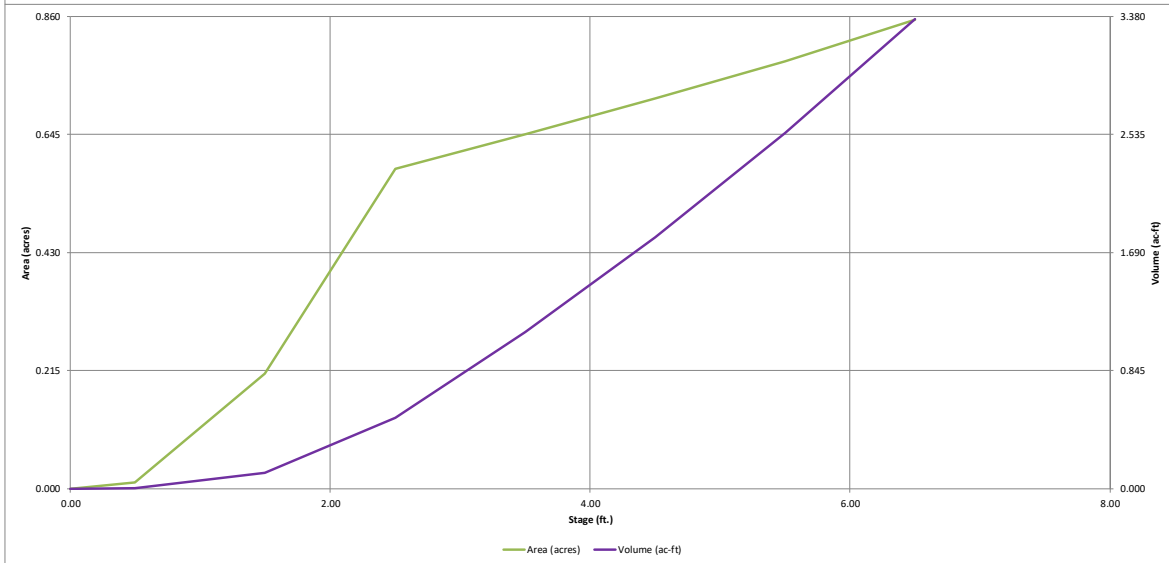
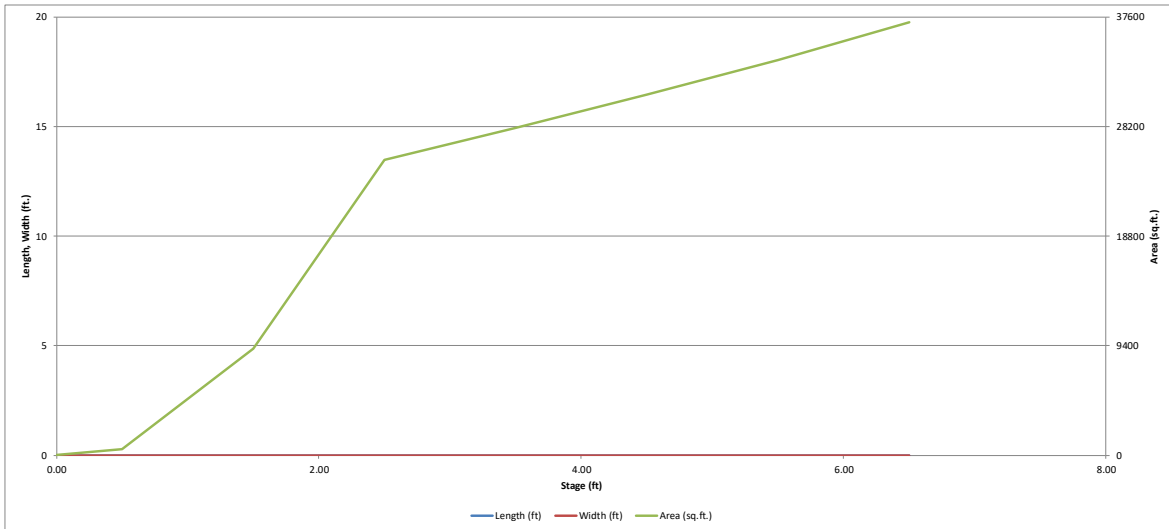
Outflow Hydrograph Workbook Filename: _____

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.25	0.02	1.24
	0:15:00	0.00	0.00	2.19	3.56	4.42	2.97	3.66	3.62	5.86
	0:20:00	0.00	0.00	7.34	9.47	11.10	6.97	8.07	8.73	12.68
	0:25:00	0.00	0.00	14.13	18.84	22.92	14.01	15.94	17.22	26.48
	0:30:00	0.00	0.00	14.85	19.59	22.99	29.26	35.75	41.14	65.72
	0:35:00	0.00	0.00	12.36	15.92	18.53	29.98	36.21	44.92	69.85
	0:40:00	0.00	0.00	10.14	12.76	14.79	25.99	31.40	38.58	60.09
	0:45:00	0.00	0.00	7.86	10.14	11.85	20.68	24.83	31.81	49.84
	0:50:00	0.00	0.00	6.39	8.49	9.71	16.92	20.13	25.34	40.06
	0:55:00	0.00	0.00	5.27	6.92	8.02	13.27	15.68	20.26	32.11
	1:00:00	0.00	0.00	4.31	5.62	6.61	10.60	12.40	16.66	26.49
	1:05:00	0.00	0.00	3.64	4.67	5.58	8.49	9.84	13.76	22.02
	1:10:00	0.00	0.00	2.98	4.28	5.22	6.41	7.33	9.66	15.18
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	1:20:00	0.00	0.00	2.44	3.57	4.69	4.52	5.11	5.56	8.42
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	1:30:00	0.00	0.00	2.25	3.20	3.70	3.43	3.86	3.73	5.40
	1:35:00	0.00	0.00	2.20	3.11	3.44	3.08	3.46	3.27	4.65
	1:40:00	0.00	0.00	2.17	2.73	3.26	2.85	3.20	2.98	4.17
	1:45:00	0.00	0.00	2.15	2.47	3.15	2.70	3.03	2.82	3.90
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	2:10:00	0.00	0.00	0.63	0.84	1.10	1.07	1.20	1.15	1.59
	2:15:00	0.00	0.00	0.40	0.52	0.68	0.68	0.76	0.73	1.00
	2:20:00	0.00	0.00	0.23	0.31	0.41	0.41	0.45	0.43	0.60
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	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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POND B: ULTIMATE DESIGN FOR REFERENCE ONLY

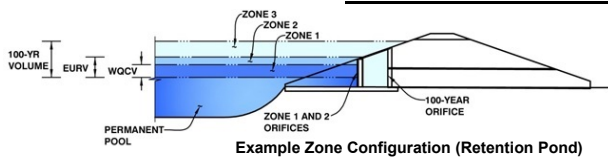


POND B: ULTIMATE DESIGN FOR REFERENCE ONLY

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: **Falcon Field Filing 1**
 Basin ID: **Pond B (with overdetection)**



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.36	0.432	Orifice Plate
Zone 2 (EURV)	4.31	1.230	Orifice Plate
Zone 3 (100-year)	5.28	0.710	Weir&Pipe (Restrict)
Total (all zones)		2.371	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth =	N/A	ft (distance below the filtration media surface)
Underdrain Orifice Diameter =	N/A	inches

Calculated Parameters for Underdrain		
Underdrain Orifice Area =	N/A	ft ²
Underdrain Orifice Centroid =	N/A	feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice =	0.00	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Orifice Plate =	4.31	ft (relative to basin bottom at Stage = 0 ft)
Orifice Plate: Orifice Vertical Spacing =	N/A	inches
Orifice Plate: Orifice Area per Row =	N/A	sq. inches

Calculated Parameters for Plate		
WQ Orifice Area per Row =	N/A	ft ²
Elliptical Half-Width =	N/A	feet
Elliptical Slot Centroid =	N/A	feet
Elliptical Slot Area =	N/A	ft ²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.00	2.00					
Orifice Area (sq. inches)	1.72	1.97	4.25					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orific		
Vertical Orifice Area =	N/A	N/A
Vertical Orifice Centroid =	N/A	N/A

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	4.40	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	5.00	N/A	feet
Overflow Weir Gate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	5.00	N/A	feet
Overflow Gate Type =	Type C Gate	N/A	
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow Weir		
Height of Gate Upper Edge, H ₁ =	4.40	N/A
Overflow Weir Slope Length =	5.00	N/A
Gate Open Area / 100-yr Orifice Area =	16.26	N/A
Overflow Gate Open Area w/o Debris =	17.40	N/A
Overflow Gate Open Area w/ Debris =	8.70	N/A

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Restrictor	Not Selected	
Depth to Invert of Outlet Pipe =	0.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Outlet Pipe Diameter =	18.00	N/A	inches
Restrictor Plate Height Above Pipe Invert =	10.50	N/A	inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Plate		
Outlet Orifice Area =	1.07	N/A
Outlet Orifice Centroid =	0.50	N/A
Half-Central Angle of Restrictor Plate on Pipe =	1.74	N/A

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	4.97	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	50.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway		
Spillway Design Flow Depth =	0.53	feet
Stage at Top of Freeboard =	6.50	feet
Basin Area at Top of Freeboard =	0.85	acres
Basin Volume at Top of Freeboard =	3.36	acre-ft

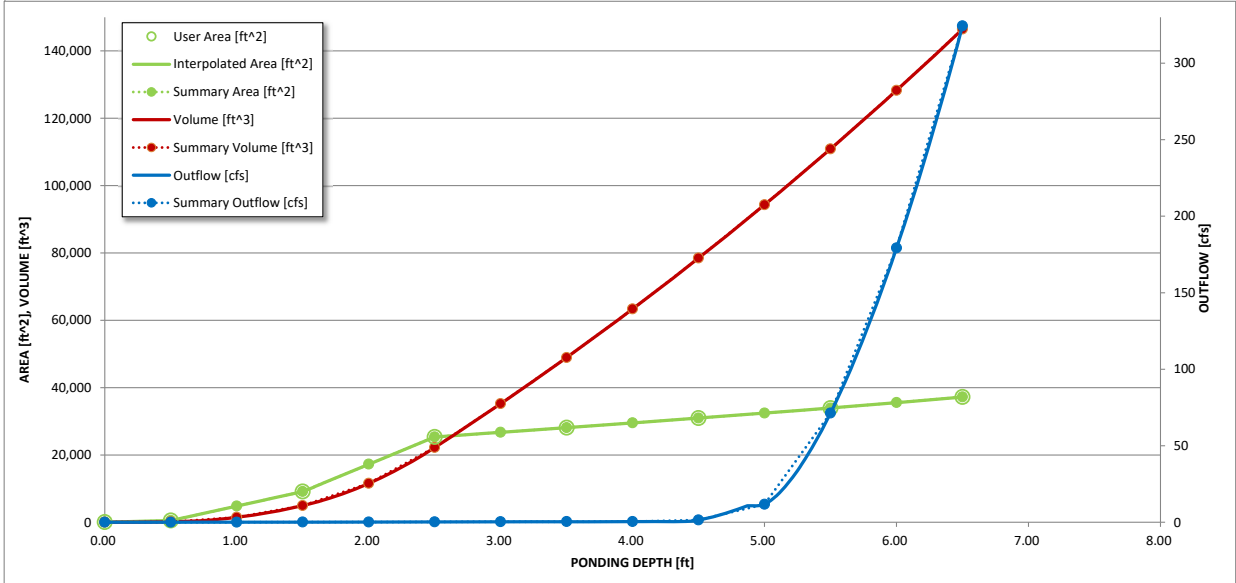
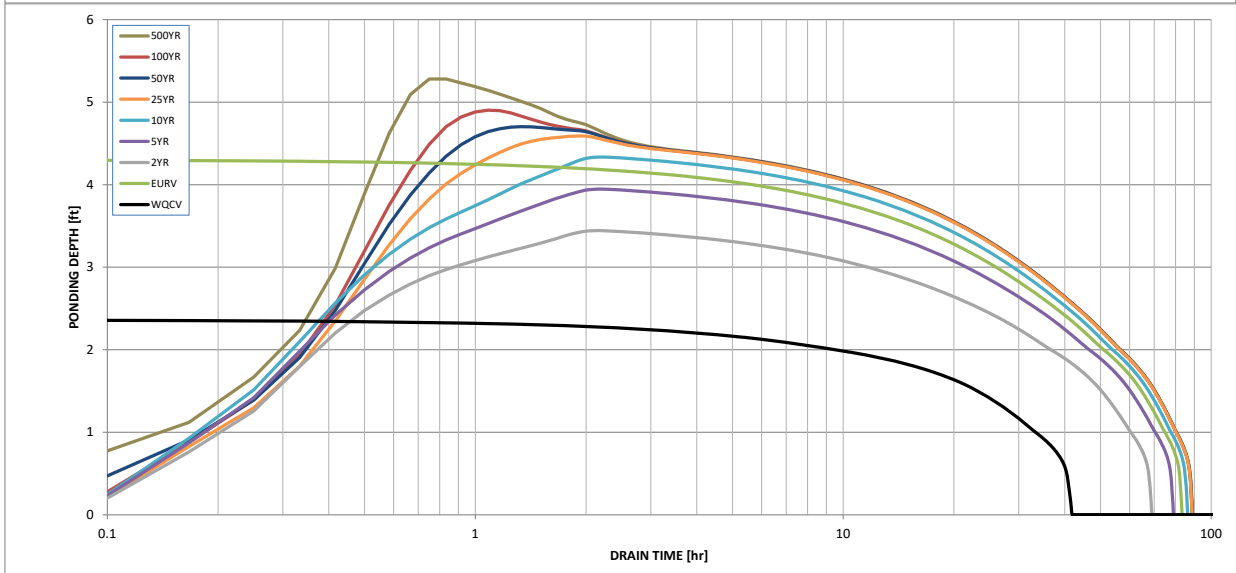
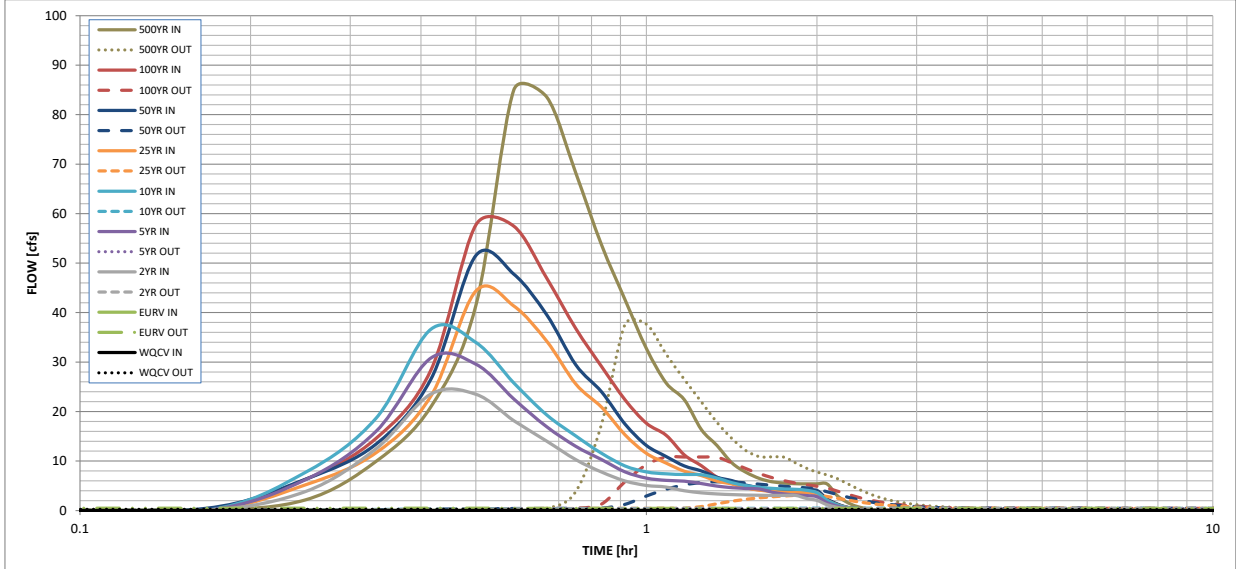
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF)

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
One-Hour Rainfall Depth (in) =	0.432	1.662	1.142	1.481	1.753	2.071	2.382	2.743
CUHP Runoff Volume (acre-ft) =	N/A	N/A	1.142	1.481	1.753	2.071	2.382	2.743
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.2	0.3	0.4	3.8	7.5	12.3
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A						
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A						
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.01	0.02	0.03	0.24	0.47	0.77
Peak Inflow Q (cfs) =	N/A	N/A	23.5	30.8	36.6	44.4	51.5	57.7
Peak Outflow Q (cfs) =	0.3	0.5	0.4	0.4	0.5	3.2	5.8	10.8
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	1.4	1.1	0.8	0.8	0.9
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Outlet Plate 1
Max Velocity through Gate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	0.2	0.3	0.6
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	38	72	60	69	75	76	75	74
Time to Drain 99% of Inflow Volume (hours) =	40	79	66	75	81	83	83	82
Maximum Ponding Depth (ft) =	2.37	4.31	3.44	3.95	4.33	4.59	4.70	4.90
Area at Maximum Ponding Depth (acres) =	0.53	0.70	0.64	0.67	0.70	0.72	0.72	0.74
Maximum Volume Stored (acre-ft) =	0.437	1.667	1.085	1.413	1.681	1.865	1.945	2.091

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.43	0.04	2.14
	0:15:00	0.00	0.00	3.84	6.23	7.71	5.17	6.32	6.28	9.94
	0:20:00	0.00	0.00	12.50	16.02	18.68	11.67	13.45	14.58	21.00
	0:25:00	0.00	0.00	23.55	30.84	36.65	23.18	26.60	28.43	41.42
	0:30:00	0.00	0.00	23.50	29.58	33.95	44.36	51.51	57.70	84.97
	0:35:00	0.00	0.00	18.17	22.54	25.80	41.34	47.78	57.46	83.57
	0:40:00	0.00	0.00	14.02	16.93	19.31	34.23	39.56	46.96	68.26
	0:45:00	0.00	0.00	10.29	12.99	15.09	25.60	29.48	36.80	53.65
	0:50:00	0.00	0.00	7.78	10.29	11.56	20.86	23.96	29.09	42.63
	0:55:00	0.00	0.00	5.97	7.80	9.01	15.30	17.48	22.39	32.73
	1:00:00	0.00	0.00	5.07	6.56	7.79	11.56	13.16	17.64	25.80
	1:05:00	0.00	0.00	4.73	6.09	7.41	9.58	10.92	15.22	22.38
	1:10:00	0.00	0.00	3.98	5.92	7.28	7.93	8.99	11.24	16.38
	1:15:00	0.00	0.00	3.58	5.44	7.22	7.07	7.99	9.06	13.10
	1:20:00	0.00	0.00	3.34	4.92	6.56	5.94	6.70	6.71	9.58
	1:25:00	0.00	0.00	3.22	4.62	5.60	5.35	6.03	5.45	7.71
	1:30:00	0.00	0.00	3.13	4.45	5.02	4.56	5.14	4.63	6.48
	1:35:00	0.00	0.00	3.08	4.35	4.68	4.11	4.63	4.19	5.82
	1:40:00	0.00	0.00	3.07	3.72	4.48	3.85	4.33	3.99	5.54
	1:45:00	0.00	0.00	3.07	3.36	4.35	3.72	4.18	3.91	5.42
	1:50:00	0.00	0.00	3.07	3.14	4.29	3.65	4.10	3.90	5.39
	1:55:00	0.00	0.00	2.43	3.03	4.10	3.61	4.07	3.90	5.39
	2:00:00	0.00	0.00	2.05	2.79	3.61	3.61	4.06	3.90	5.39
	2:05:00	0.00	0.00	1.17	1.60	2.09	2.10	2.36	2.26	3.13
	2:10:00	0.00	0.00	0.67	0.92	1.19	1.21	1.36	1.30	1.80
	2:15:00	0.00	0.00	0.34	0.49	0.63	0.65	0.73	0.70	0.96
	2:20:00	0.00	0.00	0.16	0.26	0.32	0.35	0.39	0.37	0.51
	2:25:00	0.00	0.00	0.06	0.10	0.12	0.14	0.15	0.15	0.20
	2:30:00	0.00	0.00	0.01	0.02	0.02	0.02	0.03	0.02	0.03
	2:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
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	3:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Pond B

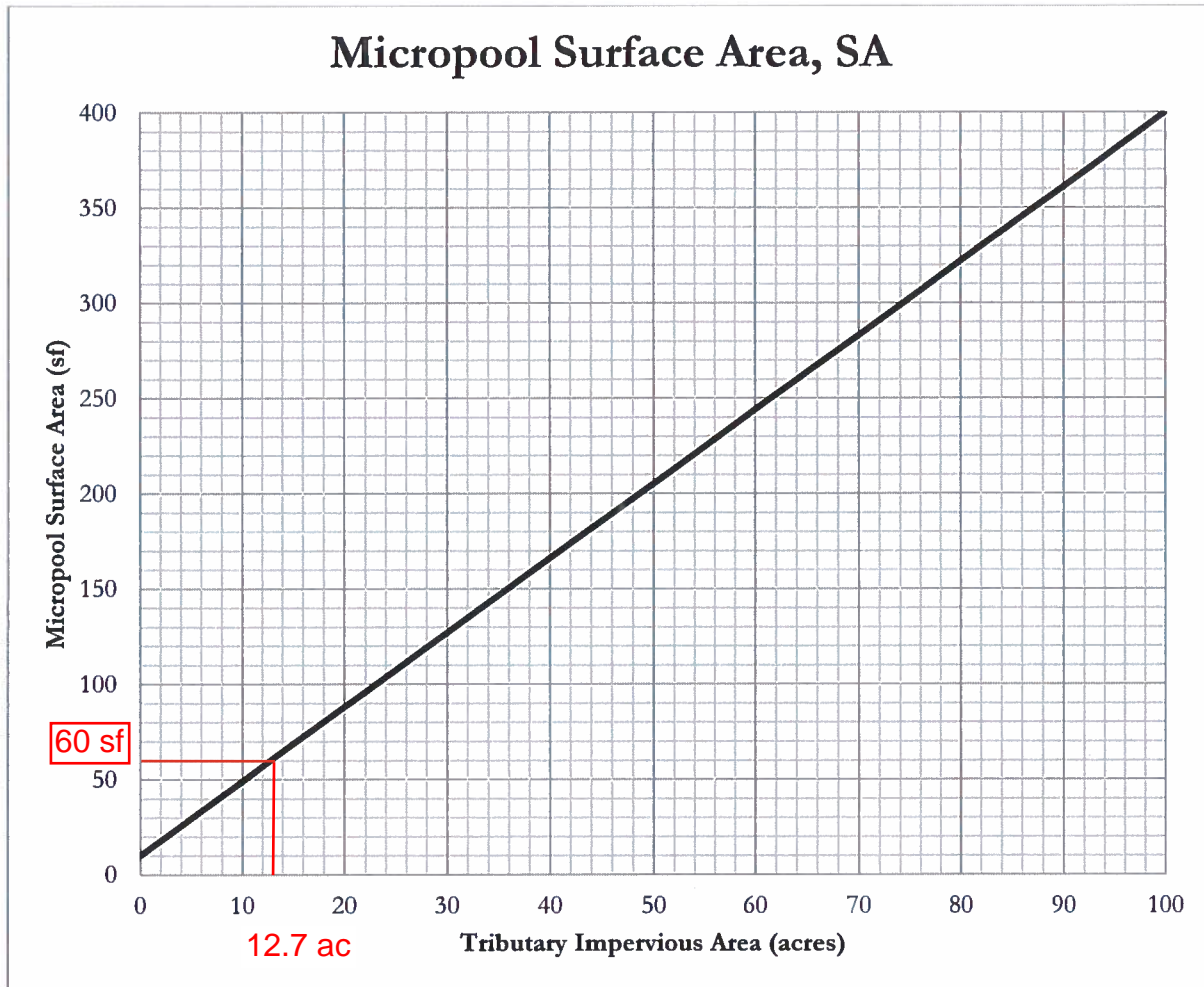


Figure 1 – Micropool surface area (SA) determination chart

The tributary impervious area is the effective number of impervious acres that will be treated by the extended detention basin (EDB). It is calculated by multiplying the tributary area to be treated by the impervious fraction of that area.

$$TIA = I \times A = (79.7/100) \times 15.9 \text{ ac} = 12.7 \text{ ac}$$

- TIA = Tributary impervious area (acres)
- I = Imperviousness (fraction)
- A = Tributary catchment area upstream (acres)

For EDBs with tributary impervious areas greater than 100 acres, the micropool surface area is 400 sf. The initial surcharge depth (ISD) is defined as the depth of the initial surcharge volume (ISV). The surface area determined using Figure 1 assumes an ISD of 4 inches. The initial surcharge volume is thus calculated by multiplying the micropool surface area by 4 inches.

$$ISV = SA \times 4 \text{ inches}$$

- ISV = Initial surcharge volume (cf)
- SA = Surface area (from Figure 1, sf)

POND B: ULTIMATE DESIGN

Figure 13-12c. Emergency Spillway Protection

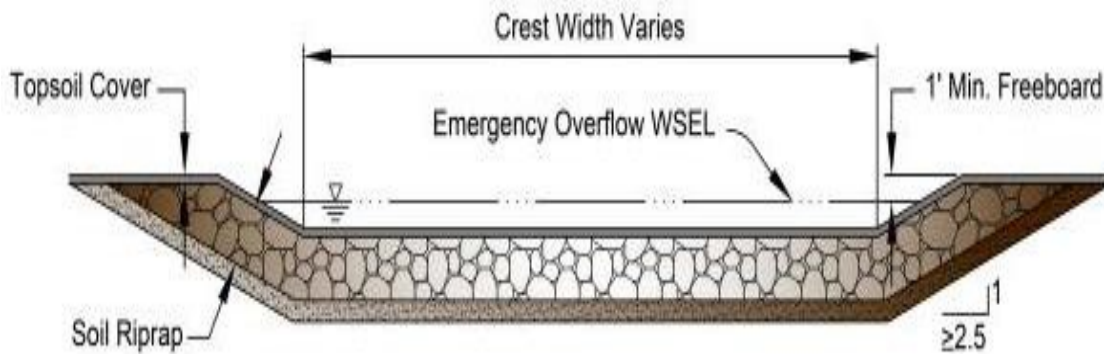
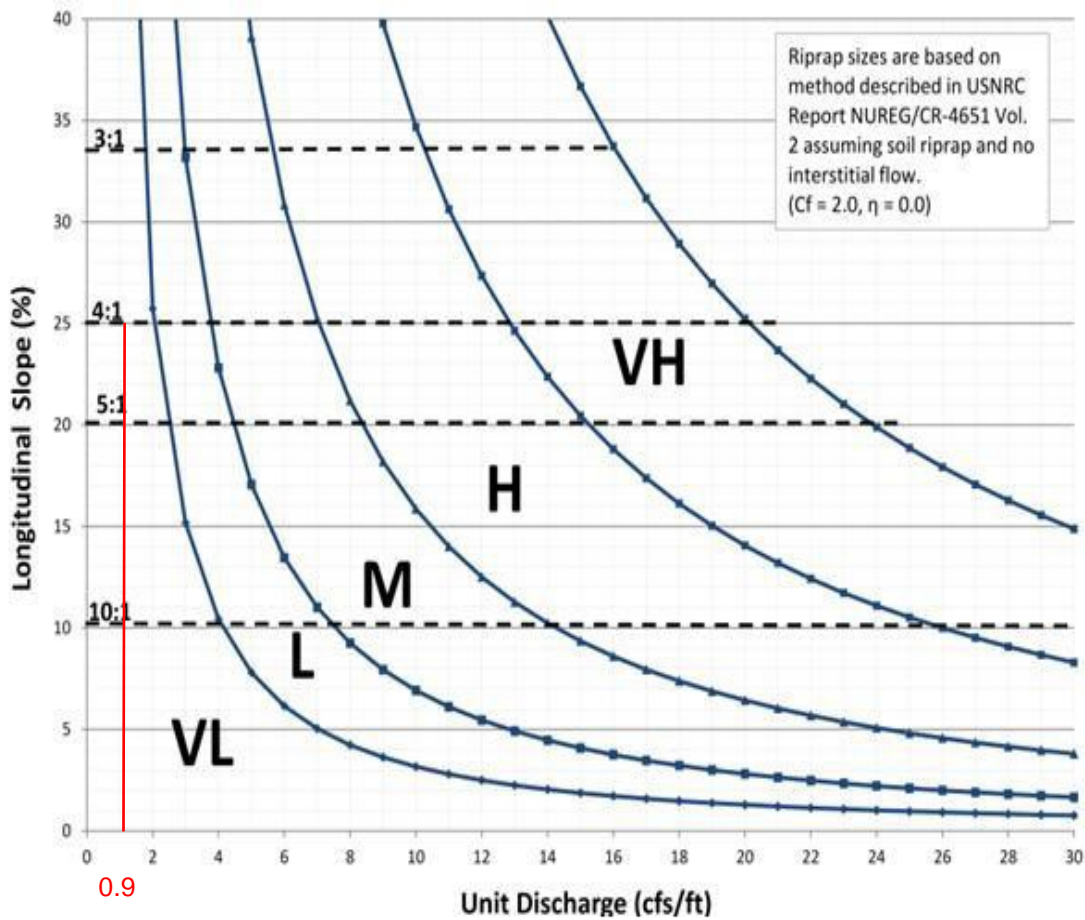
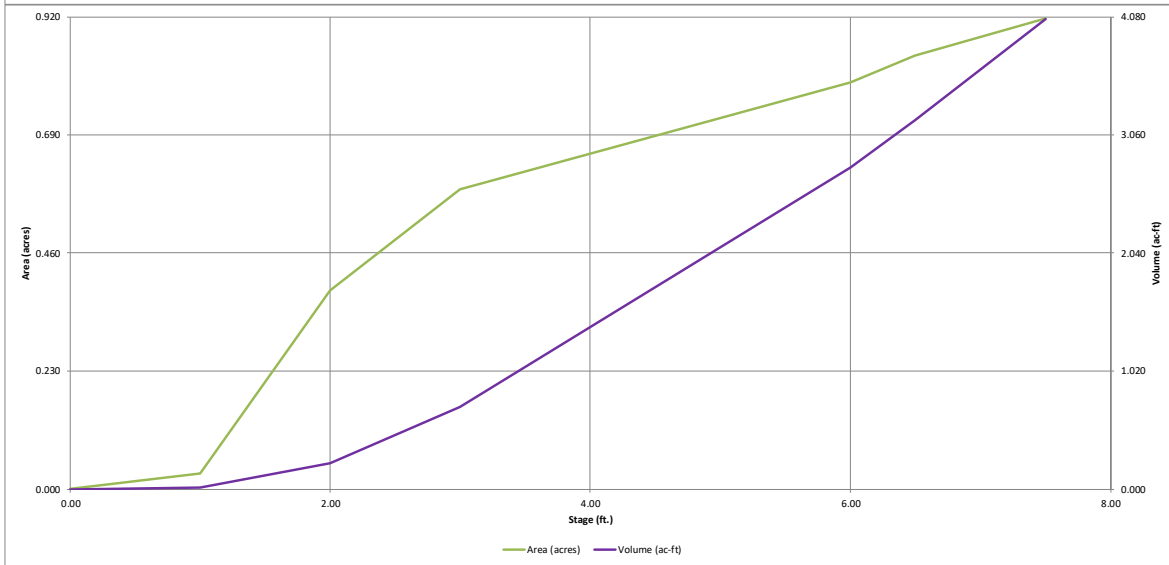
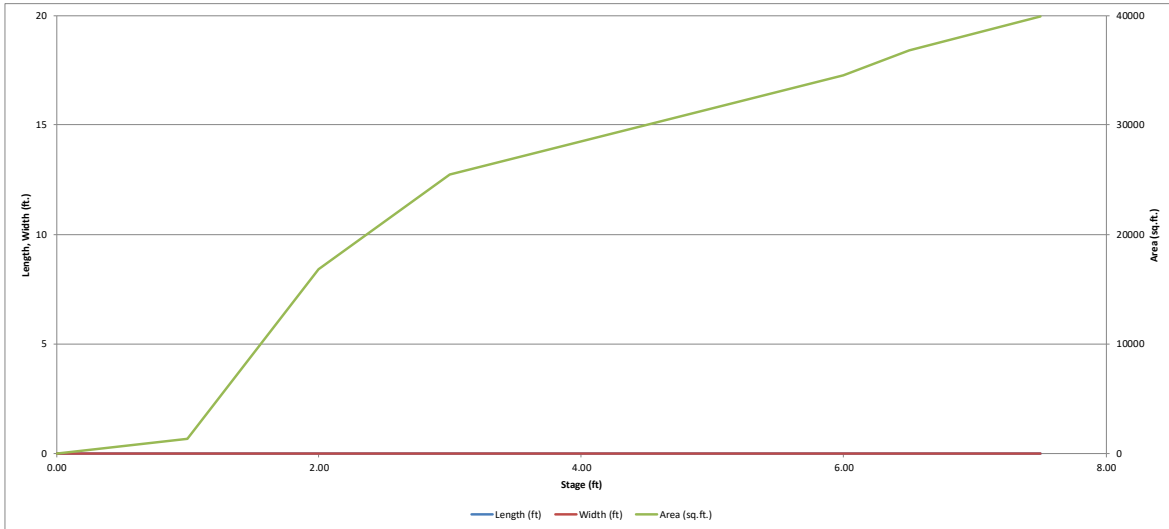


Figure 13-12d. Riprap Types for Emergency Spillway Protection



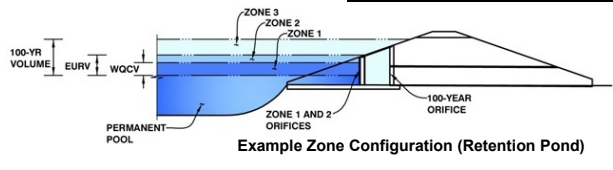
PEAK INFLOW Q100=57.7 cfs
 Spillway length=50 ft
 57.7 cfs/50 ft = 1.2 cfs/ft



DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: Falcon Field Filing 1
Basin ID: Pond C INTERIM



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.23	0.317	Orifice Plate
Zone 2 (EURV)	3.02	0.400	Orifice Plate
Zone 3 (100-year)	4.08	0.659	Weir&Pipe (Circular)
Total (all zones)		1.377	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
 Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = ft²
 Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice = ft (relative to basin bottom at Stage = 0 ft)
 Depth at top of Zone using Orifice Plate = ft (relative to basin bottom at Stage = 0 ft)
 Orifice Plate: Orifice Vertical Spacing = inches
 Orifice Plate: Orifice Area per Row = sq. inches

Calculated Parameters for Plate

WQ Orifice Area per Row = ft²
 Elliptical Half-Width = feet
 Elliptical Slot Centroid = feet
 Elliptical Slot Area = ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.25	2.50	3.50				
Orifice Area (sq. inches)	1.60	1.35	1.00	18.00				

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orific

	Not Selected	Not Selected
Vertical Orifice Area =	N/A	N/A
Vertical Orifice Centroid =	N/A	N/A

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	5.09	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	5.00	N/A	feet
Overflow Weir Gate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	5.00	N/A	feet
Overflow Gate Type =	Type C Gate	N/A	
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow We

	Zone 3 Weir	Not Selected
Height of Gate Upper Edge, H _t =	5.09	N/A
Overflow Weir Slope Length =	5.00	N/A
Gate Open Area / 100-yr Orifice Area =	3.54	N/A
Overflow Gate Open Area w/o Debris =	17.40	N/A
Overflow Gate Open Area w/ Debris =	8.70	N/A

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Circular	Not Selected	
Depth to Invert of Outlet Pipe =	0.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Circular Orifice Diameter =	30.00	N/A	inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Pl

	Zone 3 Circular	Not Selected
Outlet Orifice Area =	4.91	N/A
Outlet Orifice Centroid =	1.25	N/A
Half-Central Angle of Restrictor Plate on Pipe =	N/A	N/A

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	5.83	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	50.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

Spillway Design Flow Depth =	0.65	feet
Stage at Top of Freeboard =	7.48	feet
Basin Area at Top of Freeboard =	0.92	acres
Basin Volume at Top of Freeboard =	4.05	acre-ft

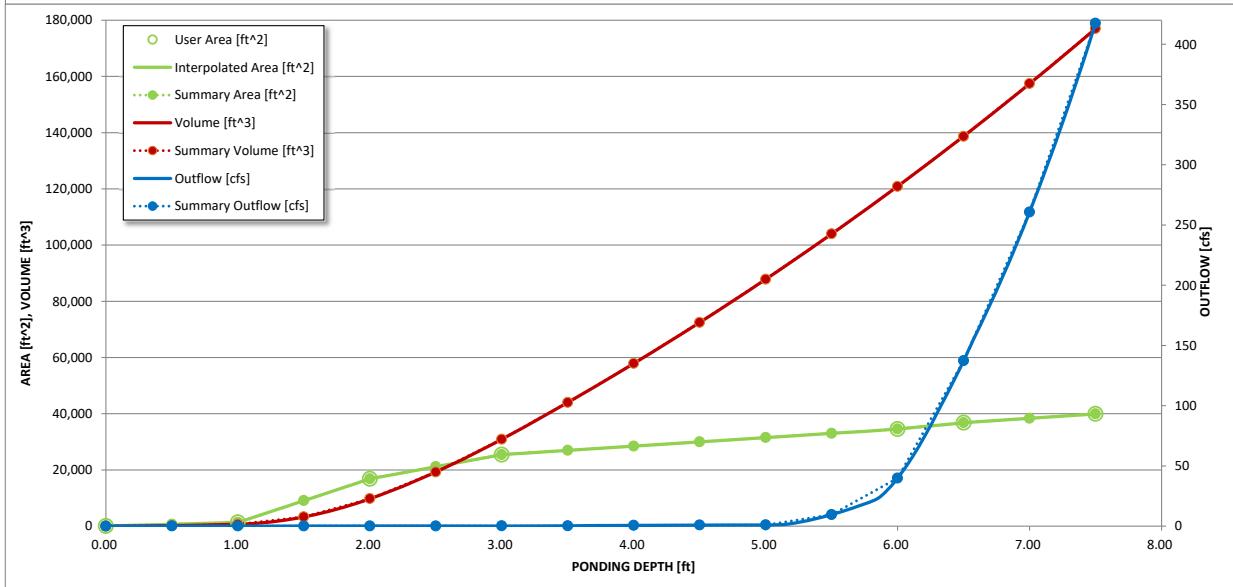
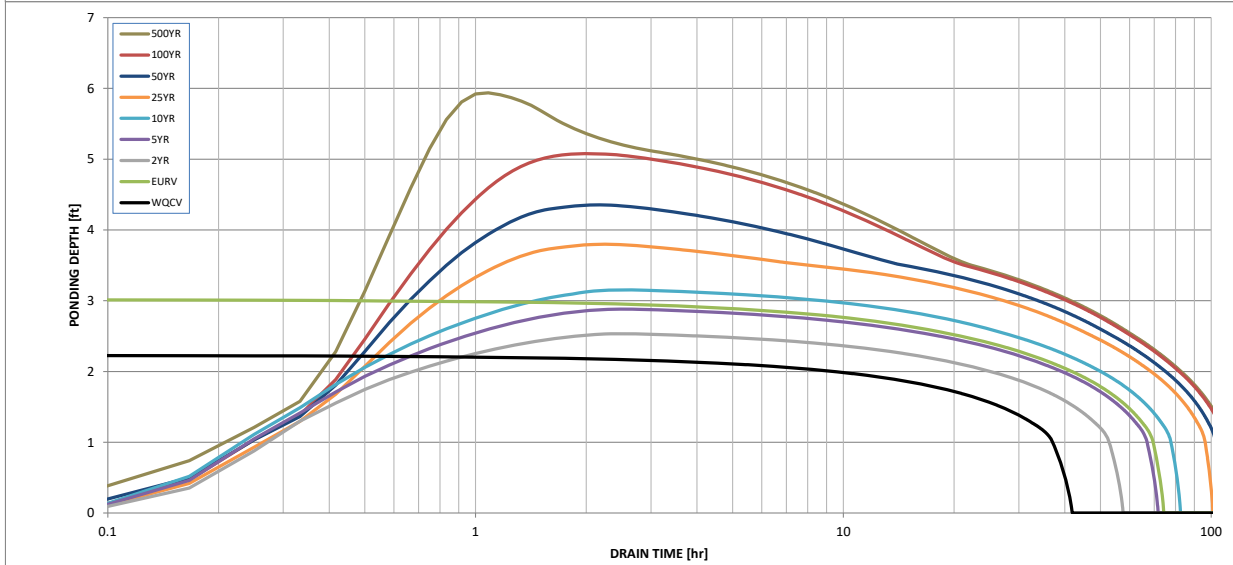
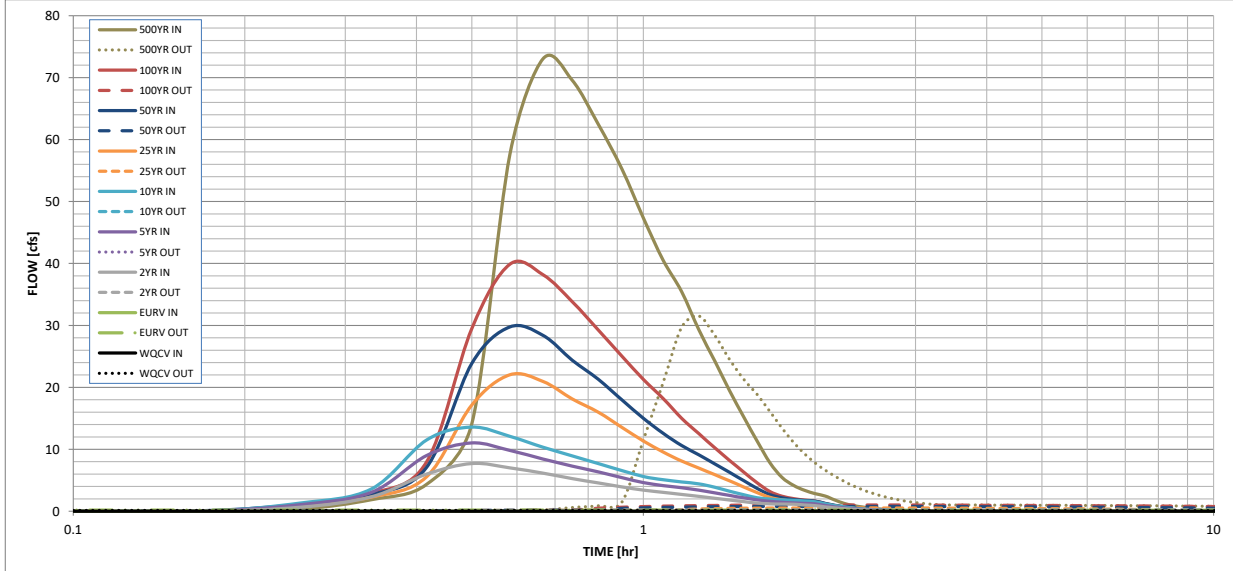
Routed Hydrograph Results

The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF)

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
One-Hour Rainfall Depth (in) =	N/A	N/A	0.484	0.671	0.832	1.262	1.663	2.202
CUHP Runoff Volume (acre-ft) =	0.317	0.717	0.484	0.671	0.832	1.262	1.663	2.202
Inflow Hydrograph Volume (acre-ft) =	N/A	N/A	0.484	0.671	0.832	1.262	1.663	2.202
CUHP Predevelopment Peak Q (cfs) =	N/A	N/A	0.3	0.6	0.8	7.2	14.0	22.8
OPTIONAL Override Predevelopment Peak Q (cfs) =	N/A	N/A						
Predevelopment Unit Peak Flow, q (cfs/acre) =	N/A	N/A	0.01	0.02	0.03	0.27	0.53	0.86
Peak Inflow Q (cfs) =	N/A	N/A	7.7	11.0	13.6	22.0	29.8	39.9
Peak Outflow Q (cfs) =	0.1	0.2	0.1	0.2	0.2	0.5	0.8	1.0
Ratio Peak Outflow to Predevelopment Q =	N/A	N/A	N/A	0.3	0.2	0.1	0.1	0.0
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Plate	Plate	Plate	Plate
Max Velocity through Gate 1 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Max Velocity through Gate 2 (fps) =	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours) =	39	68	53	66	76	92	96	101
Time to Drain 99% of Inflow Volume (hours) =	40	72	56	69	80	97	103	108
Maximum Ponding Depth (ft) =	2.23	3.02	2.53	2.88	3.15	3.80	4.35	5.08
Area at Maximum Ponding Depth (acres) =	0.43	0.59	0.49	0.56	0.59	0.64	0.68	0.73
Maximum Volume Stored (acre-ft) =	0.318	0.721	0.456	0.641	0.798	1.192	1.561	2.067

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

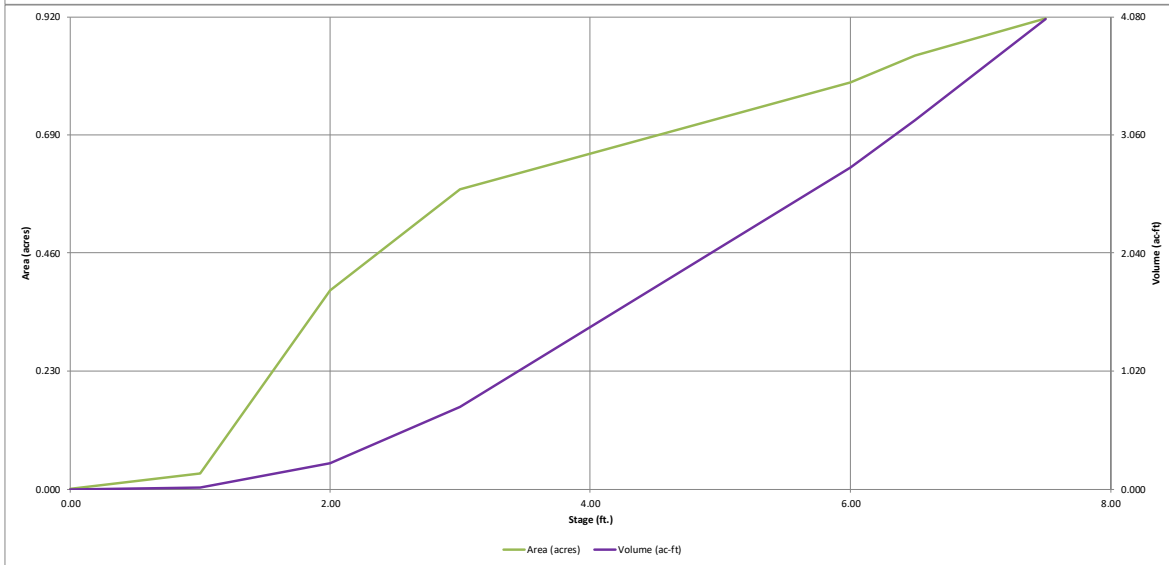
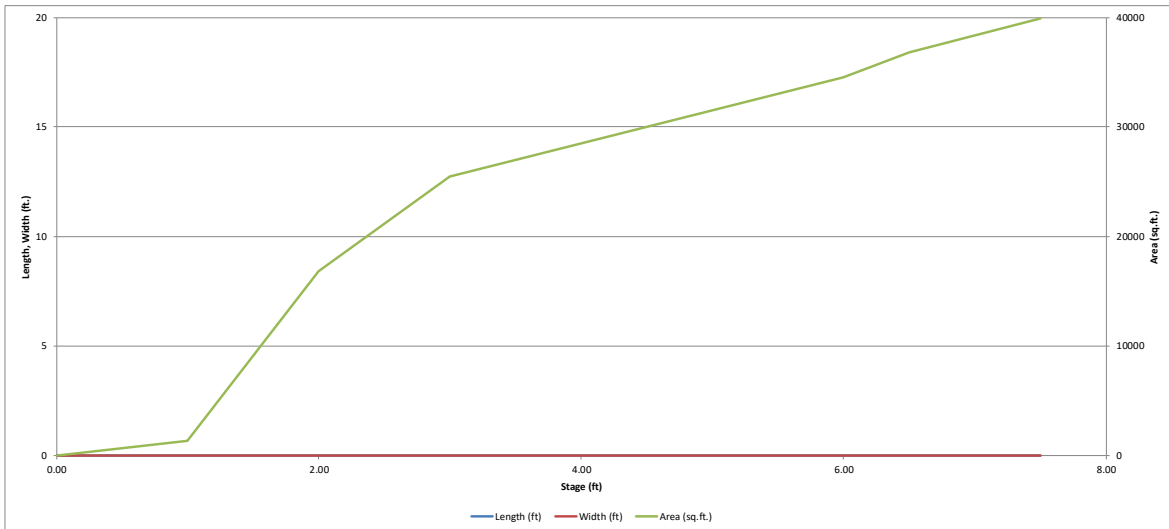
Outflow Hydrograph Workbook Filename:

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.07	0.01	0.38
	0:15:00	0.00	0.00	0.65	1.06	1.32	0.89	1.11	1.09	1.87
	0:20:00	0.00	0.00	2.33	3.05	3.60	2.28	2.66	2.85	4.34
	0:25:00	0.00	0.00	5.99	9.00	11.51	5.56	7.09	7.97	14.13
	0:30:00	0.00	0.00	7.71	11.02	13.58	17.16	23.87	29.50	57.63
	0:35:00	0.00	0.00	7.00	9.81	12.09	22.03	29.75	39.88	73.05
	0:40:00	0.00	0.00	6.14	8.43	10.34	20.95	28.39	38.17	69.57
	0:45:00	0.00	0.00	5.28	7.29	8.94	18.14	24.40	33.84	62.48
	0:50:00	0.00	0.00	4.57	6.36	7.69	16.00	21.31	29.28	55.25
	0:55:00	0.00	0.00	3.94	5.44	6.57	13.54	17.98	25.03	47.36
	1:00:00	0.00	0.00	3.40	4.63	5.61	11.35	15.00	21.30	40.51
	1:05:00	0.00	0.00	3.06	4.15	5.09	9.57	12.57	18.20	35.47
	1:10:00	0.00	0.00	2.72	3.81	4.74	8.11	10.62	15.02	29.35
	1:15:00	0.00	0.00	2.40	3.41	4.42	6.99	9.13	12.55	24.29
	1:20:00	0.00	0.00	2.10	2.98	3.88	5.91	7.65	10.26	19.45
	1:25:00	0.00	0.00	1.81	2.55	3.24	4.90	6.27	8.20	15.18
	1:30:00	0.00	0.00	1.54	2.17	2.66	3.91	4.93	6.31	11.30
	1:35:00	0.00	0.00	1.32	1.85	2.19	3.00	3.68	4.58	7.83
	1:40:00	0.00	0.00	1.21	1.60	1.96	2.24	2.68	3.18	5.47
	1:45:00	0.00	0.00	1.17	1.45	1.84	1.84	2.17	2.44	4.18
	1:50:00	0.00	0.00	1.14	1.35	1.76	1.64	1.90	2.05	3.38
	1:55:00	0.00	0.00	1.02	1.27	1.67	1.52	1.75	1.80	2.84
	2:00:00	0.00	0.00	0.91	1.18	1.53	1.43	1.64	1.64	2.47
	2:05:00	0.00	0.00	0.71	0.93	1.20	1.12	1.28	1.24	1.80
	2:10:00	0.00	0.00	0.55	0.71	0.92	0.85	0.96	0.91	1.27
	2:15:00	0.00	0.00	0.42	0.55	0.70	0.65	0.73	0.68	0.93
	2:20:00	0.00	0.00	0.32	0.42	0.53	0.49	0.55	0.51	0.69
	2:25:00	0.00	0.00	0.24	0.31	0.39	0.37	0.41	0.39	0.52
	2:30:00	0.00	0.00	0.18	0.23	0.29	0.27	0.30	0.29	0.38
	2:35:00	0.00	0.00	0.13	0.17	0.21	0.20	0.22	0.21	0.28
	2:40:00	0.00	0.00	0.10	0.12	0.16	0.15	0.17	0.16	0.20
	2:45:00	0.00	0.00	0.07	0.08	0.11	0.11	0.12	0.11	0.14
	2:50:00	0.00	0.00	0.04	0.06	0.07	0.07	0.08	0.07	0.09
	2:55:00	0.00	0.00	0.02	0.03	0.04	0.04	0.05	0.04	0.05
	3:00:00	0.00	0.00	0.01	0.02	0.02	0.02	0.02	0.02	0.02
	3:05:00	0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.01	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

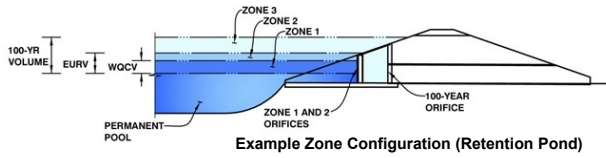
POND C: ULTIMATE DESIGN FOR REFERENCE ONLY



DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

Project: **Falcon Field Filing 3**
 Basin ID: **Pond C (with overdetection)**



	Estimated Stage (ft)	Estimated Volume (ac-ft)	Outlet Type
Zone 1 (WQCV)	2.66	0.521	Orifice Plate
Zone 2 (EURV)	4.88	1.408	Orifice Plate
Zone 3 (100-year)	6.19	0.997	Weir&Pipe (Circular)
Total (all zones)		2.926	

User Input: Orifice at Underdrain Outlet (typically used to drain WQCV in a Filtration BMP)

Underdrain Orifice Invert Depth = ft (distance below the filtration media surface)
 Underdrain Orifice Diameter = inches

Calculated Parameters for Underdrain

Underdrain Orifice Area = ft²
 Underdrain Orifice Centroid = feet

User Input: Orifice Plate with one or more orifices or Elliptical Slot Weir (typically used to drain WQCV and/or EURV in a sedimentation BMP)

Centroid of Lowest Orifice = 0.00 ft (relative to basin bottom at Stage = 0 ft)
 Depth at top of Zone using Orifice Plate = 4.88 ft (relative to basin bottom at Stage = 0 ft)
 Orifice Plate: Orifice Vertical Spacing = N/A inches
 Orifice Plate: Orifice Area per Row = N/A sq. inches

Calculated Parameters for Plate

WQ Orifice Area per Row = N/A ft²
 Elliptical Half-Width = N/A feet
 Elliptical Slot Centroid = N/A feet
 Elliptical Slot Area = N/A ft²

User Input: Stage and Total Area of Each Orifice Row (numbered from lowest to highest)

	Row 1 (required)	Row 2 (optional)	Row 3 (optional)	Row 4 (optional)	Row 5 (optional)	Row 6 (optional)	Row 7 (optional)	Row 8 (optional)
Stage of Orifice Centroid (ft)	0.00	1.25	2.50					
Orifice Area (sq. inches)	2.20	2.10	5.00					

	Row 9 (optional)	Row 10 (optional)	Row 11 (optional)	Row 12 (optional)	Row 13 (optional)	Row 14 (optional)	Row 15 (optional)	Row 16 (optional)
Stage of Orifice Centroid (ft)								
Orifice Area (sq. inches)								

User Input: Vertical Orifice (Circular or Rectangular)

	Not Selected	Not Selected	
Invert of Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Depth at top of Zone using Vertical Orifice =	N/A	N/A	ft (relative to basin bottom at Stage = 0 ft)
Vertical Orifice Diameter =	N/A	N/A	inches

Calculated Parameters for Vertical Orif

	Not Selected	Not Selected
Vertical Orifice Area =	N/A	N/A
Vertical Orifice Centroid =	N/A	N/A

User Input: Overflow Weir (Dropbox with Flat or Sloped Gate and Outlet Pipe OR Rectangular/Trapezoidal Weir and No Outlet Pipe)

	Zone 3 Weir	Not Selected	
Overflow Weir Front Edge Height, Ho =	5.09	N/A	ft (relative to basin bottom at Stage = 0 ft)
Overflow Weir Front Edge Length =	5.00	N/A	feet
Overflow Weir Gate Slope =	0.00	N/A	H:V
Horiz. Length of Weir Sides =	5.00	N/A	feet
Overflow Gate Type =	Type C Gate	N/A	
Debris Clogging % =	50%	N/A	%

Calculated Parameters for Overflow We

	Zone 3 Weir	Not Selected
Height of Gate Upper Edge, H ₁ =	5.09	N/A
Overflow Weir Slope Length =	5.00	N/A
Gate Open Area / 100-yr Orifice Area =	3.54	N/A
Overflow Gate Open Area w/o Debris =	17.40	N/A
Overflow Gate Open Area w/ Debris =	8.70	N/A

User Input: Outlet Pipe w/ Flow Restriction Plate (Circular Orifice, Restrictor Plate, or Rectangular Orifice)

	Zone 3 Circular	Not Selected	
Depth to Invert of Outlet Pipe =	0.00	N/A	ft (distance below basin bottom at Stage = 0 ft)
Circular Orifice Diameter =	30.00	N/A	inches

Calculated Parameters for Outlet Pipe w/ Flow Restriction Pl

	Zone 3 Circular	Not Selected
Outlet Orifice Area =	4.91	N/A
Outlet Orifice Centroid =	1.25	N/A
Half-Central Angle of Restrictor Plate on Pipe =	N/A	N/A

User Input: Emergency Spillway (Rectangular or Trapezoidal)

Spillway Invert Stage =	5.83	ft (relative to basin bottom at Stage = 0 ft)
Spillway Crest Length =	50.00	feet
Spillway End Slopes =	4.00	H:V
Freeboard above Max Water Surface =	1.00	feet

Calculated Parameters for Spillway

Spillway Design Flow Depth =	0.65	feet
Stage at Top of Freeboard =	7.48	feet
Basin Area at Top of Freeboard =	0.92	acres
Basin Volume at Top of Freeboard =	4.05	acre-ft

Routed Hydrograph Results

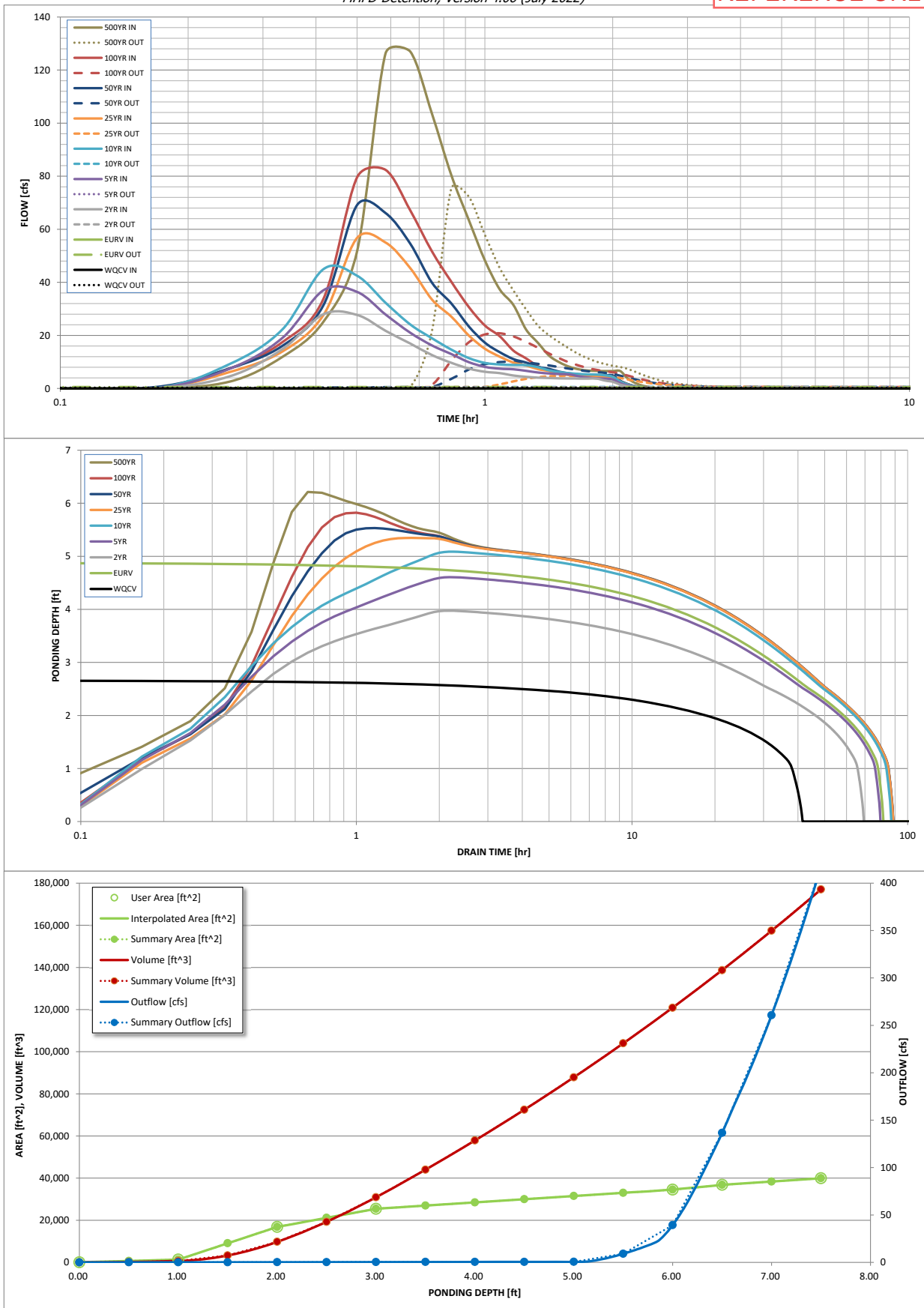
The user can override the default CUHP hydrographs and runoff volumes by entering new values in the Inflow Hydrographs table (Columns W through AF)

	WQCV	EURV	2 Year	5 Year	10 Year	25 Year	50 Year	100 Year
Design Storm Return Period =	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
One-Hour Rainfall Depth (in)	N/A	N/A	1.19	1.50	1.75	2.00	2.25	2.52
CUHP Runoff Volume (acre-ft)	0.521	1.929	1.379	1.816	2.165	2.641	3.110	3.687
Inflow Hydrograph Volume (acre-ft)	N/A	N/A	1.379	1.816	2.165	2.641	3.110	3.687
CUHP Predevelopment Peak Q (cfs)	N/A	N/A	0.3	0.6	0.8	7.2	14.0	22.8
OPTIONAL Override Predevelopment Peak Q (cfs)	N/A	N/A						
Predevelopment Unit Peak Flow, q (cfs/acre)	N/A	N/A	0.01	0.02	0.03	0.27	0.53	0.86
Peak Inflow Q (cfs)	N/A	N/A	28.0	37.0	45.0	56.7	69.1	82.4
Peak Outflow Q (cfs)	0.3	0.6	0.5	0.5	0.6	4.8	10.1	20.8
Ratio Peak Outflow to Predevelopment Q	N/A	N/A	N/A	0.9	0.7	0.7	0.7	0.9
Structure Controlling Flow =	Plate	Plate	Plate	Plate	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1	Overflow Weir 1
Max Velocity through Gate 1 (fps)	N/A	N/A	N/A	N/A	N/A	0.2	0.5	1.2
Max Velocity through Gate 2 (fps)	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A
Time to Drain 97% of Inflow Volume (hours)	38	73	62	71	78	77	76	75
Time to Drain 99% of Inflow Volume (hours)	40	78	66	76	83	83	83	82
Maximum Ponding Depth (ft)	2.66	4.88	3.97	4.61	5.09	5.35	5.53	5.82
Area at Maximum Ponding Depth (acres)	0.52	0.71	0.65	0.70	0.73	0.75	0.76	0.78
Maximum Volume Stored (acre-ft)	0.522	1.930	1.308	1.733	2.074	2.266	2.409	2.633

DETENTION BASIN OUTLET STRUCTURE DESIGN

MHFD-Detention, Version 4.06 (July 2022)

POND C: ULTIMATE DESIGN FOR REFERENCE ONLY



S-A-V-D Chart Axis Override	X-axis	Left Y-Axis	Right Y-Axis
minimum bound			
maximum bound			

DETENTION BASIN OUTLET STRUCTURE DESIGN

Outflow Hydrograph Workbook Filename: _____

POND C: ULTIMATE
DESIGN FOR
REFERENCE ONLY

Inflow Hydrographs

The user can override the calculated inflow hydrographs from this workbook with inflow hydrographs developed in a separate program.

Time Interval	SOURCE	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP	CUHP
	TIME	WQCV [cfs]	EURV [cfs]	2 Year [cfs]	5 Year [cfs]	10 Year [cfs]	25 Year [cfs]	50 Year [cfs]	100 Year [cfs]	500 Year [cfs]
5.00 min	0:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	0:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.51	0.05	2.55
	0:15:00	0.00	0.00	4.52	7.35	9.12	6.13	7.51	7.47	11.88
	0:20:00	0.00	0.00	14.82	19.02	22.24	13.93	16.07	17.43	25.17
	0:25:00	0.00	0.00	27.96	37.02	44.99	27.69	31.42	33.81	51.66
	0:30:00	0.00	0.00	27.72	36.40	42.47	56.71	69.10	79.54	126.18
	0:35:00	0.00	0.00	21.73	27.88	32.30	55.02	66.12	82.40	127.02
	0:40:00	0.00	0.00	16.93	21.02	24.16	45.39	54.60	67.15	103.65
	0:45:00	0.00	0.00	12.61	16.24	18.96	33.55	39.93	51.71	80.42
	0:50:00	0.00	0.00	9.68	12.98	14.66	27.08	32.07	40.22	63.45
	0:55:00	0.00	0.00	7.50	9.92	11.44	19.96	23.37	30.69	48.26
	1:00:00	0.00	0.00	6.28	8.15	9.64	14.98	17.27	23.73	37.38
	1:05:00	0.00	0.00	5.80	7.45	9.05	12.00	13.68	19.72	31.52
	1:10:00	0.00	0.00	4.88	7.22	8.86	9.79	11.08	14.27	22.25
	1:15:00	0.00	0.00	4.38	6.64	8.79	8.67	9.77	11.34	17.19
	1:20:00	0.00	0.00	4.09	6.02	7.99	7.29	8.20	8.36	12.35
	1:25:00	0.00	0.00	3.93	5.64	6.85	6.54	7.36	6.77	9.79
	1:30:00	0.00	0.00	3.82	5.43	6.14	5.59	6.28	5.73	8.12
	1:35:00	0.00	0.00	3.75	5.30	5.72	5.04	5.66	5.15	7.17
	1:40:00	0.00	0.00	3.73	4.55	5.47	4.70	5.29	4.87	6.75
	1:45:00	0.00	0.00	3.73	4.11	5.31	4.53	5.10	4.76	6.59
	1:50:00	0.00	0.00	3.73	3.84	5.23	4.44	4.99	4.73	6.55
	1:55:00	0.00	0.00	2.97	3.70	4.98	4.40	4.94	4.73	6.55
	2:00:00	0.00	0.00	2.52	3.41	4.40	4.38	4.93	4.73	6.55
	2:05:00	0.00	0.00	1.47	2.00	2.60	2.59	2.91	2.79	3.85
	2:10:00	0.00	0.00	0.85	1.16	1.50	1.51	1.69	1.62	2.23
	2:15:00	0.00	0.00	0.45	0.64	0.82	0.83	0.92	0.88	1.21
	2:20:00	0.00	0.00	0.22	0.35	0.43	0.46	0.51	0.49	0.66
	2:25:00	0.00	0.00	0.09	0.15	0.18	0.20	0.22	0.21	0.28
	2:30:00	0.00	0.00	0.02	0.04	0.04	0.05	0.05	0.05	0.06
	2:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	2:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	3:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	4:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:05:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:10:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:15:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:20:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:25:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:30:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:35:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:40:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:45:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:50:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	5:55:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
	6:00:00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

Pond C

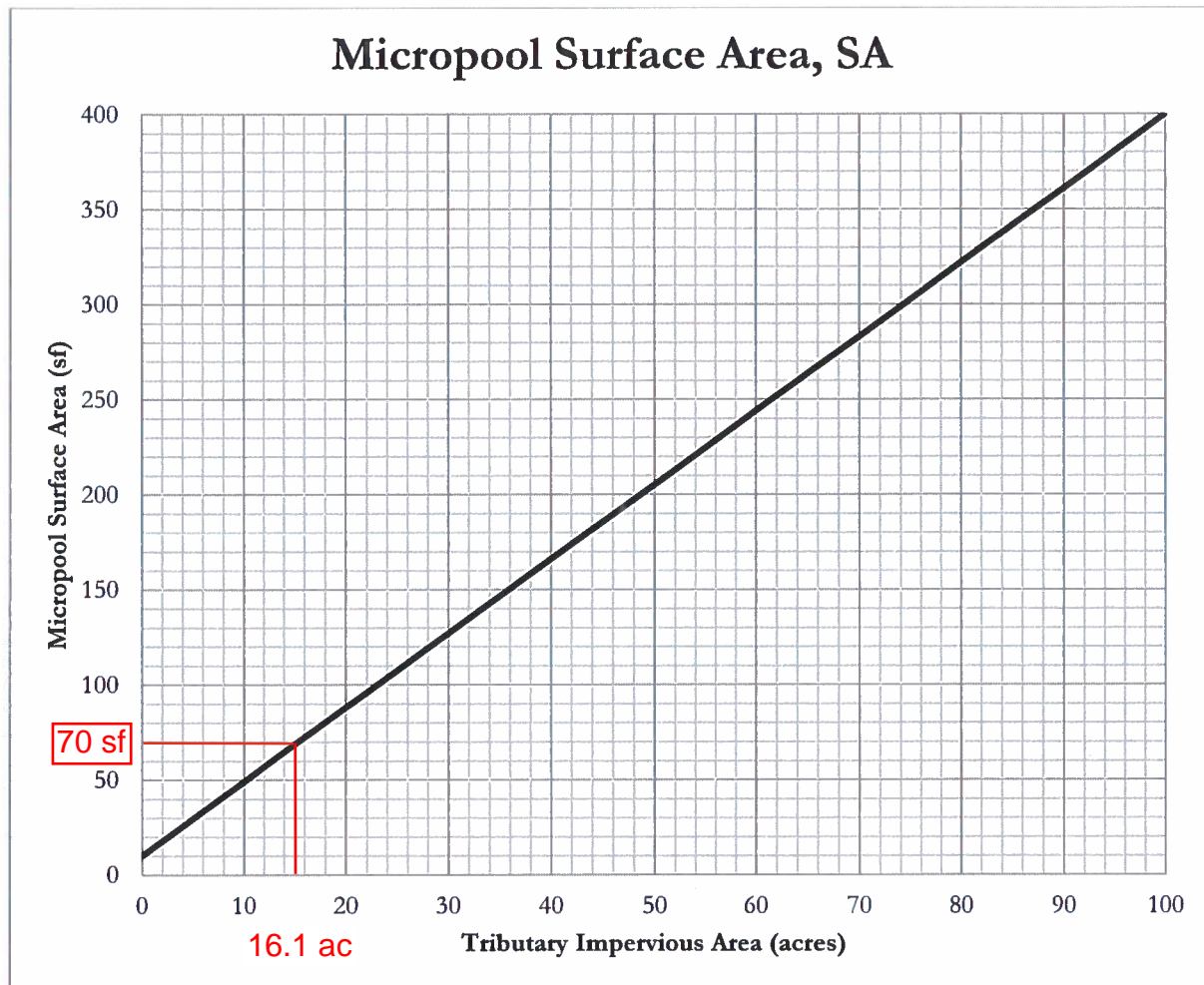


Figure 1 – Micropool surface area (SA) determination chart

The tributary impervious area is the effective number of impervious acres that will be treated by the extended detention basin (EDB). It is calculated by multiplying the tributary area to be treated by the impervious fraction of that area.

$$TIA = I \times A = (60.2/100) \times 26.4 \text{ ac} = 15.9 \text{ ac}$$

- TIA = Tributary impervious area (acres)
- I = Imperviousness (fraction)
- A = Tributary catchment area upstream (acres)

For EDBs with tributary impervious areas greater than 100 acres, the micropool surface area is 400 sf. The initial surcharge depth (ISD) is defined as the depth of the initial surcharge volume (ISV). The surface area determined using Figure 1 assumes an ISD of 4 inches. The initial surcharge volume is thus calculated by multiplying the micropool surface area by 4 inches.

$$ISV = SA \times 4 \text{ inches}$$

- ISV = Initial surcharge volume (cf)
- SA = Surface area (from Figure 1, sf)

Figure 13-12c. Emergency Spillway Protection

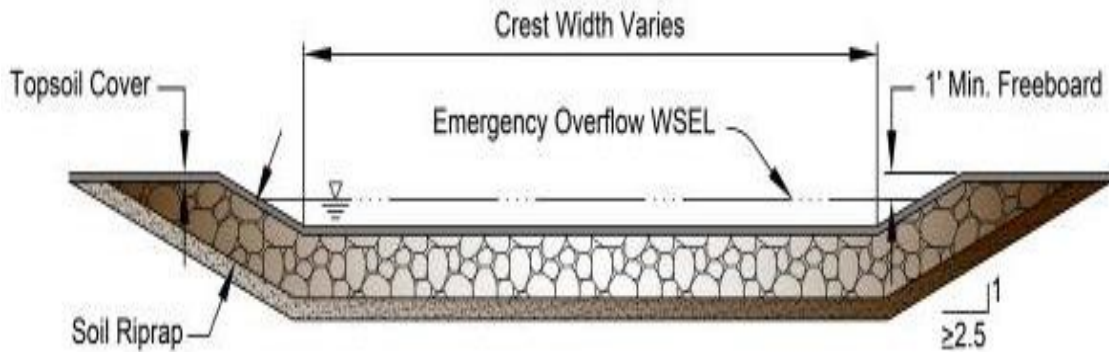
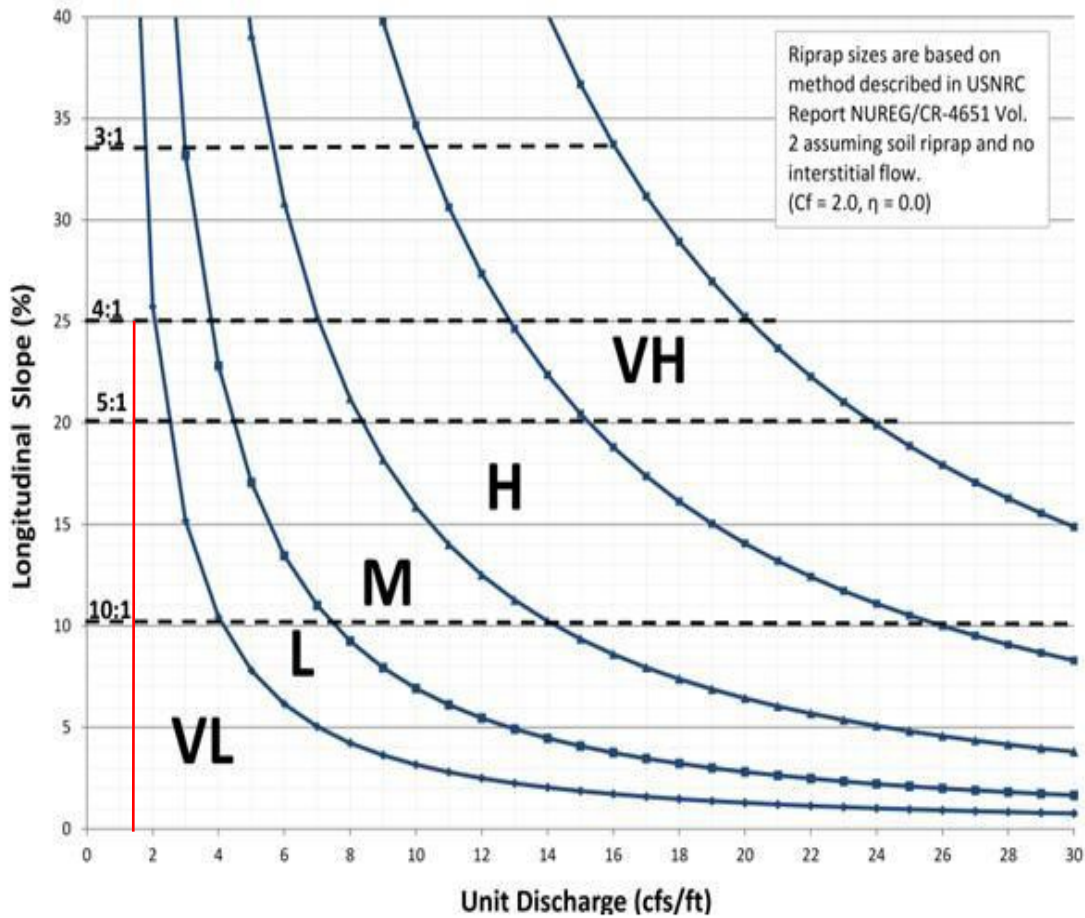


Figure 13-12d. Riprap Types for Emergency Spillway Protection



PEAK INFLOW Q100=82.4 cfs
 Spillway length=50 ft
 82.4 cfs/50 ft = 1.6 cfs/ft

Pond B - ULTIMATE DESIGN

FOREBAY VOLUME

Req'd V=3% x WQCV

West Forebay

WQCV=	0.432 ac-ft
70% of WQCV=	0.302 ac-ft
V=	0.0091 ac-ft
Actual V	0.0099 ac-ft

East Forebay

WQCV=	0.432 ac-ft
30% of WQCV=	0.130 ac-ft
V=	0.0039 ac-ft
Actual V	0.0072 ac-ft

Pond C - ULTIMATE DESIGN

FOREBAY VOLUME

Req'd V=3% x WQCV

West Forebay

WQCV=	0.521 ac-ft
80% of WQCV=	0.4168 ac-ft
V=	0.0125 ac-ft
Actual V	0.0149 ac-ft

FOREBAY RELEASE NOTCH WIDTH

$Q=CLH^{3/2}$

West Forebay

DP B7 Q ₁₀₀ =	56.0 cfs
2% of Q=	1.12 cfs
C=	2.6
H (height of forebay wall)=	1 ft
L=	4 in

East Forebay

DP B10 Q ₁₀₀ =	31.0 cfs
2% of Q=	0.62 cfs
C=	2.6
H (height of forebay wall)=	1 ft
L=	2 in
	3 in (min)

FOREBAY RELEASE NOTCH WIDTH

$Q=CLH^{3/2}$

West Forebay

Q ₁₀₀ =	86.3 cfs
2% of Q=	1.73 cfs
C=	2.6
H (height of forebay wall)=	1.5 ft
L=	2 in
	3 in (min)

TRICKLE CHANNEL FLOW DEPTH

Rectangular Channel

West Forebay

Channel Slope (ft/ft)	0.01
Depth (ft)	0.5
Bottom Width (ft)	6
2% Q ₁₀₀ (cfs)=	1.12
Flow Depth (ft)	0.11

East Forebay

Channel Slope (ft/ft)	0.01
Depth (ft)	0.5
Bottom Width (ft)	6
2% Q ₁₀₀ (cfs)=	0.62
Flow Depth (ft)	0.07

TRICKLE CHANNEL FLOW DEPTH

Rectangular Channel

West Forebay

Channel Slope (ft/ft)	0.01
Depth (ft)	0.5
Bottom Width (ft)	6
2% Q ₁₀₀ (cfs)=	1.73
Flow Depth (ft)	0.11