October 30, 2020

Sonship Properties, LLC P.O. Box 511 Rocky Ford, CO 81067

Attn: Justin Ensor

Re: OWTS – Wastewater Study

Prairie Ridge Subdivision

Brown Road

Parcel No. 61000-00-483 El Paso County, Colorado

Dear Mr. Ensor:



ENTECH ENGINEERING, INC.

505 ELKTON DRIVE COLORADO SPRINGS, CO 80907 PHONE (719) 531-5599 FAX (719) 531-5238

GENERAL SITE CONDITIONS AND PROJECT DESCRIPTION

The site is located in a portion of the SE¼ of the SE¼ of Section 12, Township 11 South, Range 66 West of the 6th Principal Meridian in El Paso County, Colorado. The site is located 8 miles east of Monument, Colorado, on Brown Road, north of Walker Road. The location of the site is as shown on the Vicinity Map, Figure 1.

The topography of the site varies from gently to moderately sloping generally to the south. The drainages on site flow in southerly and easterly directions through the property. No water was observed flowing in the minor drainage that bisects the property flowing south at the time of this investigation. Water was not observed flowing in the drainage that flows east in the southern portion of the site. Areas of ponded water were observed east of the property line in the embankment that exists east of the site. The site boundaries are indicated on the USGS Map, Figure 2. Previous land uses have included grazing and pasture land. Additionally, some fill placement has occurred in the past. The site contains primarily low grasses, and field weeds. Site photographs, taken August 26, and September 24, 2020, are included in Appendix A. The approximate locations and directions of the photographs are indicated on Figure 3.

Total acreage involved in the proposed development is approximately 40 acres. Seven (7) single-family residential lots are proposed with areas of open space. Lot sizes range from 5 acres to 6 acres. The area will be serviced by individual wells and sewage treatment systems.

SCOPE OF THE REPORT

The scope of the report will include the following:

A general geologic analysis utilizing published geologic data. Detailed site-specific mapping
will be conducted to obtain general information in respect to major geographic and geologic
features, geologic descriptions and their effects on the development of the property with
regards to on-site wastewater treatment systems (OWTS).

PREVIOUS INVESTIGATION

The site was previously investigated by Entech Engineering, Inc. in a Soil, Geology, Geologic Hazard and Wastewater Study, dated May 31, 2007 (Reference 1). Information from this report was used in evaluating the site.

FIELD INVESTIGATION

Our field investigation consisted of the preparation of a geologic map of bedrock features and significant surficial deposits. The Natural Resource Conservation Service (NRCS), previously the Soil Conservation Service (SCS) survey was also reviewed to evaluate the site. The position of mappable units within the subject property are shown on the Geologic Map. Our mapping procedures involved both field reconnaissance and measurements, and aerial photo reconnaissance and interpretation. The same mapping procedures have also been utilized to produce the Geology/Engineering Geology Map which identified pertinent geologic conditions affecting development. The field mapping was performed by personnel of Entech Engineering, Inc. on August 26, 2020.

Three test borings/percolation tests from the previous report (Reference 1) and two test pits were excavated on the site to determine general suitability for the use of on-site wastewater treatment systems and general soil characteristics for residential construction. The test pits were completed on lot 1 and 7. The locations of the test pits are indicated on the Site Plan/Test Pit Location Map, Figure 3. The Test Pit Logs are presented in Appendix B. Results of this testing will be discussed later in this report.

Laboratory testing was also performed on some of the soils to classify and determine the soils engineering characteristics. Laboratory tests included grain-size analysis, ASTM D-422, and Atterberg Limits, ASTM D-4318. Results of the laboratory testing are included in Appendix C. A Summary of Laboratory Test Results is presented in Table 1.

Test Boring Logs from the percolation tests and laboratory testing from the previous report (Reference 1) is included in Appendix D.

SOIL AND GEOLOGIC CONDITIONS

Soil Survey

The Natural Resource Conservation Service (NRCS) (Reference 2, Figure 4), previously the Soil Conservation Service (Reference 3) has mapped two soil types on the site. Complete descriptions of the soil types are presented in Appendix E. In general, they vary from sandy loam and loamy sand to clay loam. The soils are described as follows:

<u>Type</u> 15	<u>Description</u> Brussett loam, 3-5% slopes
69	Peyton-Pring complex, 8-15 % slopes

The soils have generally been described to have moderate to rapid permeabilities. Limitations on development include steep slopes, limited ability to support a load, shrink swell potential, and frost action potential. Possible hazards with soil erosion are present on the site. The erosion potential

can be controlled with vegetation. The majority of the soils have been described to have moderate to high erosion hazards (Reference 2).

Soils

The soils encountered in the Test Borings of the percolation tests can be grouped into one general soil type. Bedrock was not encountered in any of the profile holes which were drilled to 10 feet. The soils were classified using the Unified Soil Classification System (USCS).

<u>Soil Type 1</u> is a sandy clay (CL), and sandy clay – silt (CL- ML) encountered in all of the profile holes. These soils were encountered at stiff to very stiff consistencies and at moist conditions. Samples tested had approximately 69% to 79% of the soil sized particles passing the No. 200 Sieve. A swell of 1177 psf was measured in the FHA Swell Test. This swell pressure is in the moderate expansion range. The clays are also potentially collapsible. Consolidations of 0.4% and 0.6% were measured in the Swell/Consolidation Test. These values are in the low consolidation range.

The soils encountered in the test pits consisted of sandy clay. Samples tested had approximately 72% and 74% of the soil sized particles passing the No. 200 Sieve

The Test Pit Logs are presented in Appendix B, and the Laboratory test results from the test pits are presented in Appendix C. A Summary of Laboratory Test Results is presented in Table 1, the Test Boring Logs from the Profile Holes, Laboratory test results, and percolation testing results from the previous investigation are presented in Appendix D.

Groundwater

Groundwater was not encountered in any of the profile holes which were drilled to 10 feet. Groundwater was not encountered in the test pits which were excavated to 8 feet. Areas of seasonal and potentially seasonal shallow groundwater and ponded water have been mapped in the drainages on-site. These areas are discussed in the following section. Fluctuation in groundwater conditions may occur due to variations in rainfall and other factors not readily apparent at this time.

It should be noted that in the sandy materials on site, some groundwater conditions might be encountered due to the variability in the soil profile. Isolated sand and gravel layers within the soils, sometimes only a few feet in thickness and width, can carry water in the subsurface. Groundwater may also flow on top of the underlying bedrock or clays. Builders and planners should be cognizant of the potential for the occurrence of such subsurface water features during construction on-site and deal with each individual problem as necessary at the time of construction.

Geology

Approximately 11 miles west of the site is a major structural feature known as the Rampart Range Fault. This fault marks the boundary between the Great Plains Physiographic Province and the Southern Rocky Mountain Province. The site exists within a large structural feature known as the

Denver Basin. Bedrock in the area is typically gently dipping in a northeasterly direction (Reference 4). The bedrock underlying the site consists of the Dawson Formation of Cretaceous Age. The Dawson Formation typically consists of coarse-grained arkosic sandstone with interbedded layers claystone or siltstone.

The geology of the site was evaluated using the *Geologic Map of the Black Forest*, by Thorson in 2003, (Reference 5, Figure 5). The Geology Map for the site is presented in Figure 6. Four mappable units were identified on this site which is described as follows:

- Qaf Artificial Fill of Quaternary Age: These are man-made fill deposits associated with the filled gullies in the central portion of the site.
- **Recent Alluvium of Quaternary Age:** These are recent stream deposits in the channels of the main drainages on site. Some areas have recent sand deposition, while others have highly organic soils.
- Alluvium of Palmer Divide of Early Pleistocene or Pliocene Age: These materials consist of water-deposited stream terrace deposits located along the Palmer Divide. They typically consist of silty to clayey sands with gravelly lenses and may contain areas of pebble and cobble lenses.
- Qc/Tkd Colluvium of Quaternary Age overlying Dawson Formation of Tertiary to Cretaceous Age: The materials consist of colluvial or residual soils overlying the bedrock materials on-site. The colluvial soils were deposited by the action of sheetwash and gravity. The residual soils were derived from the in-situ weathering of the bedrock on site. These materials typically consist of silty to clayey sand with areas of sandy clays. The bedrock consists of the Dawson Formation. The Dawson Formation typically consists of coarse-grained, arkosic sandstone with interbedded lenses of fine-grained sandstone, siltstone and claystone. The soil layer encountered on site was more than 10 feet thick and consisted of sandy, silty clays.

The soils listed above were mapped from site-specific mapping, the *Geologic Map of the Black Forest Quadrangle* distributed by the Colorado Geologic Survey in 2003 (Reference 5, Figure 5), The *Geologic Map of the Colorado Springs-Castle Rock Area*, distributed by the US Geological Survey in 1979 (Reference 6), and the *Geologic Map of the Pueblo 1° x 2° Quadrangle*, distributed by the US Geological Survey in 1978 (Reference 7). The test borings and test pits were used in evaluating the site and is included in Appendices B and D. The Geology Map prepared for the site is presented in Figure 6.

Drainage Areas

A drainage exists in the southern portion of the site and a minor drainage in the central portion of the site that flow in easterly direction. No water was observed flowing in the drainages at the time of the investigation. These drainages have been mapped as potentially seasonal shallow and seasonal shallow groundwater areas (Figures 6 & 8).

In potentially seasonal shallow groundwater and seasonally shallow groundwater areas, we would anticipate the potential for periodically high subsurface moisture conditions and highly organic soils. Due to the potential for seasonal high groundwater conditions, on-site wastewater treatment systems are not recommended in these areas. Due to lot sizes, it is anticipated these areas can be avoided. The site does not lie within any floodplain zones according to the FEMA Map No. 08041CO305G dated December 7, 2018 (Figure 7, Reference 8). Exact locations of floodplain and specific drainage studies are beyond the scope of this report. Individual wastewater treatment systems must be located a minimum of 25 feet from dry guiches and 50 feet from water courses or floodplains.

ON-SITE WASTEWATER TREATMENT

The Natural Resource Conservation Service (Reference 2), previously the Soil Conservation Service (Reference 3) has been mapped with two soil descriptions. The Soil Survey Map (Reference 2) is presented in Figure 4, and the Soil Survey Descriptions (Reference 2) are presented in Appendix D. The soils are described as having moderate to rapid percolation rates.

Soils encountered in the tactile test pits consisted sandy clay. The limiting layers encountered in the test pits is the sandy clay, which corresponds with USDA Soil Type 4A with an LTAR values of 0.20 gallons per day per square foot. Bedrock was not encountered in the test pits, and signs of seasonally occurring groundwater were not observed in the test pits. Absorption fields must be maintained a minimum of 4 feet above groundwater or bedrock, or confining layer. Should groundwater or bedrock be encountered within 6 feet of the surface, designed systems will be required. Designed systems are for all of the lots due to the highly clayey soils, however, areas may be encountered on the lots where conventional systems would be suitable.

In summary, it is our opinion the site is suitable for individual on-site wastewater treatment systems (OWTS) and that contamination of surface and subsurface water resources should not occur provided the OWTS sites are evaluated and installed according to El Paso County and State Guidelines and properly maintained. Based on the testing performed designed systems will be required for the new lot. The Septic Suitability Map is presented in Figure 8. Areas that should be avoided by septic systems are indicated on the septic suitability map. A possible house location, water well, and two septic sites for the new lots are indicated on Figure 8. Individual soil testing is required for proposed construction on each lot prior to construction. Absorption fields must be located a minimum of 100 feet from any well, including those on adjacent properties. Absorption fields must also be located a minimum of 50 feet from any drainages, floodplains or ponded areas and 25 feet from dry gulches.

CLOSURE

This report has been prepared for Sonship Properties, LLC, for application to the proposed project in accordance with generally accepted geologic soil and engineering practices. No other warranty expressed or implied is made.

We trust that this report has provided you with all the information that you required. Should you require additional information, please do not hesitate to contact Entech Engineering, Inc.

Respectfully Submitted,

ENTECH ENGINEERING, INC.

Reviewed by:

resident

Logan L. Langford, P.G.

Geologist

LLL/ao

Encl.

Entech Job No. 201794 AAprojects/2020/201794 wws

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- 1. Entech Engineering, Inc. dated May 31, 2007. Soil, Geology, Geologic Hazard, and Wastewater Study, Prairie Ridge, El Paso County, Colorado. Entech Job No. 94477.
- 2. Natural Resource Conservation *Service*, September 13, 2019. *Web Soil Survey*. United States Department Agriculture, http://websoilsurvey.sc.egov.usda.gov/App/HomePage.htm.
- 3. United States Department of Agriculture Soil Conservation Service. June 1981. Soil Survey of El Paso County Area, Colorado.
- Scott, Glen R.; Taylor Richard B.; Epis, Rudy C; and Wobus, Reinhard A. 1978. Geologic Map of the Pueblo 1° x 2° Quadrangle, South-Central Colorado. Sheet 2. U.S. Geologic Survey. Map I-1022, Sheet 2.
- 5. Thorson, Jon P., 2003. *Geologic Map of the Black Forest Quadrangle, El Paso County, Colorado*. Colorado Geological Survey. Open-File Report 03-6.
- 6. Trimble, Donald E. and Machette, Michael N. 1979. Geologic Map of the Colorado Springs-Castle Rock Area, Front Range Urban Corridor, Colorado. USGS, Map I-857-F.
- 7. Scott, Glen R.; Taylor Richard B.; Epis, Rudy C; and Wobus, Reinhard A. 1978. *Geologic Structure Map of the Pueblo 1° x 2° Quadrangle, South-Central Colorado*. Sheet 2. U.S. Geologic Survey. Map I-1022.
- 8. Federal Emergency Management Agency. December 7, 2018. Flood Insurance Rate Maps for the City of Colorado Springs, Colorado. Map Number 08041CO305G

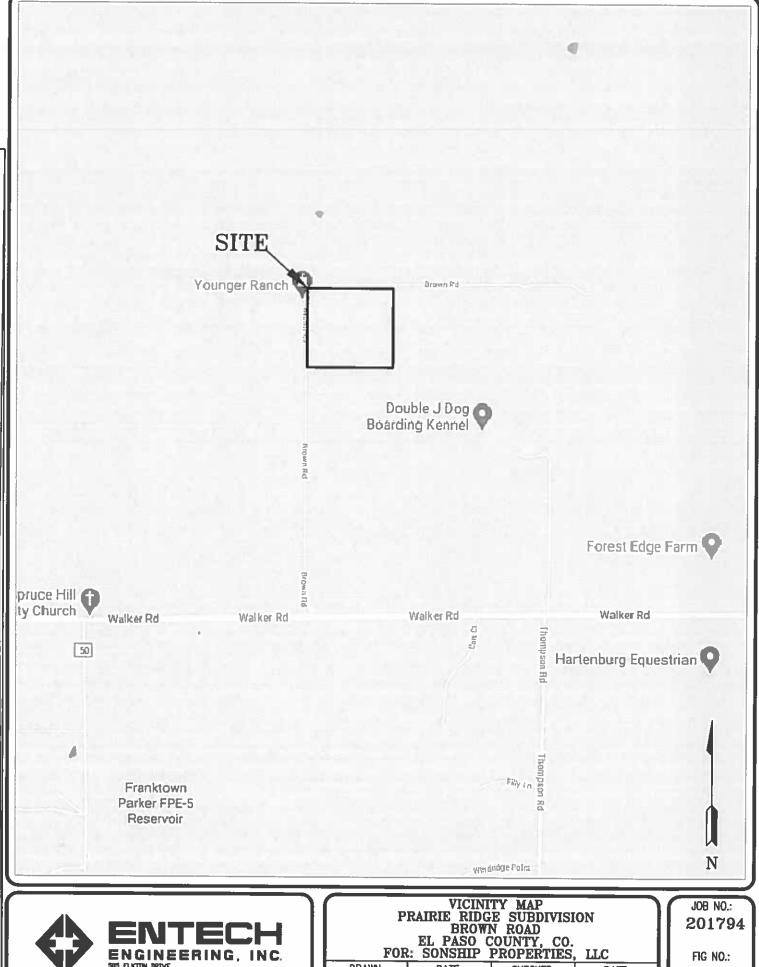
TABLES

Table 2: Summary Tactile Test Pit Results

Test	USDA Soil	LTAR	Depth	Depth to
Pit	Туре	Value	to	Seasonally
No.			Bedrock (ft.)	Occurring
				Groundwater (ft.)
1	4A*	0.20*	N/A	N/A
2	4A*	0.20*	N/A	N/A

^{*-} Conditions that will require an engineered OWTS

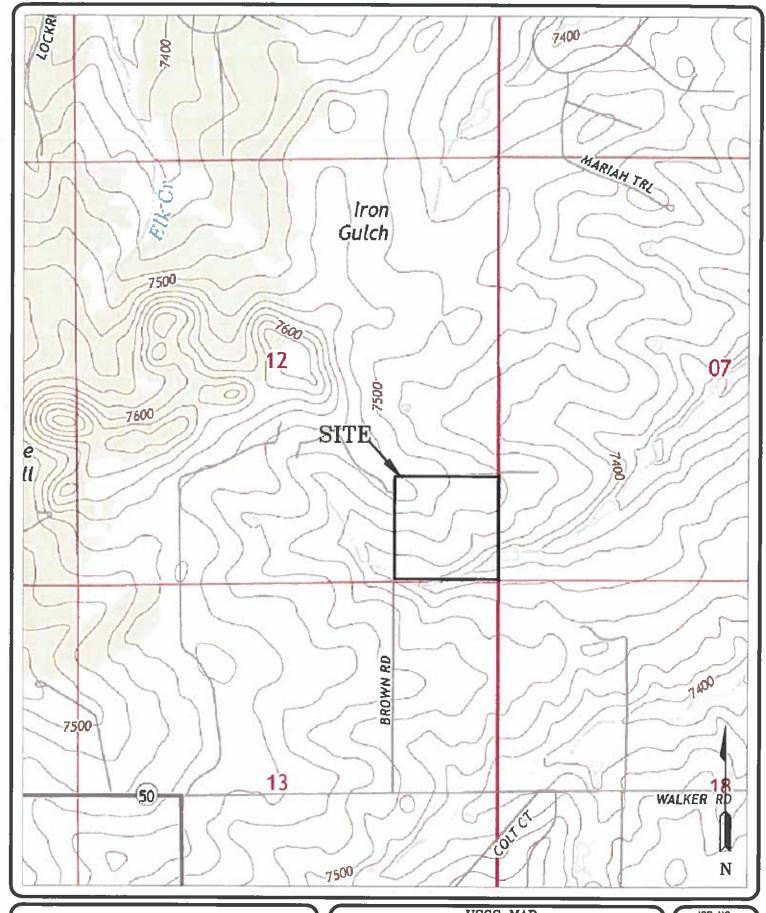
FIGURES





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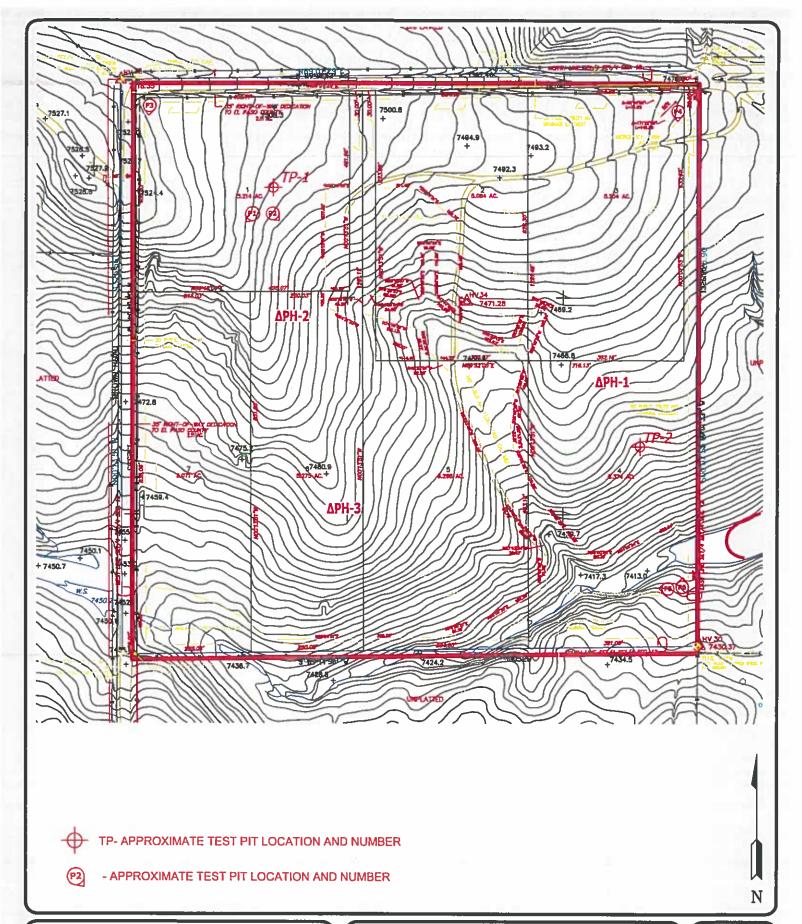
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DRAWN: LLL	DATE: 10/9/20	CHECKED:	DATE:			

JOB NO.: 201794





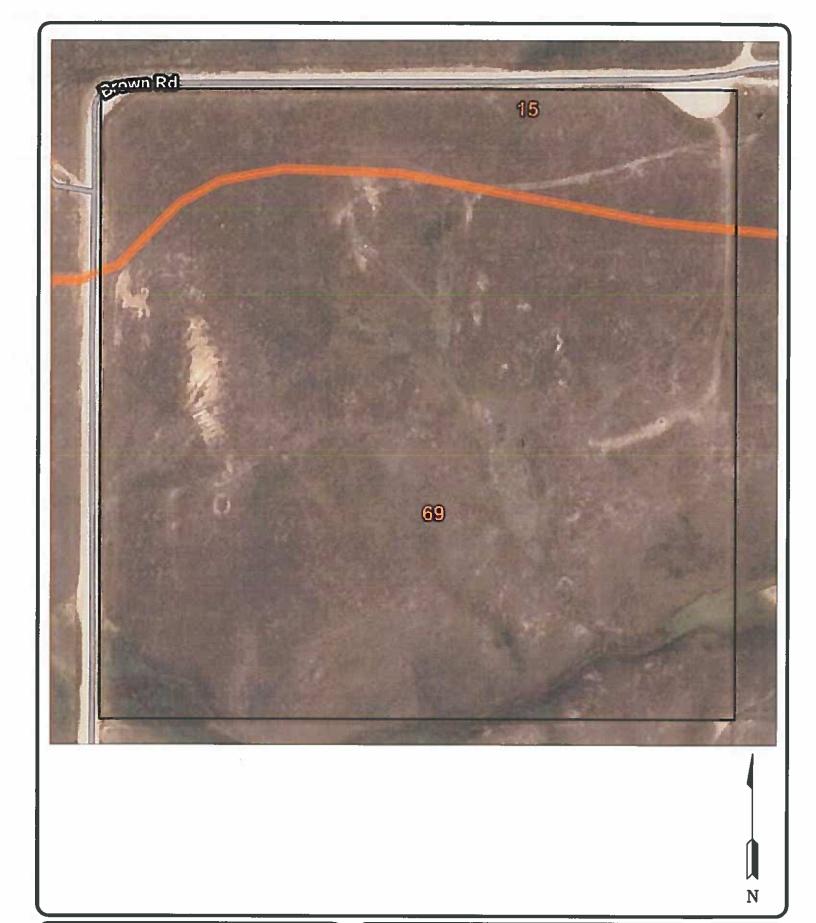
SITE PLAN/TESTING LOCATION MAP PRAIRIE RIDGE SUBDIVISION BROWN ROAD EL PASO COUNTY, CO. FOR: SONSHIP PROPERTIES, LLC

FOR: SONSHIP PROPERTIES, LLC

DRAWN: DATE: CHECKED: DATE:

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JOB NO.: 201794



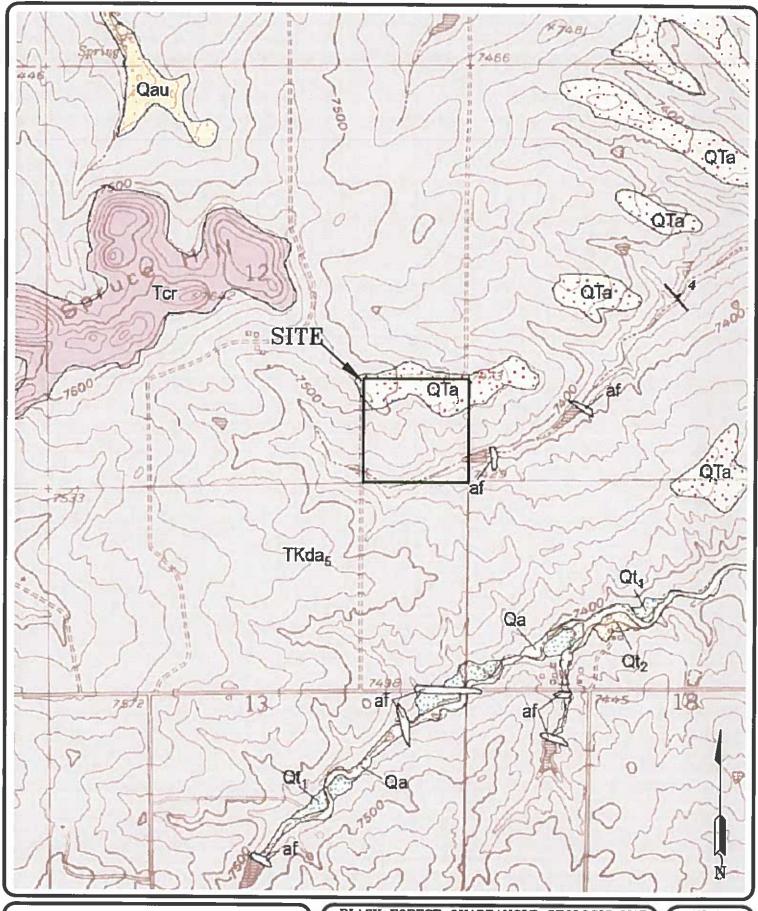


SOIL SURVEY MAP
PRAIRIE RIDGE SUBDIVISION
BROWN ROAD
EL PASO COUNTY, CO.
FOR: SONSHIP PROPERTIES, LLC

FOR: SONSHIP PROPERTIES, LLC

DRAWN: DATE: CHECKED: DATE:
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JOB NO.: 201794



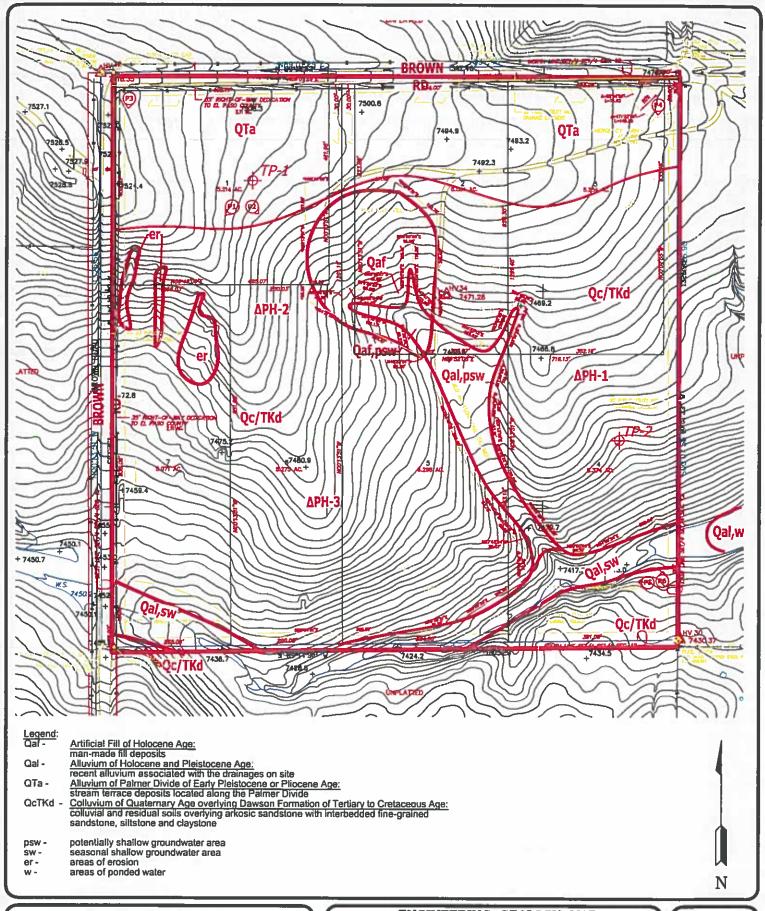
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BLACK FOREST QUADRANGLE GEOLOGIC MAP PRAIRIE RIDGE SUBDIVISION BROWN ROAD EL PASO COUNTY, CO. FOR: SONSHIP PROPERTIES, LLC

DRAWN: DATE: 10/9/20 CHECKED: DATE:

JOB NO.: 201794





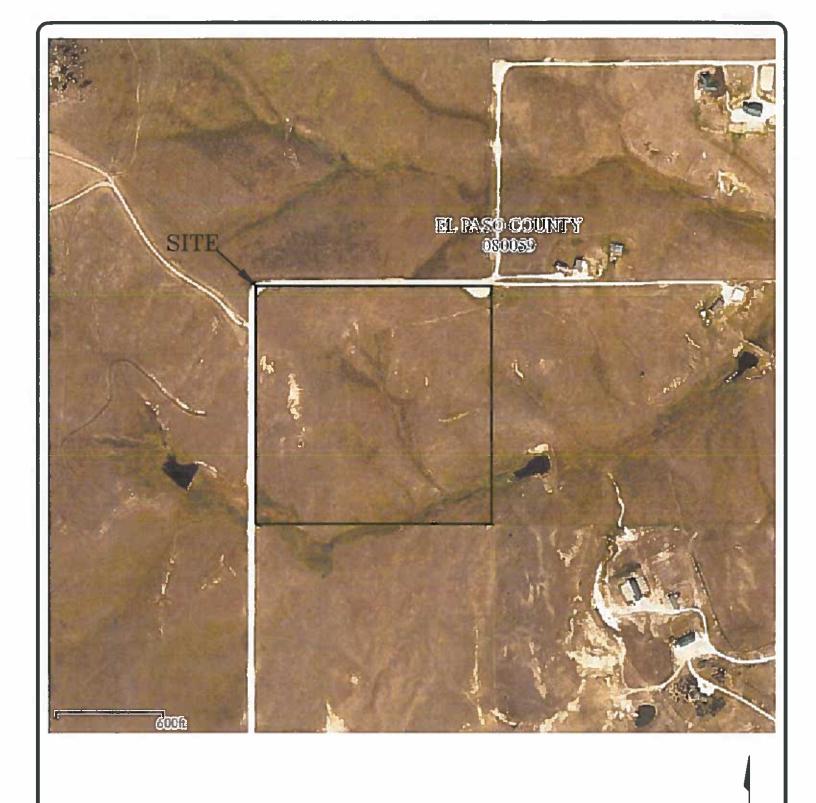
ENGINEERING GEOLOGY MAP
PRAIRIE RIDGE SUBDIVISION
BROWN ROAD
EL PASO COUNTY, CO.
FOR: SONSHIP PROPERTIES, LLC

DRAWN: DATE: CHECKED: DATE:

10/8/20

LLL

JOB NO.: 201794

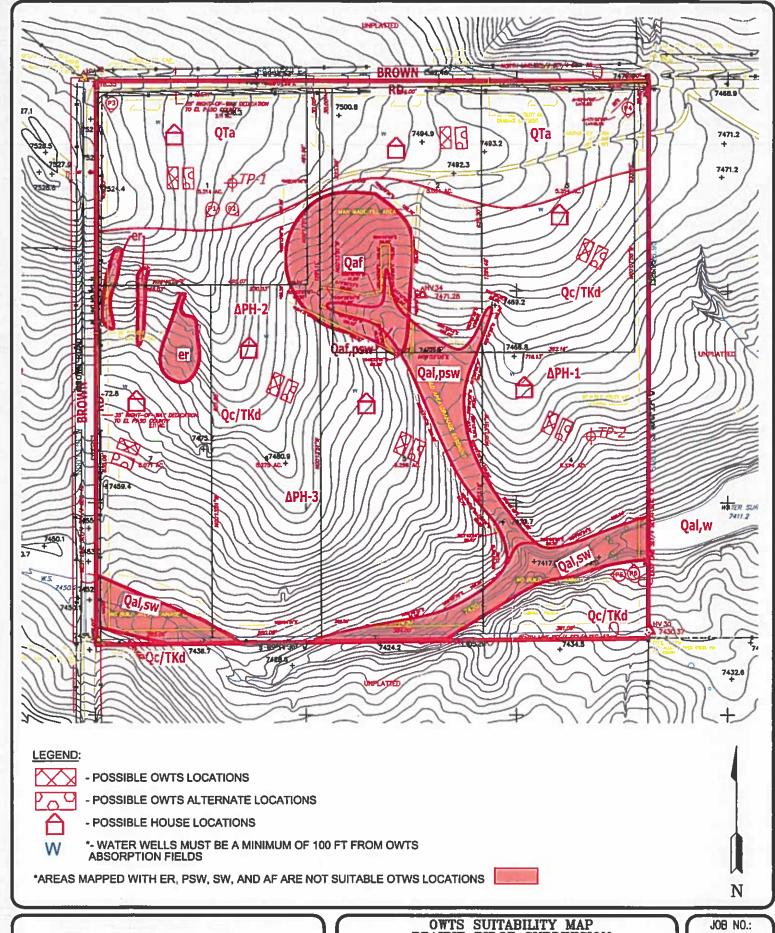




FEMA FLOODPLAIN MAP
PRAIRIE RIDGE SUBDIVISION
BROWN ROAD
EL PASO COUNTY, CO.
FOR: SONSHIP PROPERTIES, LLC

DRAWN: DATE: CHECKED: DATE:
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JOB NO.: 201794





OWTS SUITABILITY MAP
PRAIRIE RIDGE SUBDIVISION
BROWN ROAD
EL PASO COUNTY, CO.
FOR: SONSHIP PROPERTIES, LLC

DRAWN: DATE: CHECKED: DATE:

JOB NO.: 201794

APPENDIX A: Photographs





Looking southwest from the northwest portion of the site.

August 26, 2020





Looking southeast from the northwest portion of the site.

August 26, 2020

Job No. 201794





Looking south from the northwestern corner of the site.

August 26, 2020

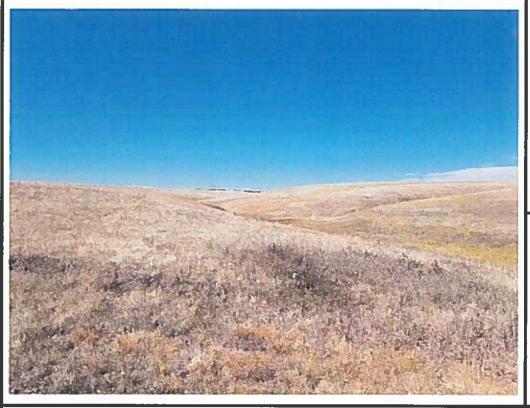




Looking south from the northeastern portion of the site.

August 26, 2020

Job No. 201794





Looking west along the drainage in the southern portion of the site.

September 24, 2020





Looking north from the southeastern corner of site.

September 24, 2020

Job No. 200160

APPENDIX B: Test Pit Logs

TEST PIT NO. 1
DATE EXCAVATED 8/26/2020
Job # 201794

TEST PIT NO. 2 DATE EXCAVATED 8/26/2020

CLIENT SONSHIP PROPERTIES, LLC BROWN ROAD

							CATION BROWN ROAD	,	
REMARKS	Depth (ft)	Symbol	Samples	Soil Structure Shape	Soil Structure Grade	USDA Soil Type	MARKS	Soil Structure Shape Soil Structure Grade	USDA Soil Type
topsoil sandy clay, dark brown sandy clay, brown	1 2 3 4 5 6 7 8 9 10			ma	s		soil sandy clay, dark brown	na s	4A

Soil Structure Shape granular - gr platy - pl blocky - bl prismatic - pr single grain - sg massive - ma

Soil Structure Grade

weak - w moderate - m strong - s loose - l



	TEST	PIT LOG	
DRAWN:	DATE	CHECKED:	DATE: (0/9/73

JOB NO.: 2-1794 FIG NO.: B-1 **APPENDIX C: Laboratory Test Results**

BORING NO. TP-1

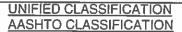
DEPTH(ft)

3

SONSHIP PROPERTIES

CLIENT PROJECT

BROWN ROAD



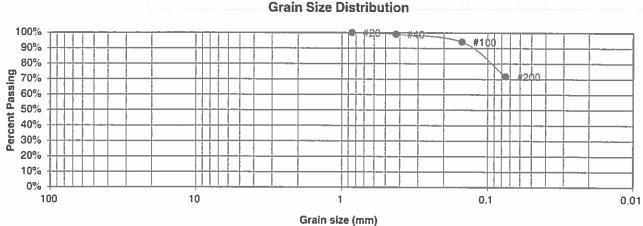
CL

TEST BY JOB NO.

BL

201794





U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
4 10		<u>Swell</u> Moisture at start
20	100.0%	Moisture at finish
40	99.0%	Moisture increase
100	94.1%	Initial dry density (pcf)
200	71.7%	Swell (psf)



LABORATORY	TEST
RESULTS	

DRAWN:

DATE

CHECKED:

DATE: 1/24/10 JOB NO. 201794

BORING NO. TP-1

DEPTH(ft)

6

SONSHIP PROPERTIES

CLIENT PROJECT

BROWN ROAD

UNIFIED CLASSIFICATION **AASHTO CLASSIFICATION**

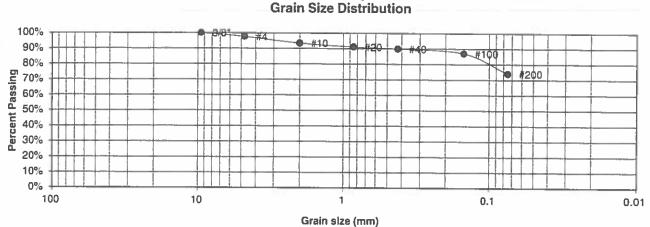
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201794





4 97.6% <u>Swell</u> 10 93.4% Moisture at start 20 91.0% Moisture at finish 40 89.9% Moisture increase	U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent Finer	Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index
2091.0%Moisture at finish4089.9%Moisture increase10087.1%Initial dry density (p	4		
midal dry density (p			Moisture at finish
			Initial dry density (pcf) Swell (psf)



	LABORAT RESULTS	ORY TEST	
DRAWN:	DATE:	CHECKED:	DATE:

JOB NO. 201794

> FIG NO C.Z

APPENDIX D: Test Boring Logs, Laboratory Test Results, & Percolation Testing Results Entech Job No. 94477

TABLE 1

SUMMARY OF LABORATORY TEST RESULTS

CLIENT PRAIRIE RIDGE PROP.
PROJECT PRAIRIE RIDGE
JOB NO. 94477

	ī		
SOIL DESCRIPTION	CLAY-SILT SANDY	CLAY SANDY	CI AV-SII T SANDY
UNIFIED	CL-ML	ರ	CL-ML
SWELL/ CONSOL (%)		-0.6	-0.4
FHA SWELL (PSF)	1177		
SULFATE (WT %)			
PLASTIC INDEX (%)		Ξ	9
LIQUID LIMIT (%)		28	24
PASSING NO. 200 SIEVE (%)	71.6	78.9	69.2
DRY DENSITY (PCF)		93.1	94.1
WATER (%)		9.0	9.9
ОЕРТН (FT)	5-10	10	5
TEST BORING NO.	-	2	3
SOIL	-	-	1

Table 2: Summary of Percolation Test Results

Percolation Test No.	Percolation Rate (min/in)	Depth to Bedrock (ft.)	Depth to Groundwater (ft.)
1	320	>10	>10
2	267	>10	>10
3	160	>10	>10

PROFILE HOLE NO PROFILE HOLE NO DATE DRILLED 5/14/2007 DATE DRILLED 5/14/2007 Job# 94477 CLIENT PRAIRIE RIDGE PROP. LOCATION PRAIRIE RIDGE REMARKS REMARKS Watercontent % Blows per foot Blows per foot Watercontent (Soil Type Samples Soil Type Depth (ft) Samples Symbol Symbol DRY TO 10', 5/15/07 DRY TO 10', 5/15/07 CLAY-5ILT, SANDY, BROWN TO CLAY, SANDY, BROWN TO TAN, TAN, STIFF TO VERY STIFF, STIFF, MOIST MOIST 18 8.7 1 19 6.8 1 10 30 8.4 1 10 16 8.9 1 15



	PRO	FILE HOLE LO	G
DRAWN:	DATE	CHECKED:	5/2/67

10B NO

PROFILE HOLE NO 3 PROFILE HOLE NO. DATE DRILLED 5/14/2007 DATE DRILLED Job# 94477 CLIENT PRAIRIE RIDGE PROP. LOCATION PRAIRIE RIDGE REMARKS REMARKS Watercontent % Watercontent % Blows per foot Blows per foot Depth (ft) Soil Type Soil Type Samples Samples Depth (ft) Symbol Symbol DRY TO 10', 5/15/07 CLAY-SILT, SANDY, BROWN TO TAN, STIFF, MOIST 18 6.5 1 10 18 7.3 1 15 20

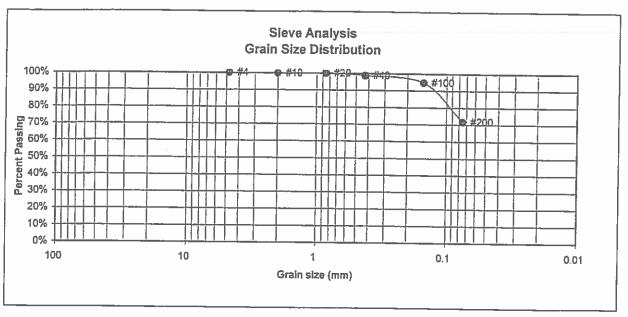
	ENTECH ENGINEERING, INC.
	505 ELKTON DRIVE COLORADO SPRINGS, COLORADO 80907

	PROF	ILE HOLE LO	G
DRAWN;	DATE:	CHECKED:	DATE:

JOB NO.:

APPENDIX C: Laboratory Test Results

UNIFIED CLASSIFICA	ATION CL-MIL	CLIENT	PRAIRIE RIDGE PROP.	
SOIL TYPE#	I	PROJECT	PRAIRIE RIDGE	
TEST BORING #	1	JOB NO.	94477	
DEPTH (FT)	5-10	TEST BY	DG	



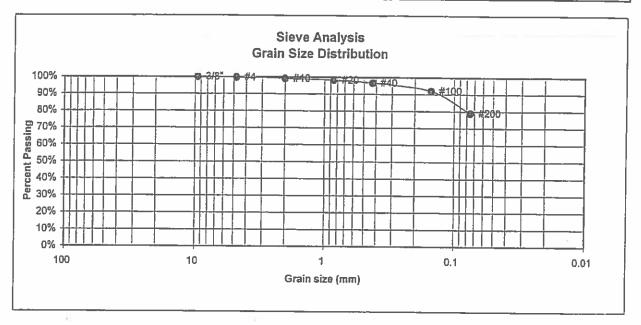
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>		Atterberg <u>Limits</u> Plastic Limit Liquid Limit Plastic Index	
3/8"				
4	100.0%		<u>S</u> well	
10	99.8%	Ì	Moisture at start	10.0%
20	99.9%	1	Moisture at finish	22.1%
40	98.7%		Moisture increase	12.1%
100	94.7%	[Initial dry density (pcf)	100
200	71.6%		Swell (psf)	1177



LABORATORY TEST RESULTS			
DRAWN:	DATE:	CHECKED:	5/21/07

ON BOL

UNIFIED CLASSIFIC	CATIONCL	CLIENT PRAIRIE RIDGE PROP.
SOIL TYPE#	I	PROJECT PRAIRIE RIDGE
TEST BORING #	2	JOB NO. 94477
DEPTH (FT)	10	TEST BY DG



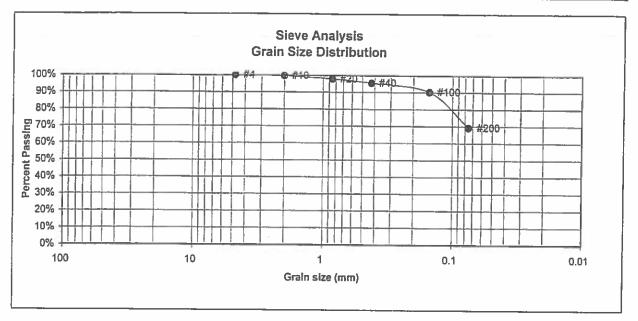
U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit 17 Liquid Limit 28 Plastic Index 11
3/8"	100.0%	
4	99.7%	Swell
10	99.0%	Moisture at start
20	98.1%	Moisture at finish
40	96.6%	Moisture increase
100 200	91.9% 78.9%	Initial dry density (pcf) Swell (psf)



LABORATORY TEST RESULTS			
DRAWN:	DATE:	CHECKED:	5/21/07

108 NO:

UNIFIED CLASSIFICA	ATION CL-ML	CLIENT	PRAIRIE RIDGE PROP.
SOIL TYPE #	1	PROJECT	PRAIRIE RIDGE
TEST BORING #	3	JOB NO.	94477
DEPTH (FT)	5	TEST BY	DG



U.S. <u>Sieve #</u> 3" 1 1/2" 3/4" 1/2" 3/8"	Percent <u>Finer</u>	Atterberg Limits Plastic Limit 18 Liquid Limit 24 Plastic Index 6
4	100.0%	<u>Swell</u>
10	99.6%	Moisture at start
20	97.8%	Moisture at finish
40	95.4%	Moisture increase
100	90.2%	Initial dry density (pcf)
200	69.2%	Swell (psf)



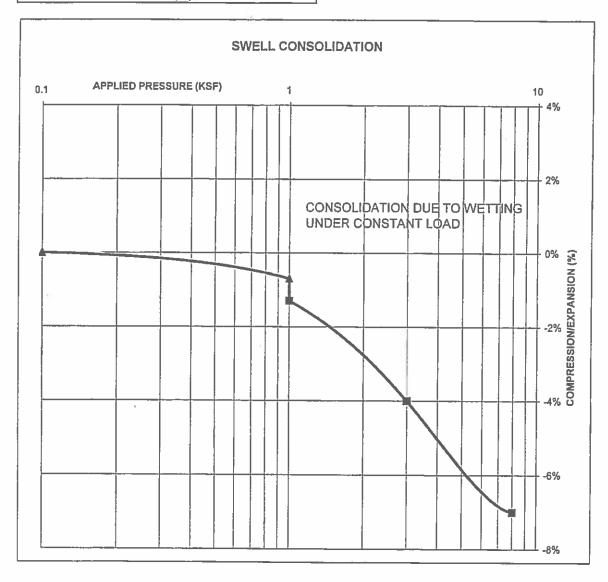
LABORATORY TEST RESULTS			
DRAWN:	DATE:	CHECKED:	5/2/07

JOB NO.:

CONSOLIDATION TEST RESULTS

TEST BORING # 2 DEPTH(ft) 10
DESCRIPTION CL SOIL TYPE 1
NATURAL UNIT DRY WEIGHT (PCF) 93
NATURAL MOISTURE CONTENT 9.0%
SWELL/CONSOLIDATION (%) -0.6%

JOB NO. 94477
CLIENT PRAIRIE RIDGE PROP.
PROJECT PRAIRIE RIDGE





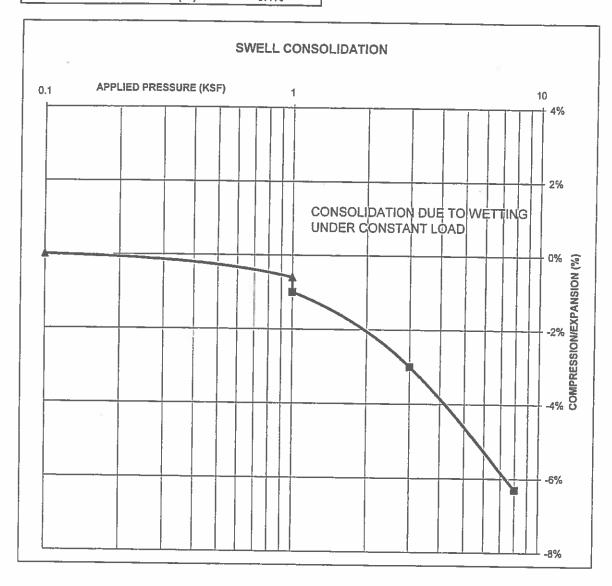
	SWELL CONSOLIDATION TEST RESULTS				
DRAWN:	DATE:	CHECKED:	5/2/107		

JOB NO.:

CONSOLIDATION TEST RESULTS

TEST BORING # 3 DEPTH(ft) 5
DESCRIPTION CL-MI SOIL TYPE 1
NATURAL UNIT DRY WEIGHT (PCF) 94
NATURAL MOISTURE CONTENT 6.6%
SWELL/CONSOLIDATION (%) -0.4%

JOB NO. 94477
CLIENT PRAIRIE RIDGE PROP.
PROJECT PRAIRIE RIDGE





		L CONSOLIDA [*] RESULTS	TION	
DRAWN:	DATE	CHECKED:	SZZIO7	

JOB NO.:

Client:

PRAIRIE RIDGE PROP.

Test Location:

PRAIRIE RIDGE

Job Number: 94477

PERCOLATION HOLES-TEST NO. 1

Date Holes Prepared:

5/14/2007

Date Hole Completed:

5/15/2007

Hole No. 1

Depth: 32" Hole No. 2 Depth: 33"

Hole No. 3

Depth: 34"

-			_ A			Dopus.	→ ∓	
		Water			Water	•		Water
	Time	Level		Time	Level		Time	Level
Trial	(min.)	Change (in.)	<u>Trial</u>	(min.)	Change (in.)	<u>Trial</u>	(min.)	Change (in.)
1	10	1/16	1:	10	1/16	1	10	1/8
2	10	0	2	10	0	2	10	0
3	10	1/16	3	10	0	3	10	0

Perc Rate (min./in.): 240

Perc Rate (min./in.): 480 Perc Rate (min./in.): 240

Average Perc Rate (min./in.)

320

PROFILE HOLE

Date Profile Hole Completed:

5/14/2007

Depth 0-10"

Visual Classification

Clay-silt, sandy, dark brown to tan

Remarks

No Bedrock No Groundwater

18 Blows / ft. @ 4'

30 Blows / ft. @ 9'

Required Area of Absorption Field: N/A* Sq. Ft./gpd sewage volume

Required Area of Absorption Field: N/A* Sq. Ft./bedroom

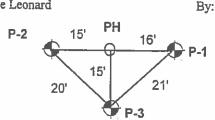
Required Area of Absorption Field: N/A* Sq. Ft./bedroom with garbage disposal and washing machine

Remarks:

* - Due to slow percolation rate, a designed system or additional drilling is recommended

Observer: Blake Leonard









DRAWN: 5/21107 JOB NO.:

Client:

PRAIRIE RIDGE PROP.

Test Location:

PRAIRIE RIDGE

Job Number: 94477

PERCOLATION HOLES-TEST NO. 2

Date Holes Prepared:

5/14/2007

Date Hole Completed:

5/15/2007

Hole No. 1

Depth: 41" Hole No. 2

Depth: 38"

Hole No. 3

Depth: 37"

		Water	_		77.5	•		
		Water			Water			Water
	Time	Level		Time	Level		Time	Level
Trial	(min.)	Change (in.)	<u>Trial</u>	(min.)	Change (in.)	<u>Trial</u>	(min.)	Change (in.)
1	10	1/16	1	10	1/8	1	10	0
2	10	0	2	10	1/4	2	10	1/8
3	10	0	3	10	0	3	10	0

Perc Rate (min./in.): 480

Perc Rate (min./in.): 80

Perc Rate (min./in.): 240

Average Perc Rate (min./in.)

267

PROFILE HOLE

Date Profile Hole Completed:

5/14/2007

Depth 0-101

Visual Classification

Clay, sandy, brown to light brown

Remarks

No Bedrock No Groundwater

19 Blows / ft. @ 4'

16 Blows / ft. @ 9'

Required Area of Absorption Field: N/A* Sq. Ft/gpd sewage volume

DRAWN:

Required Area of Absorption Field: N/A* Sq. Ft./bedroom

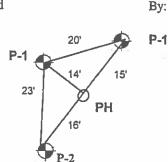
Required Area of Absorption Field: N/A* Sq. Ft./bedroom with garbage disposal and washing machine

Remarks:

* - Due to slow percolation rate, a designed system or additional drilling is recommended

Observer: Blake Leonard

N N.T.S.





PERCOLA	TION TEST R	ESULTS
DATE:	CHECKED:	A DATE:

JOB NO.:

Client:

PRAIRIE RIDGE PROP.

Test Location:

PRAIRIE RIDGE

Job Number: 94477

PERCOLATION HOLES-TEST NO. 3

Date Holes Prepared:

5/14/2007

Date Hole Completed:

5/15/2007

Hole No. 1 Depth: 32"

Hole No. 2 Depth: 34"

Hole No. 3

Depth: 38"

-				F			Depus.	J G	
			Water			Water			Water
		Time	Level		Time	Level		Time	Level
1	<u>[rial</u>	<u>(min.)</u>	Change (in.)	<u>Trial</u>	(min.)	Change (in.)	<u>Trial</u>	(min.)	Change (in.)
	1	10	1/8	1	10	1/16	1	10	0
	2	10	0	2	10	1/16	2	10	1/8
	3	10	1/8	3	10	0	3	10	1/8

Perc Rate (min./in.): 120 Perc Rate (min./in.): 240 Perc Rate (min./in.): 120

Average Perc Rate (min./in.)

160

PROFILE HOLE

Date Profile Hole Completed:

5/14/2007

Depth

Visual Classification

Remarks

0-21

Sand, clayey, dark brown

2-10"

Clay-Silt, sandy, brown to light brown

No Bedrock No Groundwater

18 Blows / ft. @ 4'

18 Blows / ft. @ 9'

Required Area of Absorption Field: N/A* Sq. Ft./gpd sewage volume

Required Area of Absorption Field: N/A* Sq. Ft./bedroom

Required Area of Absorption Field: N/A* Sq. Ft./bedroom with garbage disposal and washing machine

Remarks:

* - Due to slow percolation rate, a designed system or additional drilling is recommended

By:

P-2

DRAWN:

Observer: Blake Leonard

P-1 PH 23' N.T.S. 231



PERCOLA	TION TEST R	ESULTS
DATE	OUEDICED.	

JOB NO.:

APPENDIX E: Soil Survey Descriptions

El Paso County Area, Colorado

15—Brussett loam, 3 to 5 percent slopes

Map Unit Setting

National map unit symbol: 367k Elevation: 7,200 to 7,500 feet Frost-free period: 115 to 125 days

Farmland classification: Prime farmland if irrigated

Map Unit Composition

Brussett and similar soils: 85 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Brussett

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear Across-slope shape: Linear Parent material: Eolian deposits

Typical profile

A - 0 to 8 inches: loam BA - 8 to 12 inches: loam Bt - 12 to 26 inches: clay loam Bk - 26 to 60 inches: silt loam

Properties and qualities

Slope: 3 to 5 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Calcium carbonate, maximum content: 5 percent

Maximum salinity: Nonsaline to very slightly saline (0.0 to 2.0

mmhos/cm)

Available water capacity: High (about 9.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 4e

Hydrologic Soil Group: B Ecological site: R048AY222CO

Hydric soil rating: No



Minor Components

Other soils

Percent of map unit:

Hydric soil rating: No

Data Source Information

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 18, Jun 5, 2020

El Paso County Area, Colorado

69—Peyton-Pring complex, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 369g Elevation: 6,800 to 7,600 feet

Farmland classification: Not prime farmland

Map Unit Composition

Peyton and similar soils: 40 percent Pring and similar soils: 30 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Peyton

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Arkosic alluvium derived from sedimentary rock and/or arkosic residuum weathered from sedimentary rock

Typical profile

A - 0 to 12 inches: sandy loam

Bt - 12 to 25 inches: sandy clay loam

BC - 25 to 35 inches: sandy clay loam

C - 35 to 60 inches: sandy loam

Properties and qualities

Slope: 8 to 9 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained Runoff class: Medium

Capacity of the most limiting layer to transmit water (Ksat): Moderately high (0.20 to 0.60 in/hr)
Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water capacity: Moderate (about 7.3 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonimigated): 4e

Hydrologic Soil Group: B

Ecological site: R049XB216CO - Sandy Divide

Hydric soil rating: No

Description of Pring

Setting

Landform: Hills

Landform position (three-dimensional): Side slope

Down-slope shape: Linear Across-slope shape: Linear

Parent material: Arkosic alluvium derived from sedimentary rock

Typical profile

A - 0 to 14 inches: coarse sandy loam
C - 14 to 60 inches: gravelly sandy loam

Properties and qualities

Slope: 8 to 15 percent

Depth to restrictive feature: More than 80 inches

Drainage class: Well drained

Runoff class: Low

Capacity of the most limiting layer to transmit water (Ksat): High

(2.00 to 6.00 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water capacity: Low (about 6.0 inches)

Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 6e

Hydrologic Soil Group: B

Ecological site: R049XB222CO - Loamy Park

Hydric soil rating: No

Minor Components

Pleasant

Percent of map unit: Landform: Depressions Hydric soil rating: Yes

Other soils

Percent of map unit: Hydric soil rating: No

Data Source Information

Soil Survey Area: El Paso County Area, Colorado Survey Area Data: Version 18, Jun 5, 2020